

Foundation Data						
Type	Design Data	Bent Number				
		1	2	3	4	
Load Bearing Pile	Pile Type and Size	HP12x53	--	--	HP12x53	
	Number ea	2	--	--	2	
	Approximate Length Per Each ft	30	--	--	16	
	Pile Point Reinforcement ea	2	--	--	2	
	Min. Galvanized Penetration (Elev.) ft	Full Length	--	--	Full Length	
	Pile Driving Verification Method	WEAP	--	--	WEAP	
	Resistance Factor	0.5	--	--	0.5	
Rock Socket	Minimum Nominal Axial Compressive Resistance kip	396	--	--	396	
	Number ea	--	1	1	--	
	Layer 1	Foundation Material	--	Limestone	Limestone	--
		Elevation Range ft	--	571.5-564.5	568.4-561.4	--
	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf	--	22.4	22.4	--	
	Minimum Nominal Axial Compressive Resistance (Tip Resistance) ksf	--	400	400	--	

WEAP = Wave Equation Analysis of Piles

Load Bearing Pile:

$$\text{Minimum Nominal Axial Compressive Resistance} = \frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$$

Rock Socket (Drilled Shafts):

$$\text{Minimum Nominal Axial Compressive Resistance (Side Resistance + Tip Resistance)} = \frac{\text{Maximum Factored Loads}}{\text{Resistance Factors}}$$

Foundation inspection Hole required at Int. Bent No. 2 only.

Driven Piles:

All piles shall be galvanized full length.

Manufactured pile point reinforcement shall be used on all piles in this structure.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Prebore for piles at End Bents No. 1 and No. 4 to elevations 575.40 and 578.00, respectively.

Roadway fill shall be completed to the final roadway section and up to the elevation of the bottom of the concrete beam within the limits of the structure and for not less than 25 feet in the back of the fill face of the end bents before any piles are driven for any bents falling within the embankment section.

Prebore holes shall be backfilled with sand (or other approved material), in accordance with Sec 702, before piles are driven to bearing.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

General Notes:

Design Specifications:

2002 AASHTO LFD Standard Specification for Highway Bridges (17th Ed.) (Superstructure)
 2020 AASHTO LRFD Bridge Design Specifications (9th Ed.) (Substructure)
 Seismic Performance Category A

Design Loading:

Vehicular = HS20 Modified & Military 24,000 lb Tandem Axle (1983) (New and Existing Girders)
 Vehicular = HL-93 (Substructure)
 Future Wearing Surface = 15 lb/sf
 Earth = 120 lb/cf
 Equivalent Fluid Pressure = 45 lb/cf
 Superstructure: Simply-Supported, Non-Composite for Dead Load. Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psi
 Class B-2 Concrete (Drilled Shafts & Rock Sockets) f'c = 4,000 psi
 Class B-1 Concrete (Barrier) f'c = 4,000 psi
 Class B-2 Concrete (Superstructure, except Prestressed Girders and Barrier) f'c = 4,000 psi
 Reinforcing Steel (ASTM A706 Grade 60) fy = 60,000 psi
 Structural Steel HP Pile (ASTM A709 Grade 50) fy = 50,000 psi
 For prestressed girder stresses, see Sheets No. 18 thru 20.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Pile Protective Coatings:

Piles shall be galvanized in accordance with Sec 702 and Sec. 1081.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

MBS Refers to mechanical bar splices. Mechanical bar splices shall be in accordance with Sec 706 or 710

All exposed substructure elements will contain epoxy coated reinforcing.

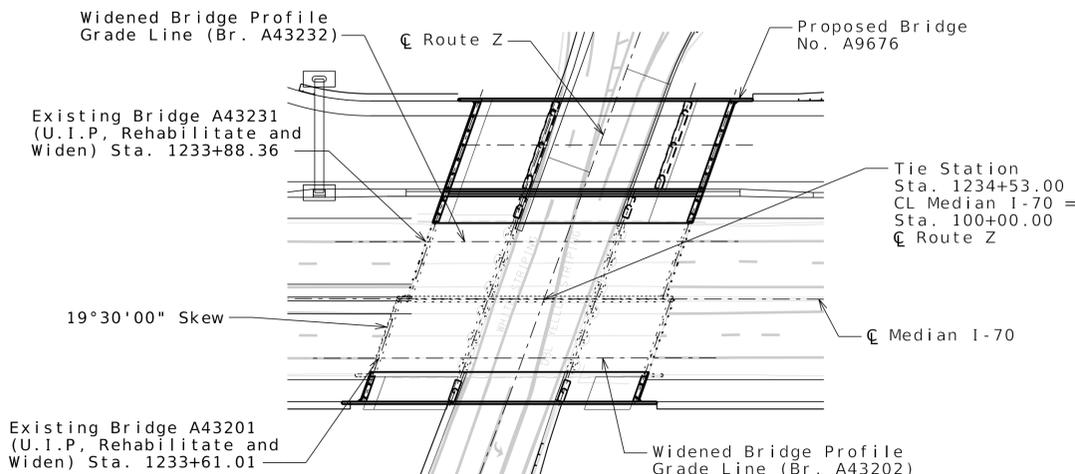
Traffic Handling:

Vertical clearance for Route Z traffic during construction shall be 15 ft. minimum over all lanes of live traffic as shown in each phase of the Maintenance of Traffic plans.

For traffic phasing during construction, see Maintenance of Traffic Plans.

Concrete Protective Coatings:

Protective coating for concrete bents and piers (Epoxy) shall be applied to all exposed substructure surfaces and in accordance with Sec 711.



LOCATION SKETCH



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DATE PREPARED
 9/2/2025

ROUTE
 I-70 STATE
 MO

DISTRICT
 BR SHEET NO.
 A43232-3

COUNTY
 ST. CHARLES

JOB NO.
 JST0020

CONTRACT ID.

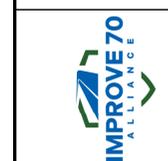
PROJECT NO.

BRIDGE NO.
 A43232

DATE	DESCRIPTION

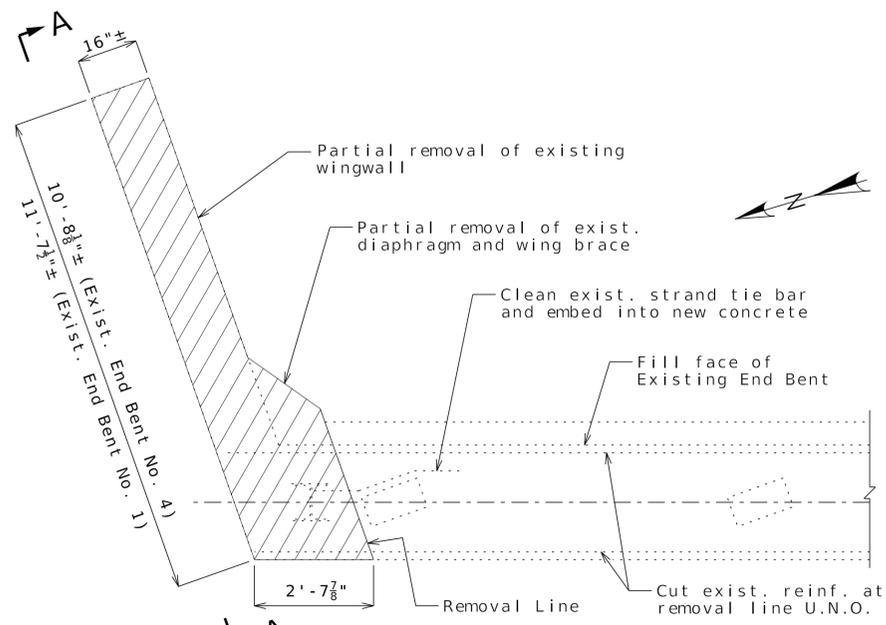
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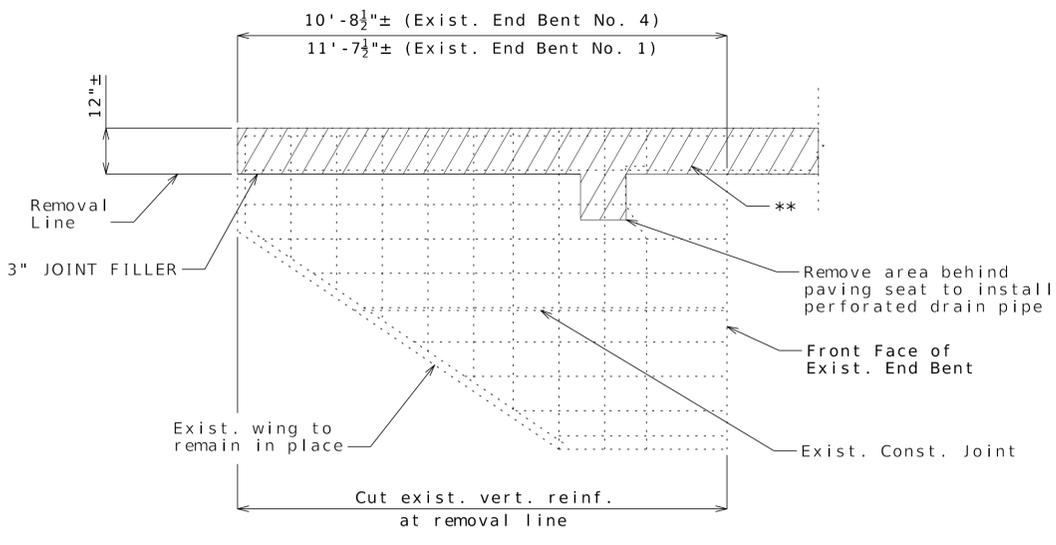


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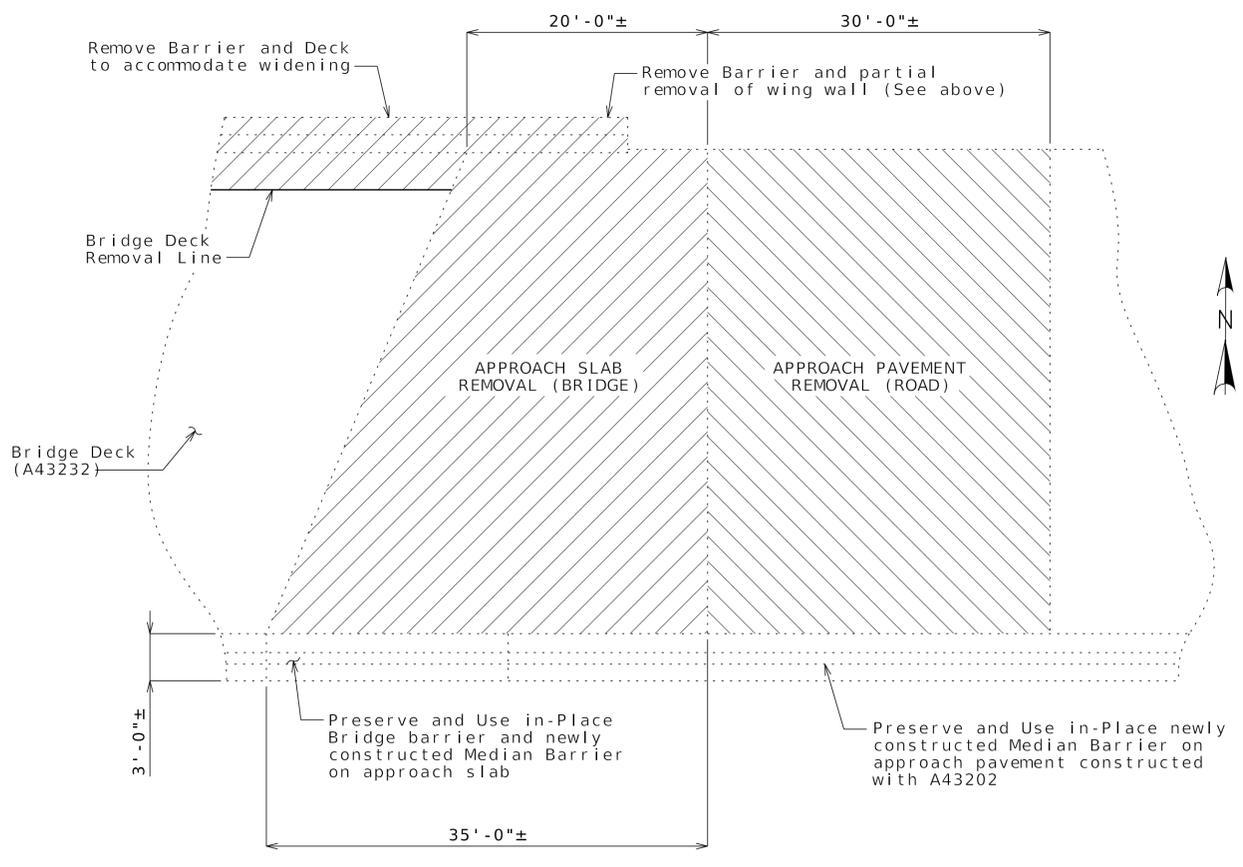
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PART PLAN OF EXISTING END BENT
(End Bent No. 4 shown, End Bent No. 1 similar, U.N.O.)

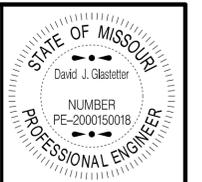


ELEVATION A-A
(**) Coordinate partial removal of existing wingwalls with the installation of vertical drain at end bents.



APPROACH SLAB REMOVAL DETAILS
(End Bent No. 4 shown, End Bent No. 1 similar)

Notes:
 // Denotes Removal
 Outline of work is indicated by light dashed lines.
 Contractor shall verify all dimensions in the field before ordering new material.
 Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete as shown.
 U.N.O. - Denotes Unless Noted Otherwise.



