

July 17, 2024

**MEMORANDUM TO: Jason Van Nice, P.E., Chief  
Bureau of Construction & Materials**

We are handing you Final Plans for the project noted below for the August 2024 letting.

\_\_\_\_\_  
Ryan Bowman/ Peter Tobaben  
Design Squad

63-66 KA-5729-01  
ACSTP-A572(901)  
\_\_\_\_\_  
Project Number

\_\_\_\_\_  
Nemaha  
County

\_\_\_\_\_  
0.682 miles  
Length

REMARKS:

Grading and Surfacing (Asphalt), Bridge, Signing, and Seeding

Bridge #023 (Turkey Creek) on K-63 in Nemaha County located 7.64 miles North of West Junction US-36/K-63.

APPROVED:

  
\_\_\_\_\_  
For: CHIEF, BUREAU OF ROAD DESIGN

BY		DATE
SURVEY	J. Bowen	2021
	B. Andrews	2024
	B. Andrews (Road) / C. Bettis (Bridge)	2024
	R. Bowman (Road) / P. Tobaben (Bridge)	2024
CADD TECHNICIAN		
DESIGNERS		
SQUAD		

Plotted by : Brogan.Andrews@ks.gov  
File : KA572901rtrt-01.dgn

16-APR-2024 17:29

INDEX OF SHEETS	
1	TITLE SHEET
2	SIGNATURE SEAL SHEET
3-4	TYPICAL SECTIONS
5	FOUNDATION TREATMENT & COMPACTION DETAIL SHEET
6	SALVAGED TOPSOIL
7	GENERAL NOTE
8-11	PLAN-PROFILE SHEETS
12	FIELD ENTRANCE PLAN-PROFILE SHEET
13	DITCH REALIGNMENT PLAN-PROFILE SHEET
14	SECTION CORNER MONUMENTATION DETAIL SHEET
15	RIGHT OF WAY MONUMENT INSTALLATION DETAIL SHEET
16-18	PAVING DETAILS
19-25	GUARDRAIL DETAILS
26-29	METAL END SECTIONS
30	CONCRETE END SECTIONS
31-32	PIPE CULVERT SUMMARY
33-66	BRIDGE NO. 63-66-54.90 (048)
67	SUMMARY OF QUANTITIES
68	PROJECT SURFACING
69-80	TEMPORARY EROSION AND POLLUTION CONTROL
81-101	SIGNING
102-104	CONSTRUCTION SEQUENCING
105-118	TRAFFIC CONTROL
119-124	CONCRETE SAFETY BARRIER (TEMPORARY)
125-128	TEMPORARY WIDENING CROSS SECTIONS
129-139	K-63 CROSS SECTIONS
140	FIELD ENTRANCE CROSS SECTIONS
141	DITCH REALIGNMENT CROSS SECTIONS

DESIGN DESIGNATION

AADT (2024) = 1,250 vpd  
AADT (2044) = 1,550 vpd  
DHV = 11 %  
D = 60 %  
T = 22.5 %  
V = 60 mph  
C of A = None  
Clear Zone = 30'

CONVENTIONAL SIGNS

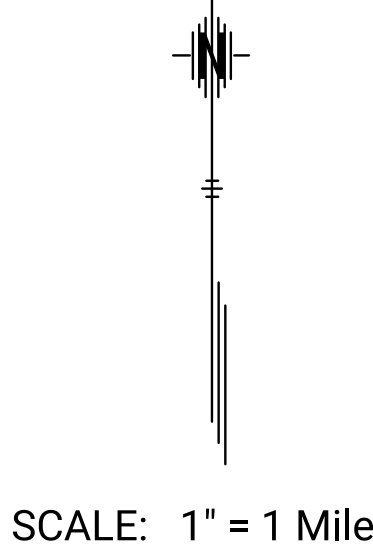
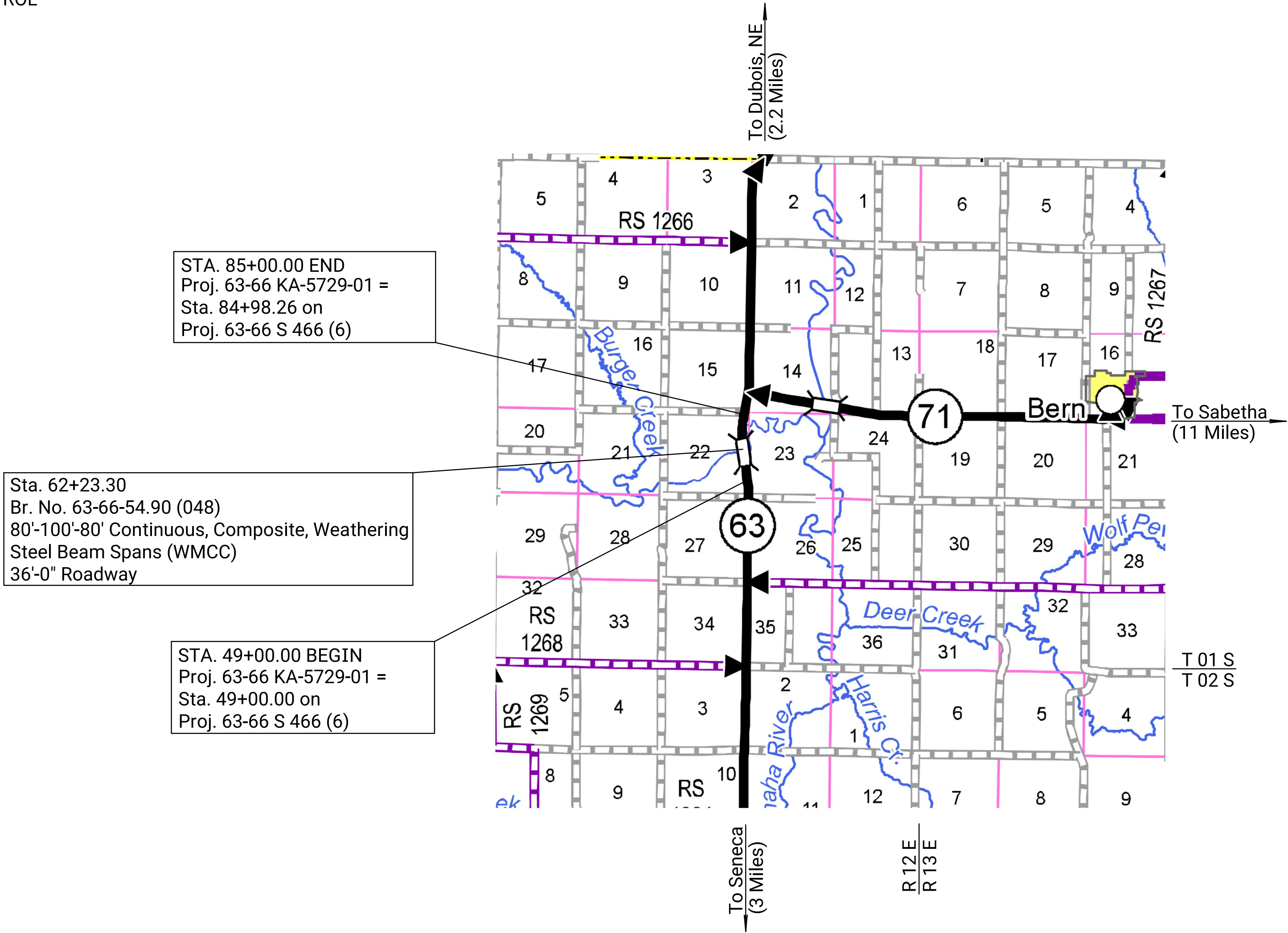
COUNTY LINE	-----	CENTER LINE OF PROJECT	-----
CITY LIMITS	=====	TERRACE	-----
STATE OR NATIONAL LINE	-----	CULVERTS	-----
TOWNSHIP, SECTION or GRANT LINE	-----	DROP INLET & STORM SEWER	-----
PROPERTY LINE	-----	ACCESS CONTROL	-----
HIGHWAY FENCE	-----	POWER POLE	-----
EXISTING FENCE	-----	TELEPHONE POLE	-----
GUARDRAIL	-----	MARSH	-----
CONSTRUCTION LIMITS	-----	HEDGE	-----
RIGHT OF WAY LINE	-----	TREES	-----
TRAVELED WAY	-----	PROFILE ELEVATION	-----
RAILROADS	-----	STREAM or CREEK	-----

STATE OF KANSAS  
DEPARTMENT OF TRANSPORTATION



PLAN AND PROFILE OF PROPOSED  
STATE HIGHWAY

FEDERAL AID PROJECT  
NEMAHA COUNTY  
K-63



PROJ. NO. 63-66 KA-5729-01  
FED. AID PROJ. NO. ACSTP-A572(901)

GRADING AND SURFACING (ASPHALT)  
BRIDGE  
SIGNING  
SEEDING

NOTE:  
TRAFFIC TO BE CARRIED THROUGH STAGED  
CONSTRUCTION. SEE SH. NO. 102-104 FOR  
CONSTRUCTION SEQUENCE DETAILS

Approved:	Jul 17, 2024
	Date
	<i>[Signature]</i>
	State Transportation Engineer
By:	<i>[Signature]</i>
	Chief, Bureau of Road Design
KANSAS DEPARTMENT OF TRANSPORTATION	



Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:32  
File : KA572901rss048-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	2	141

<div><p>Jul 15, 2024</p></div> <div>Name: Ryan Bowman Co. Name: KDOT Sheet Range: 1-32, 67-80, 102-104,119-141</div>	<div><p>Jul 15, 2024</p></div> <div>Name: Peter Tobaben Co. Name: KDOT - Bridge Design Sheet Range: 33-66</div>	<div><p>Jul 16, 2024</p></div> <div>Name: Nathan Jeffries, PE Co. Name: KDOT - Permanent Signing Sheet Range: 81-101</div>	<div><p>July 17, 2024</p></div> <div>Name: Amanda Jeanne Anderson, PE, PTOE Co. Name: Garver - Temporary Traffic Control Sheet Range: 105 - 118</div>				

01	01-24-18	Initial Release	A.L.R.	S.W.K.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
Signature Seal Sheet				
RD048				
FHWA APPROVAL		APPD.		
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	
		Scott W. King		

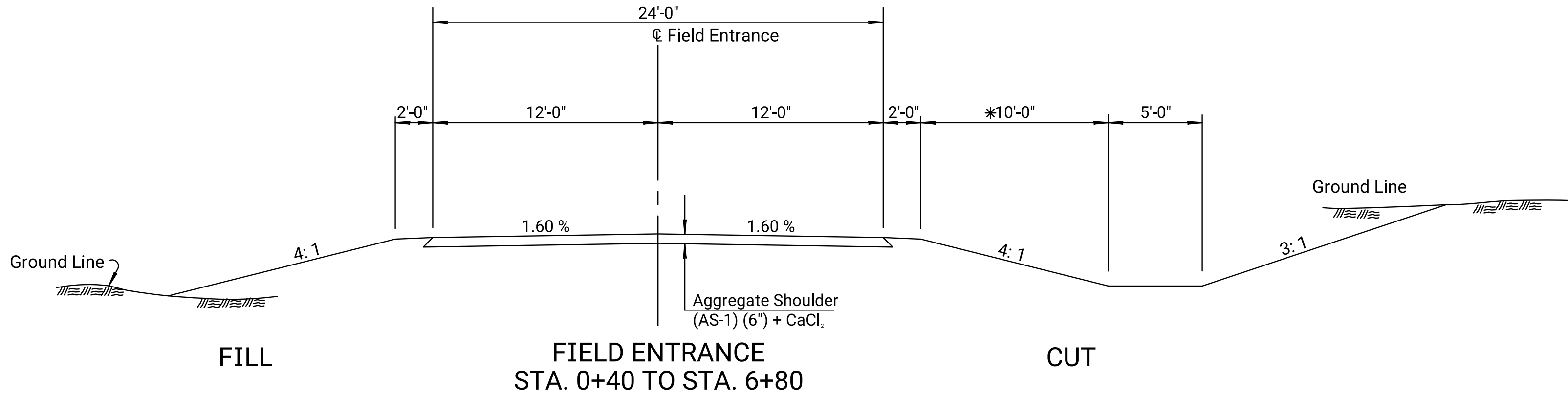
KDOT Graphics Certified



Guide to designer: Use this sheet when subgrading in rock or shale is required.  
See Soils and Geology Reports for additional plan notes.

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:34  
File : KA572901rtrts-02.dgn

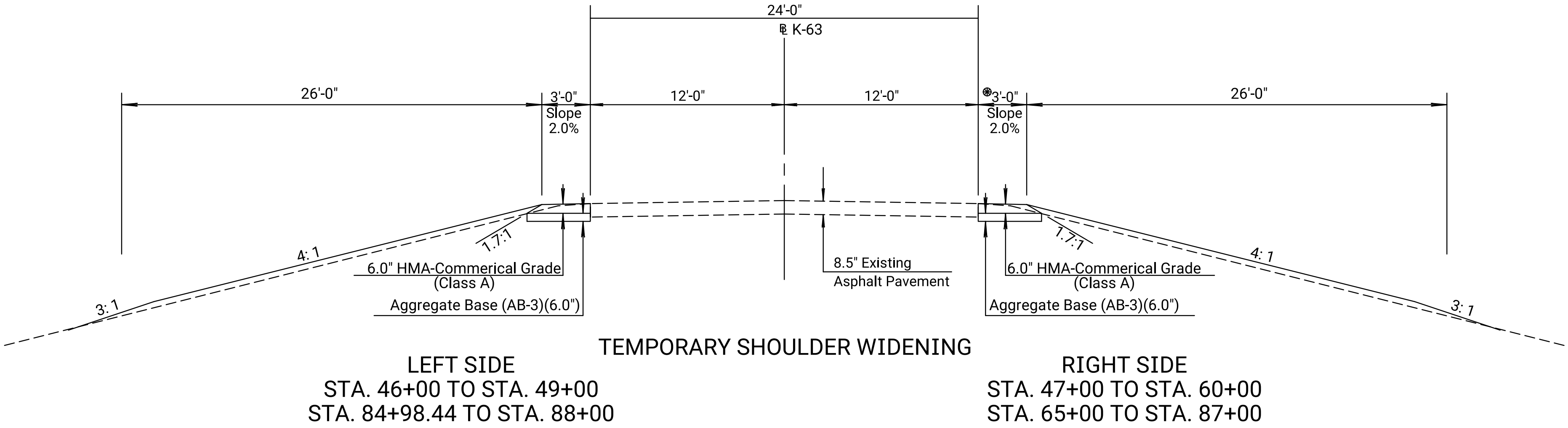
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	4	141



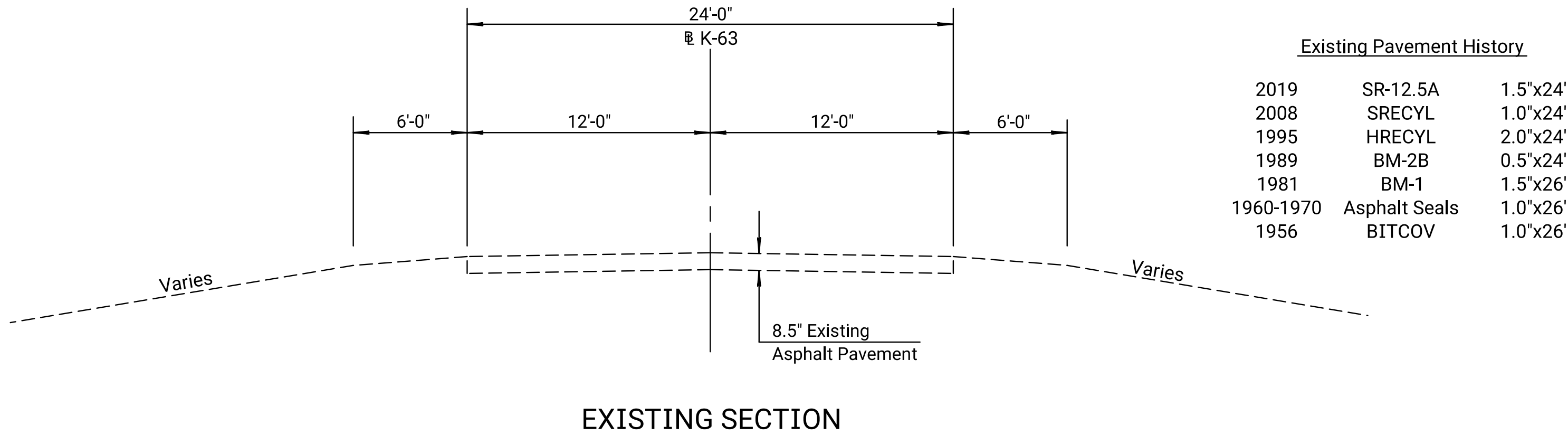
\* Dimensions and slopes for standard ditches and fills. See plan and cross-sections for variations.

Note: Intersection of all slope lines shall be softened and rounded for pleasing appearance.

Ditch Plugs within the appropriate clear zone shall have side slopes of 10:1 or flatter.



● Varies from 3'-0" to 5'-0"  
See Sh. No. 102



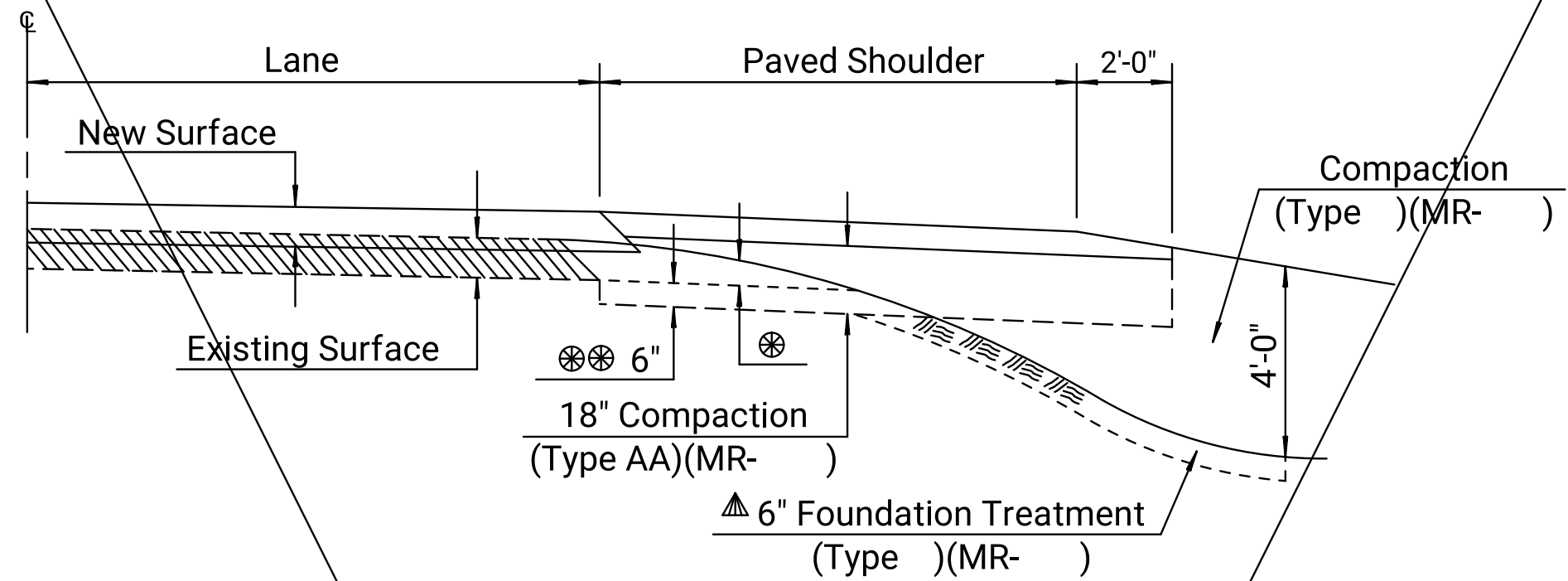
Existing Pavement History		
2019	SR-12.5A	1.5"x24'
2008	SRECYL	1.0"x24'
1995	HRECYL	2.0"x24'
1989	BM-2B	0.5"x24'
1981	BM-1	1.5"x26'
1960-1970	Asphalt Seals	1.0"x26'
1956	BITCOV	1.0"x26'

26	11-10-04	Changed slope labels to percent	S.W.K.	J.O.B.
25	05-10-00	Rev. Ditch Plug Slope 10:1	R.J.S.	J.O.B.
24	02-13-98	Rev. sd.rd.& ent. det. & fill foreslope	R.J.S.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TYPICAL SECTIONS FIELD ENTRANCE & TEMPORARY WIDENING				
RD602				
FHWA APPROVAL		06-10-05	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	W.L.H.



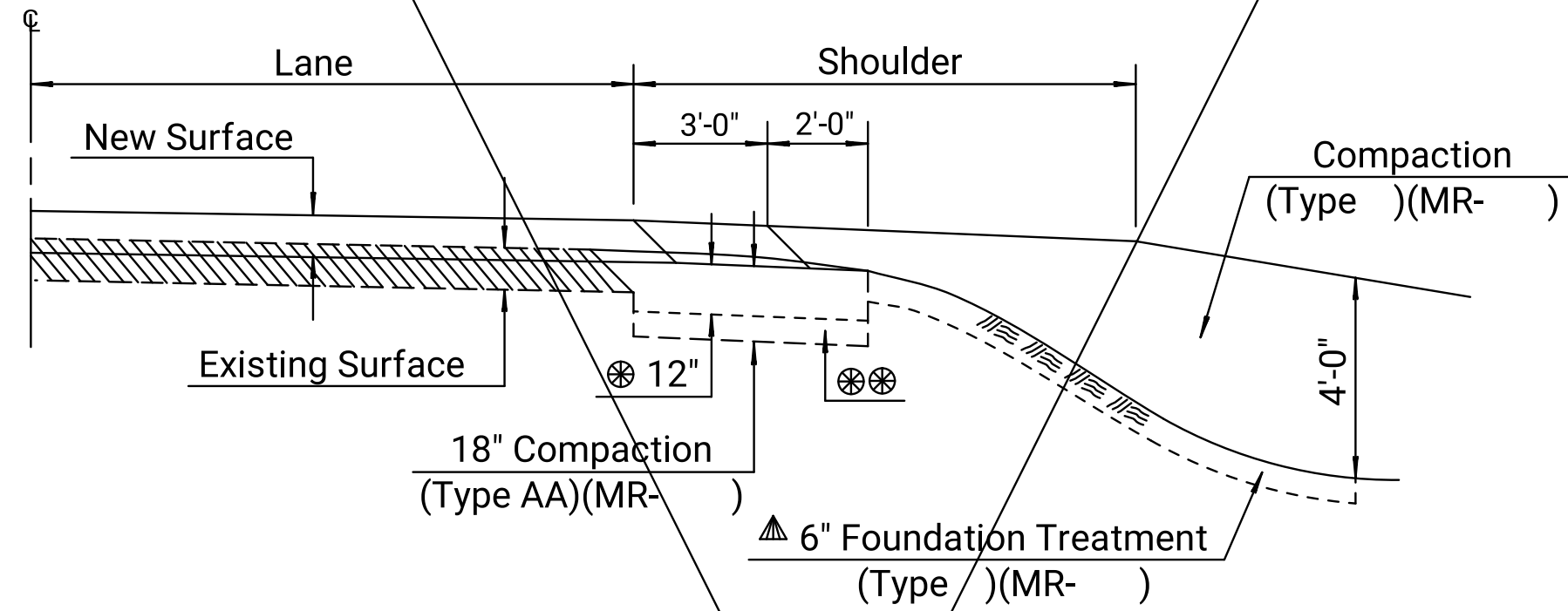
Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:31  
File : KA572901rtrts-03.dgn

## REHABILITATION



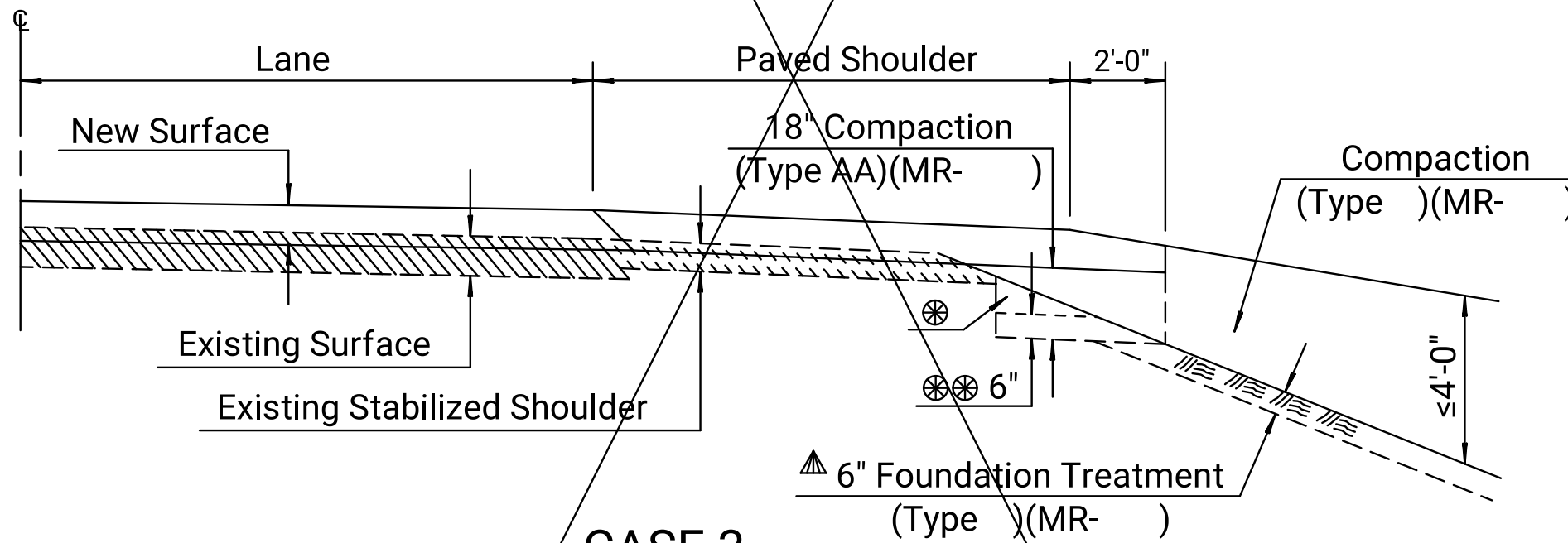
CASE 1

Overlay with Paved Shoulder



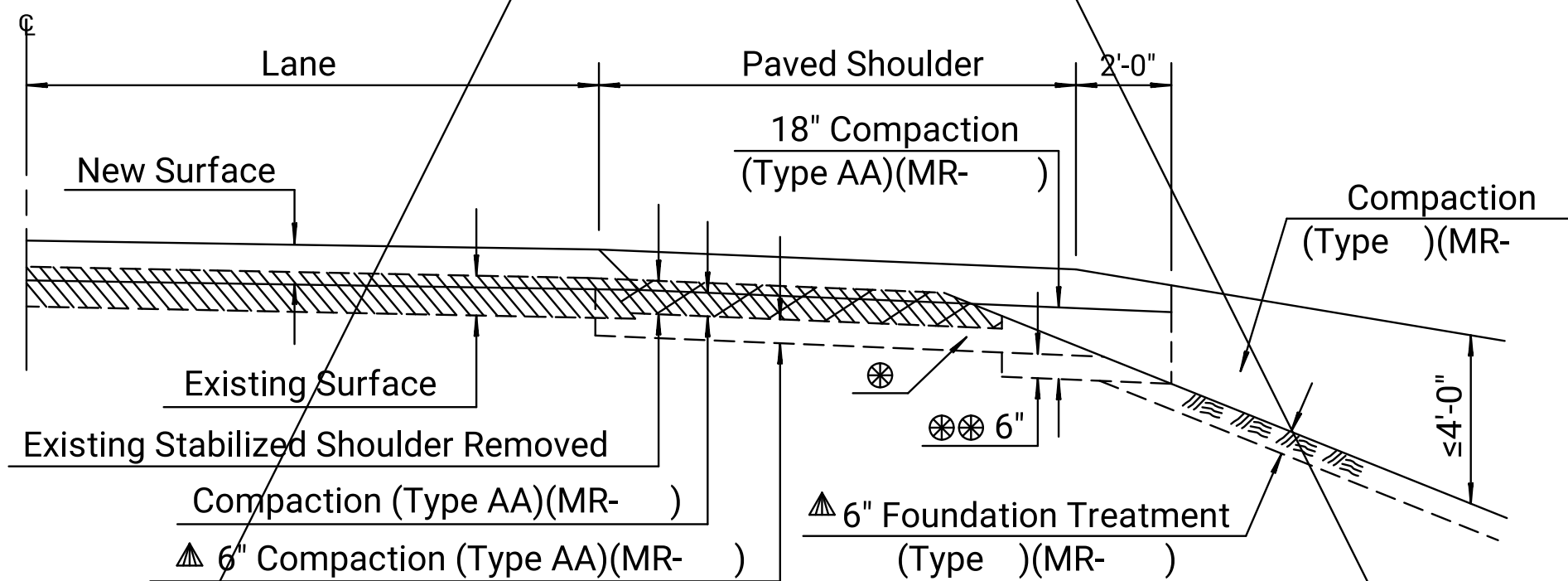
CASE 2

Overlay with Composite Shoulder



CASE 3

Overlay with Existing Paved Shoulder



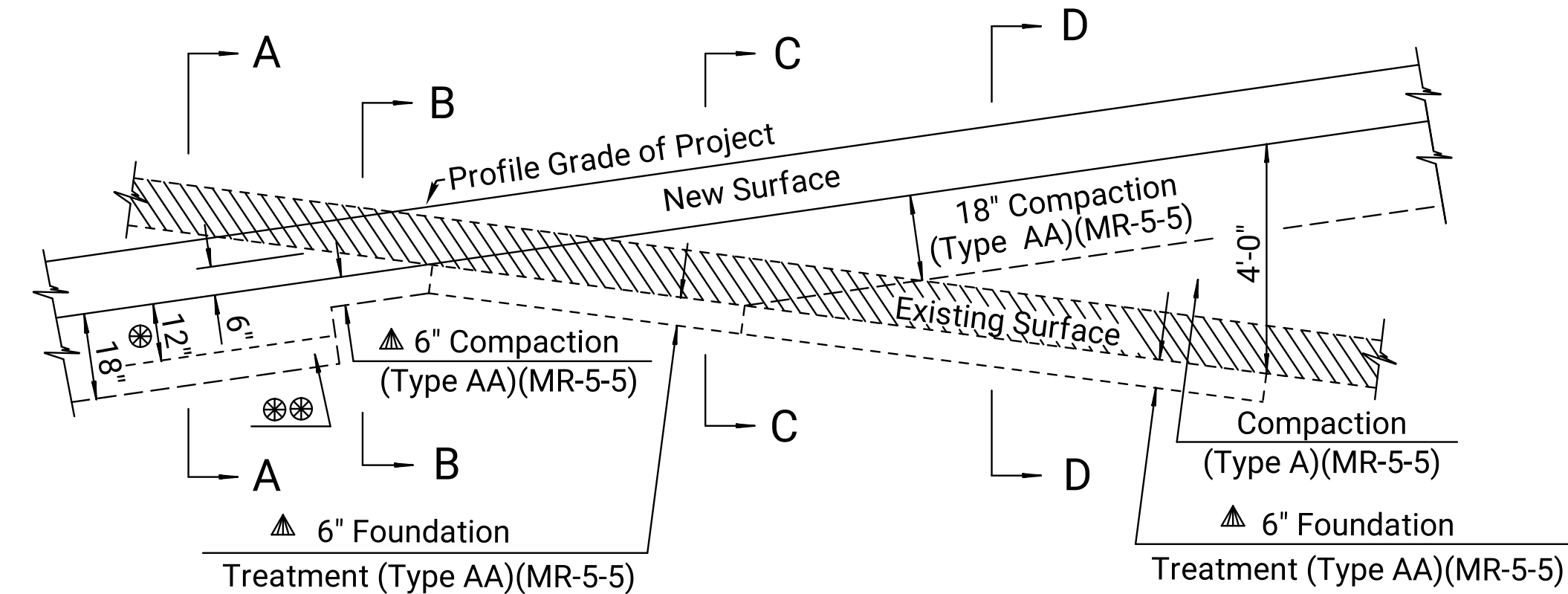
CASE 4

Overlay with Shoulder Replacement

- ⊗ Excavation thru Cuts not Subgraded
- ⊗⊗ The lower 6" of Compaction is subsidiary.
- ▲ Compaction of this material shall be subsidiary.

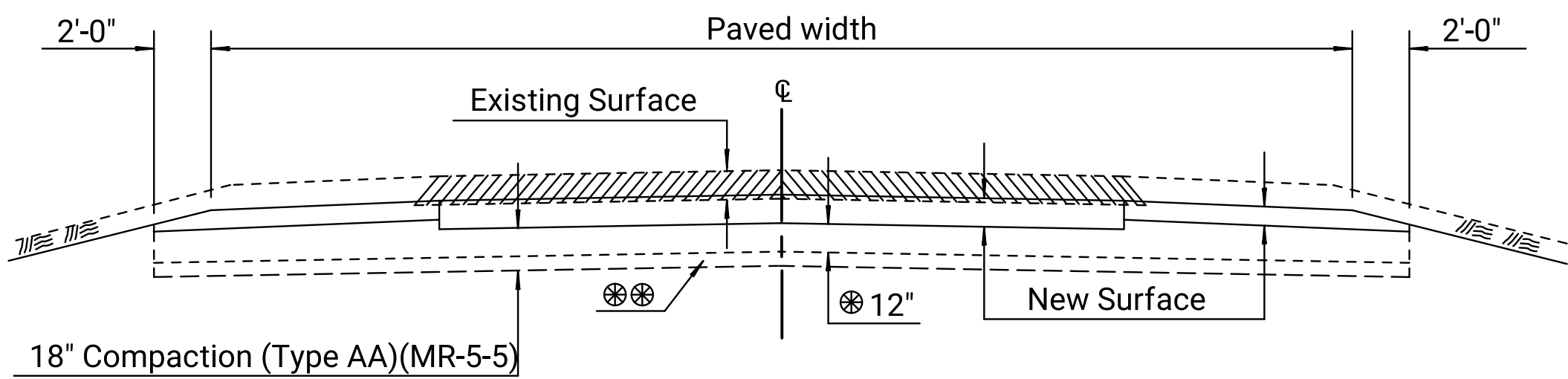
Note: These are 4 general cases. Specific compaction requirements are determined on a project-by-project basis.

## RECONSTRUCTION

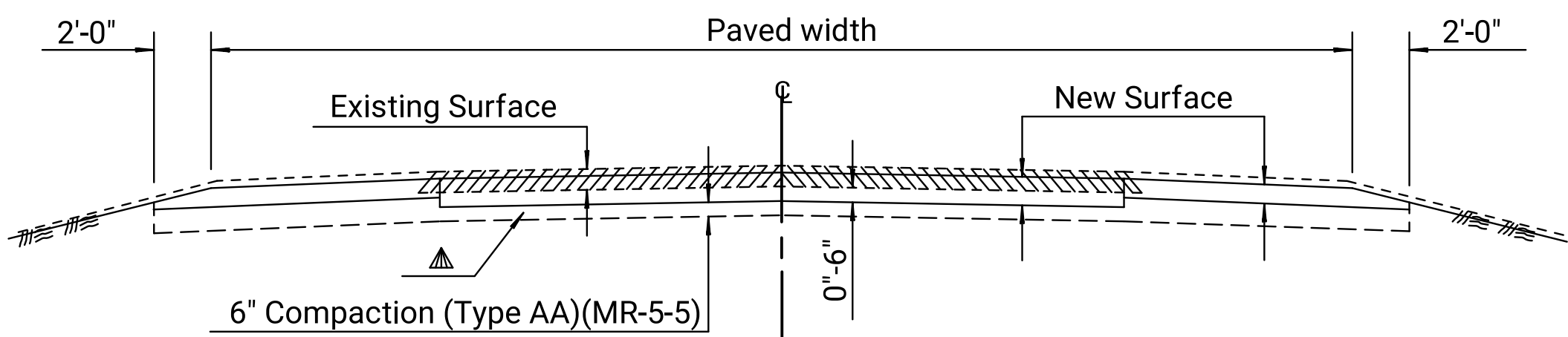


PROFILE

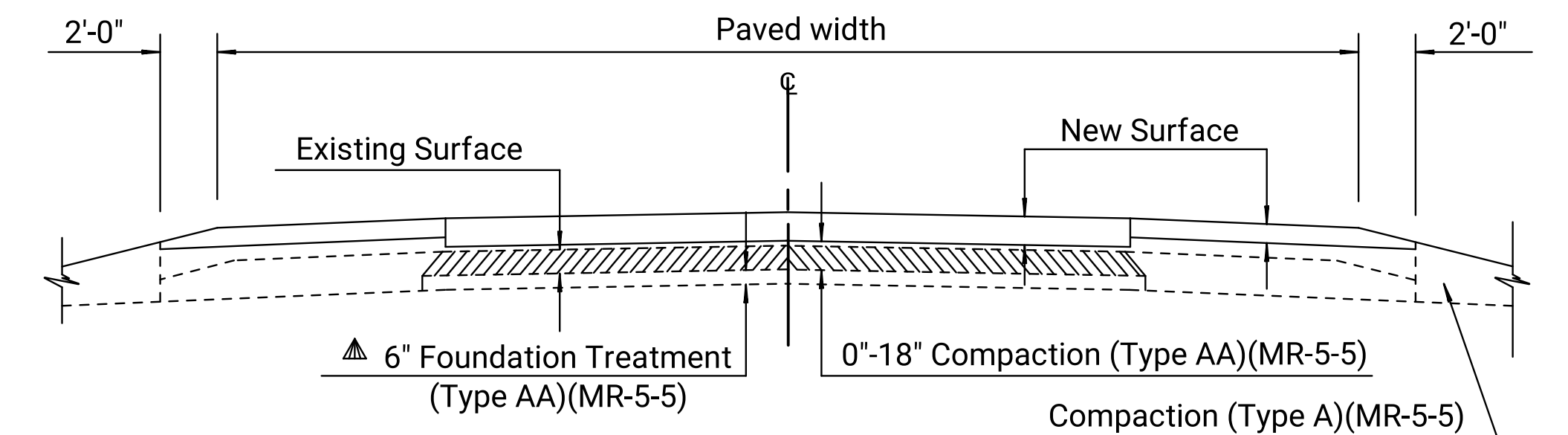
- ⊗ Excavation thru Cuts not Subgraded
- ⊗⊗ The lower 6" of Compaction is subsidiary.
- ▲ Compaction of this material shall be subsidiary.



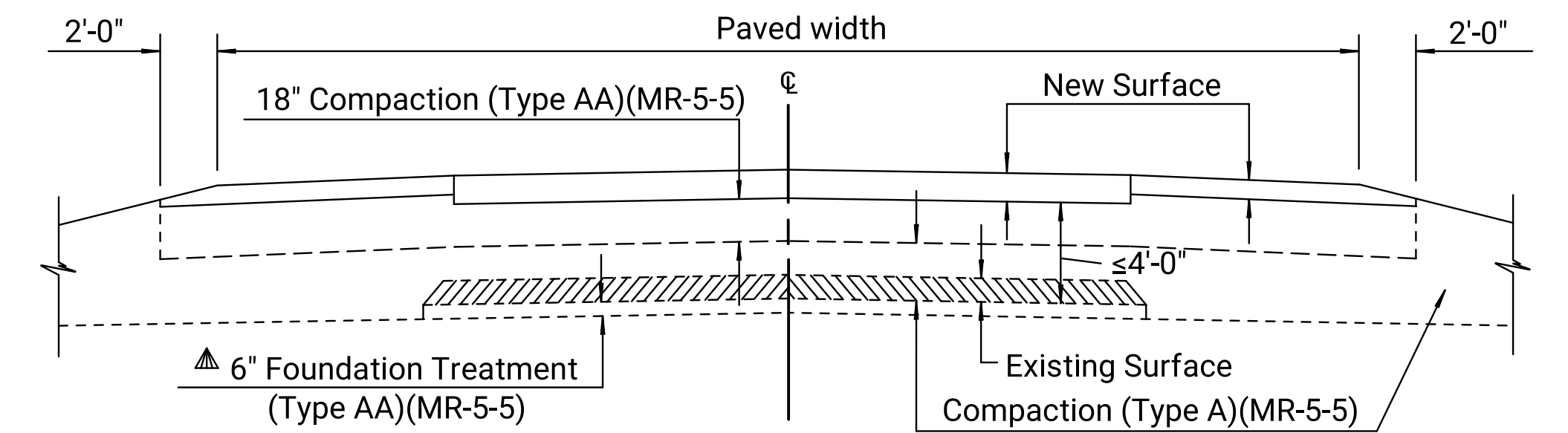
SECTION A-A



SECTION B-B

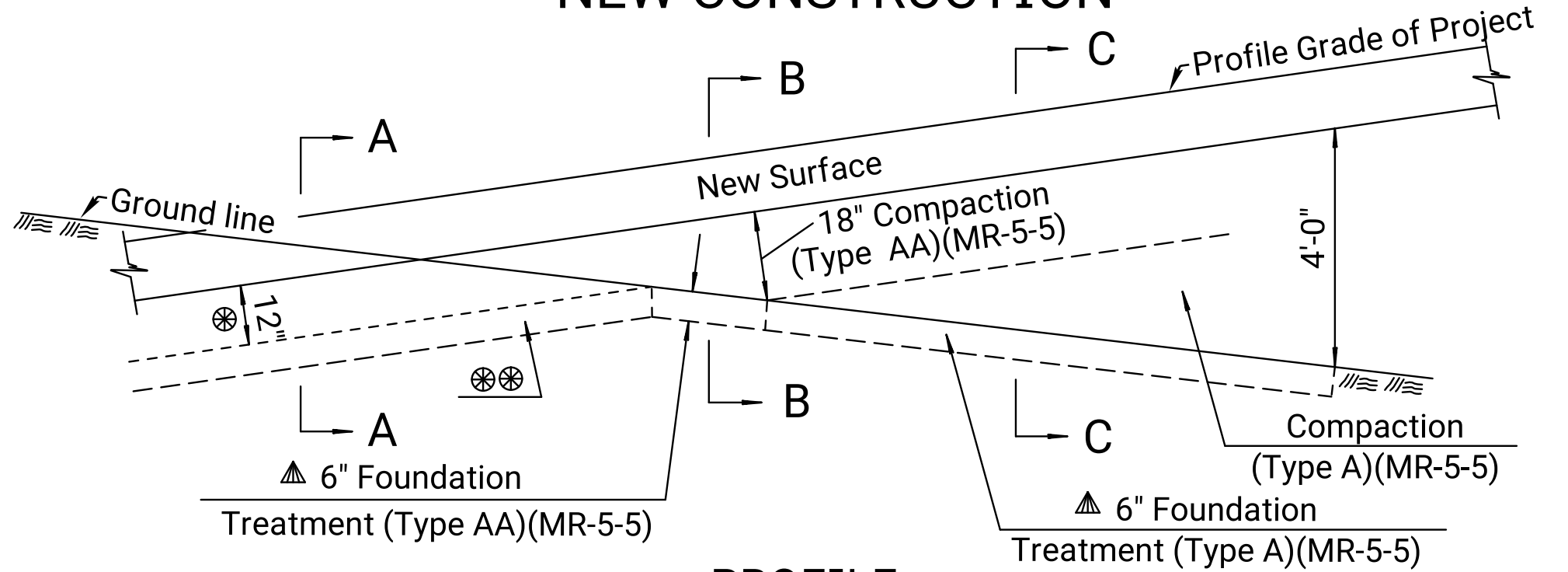


SECTION C-C



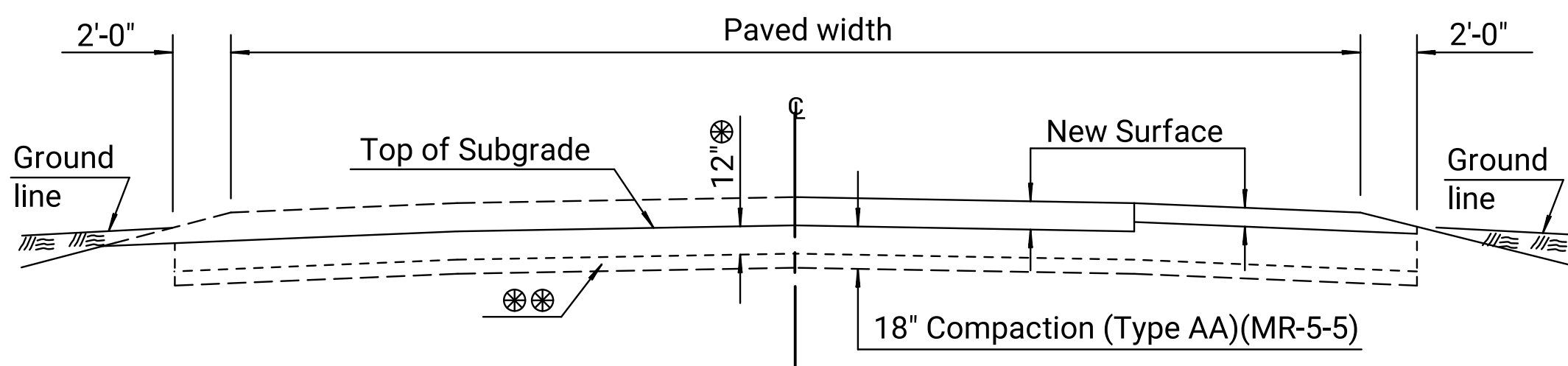
SECTION D-D

## NEW CONSTRUCTION

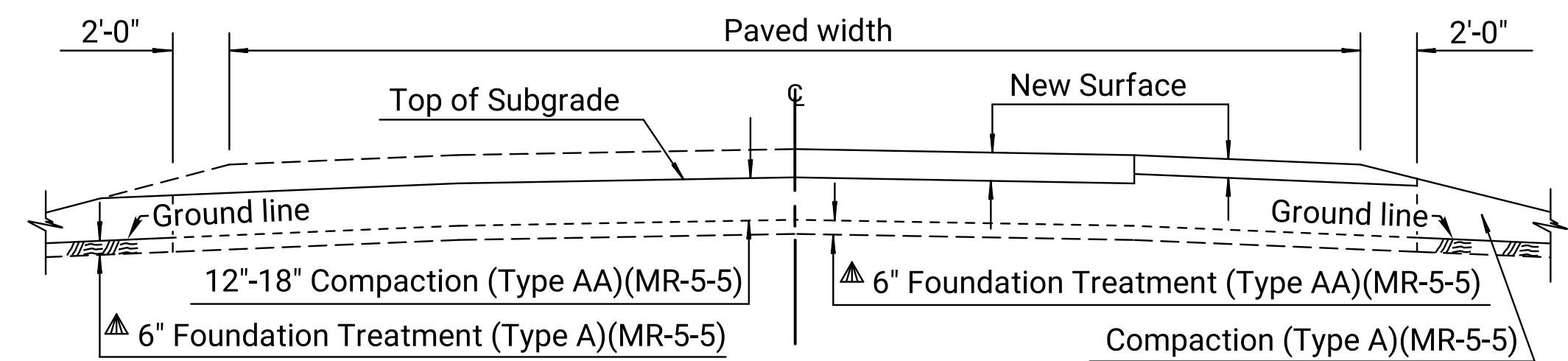


PROFILE

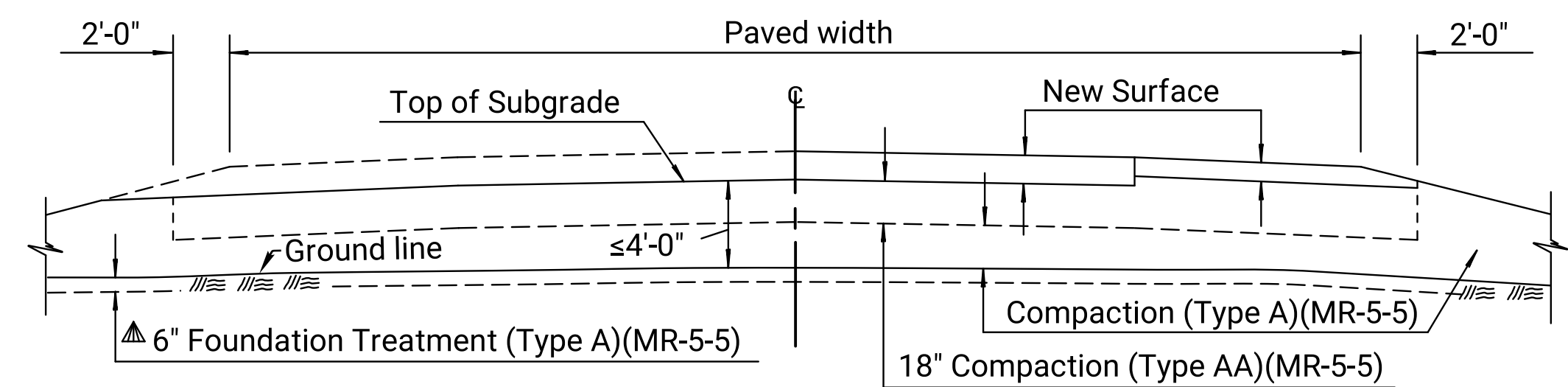
- ⊗ Excavation thru Cuts not Subgraded
- ⊗⊗ The lower 6" of Compaction is subsidiary.
- ▲ Compaction of this material shall be subsidiary.



SECTION A-A



SECTION B-B



SECTION C-C

### General Note

For materials designated to be subgraded, compaction of soils, including shales, designated for backfill refer to Standard Drawing RD605A for details.

Unless otherwise noted on the Plans, compact all embankment, including side roads and entrances.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	5	141

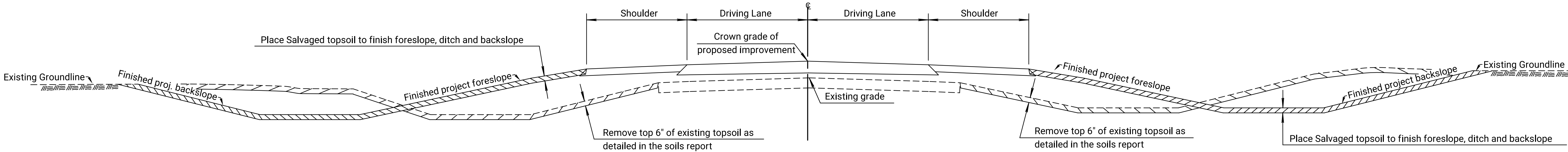
NO.	DATE	REVISIONS	BY	APPROD
05	10-17-11	Revised General Note	S.W.K.	J.O.B.
04	01-05-10	Added additional subsidiary comp.	S.W.K.	J.O.B.
03	02-16-05	Redrawn, Rev. Recon. Sec. C-C & D-D	S.W.K.	J.O.B.
KANSAS DEPARTMENT OF TRANSPORTATION				
FOUNDATION TREATMENT & COMPACTION OF EARTHWORK				
RD605				
FHWA APPROVAL 12-05-11 APPD. James O. Brewer				
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

Note to Designer: Acceptable Topsoil locations on a project will be detailed in the Soils Report. The locations will be used in conjunction with the plans to measure a horizontal area in Sq. Yds of "Salvaged Topsoil" within the R/W limits.

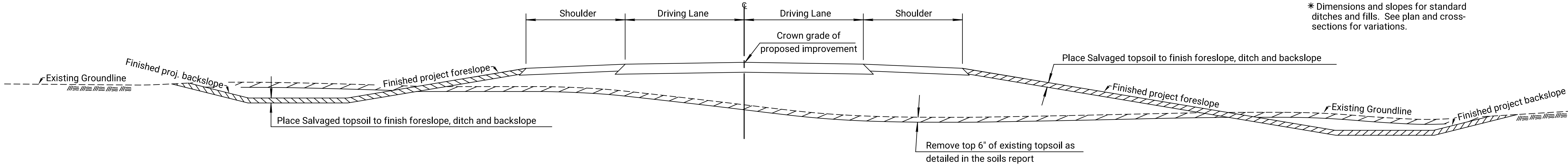
Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:30  
File : KA572901rss599a-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	6	141

GENERAL NOTE  
Adjust the cut and fill sections to accommodate the placement of the salvaged topsoil such that after placement the cross section will be at the final grade as shown on the plans.  
Salvaging, Stockpiling and Placing Topsoil bid as "Salvaged Topsoil" in Square Yards. See KDOT Standard Specifications for details.  
Soften and round the intersection of all slope lines for pleasing appearance.



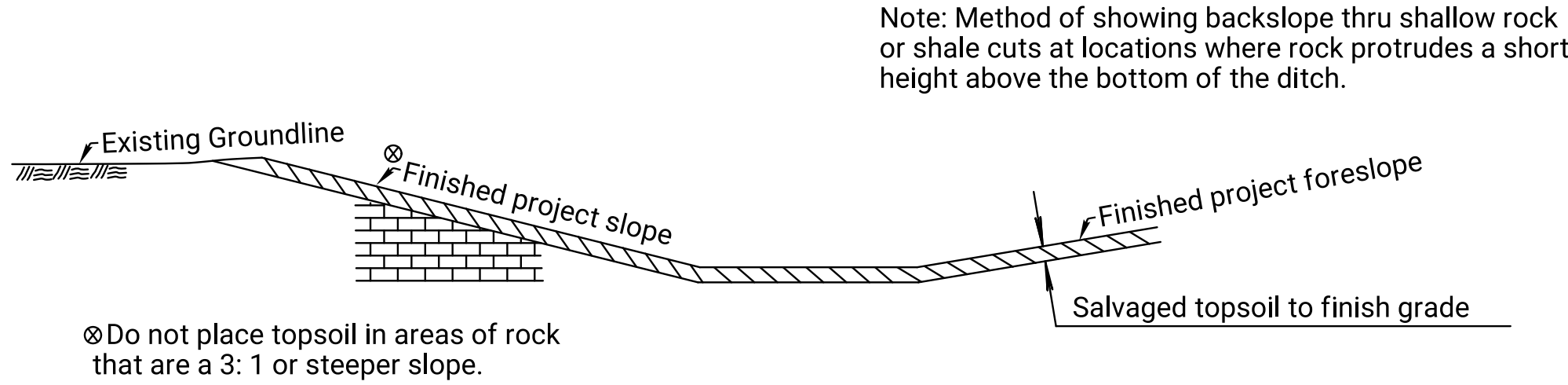
RECONSTRUCTION/REHABILITATION OF EXISTING ROADWAY  
(Removal and Placement of Salvaged Topsoil)



\* Dimensions and slopes for standard ditches and fills. See plan and cross-sections for variations.

- LEGEND
- Topsoil to be Salvaged
  - Placement of Salvaged Topsoil

NEW ROADWAY ALIGNMENT  
(Removal and Placement of Salvaged Topsoil)



CUT SECTION

01	12-16-09	Initial Release	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
SALVAGED TOPSOIL				
RD599A				
FHWA APPROVAL		APPD.		
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	S.W.K.

GENERAL NOTE

THE GEOLOGICAL INFORMATION SHOWN ON THESE PLANS IS FROM STUDIES MADE IN THE FIED AND PRESENTS THE BEST INFORMATION AVAILABLE TO THE KANSAS DEPARTMENT OF TRANSPORTATION.

