

Estimated	Quantit	es		
I t em		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	80	-	80
Removal of Bridges (X0142)	lump sum	-	-	1
Bridge Approach Slab (Minor)	sq. yard	-	109	109
Drilled Shaft (4ft. Oin. Dia.)	linear foot	80.0	-	80.0
Rock Sockets (3ft. 6in. Dia.)	linear foot	98.0	-	98.0
Video Camera Inspection	each	4	-	4
Foundation Inspection Holes	linear foot	138.0	-	138.0
Sonic Logging Test	each	4	-	4
Galvanized Structural Steel Piles (12 in.)	linear foot	372	-	372
Dynamic Pile Testing	each	2	-	2
Pre-Bore for Piling	linear foot	190	-	190
Pile Point Reinforcement	each	8	-	8
Class B Concrete (Substructure)	cu. yard	102.6	-	102.6
Type D Barrier	linear foot	-	607	607
Slab on Concrete NU-Girder	sq. yard	-	837	837
NU-43 Prestressed Concrete NU-Girder	linear foot	-	840	840
Reinforcing Steel (Bridges)	pound	33,250	-	33,250
Vertical Drain at End Bent	each	-	-	2
Laminated Neoprene Bearing Pad	each	-	12	12
Laminated Neoprene Bearing Pad (Tapered)	each	-	6	6

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantitites for Slab on Concrete NU-Girder.

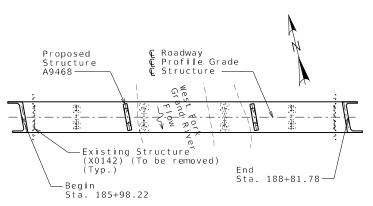
All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Load Bearing Pile: DT = Dynamic Testing

structure.

Drainage Area = 249 sq. mi. Design Flood Frequency = 50 yr. Design Flood Discharge = 18,500 cfs Design Flood (D.F.) Elevation = 924.3 ft. Base Flood (100-Year) Base Flood Elevation = 924.9 Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr Overtopping Flood Elevation = 922.7 ft.	Hydrologic Data
Design Flood Frequency = 50 yr. Design Flood Discharge = 18,500 cfs Design Flood (D.F.) Elevation = 924.3 ft. Base Flood (100-Year) Base Flood Elevation = 924.9 Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	
Design Flood Discharge = 18,500 cfs Design Flood (D.F.) Elevation = 924.3 ft. Base Flood (100-Year) Base Flood Elevation = 924.9 Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Drainage Area = 249 sq. mi.
Design Flood (D.F.) Elevation = 924.3 ft. Base Flood (100-Year) Base Flood Elevation = 924.9 Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Design Flood Frequency = 50 yr.
Base Flood (100-Year) Base Flood Elevation = 924.9 Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Design Flood Discharge = 18,500 cfs
Base Flood Elevation = 924.9 Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Design Flood (D.F.) Elevation = 924.3 ft.
Base Flood Discharge = 21,300 cfs. Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Base Flood (100-Year)
Estimated Backwater = 0.2 ft. Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Base Flood Elevation = 924.9
Average Velocity thru Opening = 4.1 ft./p Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Base Flood Discharge = 21,300 cfs.
Freeboard (50-year) Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Estimated Backwater = 0.2 ft.
Freeboard = 5.7 ft. Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Average Velocity thru Opening = 4.1 ft./p
Roadway Overtopping Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Freeboard (50-year)
Overtopping Flood Discharge = 12,800 cfs. Overtopping Flood Frequency = 12-yr	Freeboard = 5.7 ft.
Overtopping Flood Frequency = 12-yr	Roadway Overtopping
11 3 1 1	Overtopping Flood Discharge = 12,800 cfs.
Overtopping Flood Elevation = 922.7 ft.	Overtopping Flood Frequency = 12-yr
	Overtopping Flood Elevation = 922.7 ft.



LOCATION SKETCH

	Found	ation Dat	t a					
			Bent Number					
Type	Design Data	1	2	3	4			
	Pile Type and Size	HP 12x53	-	•	HP 12x53			
	Number ea	4	-	•	4			
	Approximate Length Per Each ft	49	-	-	44			
Load	Pile Point Reinforcement ea	AII	-	-	AII			
Bearing Pile	Min. Galvanized Penetration (Elev.) ft	Full Length	-	•	Full Length			
1116	Pile Driving Verification Method	DT	-	-	DT			
	Resistance Factor	0.65	-	-	0.65			
	Minimum Nominal Axial Compressive Resistance kip	358	-	_	341			
	Number ea	-	2	2	-			
	Foundation Material	-	Weak Rock	Weak Rock	-			
	Elevation Range ft	-	888 - 878	886-874	-			
	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf		4.0	4.0	-			
	Foundation Material	-	Strong Rock	Strong Rock	-			
Rock Socket	Elevation Range ft	-	878-855	874-861	-			
JUCKET	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf		12.9	12.9	-			
	Minimum Nominal Axial Compressive Resistance (Tip Resistance) ksf	-	34.0	39.2	-			

HP Piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

Detailed Oct. 2024 Note: This drawing is not to scale. Follow dimensions.

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads
Resistance Factors

Manufactured pile point reinforcement shall be used on all piles in this

Prebore for piles at Bent No. 1 to elevation 889.00.

Minimum Nominal Axial Compressive Resistance (Side Resistance + Top Resistance) = Maximum Factured Loads/Resistance Factors.

All piles shall be galvanized down to the minimum galvanized penetration

Thickness of permanent steel casing shall be in accordance with Sec 701.

Sheet No. 2 of 36

GENERAL NOTES AND QUANTITIES

General Notes: Design Specifications: 2020 AASHTO LRFD Bridge Design Specification (9th Ed) Seismic Design Category = A (Nonseismic) Design earthquake response spectral acceleration coefficient at 1.0 second Acceleration Coefficient (effective peak ground acceleration coefficient), Design Loading: Design Loading: Vehicular = HL-93 Future Wearing Surface = 35 lb/sf Earth = 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Superstructure: Simply-Supported, Non-Composite for dead load.

Continuous Composite for live load.	
Design Unit Stresses:	
Class B Concrete (Substructure, except Drilled Shaft	f'c = 3,000 psi
& Rock Sockets)	
	f'c = 4,000 psi
Class B-2 Concrete (Superstructure, except	flc = 4 000 nci
	f'c = 4,000 psi f'c = 4,000 psi
Reinforcing Steel (ASTM A615 Grade 60)	$f_V = 60,000 \text{ ps}$
Kernioleting Steel (ASIM A015 Grade 00)	ry — 00,000 psi

Reinforcing Steel (ASTM A615 Grade 60) Structural Steel HP Pile (ASTM A709 Grade 50) For NU-Girders, see Sheets No. 14 thru 19. fy = 50,000 psi

Neoprene Pads: Neoprene Bearing Pads shall be 60 durometer and shall be in accordance with Sec 716.

All joint filler shall be in accordance with Sec 1057 fo preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel: Minimum clearance to reinforcing steel shall be 11/2", unless otherwise shown

Structure to be closed to traffic during construction. Traffic to be maintained on other routes during construction. See Roadway plans for traffic control.

Estimated Ouantities for Slab on Concrete NU-Girder I t em Total Class B-2 Concrete cu. yard 262

The table of Estimated Quantities for NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit

Reinforcing Steel (Epoxy Coated)

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

JEFF A. GARDNER NUMBER PE-2016019369 11/26/2024

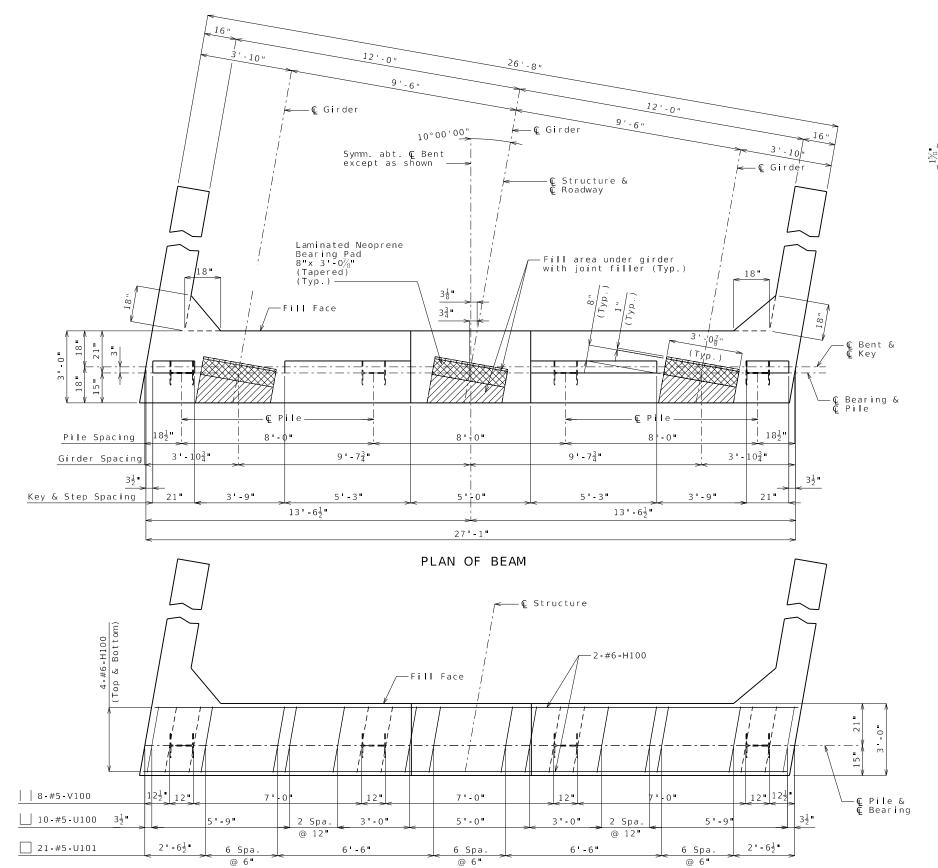
W MO SHEET NO BR 2

WORTH JNW0020

CONTRACT ID. PROJECT NO

A9468

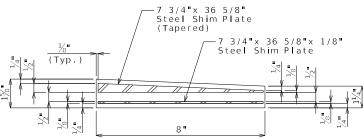




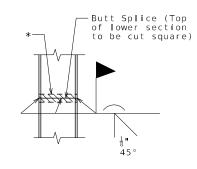
PLAN OF BEAM SHOWING REINFORCEMENT

Keys not shown for clarity

Ahead Station_



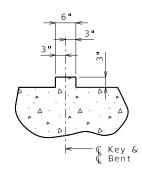
SECTION THRU LAMINATED NEOPRENE BEARING PAD



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY



DATE PREPARED					
11/26/2024					
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PROJECT NO





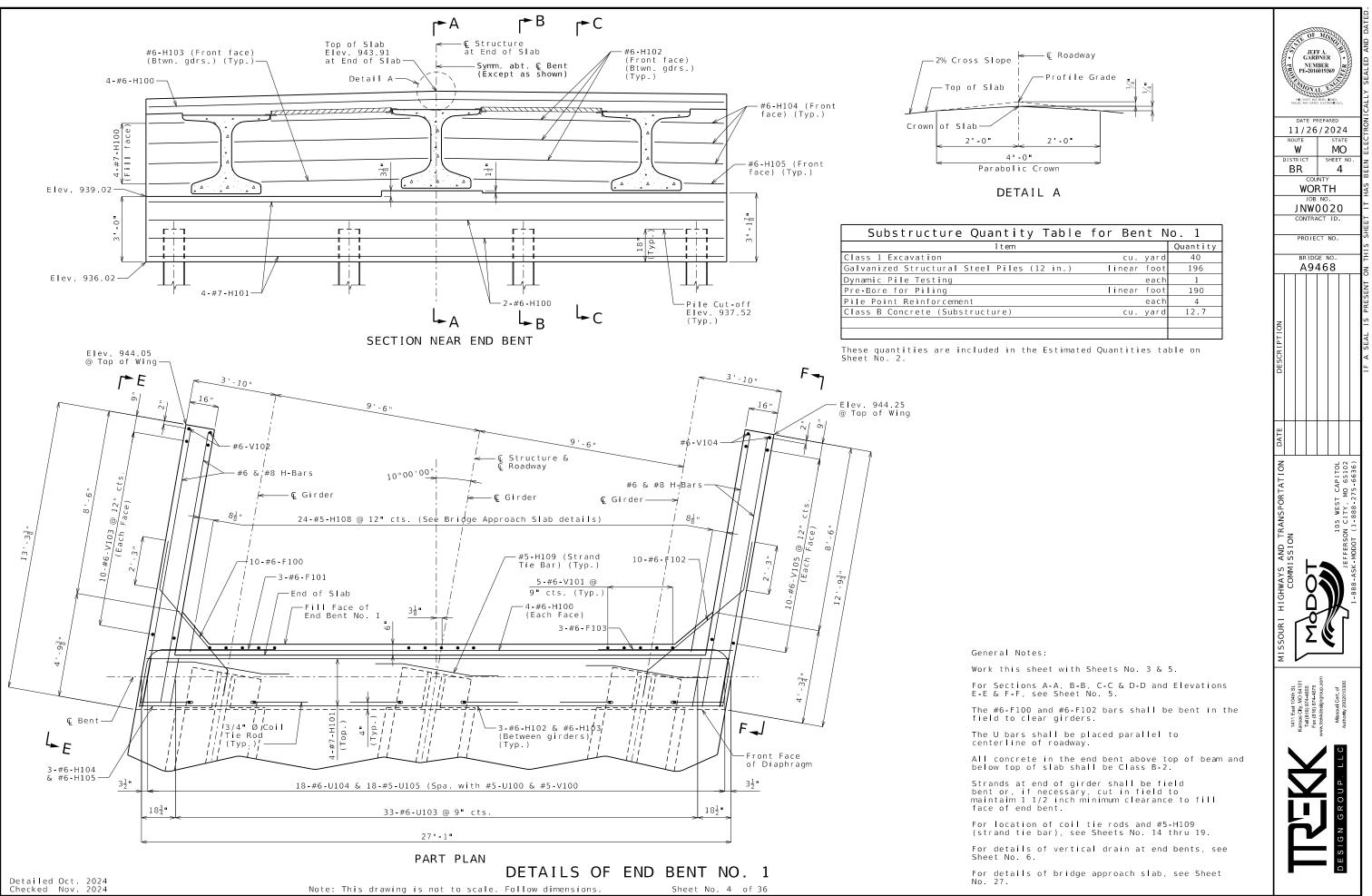


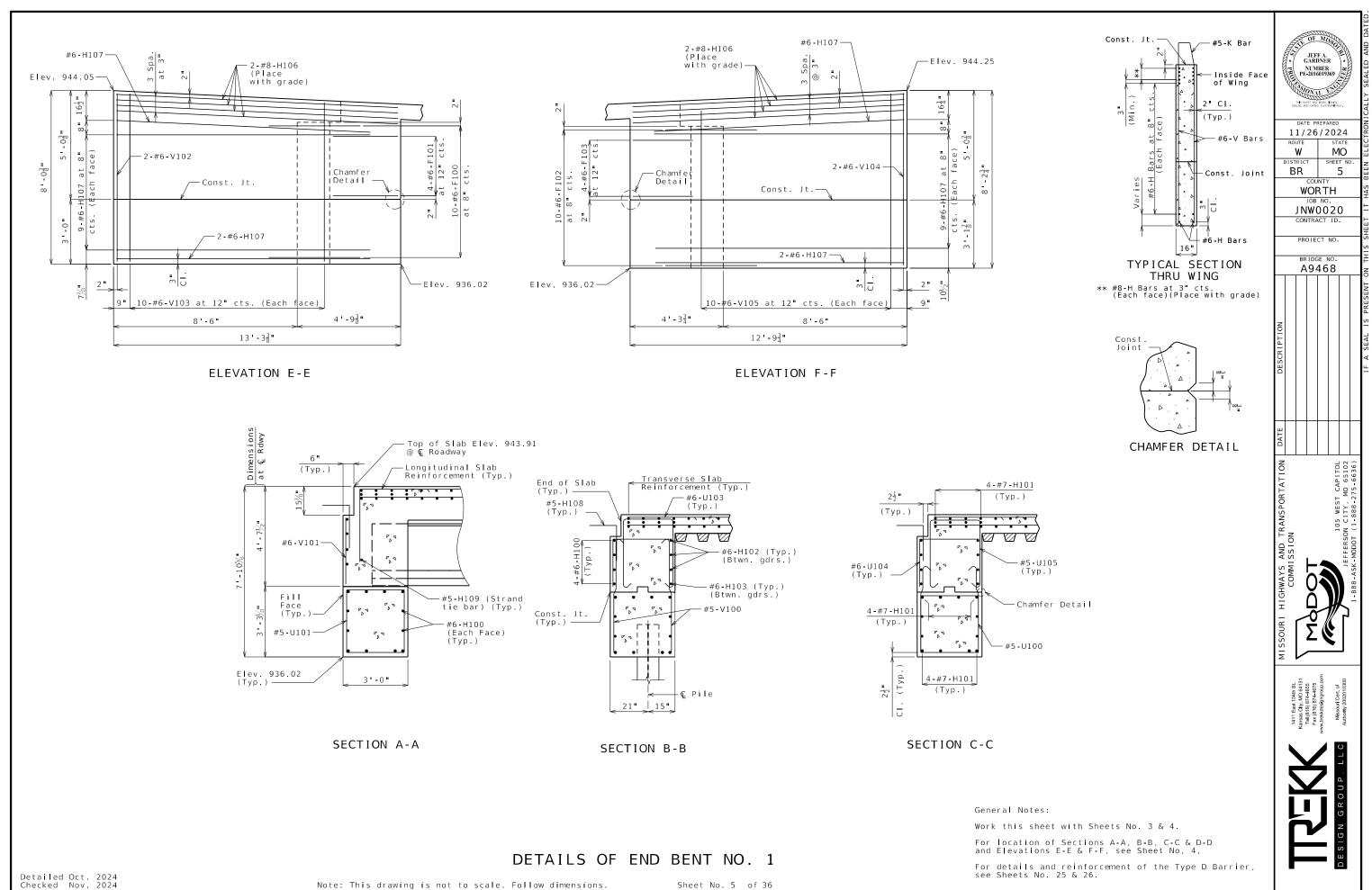
General Notes:

Work this sheet with Sheets No. 4 & 5.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".