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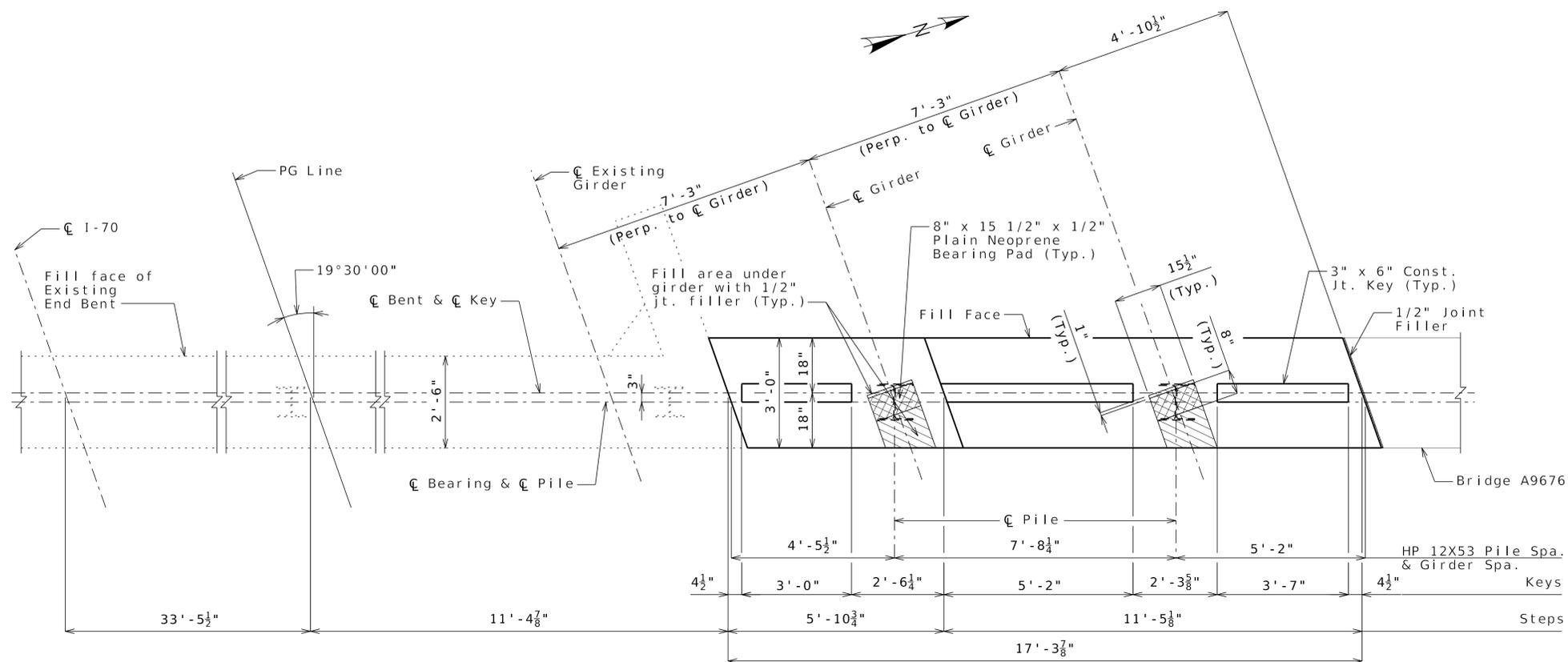
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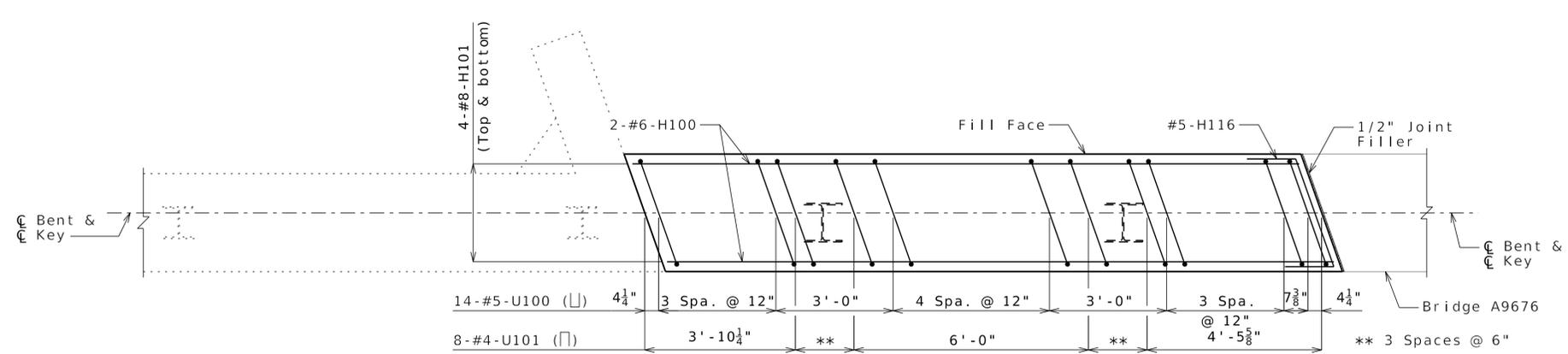
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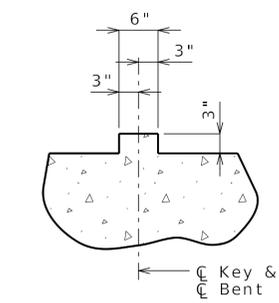
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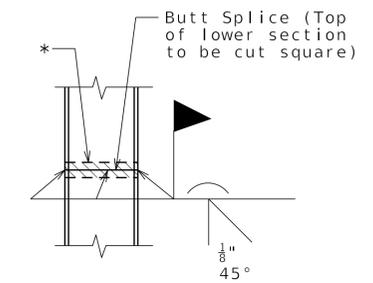
PLAN OF BEAM



PLAN OF BEAM SHOWING REINFORCEMENT
Keys not shown for clarity.



SECTION THRU KEY

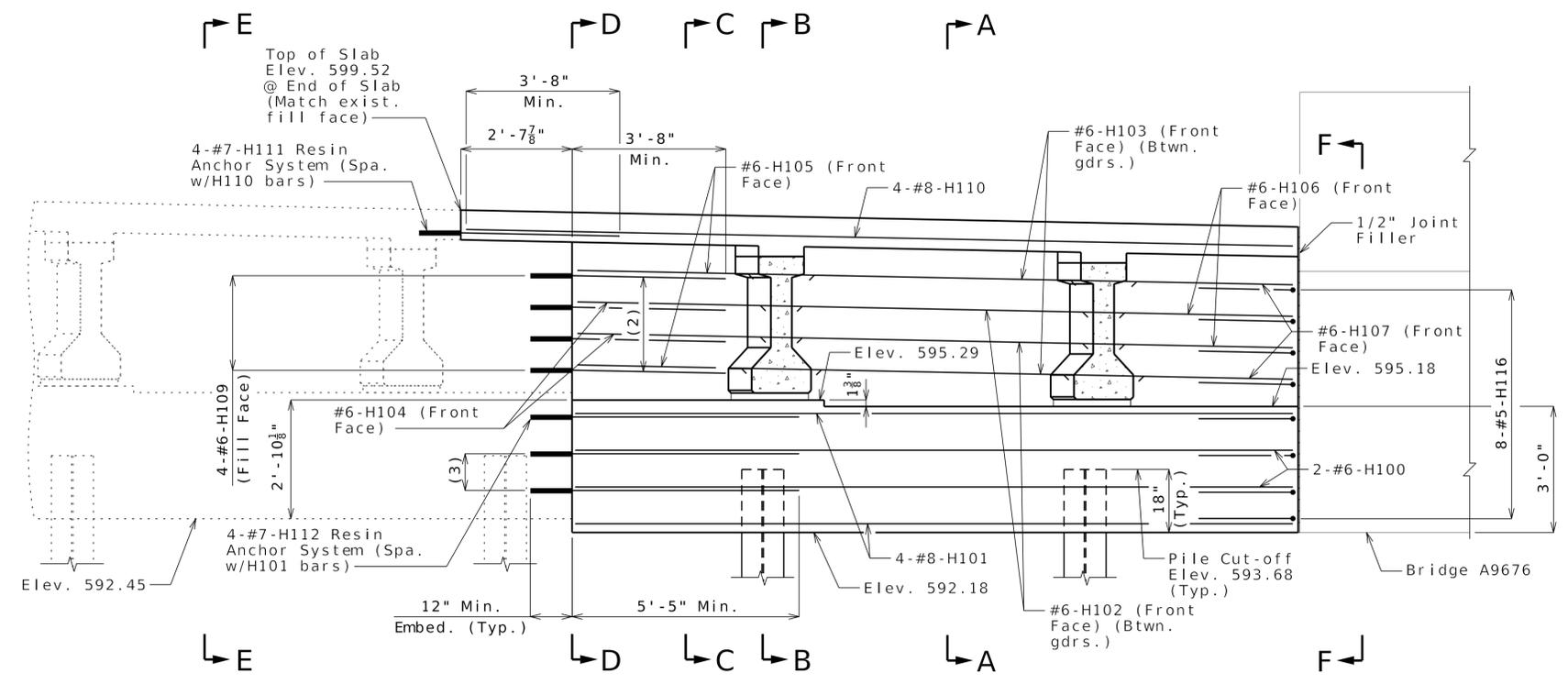


STEEL PILE SPLICE
(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.

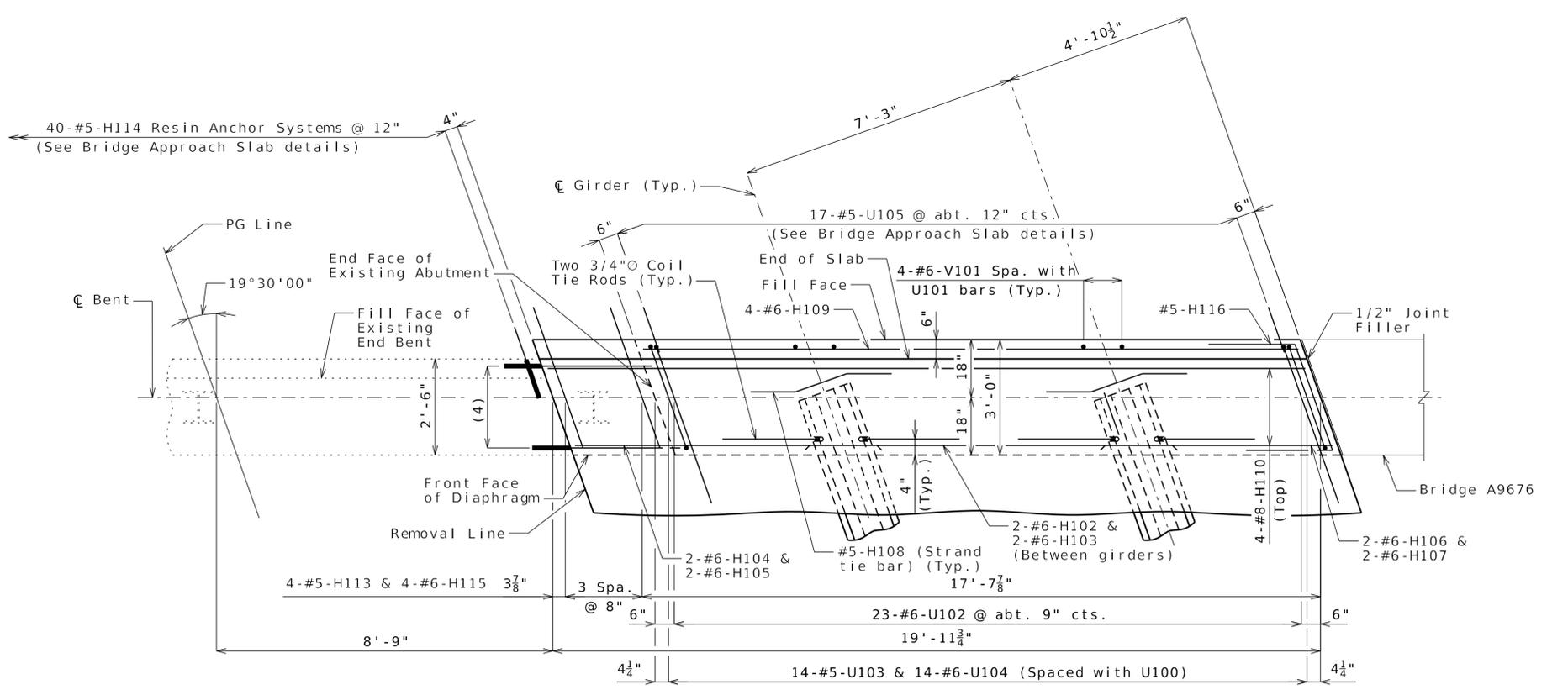
General Notes:
Work this sheet with Sheets No. 8 and 9.
All U bars and pairs of V bars shall be placed parallel to skewed steps.
Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

DETAILS OF END BENT NO. 1



SECTION NEAR END BENT
(Slab Reinforcing with MBS not shown for clarity.)

- (2) 4-#7-H111 Resin Anchor System (E.F) (Spa. w/H109 bars)
- (3) 2-#7-H112 Resin Anchor System (E.F.) (Spa. w/H100 bars)
- (4) 4-#7-H111 Resin Anchor System (Top) (Spa. w/H110 bars)



PART PLAN

General Notes:

Work this sheet with Sheets No. 7 and 9.

For Sections A-A, B-B & C-C and Elevations D-D & E-E, see Sheet No. 9.

The U bars shall be placed parallel to skewed steps.

All concrete in the end bent above top of beam and below top of slab shall be Class B-2.

Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.

For location of coil tie rods and #5-H108 (strand tie bar), see Sheet No. 18.

For details of vertical drain at end bents, see Sheet No. 11.

For details of bridge approach slab, see Sheet No. 27.

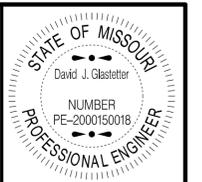
For Partial Wingwall Removal details, see Sheet No. 6.

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

All Elevations shown are based on profile grade elevations taken from the as-built plans and shall be field verified by the Contractor.

Contractor has the option to preserve existing transverse reinforcing bars over the diaphragm or to use the epoxy resin anchor systems (H111) shown.



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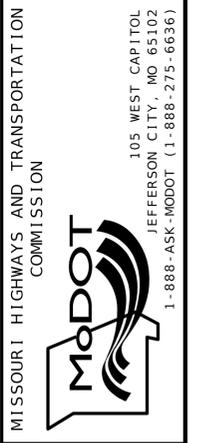
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DISTRICT BR SHEET NO. A43232-8

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JOB NO. JST0020
CONTRACT ID.

PROJECT NO.