AT BORROW AREA LOCATIONS ADJACENT TO THE RIGHT OF WAY, UTILITY POLES MAY BE SET AT THE PERMANENT LOCATIONS PRIOR TO CONSTRUCTION AS APPROVED BY THE ENGINEER PROVIDED A MINIMUM VERTICAL CLEARANCE, IN ACCORDANCE WITH THE NATIONAL ELECTRICAL SAFETY CODE, IS OBTAINED. THE CONTRACTOR WILL BE REQUIRED TO WORK AROUND THESE POLES TO COMPLETE THE WORK.

ALL BORROW TO BE OBTAINED FROM AREAS PROVIDED BY THE CONTRACTOR SHALL BE APPROVED BY THE ENGINEER, BOTH AS TO SUITABILITY OF MATERIAL AND SITE LOCATION. LOCATIONS WHICH, IN THE OPINION OF THE ENGINEER, CONTAIN UNSUITABLE MATERIAL OR WILL LEAVE AN UNSIGHTLY APPEARANCE ON THE PROJECT WILL NOT BE APPROVED. ALL BORROW/WASTE LOCATIONS SHALL BE SUBMITTED FOR CLEARANCE FROM THE KANSAS HISTORICAL SOCIETY AND THE KANSAS DEPARTMENT OF WILDLIFE AND PARKS PRIOR TO ANY EXCAVATION OR WASTING OF MATERIAL. THE CONTRACTOR IS RESPONSIBLE FOR ACQUIRING ANY PERMITS AND OTHER CLEARANCES THAT ARE REQUIRED.

CHANNELS SHALL BE CUT AT BOX CULVERTS (UNLESS OTHERWISE NOTED) TO FLOW LINE ELEVATIONS AND TO A WIDTH OF ONE FOOT OUTSIDE OF EACH OUTSIDE WALL AND WITH SLOPES 2 TO 1 PRIOR TO CONSTRUCTION OF THE CULVERT.

EMBANKMENT QUANTITIES FOR INITIAL CONSOLIDATION AND SETTLEMENT SHOWN IN THE EARTHWORK QUANTITIES ARE SUBSIDIARY TO OTHER EARTHWORK ITEMS. MATERIAL FOR THE EMBANKMENT IS INCLUDED IN THE EXCAVATION QUANTITIES.

EXCAVATION REQUIRED FOR PLACING SELECT SOIL IS INCLUDED IN THE COMMON EXCAVATION QUANTITIES.

WHERE EASEMENTS ARE SHOWN ON RAILROAD RIGHT OF WAY, THE CONTRACTOR SHALL BE REQUIRED TO WORK AROUND AND NOT DISTURB THE RAILROAD COMMUNICATION OR SIGNAL POLES OR LINES.

EXCAVATION SHOWN TO BE WASTED SHALL BE WASTED ON SITES PROVIDED BY THE CONTRACTOR. THESE SITES SHALL BE APPROVED BY THE ENGINEER AS TO SUITABILITY, APPEARANCE, AND SITE LOCATION. LOCATIONS THAT, IN THE OPINION OF THE ENGINEER, WILL LEAVE UNSIGHTLY APPEARANCE WILL NOT BE APPROVED.

ALL TREES, HEDGE ROWS, SHELTERBELTS, AND WOODY SHRUBS NOT SHOWN TO BE REMOVED AND LOCATED BETWEEN THE CONSTRUCTION LIMITS AND THE RIGHT-OF-WAY LINE OR EASEMENT LINE SHALL BE SPARED UNLESS DIRECTED BY THE ENGINEER TO BE REMOVED. ALL TREES WITHIN THE APPROPRIATE CLEAR ZONE SHALL BE REMOVED.

ALL SAWCUTS SHALL BE FULL DEPTH OR AS APPROVED BY THE ENGINEER AND SHALL NOT BE PAID FOR DIRECTLY; BUT SHALL BE SUBSIDIARY TO OTHER BID ITEMS ON THE CONTRACT.

A GROSS VMF OF 0.82 WAS USED TO COMPUTE EARTHWORK QUANTITIES FOR THIS PROJECT. THIS FACTOR INCLUDES QUANTITIES FOR INITIAL CONSOLIDATION AND SETTLEMENT.

CONTRACTOR TO MAINTAIN ACCESS TO PROPERTY OWNERS AT ALL TIMES AS DIRECTED BY THE ENGINEER.

THE CONTRACTOR SHALL FIELD VERIFY LOCATION AND DEPTH OF ALL UTILITY LINES. THE INFORMATION SHOWN IN THESE PLANS CONCERNING TYPE AND LOCATION OF UNDERGROUND UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL INCLUSIVE.

SOIL FOR EMBANKMENT CONSTRUCTION: ALL SOIL USED IN THE TOP 18" INCHES OF THE EMBANKMENT SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:  $8 \leq PI \leq 30$  AND  $20 \leq LL \leq 55$ . SOILS WHICH CONTAIN SUBSTANTIAL ORGANIC MATERIAL, SUCH AS THOSE CLASSIFIED AS OL OR OH ACCORDING TO THE UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D2487) SHOULD NOT BE USED TO CONSTRUCT THE EMBANKMENT OR SUBGRADE. THE ORGANIC MATERIAL MAY BE USED AS SELECT SOIL TO CAP THE SIDESLOPES OF THE EMBANKMENT.

KDOT WILL RETAIN ALL MILLED MATERIAL FROM THE EXISTING AND TEMPORARY PAVEMENT. MILLINGS SHALL BE HAULED BY THE CONTRACTOR TO THE KDOT MIXING STRIP LOCATED ON K-63 APPROXIMATELY 1.6 MILES NORTH OF SENECA AS DIRECTED BY THE ENGINEER. TRANSPORTING OF THIS MATERIAL SHALL BE PAID FOR UNDER THE BID ITEM "TRANSPORTING SALVAGEABLE MATERIAL". TRANSPORTING THIS MATERIAL WILL BE FEDERALLY NON-PARTICIPATING.

KDOT WILL RETAIN ALL EXISTING GUARDRAIL AND SHALL BE LEFT ON SITE TO BE PICKED UP BY KDOT FORCES.

UTILITY OWNERS

- T1ATT (Fiber Optic Line)  
(800) 778-9140
- E1Evergy (Overhead Power Lines)  
(800) 778-9140
- W1Nemaha Co. RWD#3 (Water Line)  
(785) 336-3522
- F1Blue Valley (Fiber Cable)  
(785) 799-3311

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	7	141



DATE	6/2022
BY	B. Andrews R. Bowman
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan.Andrews@ks.gov  
File : KA572901ref-01.dgn  
16-APR-2024 17:27

ℳ PT Sta. 44+24.23  
N 813,956.465 E 10,415,206.201

1. Found ½" rebar 0.3' deep
2. Spike & KDOT wshr. Top metal FCPO 2.2' NW
3. Conc. nail & KDOT wshr. N face power pole 68.6' WNW
4. TW of K-63 highway

Traverse Point #100  
N 815,193.178 E 10,415,018.175  
1. Set ½" rebar w/ KDOT cap, flush  
2. 18.54' Lt. of ℳ Sta. 56+75.01

ℳ POT Sta. 57+00.00  
N 815,220.447 E 10,415,033.154

1. Set ½" rebar 0.3' deep
1. 1/2" rebar w/KDOT cap (TP 100) 31.0' SW
3. Mag nail & KDOT wshr. S face power pole 83.5' NW
4. TW of K-63 highway 0.3' E

Traverse Point #101  
N 815,616.682 E 10,414,960.438  
1. Set ½" rebar w/ KDOT cap, flush  
2. 18.30' Lt. of ℳ Sta. 61+02.44

ℳ PC Sta. 66+51.38  
N 816,163.030 E 10,414,904.109

1. Found ½" rebar
2. Conc. nail & KDOT wshr. N face power pole 148.7' W
3. ℳ top N end CMP 173.8' SW
4. TW K-63 highway 0.3' E

Traverse Point #102  
N 815,917.806 E 10,414,955.584  
1. Set ½" rebar w/ KDOT cap, flush  
2. 17.74' Rt. of ℳ Sta. 64+01.44

ℳ POC Sta. 75+00.00  
N 817,009.250 E 10,414,851.572

1. Set ½" rebar 0.3' deep
2. Conc. nail & KDOT wshr. N face power pole 133.3' SW
3. ℳ top N end CMP 185.7' SSE
4. TW of K-63 highway 0.3' W

Traverse Point #103  
N 816,339.888 E 10,414,863.508  
1. Set ½" rebar w/ KDOT cap, flush  
2. 19.08' Lt. of ℳ Sta. 68+31.54

Traverse Point #104  
N 817,491.122 E 10,414,856.283  
1. Set ½" rebar w/ KDOT cap, flush  
2. 21.32' Lt. of ℳ Sta. 79+80.67

ℳ PI Sta. (Bk.) = 75+34.89  
ℳ PI Sta. (Ahd.) = 75+21.08  
Δ 17°31'55" (RT)

- N 817,038.380 E 10,414,784.268
1. Found ½" rebar 0.3' deep
2. Conc. nail & KDOT wshr. N face power pole 118.0' SSW
3. Conc. nail & KDOT wshr. S face power pole 93.0' NNW
4. TW of K-63 highway 73.4' ESE

ℳ PI Sta. (Bk.) = 75+15.35  
ℳ PI Sta. (Ahd.) = 75+02.10  
Δ 17°31'55" (RT)  
N 817,018.136 E 10,414,780.794  
1. 70.88' Lt. of ℳ Sta. 75+07.93  
2. Nothing Set

ℳ PC Sta. 66+67.25  
N 816,177.871 E 10,414,895.832  
1. 6.19' Lt. of ℳ Sta. 66+67.18  
2. Nothing Set

ℳ PT Sta. 59+91.80  
N 815,508.662 E 10,414,987.451  
1. 6.19' Lt. of ℳ Sta. 59+91.75  
2. Nothing Set

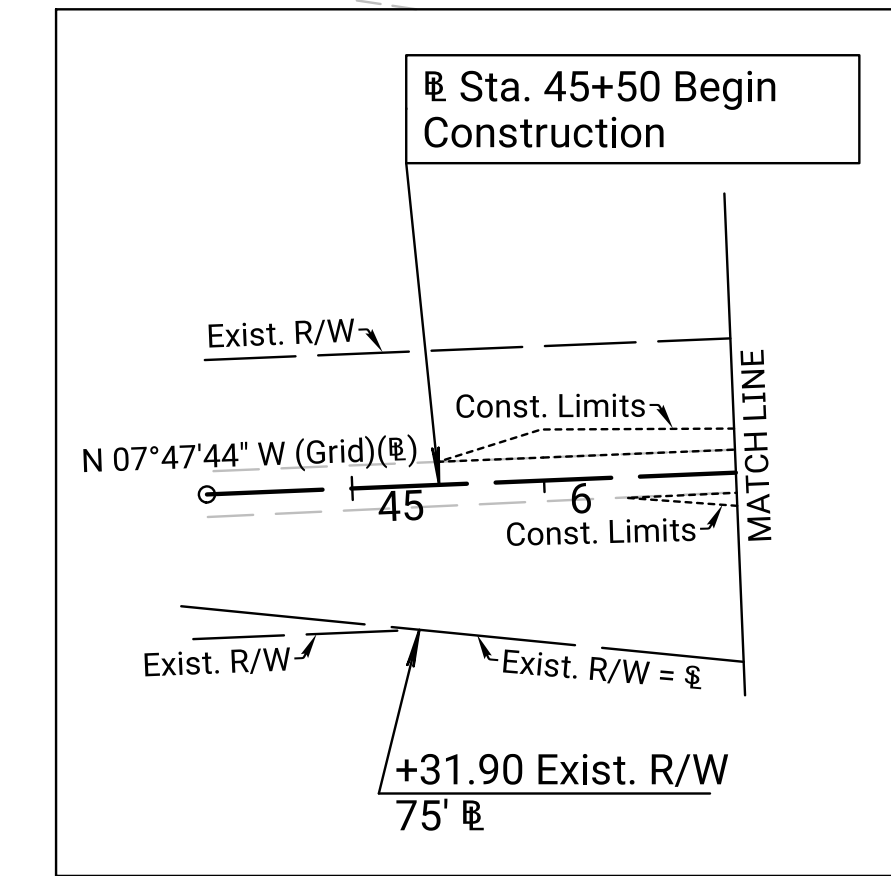
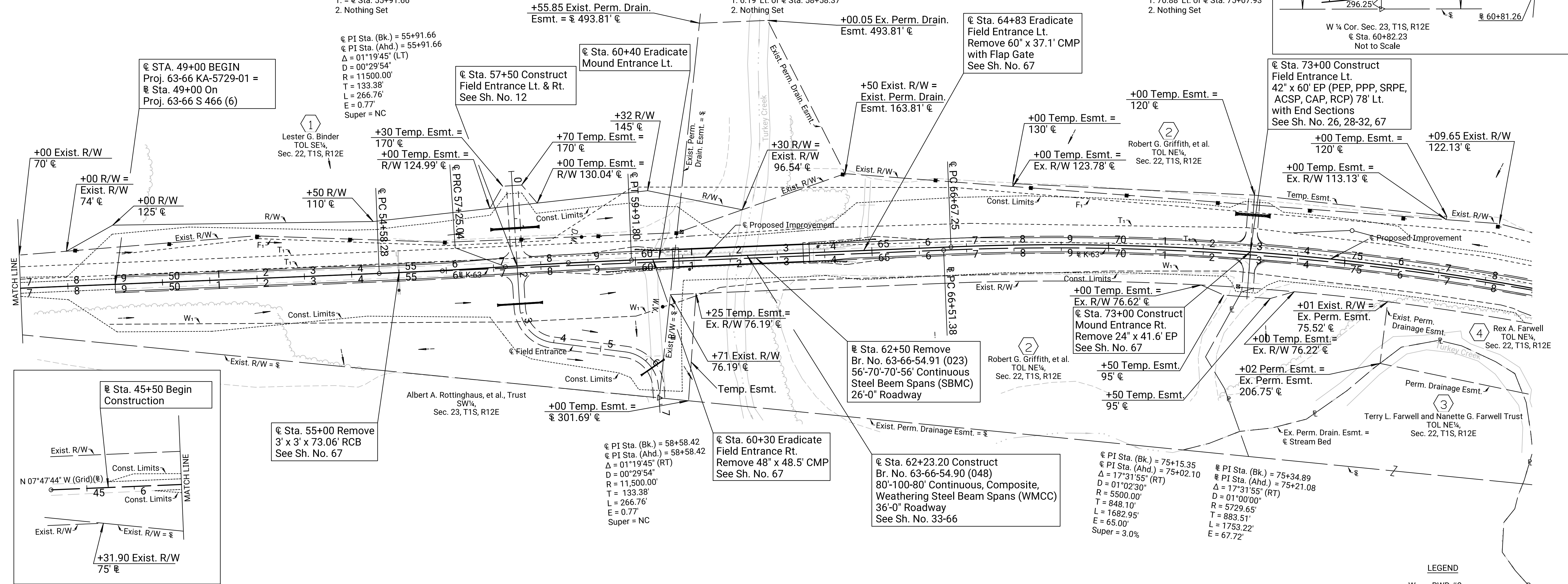
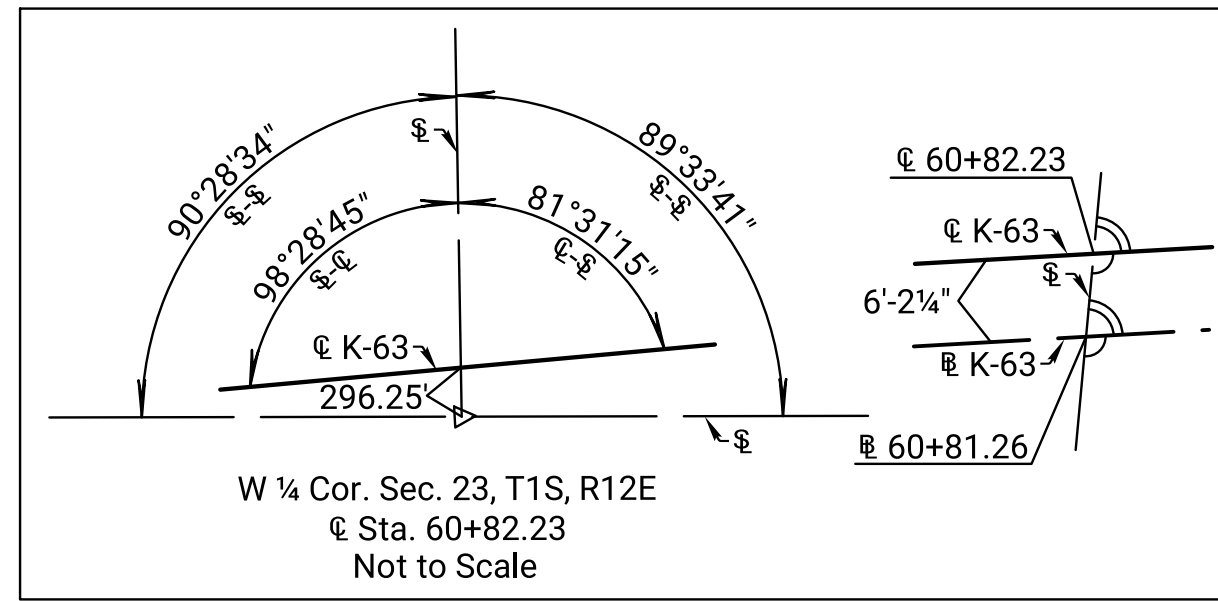
ℳ PI Sta. (Bk.) = 58+58.42  
ℳ PI Sta. (Ahd.) = 58+58.42  
Δ 01°19'45" (RT)  
N 815,376.510 E 10,415,005.543  
1. 6.19' Lt. of ℳ Sta. 58+58.37  
2. Nothing Set

ℳ PRC Sta. 57+25.04  
N 815,244.813 E 10,415,026.696  
1. 3.09' Lt. of ℳ Sta. 57+25.02  
2. Nothing Set

ℳ PI Sta. (Bk.) = 55+91.66  
ℳ PI Sta. (Ahd.) = 55+91.66  
Δ = 01°19'45" (LT)  
N 815,113.116 E 10,415,047.848  
1. = ℳ Sta. 55+91.66  
2. Nothing Set

ℳ PC Sta. 54+58.28  
N 814,980.964 E 10,415,065.941  
1. = ℳ Sta. 54+58.28  
2. Nothing Set

ℳ POT Sta. 47+00.00  
N 814,229.689 E 10,415,168.795  
1. = ℳ Sta. 47+00.00  
2. Nothing Set



#### PROJECT SURVEY CONTROL

HORIZONTAL PROJECT DATUM  
NAD83 (2011) KRCS Zone 13 Atchison

VERTICAL DATUM  
North American Vertical Datum NAVD88 (Geoid 12A)

S ¼ Cor. Sec. 22, T1S, R12E

- N 812,987.906 E 10,412,622.220
1. Found ½" bar with cap marked "LS1314", 0.5' deep
  2. 2,344.20' Lt of ℳ Tan. Sta. 87+30.72 NWΔ = 98°-57'-07"
  3. Spk. & wshr. top of fence post 26.5' SE
  4. Spk. & wshr. top of cor. post 23.4' SSW
  5. TW of 208th road 3.0' N

B.M. #11 Rivet top E hdwl. RCP  
37.2' Rt. of ℳ Sta. 54+98.5 Elev. = 1,038.56'

SW Cor. Sec. 23, T1S, R12E

- N 812,958.014 E 10,415,261.868
1. Found ½" rebar with KDOT cap, 0.3' deep
  2. 5.0' Rt of ℳ Tan. Sta. 34+23.42 NWΔ = 90°-03'-20"
  3. SW cor. W hub guard RCB 78.5' NW
  4. Center of conc. R/W marker post 93.4' SW
  5. TW of K-63 highway 4.4' W

B.M. #12 Square cut S end E hubguard of bridge  
18.3' Rt. of ℳ Sta. 61+13.5 Elev. = 1,046.50'

Center Cor. Sec. 22, T1S, R12E

- N 815,626.190 E 10,412,633.544
1. Found ½" bar with cap marked "APT LS1314", 0.3' deep
  2. 2,348.06' Lt of ℳ Tan. Sta. 60+81.26 NWΔ = 81°-31'-15"
  3. Spk. & wshr. top of cor. post 5.2' NE
  4. Spk. & wshr. top of brace post 4.5' NW
  5. Fence to W 3.0' N

B.M. #13 Square cut NW cor. W hubguard of bridge  
18.3' Lt. of ℳ Sta. 63+86.3 Elev. = 1,046.54'

W ¼ Cor. Sec. 23, T1S, R12E

- N 815,594.726 E 10,415,271.408
1. Found 3' x 12" limestone with ½" square bar on N side, 0.5' deep
  2. 289.99' Rt of ℳ Tan. Sta. 60+81.26 NWΔ = 81°-31'-15"
  3. R/W T post at ground 15.4' S
  4. Center S end CMP 92.2' SW
  5. TW of K-63 highway 290.5' W

B.M. #14 RR spike E face of power pole  
104.4' Lt. of ℳ Sta. 72+24.8 Elev. = 1,036.85'

N ¼ Cor. Sec. 22, T1S, R12E

- N 818,262.686 E 10,412,644.860
1. Found ½" bar with cap marked "APT LS1314", 0.1' deep
  2. Spk. & wshr. top of S brace post 29.0' NNW
  3. Top of water valve extending 2.5' above grade 69.1' NW
  4. Top of water hydrant 80.0' WNW
  5. In TW of 216th road

#### LEGEND

- W<sub>1</sub> RWD #3  
F<sub>1</sub> Blue Valley  
T<sub>1</sub> AT&T  
E<sub>1</sub> Every

#### SCALE

PLAN: Lat. & Long. 100' 0 100' 200'  
PROFILE: Horiz. same as above  
Vert. 0 10 20

KANSAS DEPARTMENT OF TRANSPORTATION

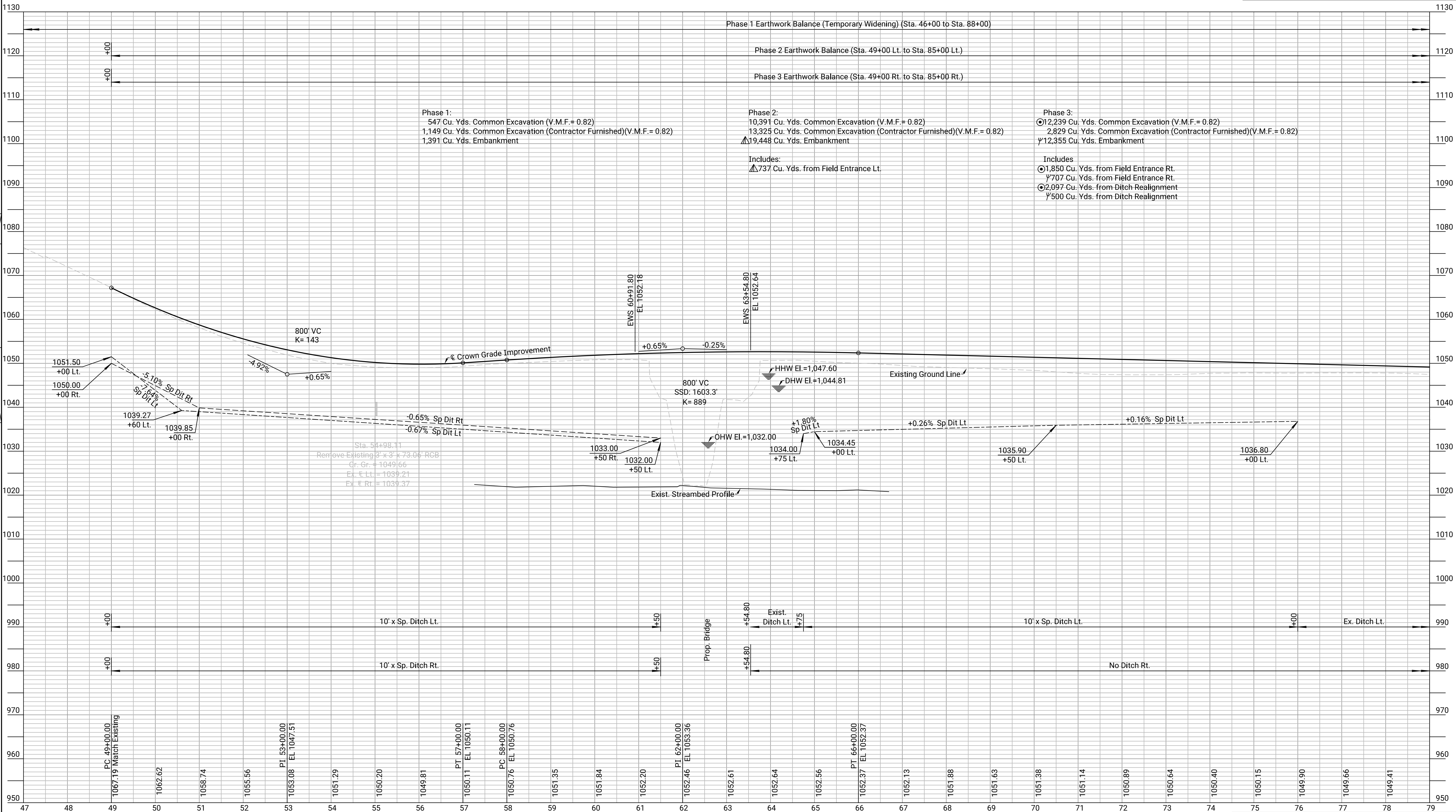
#### PLAN

STA. 44+24.23 TO STA. 79+00

DATE	BY
REFERENCES NOTED	REFERENCES CHECKED

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 19:04  
File : KA572901ref-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	9	141



KANSAS DEPARTMENT OF TRANSPORTATION

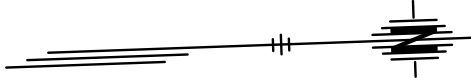
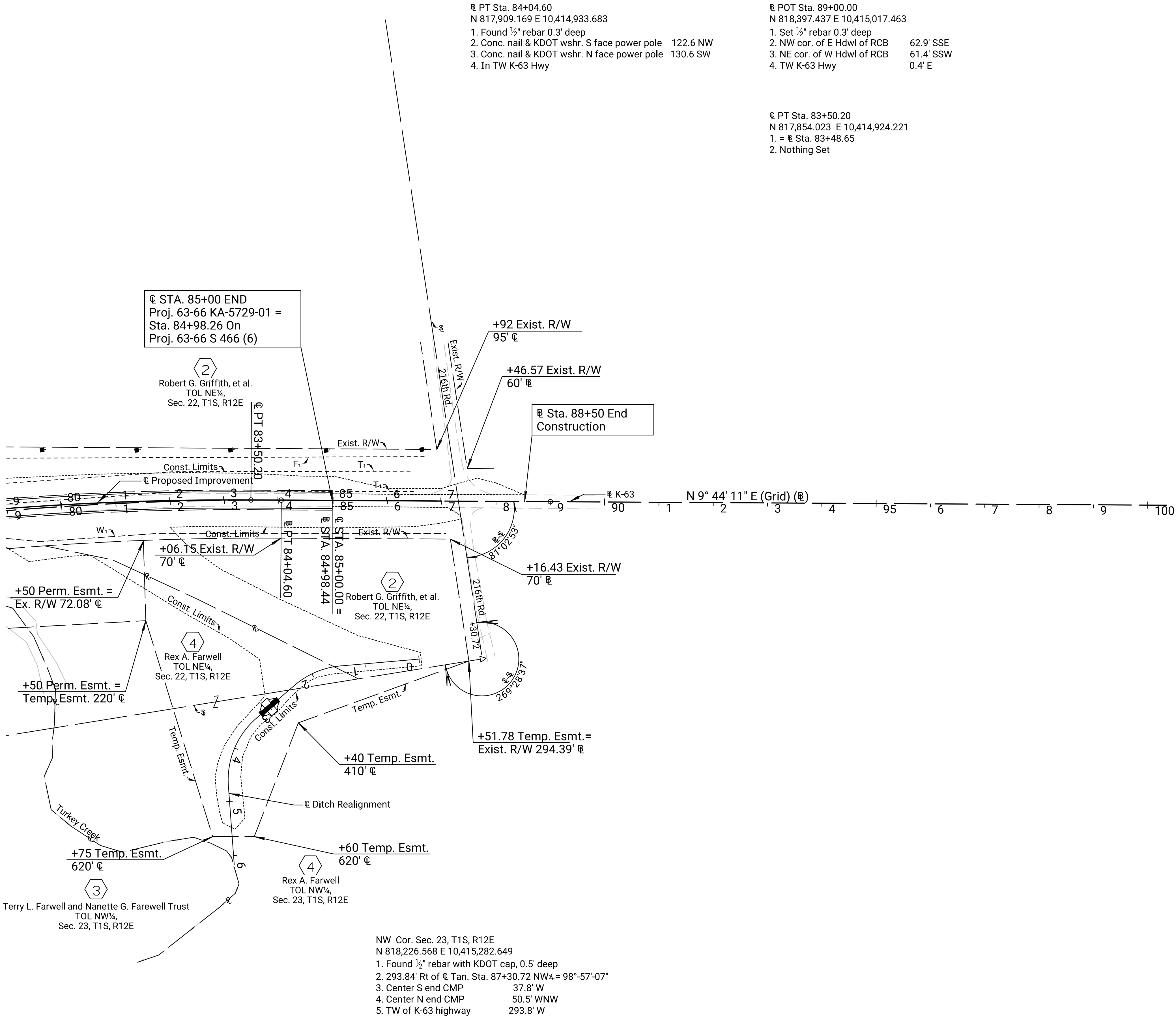
PROFILE

STA. 47+00 TO STA. 79+00

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	10	141

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED
6/2022	B. Andrews		
6/2022	R. Bowman		

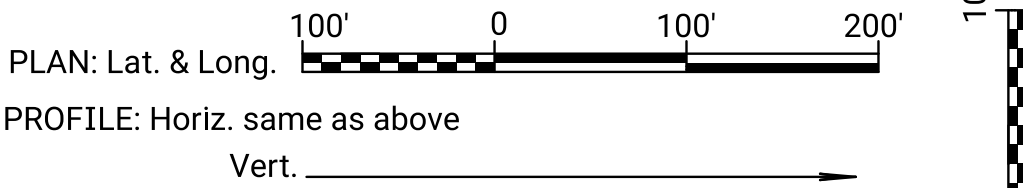
Plotted by : Brogan.Andrews@ks.gov 15-JUL-2024 17:45  
File : KA572901ref-01.dgn



LEGEND

- W<sub>1</sub> RWD #3
- F<sub>1</sub> Blue Valley
- T<sub>1</sub> AT&T
- E<sub>1</sub> Evergy

SCALE



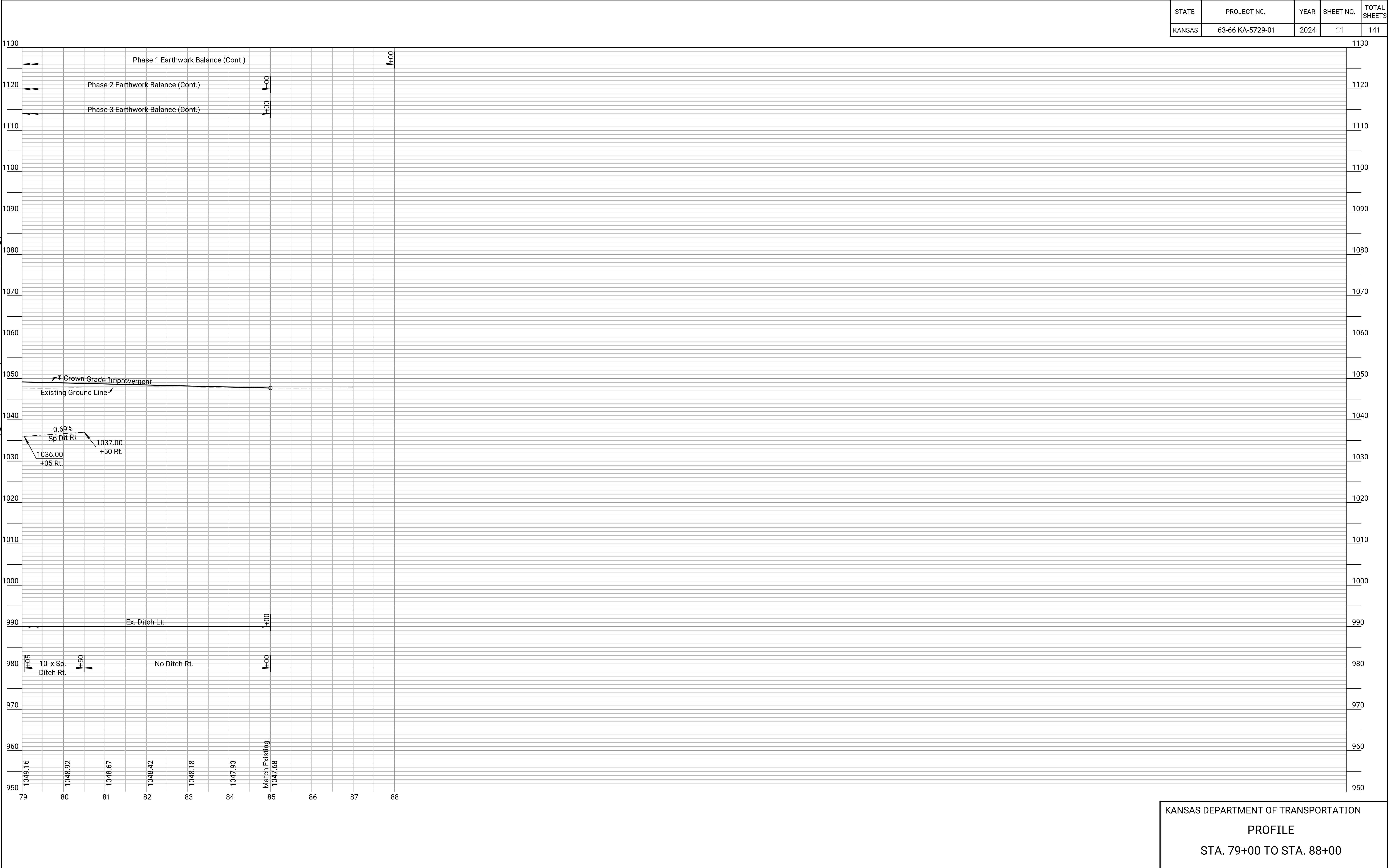
KANSAS DEPARTMENT OF TRANSPORTATION  
PLAN  
STA. 79+00 TO STA. 87+00



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	11	141

BY	DATE
REFERENCES NOTED	REFERENCES CHECKED

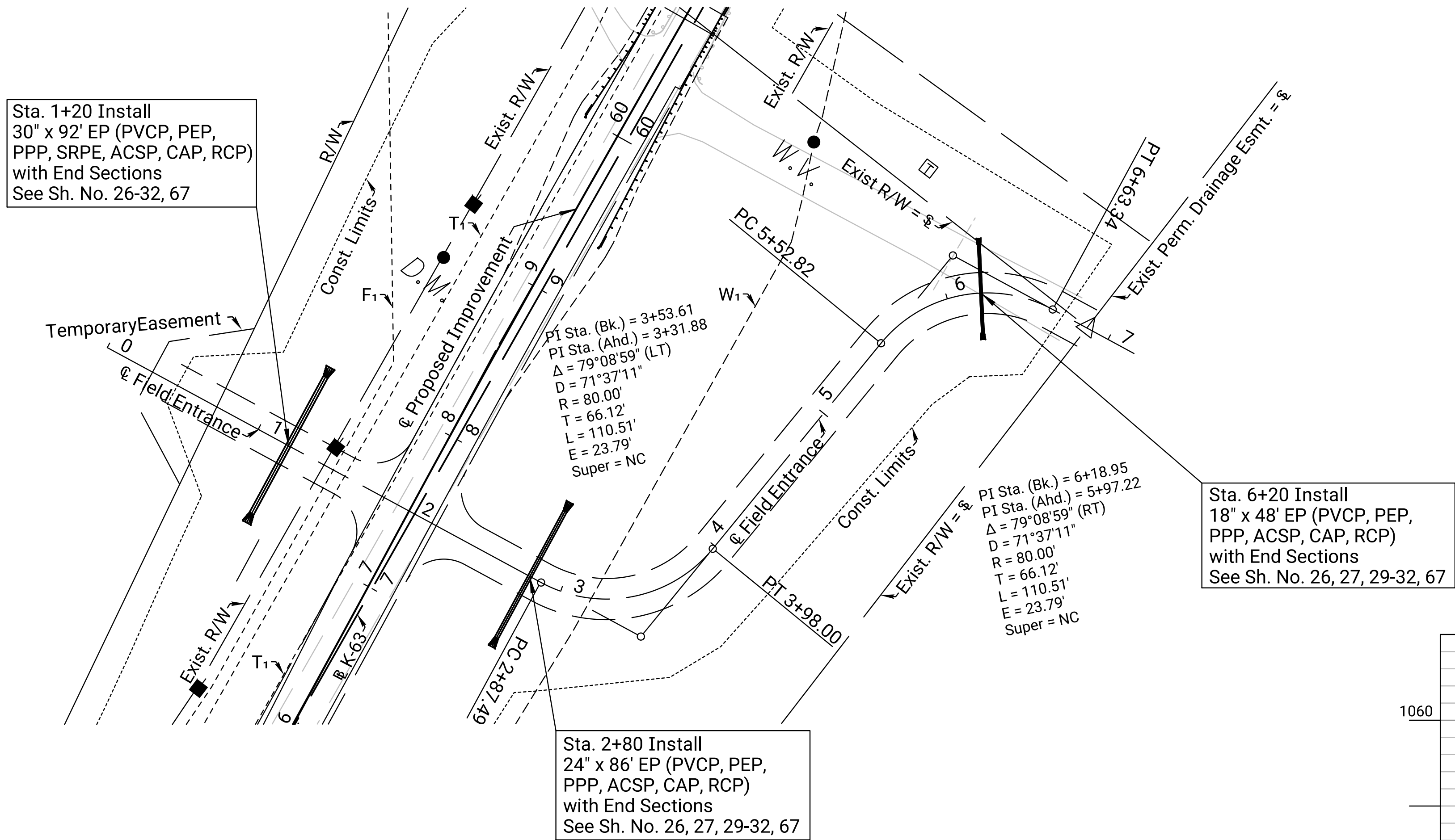
Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:29  
File : KA572901ref-01.dgn



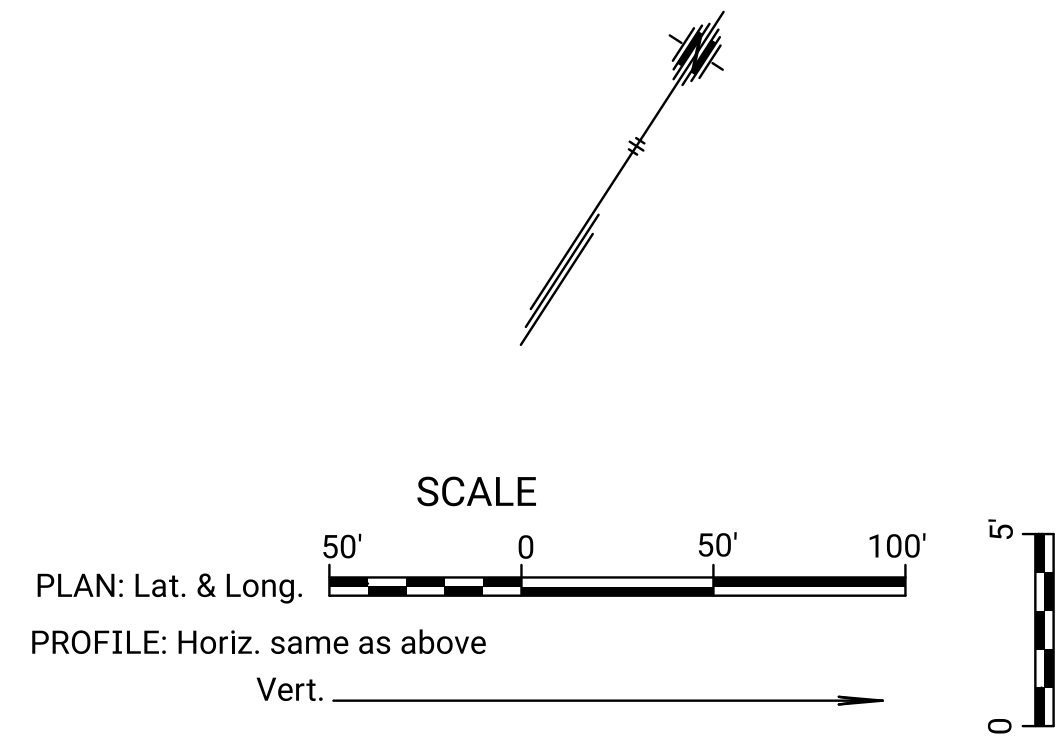
DATE	BY	REFERENCES NOTED
6/2022	B. Andrews	REFERENCES CHECKED
6/2022	J. Austin	

Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:30  
File : KA572901ref-01.dgn

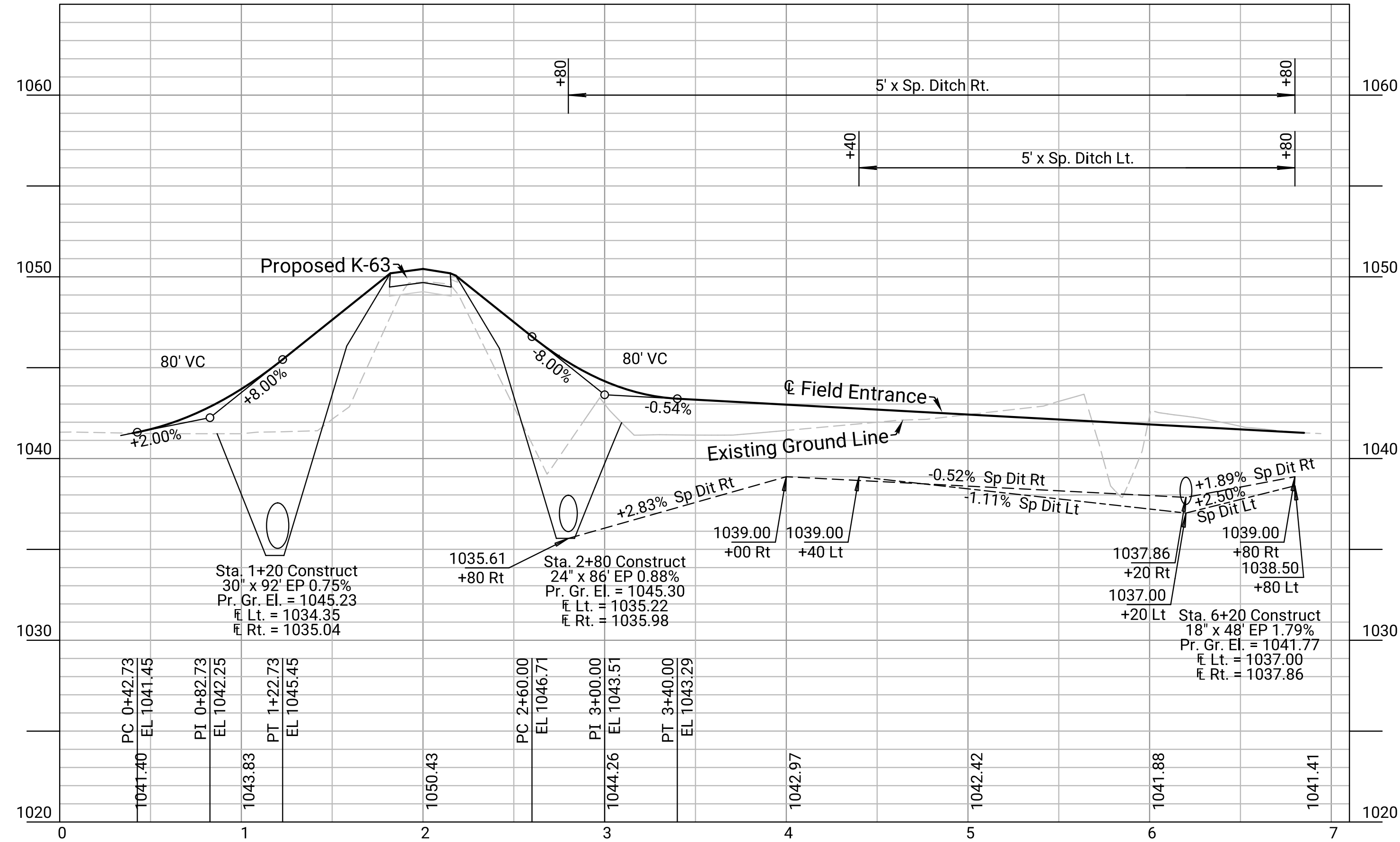
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	12	141



- LEGEND
- W<sub>1</sub> RWD #3
  - F<sub>1</sub> Blue Valley
  - T<sub>1</sub> AT&T
  - E<sub>1</sub> Every



- Field Entrance POT Sta. 0+00.00  
N 815,238.17 E 10,414,825.23  
1. 200' Lt. of K-63 Sta. 57+50  
2. Nothing Set
- Field Entrance PC Sta. 2+87.49  
N 815,283.15 E 10,415,109.18  
1. 87.49' Rt. of K-63 Sta. 57+50  
2. Nothing Set
- Field Entrance PI Sta. (Bk.) = 3+53.61  
N 815,293.49 E 10,415,174.485  
1. 153.60' Rt. of K-63 Sta. 57+50  
2. Nothing Set
- Field Entrance PT Sta. 3+98.00  
N 815,359.58 E 10,415,176.62  
1. 165.87' Rt. of K-63 Sta. 58+15.89  
2. Nothing Set
- Field Entrance PC Sta. 5+52.82  
N 815,514.32 E 10,415,181.62  
1. 193.12' Rt. of K-63 Sta. 59+70.71  
2. Nothing Set
- Field Entrance PI Sta. (Bk.) = 6+18.95  
N 815,580.41 E 10,415,183.75  
1. 204.22' Rt. of K-63 Sta. 60+36.25  
2. Nothing Set
- Field Entrance PC Sta. 6+63.34  
N 815,590.75 E 10,415,249.06  
1. 270.33' Rt. of K-63 Sta. 60+37.65  
2. Nothing Set
- Field Entrance POT Sta. 7+17.22  
N 815,599.18 E 10,415,302.27  
1. 324.19' Rt. of K-63 Sta. 60+38.78  
2. Nothing Set



KANSAS DEPARTMENT OF TRANSPORTATION

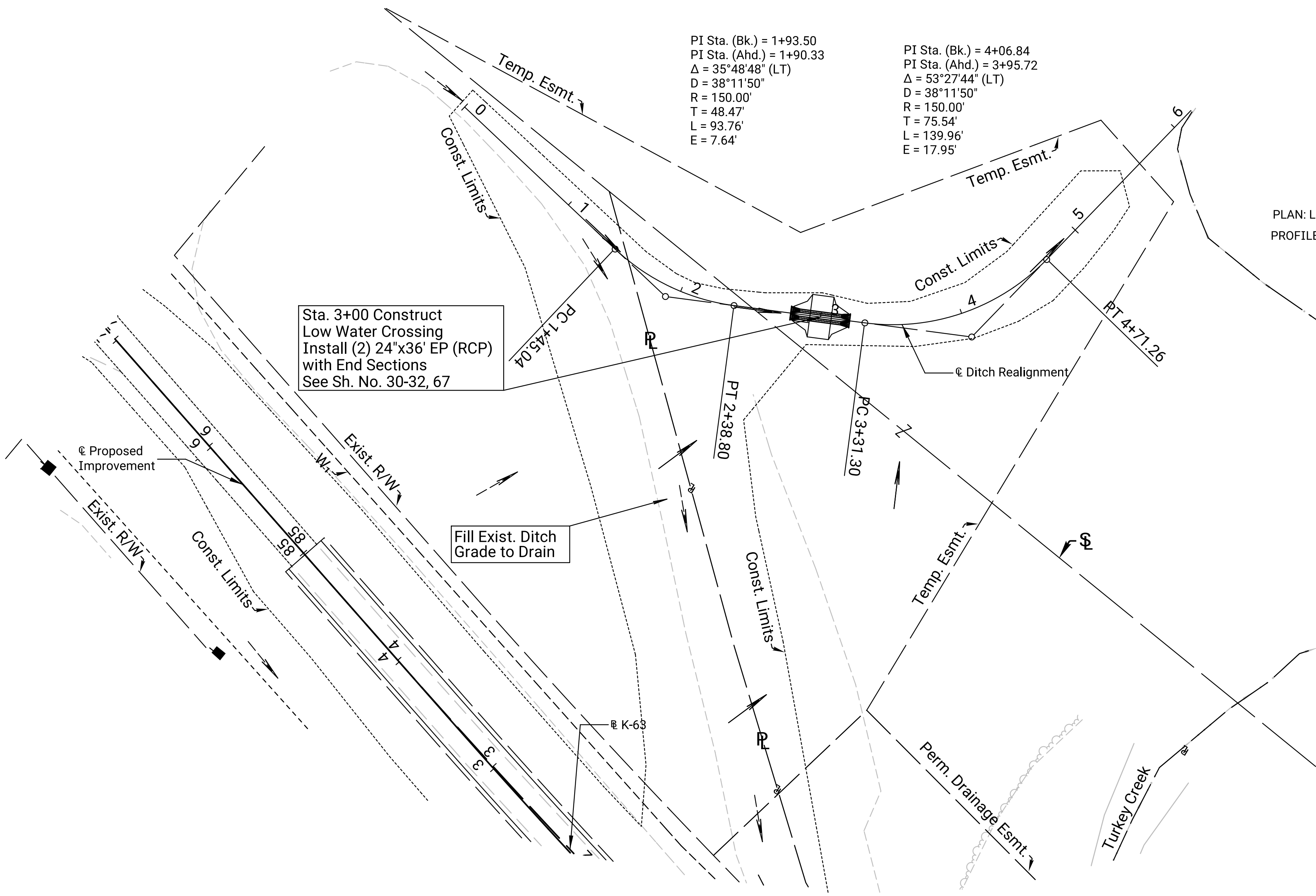
FIELD ENTRANCE

PLAN AND PROFILE

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	13	141

DATE	4/2023
BY	B. Andrews R. Bowman
REFERENCES NOTED	REFERENCES CHECKED

Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:28  
File : KA572901rco-05.dgn



Ditch Realignment POT Sta. 0+00.00  
N 818,160.916 E 10,414,976.879  
1. 291.36' Rt. of  $\odot$  K-63 Sta. 86+61.58  
2. Nothing Set

Ditch Realignment PI Sta. (Bk.) = 1+93.50  
Ditch Realignment PI Sta. (Ahd.) = 1+90.33  
N 817,971.015 E 10,414,944.295  
1. 309.24' Rt. of  $\odot$  K-63 Sta. 84+68.90  
2. Nothing Set

Ditch Realignment PC Sta. 3+31.30  
N 817,866.336 E 10,414,926.333  
1. 401.94' Rt. of  $\odot$  K-63 Sta. 83+62.69  
2. Nothing Set

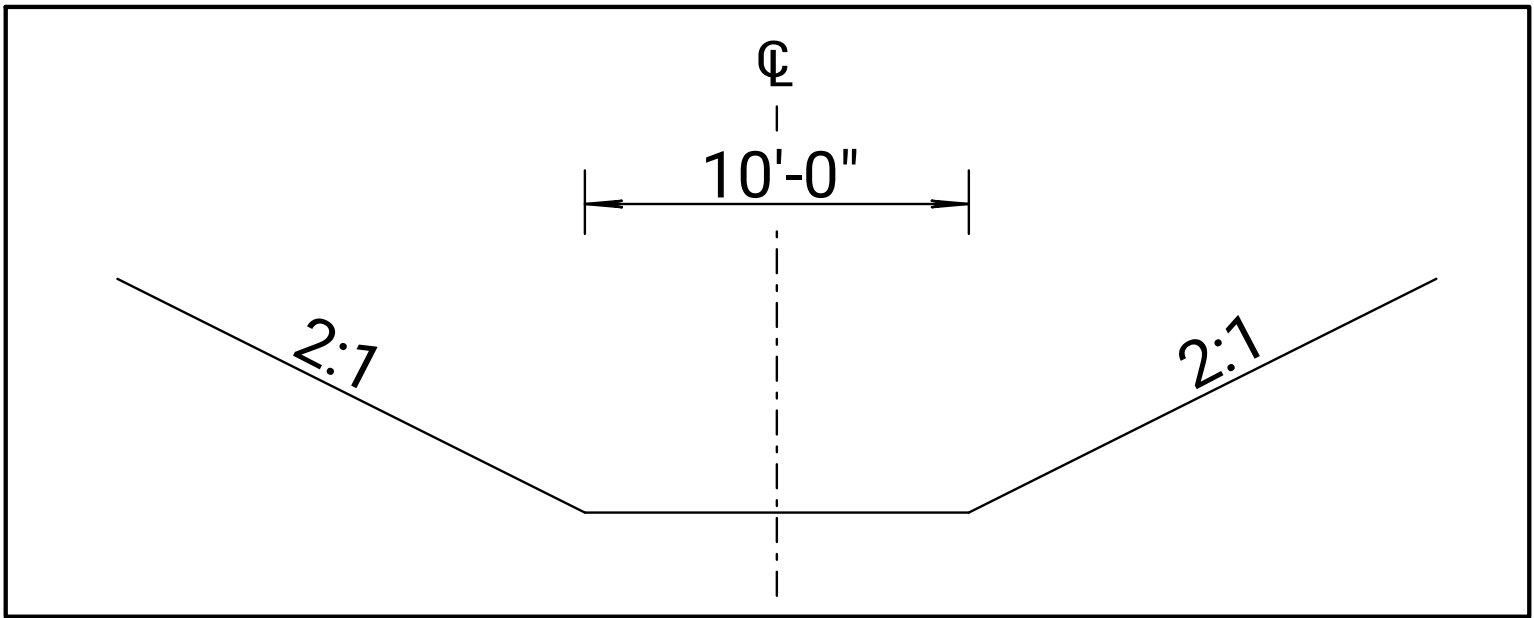
Ditch Realignment PT Sta. 4+71.26  
N 817,812.146 E 10,414,917.201  
1. 526.77' Rt. of  $\odot$  K-63 Sta. 83+07.74  
2. Nothing Set

Ditch Realignment PC Sta. 1+45.04  
N 818,018.580 E 10,414,952.456  
1. 304.76' Rt. of  $\odot$  K-63 Sta. 85+17.16  
2. Nothing Set