ELEVATION OF WING

Unperforated

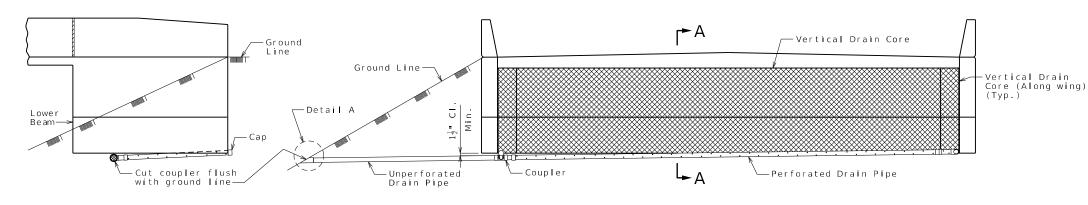
Cut coupler

to slope of ground line

Drain Pipe-

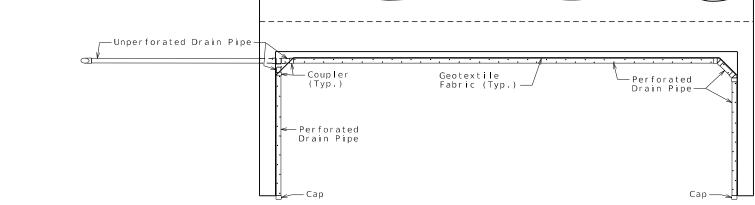
DETAIL A

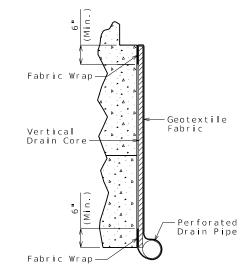
Rodent Screen-



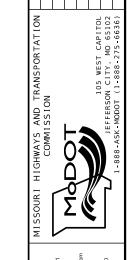
ELEVATION OF END BENT

PLAN OF END BENT





PART SECTION A-A (Section thru wing similar)



JEFF A. GARDNER

NUMBER E-2016019369

11/26/2024

WORTH

JNW0020

PROJECT NO.

BRIDGE NO.

MO SHEET NO

6

W

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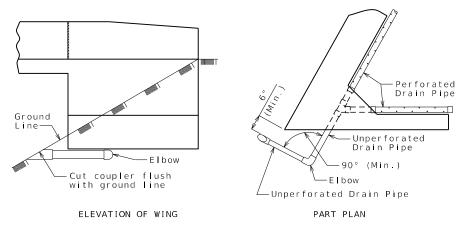
General Notes:

All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.



OPTIONAL TURNED DRAIN

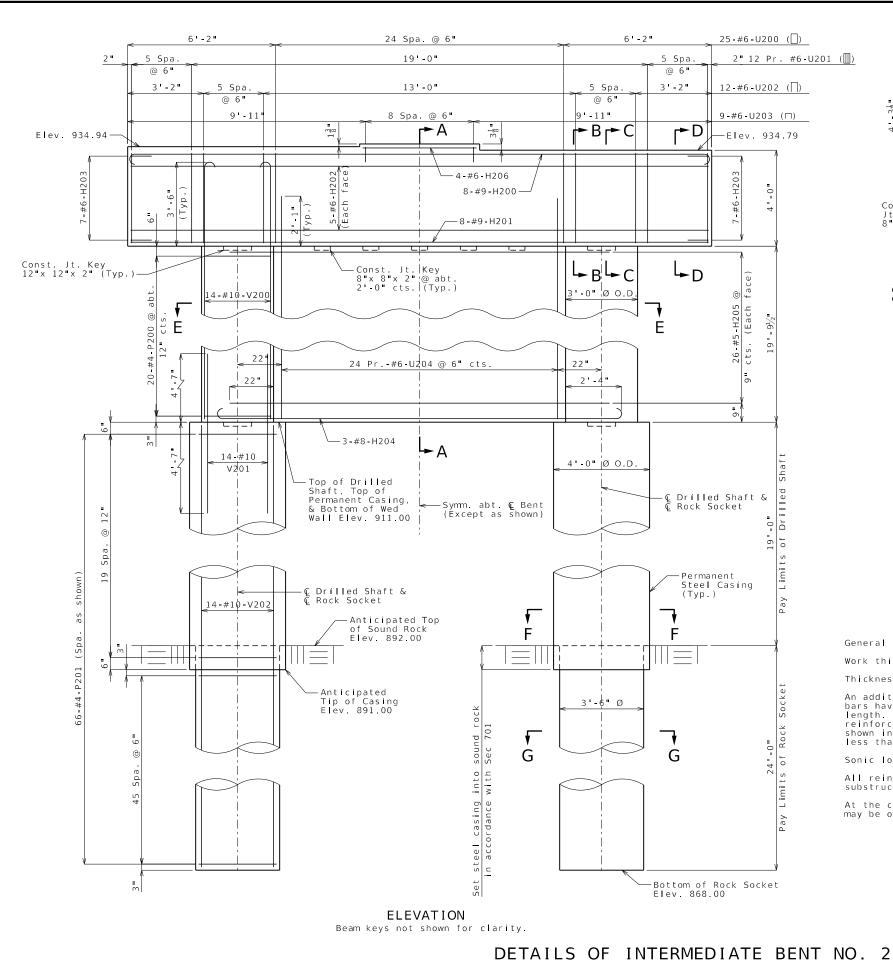
(Use only when straight drain is not practical.)

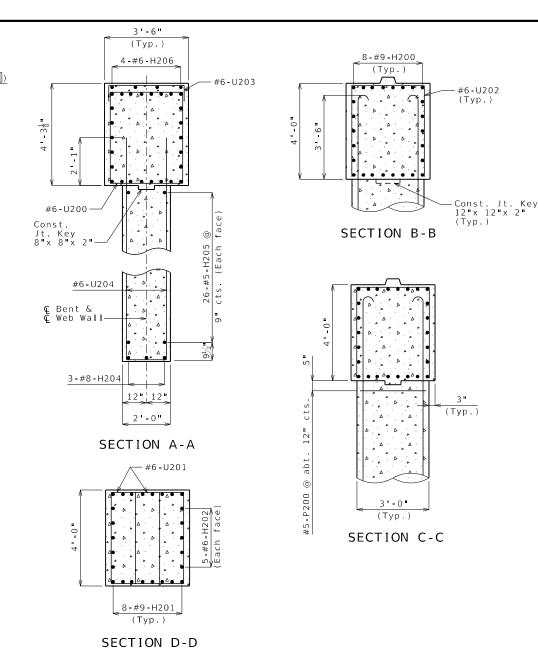
VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 6 of 36





General Notes:

Work this sheet with Sheet No. 8.

Thickness of permanent steel casing shall be in accordance with Sec 701.

An additional 4 feet has been added to V-bar lengths and additional 18-#4-P201 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

All reinforcement in drilled shafts and rock sockets is included in the substructure quantities.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

Substructure Quantity Table	e for Bent N	o. 2
I t em		Quantity
Drilled Shaft (4ft. Oin. Dia.)	linear foot	38
Rock Sockets (3ft. 6in. Dia.)	linear foot	48
Video Camera Inspection	each	2
Foundation Inspection Holes	linear foot	68
Sonic Logging Test	each	2
Class B Concrete (Substructure)	cu. yard	41.8
Reinforcing Steel (Bridges)	pound	16,940

These quantities are included in the Estimated Quantities table on Sheet No. 2.

JEFF A. GARDNER

NUMBER PE-2016019369

11/26/2024

WORTH

JNW0020

PROJECT NO

BRIDGE NO A9468

MO SHEET NO

7

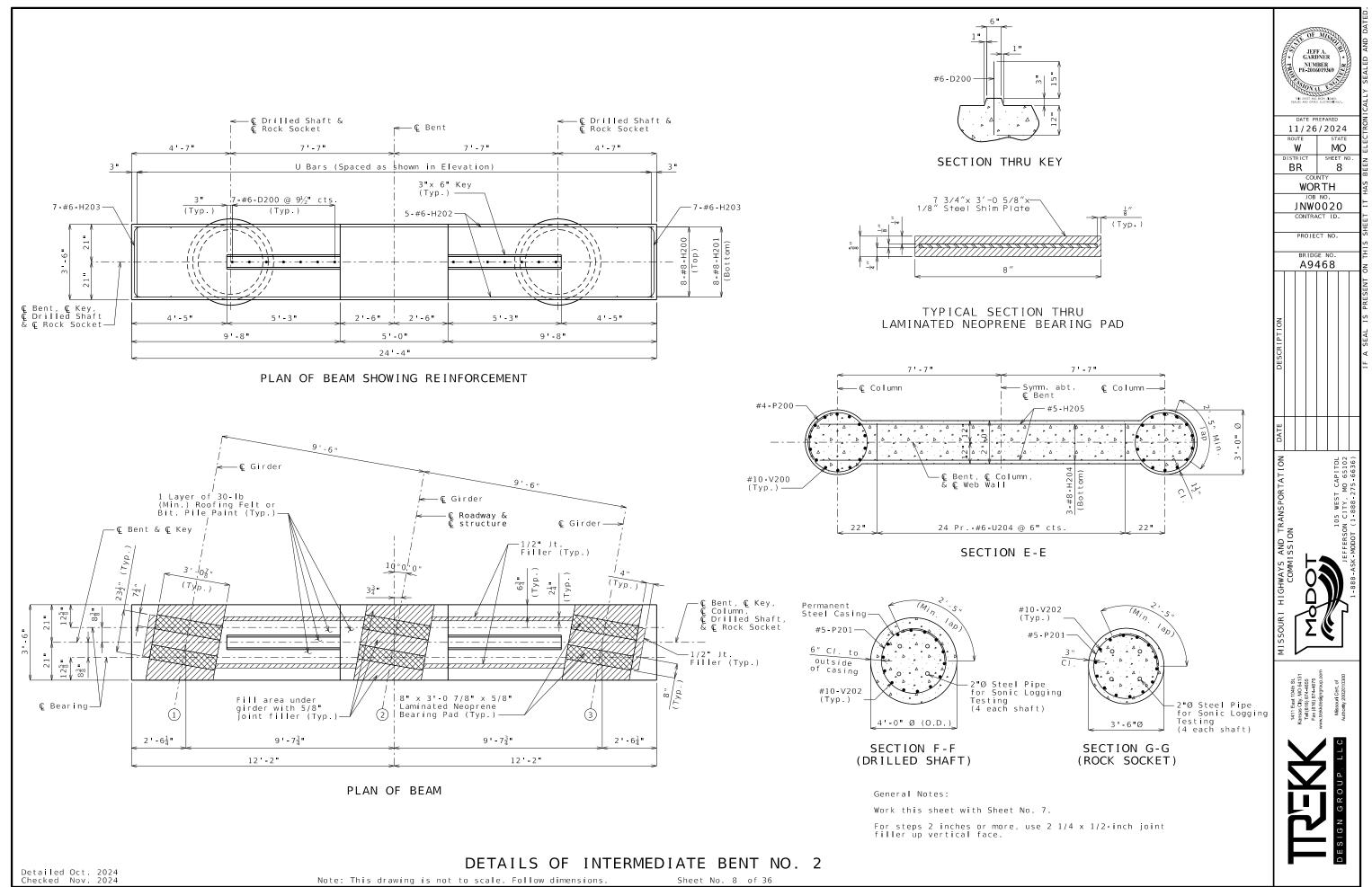
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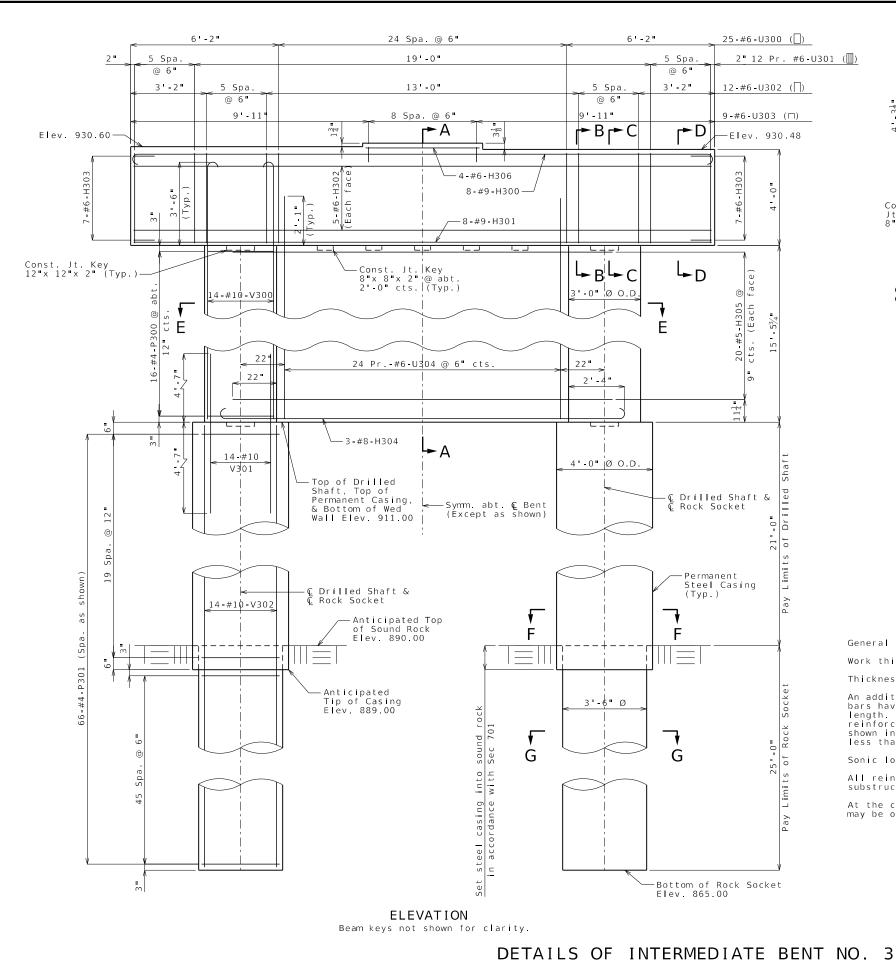
Detailed Oct. 2024 Checked Nov. 2024

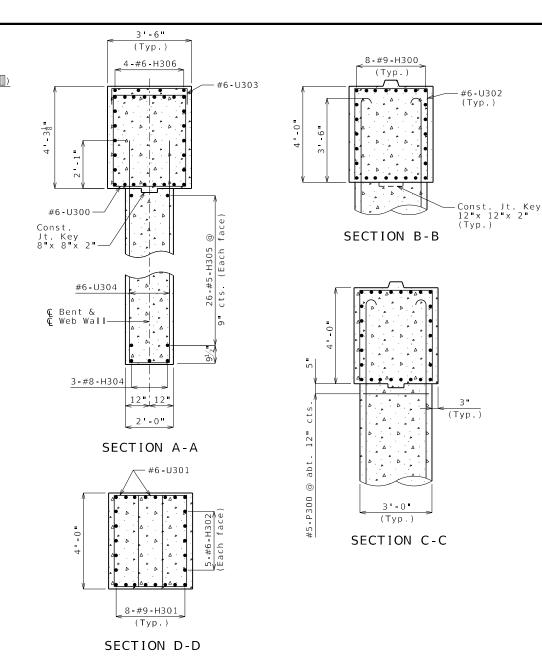
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 36



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General Notes:

Work this sheet with Sheet No. 10.