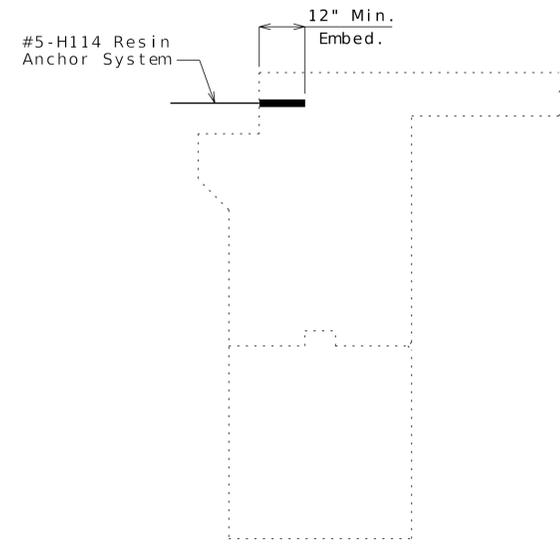
BRIDGE NO. A43232

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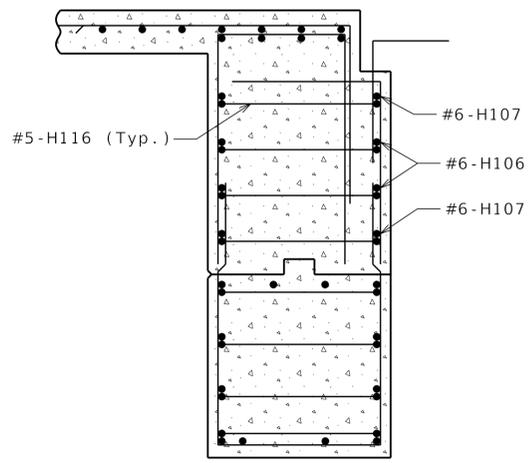


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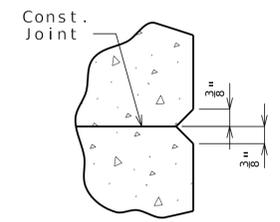
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SECTION E-E

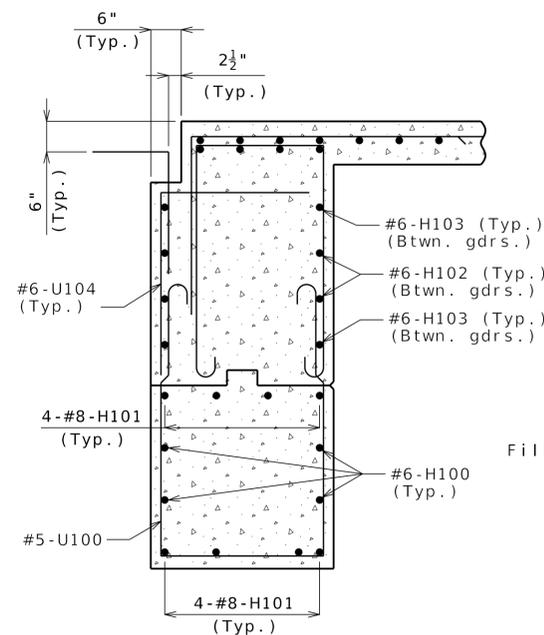


SECTION F-F

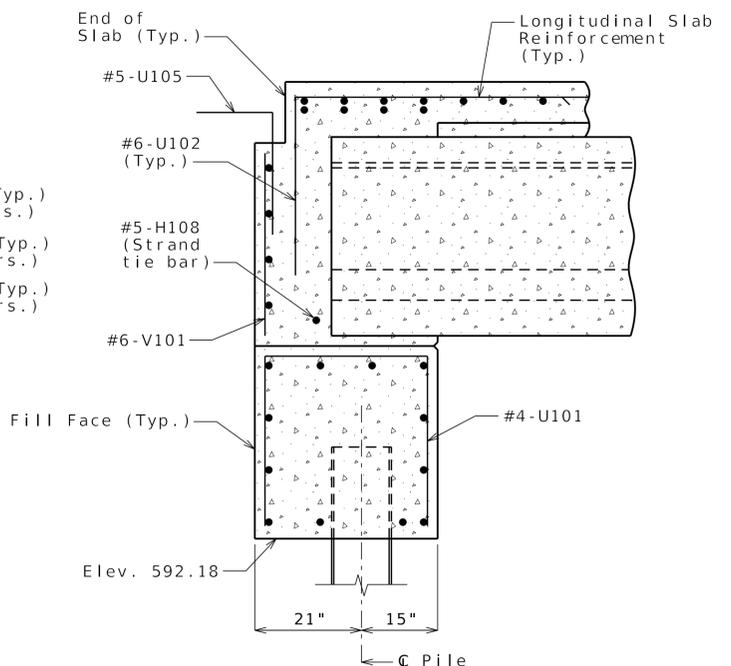


CHAMFER DETAIL

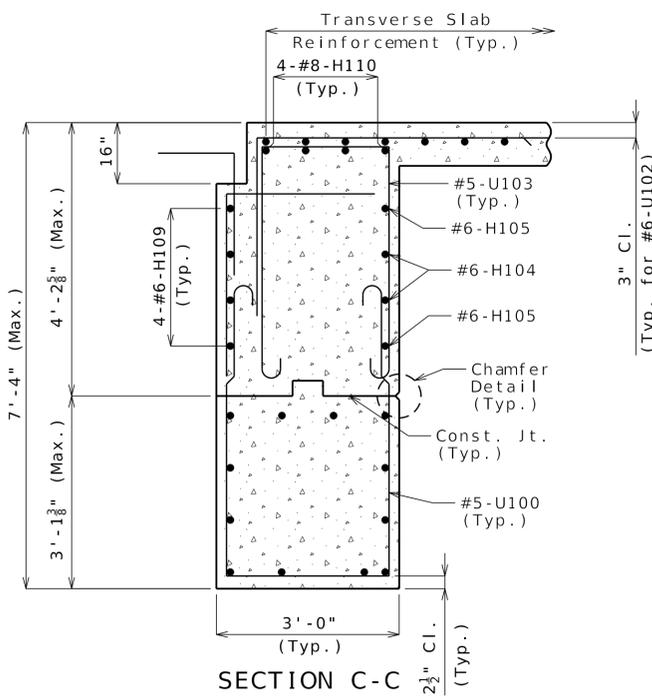
General Notes:
Work this sheet with Sheets No. 7 and 8.
For reinforcement of the barrier, see Sheet No. 26.



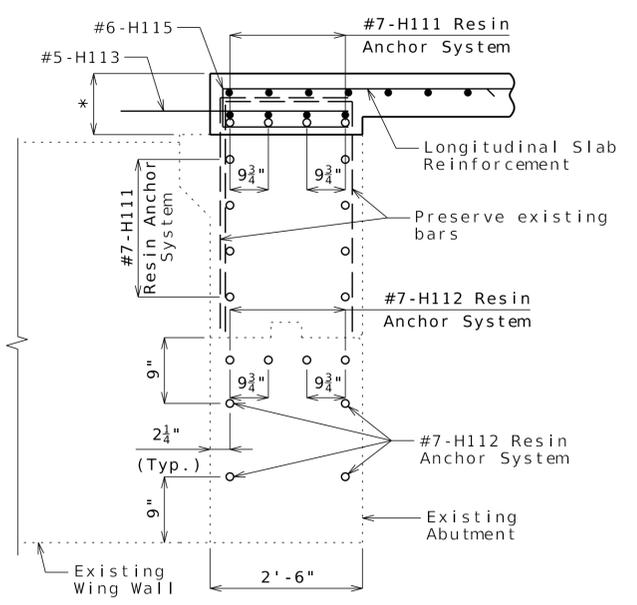
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

* Contractor to verify existing depth



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COUNTY ST. CHARLES	
JOB NO. JST0020	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A43232	

DATE	DESCRIPTION

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NO. 001270

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I - 70 MO

DISTRICT SHEET NO.
BR A43232-10

COUNTY
ST. CHARLES

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JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A43232

DATE	DESCRIPTION

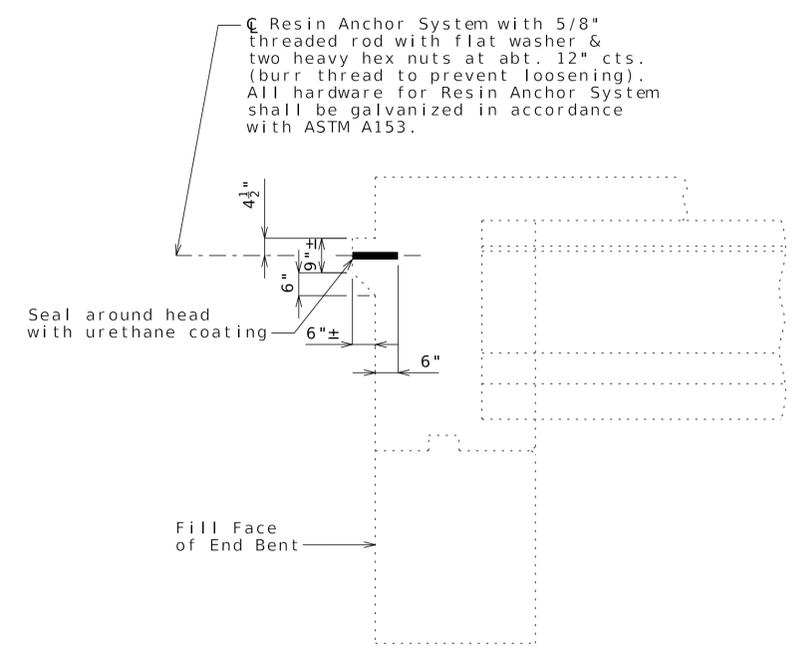
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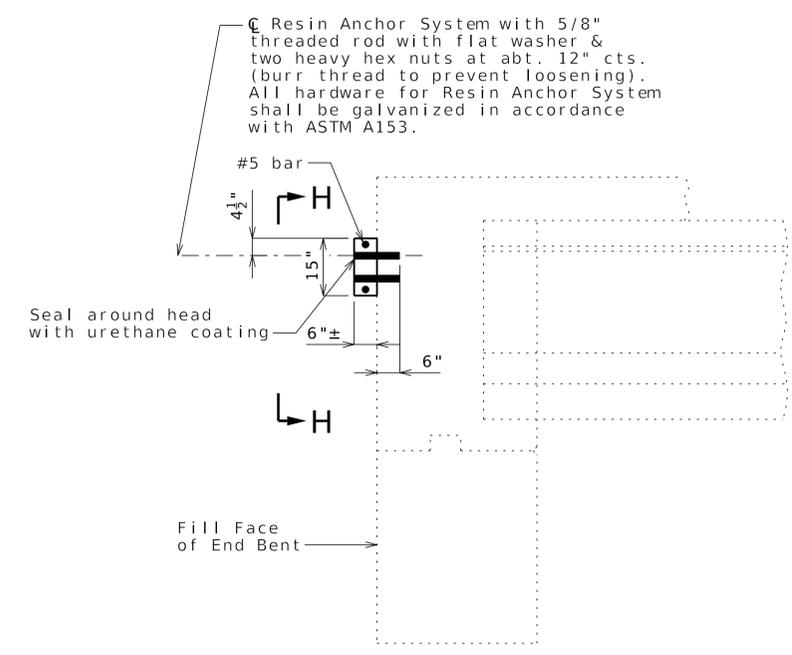


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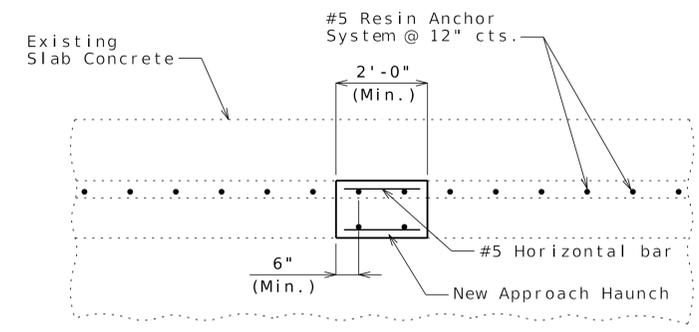
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SECTION THRU END BENT WITH APPROACH HAUNCH REPAIR
 (End Bent No.1 shown, End Bent No.4 similar)

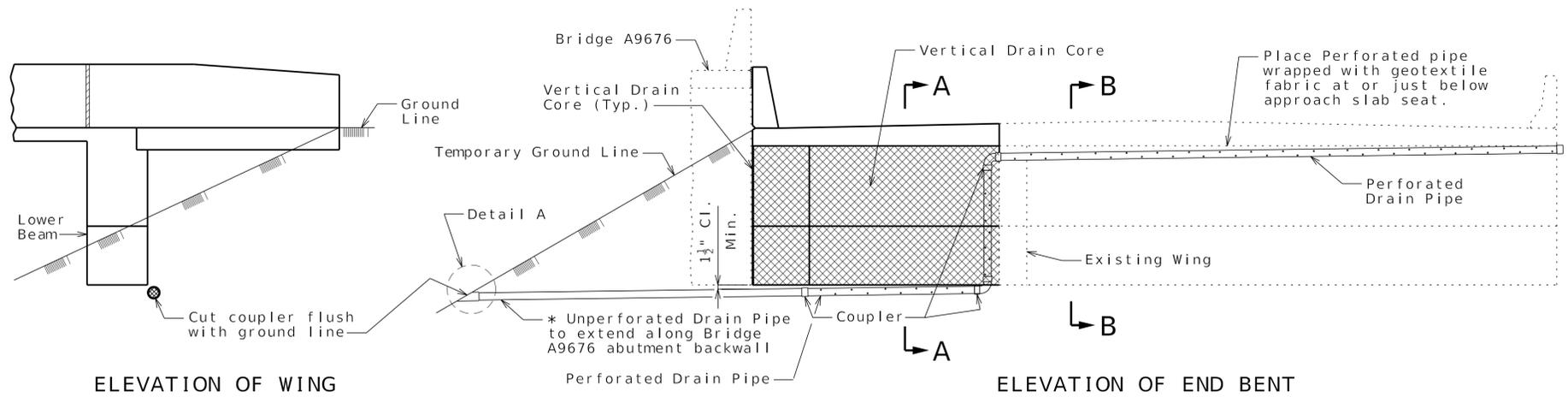


SECTION THRU END BENT WITH APPROACH HAUNCH REPLACEMENT
 (End Bent No.1 shown, End Bent No.4 similar)

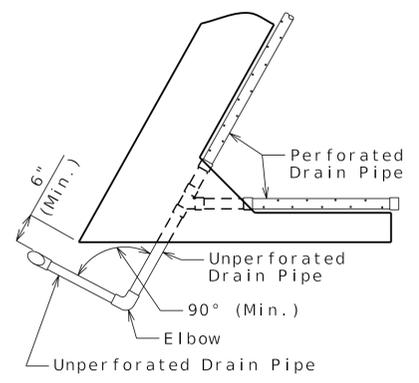
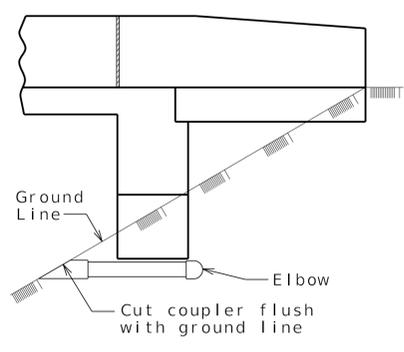
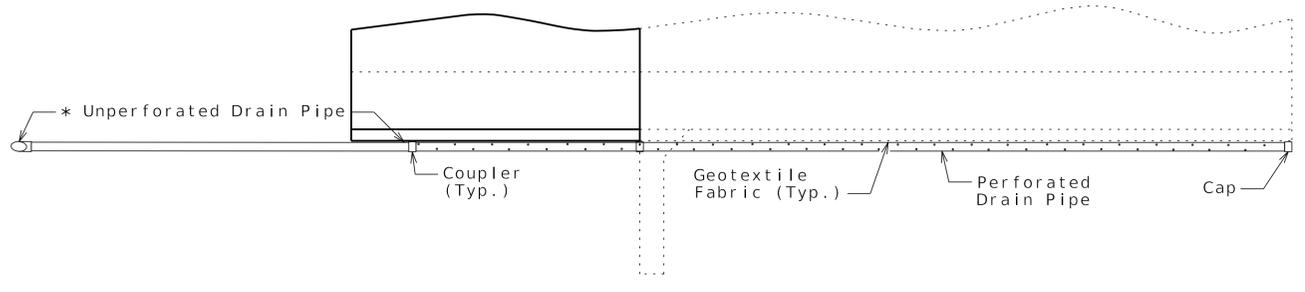
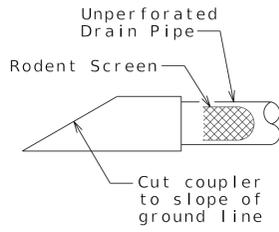


ELEVATION H-H
 (End Bent No.1 shown, End Bent No.4 similar)

Notes:
 Remove and replace deteriorated sections of approach notch as designated by the engineer and in accordance with details shown.
 Minimum length of replacement shall be 2'-0" as shown.
 Removal of concrete shall be in accordance with Sec 216.
 Concrete for approach notch replacement shall be Class B-1.
 46 - 5/8" threaded rods along End Bent No. 1 and End Bent No. 4 = 92 total

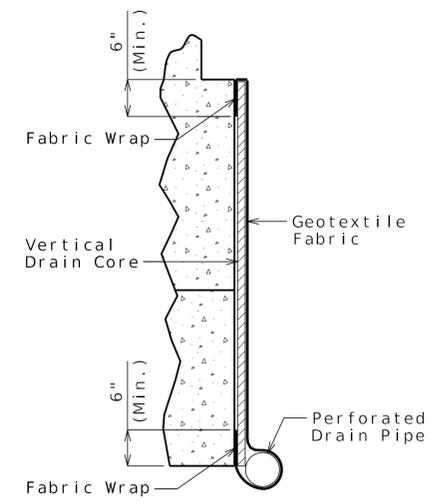


* Contractor has the option to connect Bridge A9676 perforated pipe to this pipe and use as drain.