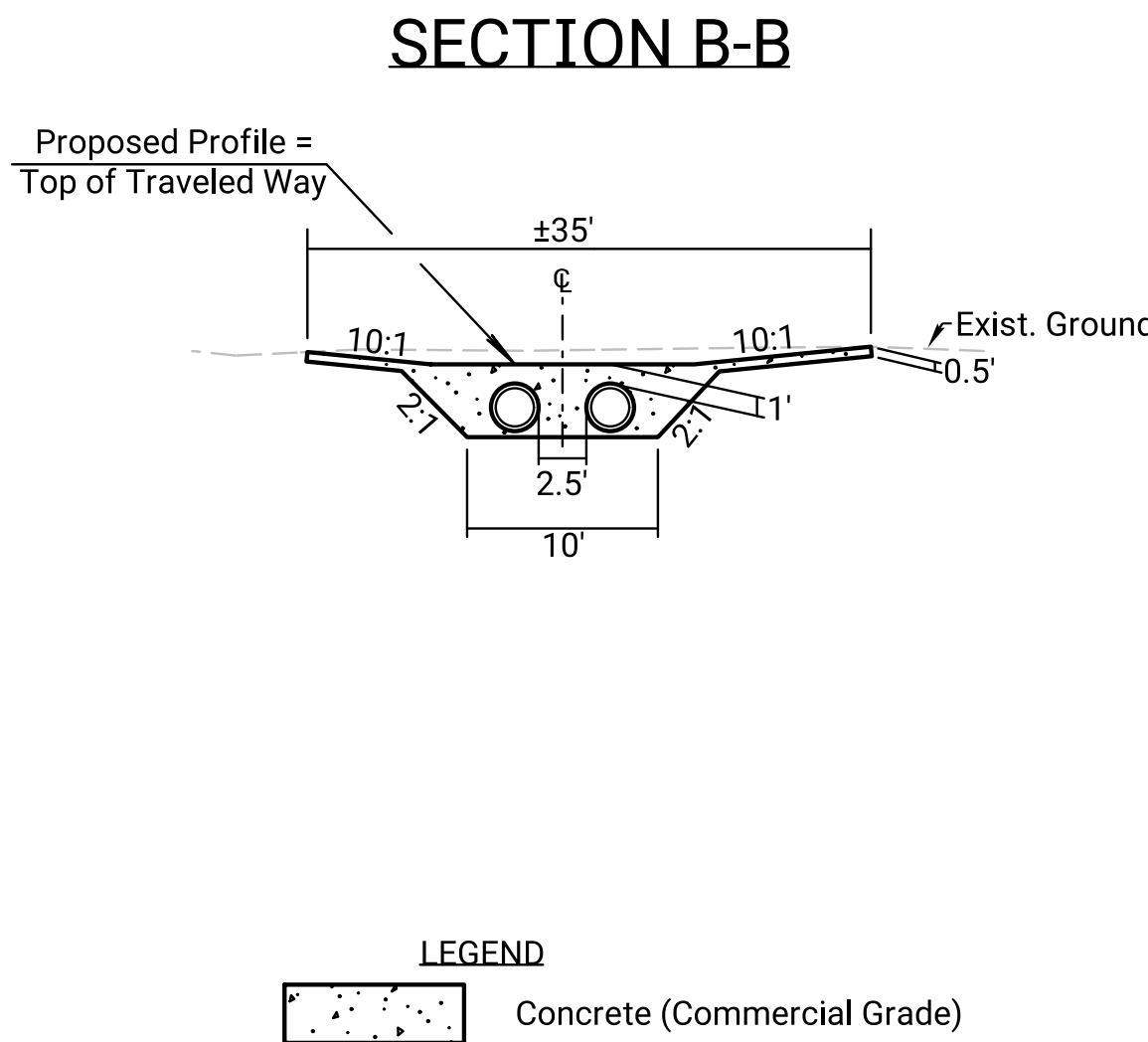
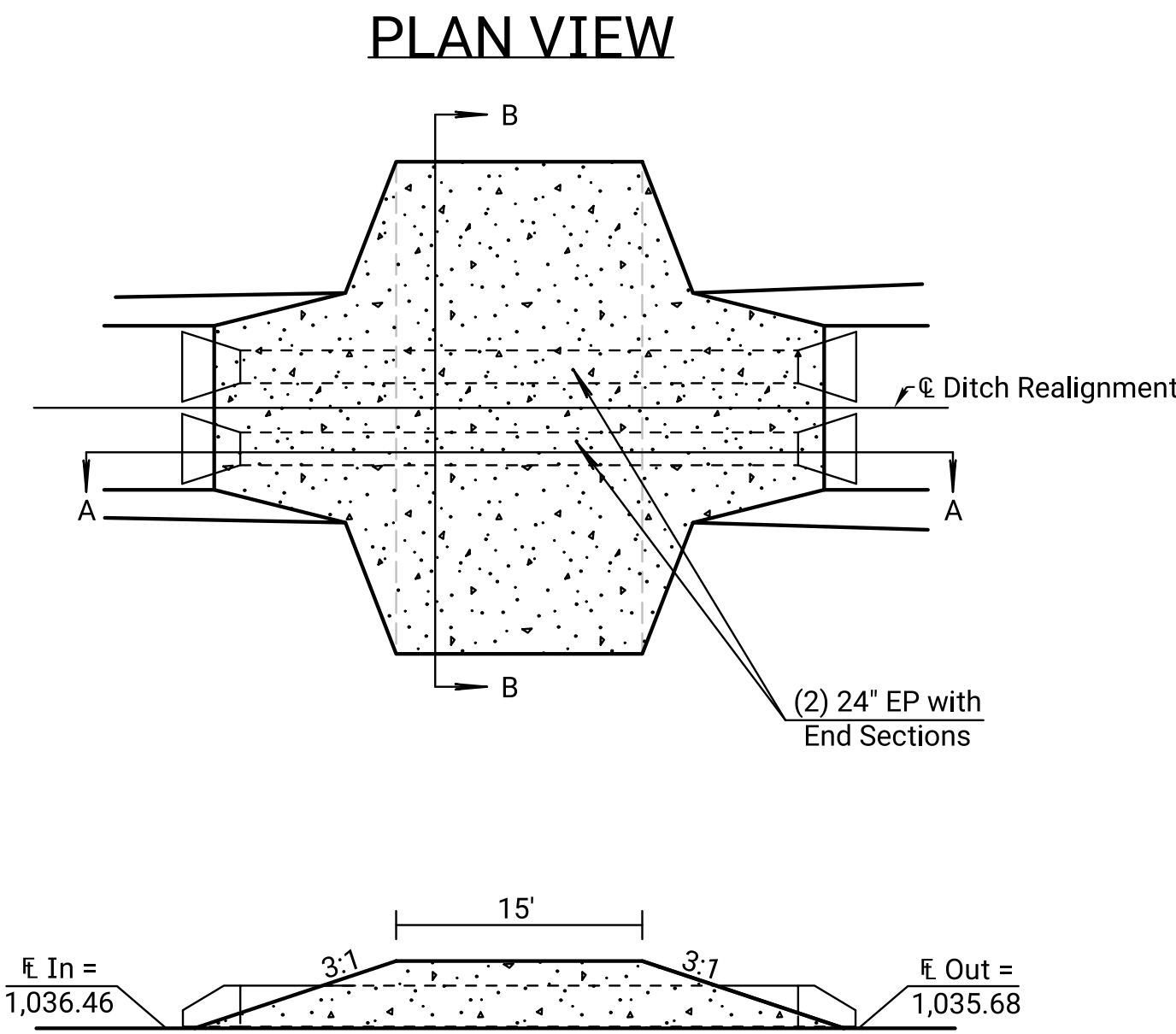
Ditch Realignment PT Sta. 2+38.80  
N 817,935.025 E 10,414,938.119  
1. 341.11' Rt. of  $\odot$  K-63 Sta. 84+32.39  
2. Nothing Set

Ditch Realignment PI Sta. (Bk.) = 4+06.84  
Ditch Realignment PI Sta. (Ahd.) = 3+95.72  
N 817,806.290 E 10,414,916.246  
1. 451.42' Rt. of  $\odot$  K-63 Sta. 83+01.81  
2. Nothing Set

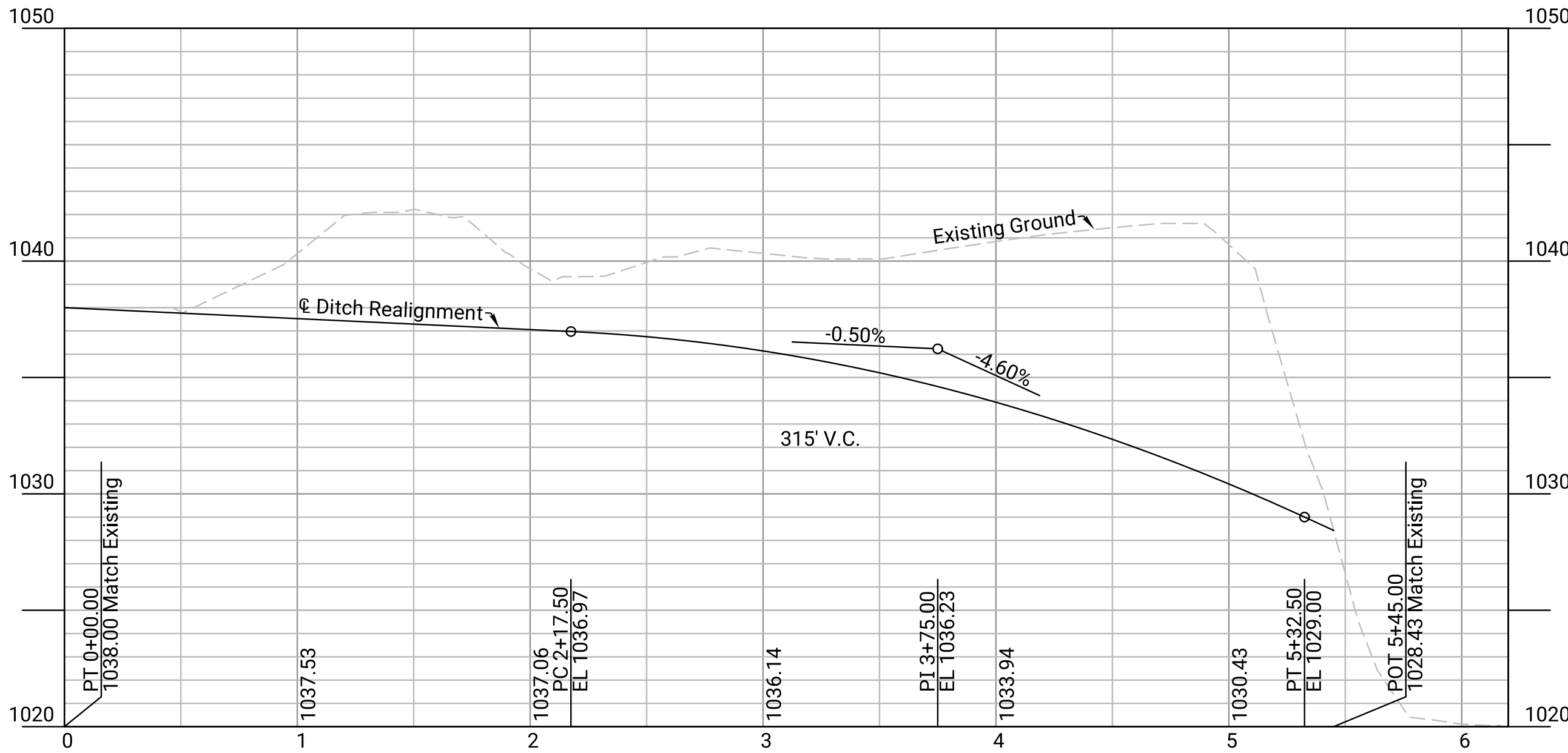
Ditch Realignment POT Sta. 6+16.60  
N 817,823.923 E 10,414,919.142  
1. 671.72' Rt. of  $\odot$  K-63 Sta. 83+19.67  
2. =  $\odot$  Turkey Creek  
3. Nothing Set



Ditch Realignment Typical Section  
Not to Scale



CONCRETE (COMMERCIAL GRADE)			
LOCATION	STA. TO STA.	CU. YD.	REMARKS
$\odot$ Ditch Realignment	2+82 to 3+18	47.4	Low Water Crossing



KANSAS DEPARTMENT OF TRANSPORTATION

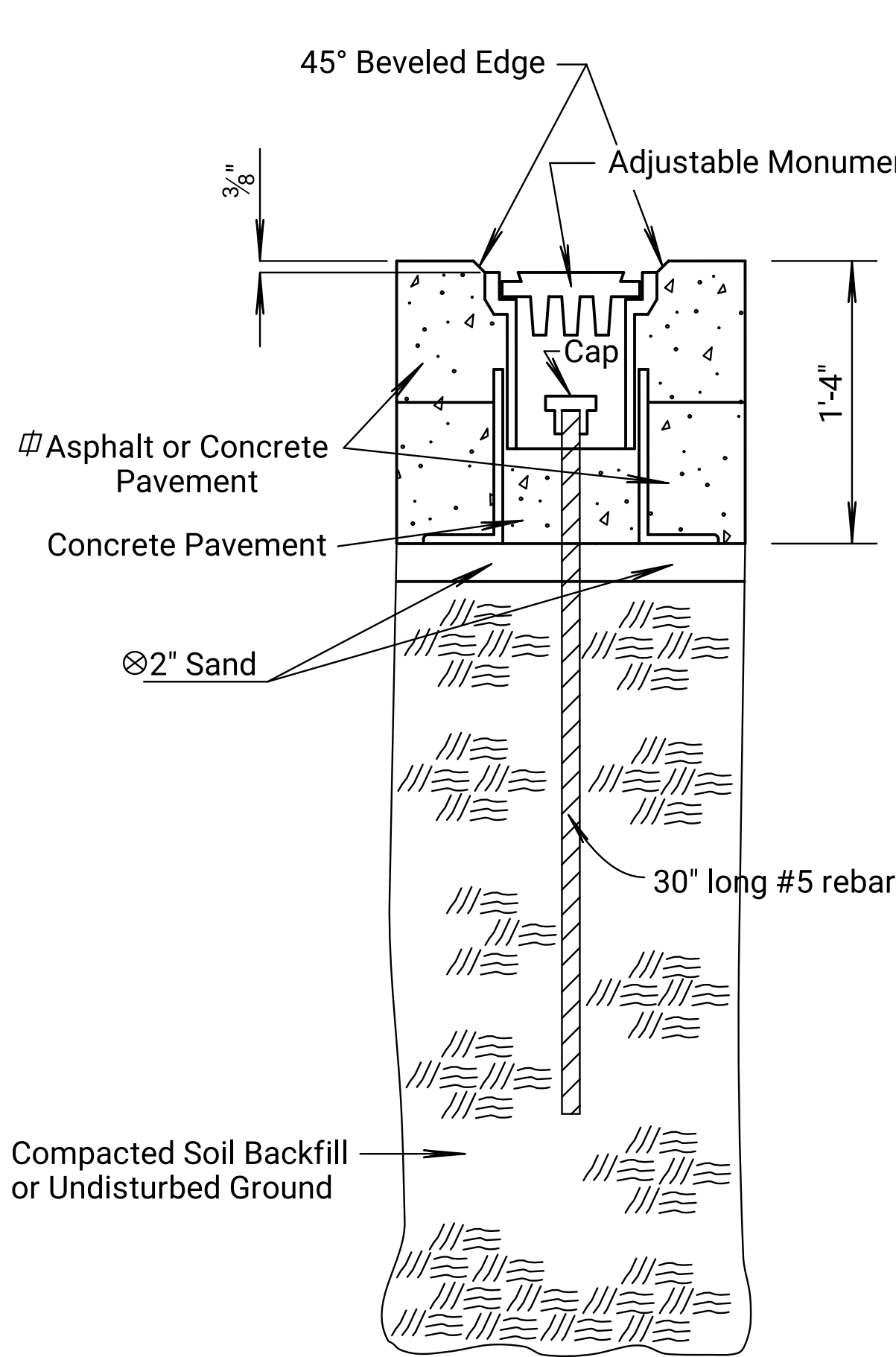
DITCH REALIGNMENT

PLAN AND PROFILE

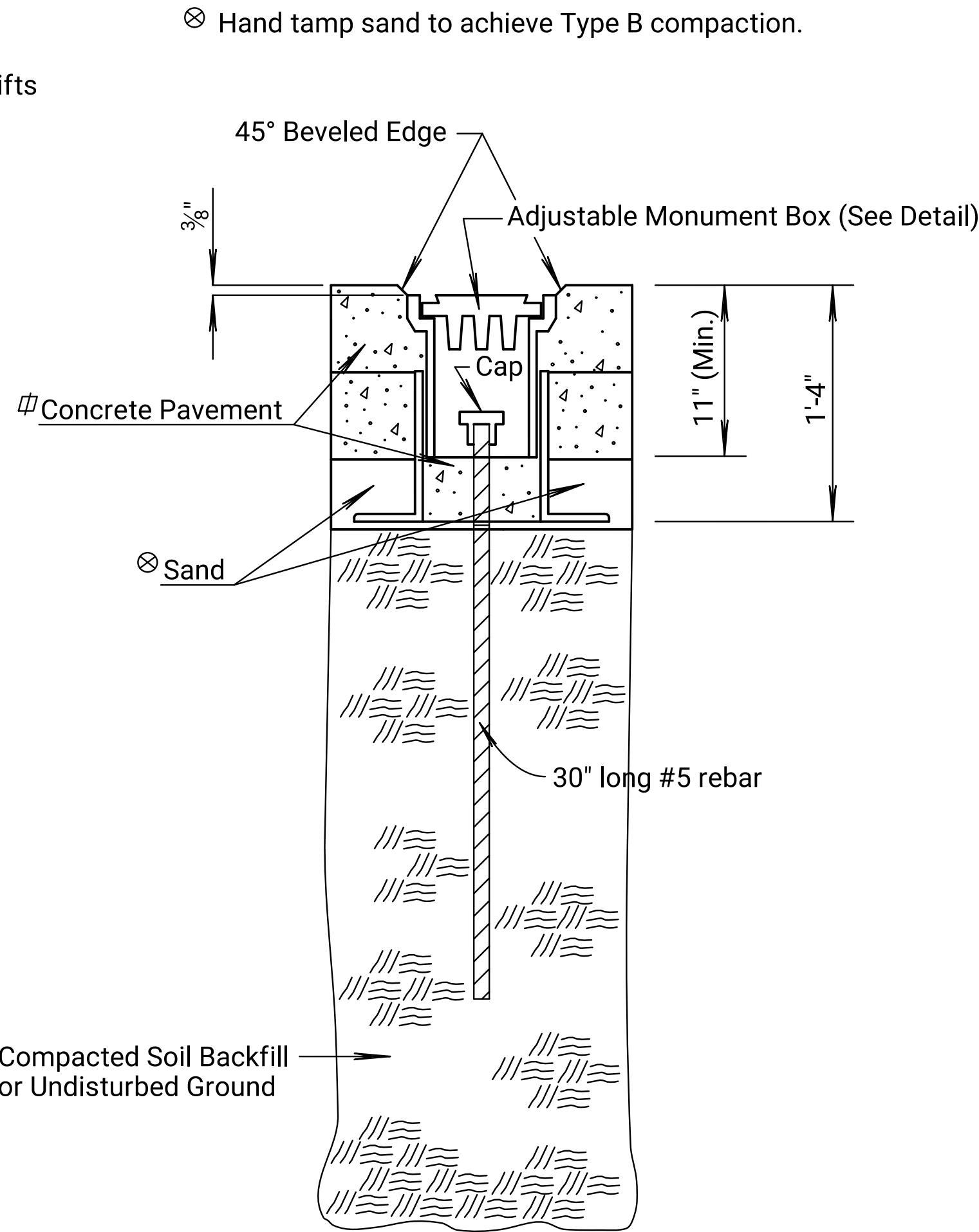


Plotted by : Brogan Andrews@ks.gov  
File : KA572901rss990-01.dgn  
16-APR-2024 17:27

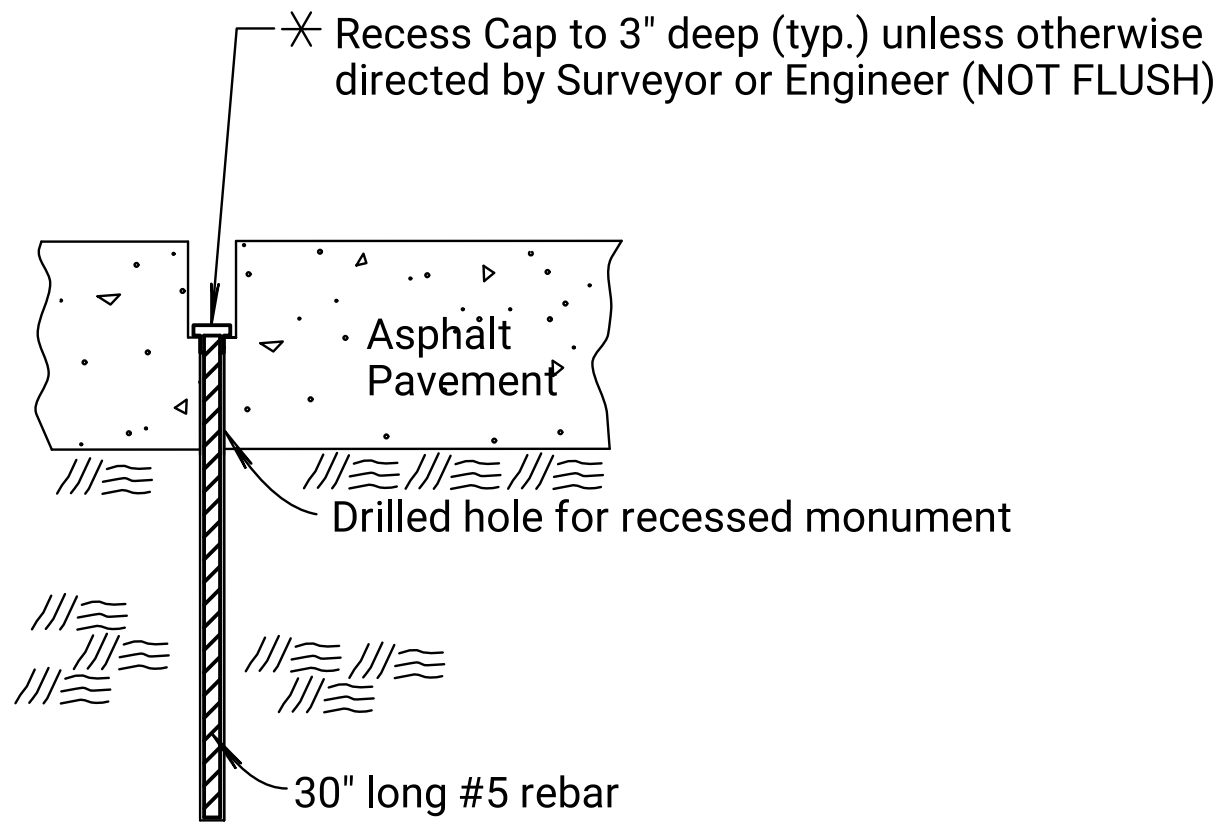
EXHIBIT		
2" DIA. PRE-STAMPED ALUMINUM CAPS		
KDOT - DISTRICT 1	KDOT - DISTRICT 2	KDOT - DISTRICT 3
KDOT - DISTRICT 4	KDOT - DISTRICT 5	KDOT - DISTRICT 6
KDOT STATE SURVEY CREW BUREAU OF RIGHT OF WAY	SURVEY COMPANY WITH A VALID CORPORATION LICENSE	INDEPENDENT SURVEYOR OR SOLE PROPRIETOR COMPANY



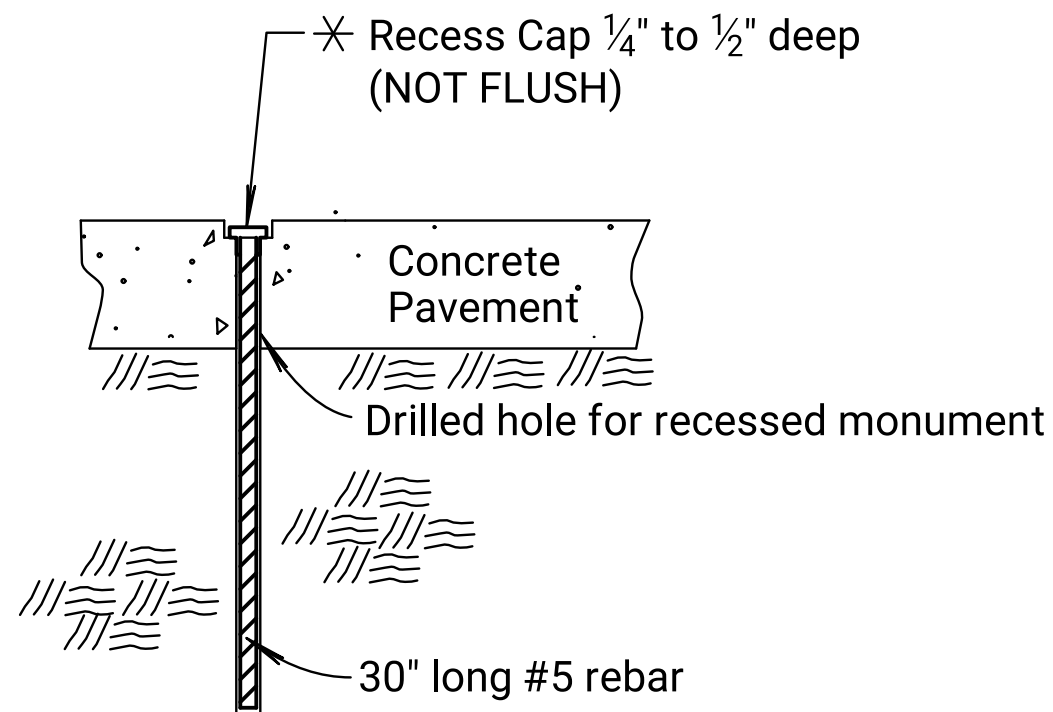
TYPE A-1  
(Standard Land Corner  
Monument Box)



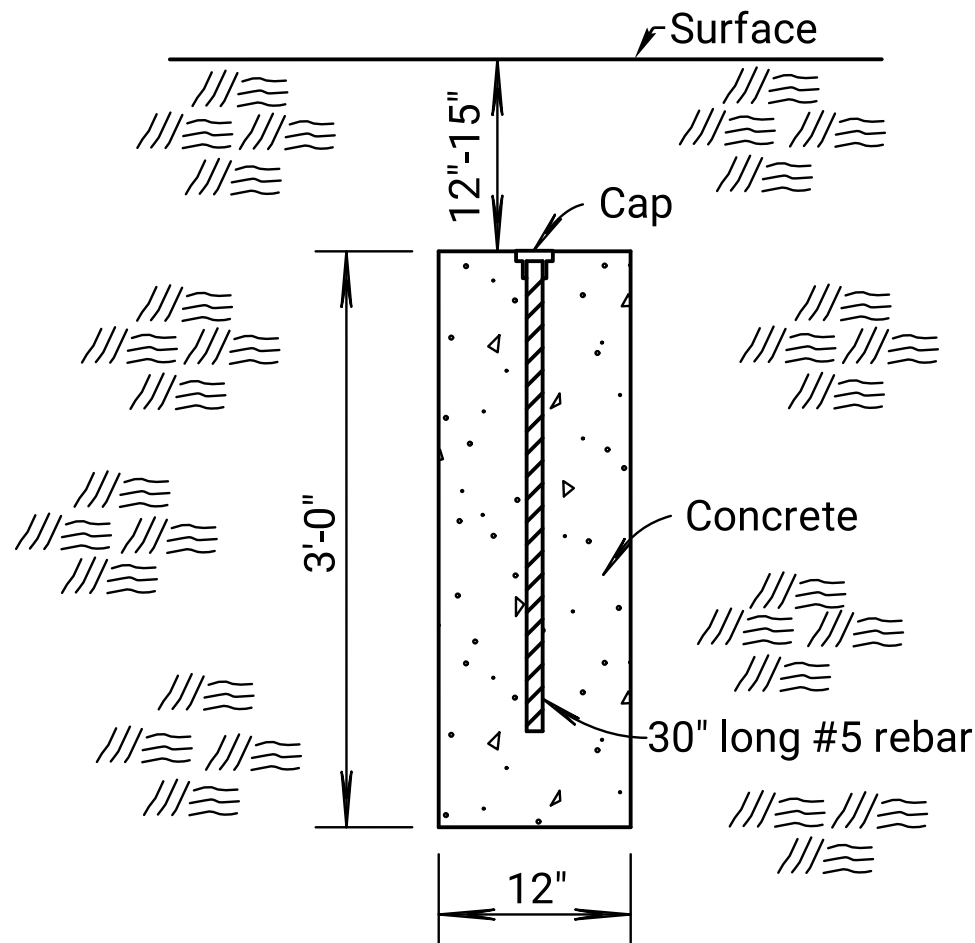
TYPE A-2  
(Standard Land Corner  
Monument Box)



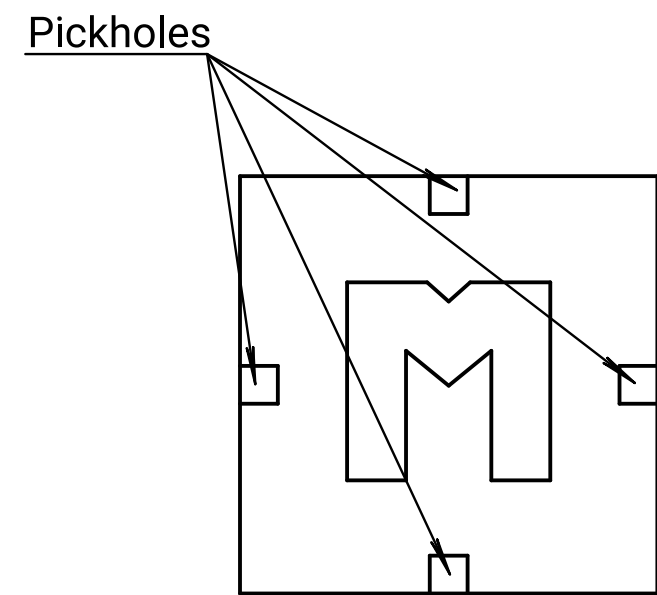
Type B  
(Recessed Bar)



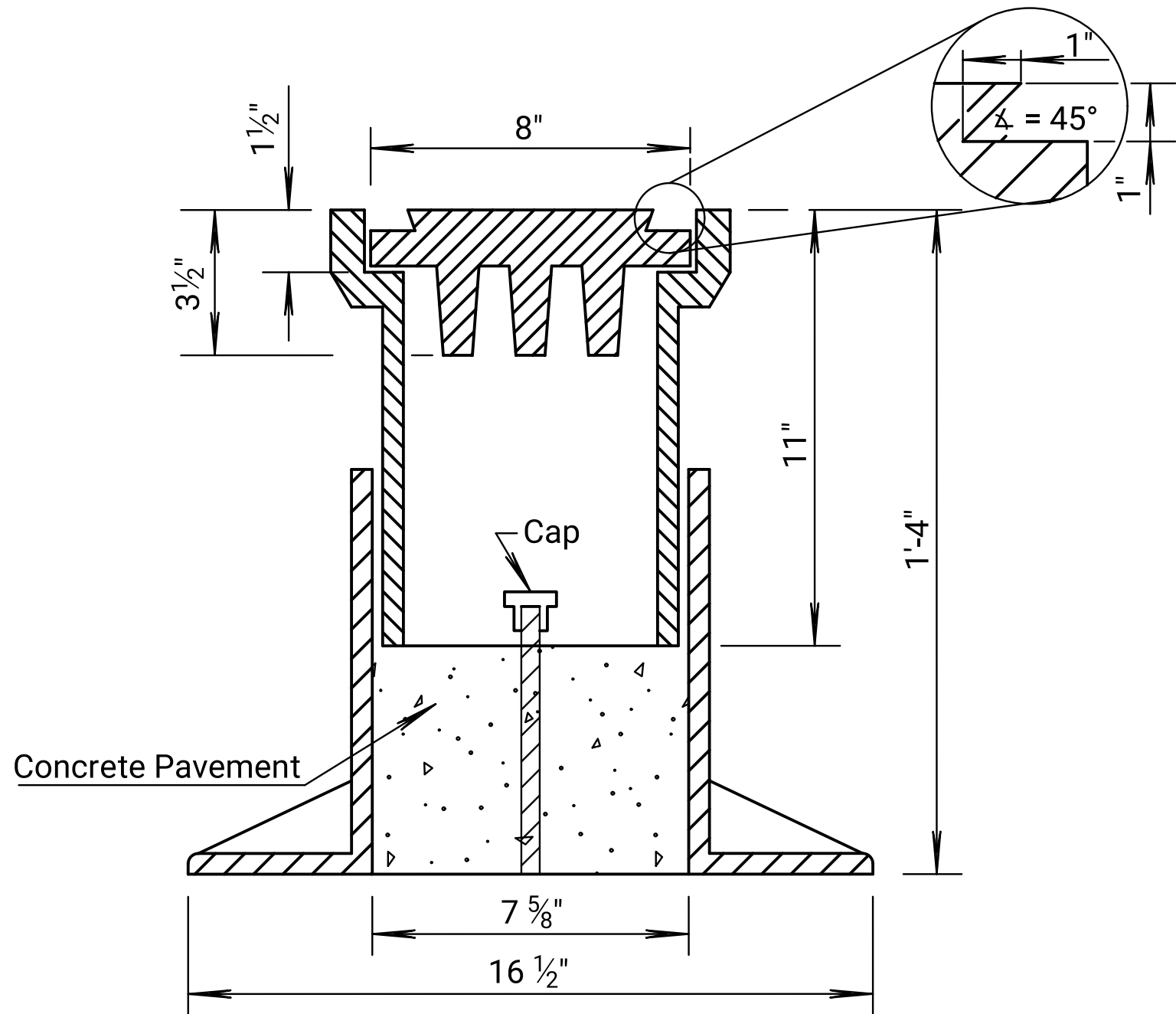
Type C  
(Drilled Hole)



Type D  
(Shoulder/Ditch Subsurface  
Monument)



MONUMENT BOX LID (PLAN VIEW)  
Not to Scale



ADJUSTABLE MONUMENT BOX DETAIL  
(Neenah R-1968 Type 36-B  
or approved equivalent)

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	14	141

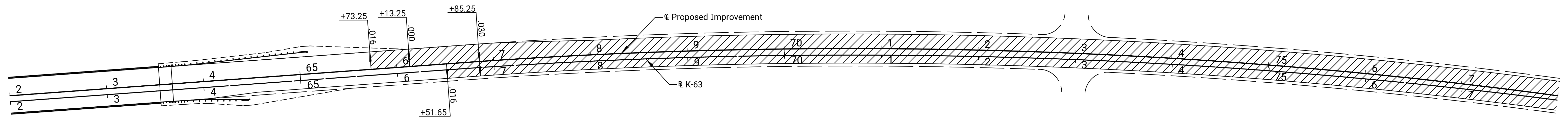
#### GENERAL NOTES

Use the proper identification cap for the party installing the monument, as shown on the exhibit.  
Make all stampings, forgings, and impressions legible. The stampings, forgings, and impressions will properly identify the location of the monument within the Public Land Survey System (PLSS).  
A "System of Marking" is available in the current "Manual of SURVEYING INSTRUCTION", which is published by the United States Department of the Interior, Bureau of Land Management.  
Reset all PLSS corners in accordance with KDOT's Standard Specifications.  
In addition to monumentation of the PLSS corner, the Engineer may direct or select specific locations for offset monumentation, as shown.  
Use Type A-1 or Type A-2 monuments as directed by the Surveyor or the Engineer.  
Type A monumentation may be used on a project as specified in the plans or as directed by the Engineer. Typically, Type A monuments are used on high traffic volume roadways, in urban areas, or as required by local governmental codes. Otherwise, use Types B-D monumentation. Avoid installing monument boxes in vehicle wheelpaths where practicable.  
All work and materials required to install the Type A-1 and Type A-2 monument boxes will be paid under the bid item "Monument Box (Each)" and will be included in the plan quantities. All work and materials required to install Types B-D monumentation will be subsidiary to the bid item "Contractor Construction Staking (Lump Sum)". See KDOT's Standard Specifications for details.

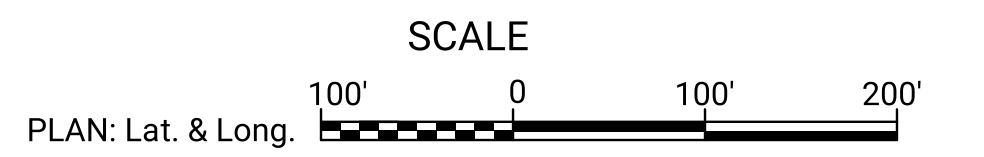
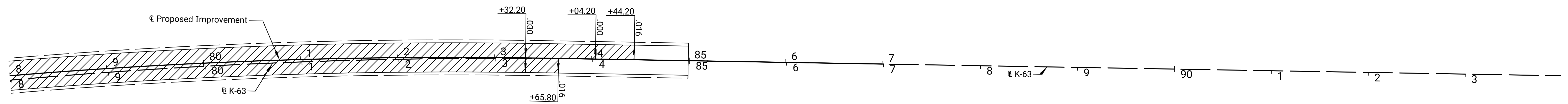
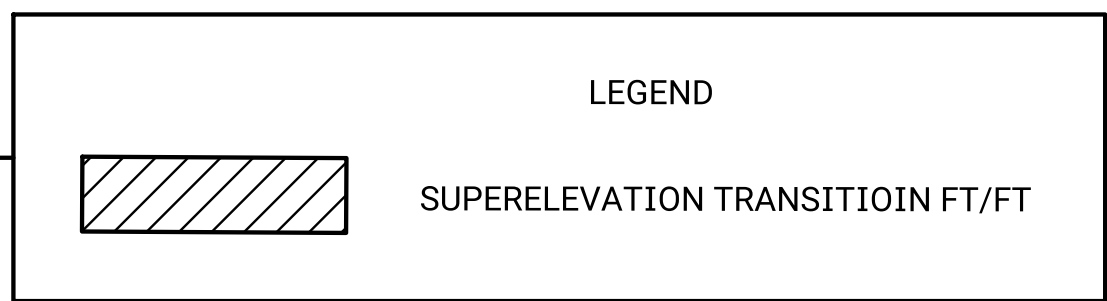
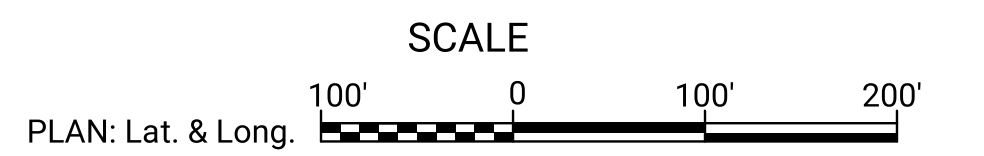
NO.	DATE	REVISIONS	BY	APPD
04	08-24-18	Rev. Det. Mon. Box AND Rev. Gen. Note	A.L.R.	T.T.R.
03	01-15-16	Add. Det. Retrofit & Rev. Gen. Note	T.T.R.	S.W.K.
02	10-17-13	Revised Detail, Recessed Bar	S.W.K.	J.O.B.
KANSAS DEPARTMENT OF TRANSPORTATION				
SECTION CORNER MONUMENTATION DETAIL SHEET				
RD990				
FHWA APPROVAL		08-07-19		APPD.
DESIGNED	DETAILED	QUANTITIES	TRACED	Scott W. King
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	16	141



	BY	DATE
REFERENCES NOTED		
REFERENCES CHECKED		



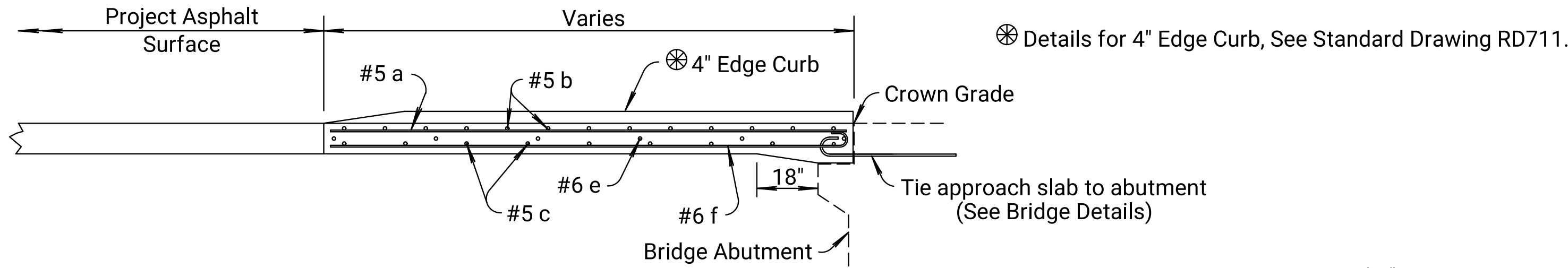
# KANSAS DEPARTMENT OF TRANSPORTATION SUPERELEVATION DETAIL



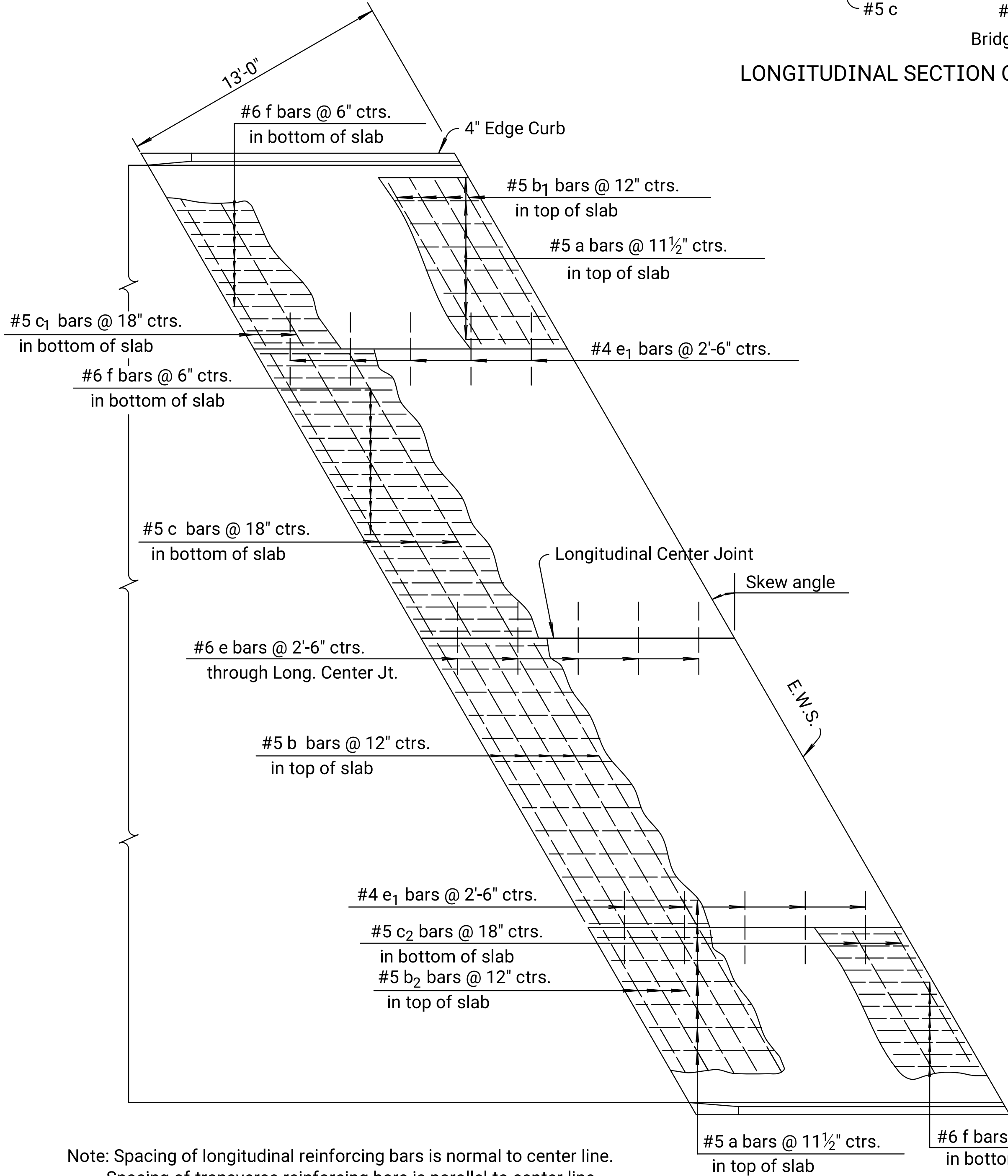
Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:32  
File : KA572901rss715-01.dgn

Note to Designer: The designer shall be responsible for designating pavement thickness and computing reinforcing steel and concrete quantities and dimensions necessary to complete this sheet.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	17	141

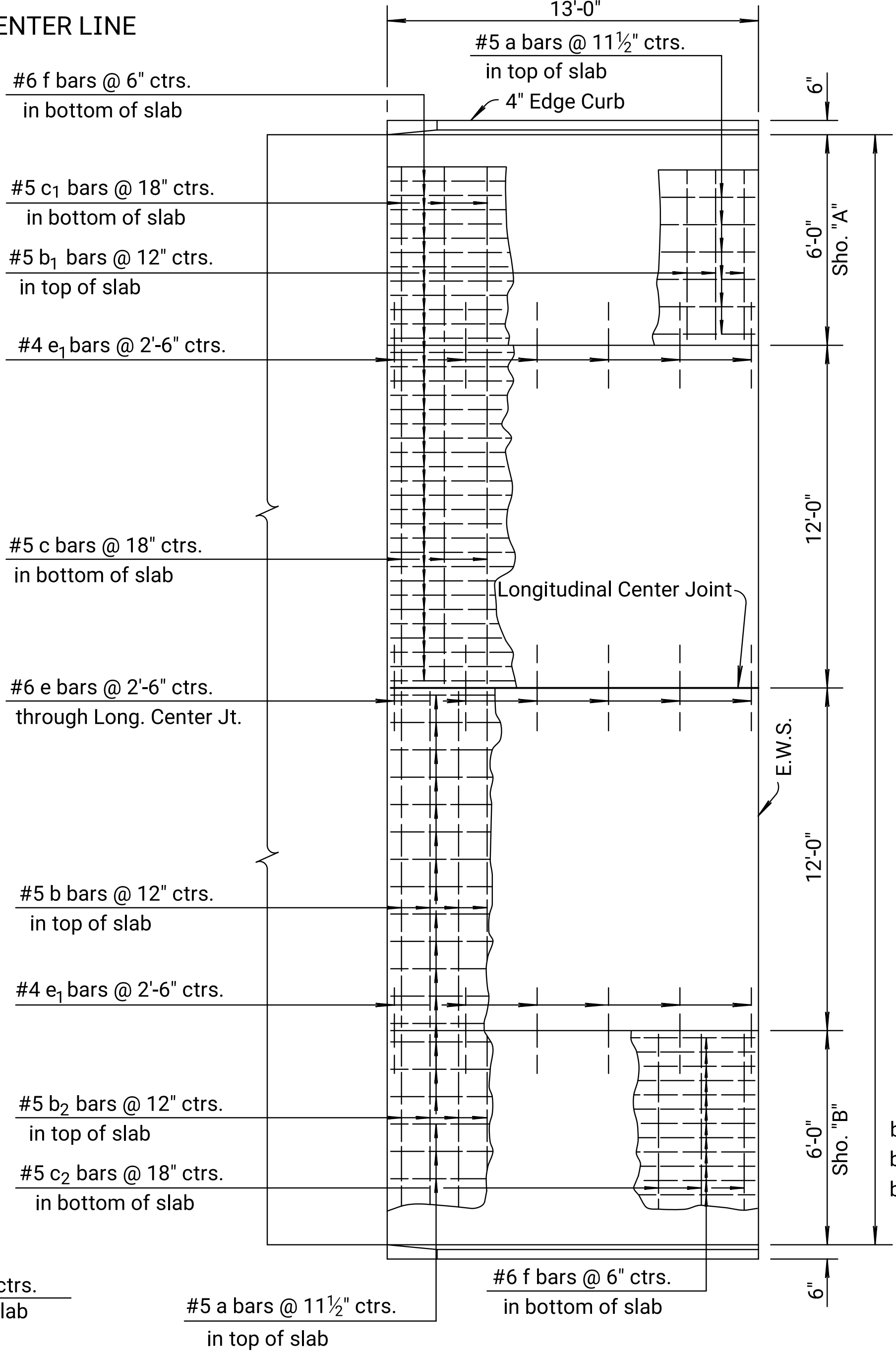


LONGITUDINAL SECTION ON CENTER LINE



Note: Spacing of longitudinal reinforcing bars is normal to center line.  
Spacing of transverse reinforcing bars is parallel to center line.

PLAN FOR SKEWED APPROACH (SKEW ≥ 5 °)  
(No Scale)



PLAN FOR NORMAL APPROACH  
(No Scale)

**GENERAL NOTE**

Special Concrete Bridge Approach shall be paid for as Sq. Yds. of Concrete Pavement (10" Unif.)(AE)(Br. App.) and includes all work and materials required to construct the approach slab as shown on this sheet.

All work and materials required for installation of joint material shall be subsidiary to this bid item.

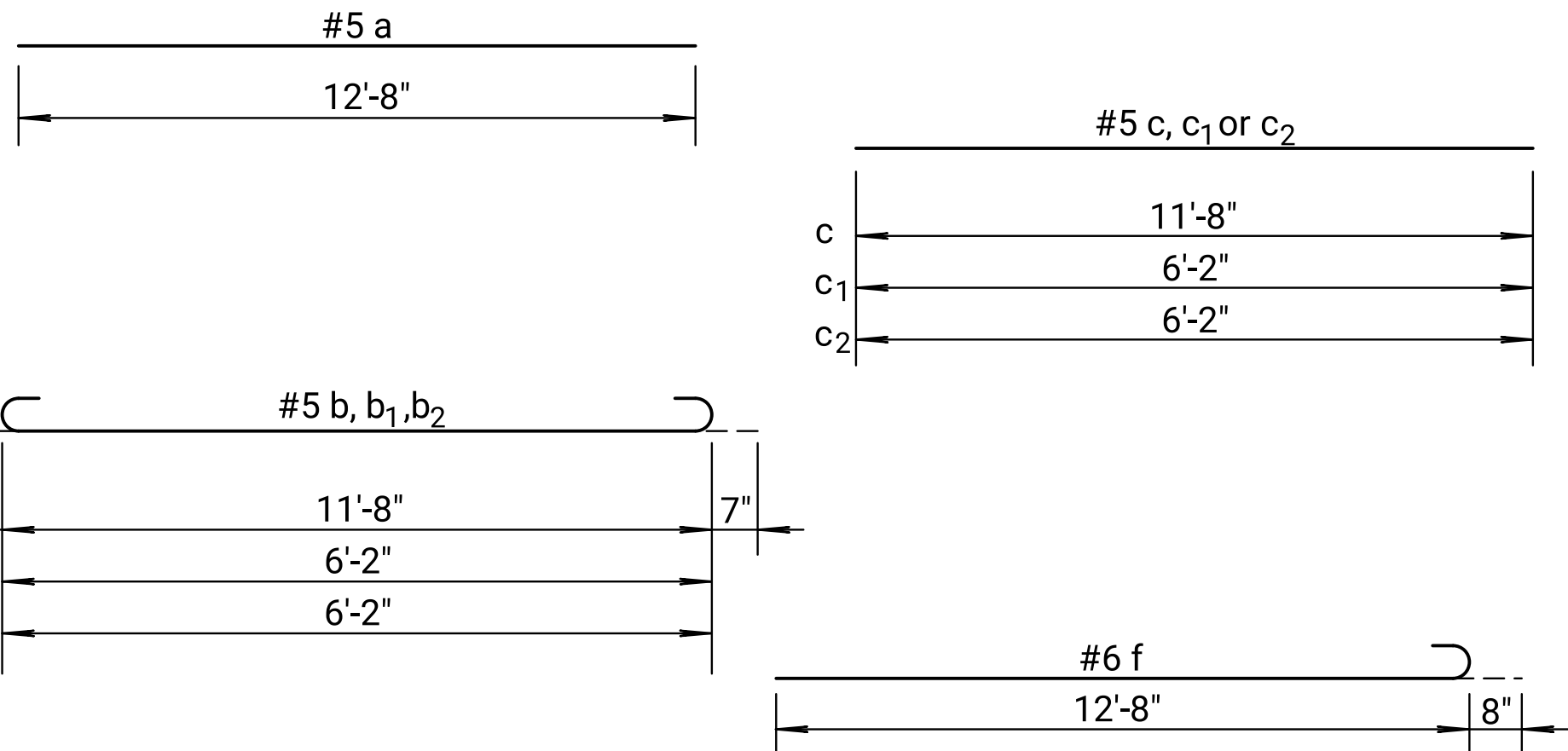
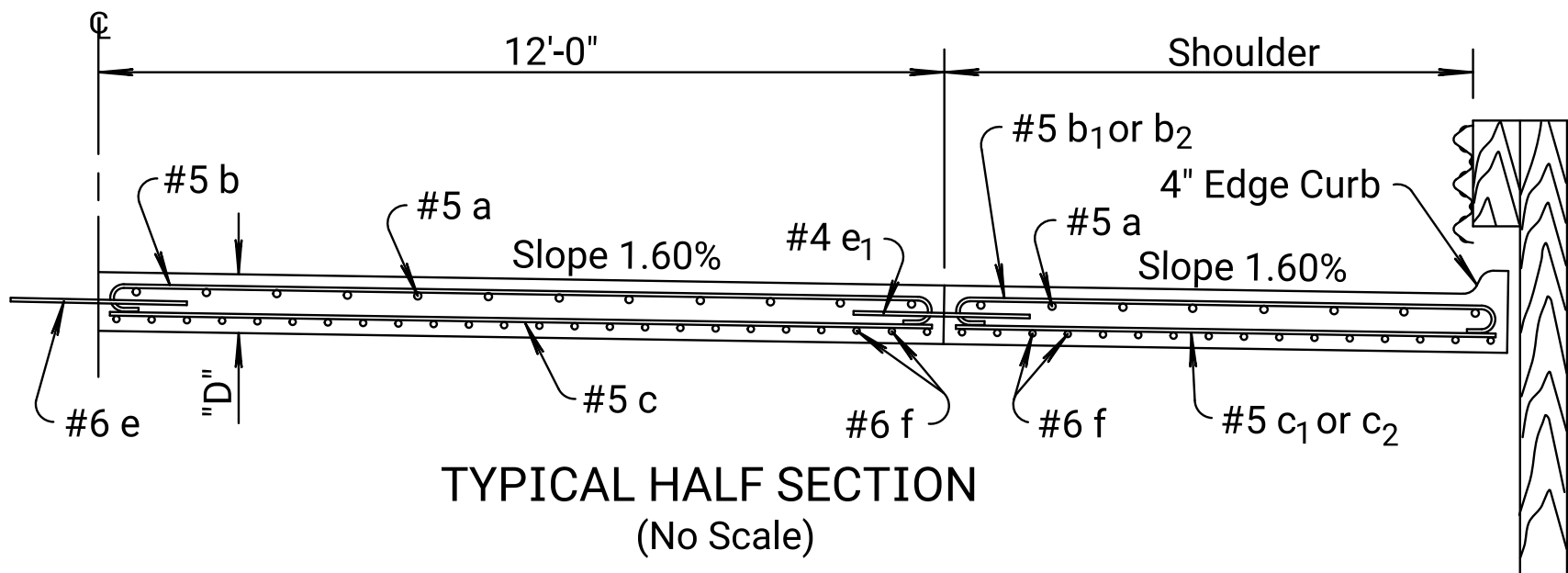
At the Contractor's option #4x3'-0" tie bars @ 15" centers may be substituted for the #6 e bars at 2'-6" centers.