Thickness of permanent steel casing shall be in accordance with Sec 701.

An additional 4 feet has been added to V-bar lengths and additional 18-#4-P301 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

All reinforcement in drilled shafts and rock sockets is included in the substructure quantities.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

Substructure Quantity 7	Table for Bent N	o. 3
I t em		Quantity
Drilled Shaft (4ft. Oin. Dia.)	linear foot	42
Rock Sockets (3ft. 6in. Dia.)	linear foot	50
Video Camera Inspection	each	2
Foundation Inspection Holes	linear foot	70
Sonic Logging Test	each	2
Class B Concrete (Substructure)	cu. yard	35.5
Reinforcing Steel (Bridges)	pound	16,310

These quantities are included in the Estimated Quantities table on Sheet No. 2.

JEFF A. GARDNER

NUMBER PE-2016019369

11/26/2024

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JNW0020

PROJECT NO

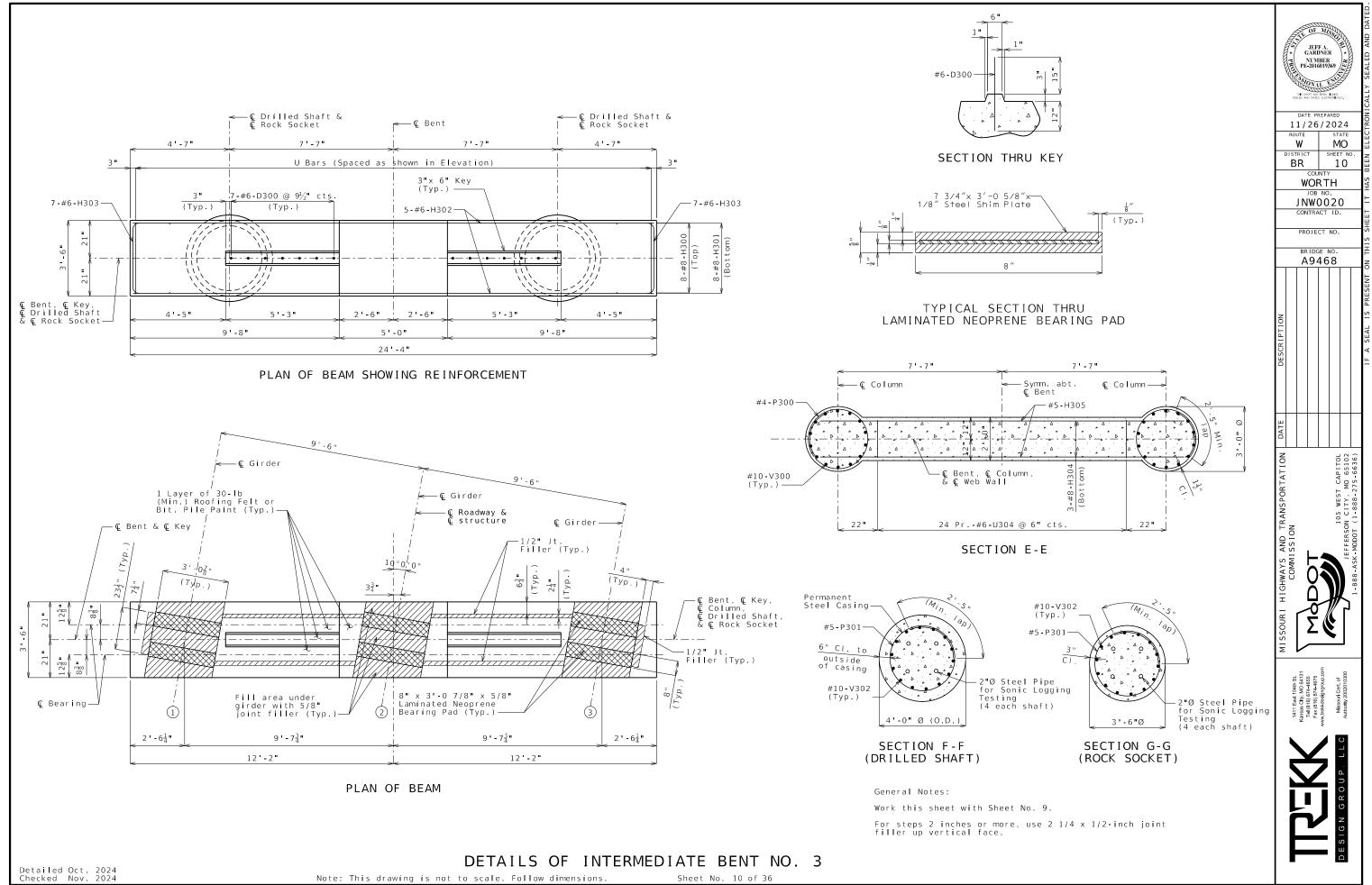
BRIDGE NO A9468

MO SHEET NO

9

W

BR



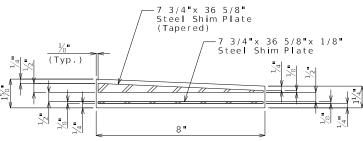
12'-0" −**Ç** Girder — Ç Girder Symm. abt. **©** Bent except as shown -**← €** Girder Ç Structure & Ç Roadway Laminated Neoprene Bearing Pad 8"x 3'-0%" - Fill area under girder with jt. filler (Typ.) (Tapered) (Typ.) — -Fill Face 33" 3'-107 -Ç Bent & Ç Key (Typ.) Bearing & 18¹/₂" 8 - 0 " Pile Spacing 8'-0" 8 ' # 0 " 3'-103" $3' - 10\frac{3}{4}"$ 9'-73" 9 ' **-** 7³ " Girder Spacing 5'-3" 5'-3" 3'-9" Key & Step Spacing 3'-9" 5 '¦0" 21" 13'-6¹/₂" 13'-6½" 27'-1" PLAN OF BEAM **– €** Structure 4-#6-H400 (Top & Bottom) 2-#6-H400 −Fill Face 8-#5-V400 12" 7'-0" 12" 12 | 12 | (E Bearing 10 - #5 - U400 $3\frac{1}{2}$ 31 " 5 ' - 0 **"** 3'-0" 2 Spa @ 12' @ 12' 21-#5-U401 $2 - 6\frac{1}{2}$ $2 - 6\frac{1}{2}$ 6'-6" 6'-6" 6 Spa. 6 Spa. 6 Spa @ 6" @ 6" @ 6"

PLAN OF BEAM SHOWING REINFORCEMENT Keys not shown for clarity

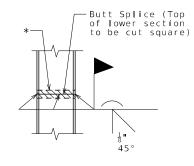
Detailed Oct. 2024 Checked Nov. 2024

DETAILS OF END BENT NO. 4 Note: This drawing is not to scale. Follow dimensions. Sheet No. 11 of 36

Ahead Station>



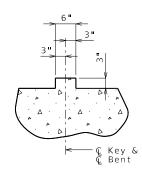
SECTION THRU LAMINATED NEOPRENE BEARING PAD



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY

General Notes:

Work this sheet with Sheets No. 12 & 13.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".



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COUNTY						
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BRIDGE NO.
A9468