OPTIONAL TURNED DRAIN
(Use only when straight drain is not practical.)

VERTICAL DRAIN AT END BENTS
(Squared end bent shown, skewed end bent similar)



Place Perforated pipe wrapped with geotextile fabric at or just below approach slab seat.

Backfill shall be similar to material for vertical drains at end bents, placed and compacted in accordance with Sec 206

PART SECTION B-B

General Notes:

- Vertical Drain pipe construction to coordinate with Bridge A9676.
- All drain pipe shall be sloped 1 to 2 percent.
- Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.
- Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.
- Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.



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DATE	DESCRIPTION

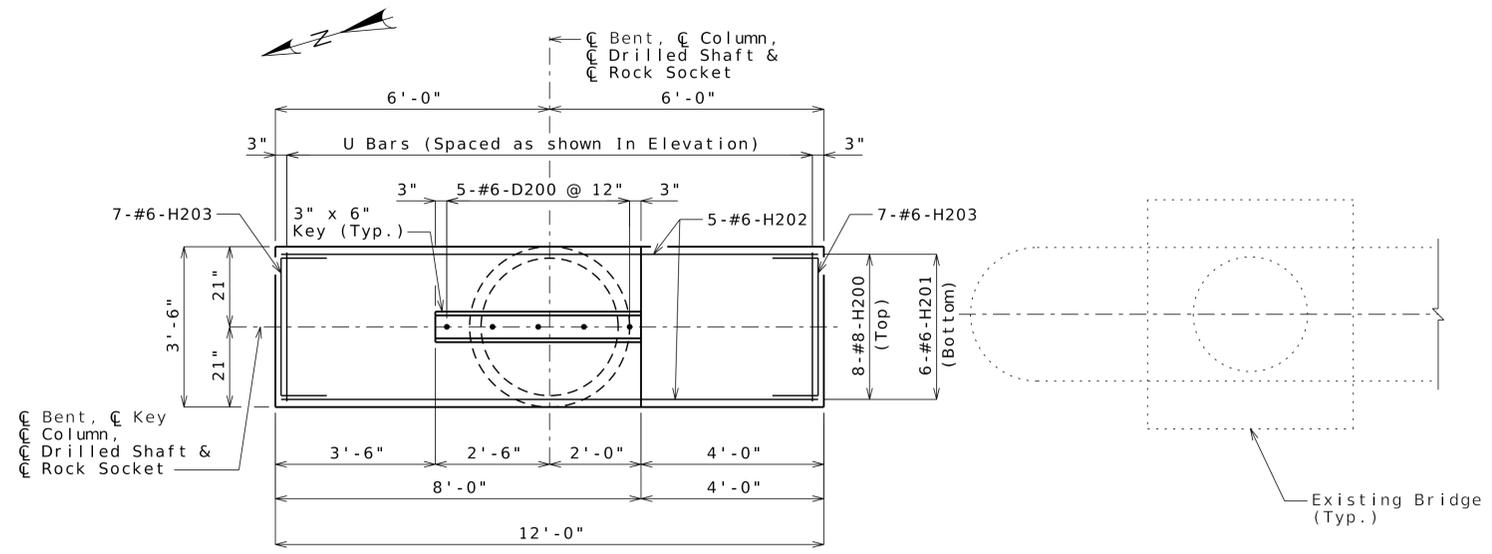
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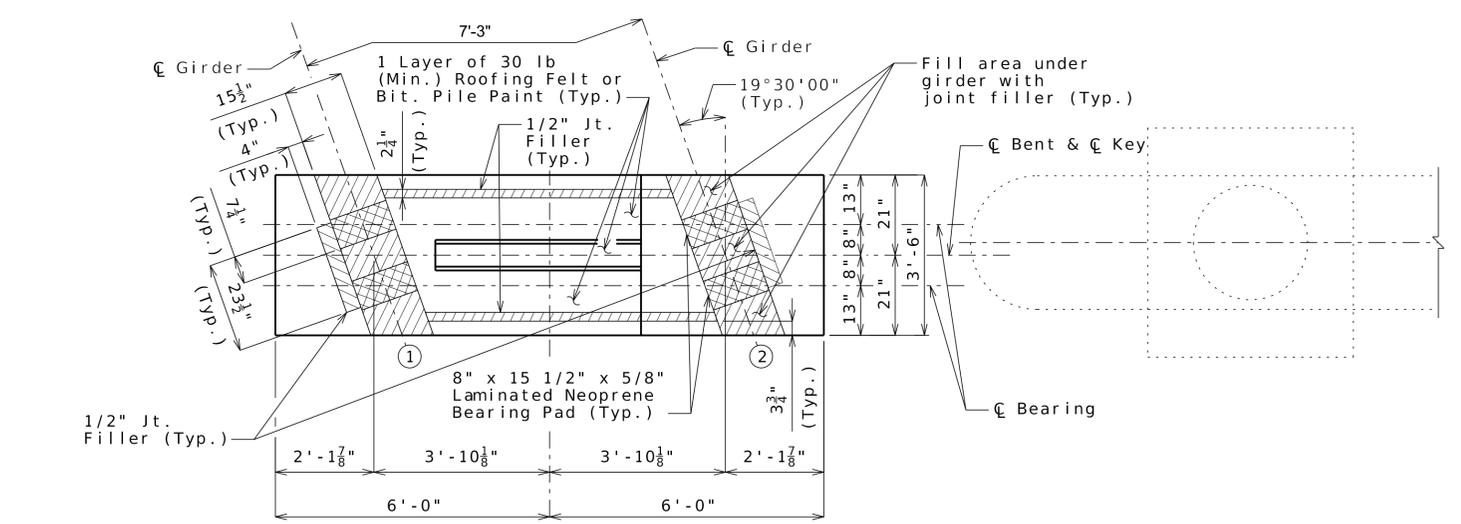


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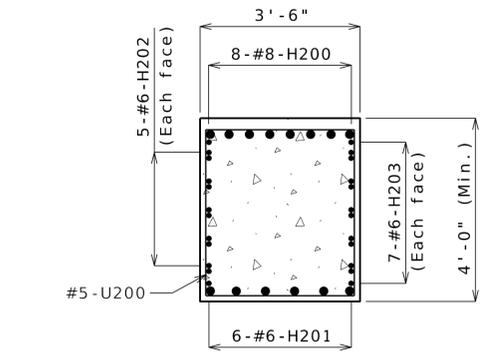
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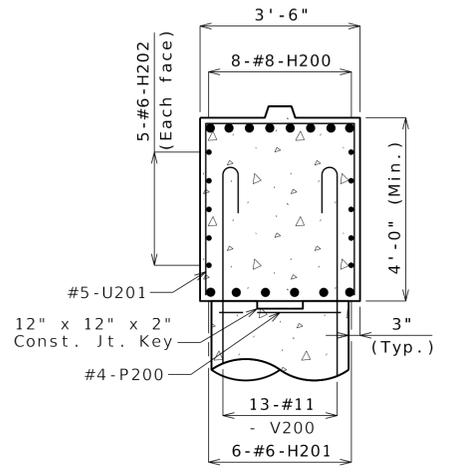
PLAN SHOWING REINFORCEMENT



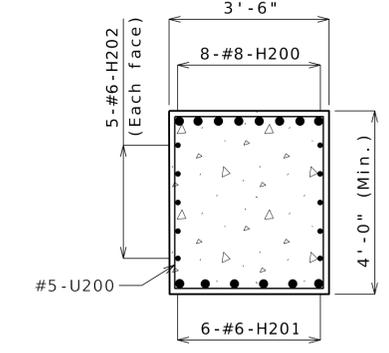
PLAN OF BEAM



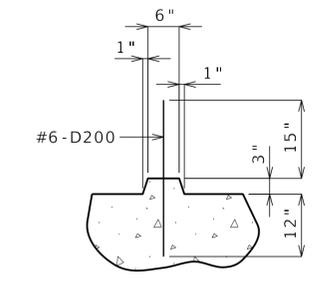
SECTION A-A



SECTION B-B



SECTION C-C



SECTION THRU KEY

General Notes:
 Work this sheet with Sheet No. 12.
 For steps 2 inches or more, use 2 1/4x 1/2-inch joint filler up vertical face.
 Bar marks are shown for Bent 2, using "200" series bar marks. Bent similar, except use "300" series bar marks.



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CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A43232	

DATE	DESCRIPTION

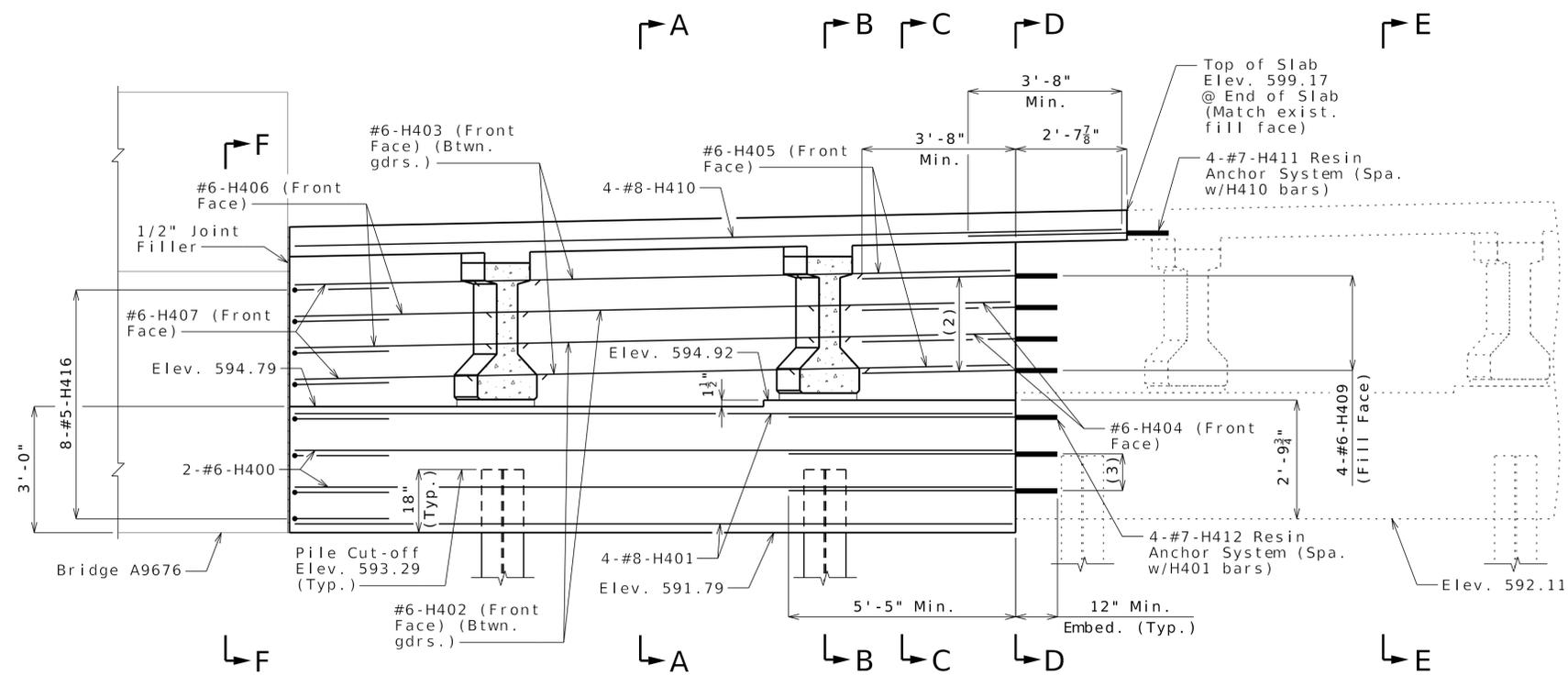
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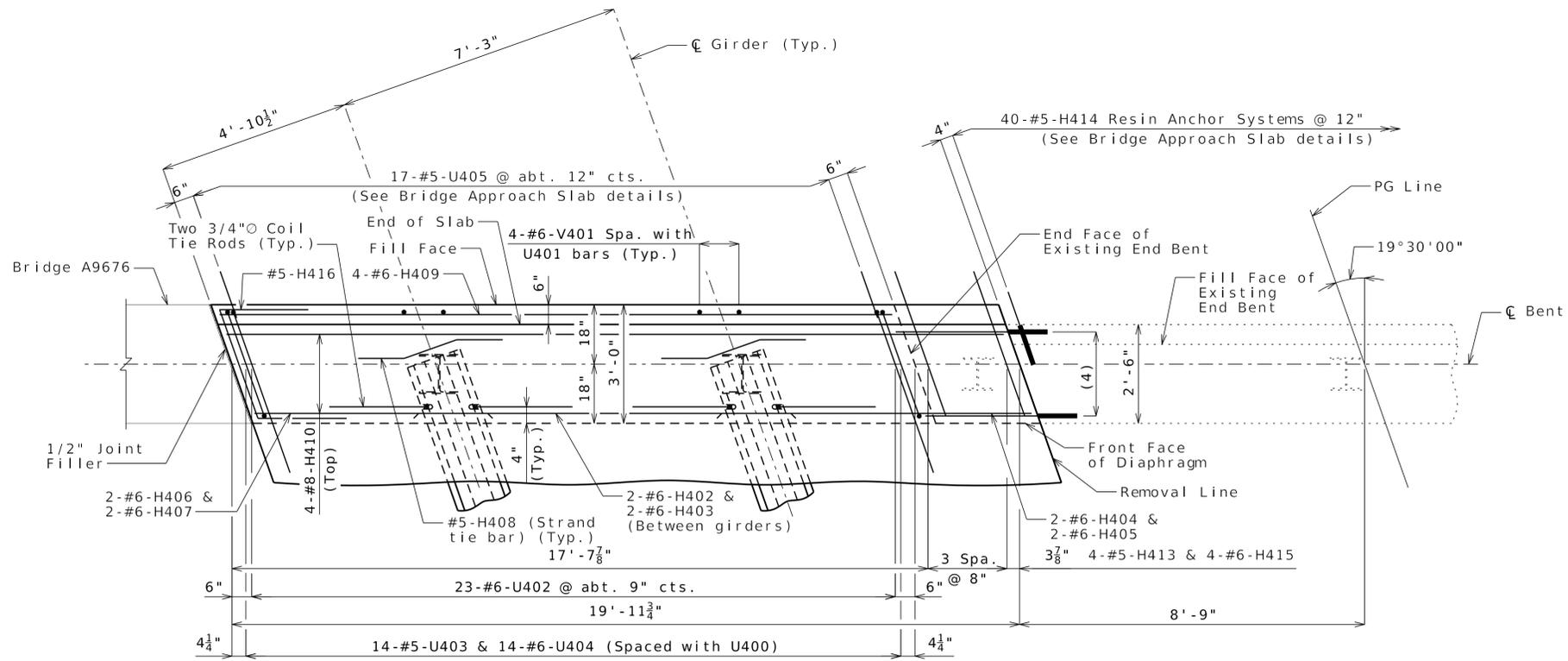


SECTION NEAR END BENT
(Slab Reinforcing with MBS not shown for clarity.)