All reinforcing steel shall be epoxy coated.

See Standard Drawing RD711 for details of joints and edge curb.

Clearance from the face of concrete for all reinforcing steel shall be 2 inches.

Standard reinforcing bar hooks in accordance with the latest ACI specifications shall be used throughout.



Note: All dimensions are out to out on bars unless noted otherwise.

BENDING DIAGRAMS

BILL OF MATERIALS

BAR SCHEDULE

NORMAL APPROACH												_ _ ° SKEW										_ _ ° SKEW											
Bar	a	b	b <sub>1</sub>	b <sub>2</sub>	c	c <sub>1</sub>	c <sub>2</sub>	e	e <sub>1</sub>	f		a	b	b <sub>1</sub>	b <sub>2</sub>	c	c <sub>1</sub>	c <sub>2</sub>	e	e <sub>1</sub>	f		a	b	b <sub>1</sub>	b <sub>2</sub>	c	c <sub>1</sub>	c <sub>2</sub>	e	e <sub>1</sub>	f	
No.	40	26	13	13	18	9	9	6	12	74		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6	
Size	#5	#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6	
Length	12'-8"	12'-10"	7'-4"	7'-4"	11'-8"	6'-2"	6'-2"	3'-0"	3'-0"	13'-4"																							
Reinforcing Steel (Grade 60) (Epoxy Coated)												Reinforcing Steel (Grade 60) (Epoxy Coated)												Reinforcing Steel (Grade 60) (Epoxy Coated)									
Concrete Pavement (10" Unif.)(AE)												Concrete Pavement ( _ " Unif.)(AE)												Concrete Pavement ( _ " Unif.)(AE)									
2,950 lbs.												lbs.												lbs.									
53.44 Sq. Yds.												Sq. Yds.												Sq. Yds.									

Note: Quantities listed for one approach slab only. Two required per bridge. Reinforcing steel and joint lengths shown for information only.

09	09-09-09	Revised Reinforcing Steel listing	S.W.K.	J.O.B.
08	05-14-09	Revised General Note	S.W.K.	J.O.B.
07	10-30-08	Added guardrail post detail at curb	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
CONCRETE BRIDGE APPROACH PAVEMENT ADJACENT TO ASPHALT SURFACE				
RD715				
FHWA APPROVAL 06-09-09 APPD. James O. Brewer				
DESIGNED	06-09-09	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	18	141

GENERAL NOTES

All work shall be done in conformity with the Standard Specifications applicable to the project.

The cost of all bars and joint material shown on this sheet is to be included in the bid price for Concrete Pavement.

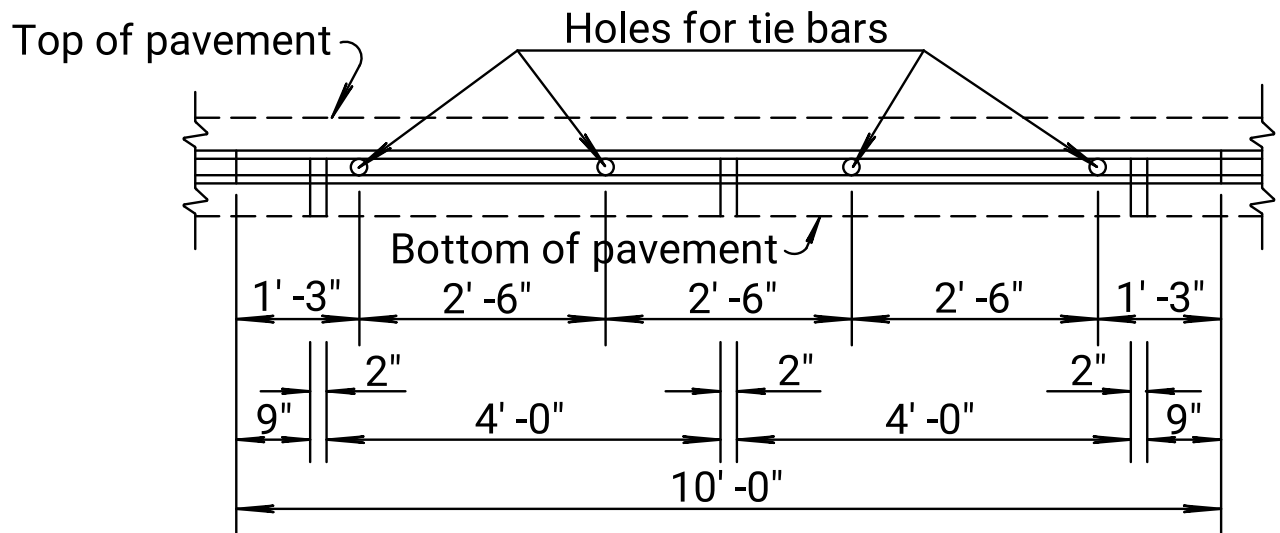
At each planned transverse joint location, a 4 to 6 inch wide strip of the pavement surface shall be protected from the texturing operation to provide a transverse textureless surface centered over the joint sawcut.

All sawed joints on this project shall be filled with sealant in accordance with Standard Specifications.

The 4 inch edge curb shall be constructed integral with the approach slab shoulder.

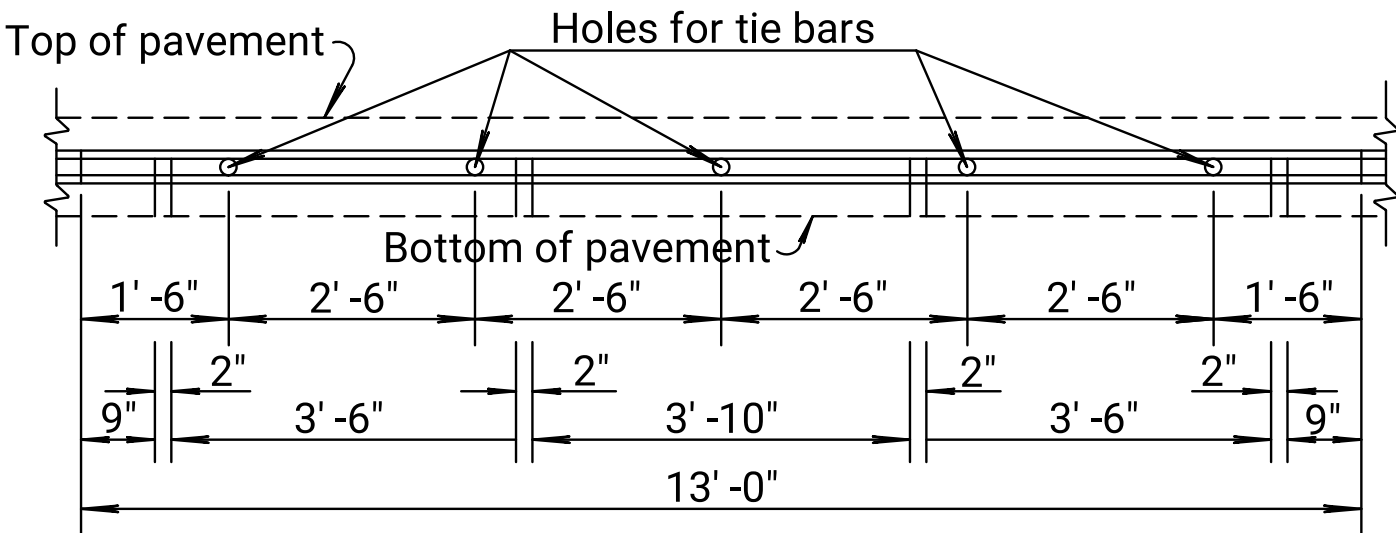
All materials and work required for this construction shall be subsidiary to the concrete approach slab.

Tie bars shall be evenly spaced along the length of the slab and no tie bars shall be within 12" of contraction joint.



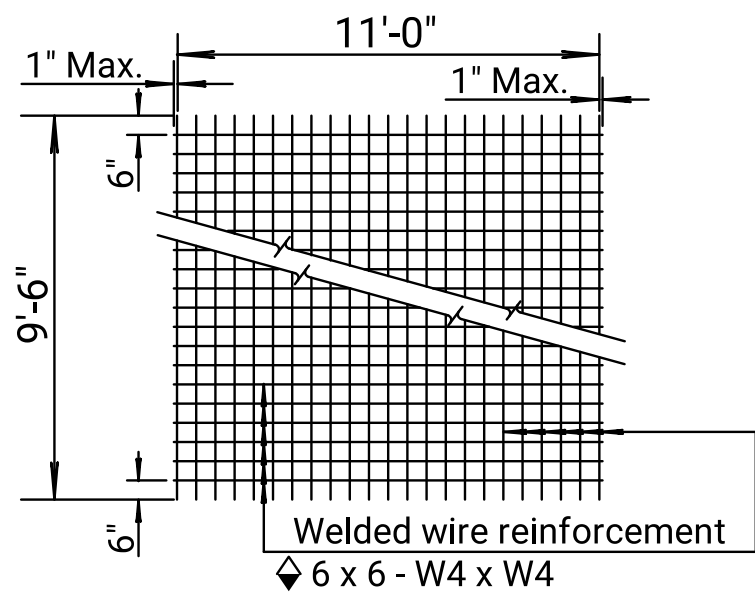
To be used only against forms. Shall not extend through contraction joints.

METAL STRIP FOR  
LONGITUDINAL CONSTRUCTION JOINT (10'-0")



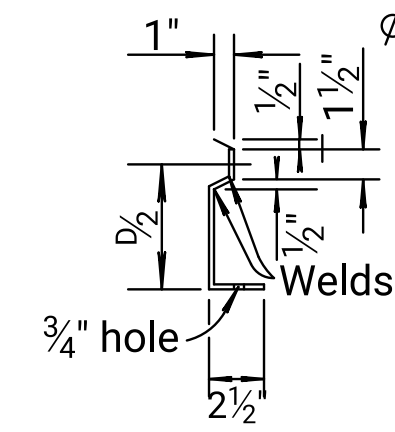
To be used only against forms. Shall not extend through contraction joints.

METAL STRIP FOR  
LONGITUDINAL CONSTRUCTION JOINT (13'-0")



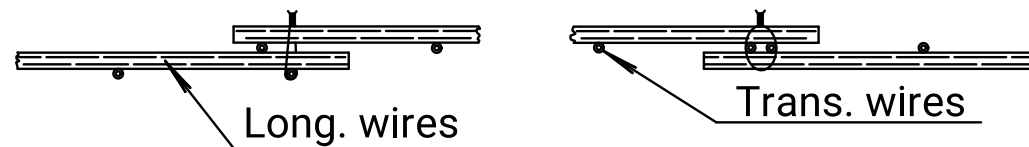
TYPICAL SHEET OF WELDED WIRE REINFORCEMENT  
FOR SPECIAL BRIDGE APPROACH PAVEMENT

◆ Note: Epoxy coated #3 bars longitudinally @ 12" ctrs. & #3 bars transversely @ 18" ctrs. may be substituted for each layer of epoxy coated welded wire reinforcement.



SECTION OF RECESSED  
FORM LEG

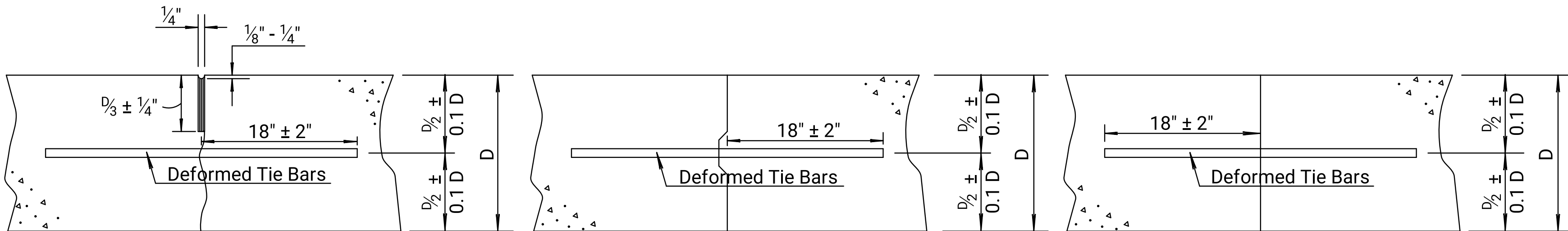
∅ Snap-in leg or other approved designs may be used in lieu of welded leg.



DETAIL OF LAP FOR WELDED WIRE REINFORCEMENT

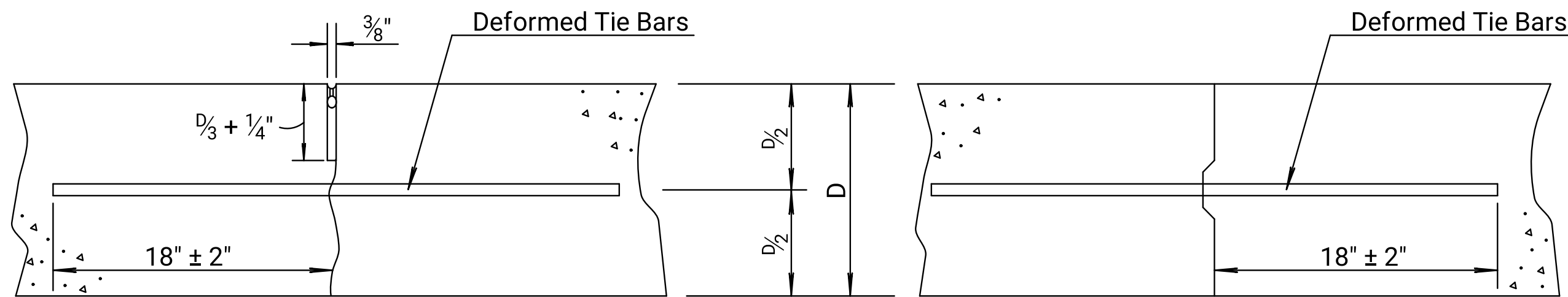
The lap shall extend beyond the first transverse or bag wire of each sheet.

The sheet shall be wired securely at the edges and at intervals not to exceed 2'-6" for the full width of the sheet. Approximate weight of welded wire reinforcement = 58 lbs. per 100 sq. ft. Other methods for fastening the sheets of welded wire reinforcement at the laps may be used with the approval of the Engineer.



LONGITUDINAL JOINTS

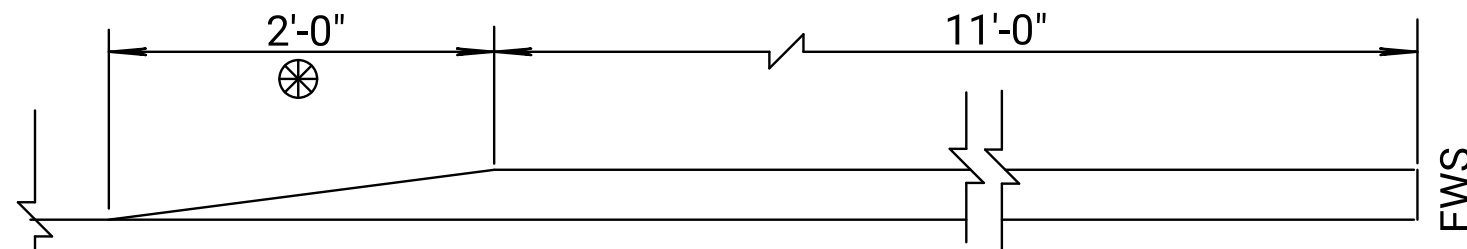
Note: For longitudinal construction joints the contractor has the option of using either the keyed or butt type. Place deformed tie bars mid-depth of the shoulder.



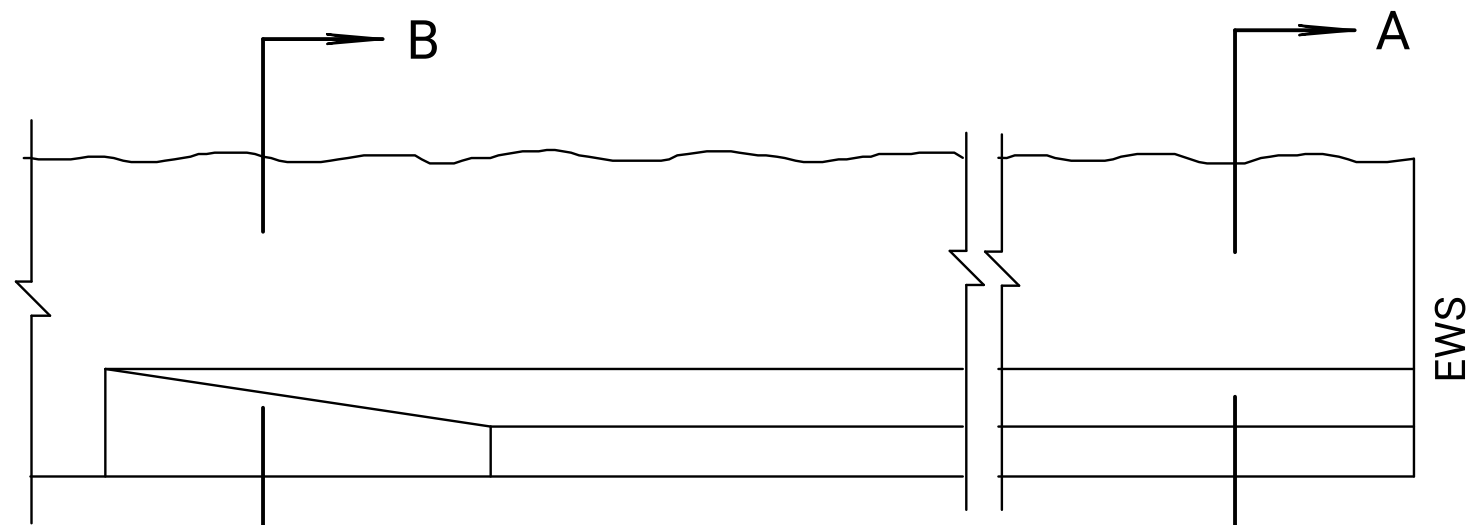
TRANSVERSE JOINTS

Note: A construction joint is required when the concrete placement has been interrupted for a substantial length of time or at the end of a day's placement.

⊗ No 4" Curb transition when adjacent to Flume Inlet.

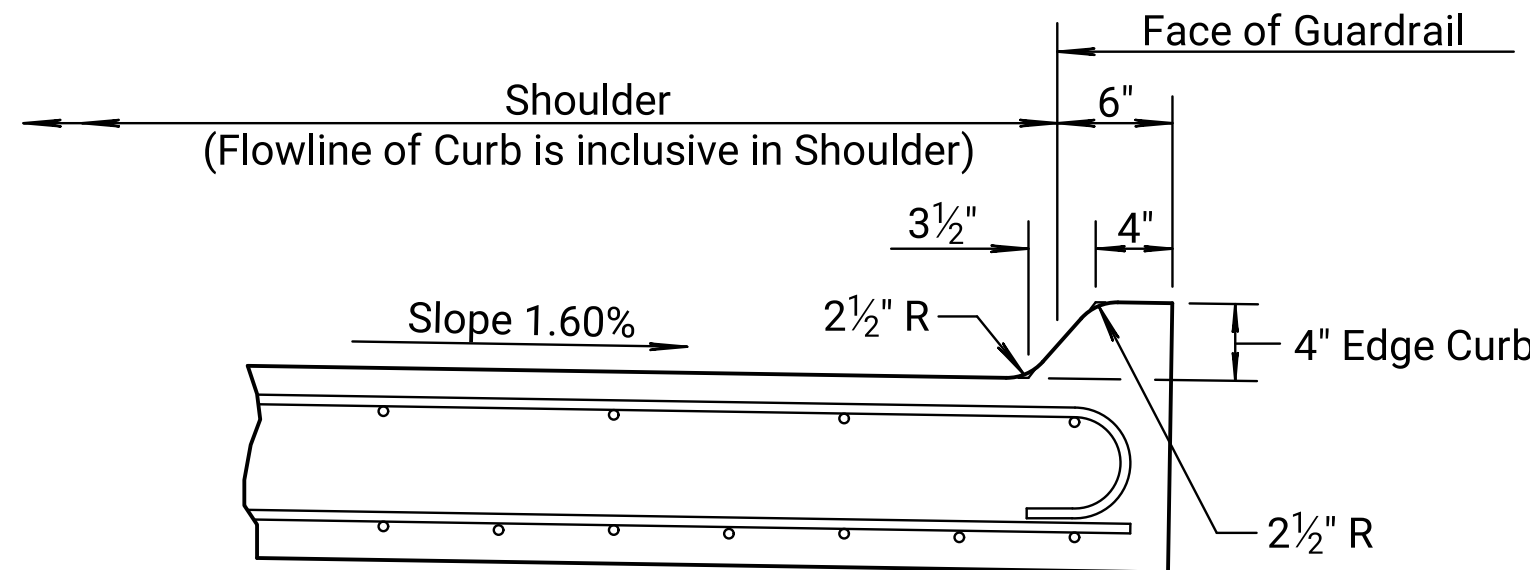


ELEVATION

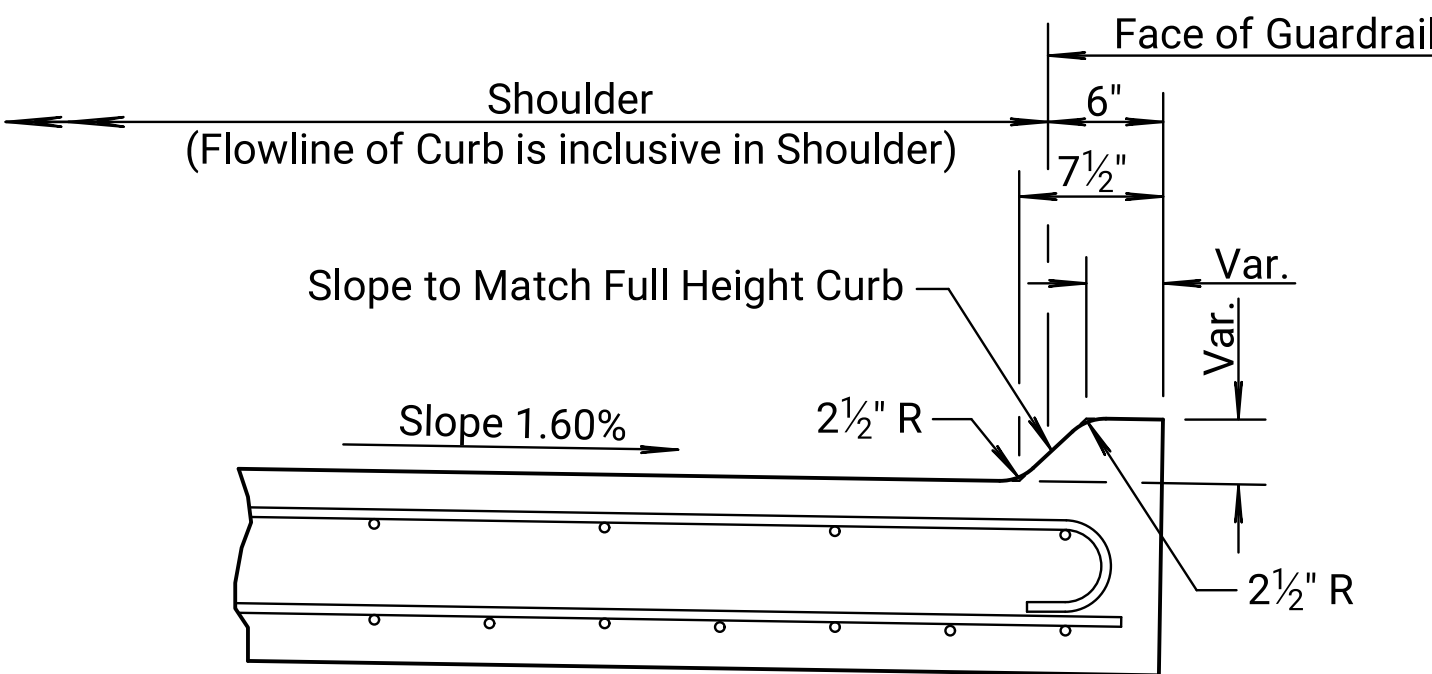


PLAN VIEW

4" EDGE CURB DETAIL



SECTION A-A



SECTION B-B

13	05-17-13	Revised Note, Longitudinal Joints	S.W.K.	J.O.B.
12	05-14-09	Pres. Relief Jt. to RD712/tie bar lab.	S.W.K.	J.O.B.
11	10-23-08	Revised Sec. A-A and Sec. B-B	S.W.K.	J.O.B.

NO.	DATE	REVISIONS	BY	APPD
-----	------	-----------	----	------

KANSAS DEPARTMENT OF TRANSPORTATION

MISCELLANEOUS DETAILS  
FOR CONCRETE  
BRIDGE APPROACH PAVEMENT

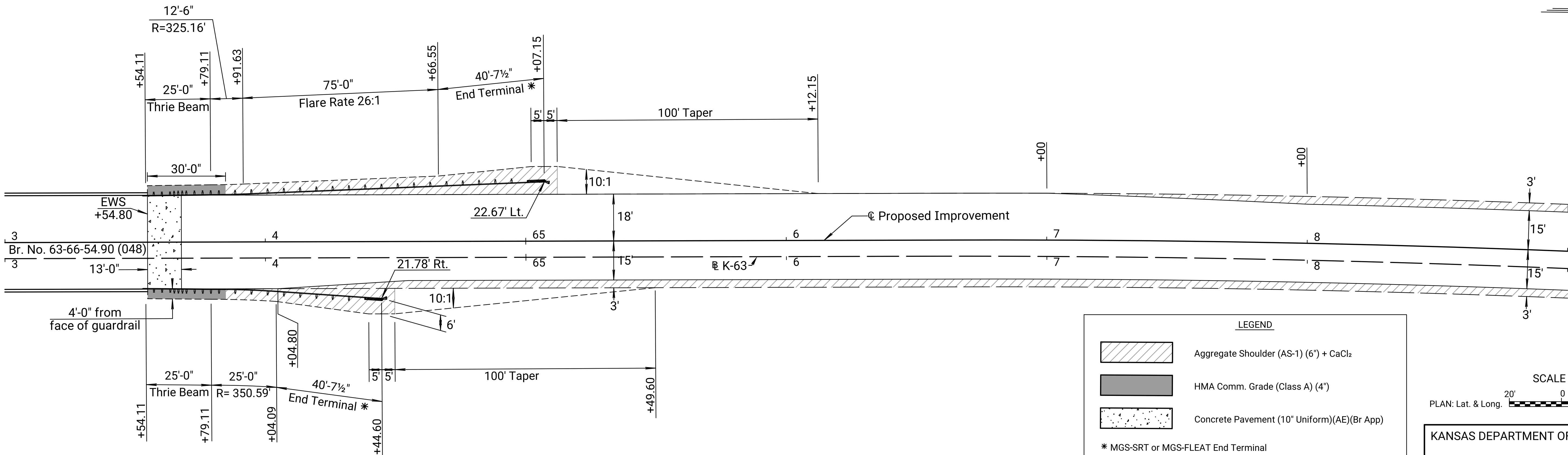
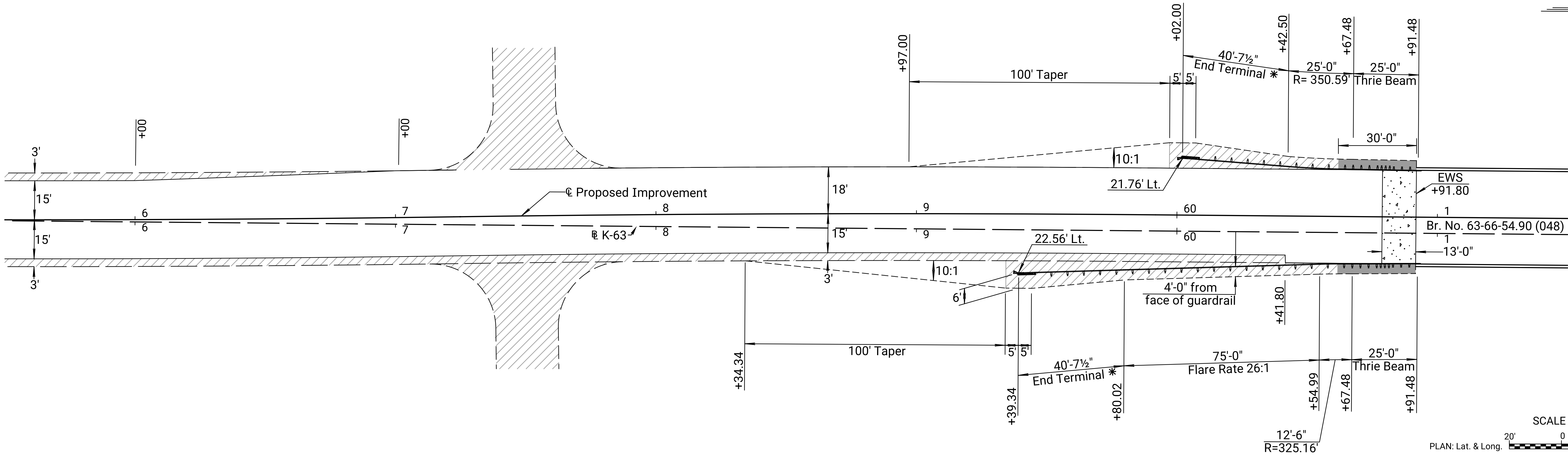
RD711

DESIGNED	10-23-13	APPD.	James O. Brewer
DESIGN CK.	10-23-13	QUANTITIES	TRACED
DESIGN CK.	10-23-13	QUAN. CK.	TRACE CK.

DOT Graphics Certified 07-18-2022 Sh. No. 18

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	19	141

DATE	12/22
BY	B. Andrews
REFERENCES NOTED	
REFERENCES CHECKED	



**LEGEND**

- Aggregate Shoulder (AS-1) (6") + CaCl<sub>2</sub>
- HMA Comm. Grade (Class A) (4")
- Concrete Pavement (10" Uniform)(AE)(Br App)

\* MGS-SRT or MGS-FLEAT End Terminal  
Note: All stations and offsets are to face of rail  
Note: All stationing shown is perpendicular to K-63

SCALE  
PLAN: Lat. & Long. 20' 0 20' 40'

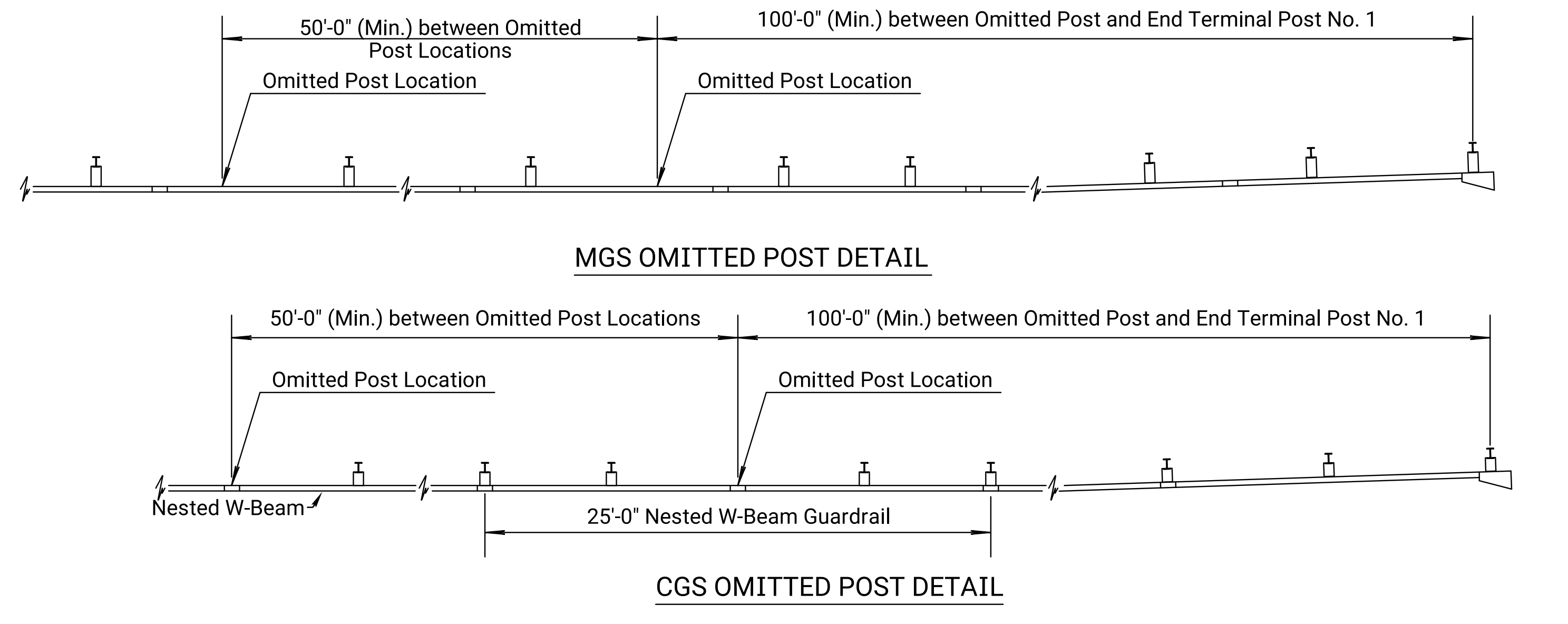
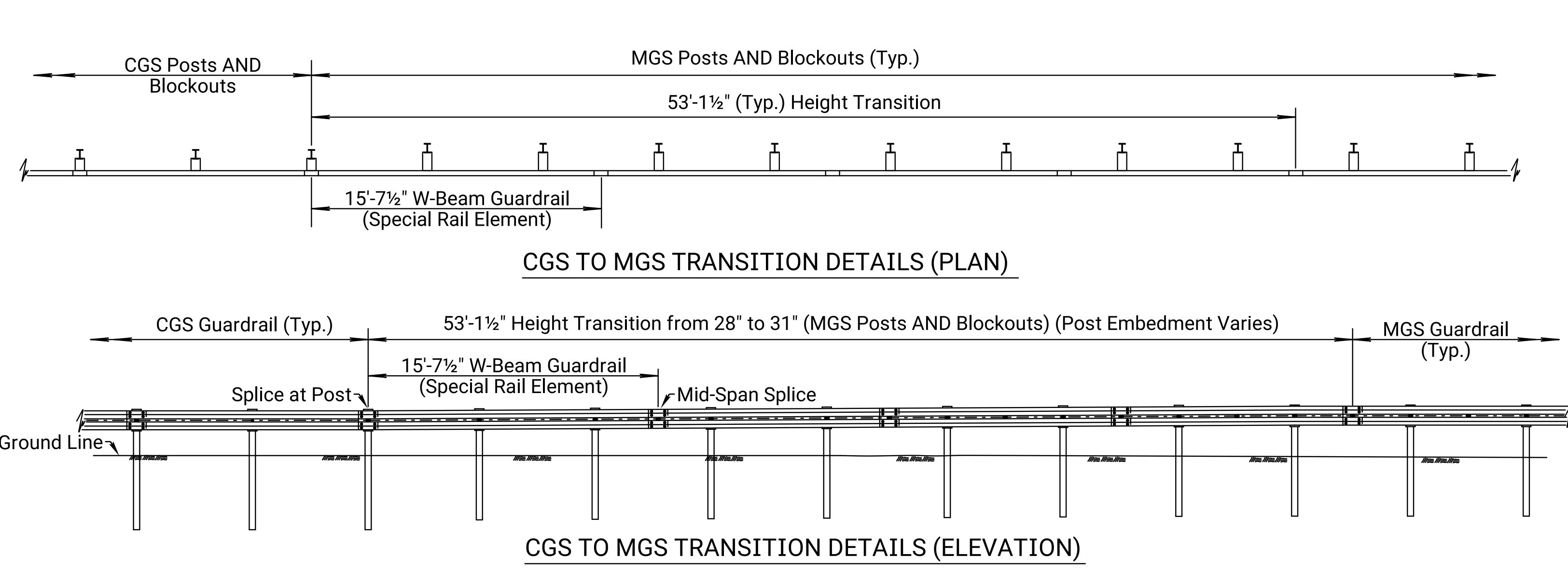
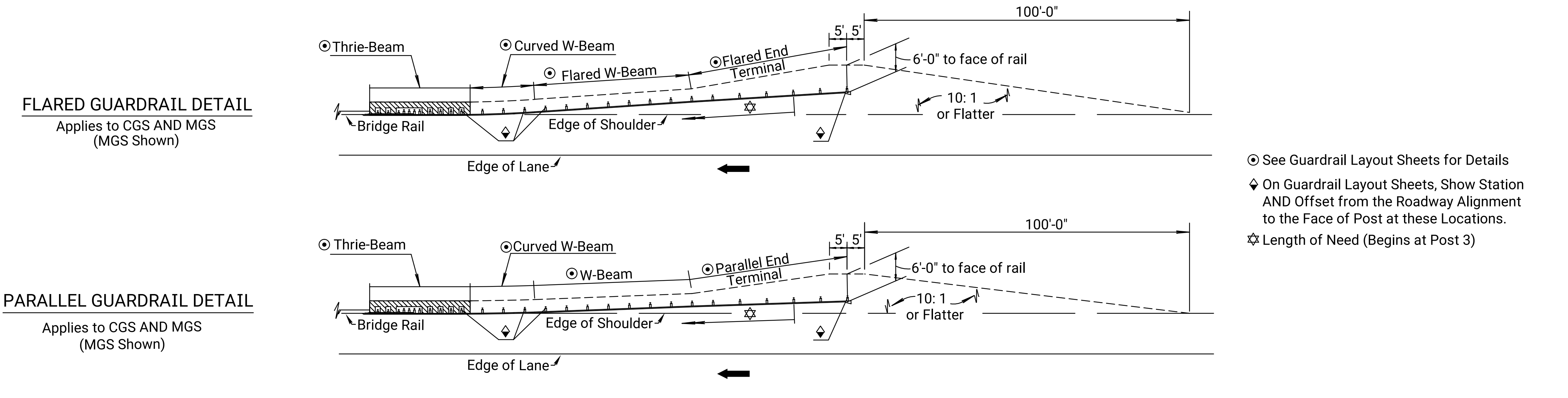
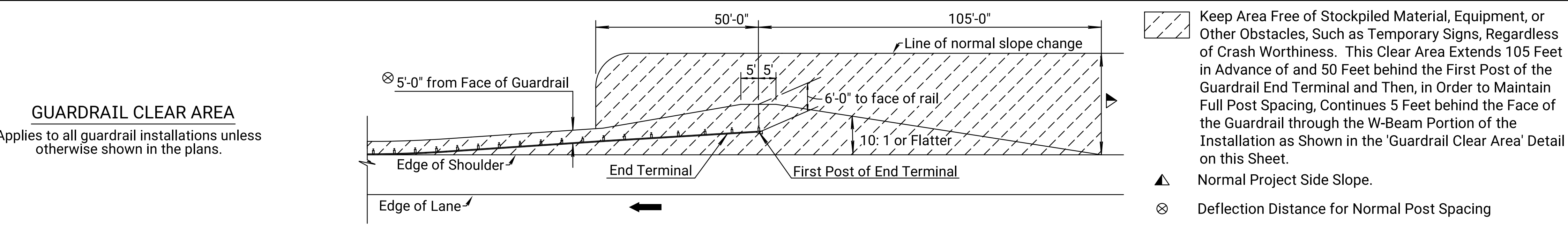
KANSAS DEPARTMENT OF TRANSPORTATION

**GUARDRAIL LAYOUT**

Note to Designer - Design guardrail installations using guidance shown on KDOT's 'Guardrail Typical Alignments' Standard Drawings. 'Flared' guardrail installations are preferred over 'Parallel' or 'Zero Flare' installations. Where 'Flared' or 'Parallel' installations are used, the flare rate of the guardrail end terminal typically matches the flare rate of the remaining guardrail installation. For 'Zero Flare' installations, 'Parallel' guardrail end terminals should be designed using typical flare rates of 50:1 or flatter for the length of the end terminal. However, while 50:1 or flatter flare rates are typical for 'Parallel' guardrail end terminals, these end terminals may be flared as steep as 26:1 or flatter in order to offset the end terminal head as far from the edge of the through traveled lane as practicable.

Plotted by : Brogan.Andrews@ks.gov  
File : KA572901rss606-01.dgn

16-APR-2024 17:28



MIDWEST GUARDRAIL SYSTEM (MGS) END TERMINALS									
END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (MGS-FLEAT)	Flared	31"	NCHRP 350	Yes	Yes	Yes	Road Systems	40'-7½"	37'-6"
Guardrail End Terminal (MGS-SRT)	Flared	31"	NCHRP 350	Yes	Yes	No	Trinity Industries	40'-7½"	37'-6"
Guardrail End Terminal (MGS-MSKT)	Parallel	31"	MASH	Yes	No	Yes	Road Systems	46'-10½"	46'-10½"
Guardrail End Terminal (MGS-SOFTSTOP)	Parallel	31"	MASH	Yes	No	Yes	Trinity Industries	46'-10½"	50'-9½"

CONVENTIONAL GUARDRAIL SYSTEM (CGS) END TERMINALS									
END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (FLEAT)	Flared	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	37'-6"	37'-6"
Guardrail End Terminal (SRT)	Flared	28"	NCHRP 350	Yes	Yes	No	Trinity Industries	37'-6"	37'-6"
Guardrail End Terminal (SKT)	Parallel	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	50'-0"	50'-0"

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	20	141

**GENERAL NOTES**

Install the guardrail end terminals according to the Manufacturer's Installation Manual. The Contractor will furnish a copy of the Manufacturer's Installation Manual to the Engineer prior to the start of the installation.

Use approved steel (preferred) or wood posts provided by the Manufacturer. The guardrail end terminal post type may be independent of the post type used in the remainder of the installation. However, no mixing of post types is permitted in the remaining w-beam and thrie-beam installation.

Use approved polymer (preferred) or wood blockouts provided by the Manufacturer. The guardrail end terminal blackout size and type may be independent of the blackout size and type used in the remainder of the installation. For blackout size and types for the remaining w-beam and thrie-beam portion of the installation see the details shown on KDOT's 'Guardrail Post Details' and 'Guardrail Thrie-Beam Transition Details' Standard Drawings.

Apply retroreflective sheeting to the end terminal impact head before installation.

Tighten all cable anchor assemblies as per the Manufacturer's Installation Manual.

Lap w-beam and thrie-beam guardrail splices, in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final traffic configuration. Lap end terminal splices per the Manufacturer's Installation Manual in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final configuration.

The minimum length of w-beam guardrail required between the thrie-beam transition and the guardrail end terminal is 12'-6" for all installations; unless otherwise stated in the Manufacturer's Installation Manual.

Where pavement with a thickness less than or equal to 8" is encountered during installation, use the details shown on KDOT's 'Guardrail Post Details' Standard Drawings to provide openings in the pavement for the guardrail posts. Where pavement with a thickness greater than 8" or geologic rock is encountered during installation, follow the Manufacturer's Installation Manual for guidance. Where the Manufacturer's Installation Manual does not address pavement with a thickness greater than 8" or geologic rock, contact the manufacturer for instructions or install the guardrail posts as directed by the Engineer.

All work and materials required for w-beam and thrie-beam guardrail installations are paid for under the appropriate bid items for either CGS or MGS guardrail depending on the type of installation.

All work and materials required for guardrail end terminal installations are paid for under the bid item for the selected guardrail end terminal. See the table on this sheet for the appropriate end terminal bid item information.

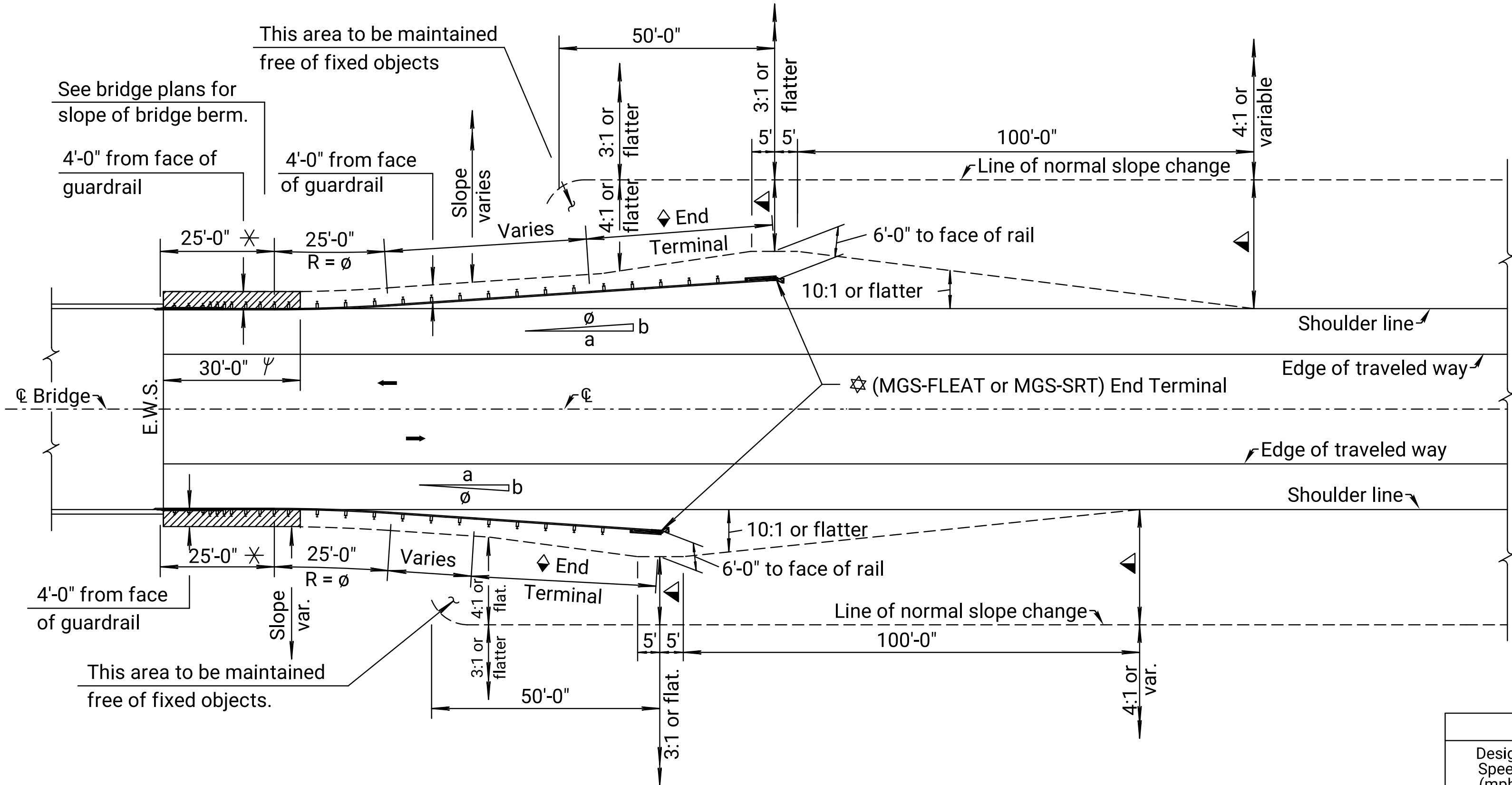
02	09-05-18	ADD. OMITTED POST AND TRANS. DETAILS		A.L.R.	T.T.R.
01	06-05-18	INITIAL RELEASE		A.L.R.	T.T.R.
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
GUARDRAIL AUXILIARY DETAILS					
RD606					
FHWA APPROVAL		09-25-18	APPD.	Scott W. King	
DESIGNED	DETAILED	QUANTITIES		TRACED	
DESIGN CK.	DETAIL CK.	QUAN CK.		TRACE CK.	



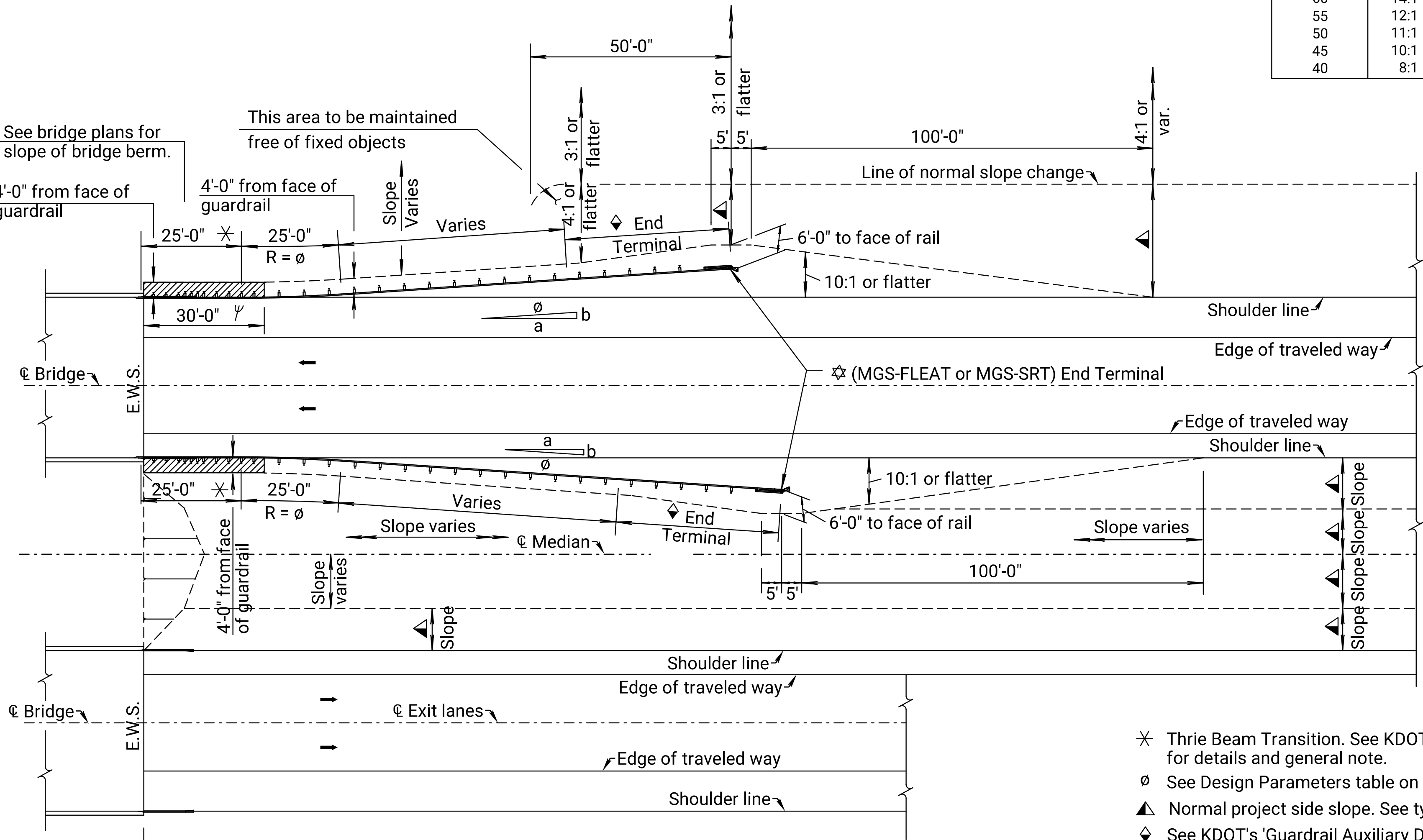
Notes to Designer: Determine guardrail length of need using either KDOT's Length of Need Equation or a graphic design approach with an L<sub>1</sub> distance measured from the edge of the area of concern to the P.I. of the curved guardrail section. Combine materials for asphalt widening in the plan quantities.

Optional: If approach side is within the shyline, use a flare rate of 2a:b for all quadrants.

Plotted by: Brogan.Andrews@ks.gov 16-APR-2024 17:32  
File: KA572901rss612c-01.dgn

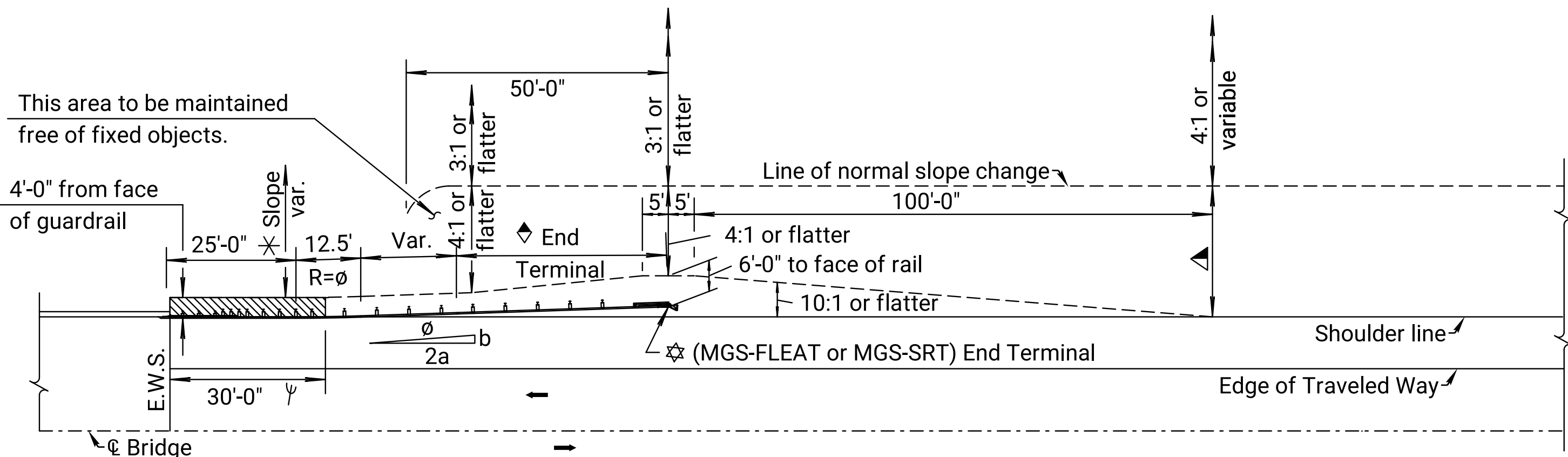


THRIE BEAM TRANSITION - TWO LANES



THRIE BEAM TRANSITION - FOUR LANES (DIVIDED)

Note: Use flare rate of a:b and curve length of 25'-0" when guardrail is beyond shyline. Use flare rate of 2a:b and curve length of 12'-6" when guardrail is located inside the shy line.



ALTERNATE TREATMENT - TWO LANES (Flare Rate = 2a:b)

DESIGN PARAMETERS				
Design Speed (mph)	Flare Rate (a:b)	Radius (R)	Flare Rate (2a:b)	Radius (R)
70	15:1	375.55'	30:1	375.14'
60	14:1	350.59'	26:1	325.16'
55	12:1	300.69'	24:1	300.17'
50	11:1	275.76'	21:1	262.70'
45	10:1	250.83'	18:1	225.23'
40	8:1	201.04'	16:1	200.26'

- ✱ Thrie Beam Transition. See KDOT's 'Thrie Beam Guardrail Transition Details' Standard Drawings for details and general note.
- ∅ See Design Parameters table on this sheet for radius, length of curve and flare rate information.
- ▲ Normal project side slope. See typical sections.
- ◆ See KDOT's 'Guardrail Auxiliary Details' Standard Drawing.
- ✱ 4" Asphalt material placed on 4'-0" embankment widening unless flume inlet and slope drain is constructed. See KDOT's 'Guardrail Post Details' Standard Drawings for "Post in Pavement" details.
- ✱ The minimum length of w-beam guardrail required between the guardrail end terminal and any transition section, including the thrie-beam transition, is 12'-6".