SOURI HIGHWAYS AND TRANSPORTATI
COMMISSION

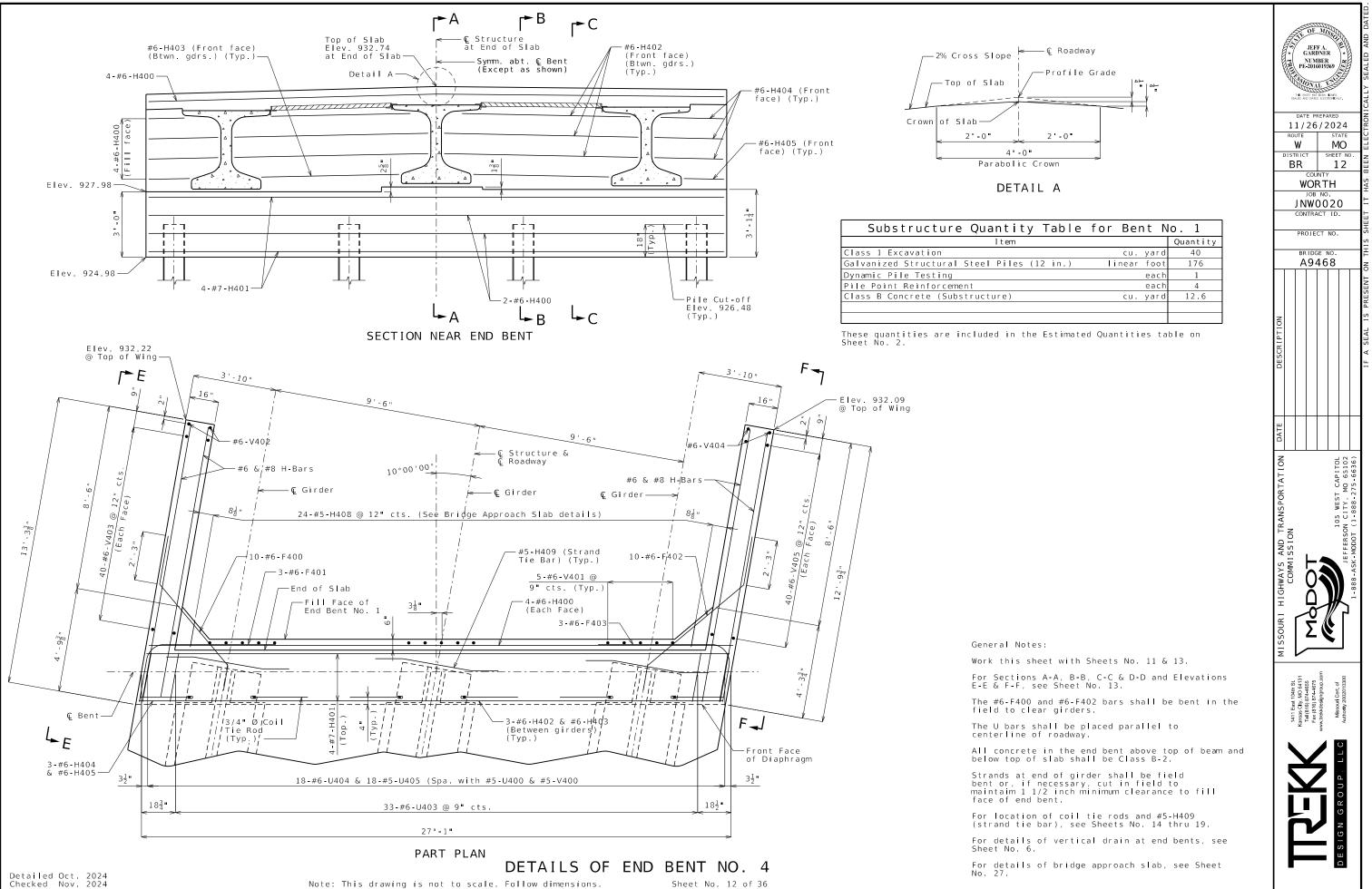
MADOT

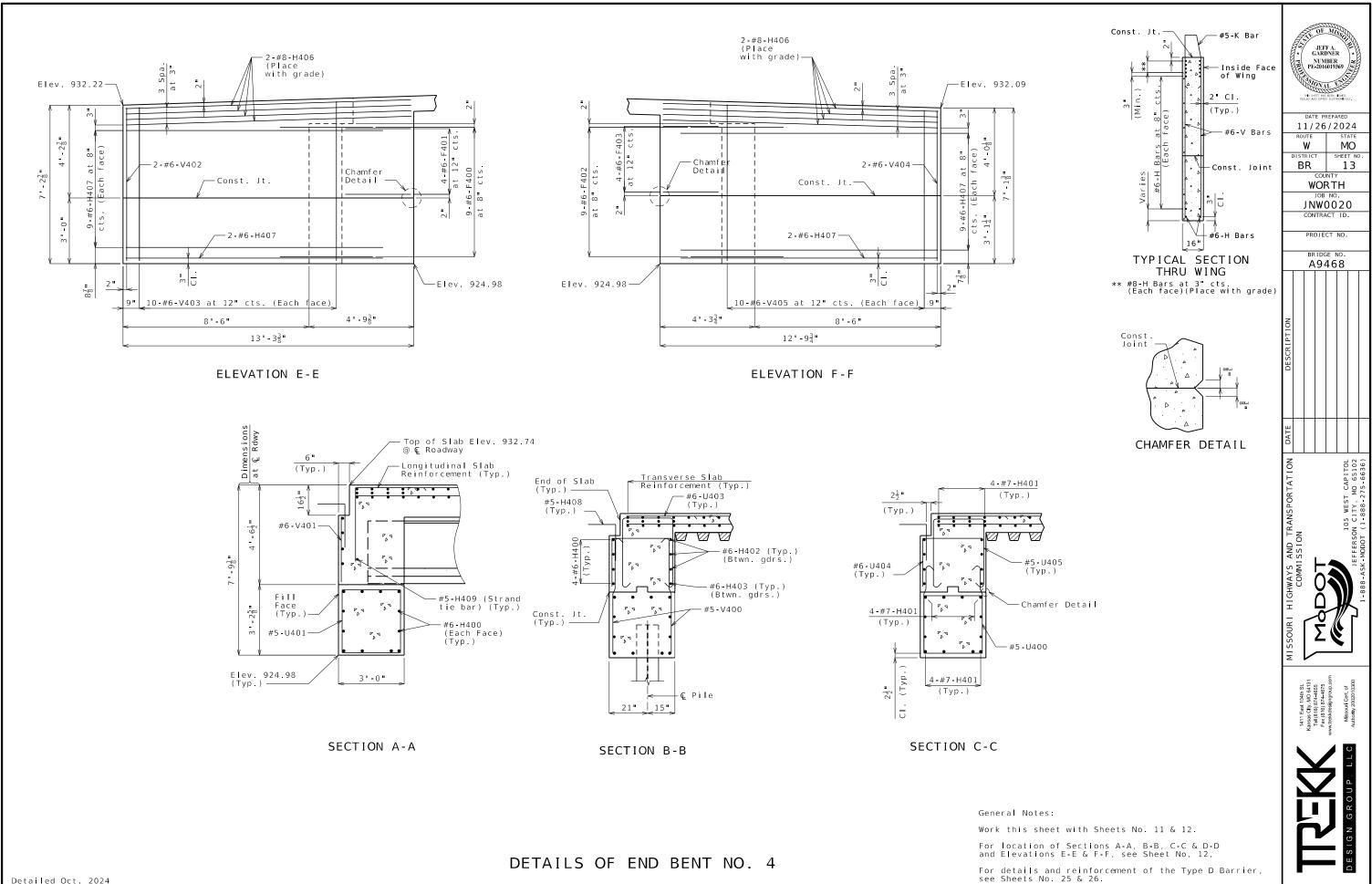
105 WEST CAPIT

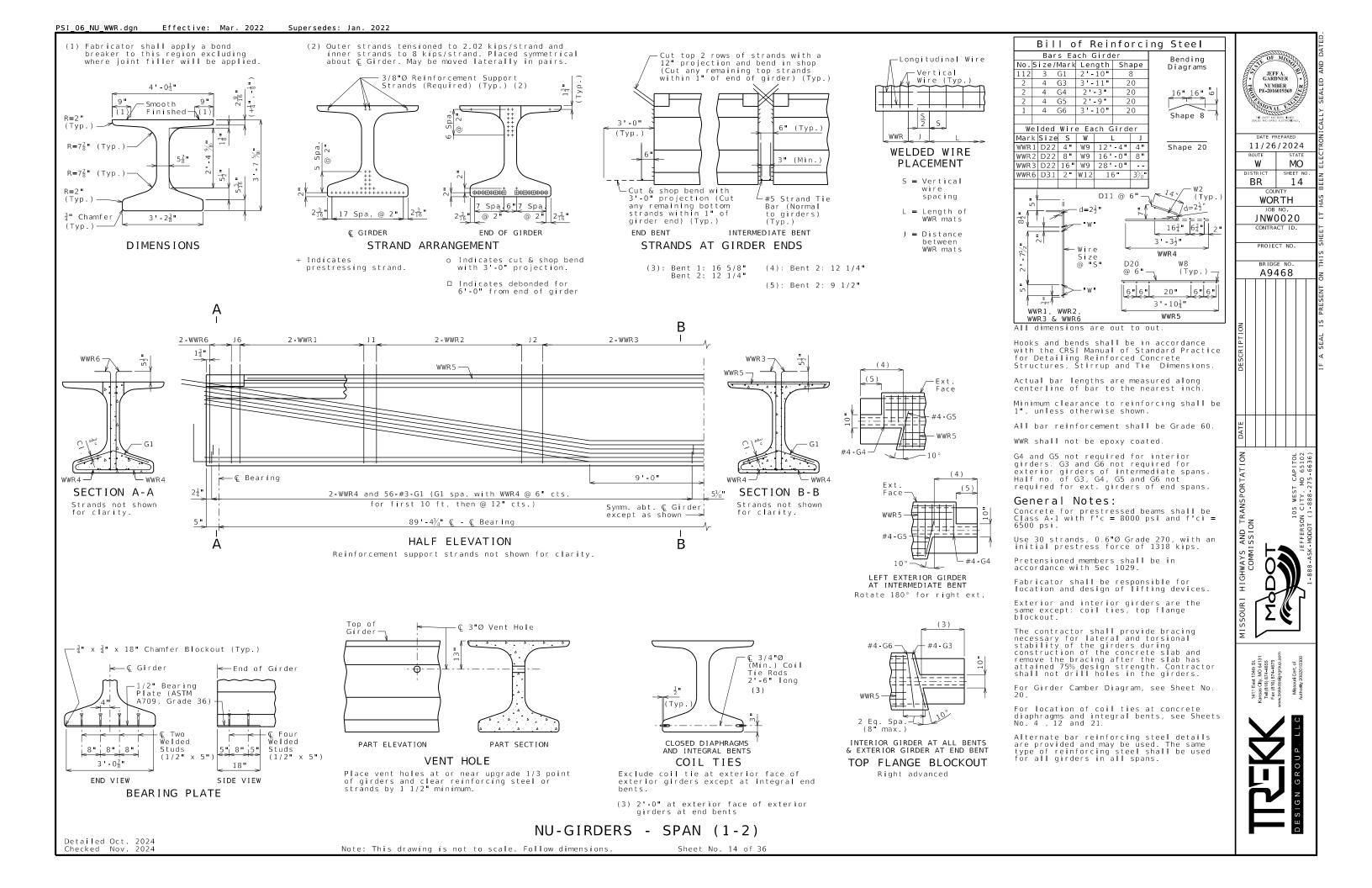
LEFERSON CITY, MO 651

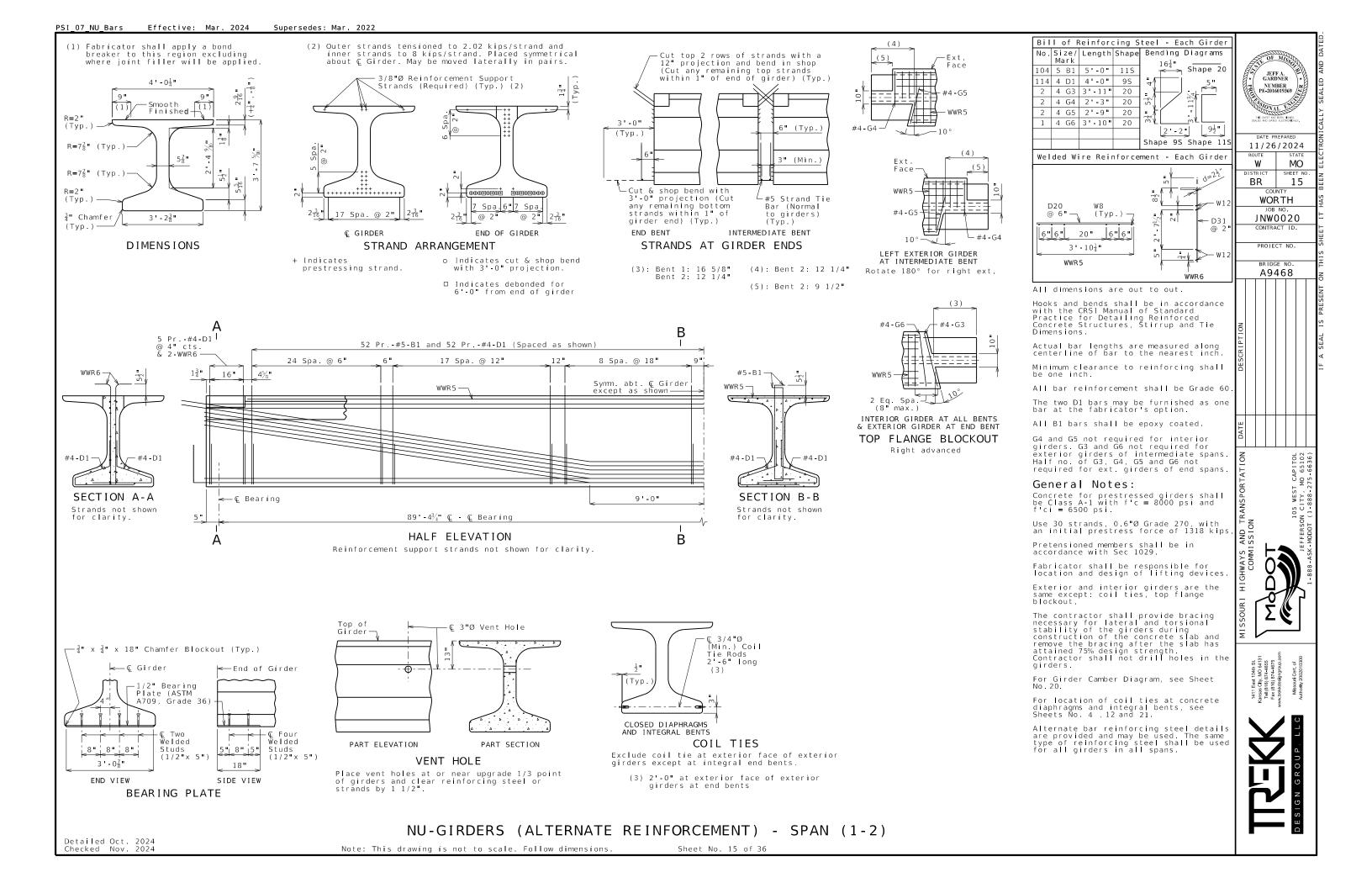
1411 East 104th St. Kansas City, MO 64131 Tel (816) 874-4655 Fax (316) 874-4675 ww.trekkdesigngroup.com Missouri Cert. of