- (2) 4-#7-H411 Resin Anchor System (E.F.) (Spa. w/H409 bars)
- (3) 2-#7-H412 Resin Anchor System (E.F.) (Spa. w/H400 bars)
- (4) 4-#7-H411 Resin Anchor System (Top) (Spa. w/H410 bars)

General Notes:

- Work this sheet with Sheets No. 14 & 16.
- For Sections A-A, B-B & C-C and Elevations D-D & E-E, see Sheet No. 16.
- The U bars shall be placed parallel to skewed steps.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.
- For location of coil tie rods and #5-H408 (strand tie bar), see Sheet No. 20.
- For details of vertical drain at end bents, see Sheet No. 11.
- For details of bridge approach slab, see Sheet No. 27.
- For Partial Wingwall Removal details, see Sheet No. 6.
- The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.
- The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".
- All Elevations shown are based on profile grade elevations taken from the as-built plans and shall be field verified by the Contractor.
- Contractor has the option to preserve existing transverse reinforcing bars over the diaphragm or to use the epoxy resin anchor systems (H411) shown.



PART PLAN

Note: This drawing is not to scale. Follow dimensions.

Sheet 15 of 35

DETAILS OF END BENT NO. 4

STATE OF MISSOURI
David J. Glasetter
NUMBER PE-200150018
PROFESSIONAL ENGINEER

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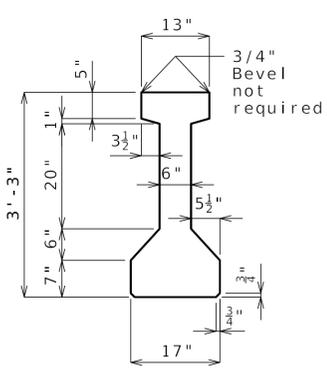
DATE PREPARED 9/2/2025	
ROUTE I-70	STATE MO
DISTRICT BR	SHEET NO. A43232-15
COUNTY ST. CHARLES	
JOB NO. JST0020	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A43232	

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

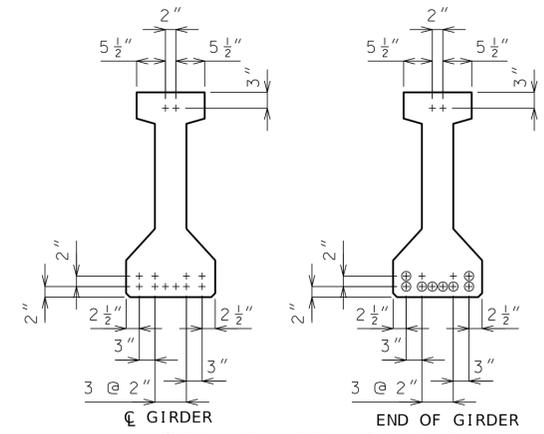
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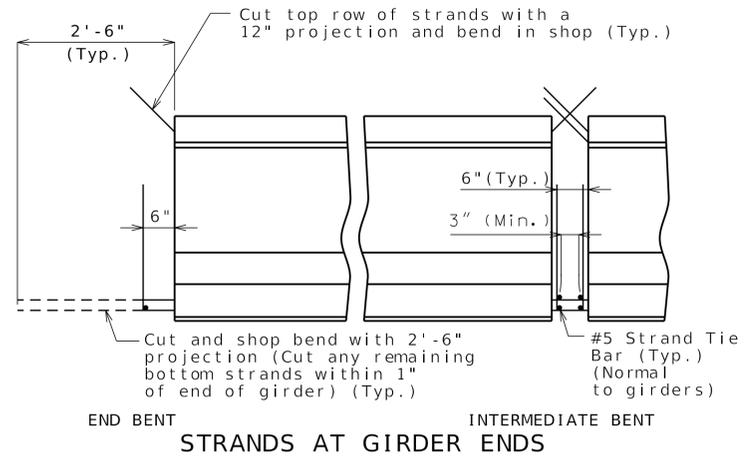


DIMENSIONS



STRAND ARRANGEMENT

+ Indicates prestressing strand. ○ Indicates cut & shop bend with 2'-6" projection.



STRANDS AT GIRDER ENDS

BILL OF REINFORCING STEEL - EACH GIRDER				
NO.	SIZE & MARK	ACTUAL LENGTH	SHAPE	BENDING DIAGRAM
2	6 A1	42'-10"	20	
86	5 B1	4'-7"	11S	
16	6 B2	4'-0"	11S	
51	4 C1	13"	10S	
102	4 D1	2'-5"	9S	

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

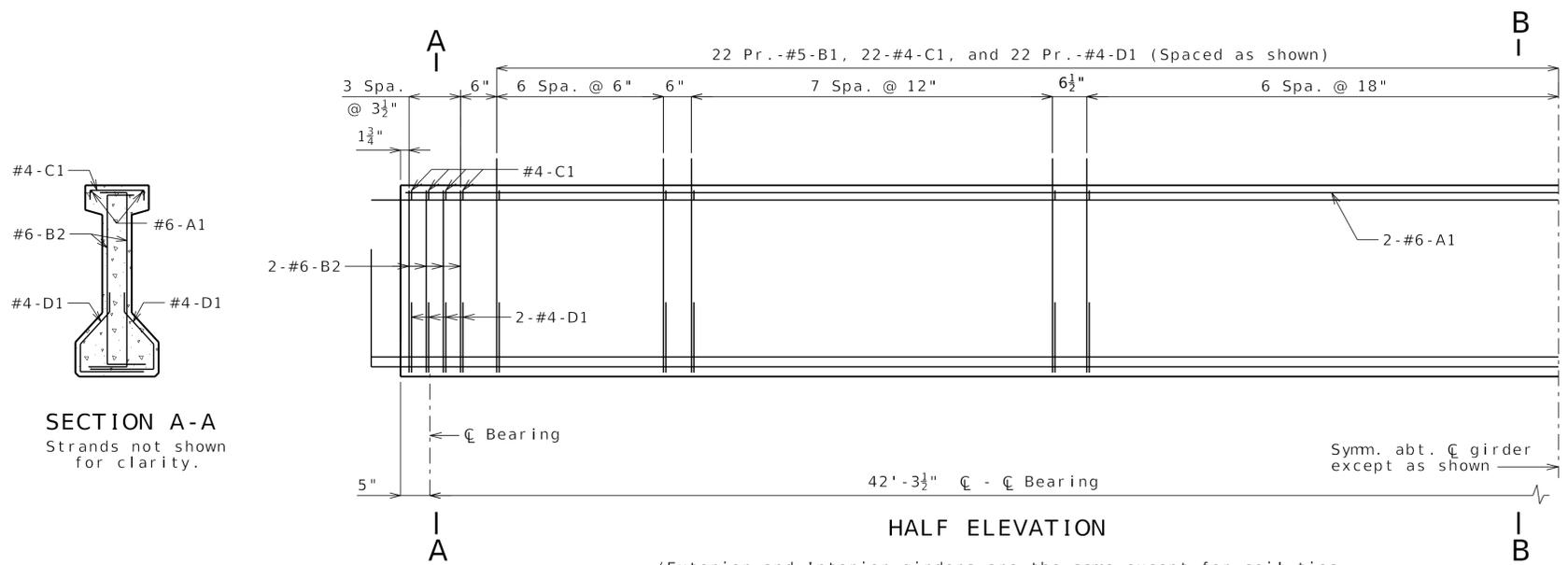
Actual lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

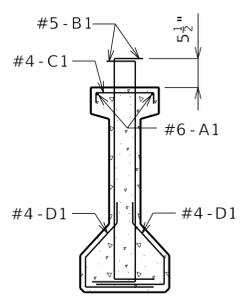
All B1 bars shall be epoxy coated.



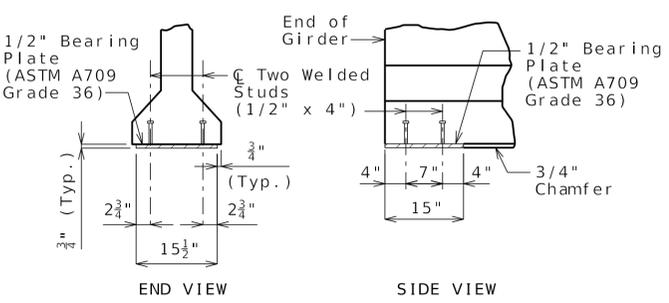
HALF ELEVATION

(Exterior and Interior girders are the same except for coil ties and coil inserts for slab drains.)

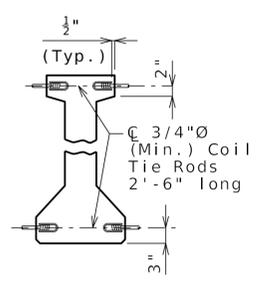
SECTION A-A
 Strands not shown for clarity.



SECTION B-B
 Strands not shown for clarity.



BEARING PLATE



COIL TIES

Exclude coil tie at intermediate bent diaphragms as shown on Sheet No. 21.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'c = 6,000$ psi and $f'ci = 4,500$ psi.

Use 12 strands, 0.6"Ø Grade 270, with an initial prestress force of 528 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

For Girder Camber Diagram, see Sheet No. 23.

For location of coil inserts at slab drains, see Sheet No. 22.

For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 8 and 21.



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DATE PREPARED
 9/2/2025

ROUTE
 I-70 STATE
 MO

DISTRICT
 BR SHEET NO.
 A43232-18

COUNTY
 ST. CHARLES

JOB NO.
 JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
 A43232

DATE	DESCRIPTION

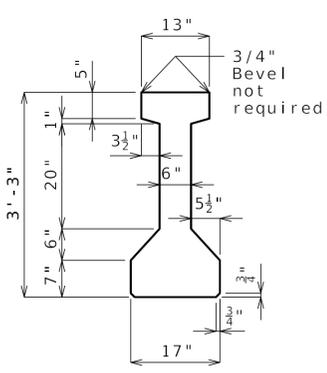
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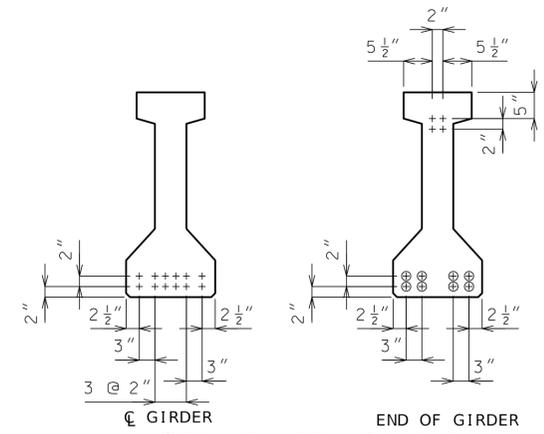


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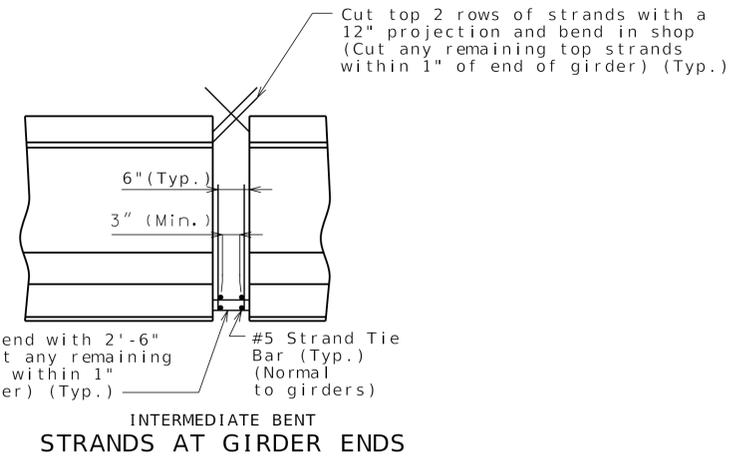


DIMENSIONS



STRAND ARRANGEMENT

+ Indicates prestressing strand. o Indicates cut & shop bend with 2'-6" projection.



INTERMEDIATE BENT STRANDS AT GIRDER ENDS

BILL OF REINFORCING STEEL - EACH GIRDER				
NO.	SIZE & MARK	ACTUAL LENGTH	SHAPE	BENDING DIAGRAM
2	6 A1	58'-8"	20	
138	5 B1	4'-7"	11S	
16	6 B2	4'-0"	11S	
77	4 C1	13"	10S	
154	4 D1	2'-5"	9S	

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

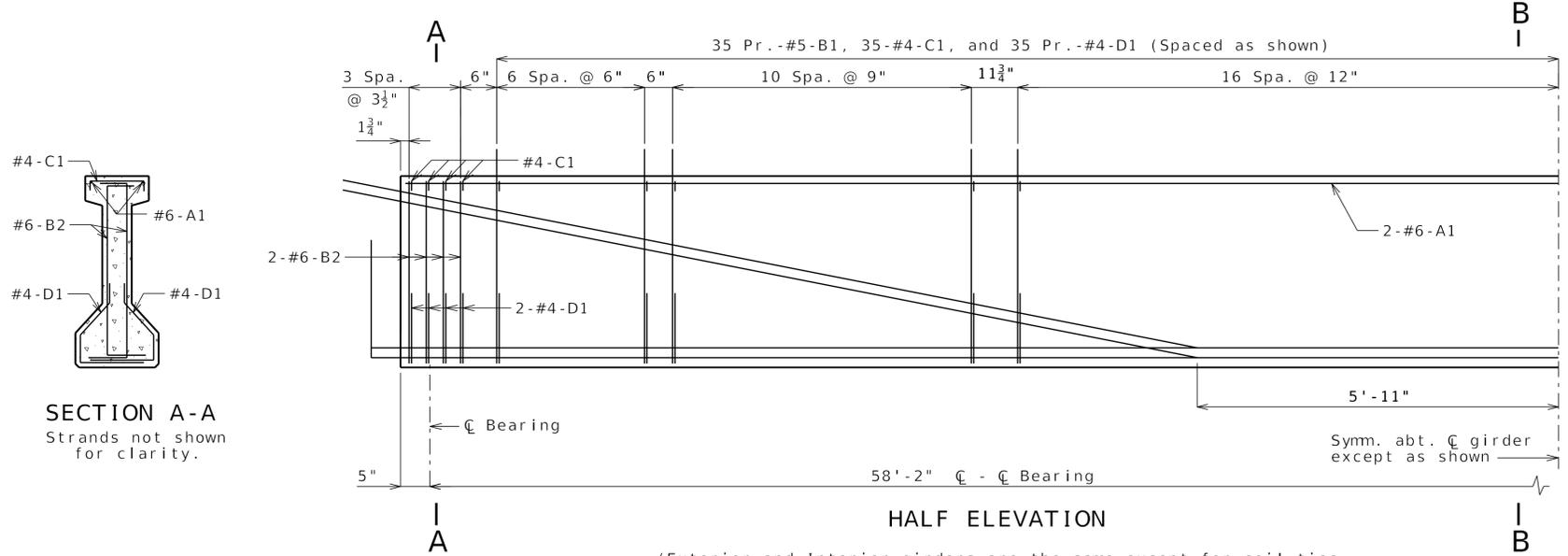
Actual lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All reinforcement shall be Grade 60.