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	21	141

04	06-05-18	Removed Flare-beyond-the-Flare	A.L.R.	T.T.R.
03	05-15-17	Removed X-LITE	A.L.R.	S.W.K.
02	06-07-12	Revised Note to Designer	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
THRIE BEAM GUARDRAIL (MGS) BRIDGE APPROACH TRANSITION TYPICAL ALIGNMENTS (FLARED)				
RD612C				
FHWA APPROVAL		06-19-18	APPD.	Scott, W. King
DESIGNED	DETAILS	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	22	141

GENERAL NOTES

Install flexible markers on a post behind the guardrail bolt head on the traffic side of guardrail installations at a spacing not to exceed 25'. No marker is installed between the head and post #5 when the guardrail is terminated with a crashworthy end terminal.

Install flexible markers on the top of bridge rails at a spacing not to exceed 50', except for long bridges (greater than 200' long), where spacing may be increased to 100'.

Install flexible markers on the top of concrete safety barrier at a spacing not to exceed 100', except for barrier along a horizontal curve or along ramps and ramp tapers, where spacing is not to exceed 50'.

Where the height of the bridge rail or concrete barrier is greater than 32", mount the flexible markers on the side of the barrier at a height of 32" as shown on this sheet.

For guardrail, bridge rail, or concrete safety barrier located on two-way roadways, use flexible markers with white/silver high intensity reflective sheeting on both sides.

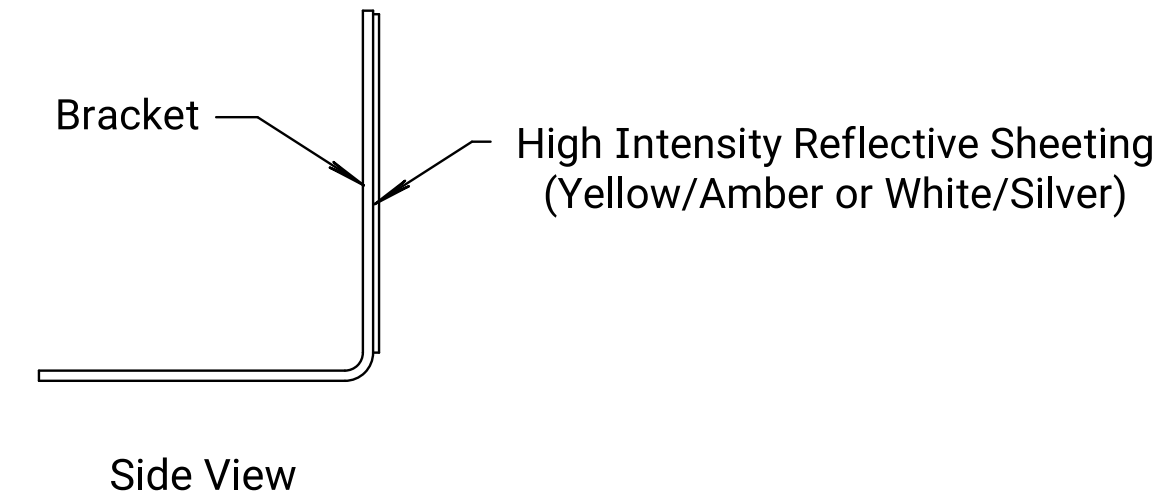
For guardrail located on one-way or divided roadways, use flexible markers with reflective sheeting installed on the approach traffic side of the bracket only. For bridge rail or concrete safety barrier located on the outside edge of one-way or divided roadways, use flexible markers with reflective sheeting installed on the approach traffic side of the bracket only. For bridge rail or concrete safety barrier located in the median, use flexible markers with reflective sheeting installed on both sides of the bracket. Match the color of the marker (yellow/amber or white/silver) to the color of the pavement marking adjacent to the traffic lane.

Use High Impact Polycarbonate Flexible Guardrail Marker with High Intensity Reflective Sheeting or an approved equivalent, see Standard Specifications.

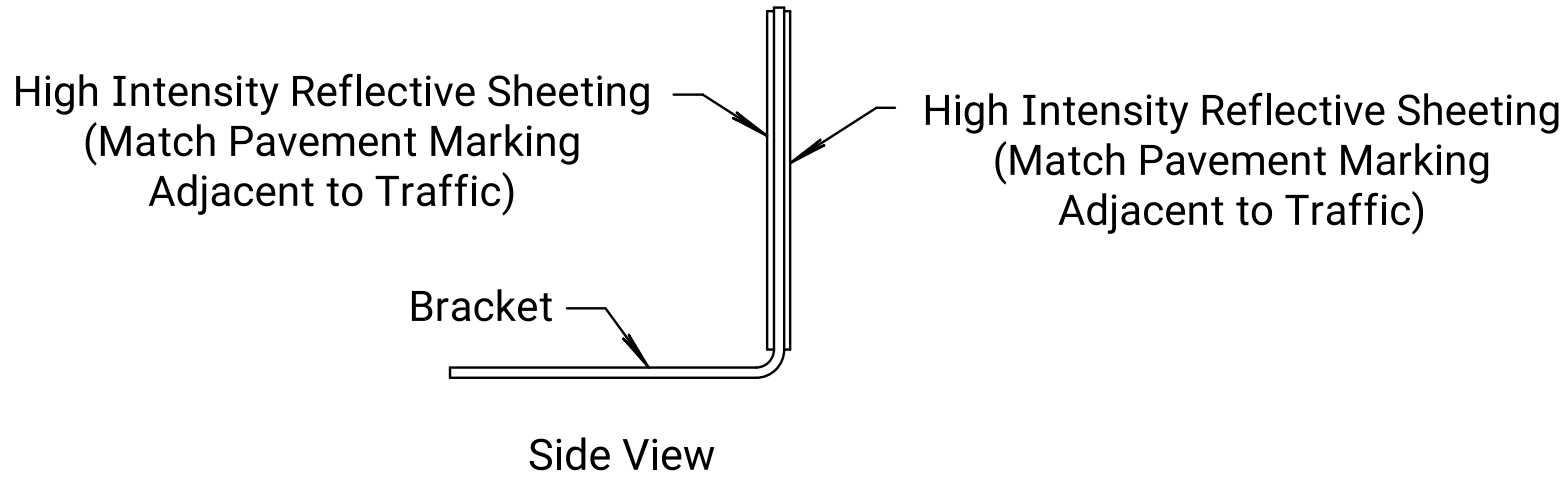
Use zinc or cadmium plated fasteners that comply with Standard Specifications.

Work and materials required for installation of markers on guardrail, bridge rail, or concrete safety barrier are subsidiary to other bid items in the contract.

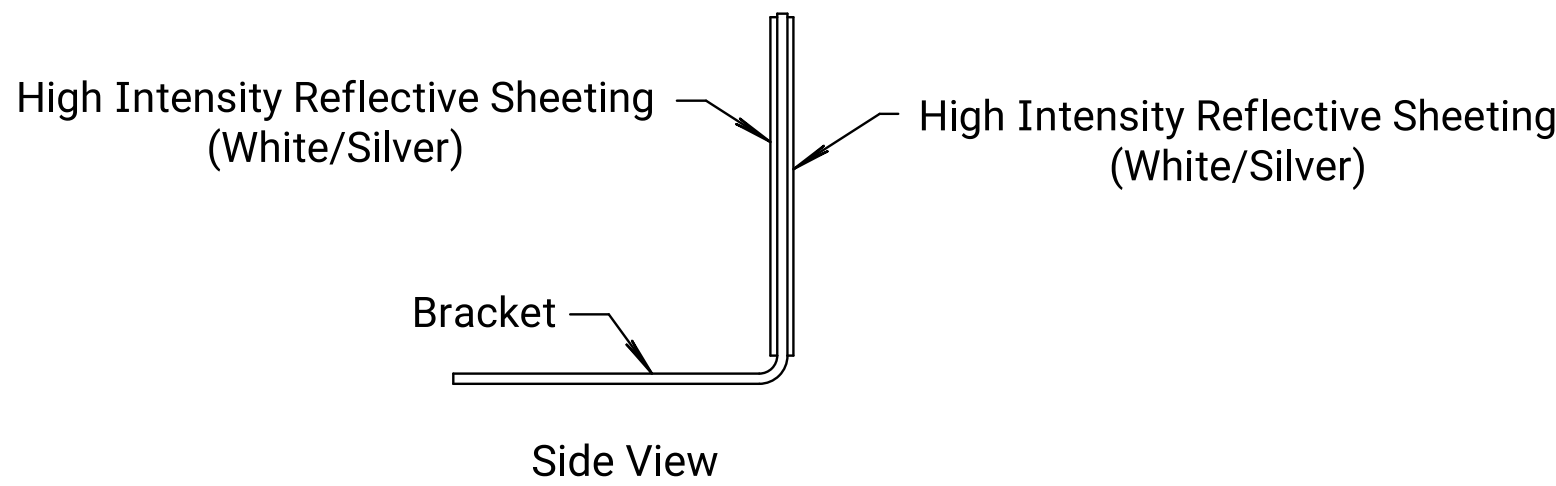
Install flexible markers for the final (permanent) traffic configuration.



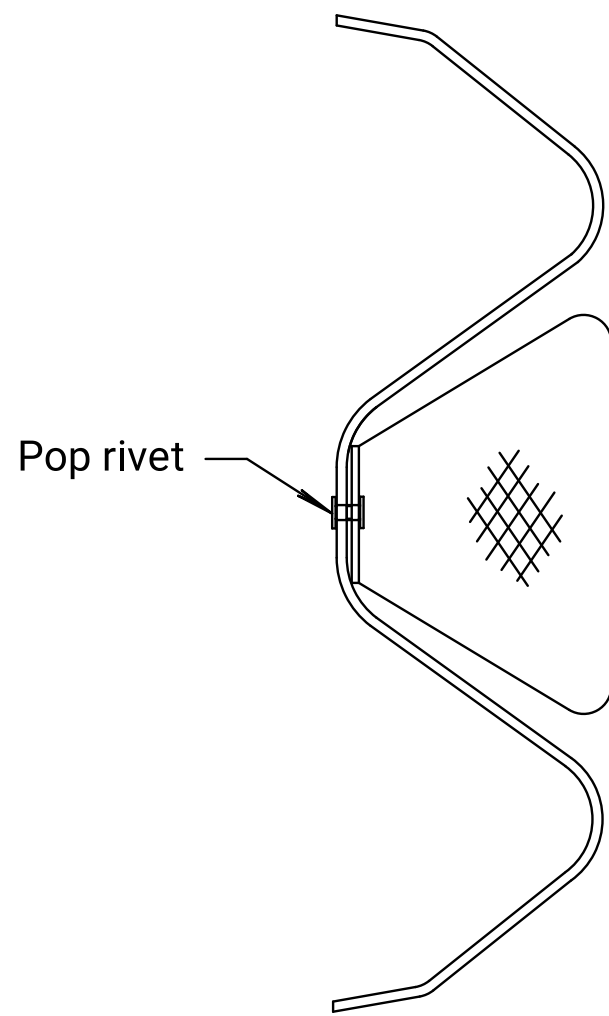
Flexible Marker  
One-Way Traffic



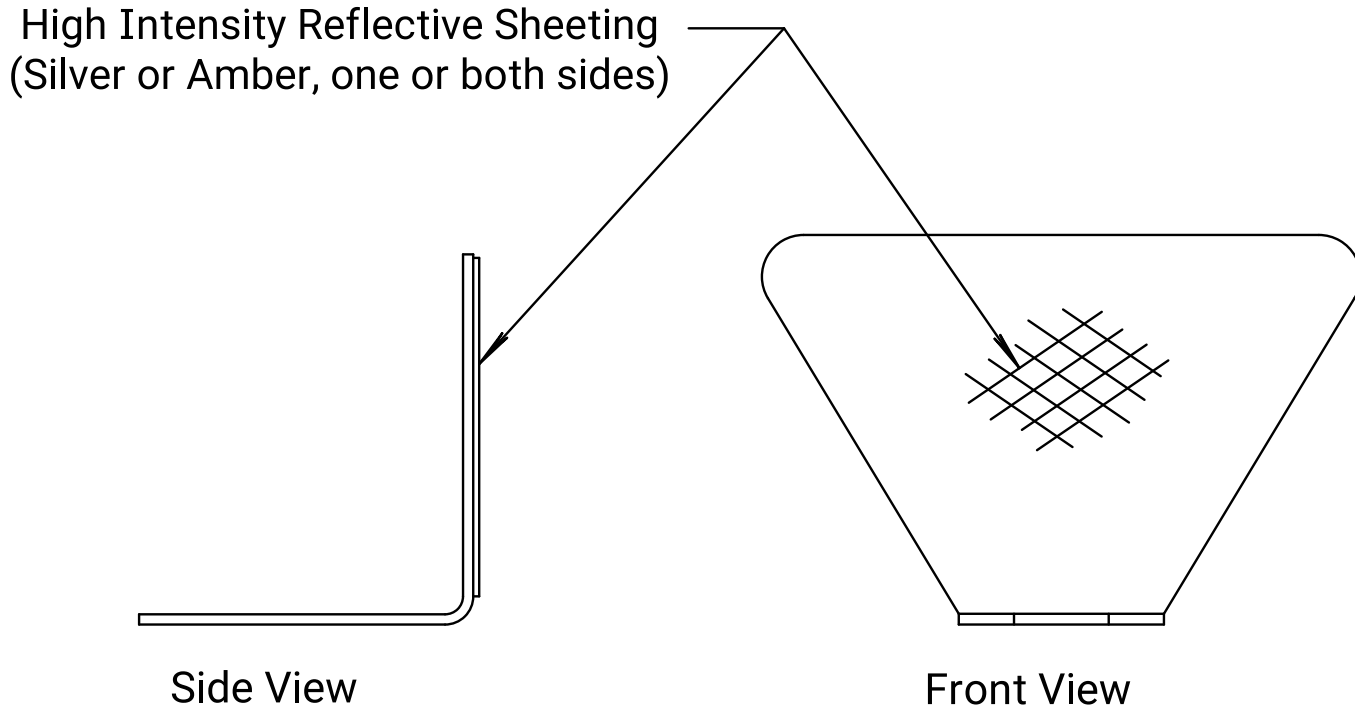
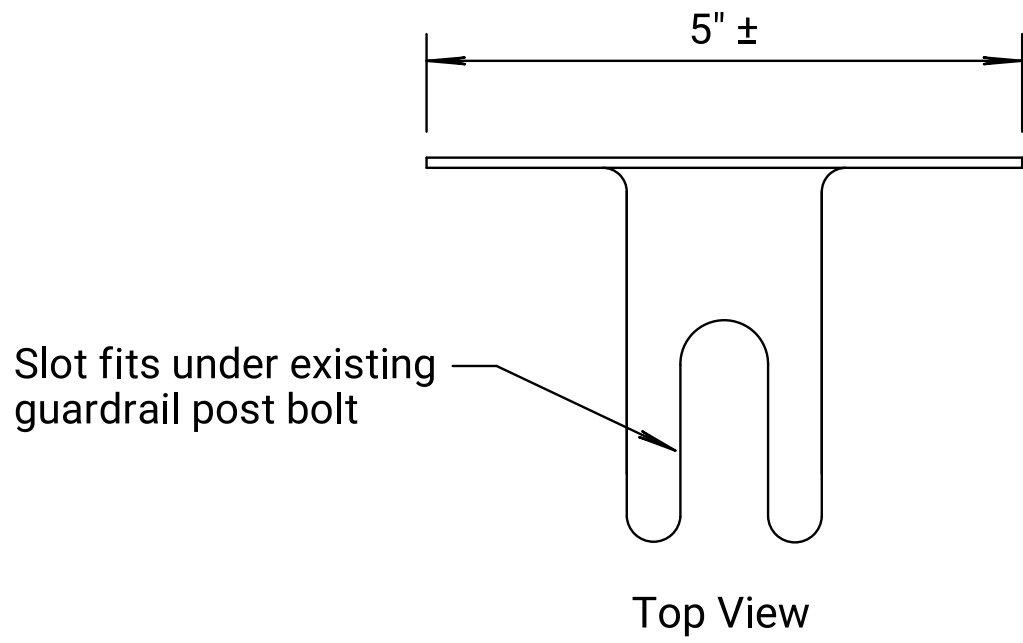
Flexible Marker  
Median Locations



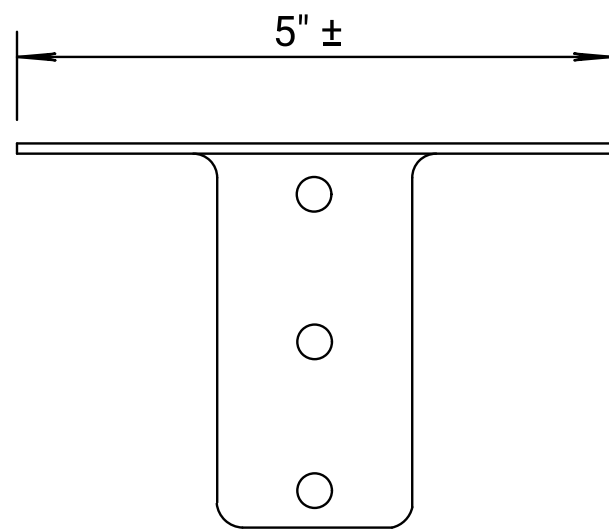
Flexible Marker  
Two-Way Traffic



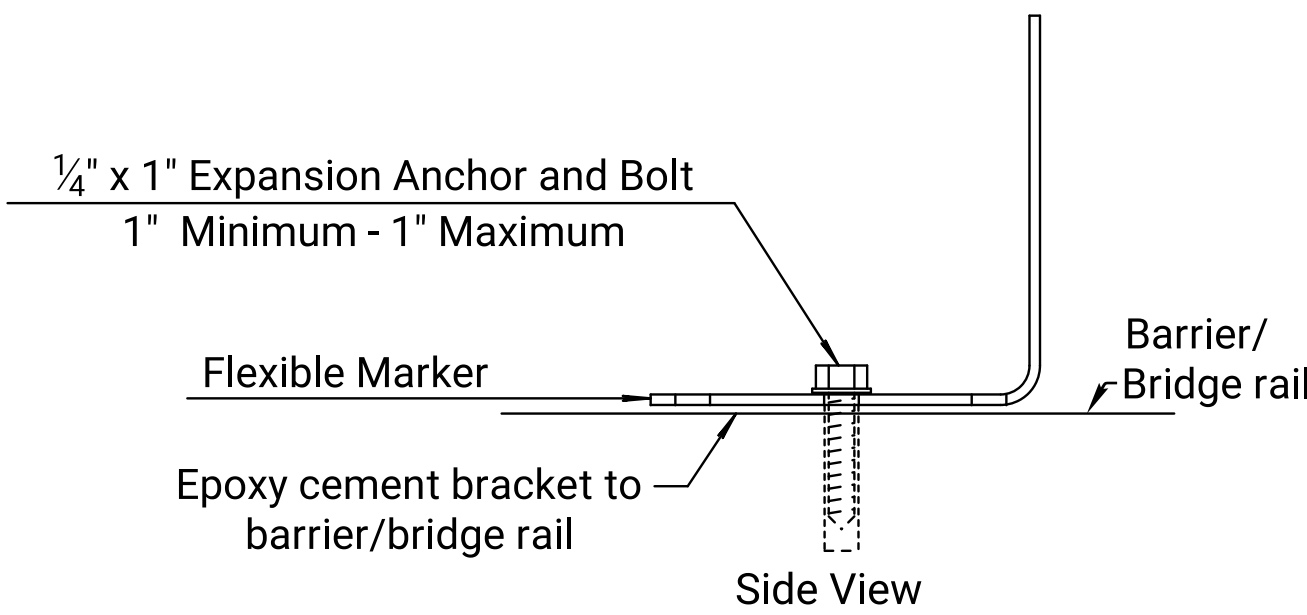
Typical Mounting on W-Beam  
Pop rivet attachment to Guardrail when necessary.



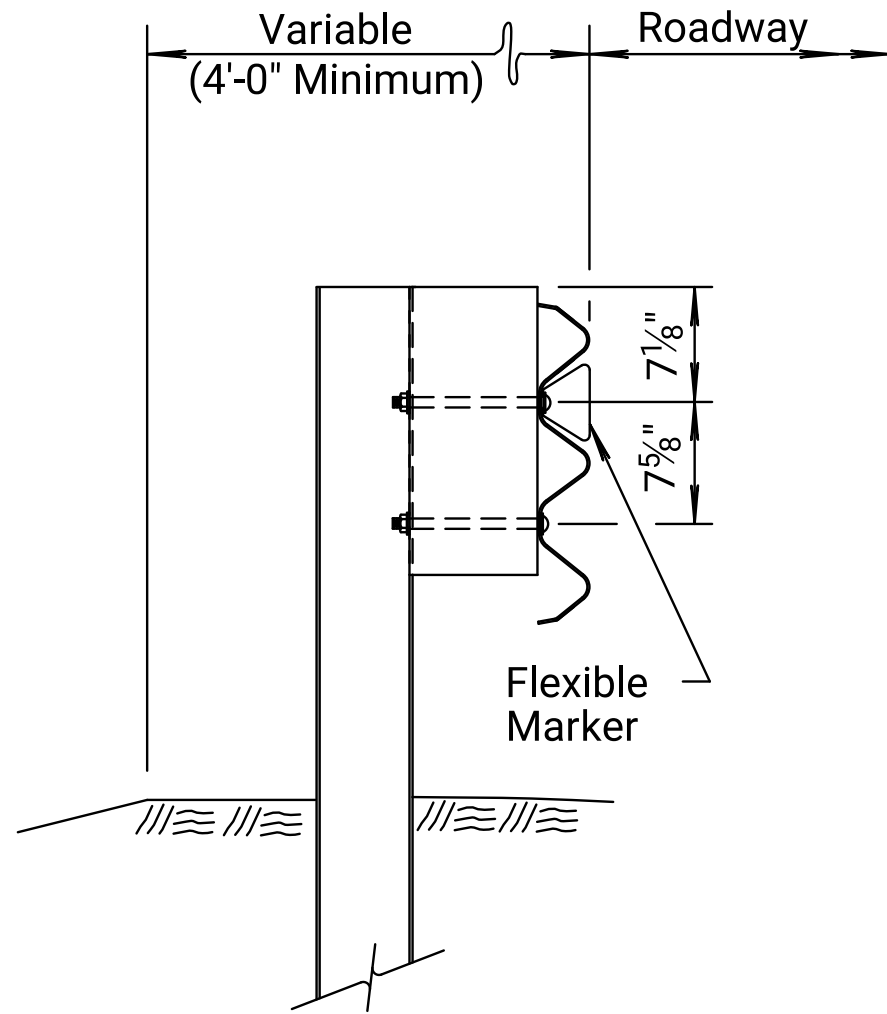
Flexible Guardrail Marker  
(High Impact Polycarbonate approx. .085" thick, 5 1/4" x 3")



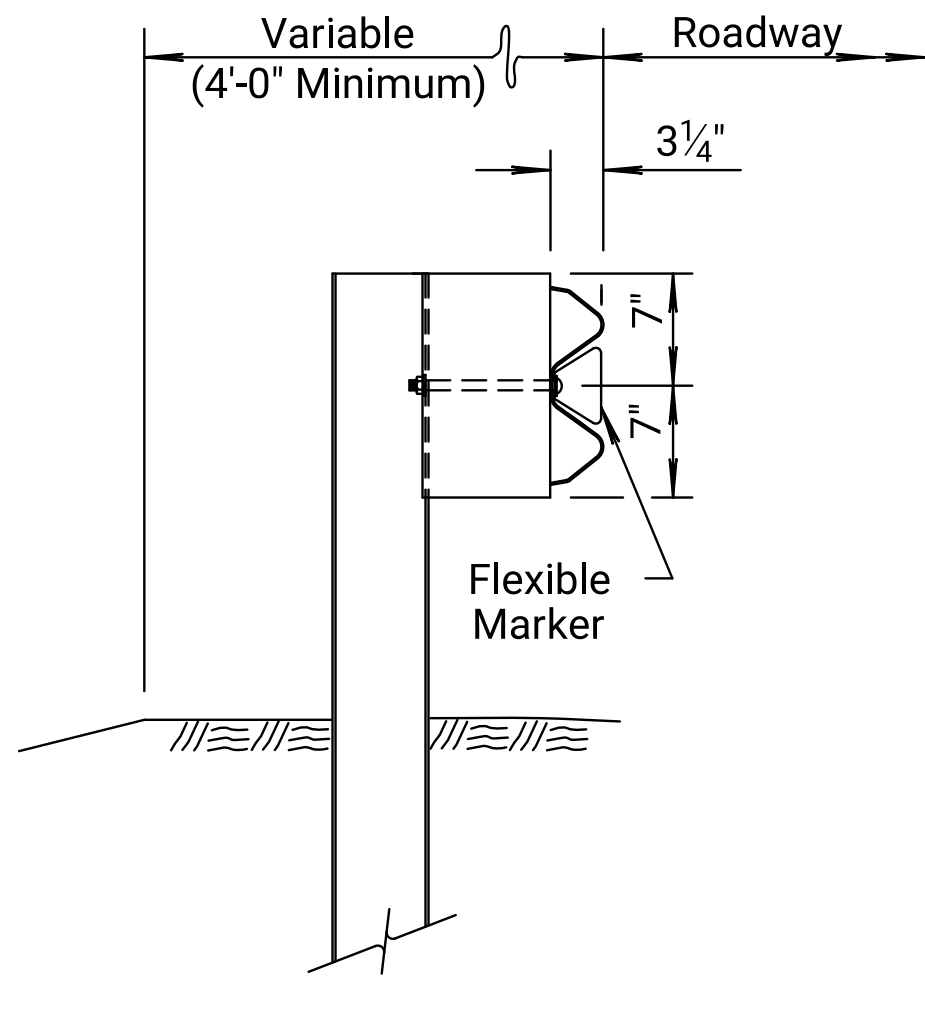
Barrier/Bridge Rail



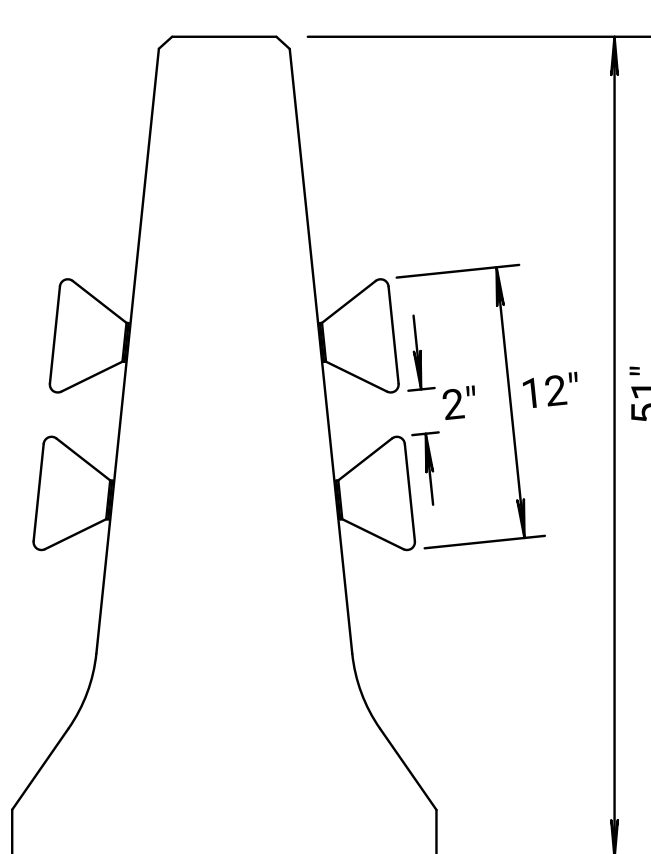
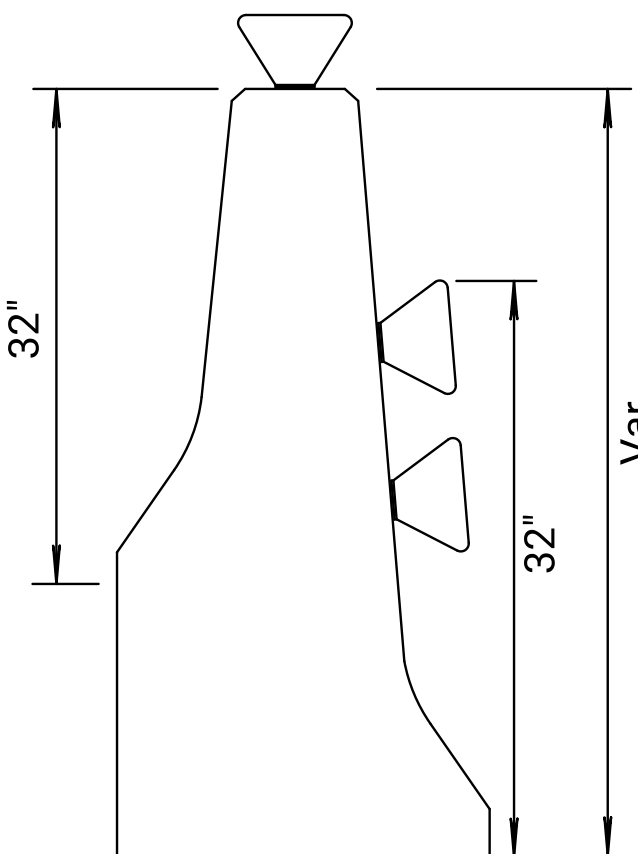
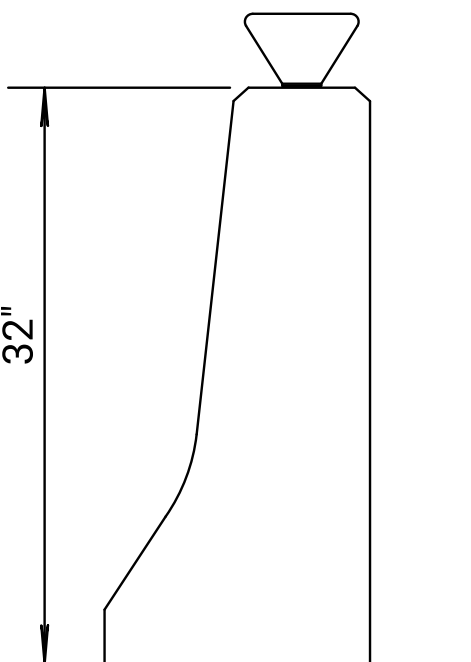
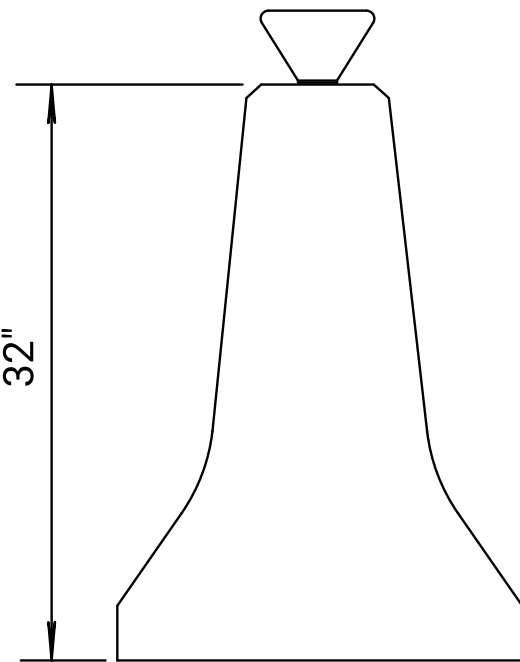
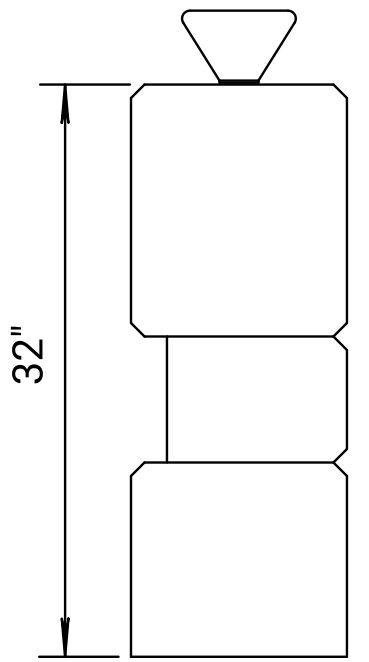
Method of Attaching Flexible  
Marker to Barrier/Bridge Rail



THRIE-BEAM GUARDRAIL



W-BEAM GUARDRAIL



TYPICAL BARRIER/BRIDGE RAIL MOUNTING DETAILS

09	09-11-17	Rev. Det. Markers, Rev. Gen. Note	A.L.R.	S.W.K.
08	11-15-10	Revised notes	S.W.K.	J.O.B.
07	12-21-08	AKT marker or approved equal	S.W.K.	J.O.B.

NO.	DATE	REVISIONS	BY	APPD
-----	------	-----------	----	------

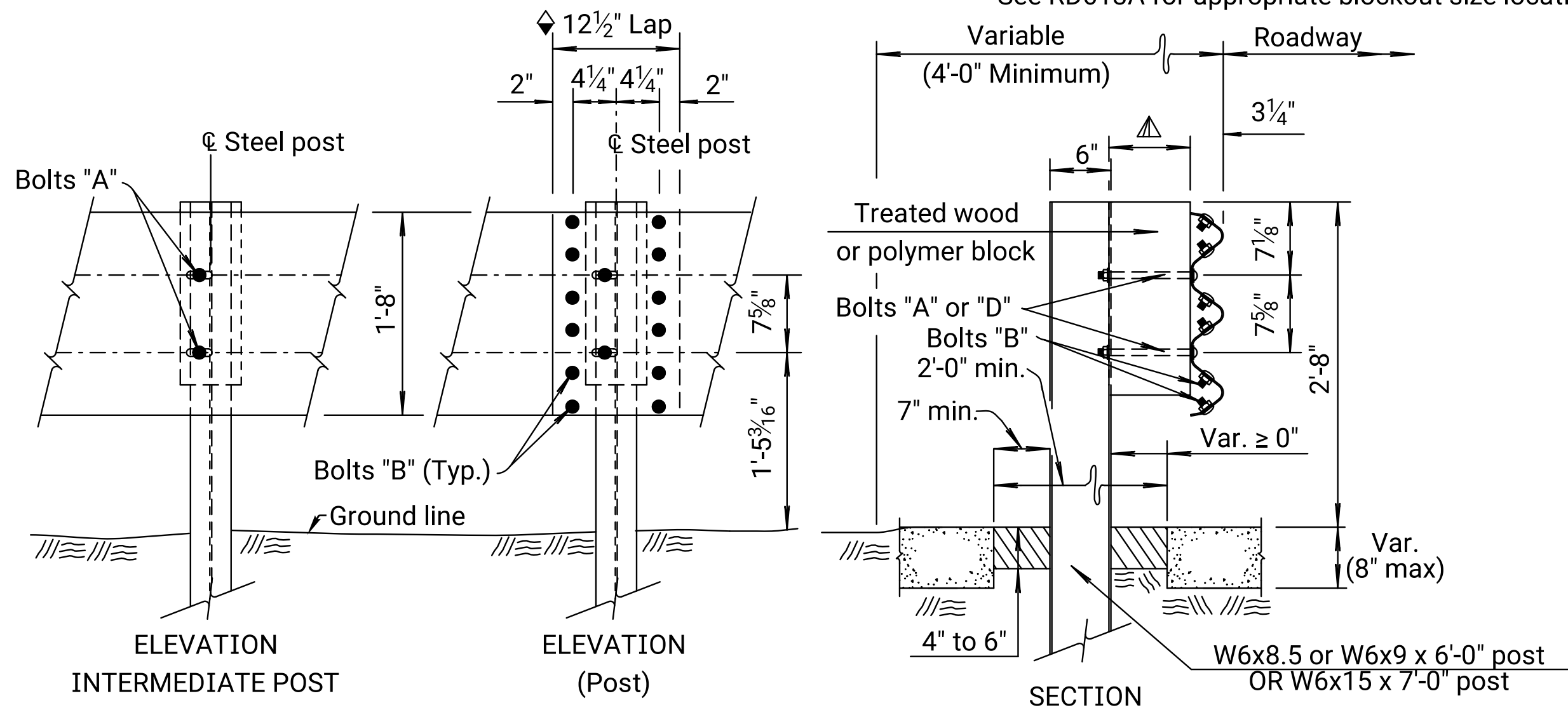
KANSAS DEPARTMENT OF TRANSPORTATION				
MARKER DETAILS FOR GUARDRAIL, BARRIER, AND BRIDGE RAILS				
RD610				
FHWA APPROVAL		03-15-18	APPD.	Scott W. King
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

Notes to Designer: For posts installed in pavement thicker than 8" or posts installed in rock formations refer to AASHTO's Roadside Design Guide for details then revise this drawing and all supporting drawings appropriately.

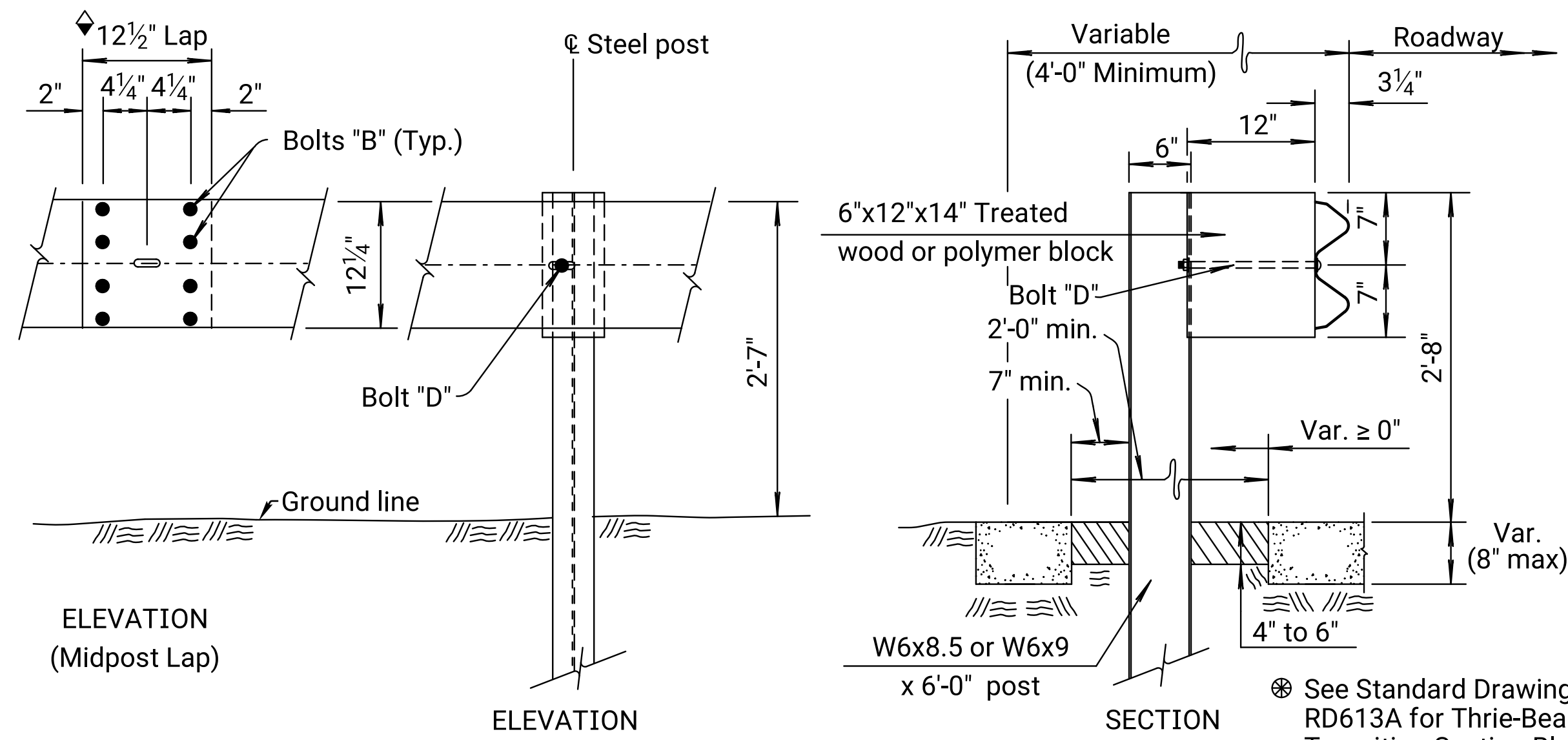
Plotted by : Brogan.Andrews@ks.gov  
File : KA572901rss611a-01.dgn  
16-APR-2024 17:31

◆ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

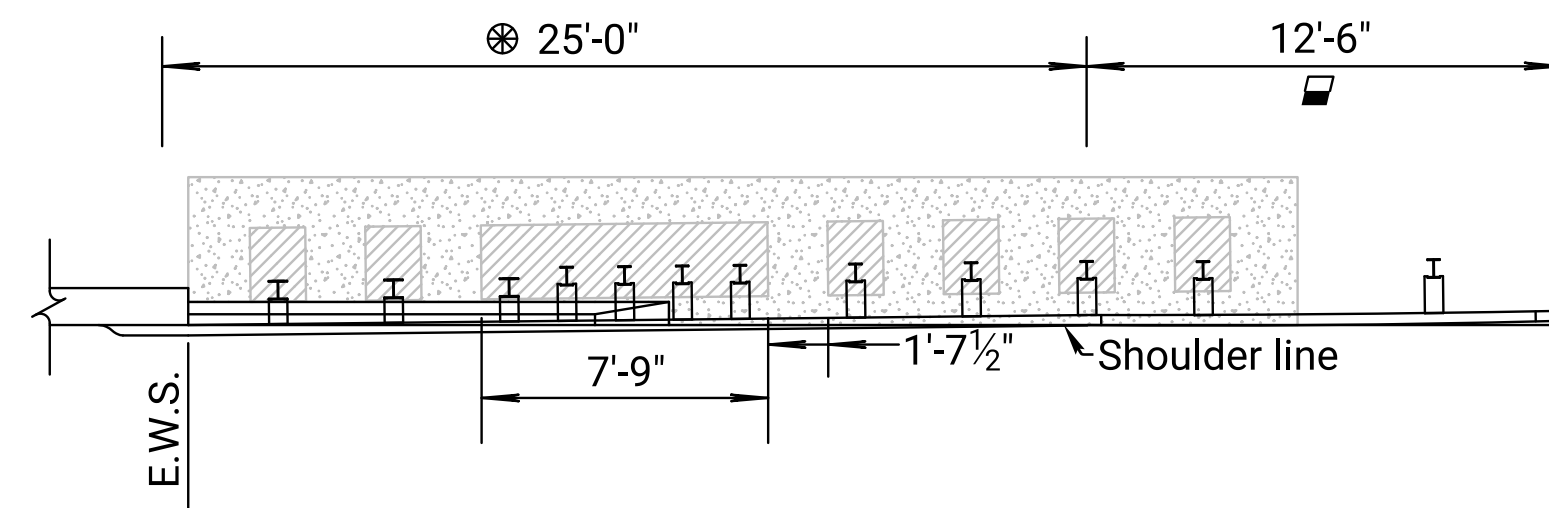
△ See RD613A for appropriate blackout size location.



THRIE BEAM POST DETAILS/POSTS IN PAVEMENT



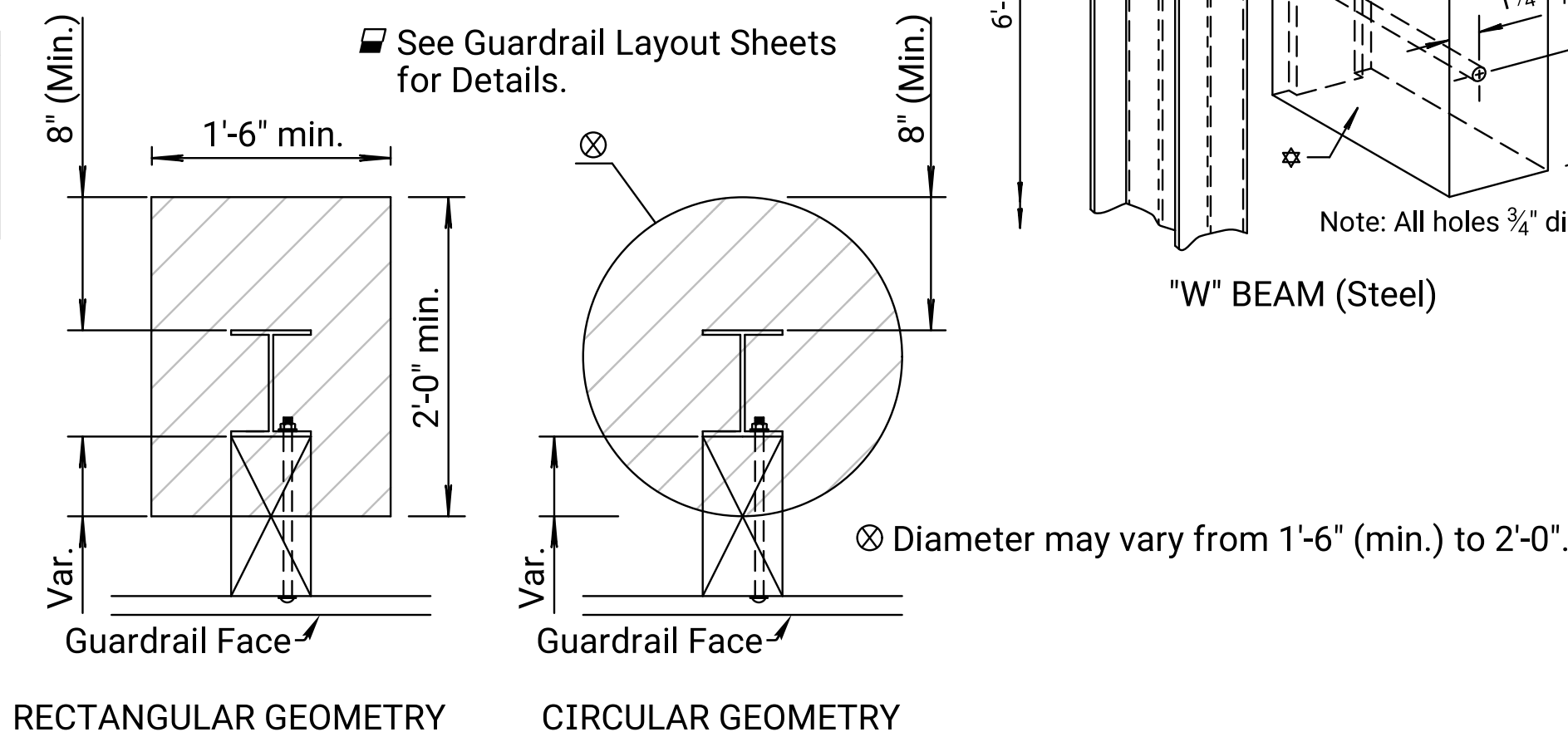
W-BEAM (MGS) POST DETAILS/POSTS IN PAVEMENT



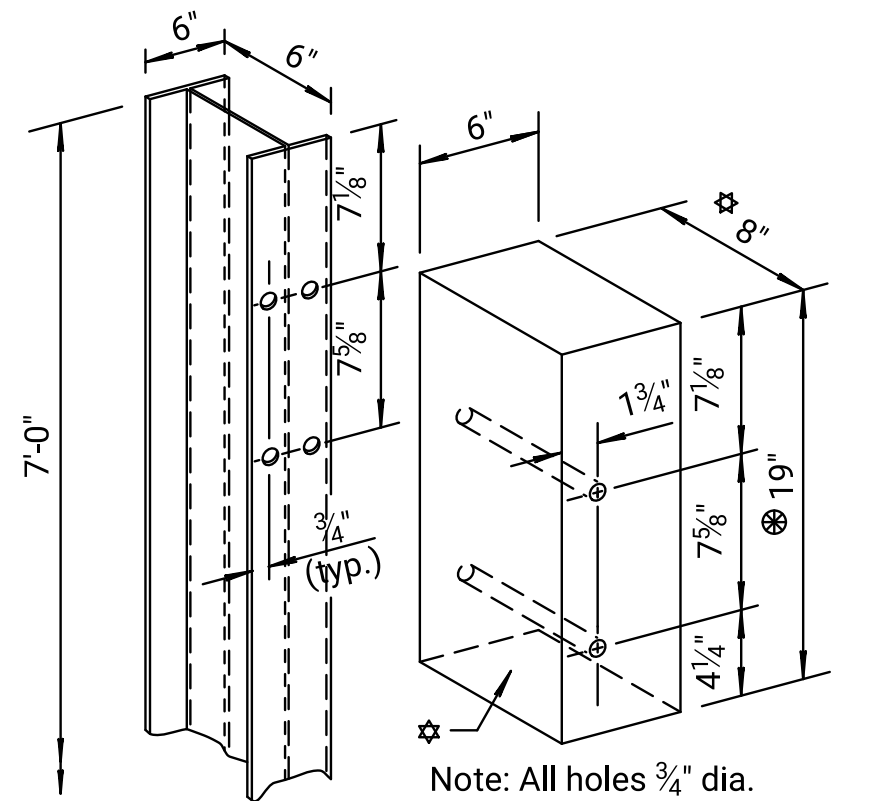
POSTS IN PAVEMENT  
(Not to Scale)

- Slurry Grout (Low Strength)  
See KDOT's Standard Specifications
- Pavement (Concrete or Asphalt)

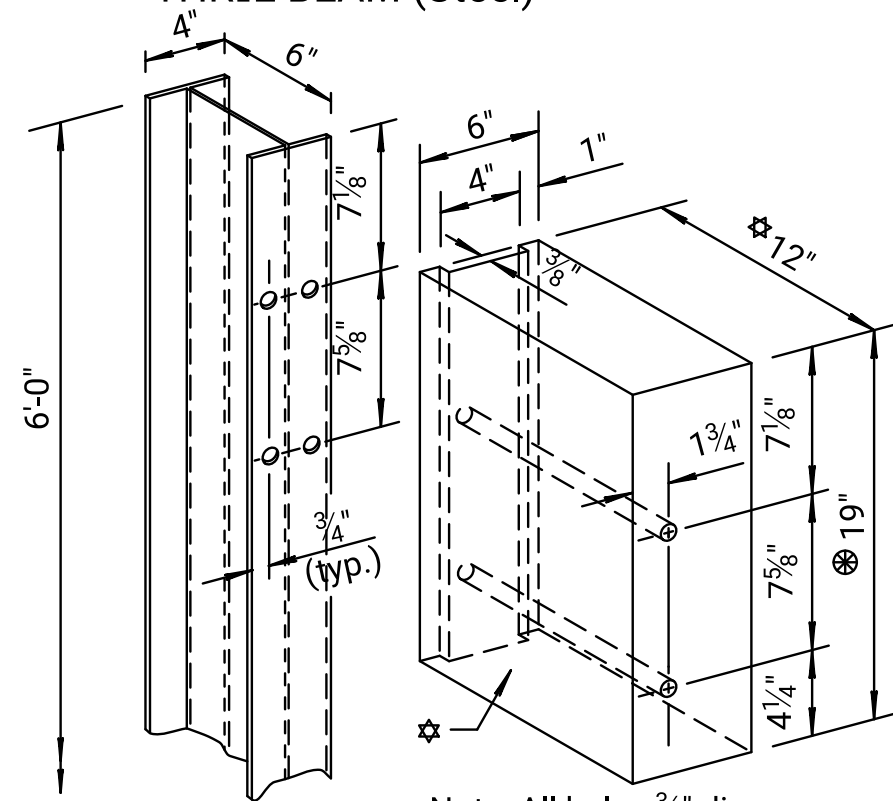
Note: Low Strength Grout must have a 28-day compressive strength of 120 psi or less. All work and materials related to posts in pavement are subsidiary to other guardrail bid items. Rectangular geometry shown in Posts in Pavement detail. Circular geometry, as shown on this sheet, may be used at the Contractor's option.



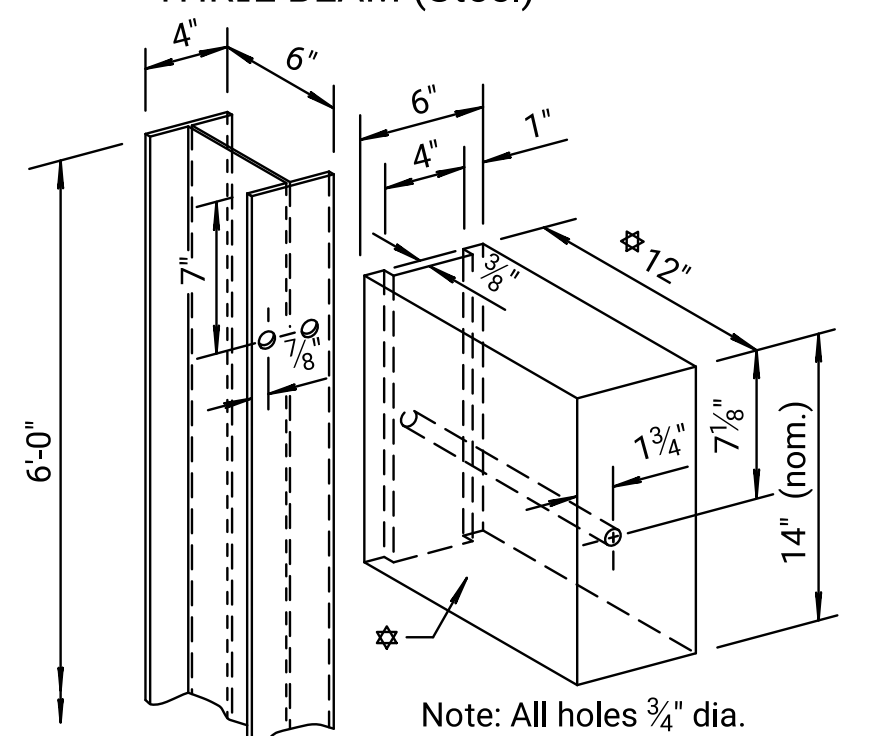
PLAN  
(ALTERNATE GEOMETRIES)



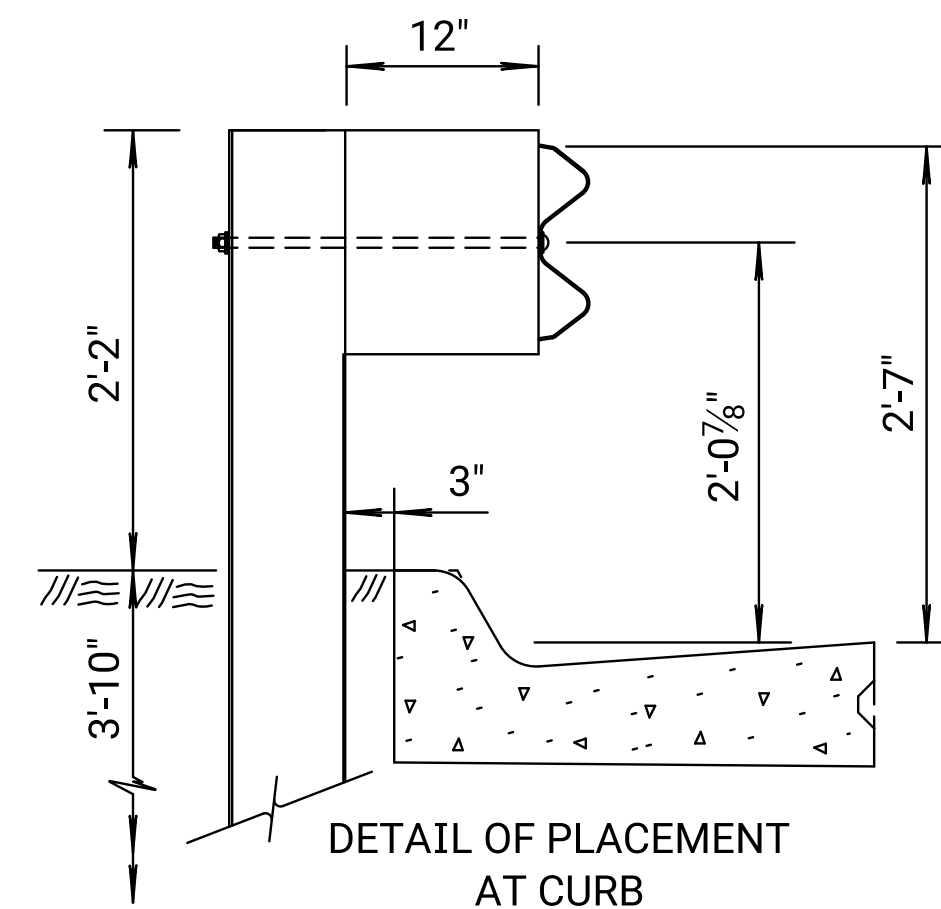
THRIE BEAM (Steel)



THRIE BEAM (Steel)



"W" BEAM (Steel)



Note: Measure height of rail from the pavement surface at the curb/pavement joint as shown. A special design is needed when guardrail is not located as detailed. A Type II (laydown) curb & gutter is preferred when guardrail is adjacent to curb.