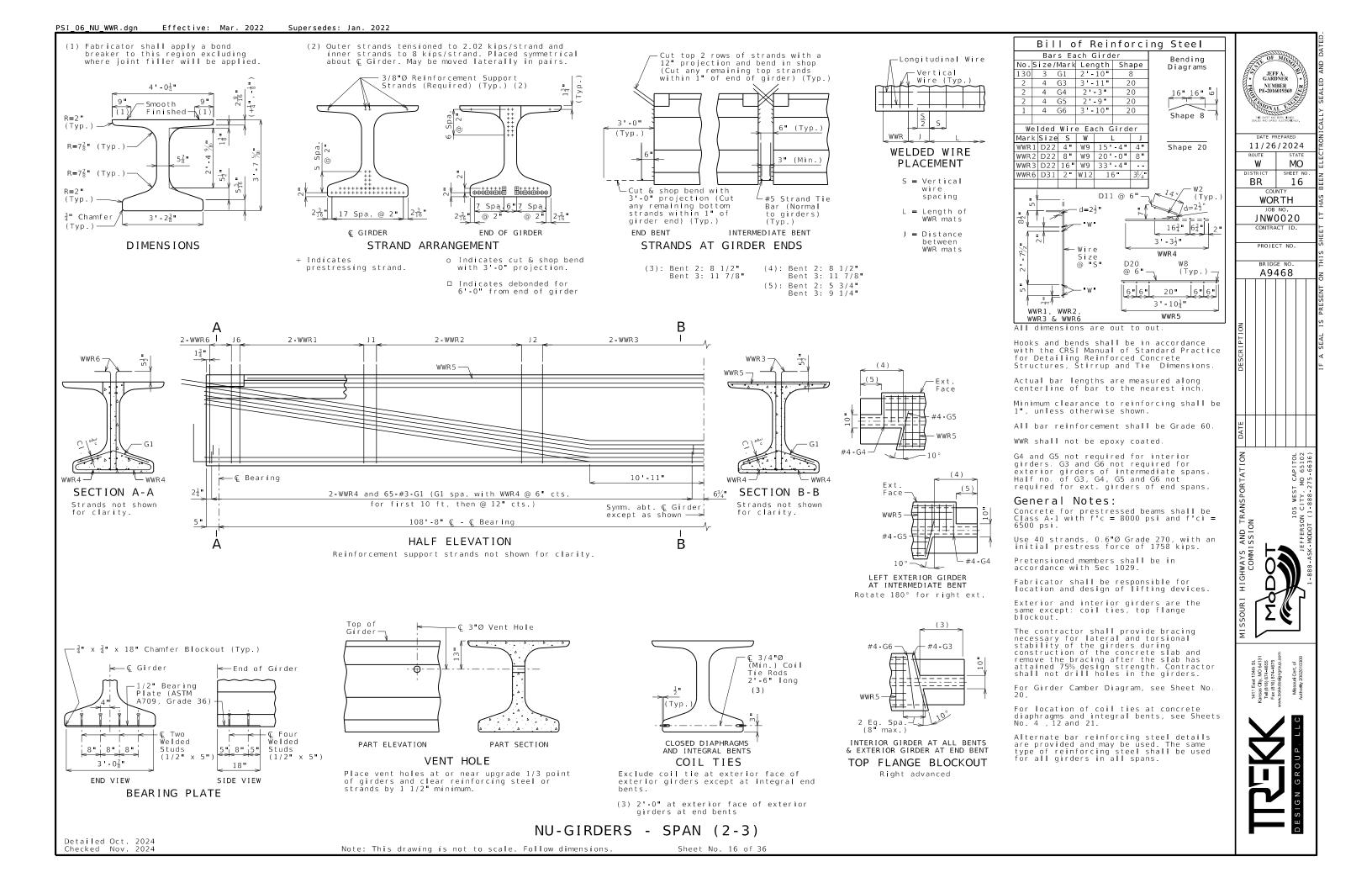


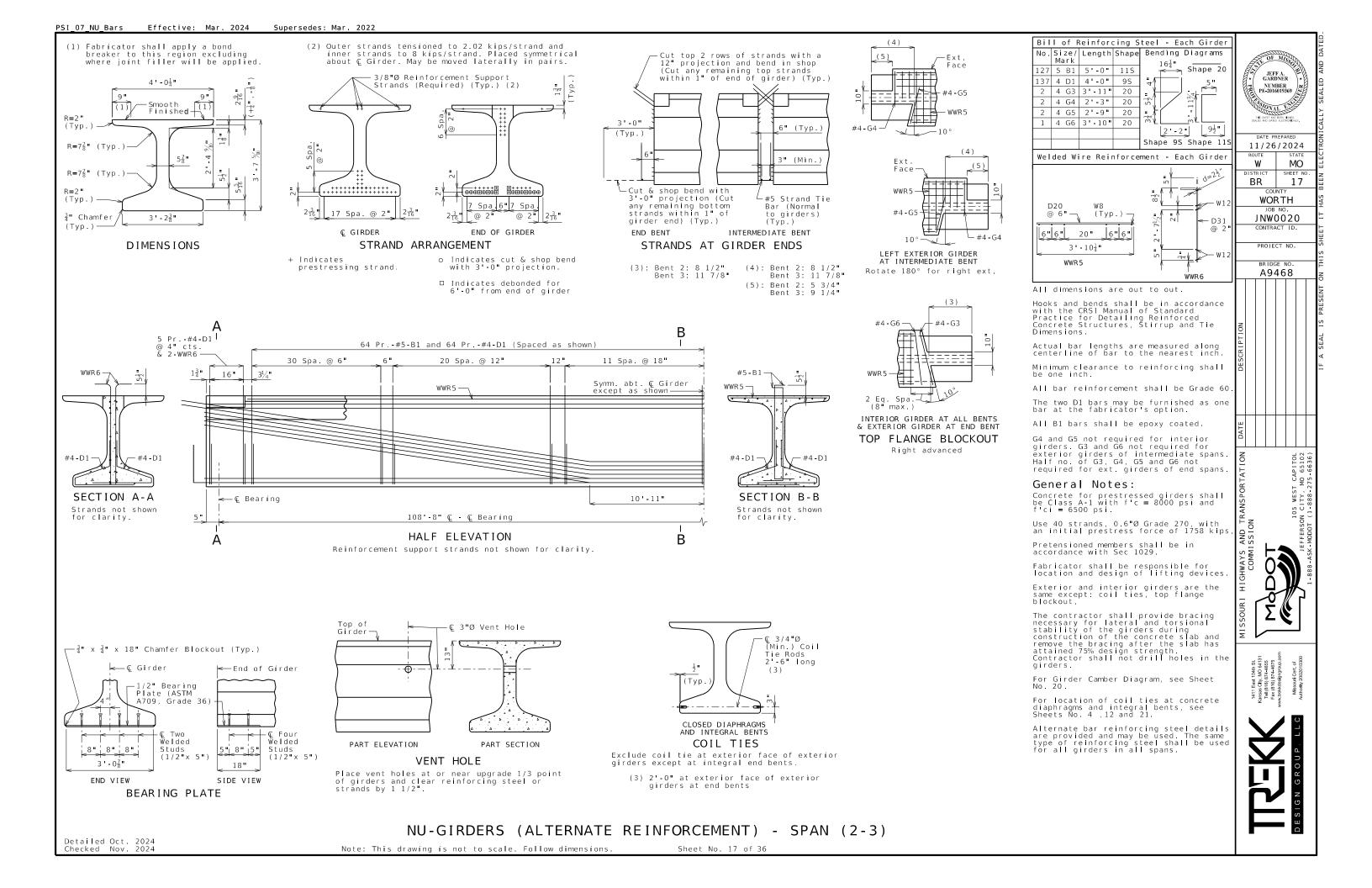


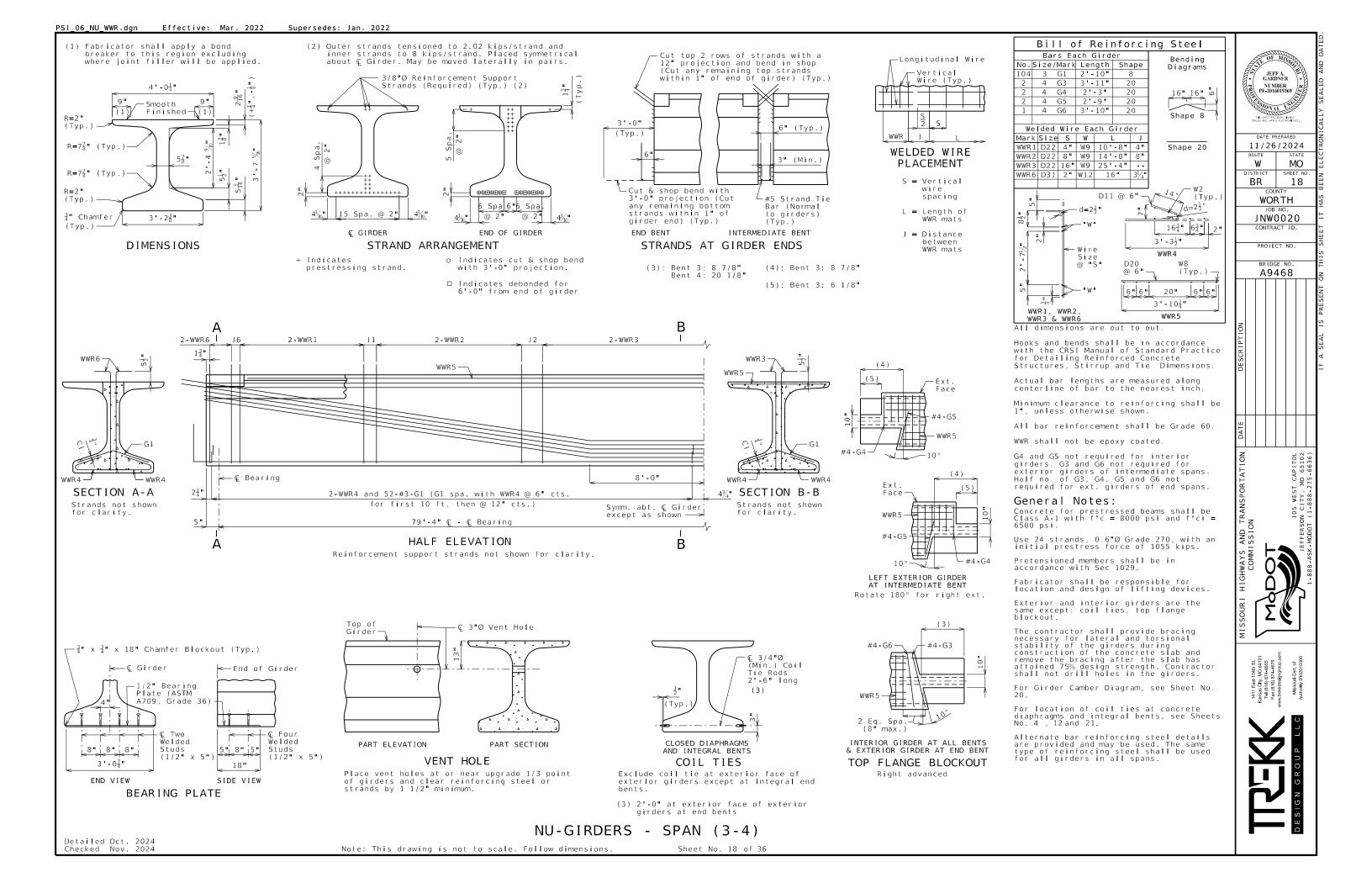


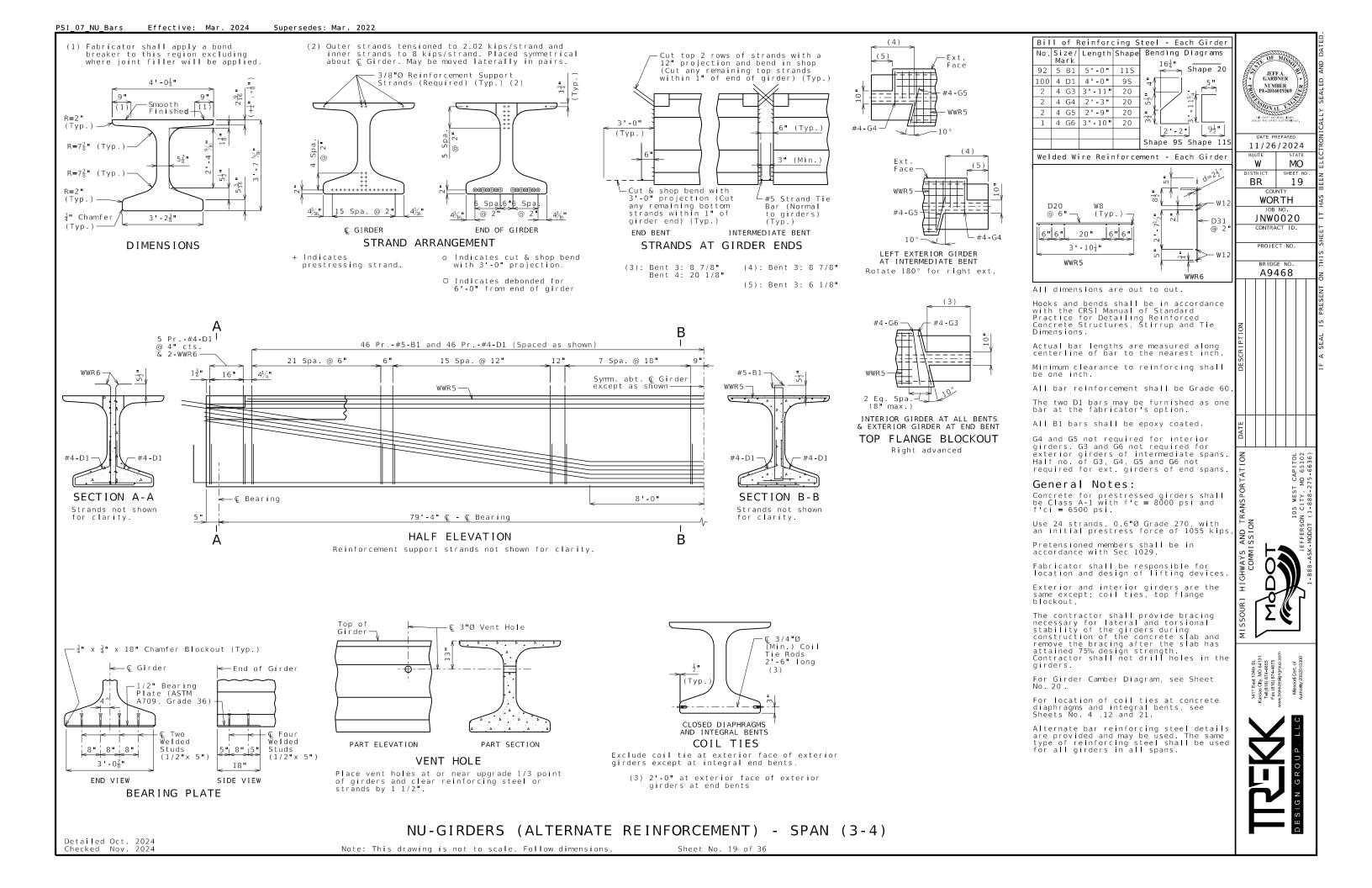


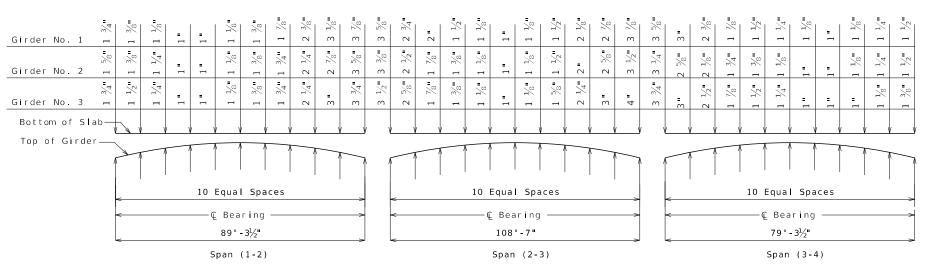


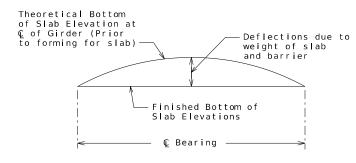












TYPICAL SLAB ELEVATIONS DIAGRAM

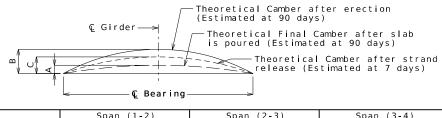
THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.

The	oretic (Pr						sat (stimat				der
Girder				Span (1-2) (89	'-3½" ©	Brg Ç	Brg.)			
Number	€ Brg	. 10	.20	. 30	. 40	. 50	.60	.70	.80	. 90	€ Brg.
1	942.89	942.47	942.05	941.63	941.22	940.81	940.40	940.00	939.61	939.22	938.84
2	943.14	942.72	942.30	941.88	941.46	941.05	940.65	940.25	939.85	939.46	939.08
3	943.05	942.63	942.21	941.79	941.37	940.96	940.55	940.15	939.76	939.37	938.98
Girder				Span (2-3) (10	8'-7" ©	Brg - ©	Brg.)			
Number	€ Brg	. 10	.20	. 30	. 40	. 50	.60	.70	.80	. 90	€ Brg.
1	938.81	938.35	937.90	937.45	937.01	936.58	936.15	935.73	935.32	934.92	934.52
2	939.06	938.59	938.14	937.69	937.25	936.81	936.38	935.96	935.55	935.15	934.75
3	938.96	938.49	938.04	937.59	937.14	936.71	936.28	935.86	935.44	935.04	934.64
Girder				Span (3-4) (79	3½ C	Brg €	Brg.)			
Number	€ Brg	. 10	.20	. 30	. 40	. 50	.60	.70	.80	. 90	€ Brg.
1	934.50	934.22	933.94	933.66	933.39	933.12	932.85	932.59	932.33	932.08	931.83
2	934.73	934.45	934.16	933.89	933.61	933.34	933.08	932.82	932.56	932.30	932.05
3	934.62	934.34	934.05	933.77	933.50	933.23	932.96	932.70	932.44	932.19	931.93

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



Girder	S	pan (1-2	2)	S	pan (2-3	3)	S	pan (3-4	4)
Girder	Α	В	С	Α	В	С	Α	В	С
Exterior	1"	2½"	13/8"	15/8"	43/4"	13/4"	3/4"	15/8"	1"
Interior	7∕8"	-/2	=/8	11/4"	74	-/4	5/8"	-78	_

GIRDER CAMBER DIAGRAM

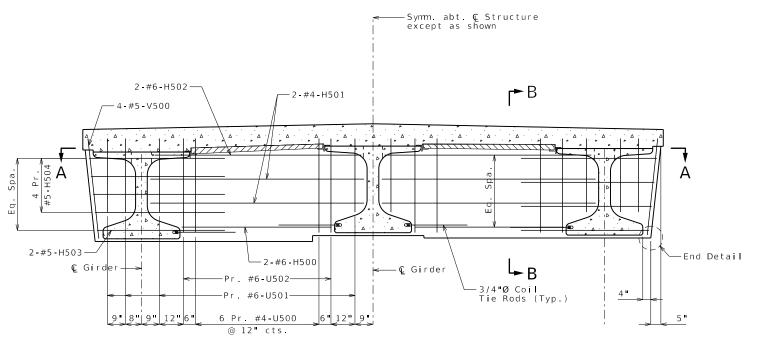
Conversion Factors for Girder Camber (Estimated at 90 days):

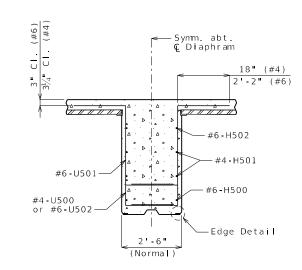
0.1 pt. = 0.314 x 0.5 pt. 0.2 pt. = 0.593 x 0.5 pt. 0.3 pt. = 0.813 x 0.5 pt. 0.4 pt. = 0.952 x 0.5 pt.



1411 East 104th St.
Kansas Clty, MO 64131
Tel (816) 874-4655
Fax (816) 874-4675
www.trekkdestgngroup.com

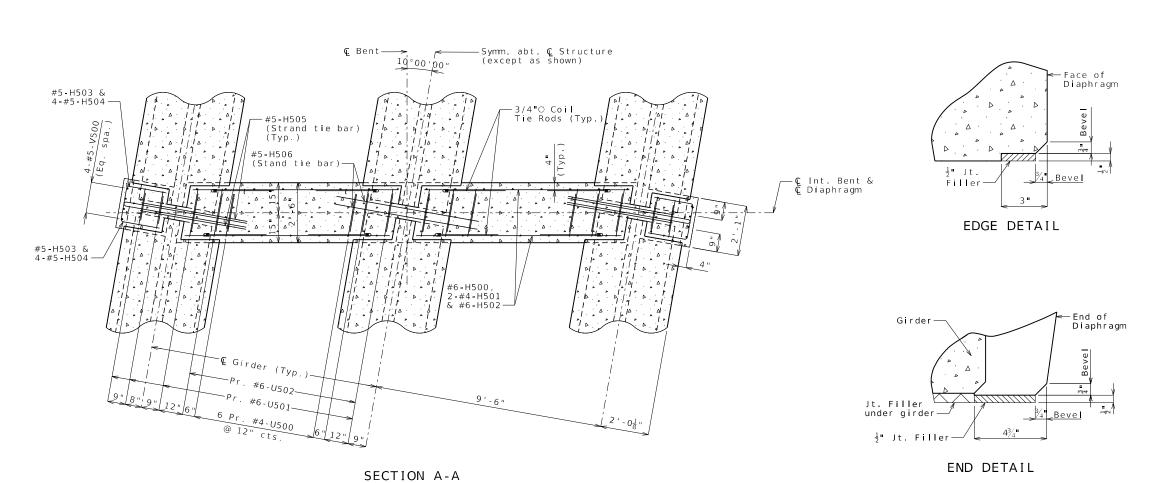






SECTION B-B

SECTION NEAR INTERMEDIATE BENT Normal to © Structure



CONCRETE DIAPHRAGMS AT INTERMEDIATE BENT NO. 2 & 3

11/26/2024

WORTH

JNW0020

PROJECT NO.