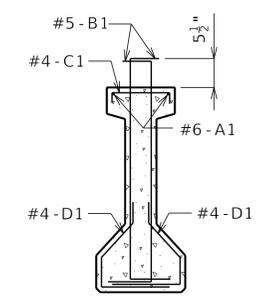
The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

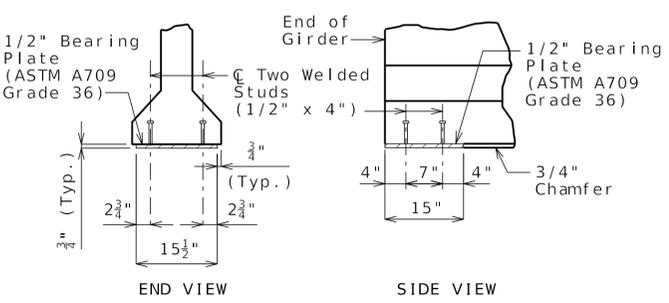


HALF ELEVATION

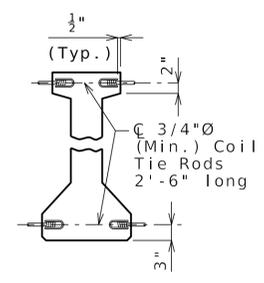
(Exterior and Interior girders are the same except for coil ties and holes for steel intermediate diaphragms.)



SECTION B-B
Strands not shown for clarity.



BEARING PLATE



COIL TIES

Exclude coil tie at intermediate bent diaphragms as shown on Sheet No. 21.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'c = 6,000$ psi and $f'ci = 4,500$ psi.

Use 12 strands, 0.6"Ø Grade 270, with an initial prestress force of 528 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

For Girder Camber Diagram, see Sheet No. 23.

The 1 1/2"Ø holes shall be cast in the web for steel intermediate diaphragms. Drilling is not allowed. For location of holes and details of steel intermediate diaphragms, see Sheet No. 17.

For location of coil inserts at slab drains, see Sheet No. 22.

For location of coil ties at concrete diaphragms, see Sheet No. 21.

David J. Glasetter
 NUMBER PE-2000150018
 PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED: 9/2/2025
 ROUTE: I-70 STATE: MO
 DISTRICT: BR SHEET NO.: A43232-19
 COUNTY: ST. CHARLES
 JOB NO.: JST0020
 CONTRACT ID.:
 PROJECT NO.:
 BRIDGE NO.: A43232

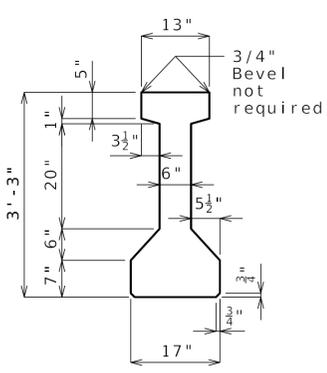
DATE	DESCRIPTION

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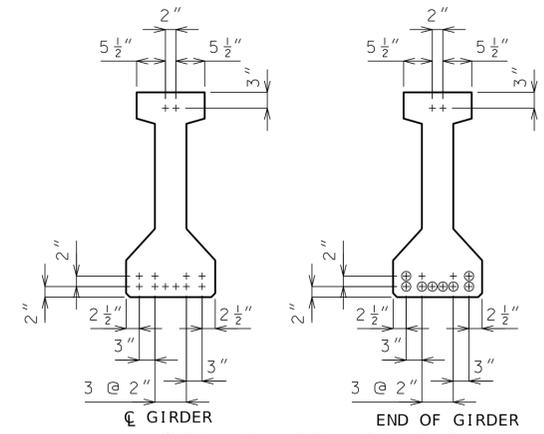
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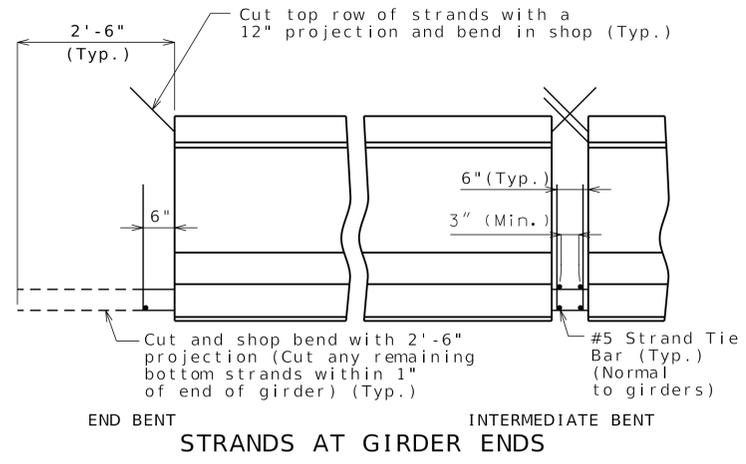


DIMENSIONS

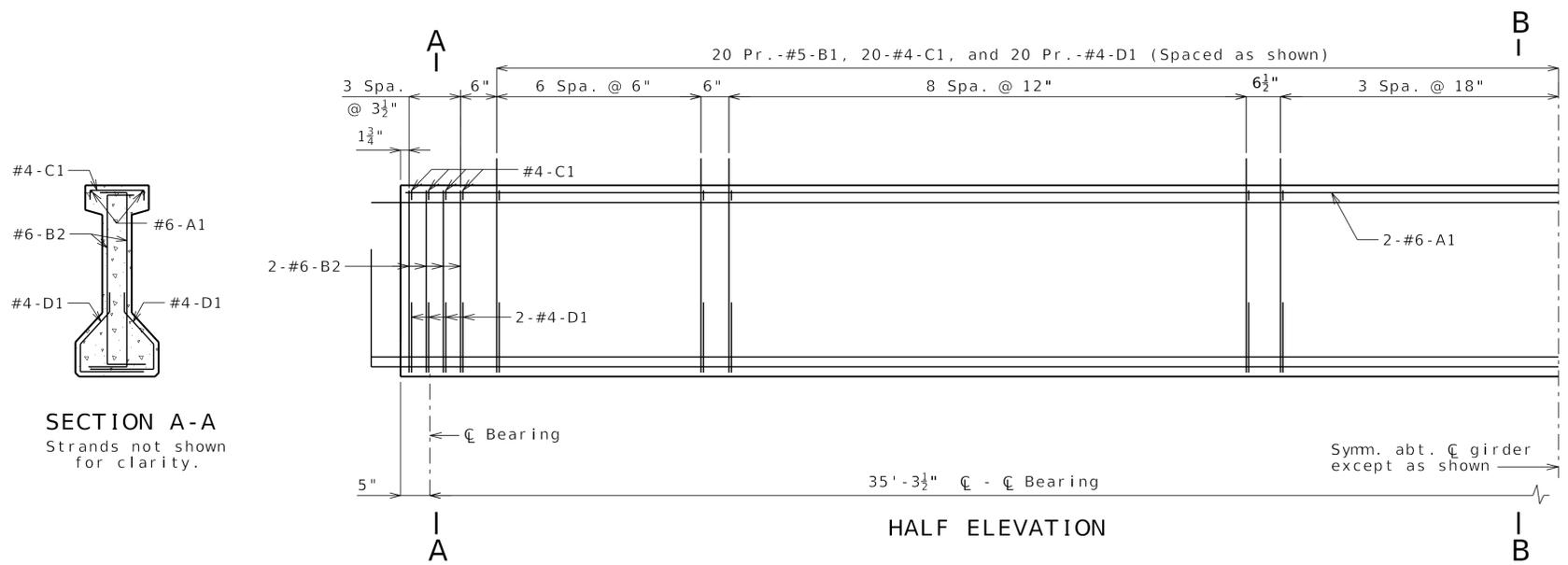


STRAND ARRANGEMENT

+ Indicates prestressing strand. ○ Indicates cut & shop bend with 2'-6" projection.

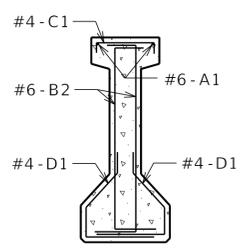


STRANDS AT GIRDER ENDS

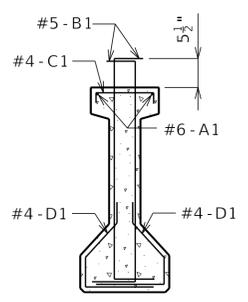


HALF ELEVATION

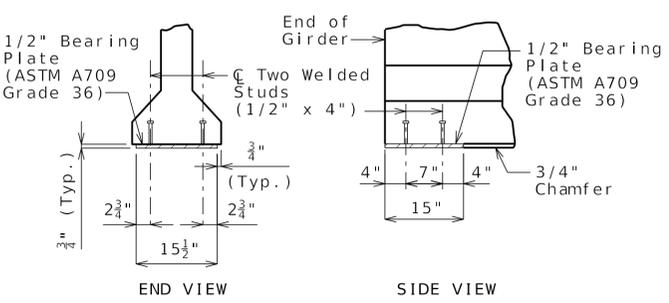
(Exterior and Interior girders are the same except for coil ties and coil inserts for slab drains.)



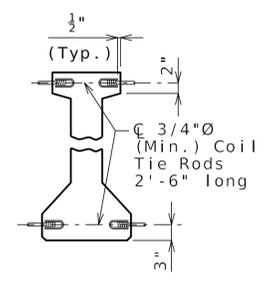
SECTION A-A
 Strands not shown for clarity.



SECTION B-B
 Strands not shown for clarity.



BEARING PLATE



COIL TIES

Exclude coil tie at intermediate bent diaphragms as shown on Sheet No. 21.

BILL OF REINFORCING STEEL - EACH GIRDER					
NO.	SIZE & MARK	ACTUAL LENGTH	SHAPE	BENDING DIAGRAM	
2	6 A1	35'-10"	20	SHAPE 10S	
78	5 B1	4'-7"	11S	SHAPE 9S	
16	6 B2	4'-0"	11S	SHAPE 20	
47	4 C1	13"	10S	SHAPE 11S	
94	4 D1	2'-5"	9S		

All dimensions are out to out.
 Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.
 Actual lengths are measured along centerline of bar to the nearest inch.
 Minimum clearance to reinforcing shall be one inch.
 All reinforcement shall be Grade 60.
 The two D1 bars may be furnished as one bar at the fabricator's option.
 All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'c = 6,000$ psi and $f'ci = 4,500$ psi.
 Use 12 strands, 0.6"Ø Grade 270, with an initial prestress force of 528 kips.
 Pretensioned members shall be in accordance with Sec 1029.
 Fabricator shall be responsible for location and design of lifting devices.
 For Girder Camber Diagram, see Sheet No. 23.
 For location of coil inserts at slab drains, see Sheet No. 22.
 For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 15 and 21.



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 ROUTE: I-70
 STATE: MO
 DISTRICT: BR
 SHEET NO.: A43232-20

COUNTY: ST. CHARLES
 JOB NO.: JST0020
 CONTRACT ID.:
 PROJECT NO.:
 BRIDGE NO.: A43232

DATE	DESCRIPTION

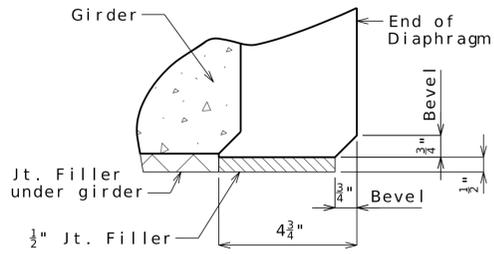
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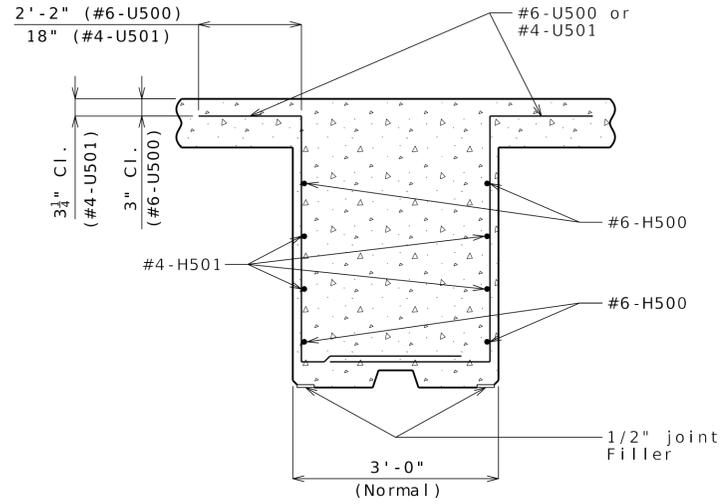