GENERAL NOTES (Steel Posts)

Use grade of steel for steel posts that meet the requirements of the standard specifications.

Hot dip galvanize the posts after fabrication, see standard specifications.

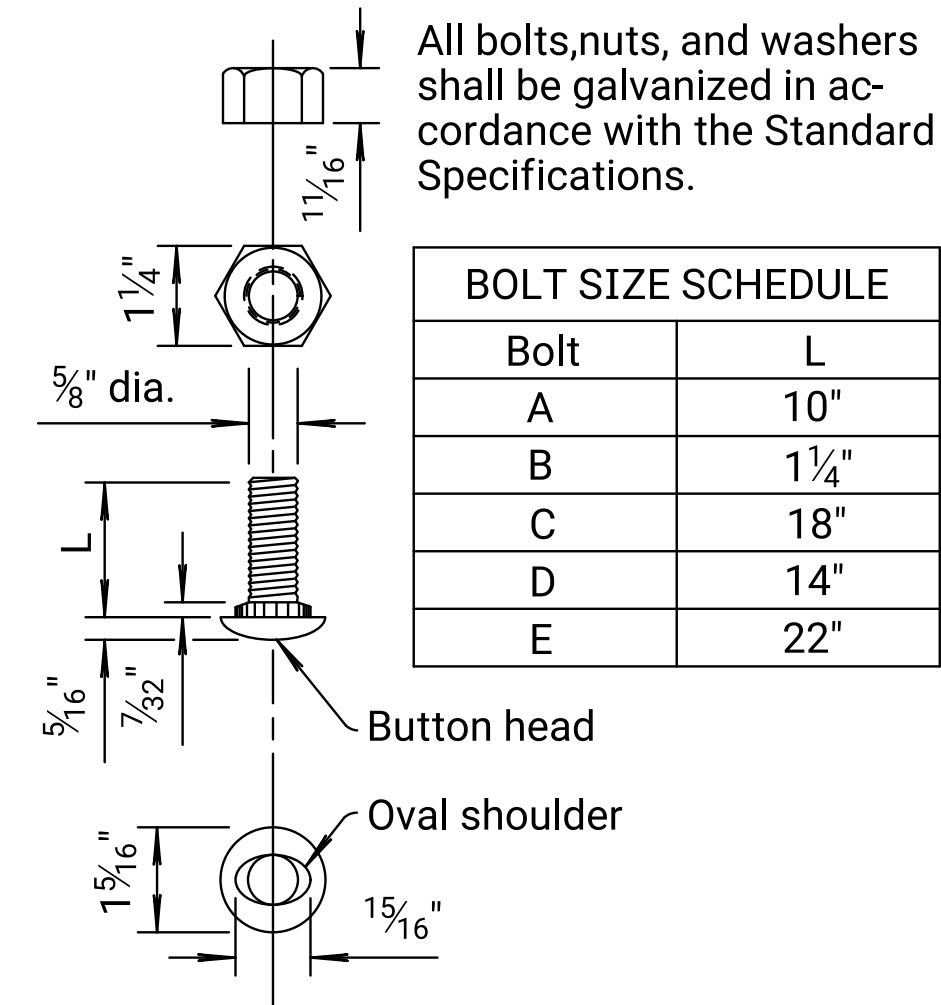
Wood blockouts may be used through the 25'-0" thrie-beam section with wood or polymer blockouts used throughout the remainder of the w-beam installation. The blackout size and material used in the guardrail end terminal may be independent from the remainder of the installation. For wood/polymer blackout requirements see standard specifications.

Use S4S rectangular blockouts for Thrie-Beam/W-Beam installation.

Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations.

Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered that prevents installation of a full length post.

All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made.



BOLT & NUT DETAILS

BOLT SIZE SCHEDULE	
Bolt	L
A	10"
B	1 1/4"
C	18"
D	14"
E	22"

05	09-24-15	Separated Steel/Wood Post Details	S.W.K.	S.W.K.
04	11-08-12	Revised Detail, Posts in Pavement	S.W.K.	J.O.B.
03	08-01-12	Revised Note to Designer	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
GUARDRAIL POST (STEEL) (MGS) DETAILS				
RD611A				
FHWA APPROVAL		01-29-16   APPD.		Scott W. King
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	









Note to Designer:  
KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCp, PPP, SRPEP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

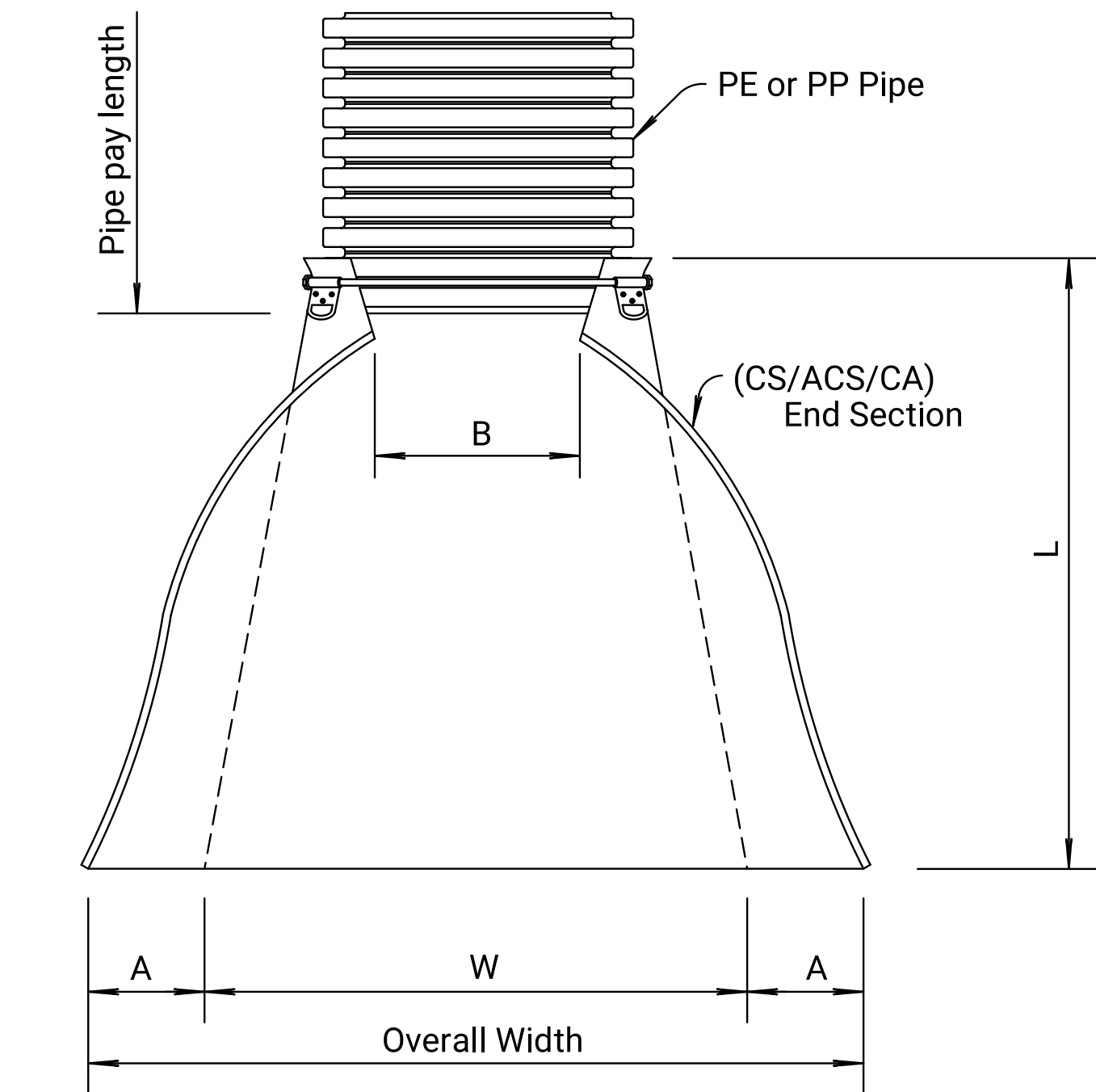
Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:33  
File : KA572901rss667-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	26	141

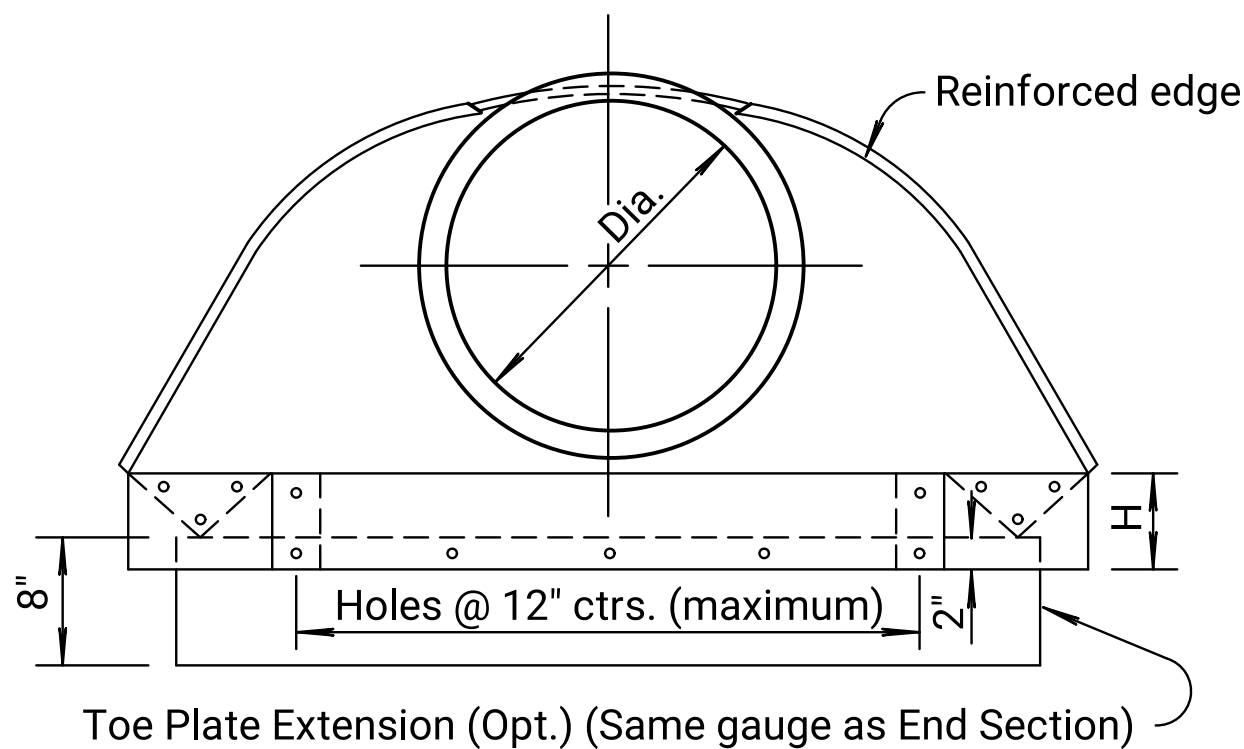
GENERAL NOTES

The culvert type shall meet the KDOT Pipe Policy & Standard Specifications.  
The size of pipe designated on the plan shall be the nominal inside diameter of a two wall corrugated PE pipe (Type S) or PP pipe (Type S).  
PE or PP pipe couplings shall be designed to cover at least two full corrugations on each side of a joint.  
No additional payment shall be made for any gain in length due to the fit of the pipe at connections.  
All corrugated PE or PP pipe, end sections, couplings, and fittings shall conform with the Standard Specifications.  
See Standard Specifications for PE or PP Pipe bedding and backfill.  
Multiple panel end sections shall have lap seams which are to be tightly joined by bolts & nuts. Corner plate and toe plate to be same gauge and material as end section. When required optional toe plate extension shall be overall width less 6" x 8" high.  
Attachment to PE or PP pipe 12" diameter and up shall be made with Type 2 Connector.  
All work and materials required for construction and installation of end section shall be included in the bid item "End Section".

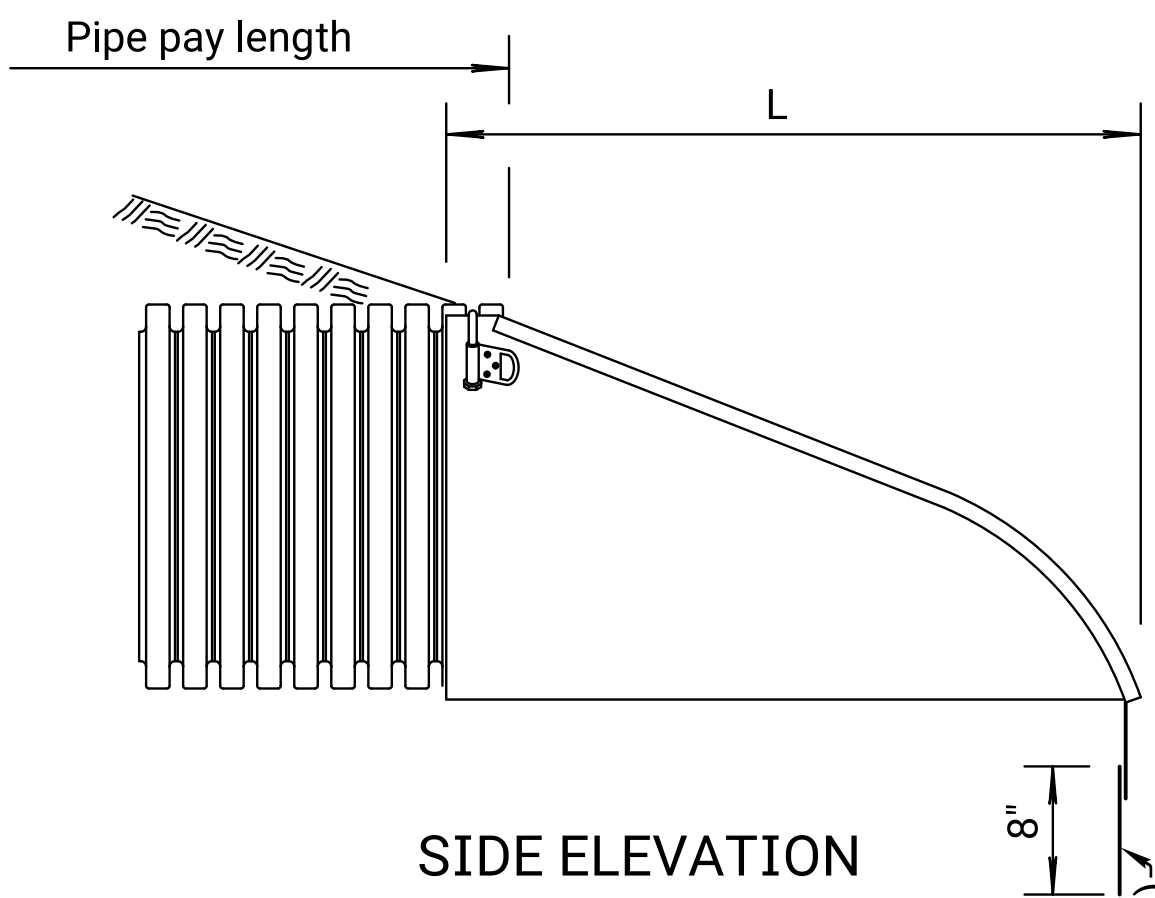
(CS/ACS/CA) END SECTION FOR PE or PP PIPE							
Pipe Dia. (In.)	Min. Gauge Ends	Dimensions in Inches					Slope
		A (min.)	B (max.)	H (min.)	L (+/-2")	W (min.)	
12"	16	6	7	6	21	24	2½:1
15"	16	7	8	6	26	30	2½:1
18"	16	8	10	6	31	36	2½:1
21"	16	9	12	6	36	42	2½:1
24"	16	10	13	6	41	48	2½:1
30"	14	12	16	8	51	60	2½:1
36"	14	14	19	9	60	72	2½:1
42"	12	16	25	11	69	84	2½:1
48"	12	18	29	12	78	90	2¼:1
54"	12	18	33	12	84	102	2¼:1
60"	12/10	18	36	12	87	114	2:1



PLAN  
(Illustrated with Type 2 Connector)

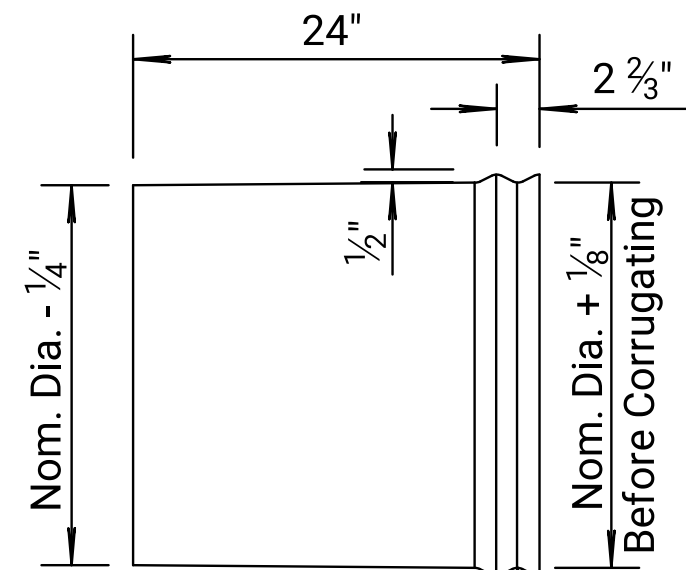
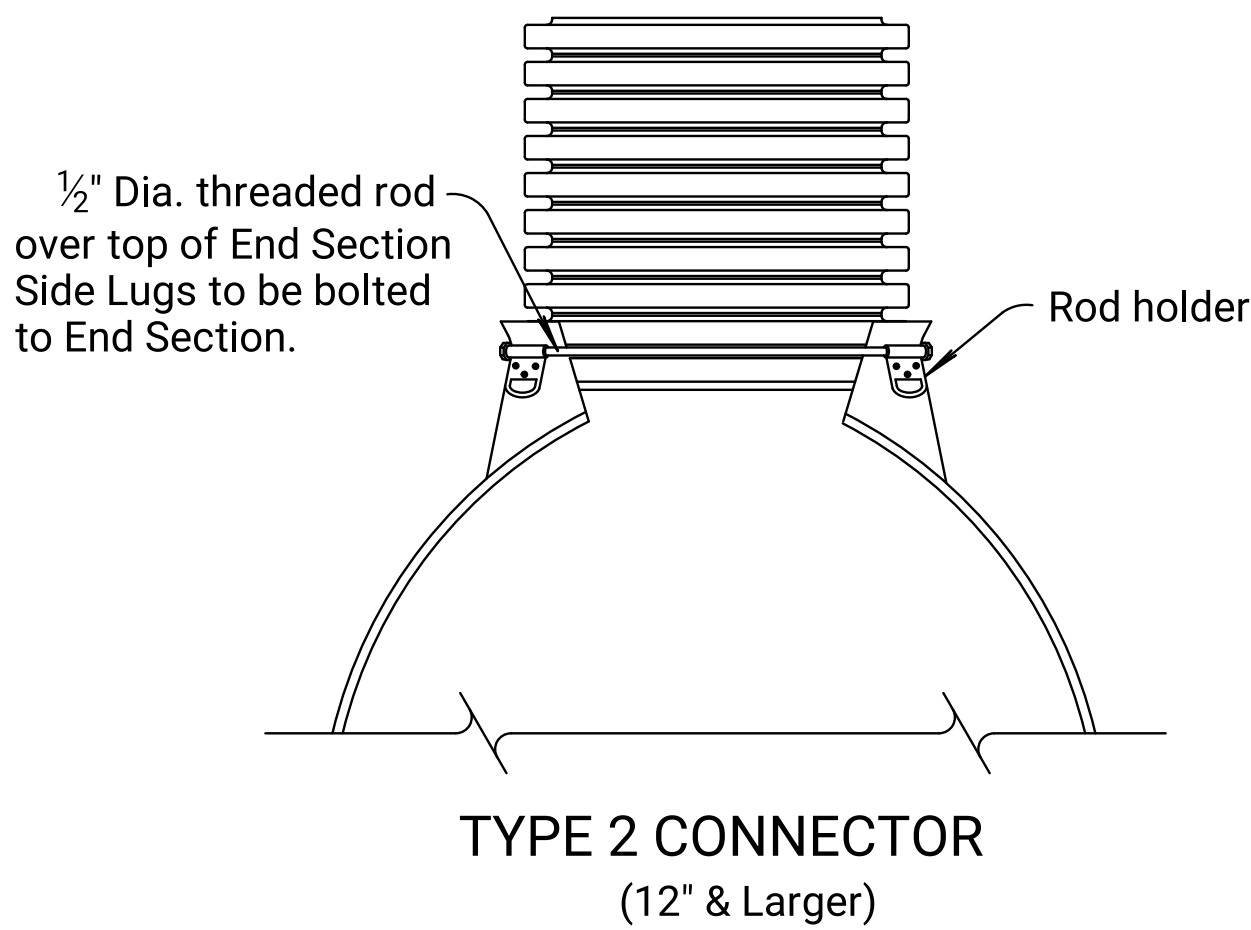


FRONT ELEVATION



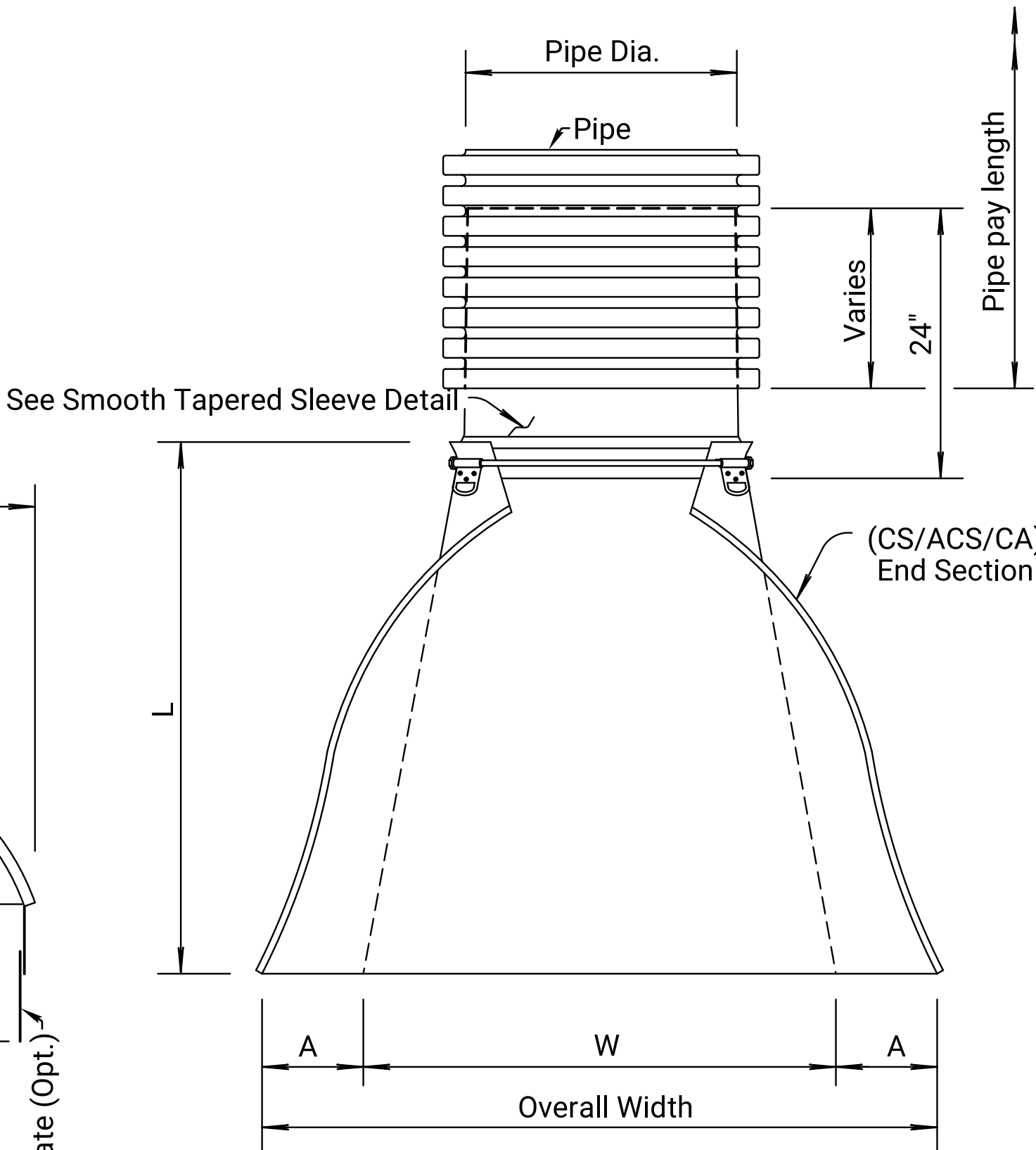
SIDE ELEVATION

⊗ Toe plate extensions where specified, to be punched to match holes in apron lip and bolted with 3/8" dia. bolts. The length of toe plate to be as follows:  
W + 10" for 12" to 30" diameter pipe inclusive.  
W + 20" for 36" to 60" diameter pipe inclusive.

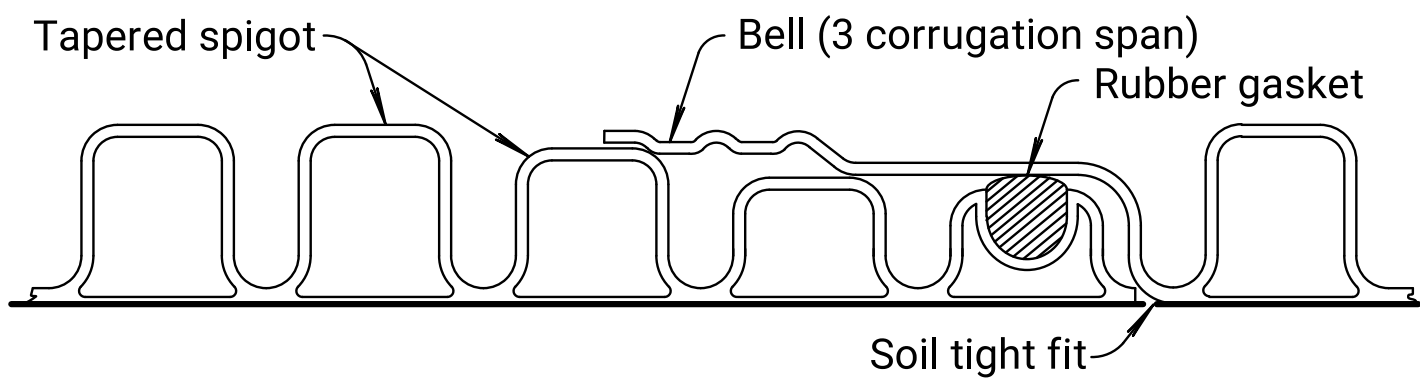


Form 1/2" x 2 2/3" Corrugations in 12 ga. smooth CS (Galvanized) or ACS (Aluminized) metal, maintain inside diameter of sleeve.

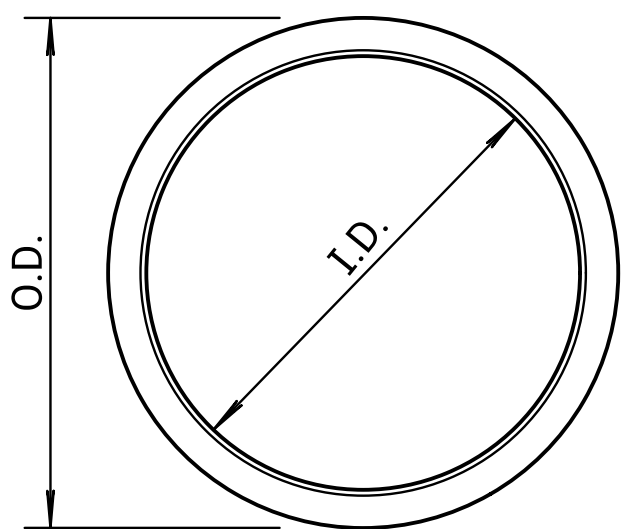
SMOOTH TAPERED SLEEVE DETAIL  
(Required)



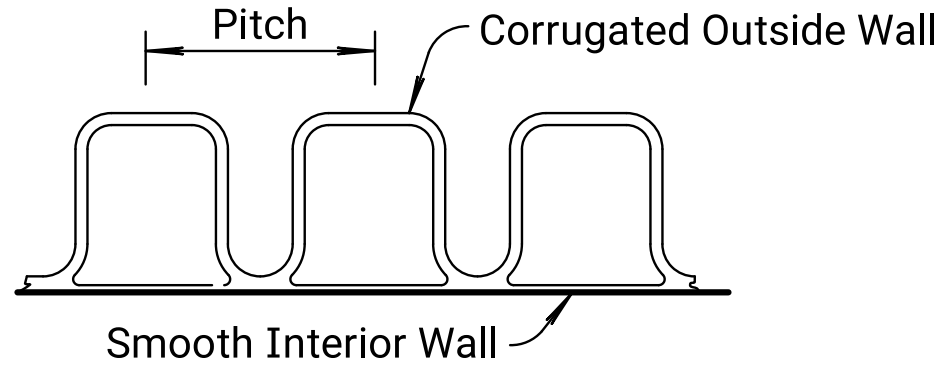
PLAN  
TAPERED SLEEVE FOR ATTACHING  
METAL END SECTION TO PE OR PP PIPE  
(Illustrated with Type 2 Connector)



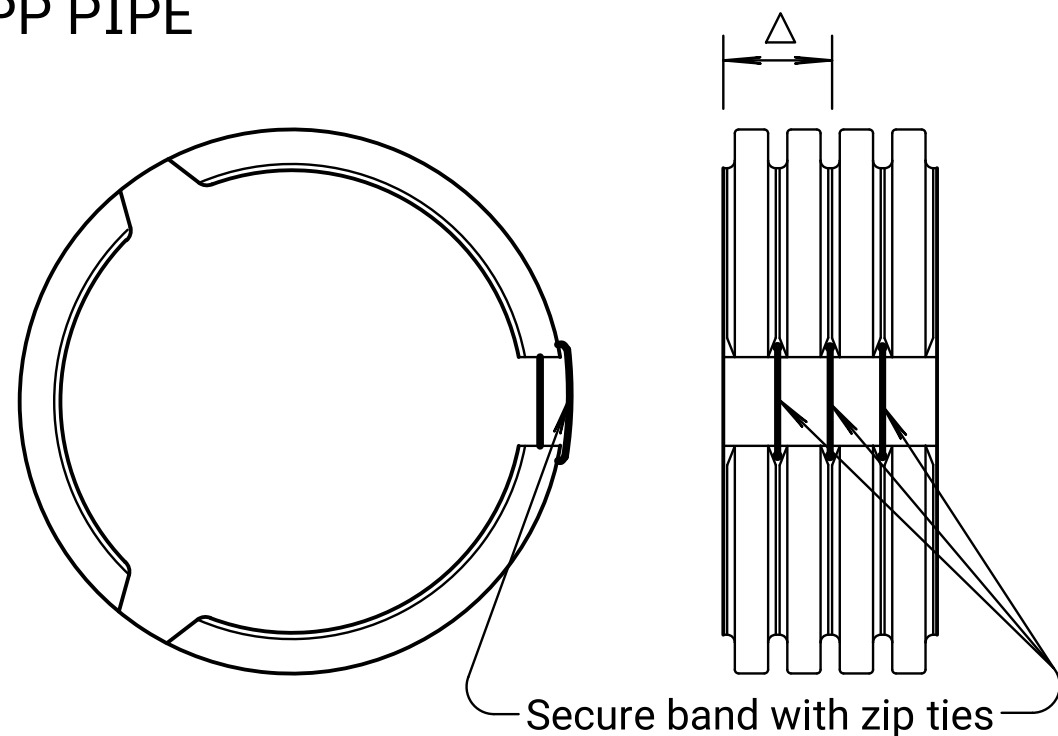
PE or PP PIPE BELL & SPIGOT CONNECTION  
SOIL TIGHT JOINT



DETAILS OF CORRUGATED  
PE or PP PIPE



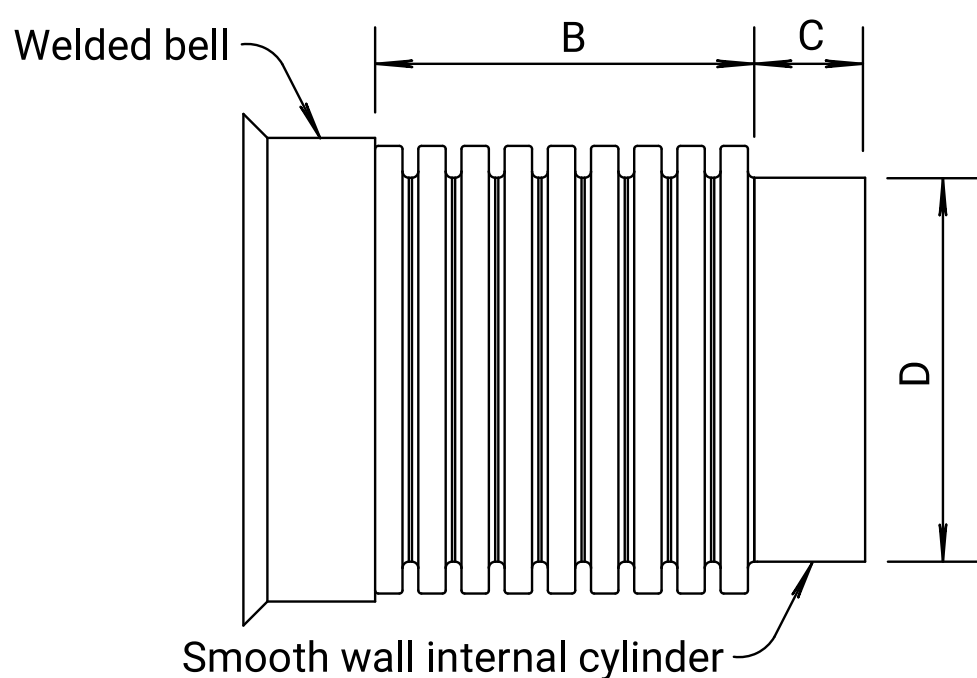
TYPICAL SECTION  
CORRUGATED PE or PP PIPE



Δ Two Full Corrugations (Minimum Overlap)

PE or PP PIPE SPLIT BAND COUPLER  
SOIL TIGHT JOINT

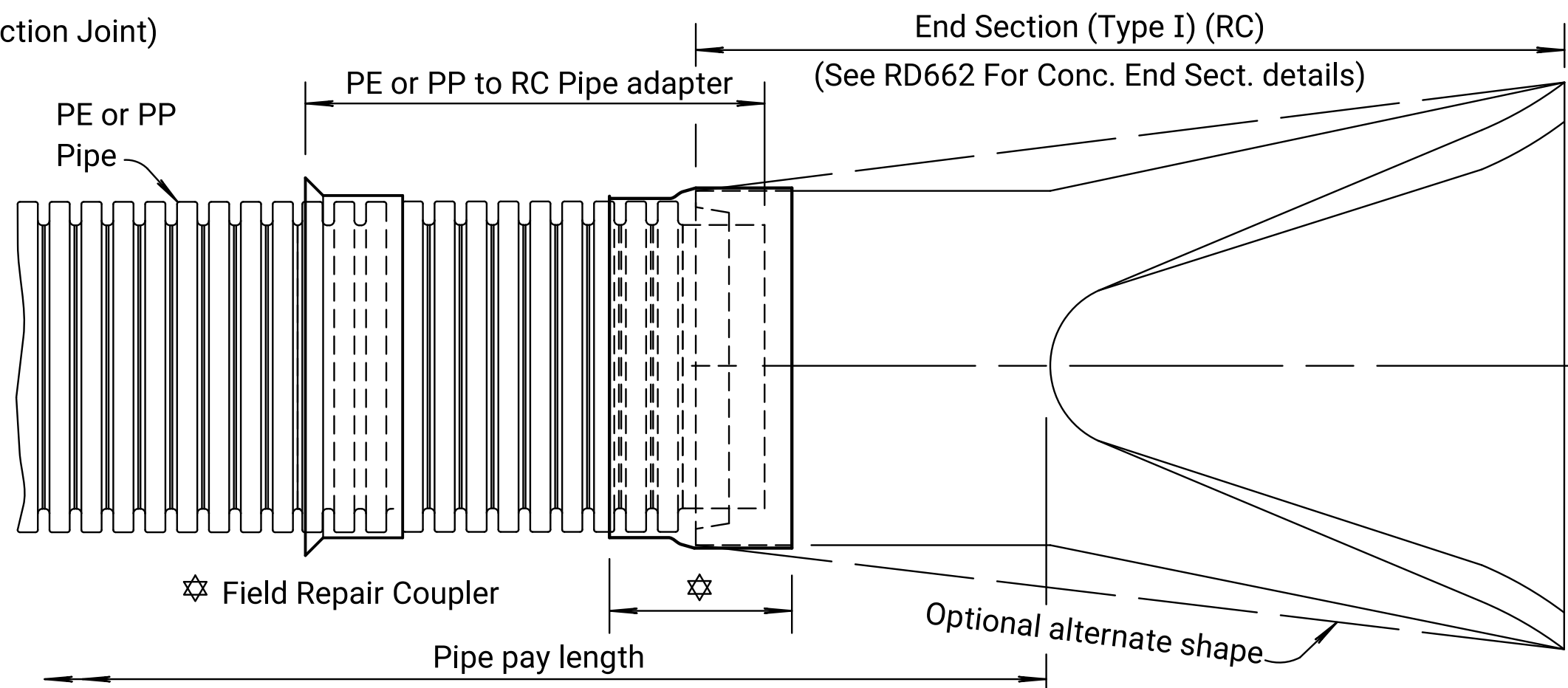
This band is used for (Field Splice Construction Joint)



PE or PP to RC PIPE ADAPTER

PE or PP TO RC PIPE ADAPTER

Pipe Dia. (In.)	B	C	D
18"	18 1/4"	6"	18"
24"	25"	6"	24"
30"	32 13/16"	6"	30"
36"	36 3/4"	6"	36"
42"	36"	6"	41 1/4"
48"	36 3/4"	6"	41 1/4"
60"	36"	6"	59"



PE or PP to RC PIPE ADAPTER to CONCRETE END SECTION  
(This installation is for Acidic Soil Conditions)

03	5-9-22	Added Polypropylene pipe (PP) type	A.L.R.	S.W.K.
02	07-17-17	Changed tapered slv. requirement	A.L.R.	S.W.K.
01	02-08-08	Added ref. to KDOT pipe policy	S.W.K.	J.O.B.

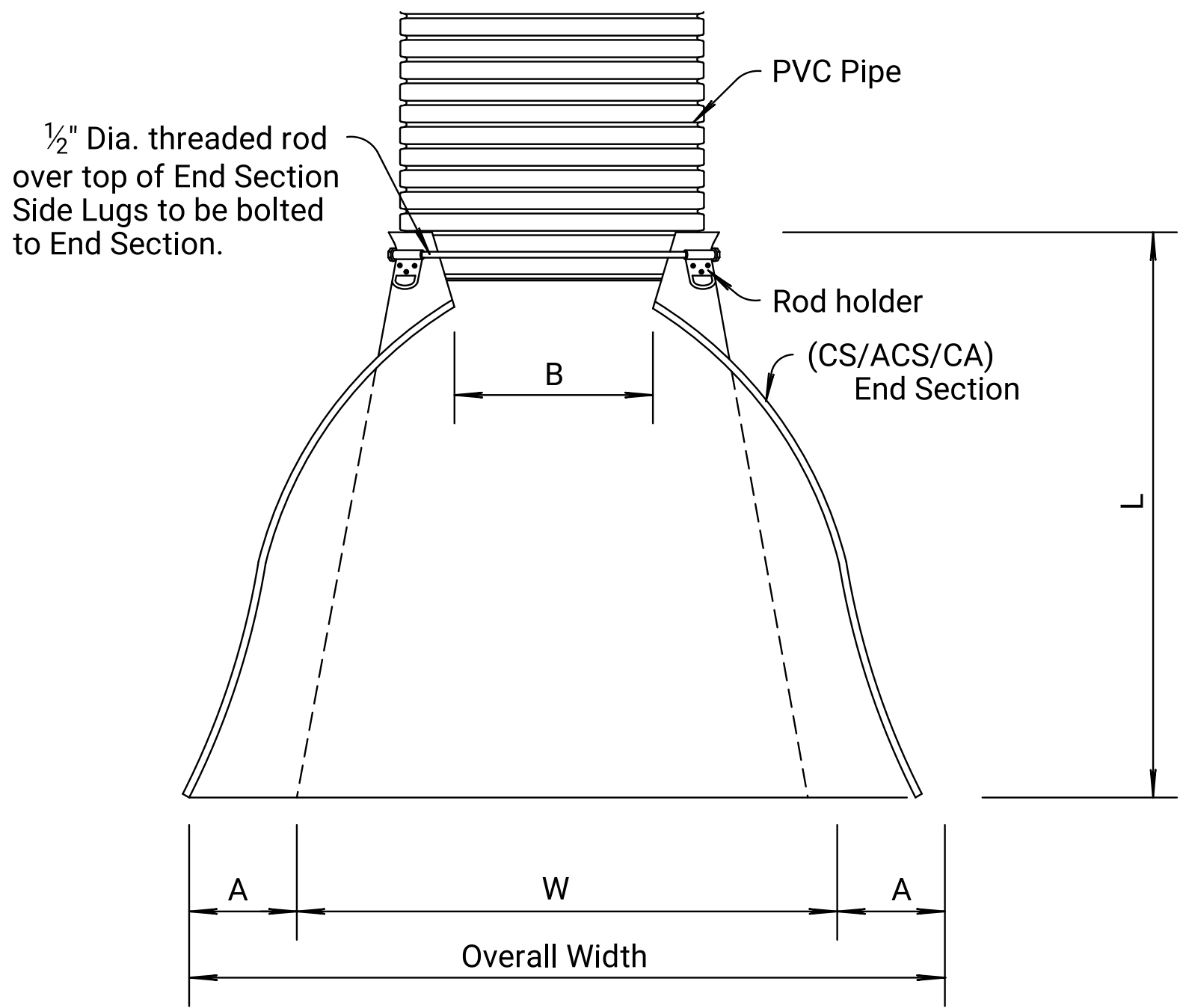
NO.	DATE	REVISIONS	BY	APPD
-----	------	-----------	----	------

KANSAS DEPARTMENT OF TRANSPORTATION				
METAL/CONCRETE END SECTION (TYPE I) for PE or PP PIPE				
RD667				
FHWA APPROVAL		06-08-22	APPD.	Scott W. King
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

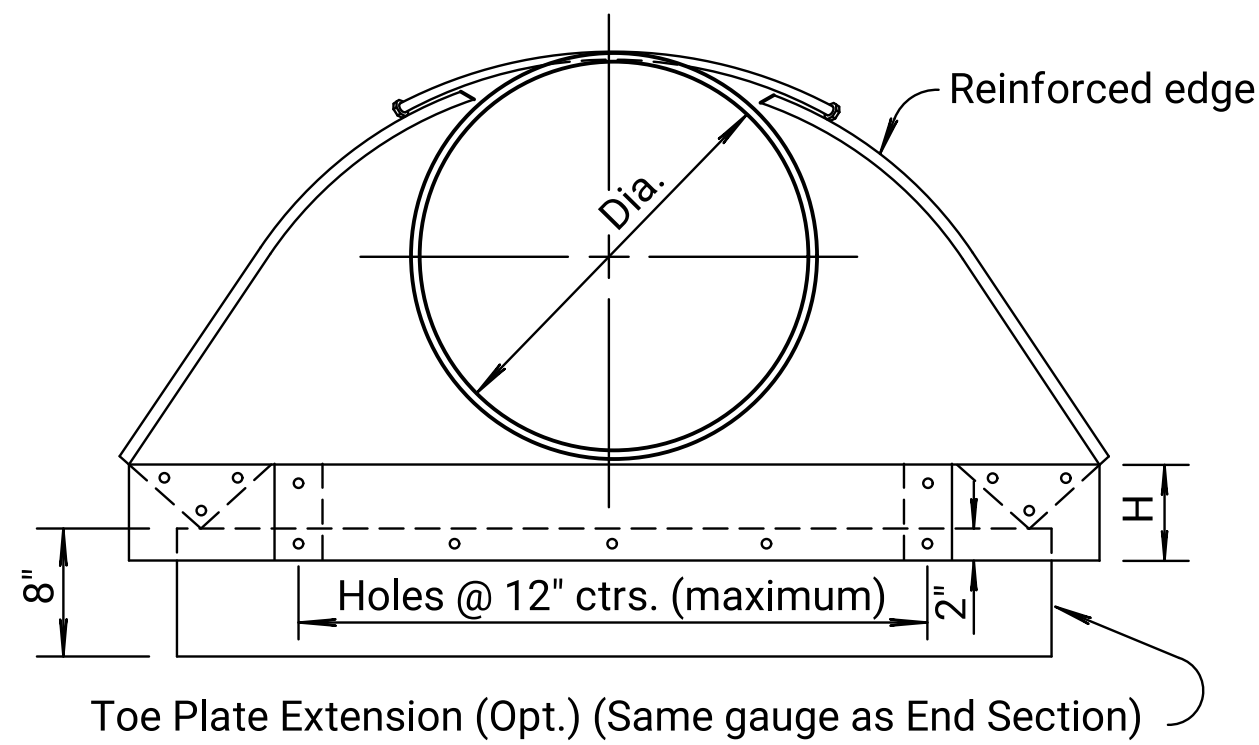
Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:38

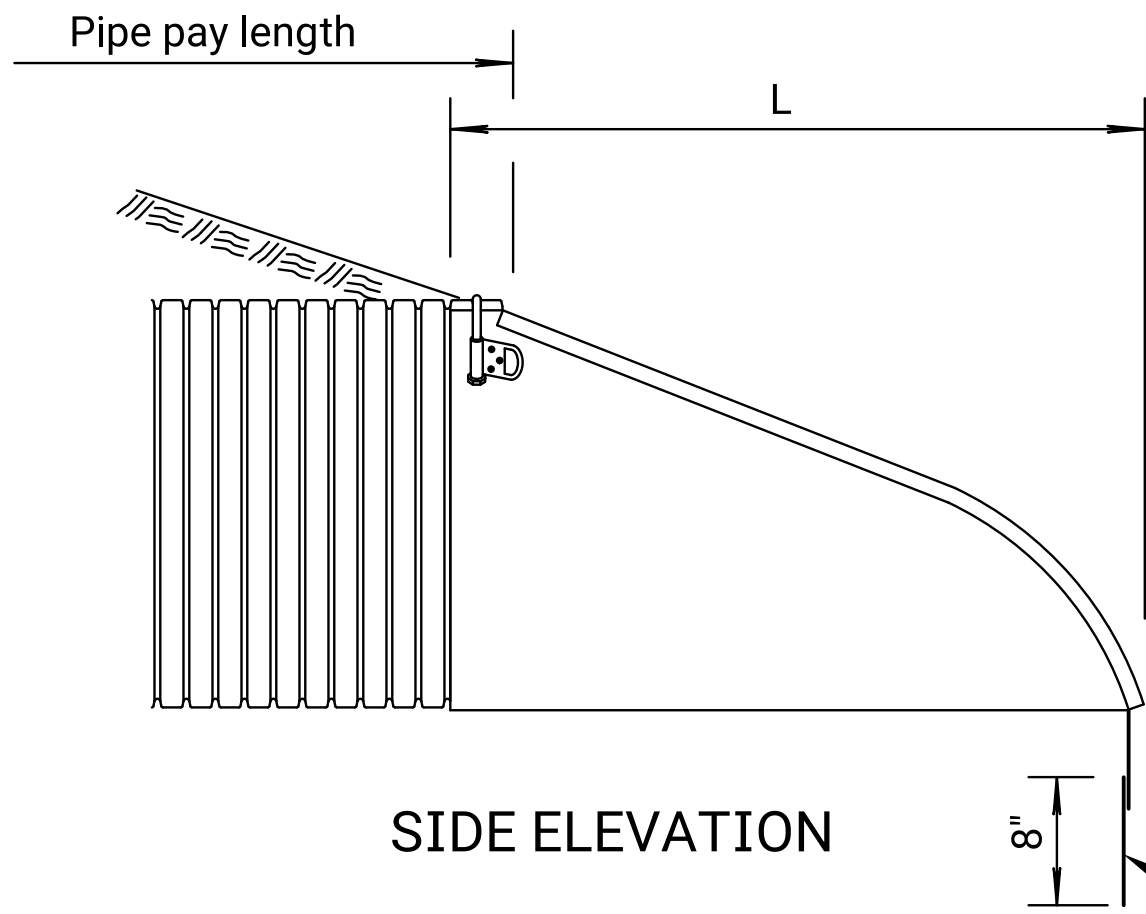
File : KA572901rss667b-01.dgn



PLAN  
(Illustrated with Type 2 Connector on 12" or larger)

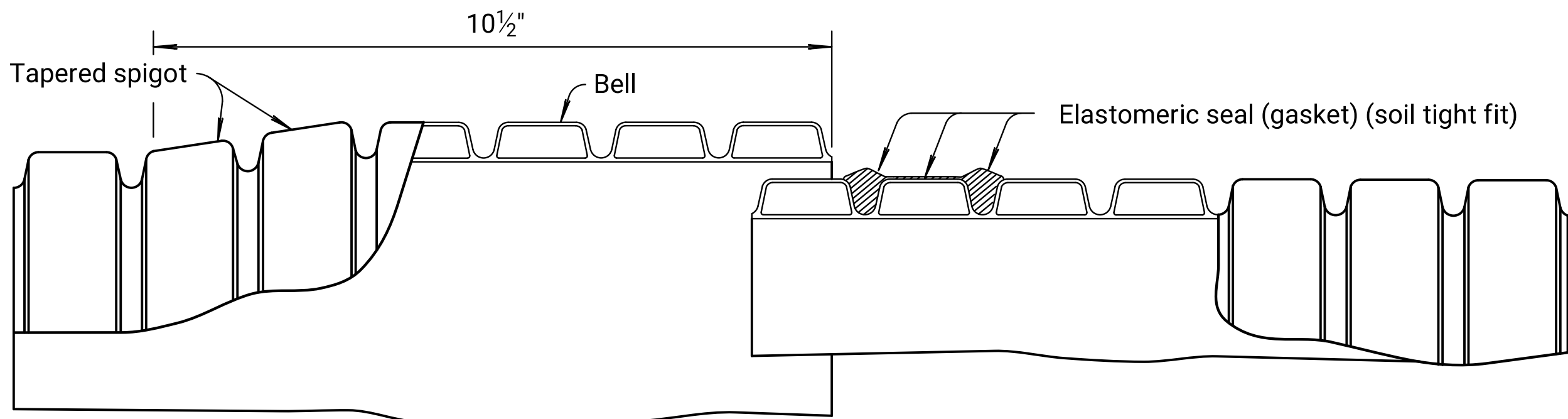
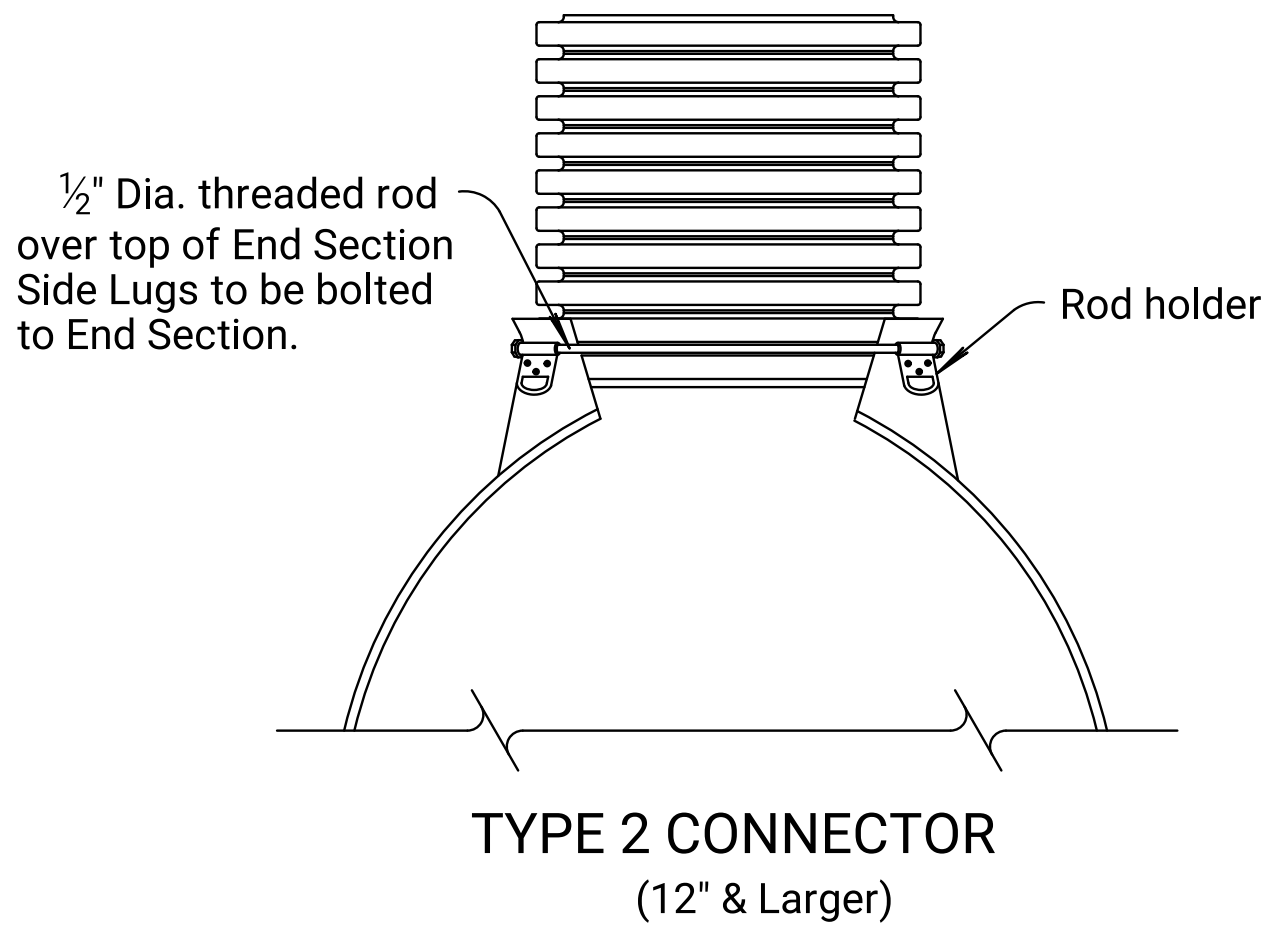


FRONT ELEVATION

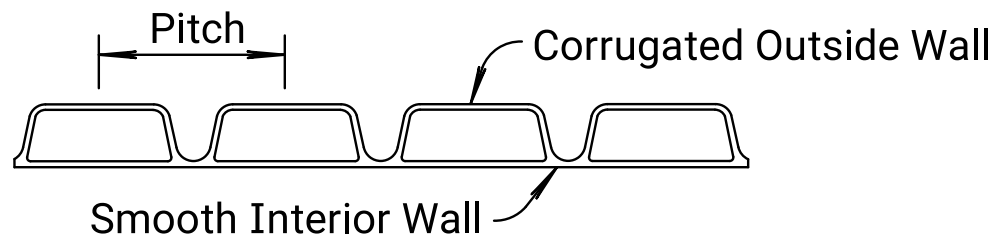


SIDE ELEVATION

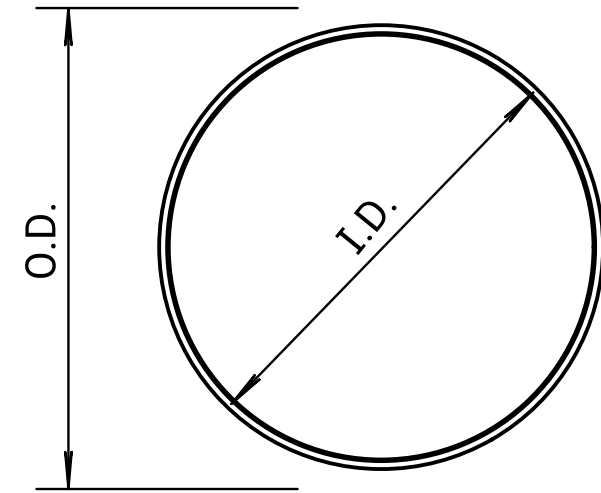
⊗ Toe plate extensions where specified, to be punched to match holes in apron lip and bolted with 3/8" dia. bolts. The length of toe plate to be as follows:  
W + 10" for 12" to 30" diameter pipe inclusive.  
W + 20" for 36" to 60" diameter pipe inclusive.