BRIDGE NO

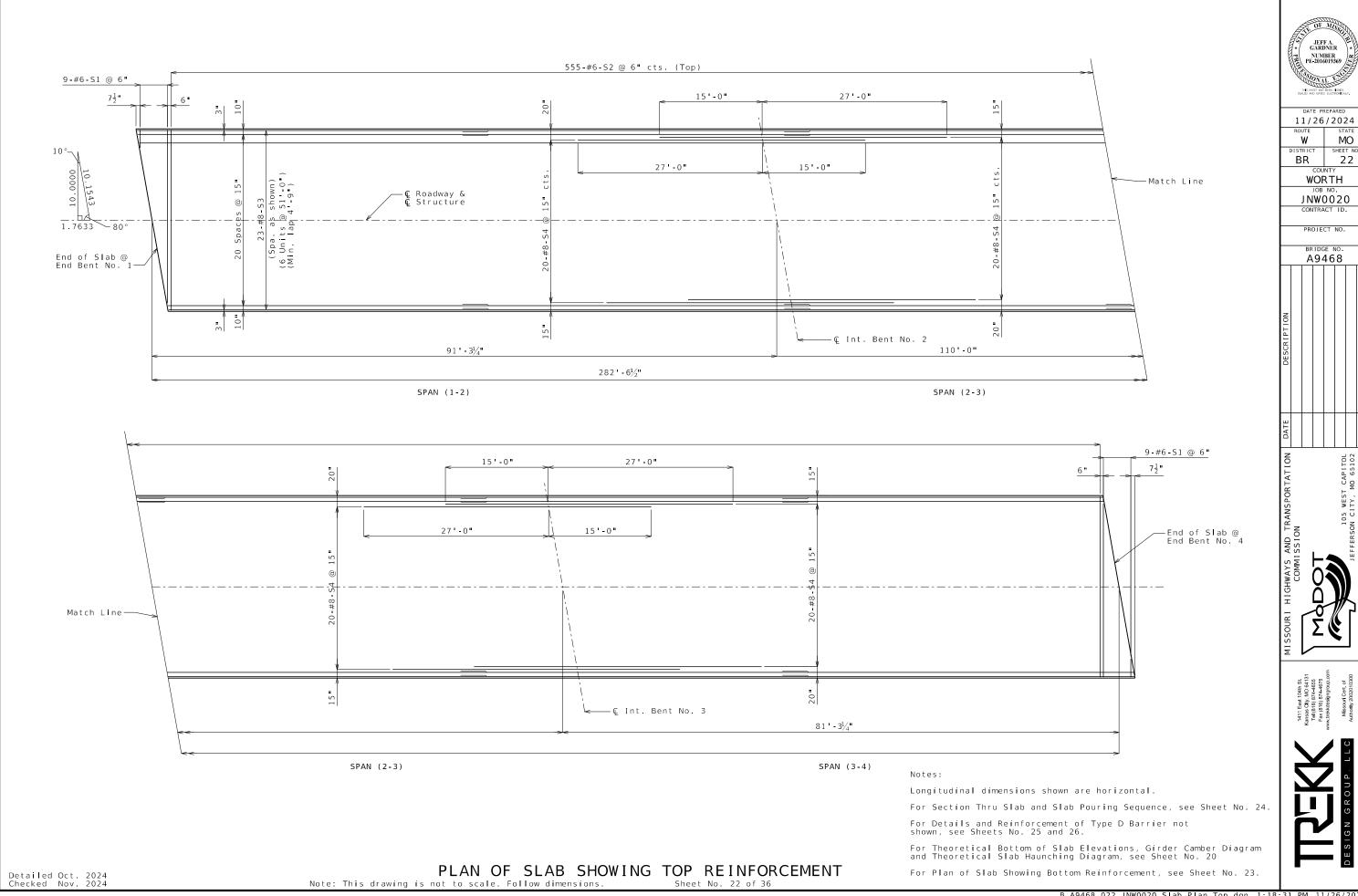
A9468

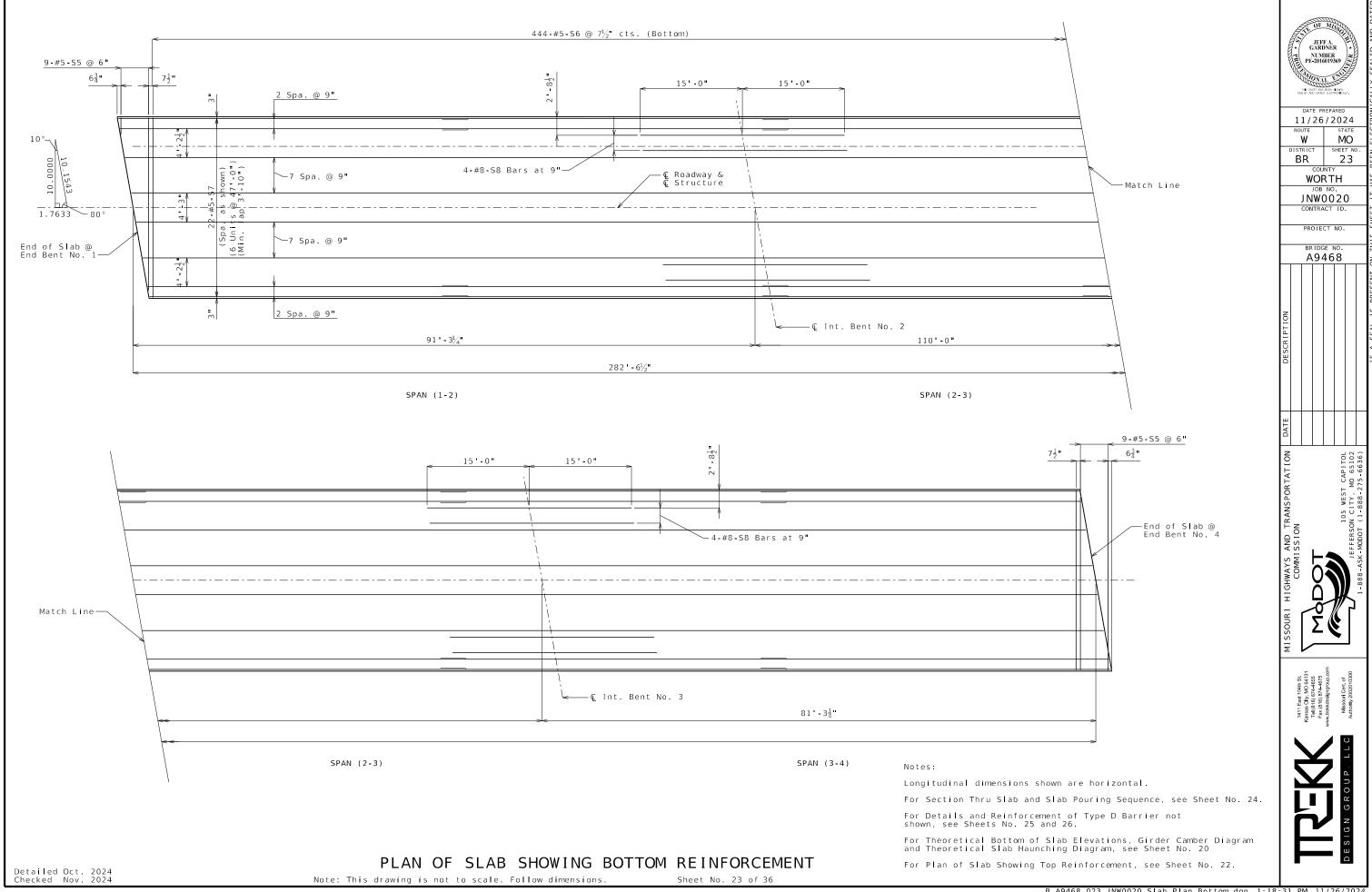
MO

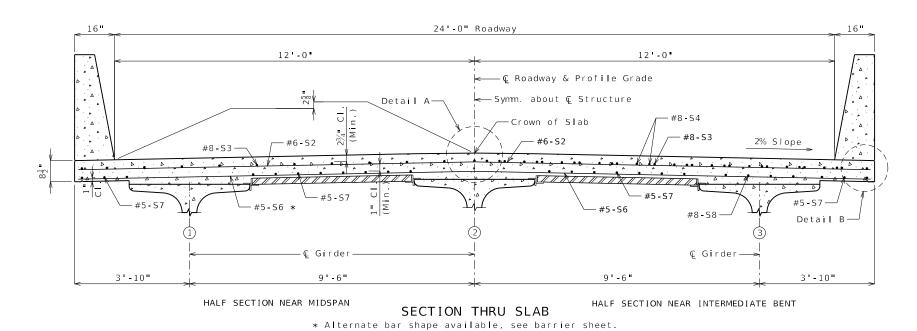
21

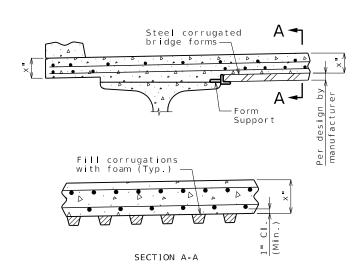
W

BR









OPTIONAL STAY-IN-PLACE FORM DETAILS

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

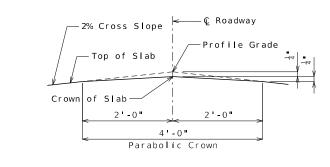
Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

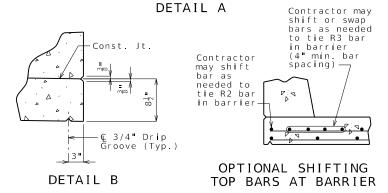
Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange.

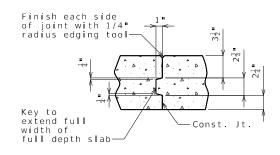
Drilling holes in the girder flanges will not be permitted.

All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

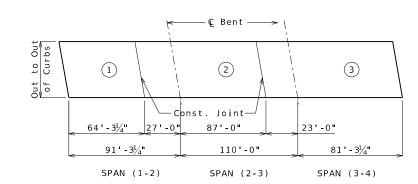
The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.







FULL DEPTH SLAB SLAB CONSTRUCTION JOINT



	Se	Min. Rate of Pour Cu. Yds./Hr.				
		Direction				
Basic	1	2	3	25		
Sequence] 23				
	e pours to the basic skip sequence are subject to the approval of the in accordance with Sec 703.					
Alternate A	1	+ 2	3	37		
Pours	End to 3		2 to End	37		
Alternate B			59			
Pours	End to End			39		

The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to 2.5 hours.

SLAB POURING SEOUENCE

Notes:

For reinforcement of barrier not shown, see Sheets No. 25 & 26.

For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 20.

For Plan of Slab Showing Reinforcement, see Sheets No. 22 & 23.

SLAB DETAILS

Detailed Oct. 2024 Checked Nov. 2024

JEFF A. GARDNER

NUMBER PE-2016019369

05/22/2024

WORTH

JNW0020

PROJECT NO.

BRIDGE NO

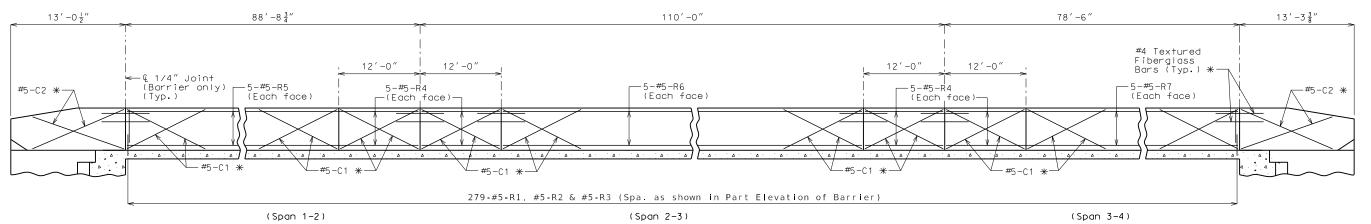
A9468

MO SHEET NO

24

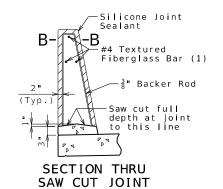
W

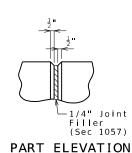
BR



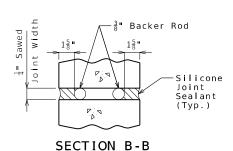
ELEVATION OF BARRIER

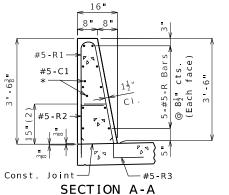
(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.

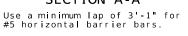




AT FORMED JOINT

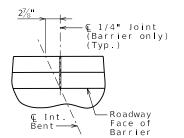




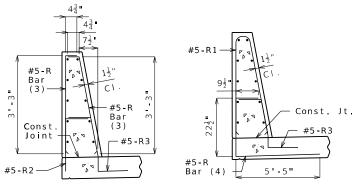


The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar



PART PLAN SHOWING JOINT LOCATION



R-BAR PERMISSIBLE ALTERNATE SHAPE

- (3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)
- (4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot

Concrete in barrier shall be Class B-1.

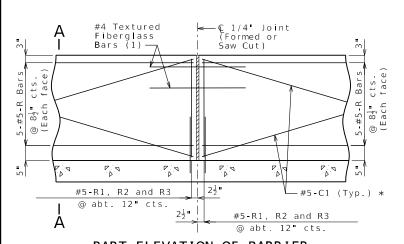
Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed ioints

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

Plastic waterstop shall not be used with saw cut joints.