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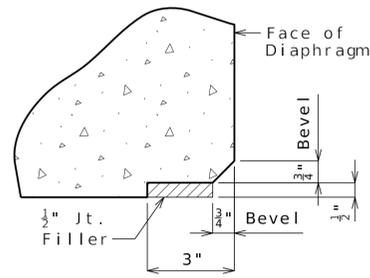
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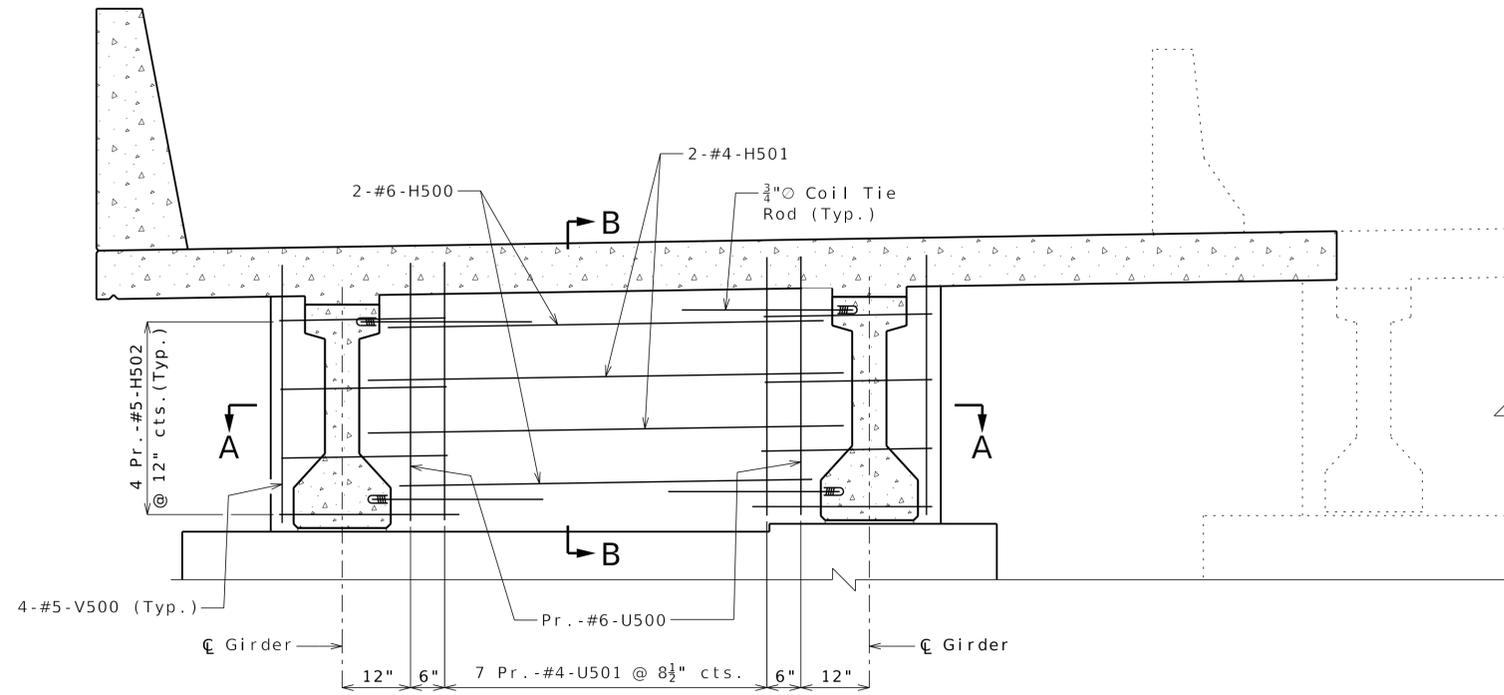
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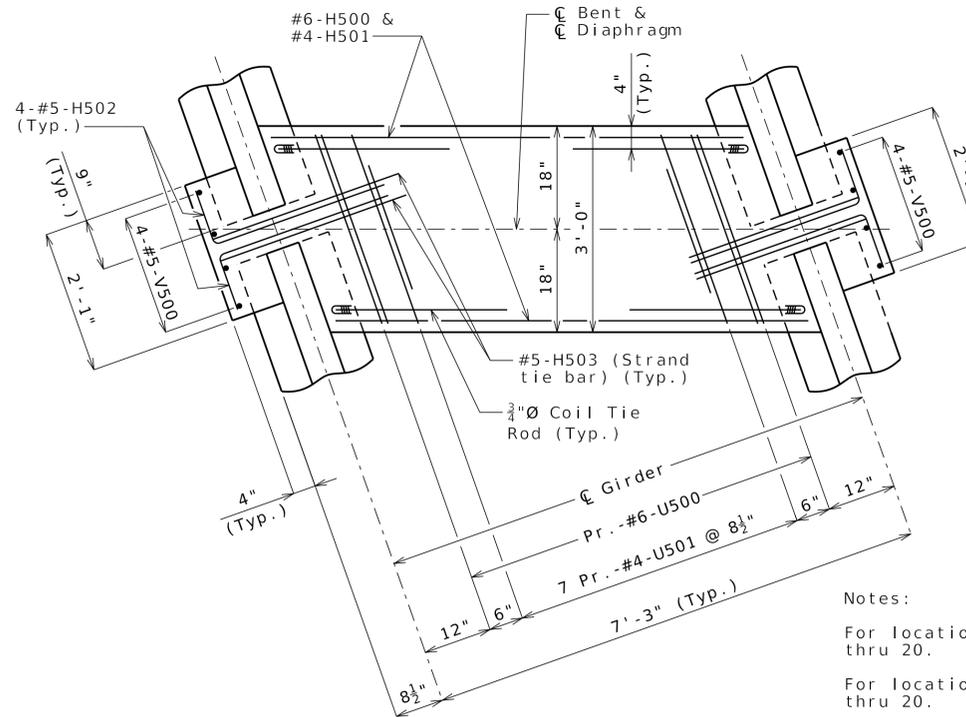
SECTION B-B



EDGE DETAIL



SECTION NEAR INTERMEDIATE BENT
Normal to \bar{C} Structure



SECTION A-A

Notes:

- For location and number of Strand Tie Bars, see Sheets No. 18 thru 20.
- For location and details of Coil Tie Rods, see Sheets No. 18 thru 20.
- Diaphragms at intermediate bents shall be built vertical.
- All U-bars in diaphragms are to be placed parallel to \bar{C} Roadway.
- See Details of Intermediate Bents for shear key, dowel placement, and joint filler details not shown here.



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COUNTY ST. CHARLES	
JOB NO. JST0020	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A43232	

DATE	DESCRIPTION

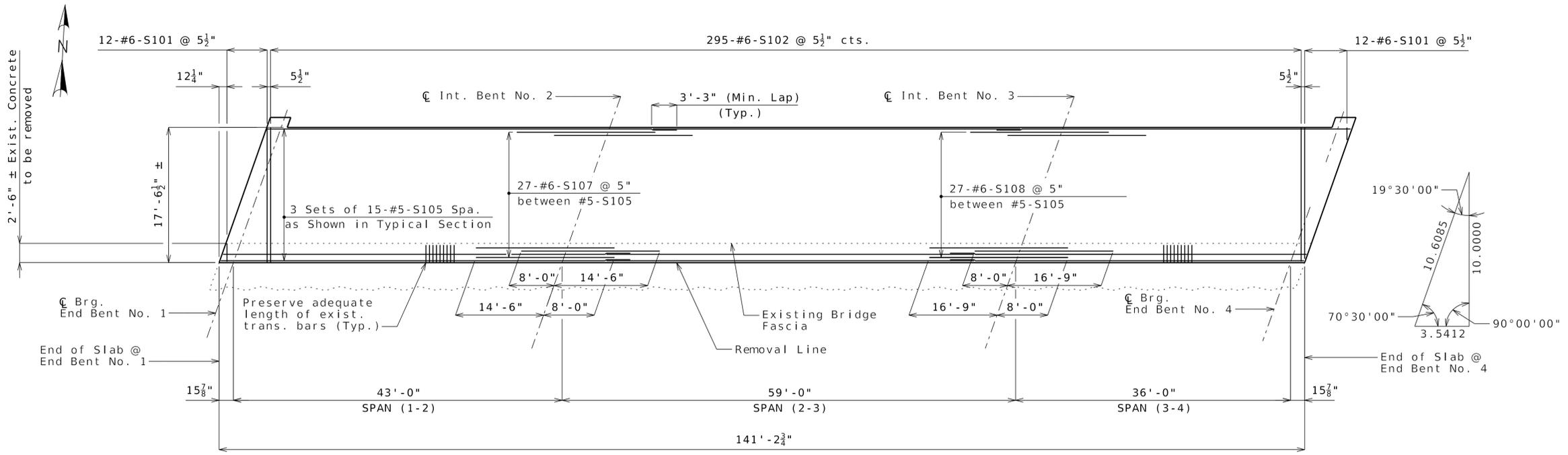
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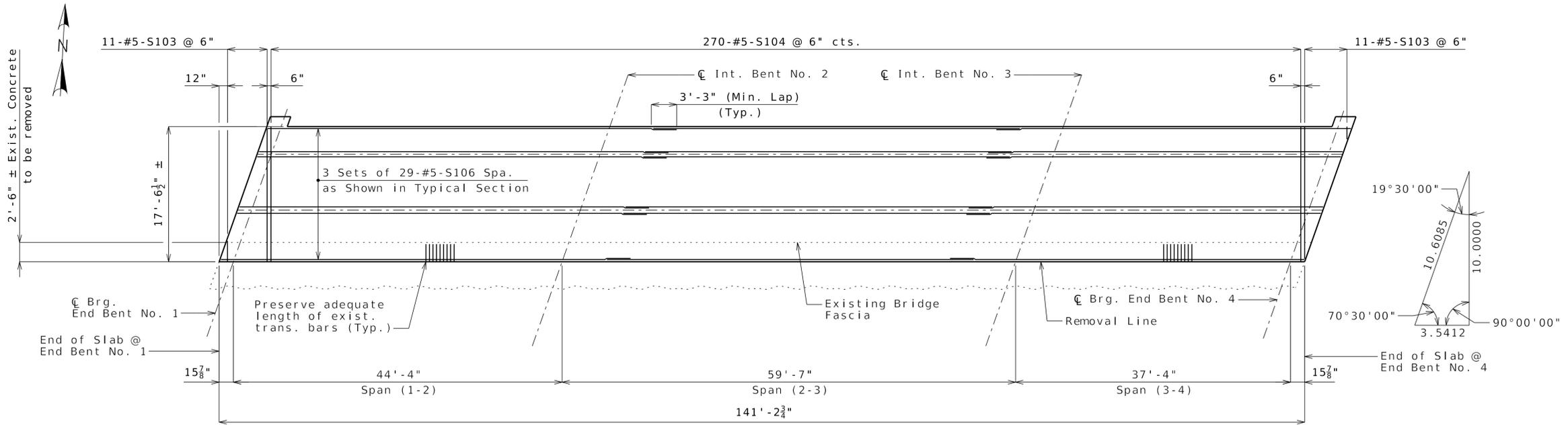


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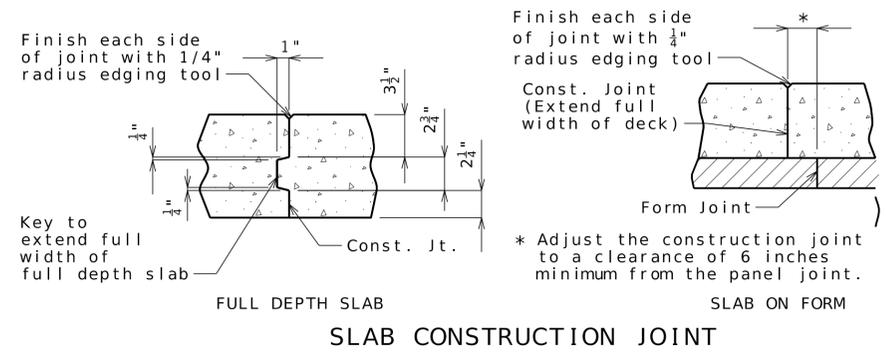
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TOP REINFORCING PLAN



BOTTOM REINFORCING PLAN



SLAB CONSTRUCTION JOINT

Notes:

Longitudinal slab dimensions are measured horizontally.

For Section Thru Slab and Slab Pouring Sequence, see Sheet No. 25.

For Details and Reinforcement of Type D Barrier not shown, see Sheet No. 26.

For Theoretical Slab Haunching Diagram, see Sheet No. 23.

For Theoretical Bottom of Slab Elevations, see Sheet No. 23.

For details and locations of Slab Drains, see Sheet No. 22.



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DISTRICT BR	SHEET NO. A43232-24
COUNTY ST. CHARLES	
JOB NO. JST0020	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A43232	

DATE	DESCRIPTION

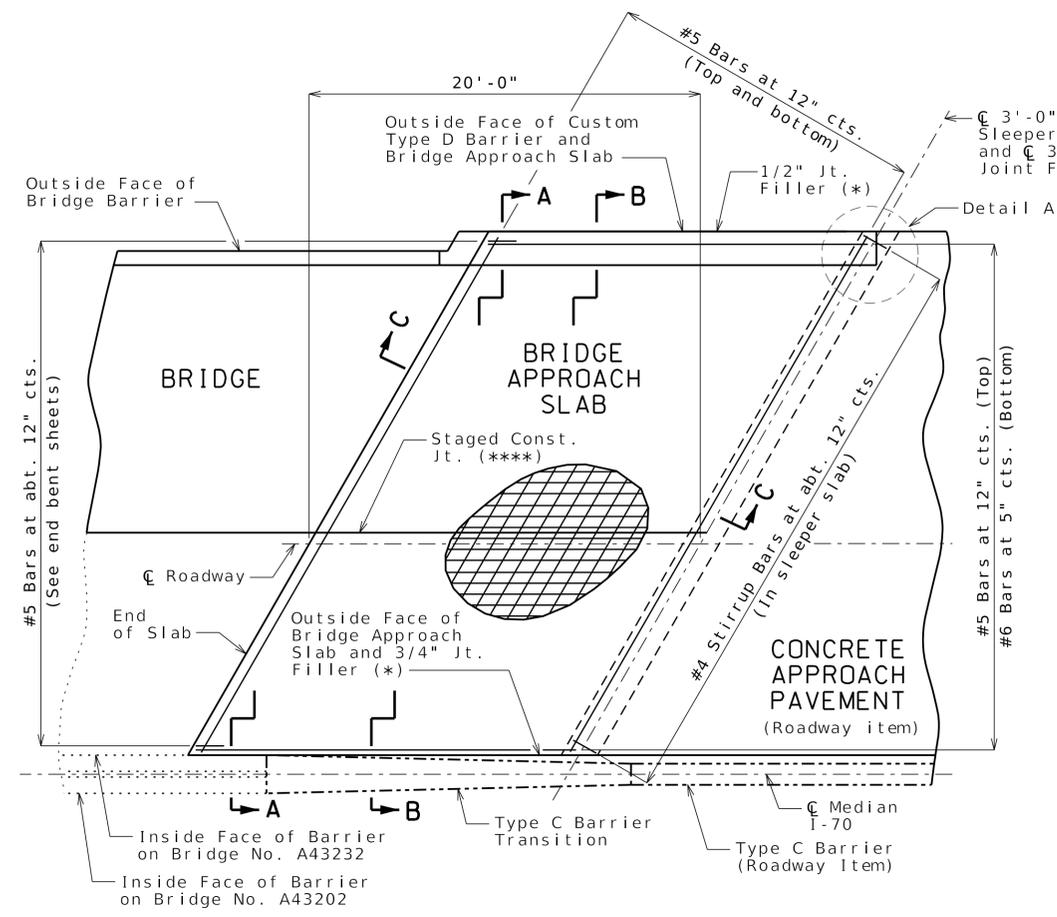
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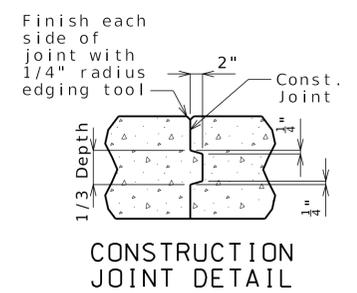


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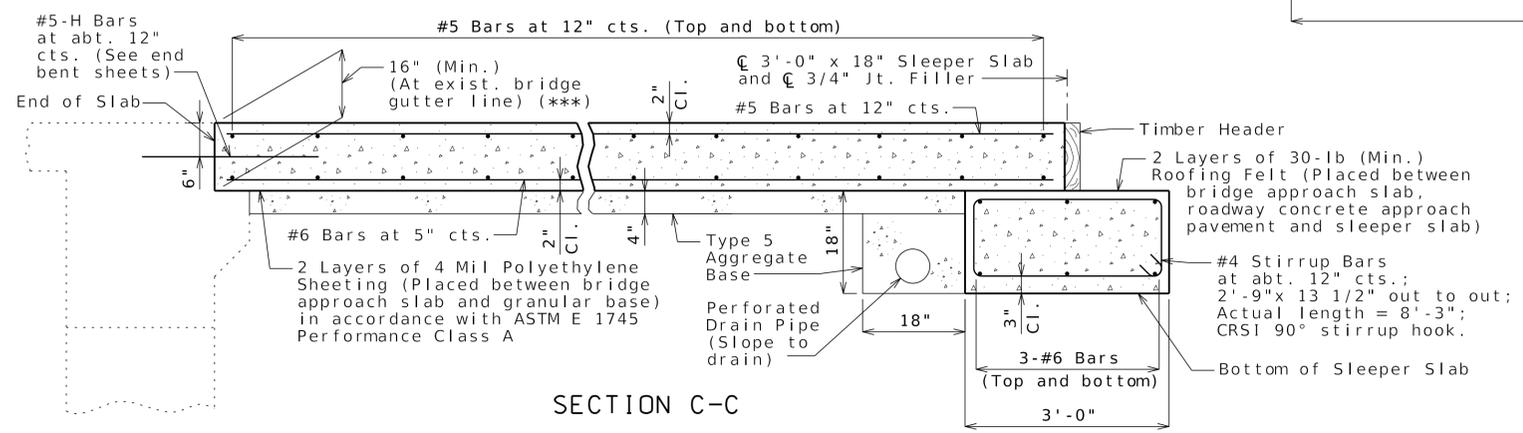
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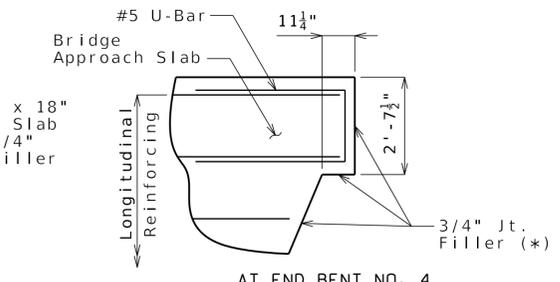
PART PLAN SHOWING REINFORCEMENT
Longitudinal reinforcement in sleeper slab not shown for clarity.
(End Bent No. 4 shown, End Bent No. 1 similar, U.N.O.)