PVC BELL & SPIGOT CONNECTION  
SOIL TIGHT JOINT



TYPICAL SECTION  
CORRUGATED PVC PIPE



DETAILS OF  
CORRUGATED PVC PIPE

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	27	141

GENERAL NOTES

The culvert type shall meet the KDOT Pipe Policy & Standard Specifications. The size of pipe designated on the plan shall be the nominal inside diameter of a two wall corrugated PVC pipe.

PVC pipe shall be joined with an integral bell gasket joint and Flexible Elastomeric Seals. PVC Pipe is available in lengths of 2.5' to 20'. PVC pipe can be field cut to length, cut through a corrugation valley using a hand or power saw. Gaskets are shipped loose and fitted on spigot/cut pipe end following Manufacturer's installation instructions.

No additional payment shall be made for any gain in length due to the fit of the pipe at connections.

All corrugated PVC pipe, end sections and fittings shall conform with the Standard Specifications.

See Standard Specifications for PVC Pipe bedding and backfill.

Multiple panel end sections shall have lap seams which are to be tightly joined by bolts & nuts. Corner plate and toe plate to be same gauge and material as end section. When required optional toe plate extension shall be overall width less 6" x 8" high.

The End Section attachment to PVC pipe shall be made with a Type 2 Connector for 12" or greater pipe size.

All work and materials required for construction and installation of end section shall be included in the bid item "End Section".

(CS/ACS/CA) END SECTION FOR PVC PIPE								
Pipe Dia. (In.)	Min. Gauge Ends	Dimensions in Inches					Slope	
		A 1" Tol.	B (max.)	H (min.)	L (+/- 2")	W (min.)		
12"	16	6	7	6	21	24	2 1/2: 1	
15"	16	7	8	6	26	30	2 1/2: 1	
18"	16	8	10	6	31	36	2 1/2: 1	
21"	16	9	12	6	36	42	2 1/2: 1	
24"	16	10	13	6	41	48	2 1/2: 1	
30"	14	12	16	8	51	60	2 1/2: 1	
36"	14	14	19	9	60	72	2 1/2: 1	

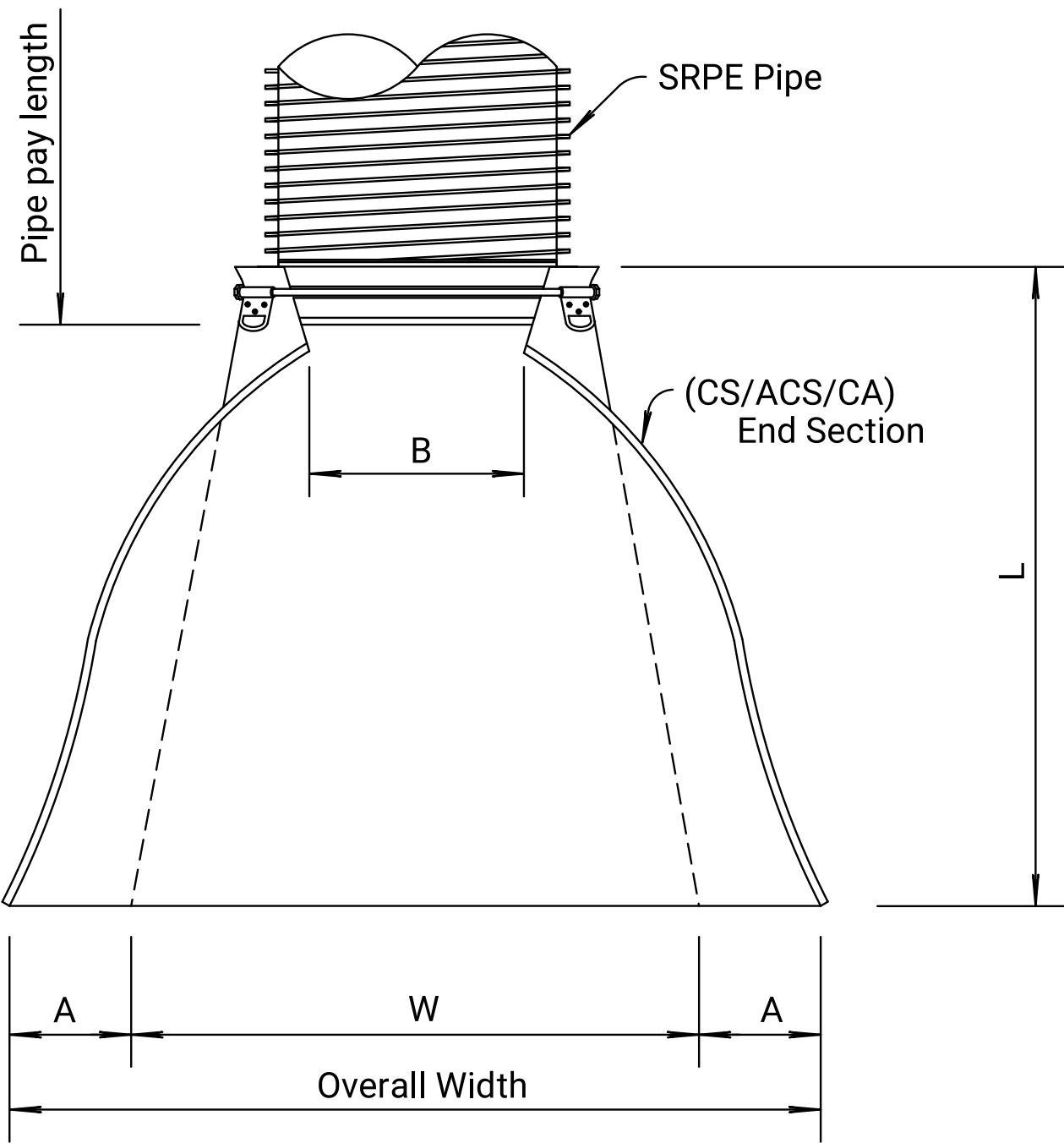
01	02-08-08	Added ref. to KDOT Pipe Policy	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
METAL END SECTION (TYPE I) for PVC PIPE				
RD667B				
FHWA APPROVAL		06-27-08	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	



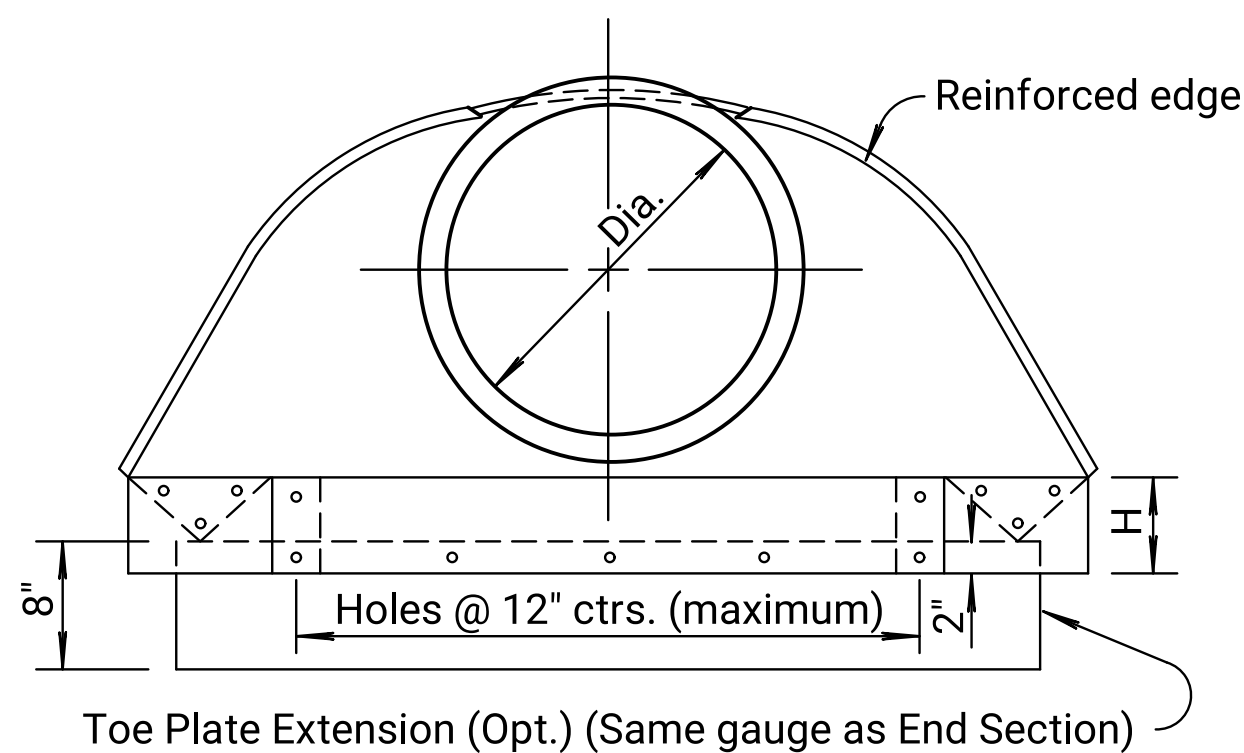
Note to Designer:

KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, PPP, SRPEP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

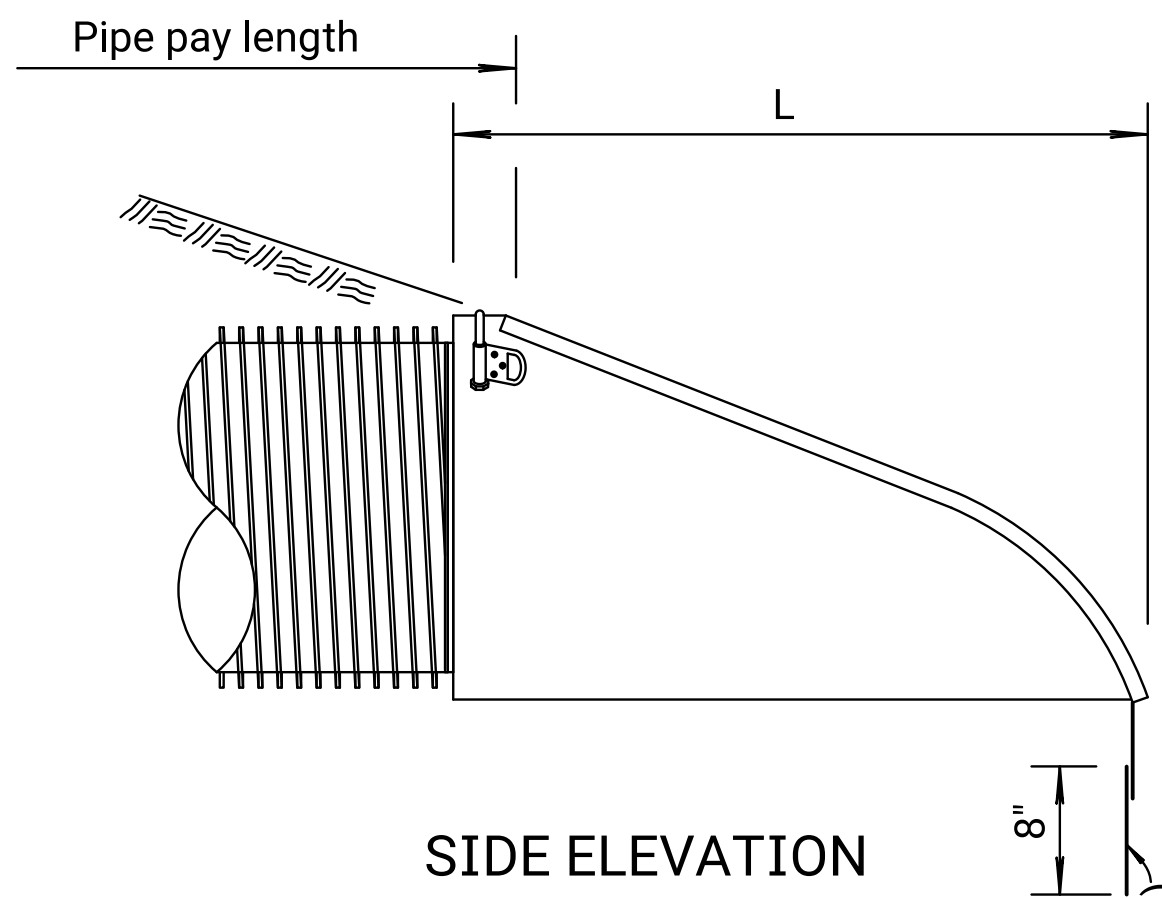
Plotted by : Brogan.Andrews@ks.gov  
File : KA572901rss667d-01.dgn  
16-APR-2024 17:35



PLAN  
(Illustrated with Type 2 Connector)

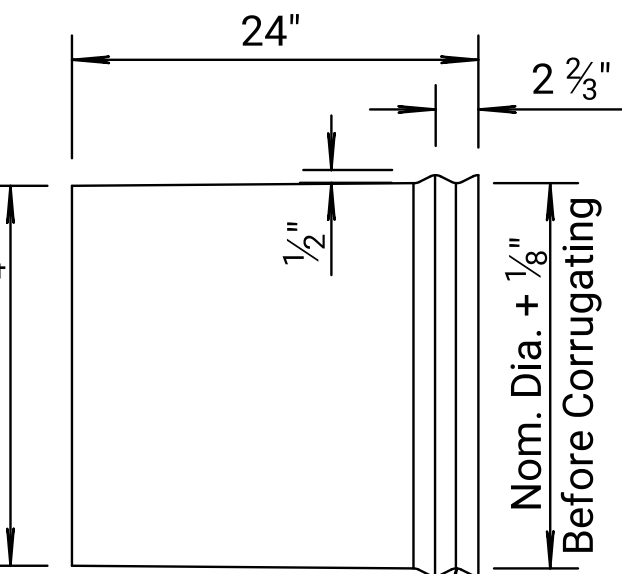
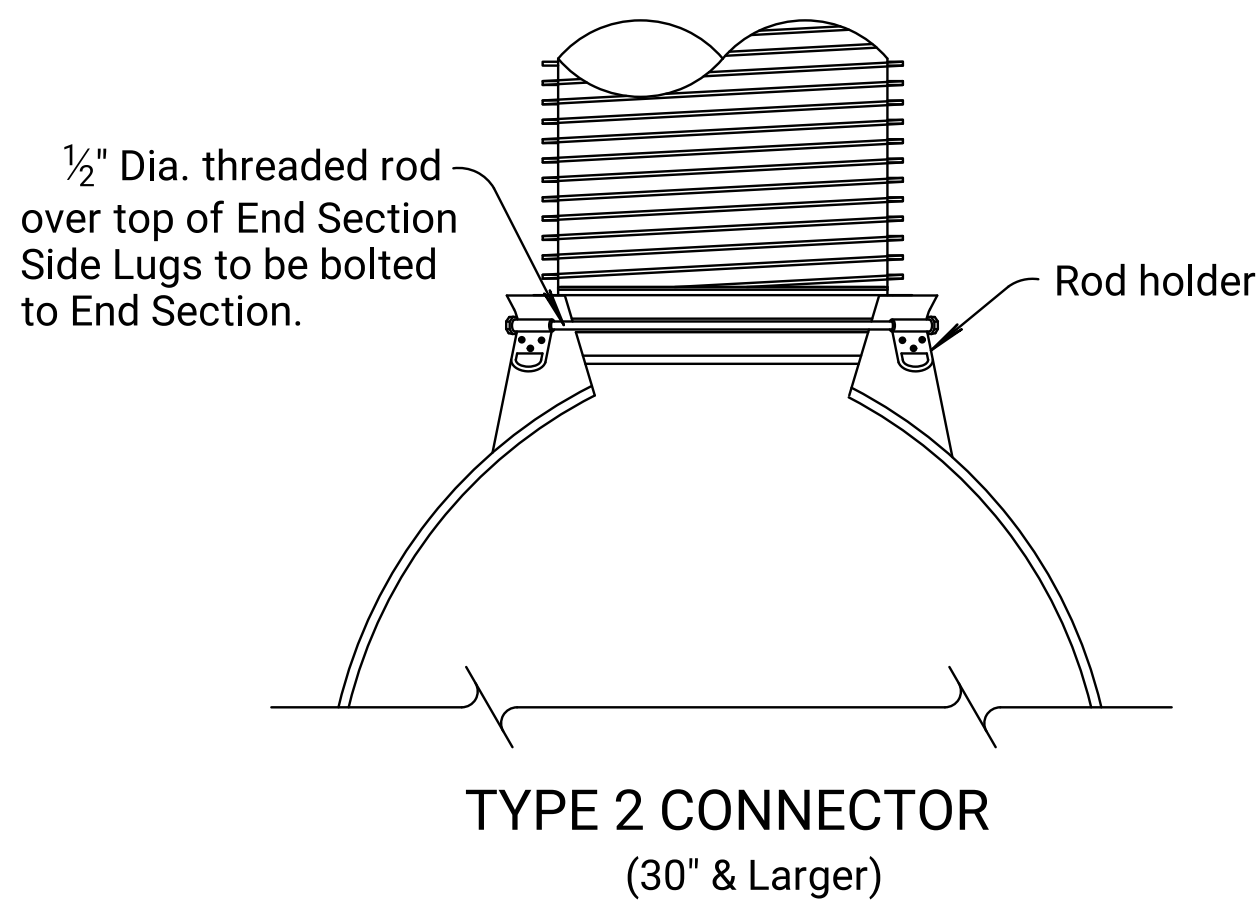


FRONT ELEVATION



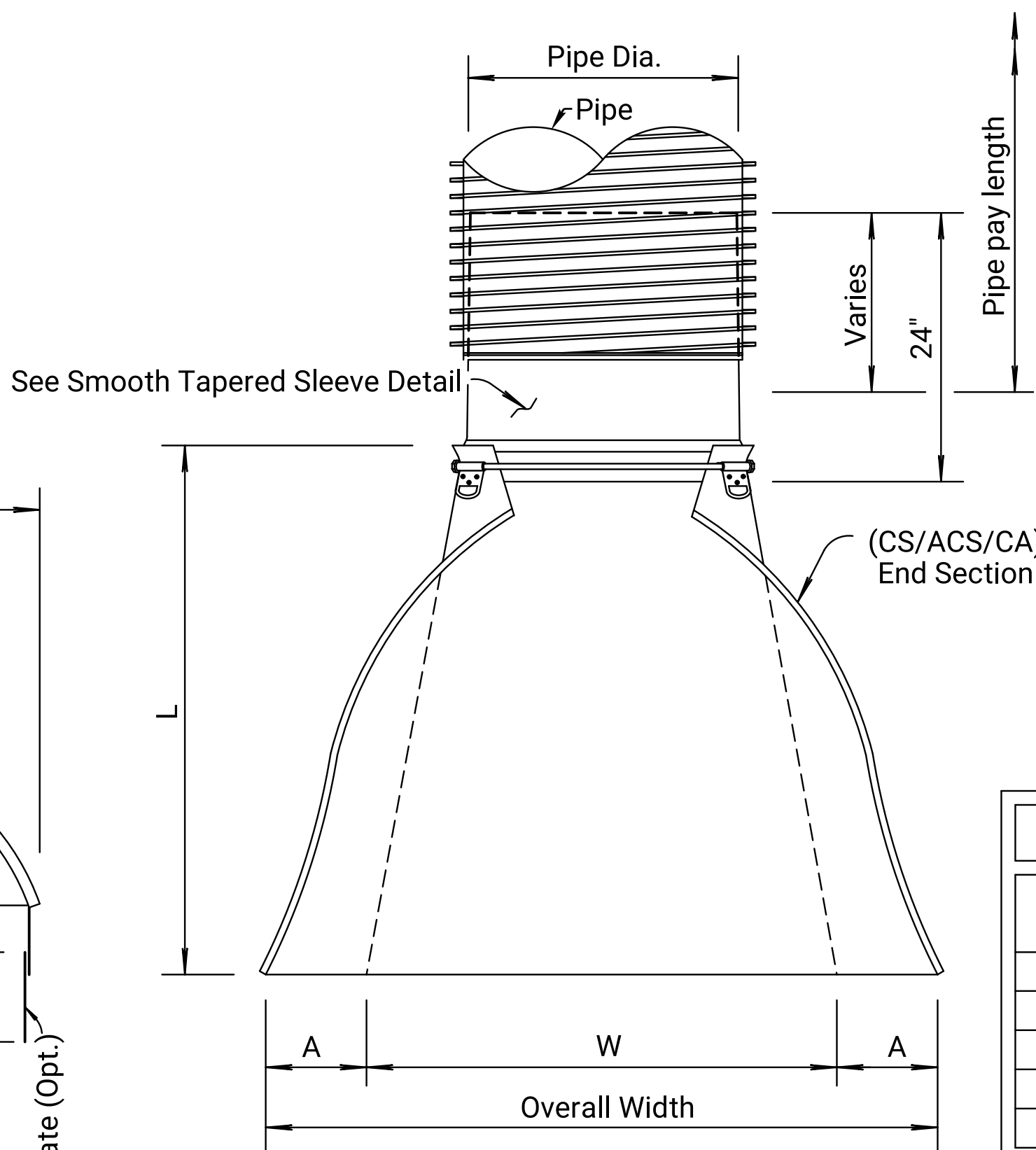
SIDE ELEVATION

⊗ Toe plate extensions where specified, to be punched to match holes in apron lip and bolted with 3/8" dia. bolts. The length of toe plate to be as follows:  
W + 10" for 12" to 30" diameter pipe inclusive.  
W + 20" for 36" to 60" diameter pipe inclusive.

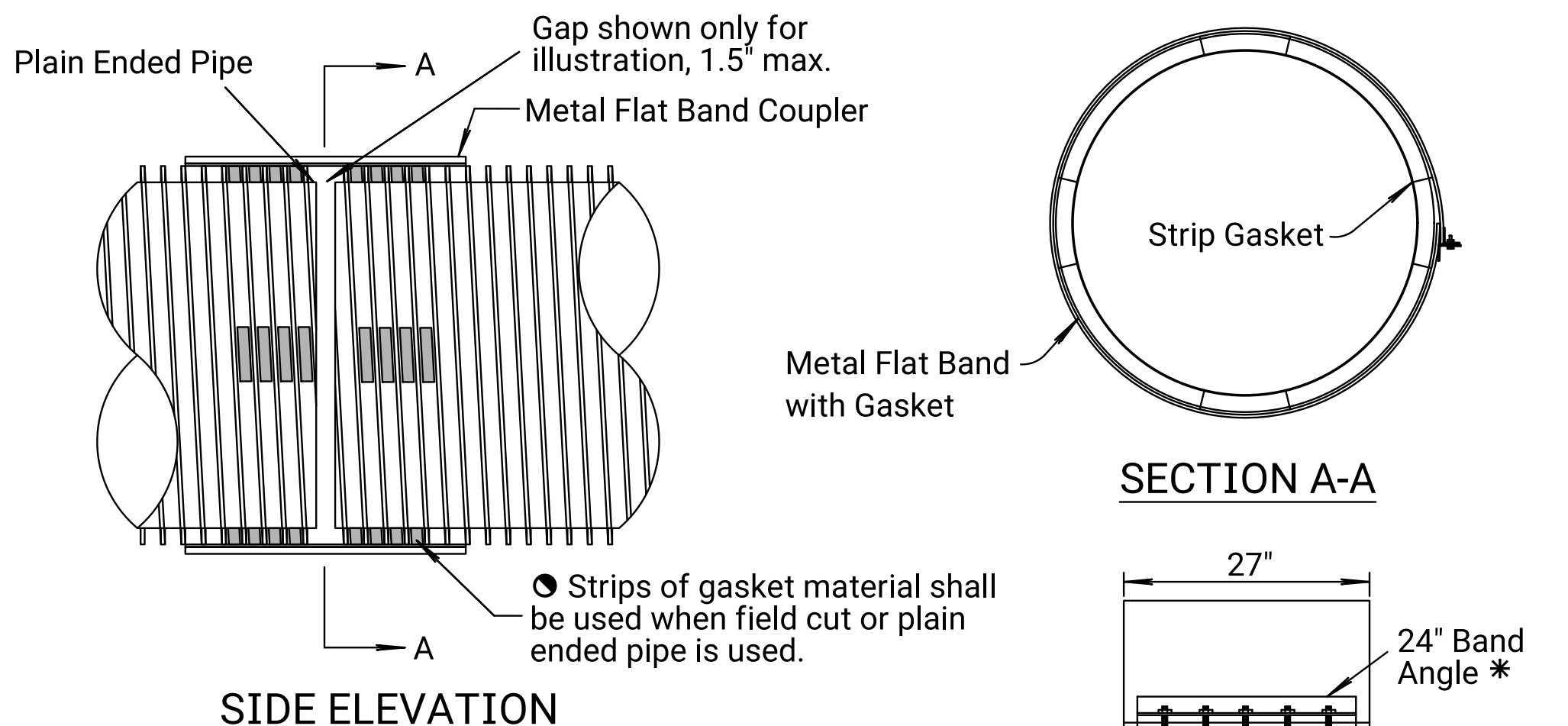


Form 1/2" x 2 2/3" Corrugations in 12 ga. smooth CS (Galvanized) or ACS (Aluminized) metal, maintain inside diameter of sleeve.

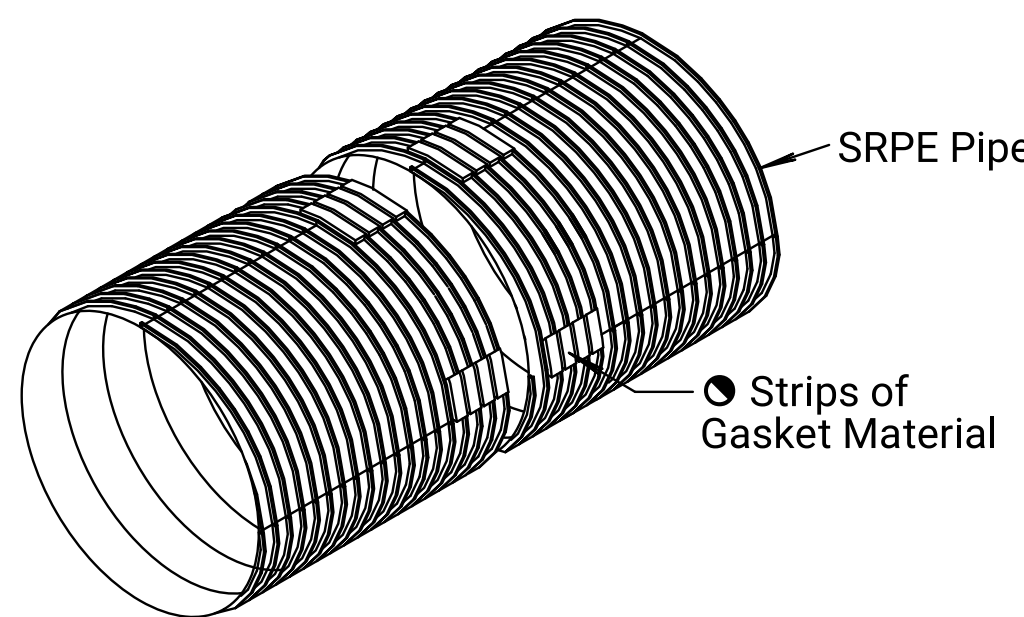
SMOOTH TAPERED SLEEVE DETAIL  
(Required)



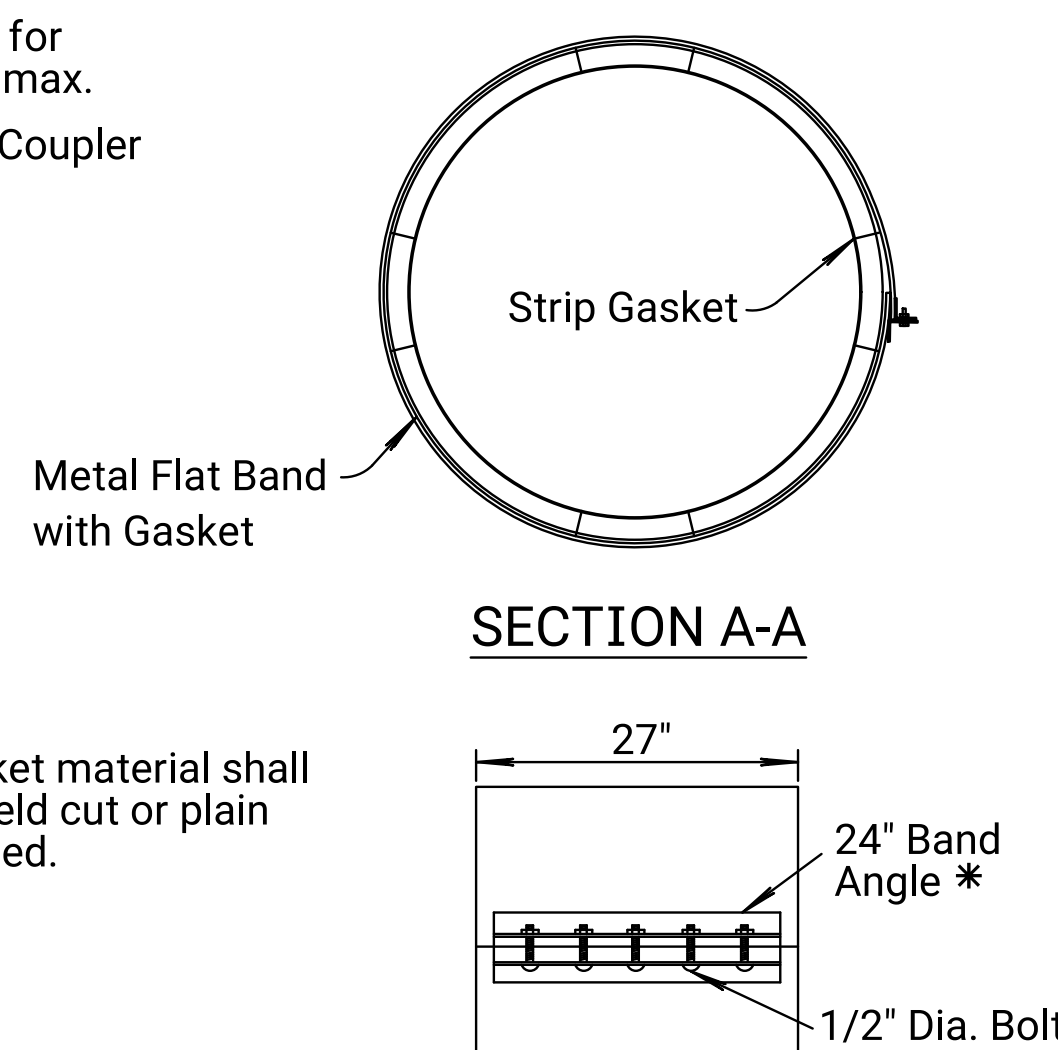
PLAN  
TAPERED SLEEVE FOR ATTACHING  
METAL END SECTION TO SRPE PIPE  
(Illustrated with Type 2 Connector)



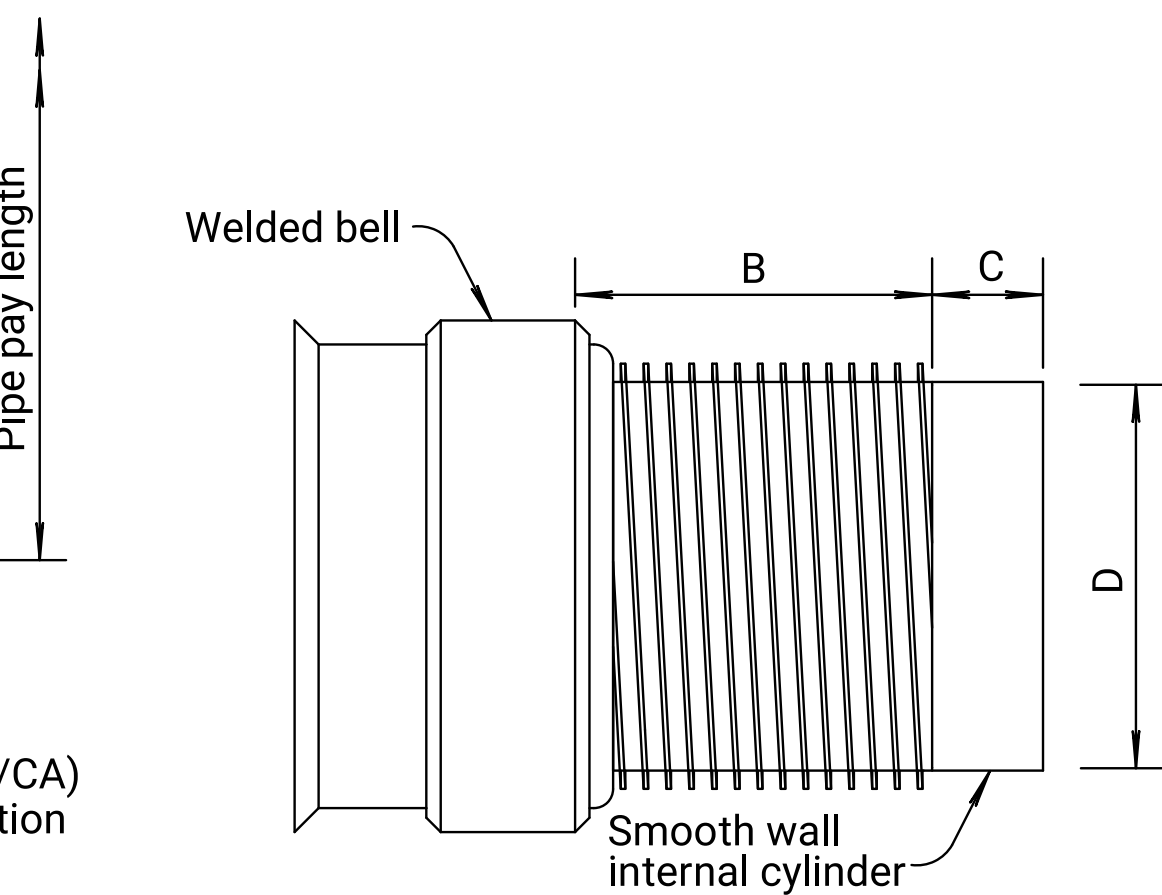
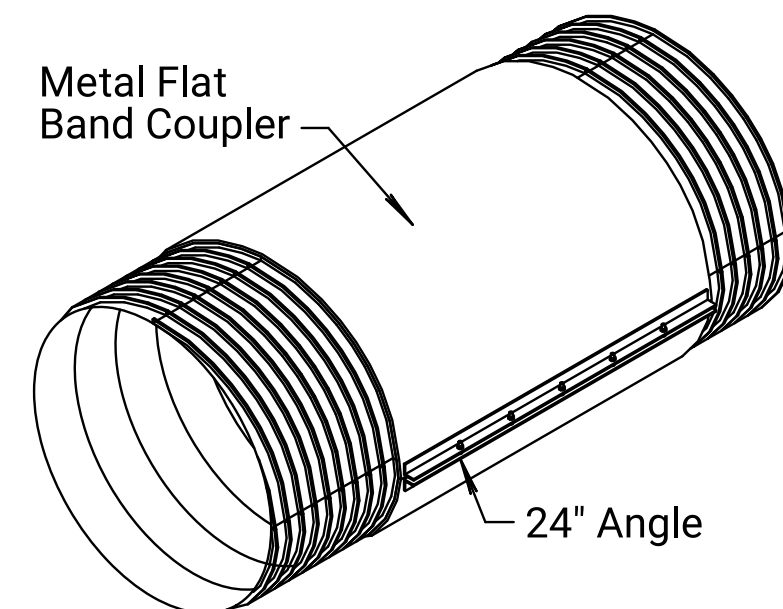
⦿ Different pipe diameters require a different number of gasket strips. See Manufacturers Guide for quantities.



SRPE FLAT BAND COUPLER - SOIL TIGHT JOINT (Used for Field Splice Construction Joints)



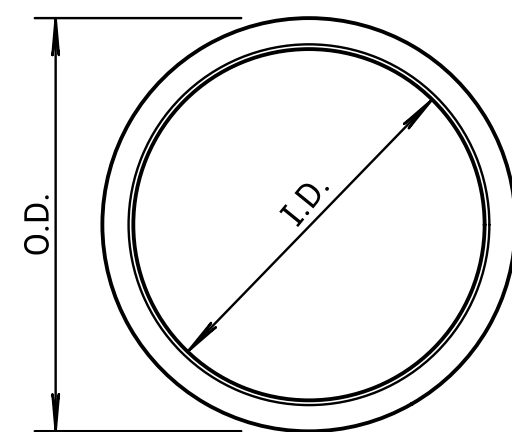
FLAT BAND COUPLER  
\* Alternate option: Double bar bolt & strap



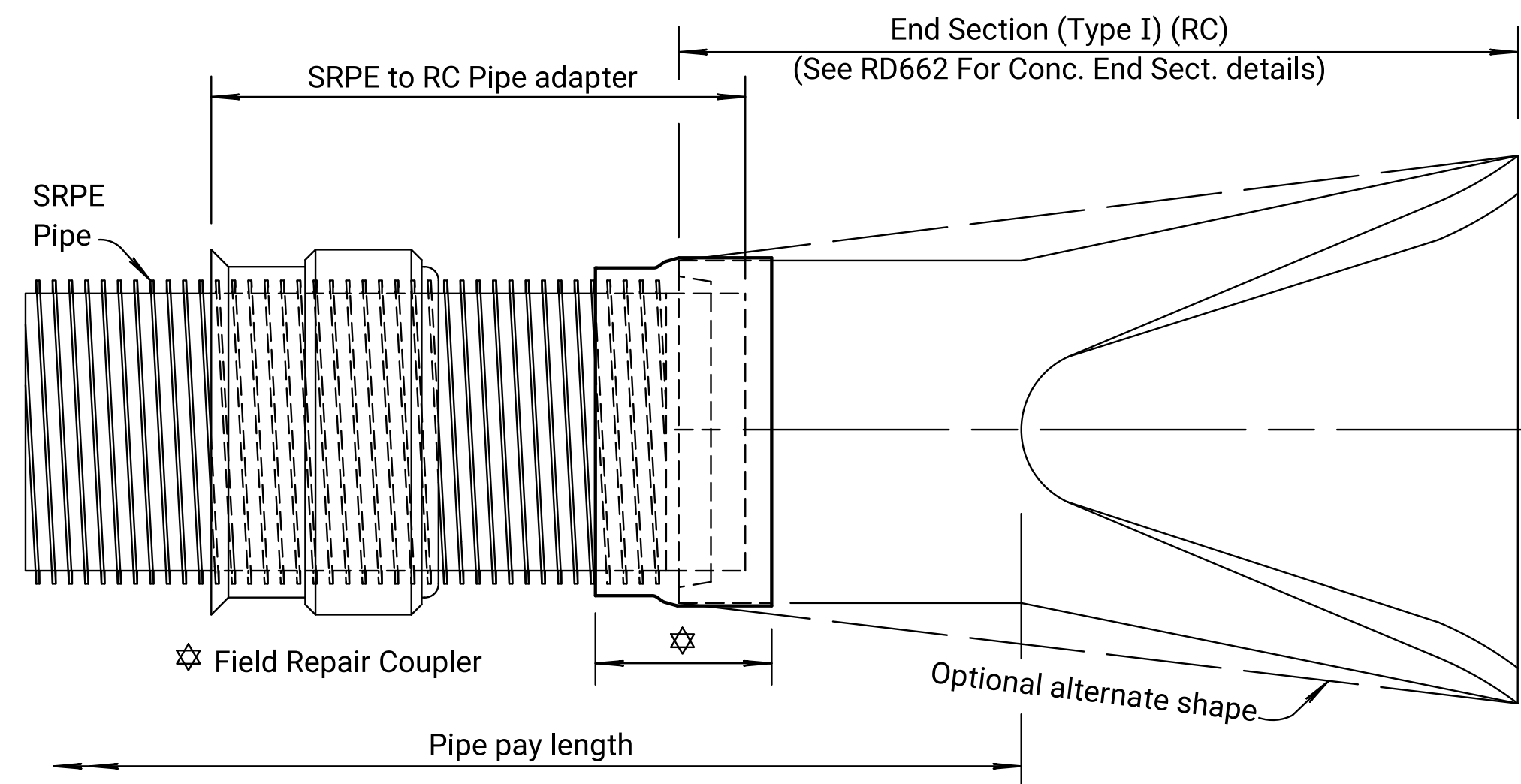
SRPE to RC PIPE ADAPTER

SRPE TO RC PIPE ADAPTER			
Pipe Dia. (In.)	B	C	D
30"	32 13/16"	6"	29.5"
36"	36 3/4"	6"	35.4"
42"	36"	6"	41.3"
48"	36 3/4"	6"	47.2"
60"	36"	6"	59.1"

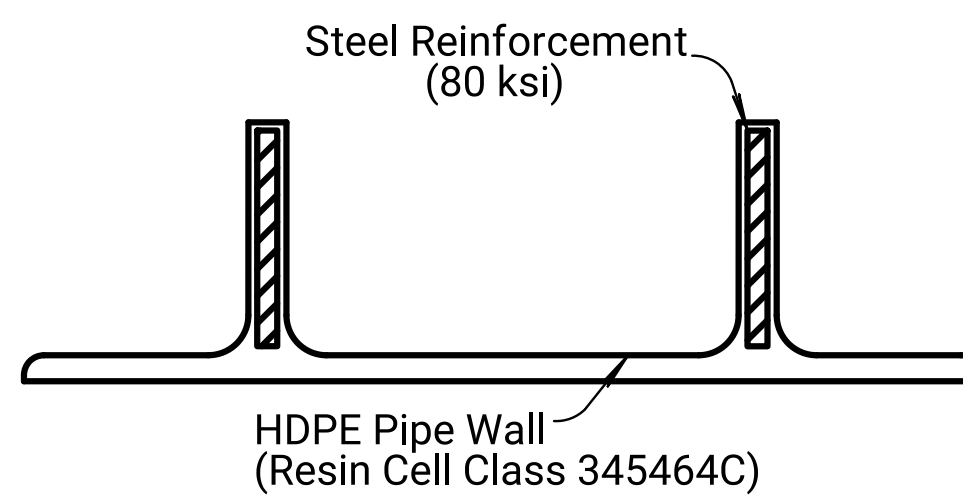
Pipe adapters are available up to 96" pipe diameter.



DETAILS OF  
SRPE PIPE



(This installation is for Acidic Soil Conditions)



TYPICAL SECTION SRPE PIPE

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	28	141

#### GENERAL NOTES

The culvert type shall meet the KDOT Pipe Policy & Standard Specifications. The size of pipe designated on the plan shall be the nominal inside diameter of an SRPE pipe.  
No additional payment shall be made for any gain in length due to the fit of the pipe at connections.  
All SRPE pipe, end sections, couplings, and fittings shall conform with the Standard Specifications.  
See Standard Specifications for SRPE Pipe bedding and backfill.  
Multiple panel end sections shall have lap seams which are to be tightly joined by bolts & nuts. Corner plate and toe plate to be same gauge and material as end section. When required optional toe plate extension shall be overall width less 6" x 8" high.  
Attachment to SRPE pipe 30" diameter and up shall be made with Type 2 Connector.  
All work and materials required for construction and installation of end section shall be included in the bid item "End Section".

#### (CS/ACS/CA) END SECTION FOR SRPE PIPE

Pipe Dia. (In.)	Min. Gauge Ends	Dimensions in Inches					Slope
		A (min.)	B (max.)	H (min.)	L (+/-2")	W (min.)	
30"	14	12	16	8	51	60	2 1/2:1
36"	14	14	19	9	60	72	2 1/2:1
42"	12	16	25	11	69	84	2 1/2:1
48"	12	18	29	12	78	90	2 1/4:1
54"	12	18	33	12	84	102	2 1/4:1
60"	12/10	18	36	12	87	114	2:1

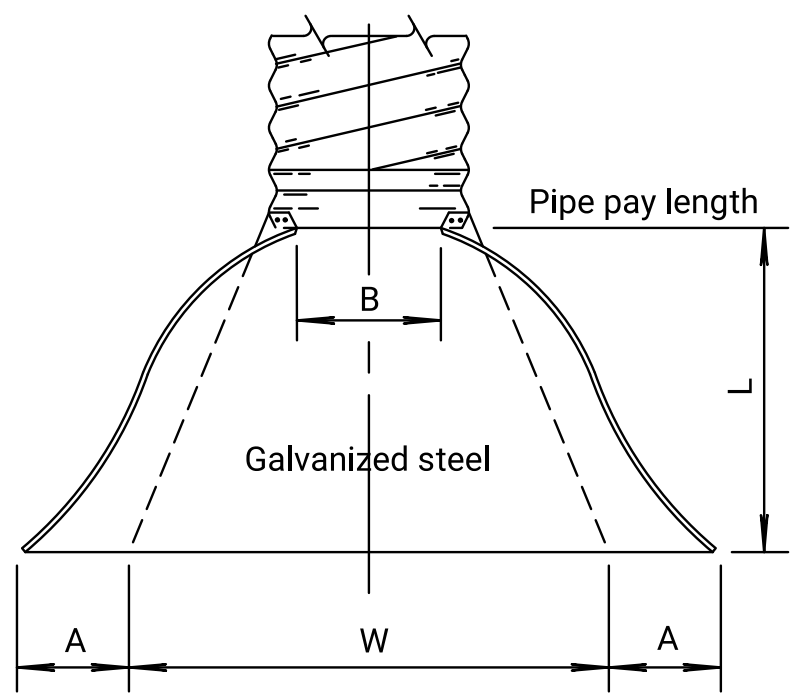
Pipe diameters up to 120" are acceptable for use. End sections are available only up to 96" pipe diameter.

01	05-09-22	Initial Release	A.L.R.	S.W.K.	
NO.	DATE	REVISIONS	BY	APPD	
KANSAS DEPARTMENT OF TRANSPORTATION					
METAL/CONCRETE END SECTION (TYPE I) FOR SRPE PIPE					
RD667D					
FHWA APPROVAL		06-08-22	APPD.	Scott W. King	
DESIGNED	DETAIL	QUANTITIES	TRACED		
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		



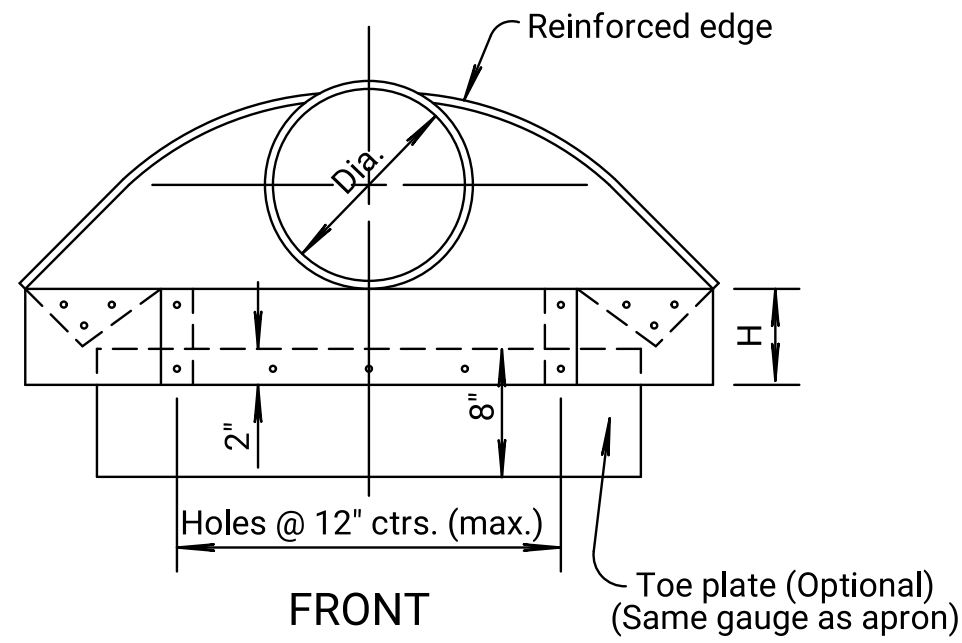
Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:31  
File : KA572901rss660-01.dgn

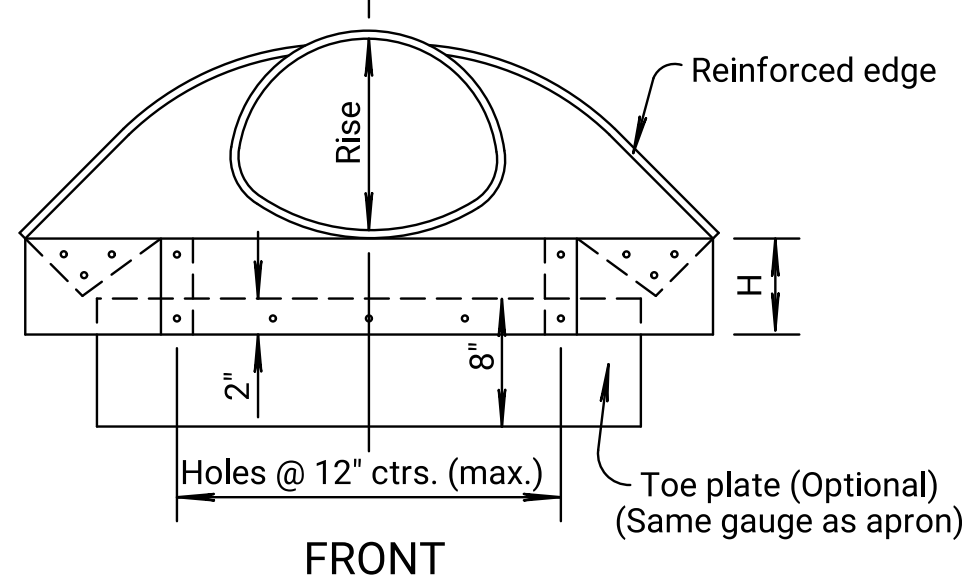


PLAN

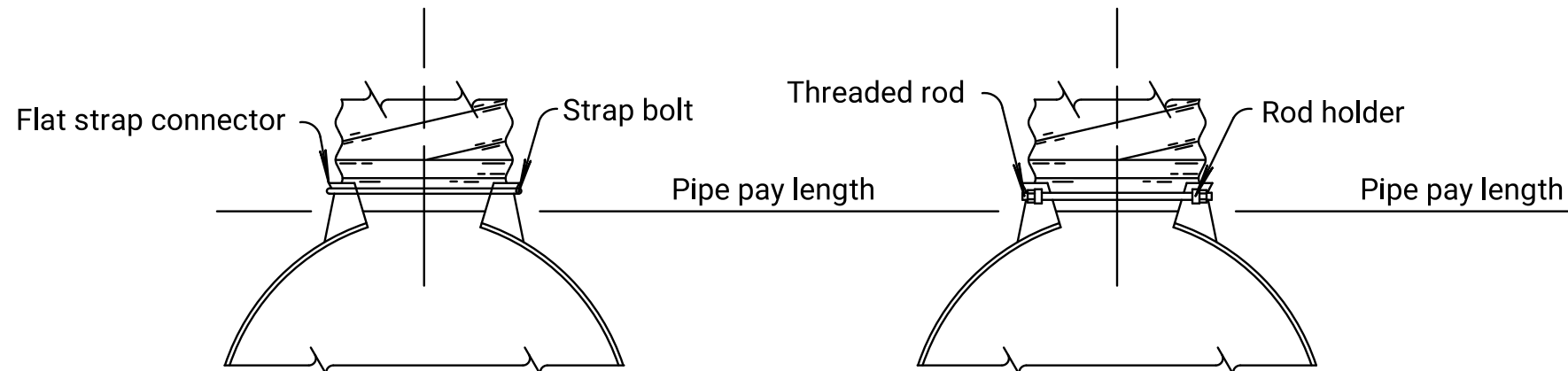
(Illustrated with Type #3 Connection)



FRONT

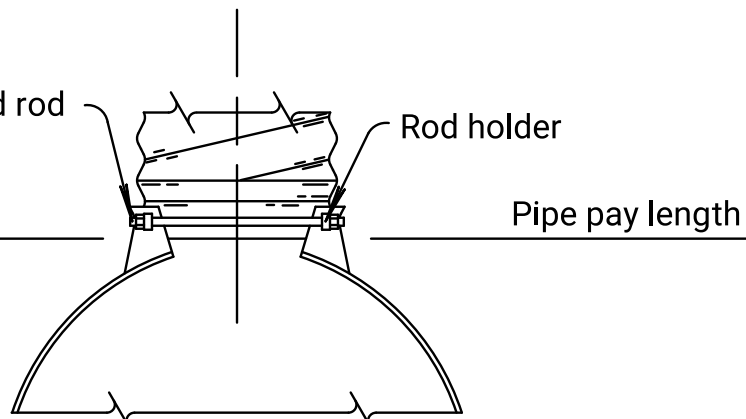


FRONT



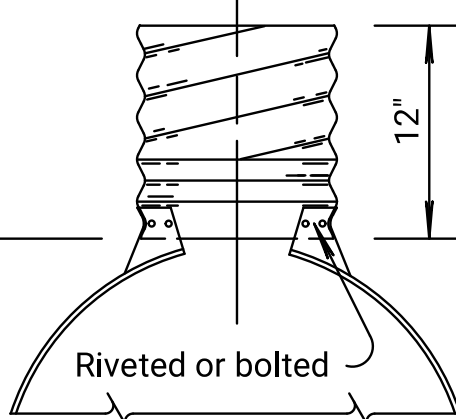
TYPE 1

Available in sizes 12" through 24" only.