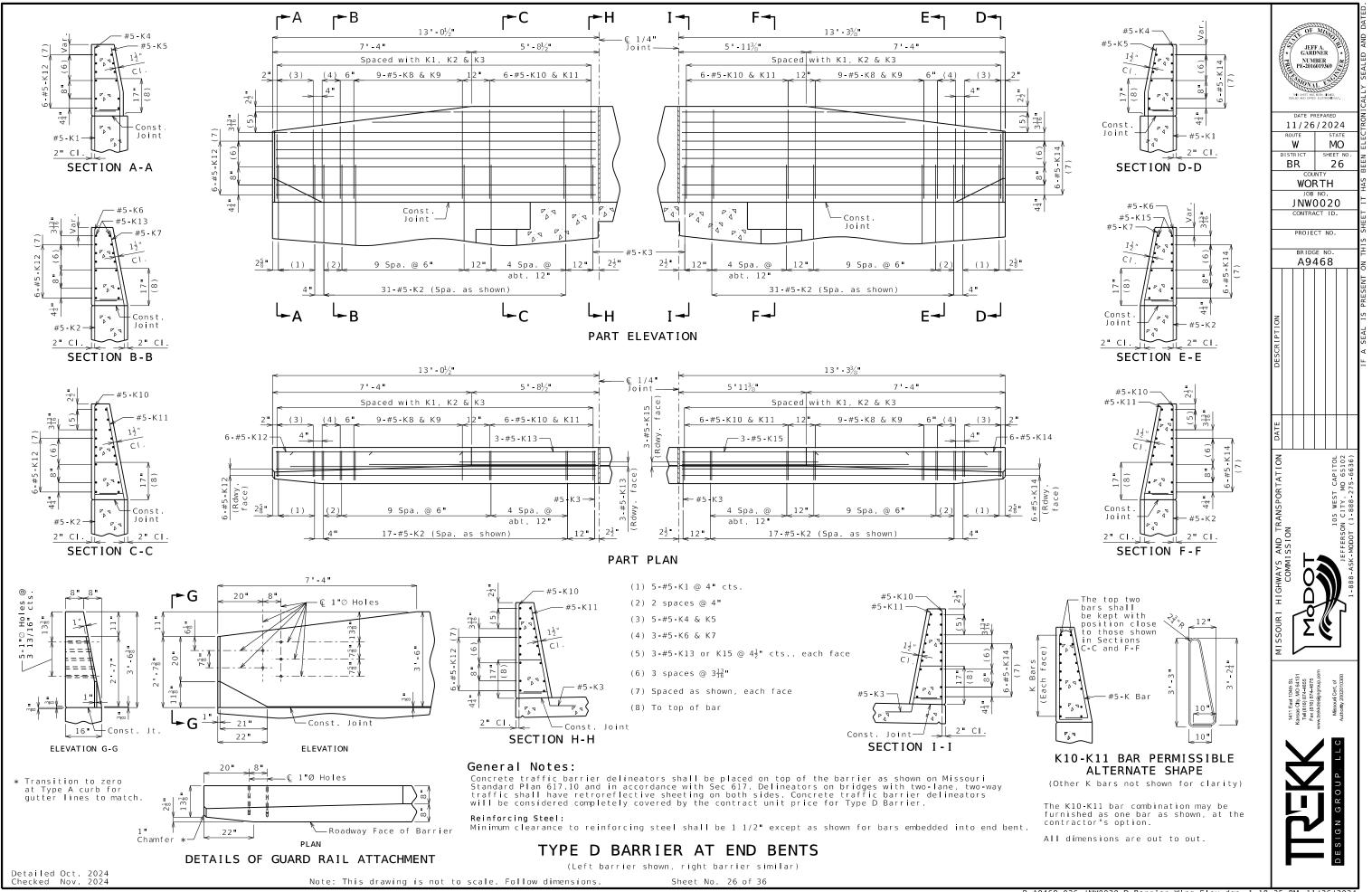


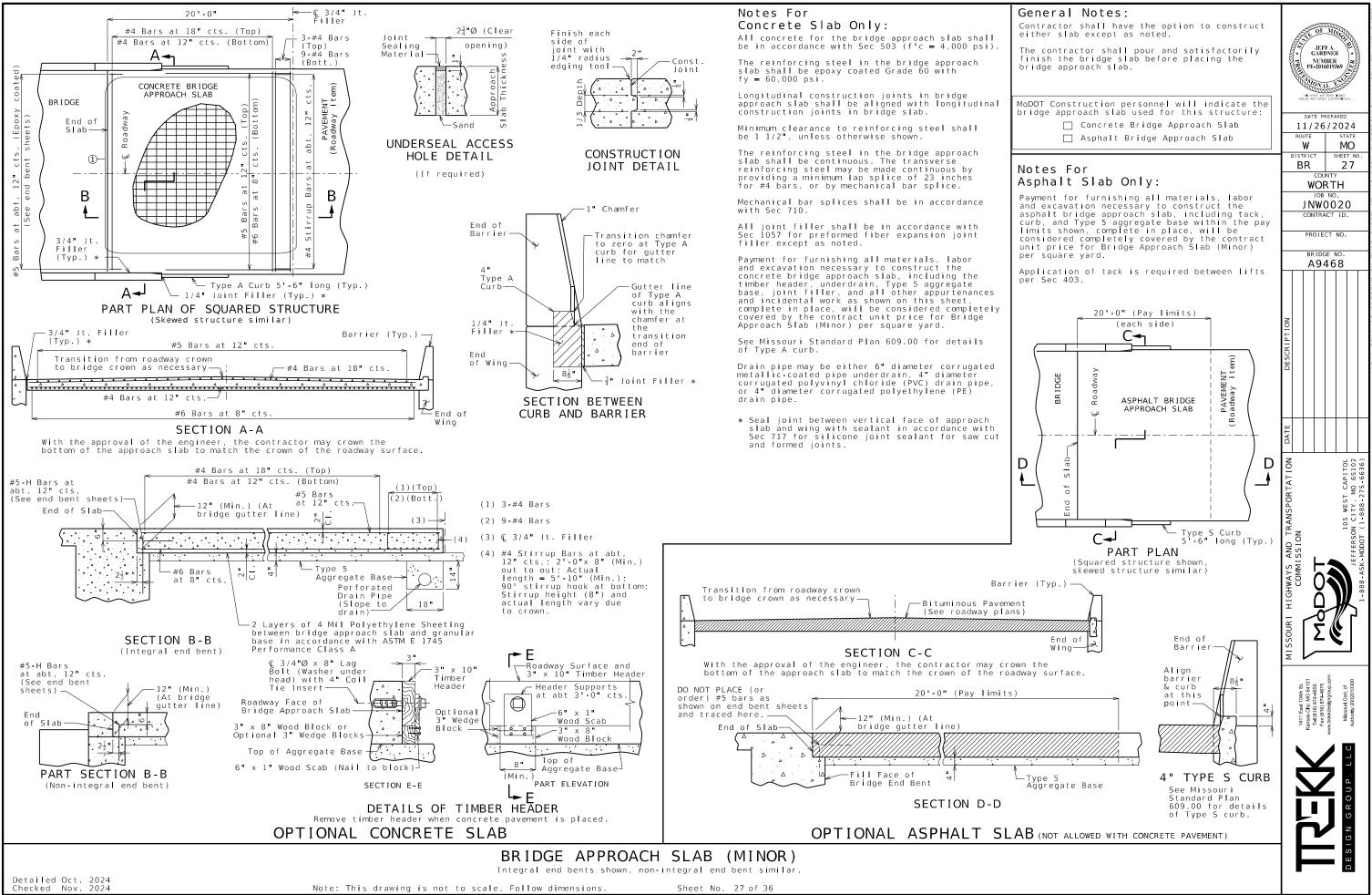
PART ELEVATION OF BARRIER

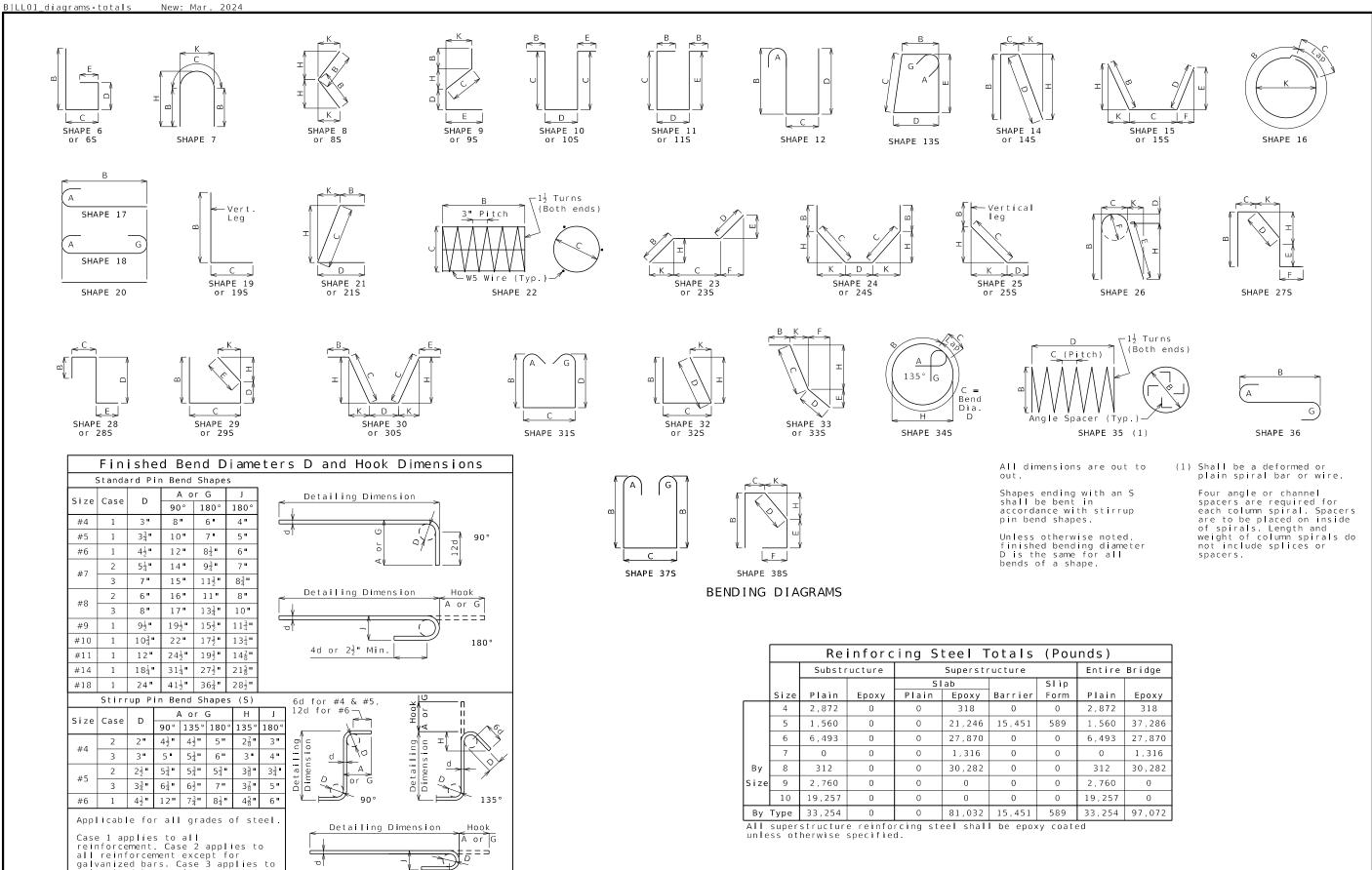
(1) Four feet long, centered on joint, slip-formed option only

Detailed Oct. 2024









BENDING DIAGRAMS AND REINFORCING STEEL TOTALS

Note: This drawing is not to scale. Follow dimensions.

180°

4d or $2\frac{1}{2}$ Min.

ğalvanized bars only.

Detailed Oct. 2024 Checked Nov. 2024

Sheet No. 28 of 36



DATE PREPARED				
11/26/2024				
ROUTE	STATE			
W	MO			
DISTRICT	SHEET NO.			
BR	28			
COUNTY				

WORTH

JOB NO.

JNW0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9468

HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL

1411 East 104th St. Kansas City, MO 6413 Tel (816) 874-4655 Fax (816) 874-4675 www.trekkdesigngroup.c



BILLO2 data New: Mar. 2024

No. Size/ Codes B C D E F H K Length Length Weight No. Size/ Codes B C D E F H K Length Length Weight	Bill of Reinforcing Steel										В	ill of	Rein	forci	ing Stee	<u> </u>				
Part	Dimensions Nom. Actual													imensions	3			Nom.	Actual	
Separation	No. Size/	Codes B C	D	E F	H K Lengt	th Length Wei	ght No	. Size/		Codes	s B	С	D	E	F	Н	K	Length	_ength	Weight
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Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

11/26/2024 W MO SHEET NO BR 29 WORTH JNW0020 PROJECT NO. BRIDGE NO A9468 HIGHWAYS AND TRANSPORTATION COMMISSION



For bending diagrams and steel reinforcing totals, see Sheet No. _.

Detailed Oct. 2024 Checked Nov. 2024

BILL OF REINFORCING STEEL Sheet No. 29 of 36 BILLO2 data New: Mar. 2024

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		26 5.000							26	5 26		22021																
8 8 S3 SLAB	E 20	51 0.000							51	0 51	200	18791																
8 S4 SLAB	E 20	42 0.000							42	0 42	0	8971																
5 S5 SLAB	E 20 V	2 4.000							2	4 2	4	257																
INC = 34.000 INCH		25 0.000							25	0 25	0																	
	E 20	26 5.000							26	5 26	5	12233																
	-	49 6.500							49	7 49	7	6826																-
		30 0.000							30	0 30	0	1282										+ +						
U GO GEAB	L 20	30 0.300					1		50	0 30						++++						+ +	-					_
TYPE D BARRIER										_		—				++-+		+				+						+
N BOOK STANK OF BUILDING	= 00		5 500	0 005				0.750		40.0		2020						-	_			_						_
		3 3.000	5.500	3 3.625			3 3.000	6.750	6	10 6		3928				++-+												
	E 19	20.500	9.500						2	6 2		1406																
	E 27		9.500	15.500	5.000	12.00	0 15.000	3.000	3	6 3	4	1940																
5 R4 BARRIER	E 20	11 9.000							11	9 11	9	980																
5 R5 BARRIER	E 20	39 11.000							39	11 39	11	1665																
5 R6 BARRIER	E 20	44 7.000							44	7 44	7	1860																
5 R7 BARRIER	E 20	34 10.000							34	10 34	10	1453																
5 K1 BARRIER	E 27	3 8.000	9.250	5.375	3 2.750		5.250	1.000	8	0 7	11	165																
	E 27	3 8.000	9.250				14.250	2.750		2 7	11	561				++++						+ -						
	E 27	3 8.000	9.250		8.000	12.00			100	65	2	22										+ +						
	E 19 V		10.000		8.000	12.00	14.123		3	2 3	1	66		_		++++		+				+ +					+ +	+
INC. = 0.500 INCH		2 6.250	10.000						2	4 3	2					++		_	-									+
							4.000	40,000	3		3	62																
	E 14 V	8.250	9.500				4.000	18.000		0 2	11	63																
INC. = 0.500 INCH		8.250	9.500				4.500	20.000		2 3	1																	
	E 19 V								3	5 3	4	42				+						\perp						
INC. = 0.500 INCH		2 7.750	10.000						3	6 3	5					+												
	E 21 V						2 6.000			5 3	3	41				\Box												
INC. = 0.500 INCH		2 7.625	10.000				2 7.000			6 3	4																	
5 K8 BARRIER	E 19 V	2 8.500	10.000						3	7 3	5	138																
INC. = 0.750 INCH		3 2.500	10.000						4	1 3	11																	
	E 21 V		10.000				2 7.750	6.750	3	7 3	5	46																
INC. = 0.750 INCH		3 2.500					3 1.750			1 3	11																	
		3 3.000					1.750		4	1 4	0	100										+ +						
		3 3.000					3 2.250	7.750		1 3	11	98	_			++++	+	-				++	_				+	
11 (10000			10.000				3 2.250				111					+						++						+
		12 9.500							12	9 12	9	319																-
		6 2.000							6	2 6	2	115																
INC. = 36.000 INCH		12 2.000							12	2 12	2																	
		12 0.275							13	0 13	0	325																
	E 20	13 0.375																										
5 K14 BARRIER		6 5.750							6	5 6	5	118																

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

11/26/2024

WORTH JNW0020

PROJECT NO. BRIDGE NO. A9468

MO

SHEET NO

30

For bending diagrams and steel reinforcing totals, see Sheet No. _.

Detailed Oct. 2024 Checked Nov. 2024

BILL OF REINFORCING STEEL Note: This drawing is not to scale. Follow dimensions. Sheet No. 30 of 36



PART PLAN SHOWING PILE & DRILLED SHAFT NUMBERING FOR RECORDING AS-BUILT PILE DATA & AS-BUILT DRILLED SHAFT DATA

	As-Built Pile Data													
Pile No.	Length in Place (ft)	PDA Nom. Axial Compressive Resistance (kips)	Count	Actual End of Drive Blow Count (blows/in.)	Remarks									
					End Bent No. 1									
1														
2														
3														
4														
					End Bent No. 4									
5														
6														
7														
8														

			As-B	uilt Drilled Shaft Data
Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	Remarks
				Intermediate Bent No. 2
1				
2				
				Intermediate Bent No. 3
3				
4				

ndicate in remarks column:			
. Pile type and grade			
Batter and a second			
Driven to practical refusal			
). PDA test pile			
. Minimum tip elevation controlled			
	 D D A	1 1	

(Use when actual blow count is less than PDA blow count due to minimum tip elevation requirement. A plus sign (+) shall be placed after the PDA nominal axial compressive resistance value indicating actual value is higher than PDA value.)

This sheet to be completed by MoDOT construction personnel.

JEFF GARLI NUM PE-2016 SPAID AND DATE:	DNER * DN
11/26	/2024
ROUTE	STATE

DATE PREPARED										
11/26/2024										
ROUTE STATE										
W MO										
DISTRICT	SHEET NO.									
BR	31									
COU	NTY									
WOF	RTH									
JOB	NO.									
JNW0020										
CONTRACT ID.										

		PR	DIE	СТ	NO.						
BRIDGE NO. A9468											
7											
TION											







/lissouri	Department of	Transportation
Co	netruction and	Materials

BORING NO. B-101 Page 1 of 2

	Construction and Materials	
ob No.: NW0020-A9468	County: Worth	Route: W
esign: A9468	Skew: 10 RA	Location: Over W. Fork Grand River
ent: _1	Logged By: Matthew Kistler	Operator: _Josh Starkey
tation:	Northing: _1549438.9	Date of Work: 05/29/24-05/29/24
ffset:	Easting: _2772694.5	Depth to Water: 40.1
levation: 943.8	Requested Northing:	Depth Hole Open:
equested Station:	Requested Easting:	Time Change: At Time of Drilling
equested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sa	impler, NQ

Re	quested	Offset: Equipment:	PPI C	ME 5	55S/N ,S	plit-Spoon Samp	oler, NQ		
Re	quested	Elevation: Location No	te : <u>On</u>	Stak	е				
Dri	II No.: _	Rig #408095 Hammer Effi	ciency	789	%		orilling Method:	Hollow Stem Aug	er
O		Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
-	-	0.0-4.1' Brown, LEAN CLAY, medium stiff, moist	940		93	2-3-6		PP = 1.75 tsf	MC = 17.4%
VER.GPJ		4.1-6.0' Brown and gray, LEAN CLAY with sand, stiff, moist 6.0-8.8' Dark brown, LEAN CLAY, stiff, moist	- - - - -		93	(12)	_	FF - 1.73 (S)	Y _{sat} = 134 pcf ¹¹ LL = 37 PL = 16
WFORKGRANDRIVER.GP	- - - -	8.8-13.6' Light brown, LEAN CLAY trace fine gravel, moist	935	X	87	2-3-5 (10)	_	PP = 1.50 tsf	MC = 22.8% Y _{sat} = 127 pcf ⁽¹⁾ LL = 41 PL = 19
WORTH RTW	5	13.6-20.0' Gray and tan, SAND, medium dense, moist 14.9-20.0' scattered Iron Manganese concretions	930		67	4-4-6 (13)	-		Sieve Analysis Sieve # % Passing 3/4" 100.0 3/8" 96.8 #4 94.0 #10 90.0 #16 87.0 #40 73.6
PROJECT FILES\NW0020-A9468	5	20.0-27.6' Dark gray, LEAN CLAY trace fine gravel, stiff, moist, (Glacial Till)	925	X	0	6-6-9 (20)			#50 48.7 #100 7.9 #200 4.6
14:56 - Z:\SG\GINT\P	5		920	X	73	3-5-8 (17)	-	PP = 3.25 tsf	MC = 23.3% Y _{sat} = 127 pcf ⁽¹⁾ LL = 49 PL = 21
20151118.GDT - 7/31/24		27.6-30.8' COBBLES, dense, Limestone Cobbles and Boulders	915	X	33	30-15-23 (49)	-		
- MODOT 2015111	- - - - 5	30.8-53.2' Dark gray, LEAN CLAY trace fine gravel, very stiff, moist, (Glacial Till)	910	X	93	9-6-7 (17)	_	PP = 2.75 tsf	MC = 20.5% y sat = 130 pcf ⁽¹⁾

N_{so} = (Em/60)Nm N_{so} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

Missouri Department of Transportation Construction and Materials

BORING NO. B-101 Page 2 of 2

Job No.: NW0020-A9468	County: Worth	Route: W
Design: A9468	Skew: 10 RA	Location: Over W. Fork Grand River
Bent: _1	Logged By: Matthew Kistler	Operator: Josh Starkey
Station:	Northing: 1549438.9	Date of Work: 05/29/24-05/29/24
Offset:	Easting: 2772694.5	Depth to Water: 40.1
Elevation: 943.8	Requested Northing:	Depth Hole Open:
Requested Station:	Requested Easting:	Time Change: At Time of Drilling
Requested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sar	mpler, NQ
Requested Elevation:	Location Note: On Stake	-9

Drill No.:	Rig #408095 Hammer	Efficiency	: <u>78</u>	%	D	rilling Method:	Hollow Stem Auge	er
Depth (ft) Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
35 	30.8-53.2' Dark gray, LEAN CLAY trace fin gravel, very stiff, moist, (Glacial Till) (continued)	e		93	4-5-7		PP = 2.50 tsf	MC = 22.1% y sat = 128 pcf ⁽¹⁾
- 40 45	₹	900		100	(16) 4-6-11 (22)		PP = 2.50 tsf	MC = 21.1% y sal = 129 pcf ⁽¹⁾
50		895		100	5-8-5 (17)		PP = 2.25 tsf	MC = 20.7% y sal = 130 pcf ⁽¹⁾
- - - - - 55	53.2-54.8' Shale, dark gray, highly weather 54.8-67.4' Shale, dark gray, very weak rock			88	48-50/0.3*	Qu Test Results	PP = 9.00 tsf	
60	moderately weathered	885		100 (0) 100 (48)		UCS = 15.0 ksf MC = 10.8% Y mosst = 142.3 pcf Qu Test Results UCS = 15.6 ksf MC = 11.1%	PP = 5.00 tsf	
65	62.8-67.8' water used to extrude core.	880		100 (36)		γ most = 133.9 pcf Qu Test Results UCS = 4.37 ksf MC = 18.2%		
	67.4-67.8' Fossiliferous Limestone, light grasslightly weathered Bottom of borehole at 67.8 feet.	ау		(33)		γ _{moist} = 133.1 pcf		

N_{so} = (Em/60)Nm N_{so} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent, Nm - Observed N-value; (1) = Assumed, (2) = Actual

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 32 of 36

JEFF A.

GARDNER

NUMBER

PE-2016019369

DATE PREPARED

1 1/26/2024

ROUTE STATE

W MO

DISTRICT SHEET NO.

BR 32

COUNTY
WORTH

WORTH

JOB NO.

JNW0020

PROJECT NO.