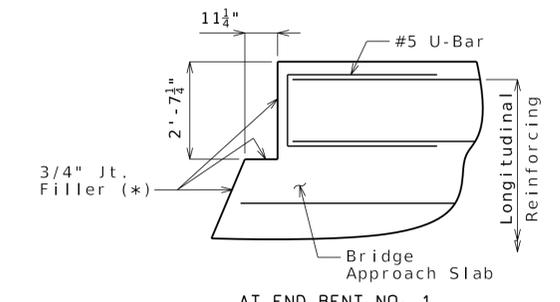
CONSTRUCTION JOINT DETAIL



SECTION C-C



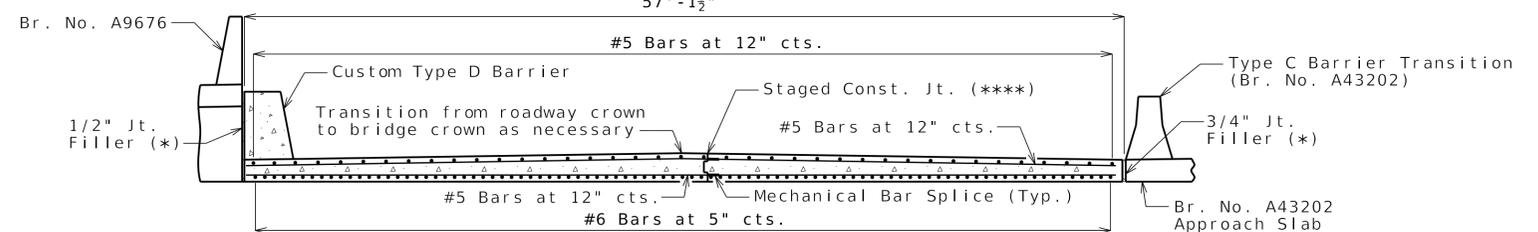
AT END BENT NO. 4



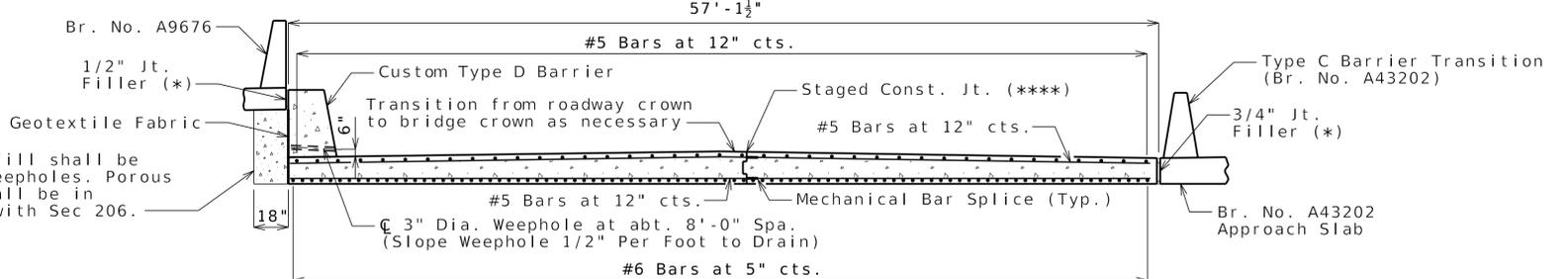
AT END BENT NO. 1

DETAIL A

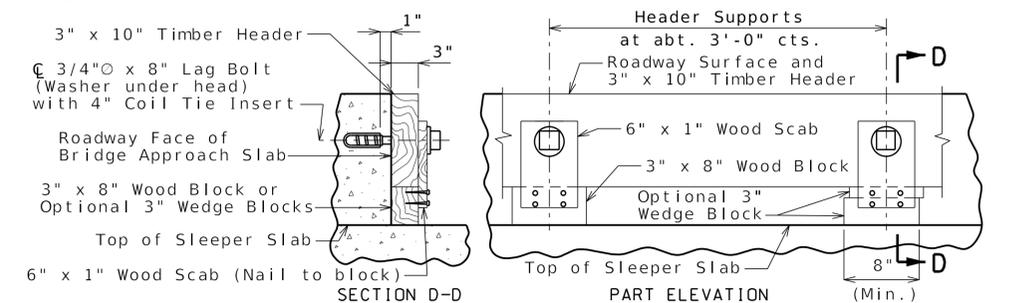
All reinforcing shown top and bottom. Transverse Reinforcing and Concrete Appr. Pavement not shown for clarity.



SECTION A-A
57'-1 1/2"



SECTION B-B



DETAILS OF TIMBER HEADER

Remove timber header when concrete pavement is placed.

BRIDGE APPROACH SLAB (MAJOR)

General Notes:

Existing bridge approach slab to be completely removed (Roadway Item).

All concrete for the bridge approach slab and sleeper slab shall be in accordance with Sec 503 ($f'c = 4,000$ psi).

The reinforcing steel in the bridge approach slab and the sleeper slab shall be epoxy coated Grade 60 with $f_y = 60,000$ psi.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab and the sleeper slab shall be continuous. The transverse reinforcing steel may be made continuous by lap splicing the #5 bars 29" or by mechanical bar splice.

Mechanical bar splices shall be in accordance with Sec 710. Estimated 38 splices per slab.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

Longitudinal construction joints in approach slab and sleeper slab shall be aligned with longitudinal construction joints in bridge slab.

For concrete approach pavement details, see roadway plans.

For Type C Barrier Transition Details, see Bridge A43202 Plans.

Coordinate construction of approach slab of Bridge No. A43232 with Bridge No. A43202.

(*) Seal joint between vertical faces of Bridge No. A43232 approach slab and wing or Bridge No. A43202 approach slab with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

(**) Contractor to field verify depth of existing approach slab notch.

(****) See roadway plans for traffic handling to place staged construction joint in each approach slab.

STATE OF MISSOURI
David J. Glasetter
NUMBER PE-200150018
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED: 9/2/2025

ROUTE: I-70 STATE: MO

DISTRICT: BR SHEET NO.: A43232-27

COUNTY: ST. CHARLES

JOB NO.: JST0020

CONTRACT ID.:

PROJECT NO.:

BRIDGE NO.: A43232

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

IMPROVE 70 ALLIANCE

HNTB

715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY NO. 001270

Bartlett & West

601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE: 872-003181
FAX: 872-003182
WWW.BARTLETTWEST.COM

Bill of Reinforcing Steel																		
No. Req.	Size/Mark	Location	Codes			Dimensions							Nom. Length ft in.	Actual Length ft in.	Weight lb			
			C	SH	V	B ft in.	C ft in.	D ft in.	E ft in.	F ft in.	H ft in.	K ft in.						
		End Bent No. 1																
4	6 H100	Beam	E	20		16	11.75						17	0	17	0	102	
8	8 H101	Beam	E	20		16	11.75						17	0	17	0	363	
2	6 H102	Diaphragm	E	20		6	9.75						6	10	6	10	21	
2	6 H103	Diaphragm	E	20		5	10.00						5	10	5	10	18	
2	6 H104	Diaphragm	E	20		3	10.25						3	10	3	10	12	
2	6 H105	Diaphragm	E	20		3	4.50						3	5	3	5	10	
2	6 H106	Diaphragm	E	20		4	6.75						4	7	4	7	14	
2	6 H107	Diaphragm	E	20		4	1.00						4	1	4	1	12	
2	5 H108	Strand Tie	E	23		15.13		17.00	15.13	5.00	14.25	5.00	14.25	3	11	3	11	8
4	6 H109	Diaphragm	E	20		16	11.75						17	0	17	0	102	
4	8 H110	Diaphragm	E	20		19	7.50						19	8	19	8	210	
12	7 H111	Diaphragm *	E	20		4	8.00						4	8	4	8	114	
8	7 H112	Beam *	E	20		6	5.00						6	5	6	5	105	
4	5 H113	Diaphragm	E	20		4	1.00						4	1	4	1	17	
40	5 H114	Diaphragm *	E	20		3	0.00						3	0	3	0	125	
4	6 H115	Diaphragm	E	6		5	1.00	11.00	2	3.00			8	3	7	11	48	
8	5 H116	Beam/Diaphragm	E	21		2	3.00	2	9.88	2	3.00						58	
14	5 U100	Beam	E	31S		4	6.50	2	11.00	4	6.50		12	11	12	8	185	
8	4 U101	Beam	E	10S		2	8.00	2	11.00				8	3	8	1	43	
23	6 U102	Diaphragm	E	19S		3	4.50	4	9.00				8	2	8	0	276	
14	5 U103	Diaphragm	E	31S		3	9.50	2	5.00	3	9.50		10	11	10	8	156	
14	6 U104	Diaphragm	E	19S		2	5.50	2	10.00				5	4	5	2	109	
17	5 U105	Diaphragm	E	19S		2	4.00	15.00					3	7	3	6	62	
8	6 V101	Diaphragm	E	20		2	5.50						2	6	2	6	30	
		End Bent No. 4																
4	6 H400	Beam	E	20		16	11.75						17	0	17	0	102	
8	8 H401	Beam	E	20		16	11.75						17	0	17	0	363	
2	6 H402	Diaphragm	E	20		6	9.75						6	10	6	10	21	
2	6 H403	Diaphragm	E	20		5	10.00						5	10	5	10	18	
2	6 H404	Diaphragm	E	20		3	10.25						3	10	3	10	12	
2	6 H405	Diaphragm	E	20		3	4.50						3	5	3	5	10	
2	6 H406	Diaphragm	E	20		4	6.75						4	7	4	7	14	
2	6 H407	Diaphragm	E	20		4	1.00						4	1	4	1	12	
2	5 H408	Strand Tie	E	23		15.13		17.00	15.13	5.00	14.25	5.00	14.25	3	11	3	11	8
4	6 H409	Diaphragm	E	20		16	11.75						17	0	17	0	102	
4	8 H410	Diaphragm	E	20		19	7.50						19	8	19	8	210	
12	7 H411	Diaphragm *	E	20		4	8.00						4	8	4	8	114	
8	7 H412	Beam *	E	20		6	5.00						6	5	6	5	105	
4	5 H413	Diaphragm	E	20		4	1.00						4	1	4	1	17	
40	5 H414	Diaphragm *	E	20		3	0.00						3	0	3	0	125	
4	6 H415	Diaphragm	E	6		5	1.00	11.00	2	3.00			8	3	7	11	48	
8	5 H416	Beam/Diaphragm	E	21		2	3.00	2	9.88	2	3.00						58	
14	5 U400	Beam	E	31S		4	6.50	2	11.00	4	6.50		12	11	12	8	185	
8	4 U401	Beam	E	10S		2	8.00	2	11.00				8	3	8	1	43	
23	6 U402	Diaphragm	E	19S		3	4.50	4	9.00				8	2	8	0	276	
14	5 U403	Diaphragm	E	31S		3	9.50	2	5.00	3	9.50		10	11	10	8	156	
14	6 U404	Diaphragm	E	19S		2	5.50	2	10.00				5	4	5	2	109	
17	5 U405	Diaphragm	E	19S		2	4.00	15.00					3	7	3	6	62	
8	6 V401	Diaphragm	E	20		2	5.50						2	6	2	6	30	

Bill of Reinforcing Steel																	
No. Req.	Size/Mark	Location	Codes			Dimensions							Nom. Length ft in.	Actual Length ft in.	Weight lb		
			C	SH	V	B ft in.	C ft in.	D ft in.	E ft in.	F ft in.	H ft in.	K ft in.					
		Int. Bent No. 2															
5	6 D200	Key	E	20		2	6.00										
8	8 H200	Beam	E	18		11	8.00										288
6	6 H201	Beam	E	20		11	8.00										105
10	6 H202	Beam	E	20		11	8.00										175
14	6 H203	Beam	E	10				12.00	3	1.75							102
16	4 P200	Column	E	16		7	10.25	2	10.00								113
24	4 P201	Drilled Shaft		16		7	10.25	2	10.00								170
18	5 U200	Beam	E	13S		3	3.00	3	9.00	3	3.00	3	9.00				274
4	5 U201	Beam	E	10S				3	9.00	3	3.00						44
13	11 V200	Column	E	17		17	9.00										1335
13	11 V201	Drilled Shaft		20		23	5.38										1617
		Int. Bent No. 3															
5	6 D300	Key	E	20		2	6.00										19
8	8 H300	Beam	E	18		11	8.00										288
6	6 H301	Beam	E	20		11	8.00										105
10	6 H302	Beam	E	20		11	8.00										175
14	6 H303	Beam	E	10				12.00	3	1.75							102
16	4 P300	Column	E	16		7	10.25	2	10.00								113
29	4 P301	Drilled Shaft		16		7	10.25	2	10.00								205
18	5 U300	Beam	E	13S		3	3.00	3	9.00	3	3.00	3	9.00				274
4	5 U301	Beam	E	10S				3	9.00	3	3.00						44
13	11 V300	Column	E	17		17	9.00										1335
13	11 V301	Drilled Shaft		20		26	6.00										1830

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

Codes: C = Required coatings, where E = Epoxy Coated and G = Galvanized.
SH = Required shape, see bending diagrams.
V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

All bars shall be Grade 60.

* Resin Anchor System

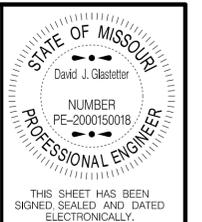
For bending diagrams and steel reinforcing totals, see Sheet No. 28.

Detailed JUL 2025
Checked JUL 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet 29 of 35

BILL OF REINFORCING STEEL (1 OF 2)



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.
DATE PREPARED: 9/2/2025
ROUTE: I-70 STATE: MO
DISTRICT: BR SHEET NO.: A43232-29
COUNTY: ST. CHARLES
JOB NO.: JST0020
CONTRACT ID.:
PROJECT NO.:
BRIDGE NO.: A43232