DESCRIPTION

BY 100 PESCRIPTION

BY 100 PESCRI

SSOURI HIGHWAYS AND TRANSPORTATIC
COMMISSION

105 WEST CAPITO

105 WEST CA

1411 East 104th St. Kansas City, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 wv.trekkdesigngroup.com



Missouri Department of Transportation **Construction and Materials**

BORING NO. B-203 Page 1 of 3

County: Worth Job No.: NW0020-A9468 Route: W Design: A9468 Skew: 10 RA Location: Over W. Fork Grand River Bent: 2 Logged By: Ricardo Todd Operator: Bradley Wadlow Northing: 1549396.6 Date of Work: 06/04/24-06/04/24 Offset: Easting: 2772780.7 Depth to Water: 30.0 Elevation: 940.1 Requested Northing: 1549393.6 Depth Hole Open: Requested Station: Requested Easting: 2772780.5 Time Change: At Time of Drilling Requested Offset: Equipment: Acker Soil XLS ,Split-Spoon Sampler, NQ

Requested Elevation: Location Note: Offset to avoid bridge cross member Hammer Efficiency: 81% Drill No.: G-9462 Drilling Method: Casing Advancer Depth (ft) REC (RQD Description _\ 0.0-0.1' ASPHALT 0.1-0.6' CONCRETE 0.6-30.0' Air 935 930 925 920 915 30.0-31.0' Light tan black, SAND, very loose, 31.0-34.1' BOULDERS

34.1-40.1' Gray, SAND, very loose, wet N₅₀ = (Em/60)Nm N₅₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet * Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and

by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

Missouri Department of Transportation Construction and Materials

BORING NO. B-203 Page 2 of 3

	Contraction and materials	
bb No. : NW0020-A9468	County: Worth	Route: W
esign: <u>A9468</u>	Skew: 10 RA	Location: Over W. Fork Grand River
ent: <u>2</u>	Logged By: Ricardo Todd	Operator: Bradley Wadlow
ation:	Northing: 1549396.6	Date of Work: 06/04/24-06/04/24
ffset:	Easting: 2772780.7	Depth to Water: 30.0
evation: 940.1	Requested Northing: 1549393.6	Depth Hole Open:
equested Station:	Requested Easting: 2772780.5	Time Change: At Time of Drilling
equested Offset:	Equipment: Acker Soil XLS ,Split-Spoon Sam	pler, NQ
	Landin Nata Official according to the	

Requested Elevation: Location Note: Offset to avoid bridge cross member **Drill No.:** G-9462 Hammer Efficiency: 81% Drilling Method: Casing Advancer %% Depti (ft) Description 34.1-40.1' Gray, SAND, very loose, wet 5-1-1 73 (continued) 40 MC = 21.4% **y** sal = 129 pcf⁽¹⁾ LL = 36 PL = 19 40.1-45.7 Gray, LEAN CLAY scattered gravel, 3-7-8 73 PP = 2.25 tsf very stiff, moist 45 MC = 16.4% 5-8-13 73 PP = 7.50 tsf **y** sal = 135 pcf⁽¹⁾ 45.7-55.1' Shale, gray, very weak rock, highly weathered 37/0.1', 10/0' PP = 7.50 tsf 890 885 37/0.1', 10/0' Qu Test Results UCS = 2.54 ksf PP = 7.50 tsf 55.1-60.1' Shale, gray, very weak rock, moderately weathered MC = 12.9% y moist = 124 pcf 60.1-85.1' Calcareous Shale, gray, medium strong rock, moderately weathered to slightly **Qu Test Results** weathered UCS = 226 ksf MC = 7.3% (32)γ moist = 151.1 pcf Qu Test Results 875 MC = 7.3%γ _{moist} = 151.1 pcf Qu Test Results UCS = 193 ksf MC = 6.1% (60) $\gamma_{\text{moist}} = 154.4$

N₅₀ = (Em/60)Nm N₅₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate Proj. Factor: 1.0000983827

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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(Continued Next Page)

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024 Note: This drawing is not to scale. Follow dimensions.

(Continued Next Page)

Sheet No. 33 of 36

JEFF A. GARDNER

11/26/2024 W MO SHEET NO BR 33

WORTH

JNW0020 PROJECT NO

BRIDGE NO A9468



Job No.: NW0020-A9468

Design: A9468

Elevation: 940.1

Requested Station:

Requested Offset:

Bent: 2

Offset:

	uri Department of Transport Construction and Materials	ation
ounty:	Worth	R

BORING NO. B-203 Page 3 of 3

 County:
 Worth
 Route:
 W

 Skew:
 10 RA
 Location:
 Over W. Fork Grand River

 Logged By:
 Ricardo Todd
 Operator:
 Bradley Wadlow

 Northing:
 1549396.6
 Date of Work:
 06/04/24-06/04/24

 Easting:
 2772780.7
 Depth to Water:
 30.0

 Requested Northing:
 1549393.6
 Depth Hole Open:

 Requested Easting:
 2772780.5
 Time Change:
 At Time of Drilling

 Equipment:
 Acker Soil XLS, Split-Spoon Sampler, NQ

Requested Elevation: _____ Location Note: Offset to avoid bridge cross member

Requ	ested I	Elevation: Location Not	e: <u>Off</u>	set to	avoid b	ridge cross mem	ber		
Drill N	lo.: _C	9-9462 Hammer Effic	ciency:	81	%	D	rilling Method: _	Casing Advancer	
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{so})	Shear Data	Field Tests	Index Tests
 75		60.1-85.1' Calcareous Shale, gray, medium strong rock, moderately weathered to slightly weathered (continued)	870		100 (88)		pcf Qu Test Results UCS = 334 ksf MC = 4.8% 7 moist = 156.3 pcf Qu Test Results UCS = 346 ksf MC = 6.3%		
 80			860		100 (70)		γ moist = 157.1 pcf Qu Test Results UCS = 455 ksf MC = 5.5% γ moist = 156.7 pcf Qu Test Results		
85		Bottom of borehole at 85.1 feet.	855		100 (90)		UCS = 163 ksf MC = 7.3%		
80		Bottom of Borende at 65. Treet.					Qu Test Results UCS = 113 ksf MC = 8.9%		

N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

Missouri Department of Transportation Construction and Materials

BORING NO. B-301 Page 1 of 3

	Concentration and materials	
Job No.: NW0020-A9468	County: Worth	Route: W
Design : <u>A9468</u>	Skew: 10 RA	Location: Over W. Fork Grand River
Bent: 3	Logged By: Matthew Kistler	Operator: Josh Starkey
Station:	Northing: 1549388.2	Date of Work: 06/11/24-06/11/24
Offset:	Easting: 2772888.8	Depth to Water: 31.2
Elevation: 935.8	Requested Northing: 1549392.2	Depth Hole Open:
Requested Station:	Requested Easting: 2772889.0	Time Change: At Time of Drilling
Requested Offset:	Equipment: PPI CME 55LC ,Split-Spoon Sam	npler, NQ
Requested Elevation:	Location Note: Offset to avoid bridge cross m	ember

			mmer Efficienc					Casing Advancer	
Depth (ft)	Graphic	Description	Elevation (#)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0	p 6 4	D 0 0 0 41 ACRUAL T							
4	P. N. 9.	0.0-0.1' ASPHALT 0.1-0.6' CONCRETE	935	<u> </u>					
1-		0.6-28.0' Air	/ 	+					
-			t	+					
			t	+					
5			930	, -					
-			930	-					
-	ė		†	-					
	5		†	1					
10-			T T						
10	2		925	, –					
٠ -									
			Ť	1					
4			Ť	7					
15			Ť						
			920						
· -	e e								
٠ ٦			Ι						
1			Ι						
20			Ι						
			915	<u>. </u>					
1									
٦									
25									
			910)					
1			1						
1			1						
1		28.0-31.2' Dark brownish gray, LEAN soft to medium stiff, moist	CLAY,						
30		soft to medium stiff, moist	1						
		7 30 5' switched to mud rotary after an	905	<u> </u>			_		
	/////	30.5' switched to mud rotary after sp 31.2-42.0' Dark gray, SAND, very loc 31.2-42.0' Dark gray, SAND,	se wet	\bot	20	2-0-0 (0)			
		fine grained, poorly graded				(-)	7		
1		2 2	1						
35			Ţ	\rightarrow	40	2-1-1			

N_{so} = (Em/60)Nm N_{eo} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

 Coordinate System:
 Modified U.S. State Plane 1983
 Coordinate Zone:
 Missouri West
 Coordinate Proj. Factor:
 1.0000983827

 Coordinate Datum:
 NAD 83 (CONUS)
 Coordinate Units:
 U.S. Survey Feet

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(Continued Next Page)

BORING DATA

JEFF A.

NUMBER
PF-2016019369

		Ċ
DATE PE	REPARED	Ę
11/26	/2024	TD
OUTE	STATE	Ľ
W	MO	ū
STRICT	SHEET NO.	Į
BR	34	П

SR 34
COUNTY
WORTH
JOB NO.

JOB NO.
JNW0020
CONTRACT ID.

PROJECT NO.

		16	
DESCRIPTION			
DATE			
			_

ISSOURI HIGHWAYS AND TRANSPORTATIO

COMMISSION

105 WEST CAPITO

HEFFERSON CITY, MO 6510

1411 East 104th St. Kansas City, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 ww.trekkdesigngroup.com



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Missouri Department of Transportation **Construction and Materials**

BORING NO. B-301 Page 2 of 3

County: Worth Job No.: NW0020-A9468 Route: W Design: A9468 Skew: 10 RA Location: Over W. Fork Grand River Bent: 3 Logged By: Matthew Kistler Operator: Josh Starkey Date of Work: 06/11/24-06/11/24 Northing: 1549388.2 Offset: Easting: 2772888.8 Depth to Water: 31.2 Elevation: 935.8 Requested Northing: 1549392.2 Depth Hole Open: Requested Station: Requested Easting: 2772889.0 Time Change: At Time of Drilling Equipment: PPI CME 55LC ,Split-Spoon Sampler, NQ Requested Offset:

Requested Elevation: Location Note: Offset to avoid bridge cross member Hammer Efficiency: 84.7% Drill No.: Rig #360485 Drilling Method: Casing Advancer Depth (ff) Description 31.2-42.0' Dark gray, SAND, very loose, wet, 900 fine grained, poorly graded (continued) 3-2-1 40 20 895 42.0-44.5' Dark gray, LEAN CLAY, stiff, (completely weathered shale) 31-37/0.3' PP = 9.00 tsf 93 44.5-44.8' Shale, dark gray, thinly laminated, moderately weathered 44.8-59.4' Shale, dark gray, very weak rock, 100 slightly weathered γ _{moist} = 140.2 pcf Qu Test Results 100 UCS = 4.46 ksf MC = 12.2% **y** moist = 138.6 pcf Qu Test Results 100 (52) γ _{moist} = 133.1 pcf 59.4-60.1' Limestone, light gray, thin bedded, slightly weathered Qu Test Results 60.1-61.9' Shale, dark gray, very weak rock, UCS = 50.8 ksf MC = 9.9% 100 slightly weathered (76)γ moist = 145.7 pcf
Qu Test Results
UCS = 71.7 ksf 61.9-74.8' Calcareous Shale, light gray, medium bedded, weak rock to medium strong rock 870 MC = 7.6%y moist = 148.3 pcf Qu Test Results 100 (96) UCS = 185 ksf MC = 6.6%γ _{moist} = 153 pcf

N₅₀ = (Em/60)Nm N₅₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827 Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

Missouri Department of Transportation **Construction and Materials**

BORING NO. B-301 Page 3 of 3

ob No.: NW0020-A9468	County: Worth	Route: W
esign: _A9468	Skew: 10 RA	Location: Over W. Fork Grand River
ent: _3	Logged By: Matthew Kistler	Operator: Josh Starkey
tation:	Northing: 1549388.2	Date of Work: 06/11/24-06/11/24
ffset:	Easting: _2772888.8	Depth to Water: 31.2
levation: 935.8	Requested Northing: 1549392.2	Depth Hole Open:
equested Station:	Requested Easting: 2772889.0	Time Change: At Time of Drilling
equested Offset:	Equipment: PPI CME 55LC ,Split-Spoon Sar	npler, NQ
equested Elevation:	Location Note: Offset to avoid bridge cross m	ember

	d Station:						me Change: At		
	ested Offset: Equipment: PPI CME 55LC ,Split-Spoon Sampler, NQ								
Requested	d Elevation:	Location Not	e: Off	set to	avoid b	avoid bridge cross member			
Drill No.:	Rig #360485	Hammer Effic	ciency:	84.	7%	Dı	rilling Method: _	Casing Advancer	
Obepth (ft) (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
70	61.9-74.8' Calcareous Shale, lig medium bedded, weak rock to rock (continued)	ght gray, medium strong	865		100 (96)		Qu Test Results UCS = 125 ksf MC = 6.8% 7 moist = 151.1 pcf		
	Bottom of borehole at 7								

N_{so} = (Em/60)Nm N_{so} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate Proj. Factor: 1.0000983827 Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 35 of 36 JEFF A. GARDNER

THIS SHEET HAS SEALED AND DATES	BEEN SIGNED.							
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Construction and Materials	

BORING NO. B-403 Page 1 of 2

	Construction and materials	
Job No.: NW0020-A9468	County: Worth	Route: W
Design: _A9468	Skew: 10 RA	Location: Over W. Fork Grand River
Bent: _4	Logged By: Matthew Kistler	Operator: Josh Starkey
Station:	Northing: 1549351.2	Date of Work: 05/30/24-06/04/24
Offset:	Easting: _2772965.4	Depth to Water: 24.2
Elevation: 932.3	Requested Northing: 1549349.2	Depth Hole Open:
Requested Station:	Requested Easting: 2772965.3	Time Change: At Time of Drilling
Requested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sa	impler, NQ

Requ	ested l	Elevation:	Location Note: _C	n Stak	e (offset	by surveyor jus	st north of requeste	d location)	
Drill N	No.: _F	Rig #408095	Hammer Efficienc	y : _78	%		Drilling Method: _	Hollow Stem Aug	er
o Depth (ft)	Graphic	Description	Elevation (#)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
		0.0-2.2' Light brown, LEAN CLA stiff, dry 2.2-9.7' Dark brown mottled dark	930	-					
5		CLAY, soft, moist	925	X	20	2-1-2 (4)		PP = 0.25 tsf	MC = 24.1% y sat = 126 pcf ⁽¹⁾
10		9.7-14.3' Dark gray, SILTY LEA	N CLAY		80	2-3-4 (9)		PP = 1.75 tsf	MC = 28.2% y _{out} = 122 pcf ⁽¹⁾ LL = 28 PL = 21
 		scattered wood fragments, med	920	-					PL = 21
20 25 30 30		14.3-24.0' Grayish brown to ligh scattered lean clay, loose, moist		- X	80	2-2-3 (7)			
20			910		80	2-2-2 (5)			
25		24.0-29.5' Gray to tan, trace fine wet	1	-	73	1-2-2 (5)			
			905	-					
30		29.5-46.0' Dark gray, SAND with medium dense to loose, wet	ean clay,	- X -	60	3-3-3 (8)			Sieve Analysis Sieve # % Passin
			Ī			0.1.2			Sieve # % Passin 3/4" 100.0 3/8" 99.8

N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual ____ Coordinate Proj. Factor: <u>1.0000983827</u> Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Units: U.S. Survey Feet Coordinate Datum: NAD 83 (CONUS)

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

Missouri Department of Transportation **Construction and Materials**

BORING NO. B-403 Page 2 of 2

bb No. : NW0020-A9468	County: Worth	Route: W
esign: <u>A9468</u>	Skew: 10 RA	Location: Over W. Fork Grand River
ent: _4	Logged By: Matthew Kistler	Operator: Josh Starkey
ation:	Northing: _1549351.2	Date of Work: 05/30/24-06/04/24
ffset:	Easting: 2772965.4	Depth to Water: 24.2
evation: 932.3	Requested Northing: 1549349.2	Depth Hole Open:
equested Station:	Requested Easting: 2772965.3	Time Change: At Time of Drilling
equested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sa	mpler, NQ
equested Elevation:	Location Note: On Stake (offset by surveyor ju	ust north of requested location)

Requ	ested (Offset: Equipm	nent: <u>PPI</u>	CME	55S/N ,S	plit-Spoon Samp	ler, NQ		
Requ	ested I	Elevation: Location	on Note: _(On Sta	ke (offse	t by surveyor just	north of requeste	d location)	
Drill N	No.: _F	tig #408095 Hamme	er Efficiend	y: _78	3%	D	rilling Method: _	Hollow Stem Auge	r
Depth (ft)	Graphic	Description	Elevation	(II) Sample Type	REC % (RQD %)	Blow Counts (N _{En})	Shear Data	Field Tests	Index Tests
40		29.5-46.0' Dark gray, SAND with lean cla medium dense to loose, wet (continued) 39.2' sand heaved before SPT could be performed 40.0' switch to mud rotary	y, 899	-	93	(4)			#10 96.9 #16 94.7 #40 57.2 #50 28.6 #100 10.9 #200 9.0
45		46.0-48.9' Shale, dark gray, completely weathered to highly weathered 48.9-54.0' Shale, dark gray, very weak ro moderately weathered to slightly weather		5	93 100 (65)	4-3-4 (9) 24-36/0.3', 10/0'	Qu Test Results UCS = 6.42 ksf MC = 12.8%	PP = 9.00 tsf PP = 9.00 tsf	
 55		54.0-55.1' Fossiliferous Limestone, light o	980 gray,	- - - -	94 (42)		7 moist = 140 pcf Qu Test Results UCS = 6.30 ksf MC = 14.4% γ moist = 137.5 pcf		
 60		55.1-60.1' Shale, dark gray, weak rock, slightly weathered	878	- 5 - -	100 (90)		Qu Test Results UCS = 65.4 ksf MC = 10.6%		
		Bottom of borehole at 60.1 feet.							

 $\frac{1}{2}$ N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827 Coordinate Units: U.S. Survey Feet Coordinate Datum: NAD 83 (CONUS)

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BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 36 of 36

11/26/2024 W MO SHEET NO BR 36

WORTH

JNW0020

PROJECT NO.

A9468