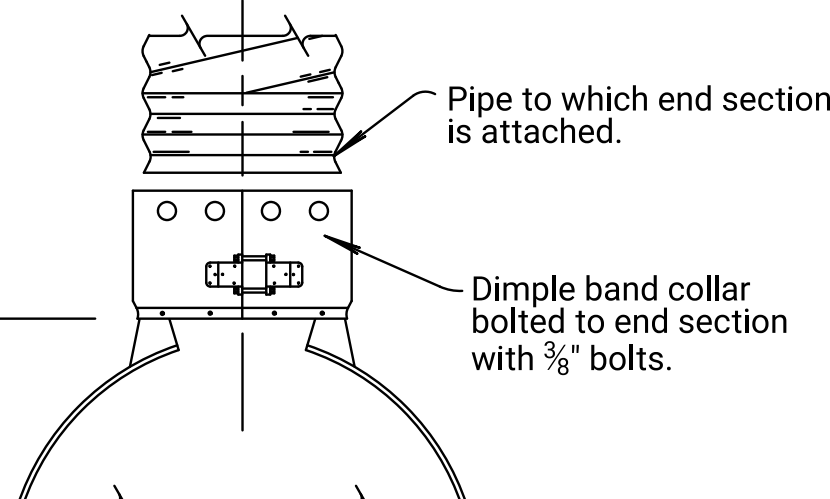
TYPE 2

Available in sizes 30" and 36" Round and 17"x13" through 57"x38" Pipe-Arches.



TYPE 3

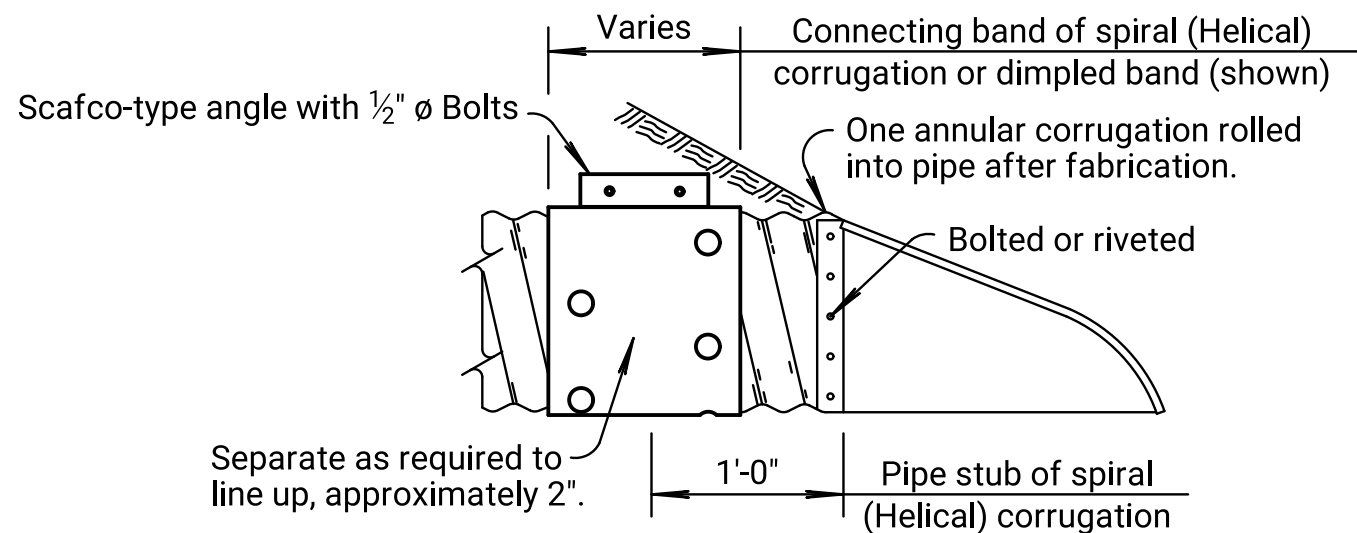
Available in sizes 42" through 96" Round and 60"x46" through 81"x59" Pipe-Arches.



TYPE 5

Available for all Round and equivalent Pipe-Arch sizes, (Type 1 and Type 2 connections are recommended for the smaller sizes with annular ends).

Note: Type 3 connection may be furnished instead of Type 1 or Type 2 for smaller round or arch pipe.



SPIRAL (HELICAL) CORRUGATION

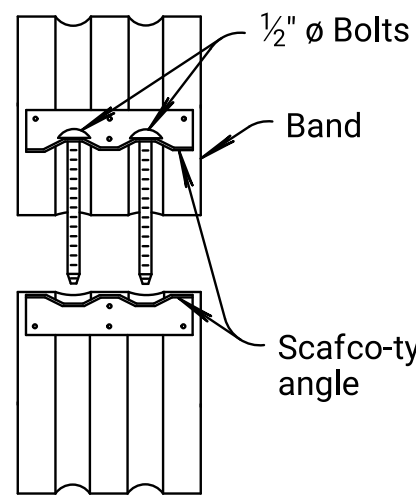
For all sizes of round and arch culvert pipes having Spiral (Helical) corrugations, the end sections and connecting bands shall be as shown above.

Thickness CSP/ACSP	Thickness CAP	Gauge
0.064"	0.060"	16 ga.
0.079"	0.075"	14 ga.
0.109"	0.105"	12 ga.
0.138"	0.135"	10 ga.
0.168"	0.164"	8 ga.

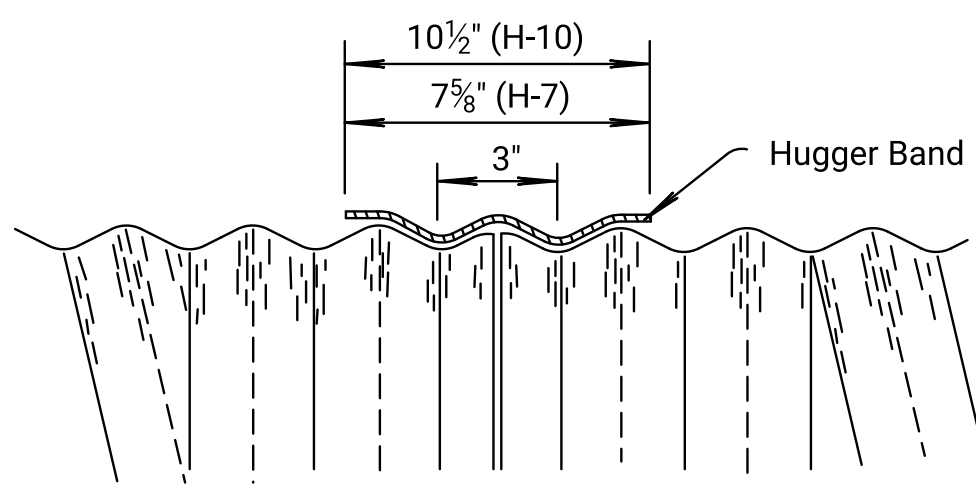
Pipe Dia. (In.)	CS, ACS or CA Gauge	Dimensions in Inches					Approx. Slope
		A (min.)	B (max.)	H (min.)	L (±2")	W (min.)	
12"	16	5	7	6	21	22	2 1/2: 1
15"	16	6	8	6	26	28	2 1/2: 1
18"	16	7	10	6	31	34	2 1/2: 1
21"	16	8	12	6	36	40	2 1/2: 1
24"	16	9	13	6	41	46	2 1/2: 1
30"	14	11	16	8	51	55	2 1/2: 1
36"	14	13	19	9	60	70	2 1/2: 1
42"	12	15	25	10	69	82	2 1/2: 1
48"	12	17	29	12	78	88	2 1/4: 1
54"	12	17	33	12	84	100	2 1/4: 1
60"	12/10	17	36	12	87	112	2: 1
66"	12/10	17	39	12	87	118	2: 1
72"	12/10	17	44	12	87	120	2: 1
78"	12/10	17	48	12	87	130	1 1/2: 1
84"	12/10	17	52	12	87	136	1 1/2: 1
90"	12/10	17	58	12	87	142	1 1/2: 1
96"	12/10	17	58	12	87	144	1 1/2: 1

Bid Designation Sq. Ft.	Nom. W.W. Area Sq. Ft.	Pipe Arch	Dimensions in Inches 2 3/4" x 1/2" Corrugations					Dimensions in Inches 3" x 1" or 5" x 1" Corr.					Approx. Slope		
		Span & Rise	CS, ACS or CA Gauge	A (min.)	B (max.)	H (min.)	L (±2")	W (min.)	CS, ACS or CA Gauge	A (min.)	B (max.)	H (min.)		L (±2")	W (min.)
1.0	1.1	17" x 13"	16	5	9	6	20	28							2 1/2: 1
1.5	1.6	21" x 15"	16	6	11	6	24	34							2 1/2: 1
2.0	2.2	24" x 18"	16	7	12	6	28	40							2 1/2: 1
2.5	2.9	28" x 20"	16	7	16	6	32	46							2 1/2: 1
3.0 or 4.0	4.5	35" x 24"	14	9	16	6	39	58							2 1/2: 1
5.0 or 6.0	6.5	42" x 29"	14	11	18	7	46	73							2 1/2: 1
7.0 or 8.5	8.9	49" x 33"	12	12	21	9	53	82							2 1/2: 1
10.0 or 11.0	11.7	53" x 41"							12	17	26	12	63	88	2: 1
10.0 or 11.0	11.6	57" x 38"	12	16	26	12	62	88							2: 1
12.5 or 14.0	15.6	60" x 46"							12	17	36	12	70	100	2: 1
12.5 or 14.0	14.7	64" x 43"	12	17	30	12	69	100							2: 1
16.5	19.3	66" x 51"							12/10	17	36	12	70	112	1 1/2: 1
16.5	18.1	71" x 47"	12/10	17	36	12	77	112							1 1/2: 1
21.0	23.2	73" x 55"							12/10	17	36	12	77	124	1 1/2: 1
21.0	21.9	77" x 52"	12/10	17	36	12	77	124							1 1/2: 1
25.0	27.4	81" x 59"							12/10	17	44	12	77	136	1 1/2: 1
25.0	26.0	83" x 57"	12/10	17	44	12	77	130							1 1/2: 1
32.0	32.1	87" x 63"							12/10	17	44	12	77	136	1 1/2: 1
36.0	37.0	95" x 67"							12/10	17	44	12	87	160	1 1/2: 1
42.0	42.4	103" x 71"							12/10	17	44	12	87	172	1 1/2: 1
47.0	48.0	112" x 75"							12/10	17	44	12	87	172	1 1/2: 1

(Information listed in these tables are nominal and may vary by manufacturer.)

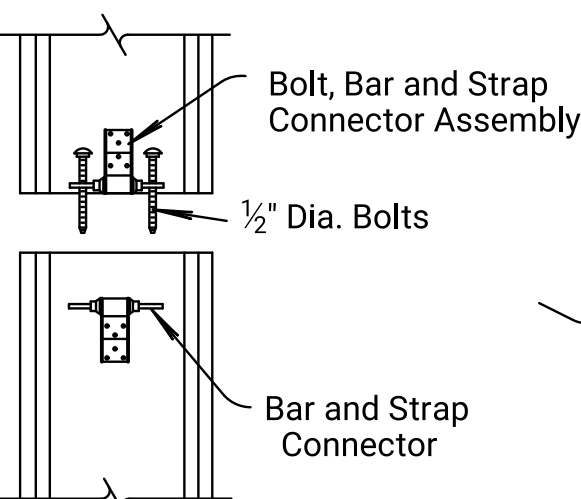


CONNECTION DETAIL H-7 or H-10 BAND

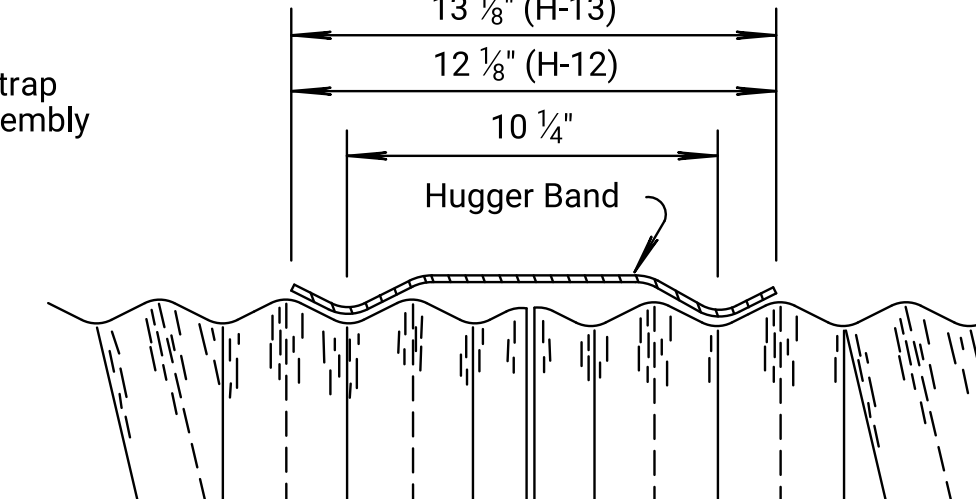


UNIVERSAL REFORMED END with H-7 or H-10 HUGGER BAND

DETAILS FOR H-7 HUGGER BAND (12" thru 36") or H-10 HUGGER BAND (12" thru 120")

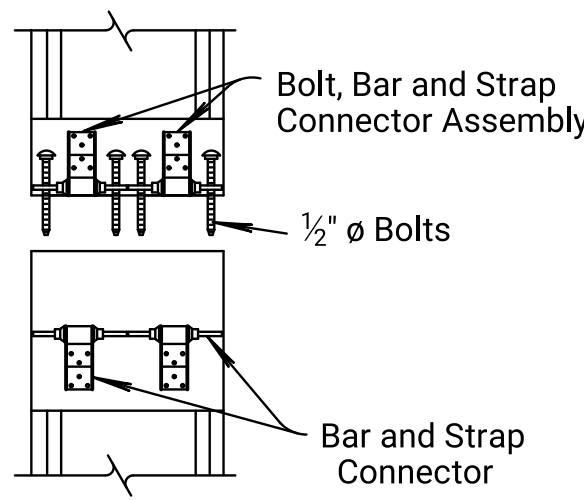


CONNECTION DETAIL SINGLE HARNESS



UNIVERSAL REFORMED END with HUGGER BAND

DETAILS FOR H-12 or H-13 HUGGER BAND



CONNECTION DETAIL DOUBLE HARNESS

Pipe Dia. Inches	Minimum Gauge of Round Pipe				
	2 3/4" x 1/2" Corr. CSP or ACSP	3" x 1" Corr. CSP or ACSP	5" x 1" Corr. CSP or ACSP	2 3/4" x 1/2" Corr. CAP	3" x 1" Corr. CAP
12"	14			16	
15"	14			16	
18"	14			16	
21"	14			16	
24"	14			16	
30"	14			14	
36"	14			14	16
42"	14			12	16
48"	12	14	16	14	16
54"	12	14	16	14	16
60"	10	14	16	14	16
66"	10	14	16	14	16
72"	10	14	16	14	16
78"	8	14	14	14	14
84"	8	14	14	14	12
90"		14	14	14	12
96"		12	12	12	12
102"		12	12	12	10
108"		12	12	12	10
114"		12	12	12	8
120"		10	10	10	8

Bid Designation Sq. Ft.	Pipe Dimension Span & Rise	Sq. Ft.	Equiv. Round Pipe Diameter	Minimum Gauge of Arch Pipe				
				2 3/4" x 1/2" Corr. CSP or ACSP	3" x 1" Corr. CSP or ACSP	5" x 1" Corr. CSP or ACSP	2 3/4" x 1/2" Corr. CAP	3" x 1" Corr. CAP
1.0	17" x 13"	1.1	15"	14			16	
1.5	21" x 15"	1.6	18"	14			16	
2.0	24" x 18"	2.2	21"	14			16	
2.5	28" x 20"	2.9	24"	14			14	
3.0 or 4.0	35" x 24"	4.5	30"	14			14	
5.0 or 6.0	42" x 29"	6.5	36"	14			12	
7.0 or 8.5	49" x 33"	8.9	42"	14			12	
10.0 or 11.0	53" x 41"	11.7	48"		14			
10.0 or 11.0	57" x 38"	11.6	48"	12			10	
12.5 or 14.0	60" x 46"	15.6	54"		14			14
12.5 or 14.0	64" x 43"	14.7	54"	12			10	
16.5	66" x 51"	19.3	60"		14			14
16.5	71" x 47"	18.1	60"	10			8	
21.0	73" x 55"	23.2	66"		14			14
21.0	77" x 52"	21.9	66"	8				
25.0	81" x 59"	27.4	72"		14	12		12
25.0	83" x 57"	26.0	72"	8				
32.0	87" x 63"	32.1	78"		12	12		12
36.0	95" x 67"	37.0	84"		12	12		12
42.0	103" x 71"	42.4	90"		12	12		10
47.0	112" x 75"	48.0	96"		12	12		8
54.0	117" x 79"	54.2	102"		10	10		
60.0	128" x 83"	60.5	108"		10	10		
67.0	137" x 87"	67.4	114"		10	10		
74.0	142" x 91"	74.5	120"		8	8		

**GENERAL NOTE for METAL PIPE**  
Culvert "Type" listed may be CSP, ACSP, CAP, RCP, PVCP & PEP within guidelines of KDOT Pipe Policy for geographic location. More than one pipe "Type" may be acceptable for a design location with allowable types listed for each site.  
There shall be no payment for gain in pipe length due to fit of pipe at connecting band.

When Hugger Bands are used, the H-7 Hugger Band may be used on circular pipes 36" diameter and smaller or pipe arches 42"x 29" and smaller. The H-10 Hugger Band may be used on 12" thru 120" pipe. The H-12 or H-13 Hugger Band are for pipe sizes larger than 36" diameter or 42"x29" arch pipe.  
Pipe gauge listed in the tables on this sheet are minimum for E=750 p.s.i. soil. Pipe gauge will be determined for each site based on the Design Manual Volume I- Part C Fill Height Tables and shall be listed in the Pipe Culvert Summary. Gauges shown on this Standard Drawing are KDOT minimum and may not be industry minimum gauge.  
In geographic areas that allow CSP (24" or smaller arched or round pipe) for entrance and side road installation with less than 3,000 AADT, 16 gauge ACSP may be substituted for 14 gauge CSP.  
Aluminum or aluminized pipes or end sections shall be coated with an asphaltic paint when in contact with fresh concrete in accordance with the Standard Specifications.

04	09-10-09	Rev. Round and Arch tables, add. Alum.	S.W.K.	J.O.B.
03	01-20-09	Rev. Round and Arch tables, add. Alum.	S.W.K.	J.O.B.
02	04-18-08	Rev. layout, details, tables and notes	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

**METAL END SECTION FOR ROUND & ARCH METAL CULVERTS (TYPE I) & PIPE GAUGE TABLES**

RD660

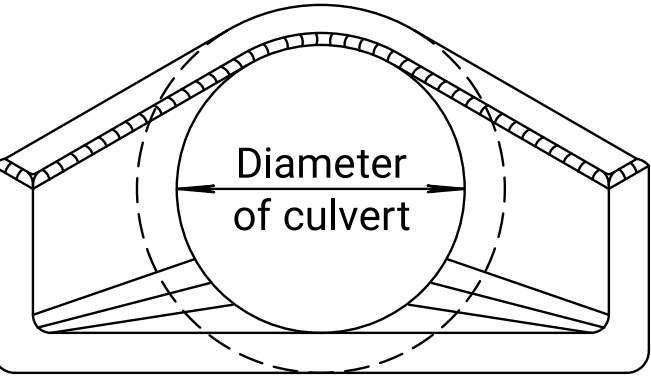
FHWA APPROVAL		12-16-09	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

KDOT Graphics Certified 05-16-2022

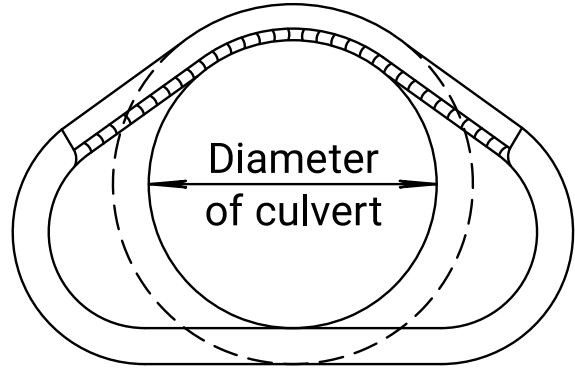
Sh. No. 29

KDOT Graphics Certified

Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

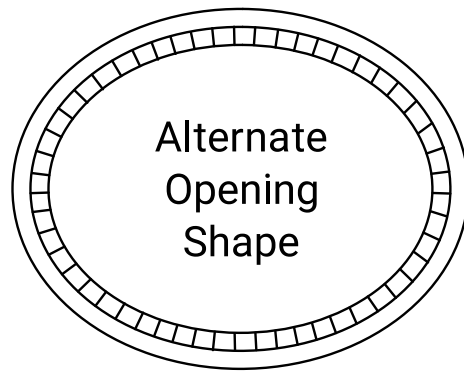


END ELEVATION (TYPE I)

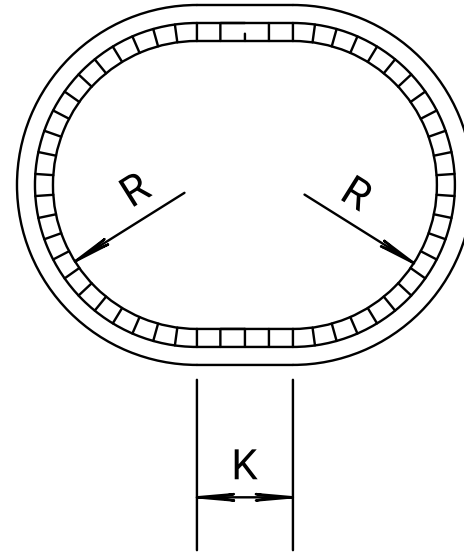


SECTION A-A

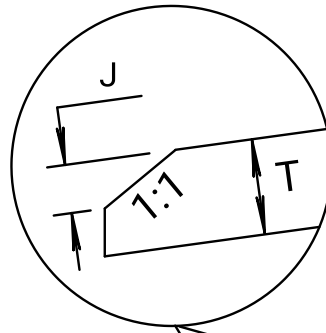
Showing rounding of inside edge of end section.



END ELEVATION (TYPE III)

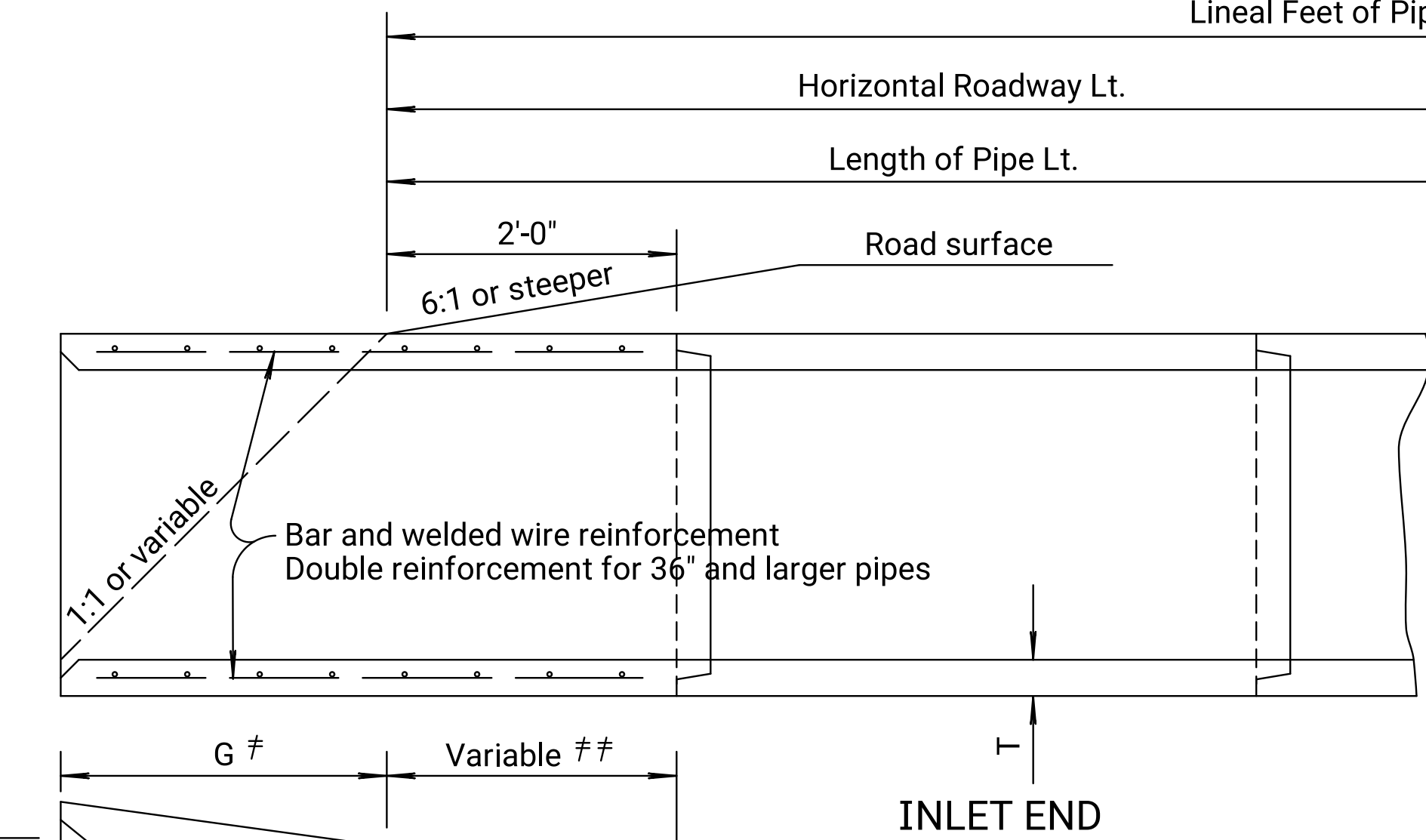


END ELEVATION (TYPE III)

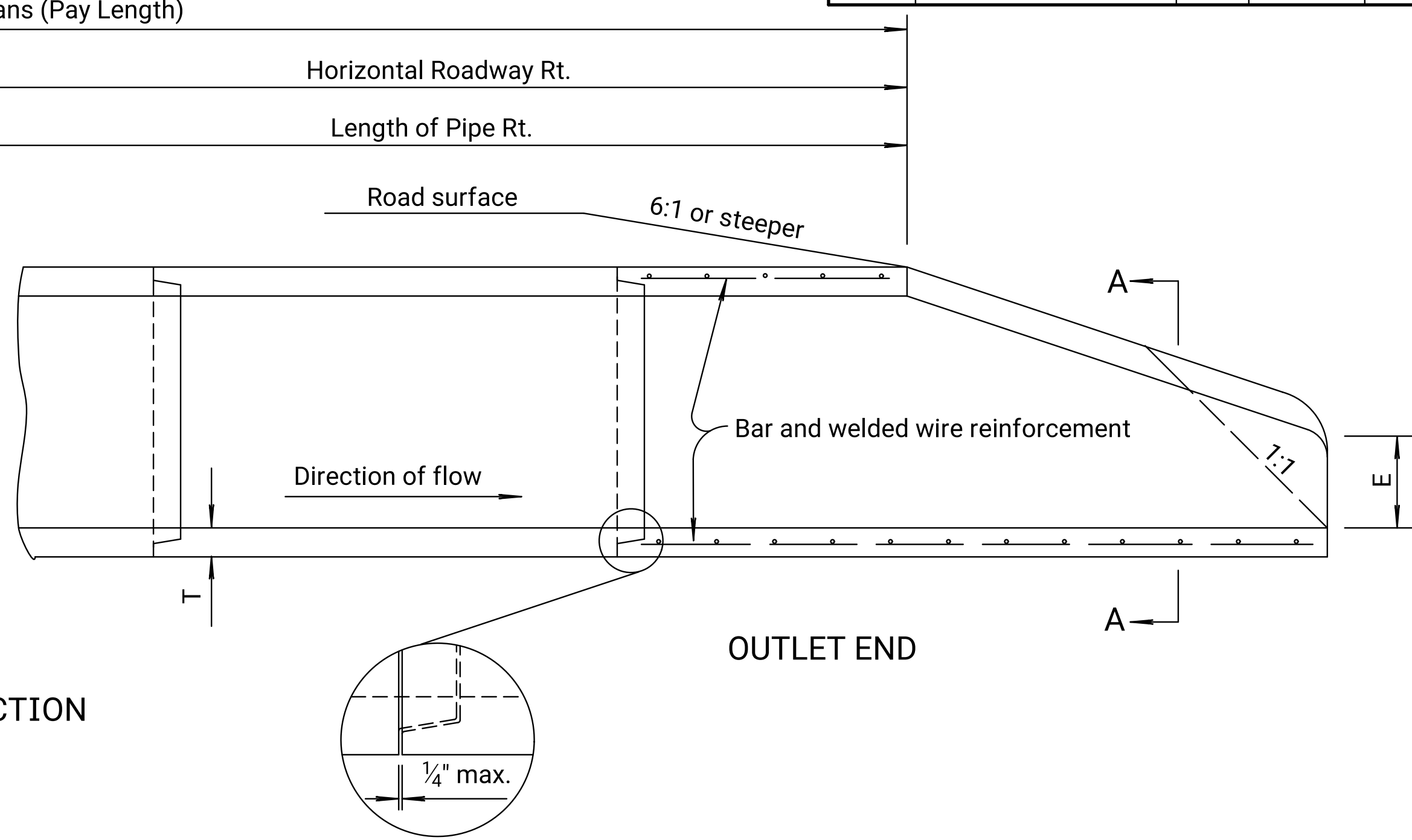


- ∅ Transition to round pipe.
- ≠ Paid for as separate item of End Section, except when structures shall bid as alternates. In that case End Sections shall be subsidiary to bid item. "Drainage Structure No. ".
- ≠≠ Included in pay length of pipe.
- ✱ Minimum waterway area is calculated at the inside of the bevel.

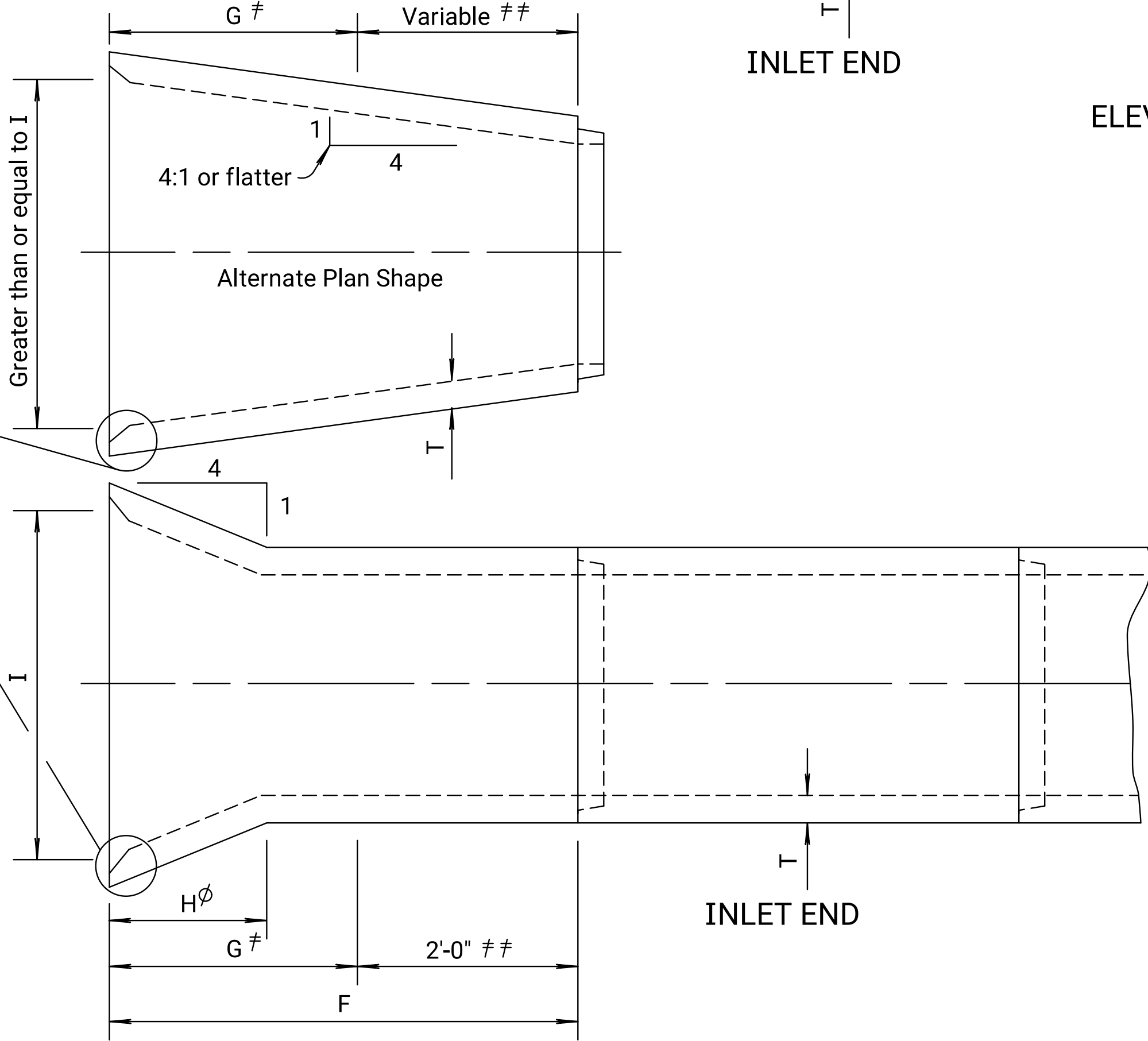
END SECTION (TYPE I) NOMINAL DIMENSIONS								
Diam.	A	B≠≠	C≠	D	E	Ri	Slope	T
12"	6'-0 7/8"	4'-0 7/8"	2'-0"	2'-0"	4"	9	3:1	2"
15"	6'-1"	3'-10"	2'-3"	2'-6"	6"	11	3:1	2 1/4"
18"	6'-1"	3'-10"	2'-3"	3'-0"	9"	12	3:1	2 1/2"
24"	6'-1 1/2"	2'-6"	3'-7 1/2"	4'-0"	9 1/2"	14	3:1	3"
30"	6'-1 3/4"	1'-7 3/4"	4'-6"	5'-0"	1'-0"	15	3:1	3 1/2"
36"	8'-1 3/4"	2'-10 3/4"	5'-3"	6'-0"	1'-3"	20	3:1	4"
42"	8'-2"	2'-11"	5'-3"	6'-6"	1'-9"	22	3:1	4 1/2"
48"	8'-2"	2'-2"	6'-0"	7'-0"	2'-0"	22	3:1	5"
54"	8'-2 1/4"	2'-9 1/4"	5'-5"	7'-6"	2'-3"	24	2.4:1	5 1/2"
60"	8'-3"	3'-3"	5'-0"	8'-0"	2'-11"	24	21	6"
72"	8'-3"	1'-9"	6'-6"	9'-0"	3'-0"	24	1.86:1	7"
84"	9'-3 1/2"	1'-9"	7'-6 1/2"	10'-0"	3'-0"	24	1.6:1	8"



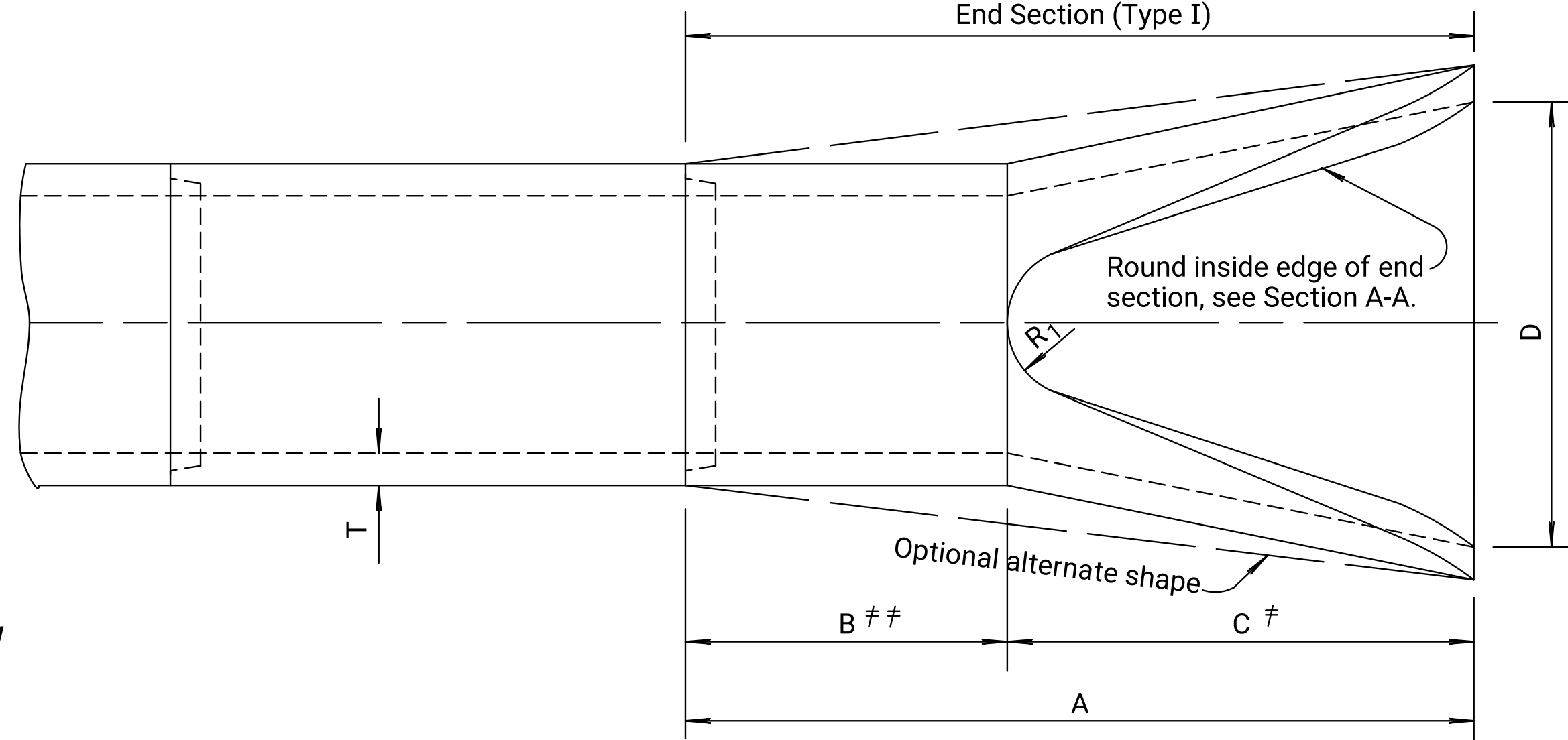
ELEVATION SECTION



Note: There shall be no payment for gain in length due to joint fit tolerance.



PLAN VIEW



OUTLET END

SIDE TAPERED INLET SECTION (TYPE III)-NOMINAL DIMENSIONS									
Diam.	Min. W.W.✱ Area Sq. Ft.	F	G	H	I	J	K	R	T
24"	4.5	4'-3"	2'-3"	1'-5 1/8"	2'-8"	1 1/2"	8"	1'-0"	3"
30"	7.0	4'-9 1/2"	2'-9 1/2"	1'-9 1/2"	3'-4"	2"	10"	1'-3"	3 1/2"
36"	10.1	5'-4"	3'-4"	2'-1 1/2"	4'-0"	2"	1'-0"	1'-6"	4"
42"	13.7	5'-10 1/2"	3'-10 1/2"	2'-5 7/8"	4'-8"	2 1/2"	1'-2"	1'-9"	4 1/2"
48"	17.9	6'-5"	4'-5"	2'-10 1/8"	5'-4"	3"	1'-4"	2'-0"	5"
54"	22.7	6'-11 1/2"	4'-11 1/2"	3'-2 1/2"	6'-0"	3 1/2"	1'-6"	2'-3"	5 1/2"
60"	28.0	7'-6"	5'-6"	3'-6 7/8"	6'-8"	4"	1'-8"	2'-6"	6"
72"	40.3	8'-7"	6'-7"	4'-3 3/8"	8'-0"	5"	2'-0"	3'-0"	7"
84"	54.8	9'-8"	7'-8"	5'-0 3/8"	9'-4"	6"	2'-4"	3'-6"	8"

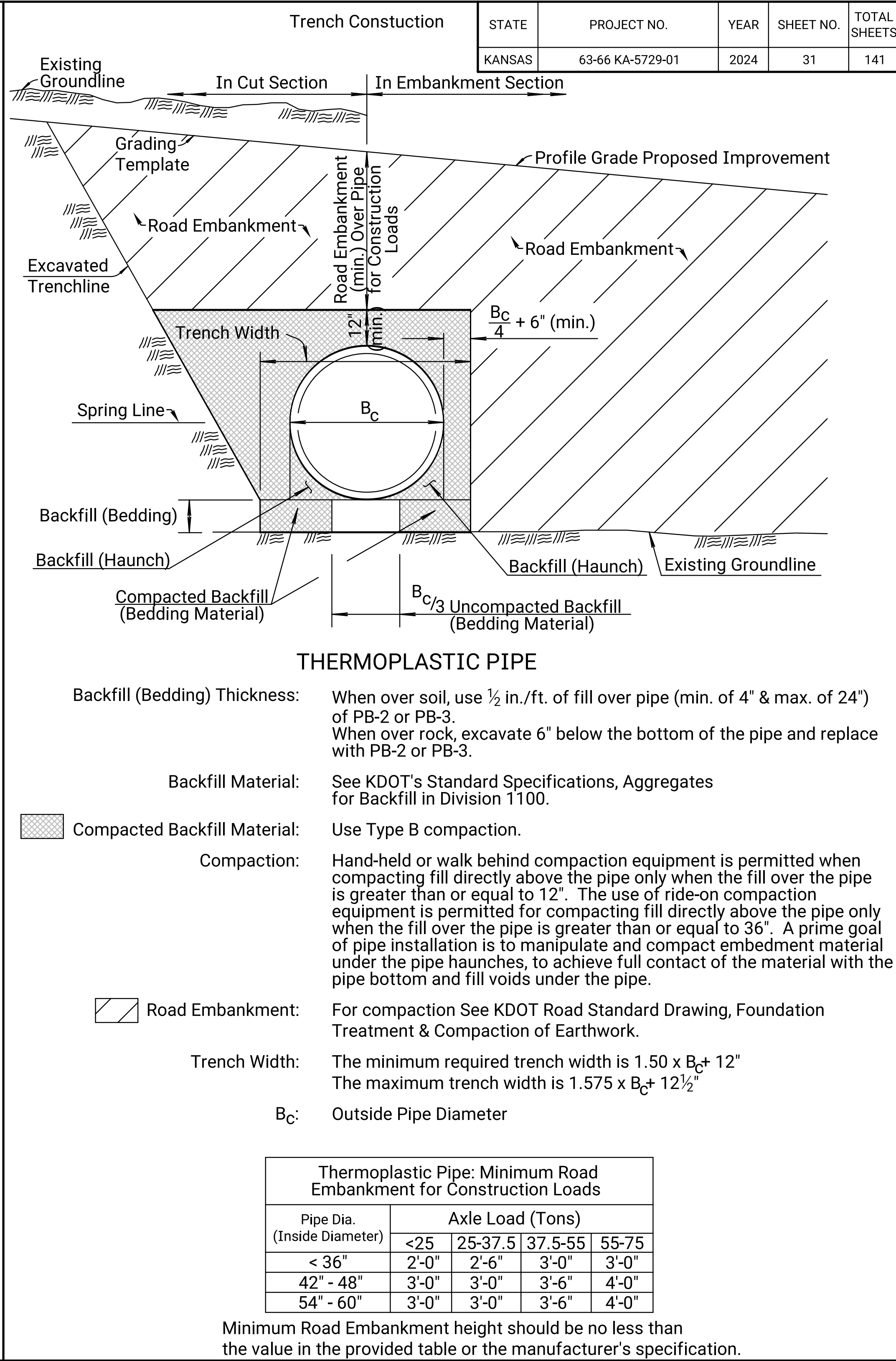
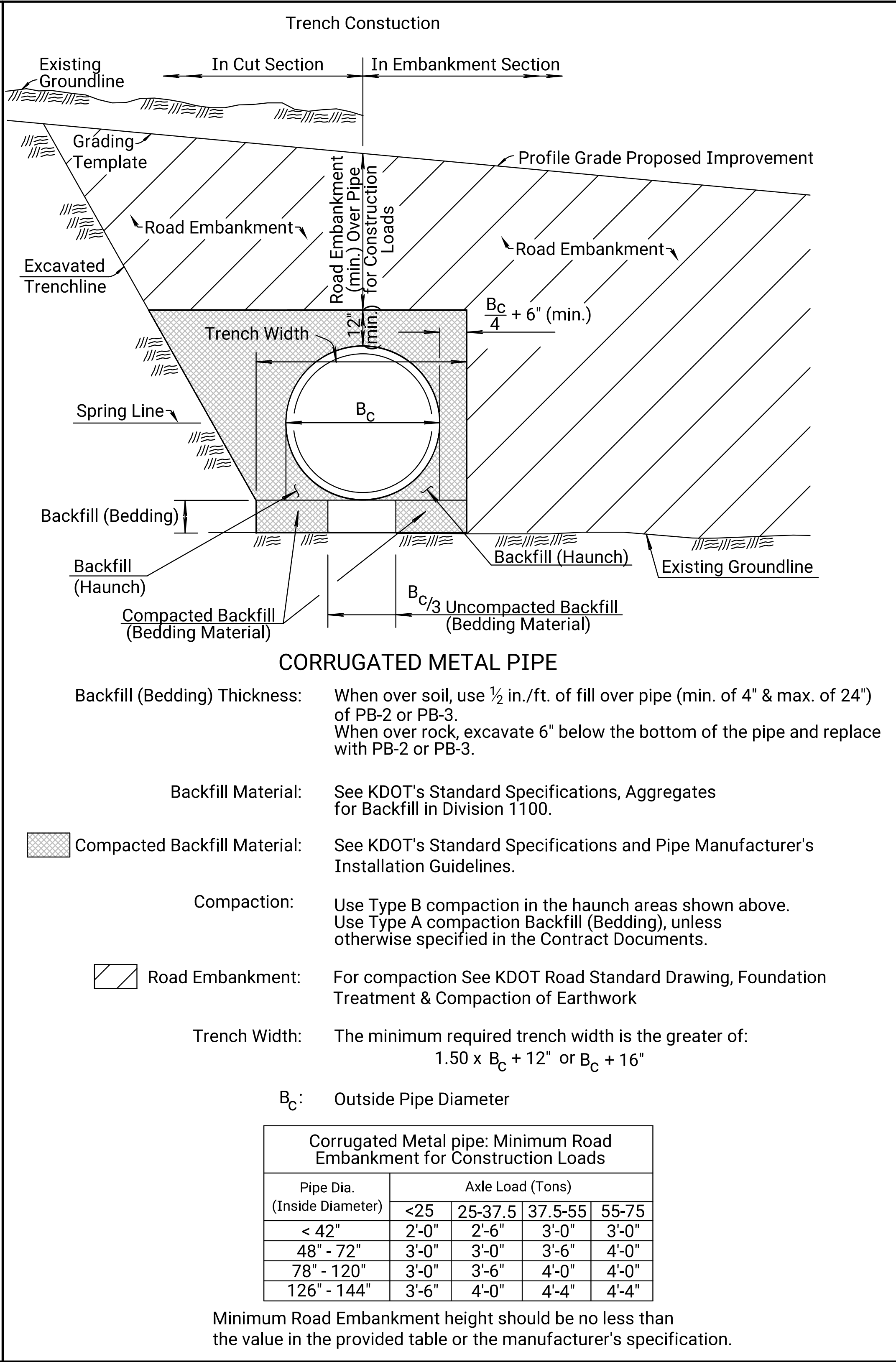
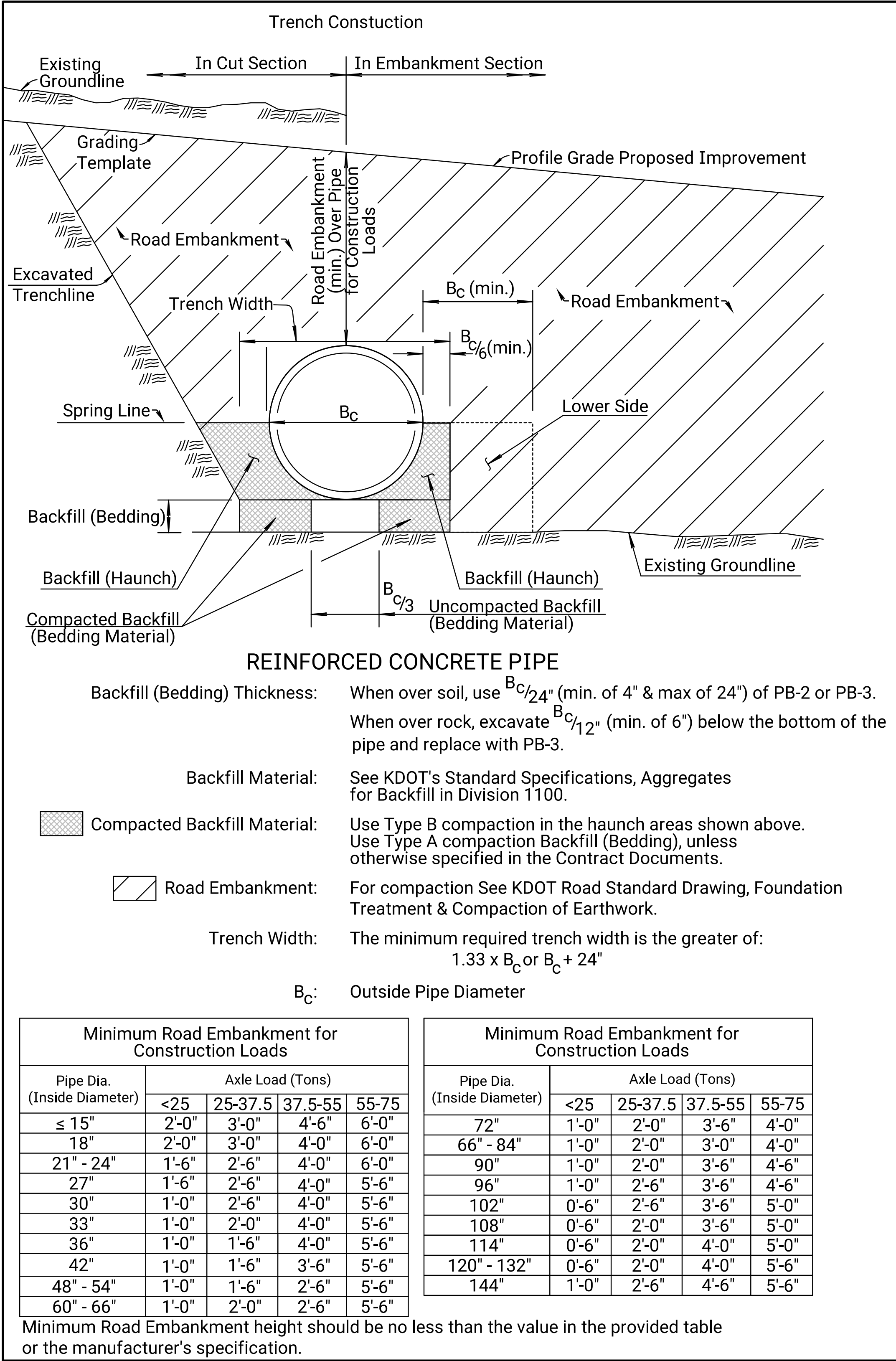
Dimensions for alternate shapes shall be equal to or greater than those shown in the table, unless otherwise shown.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	30	141

02	04-18-08	Added ref. to KDOT Pipe Policy	S.W.K.	J.O.B.
01	04-05-05	Revised reinforcement callout	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
CONCRETE END SECTIONS				
FOR CONCRETE PIPES				
TYPE I & SIDE TAPERED				
INLET SECTION (TYPE III)				
RD662				
FHWA APPROVAL		06-27-08	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	



Plotted by : Brogan Andrews@ks.gov  
File : KA572901rss659-01.dgn  
16-APR-2024 17:40



GENERAL NOTES

Do not drop, drag or otherwise handle pipe sections in a manner which may cause damage. Inspect the line and grade before and during placement of compacted backfill and uncompacted backfill materials.

For trench installations place pipe in the center of the excavated trench.

When installing pipe, place the uncompacted backfill and compacted backfill material in the bedding area to grade, install pipe to grade, place and compact the haunch area up to the spring line of the pipe and complete the backfill as specified in KDOT's Standard Specifications unless otherwise noted in the contract documents.

$B_c$  for horizontal elliptical pipe, vertical elliptical pipe, arch pipe, and non bridge-sized concrete box structures will be measured along the horizontal axis; similar to the dimension shown for circular pipe on this sheet.

The spring line is a line along the side of the culvert where the tangent to the culvert wall is vertical. It occurs at the widest point in the culvert.

Material used for the roadway embankment may be used in lieu of compacted backfill material as approved by the Engineer.

The backfill load transmitted to the pipe is directly dependent on the trench width. Where maximum trench widths are not indicated in any of the contract documents, trench widths should be as narrow as possible with side clearance adequate enough to ensure proper compaction of backfill material at the sides of the pipe. The trench width formulas provided can be used as a general guide.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	31	141

01	05-09-22	Initial Release	A.L.R.	T.T.R.
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
PIPE INSTALLATION DETAILS				
RD658				
FHWA APPROVAL		06-08-22	APP'D.	
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN CK.	TRACE CK.	

KDOT Graphics Certified

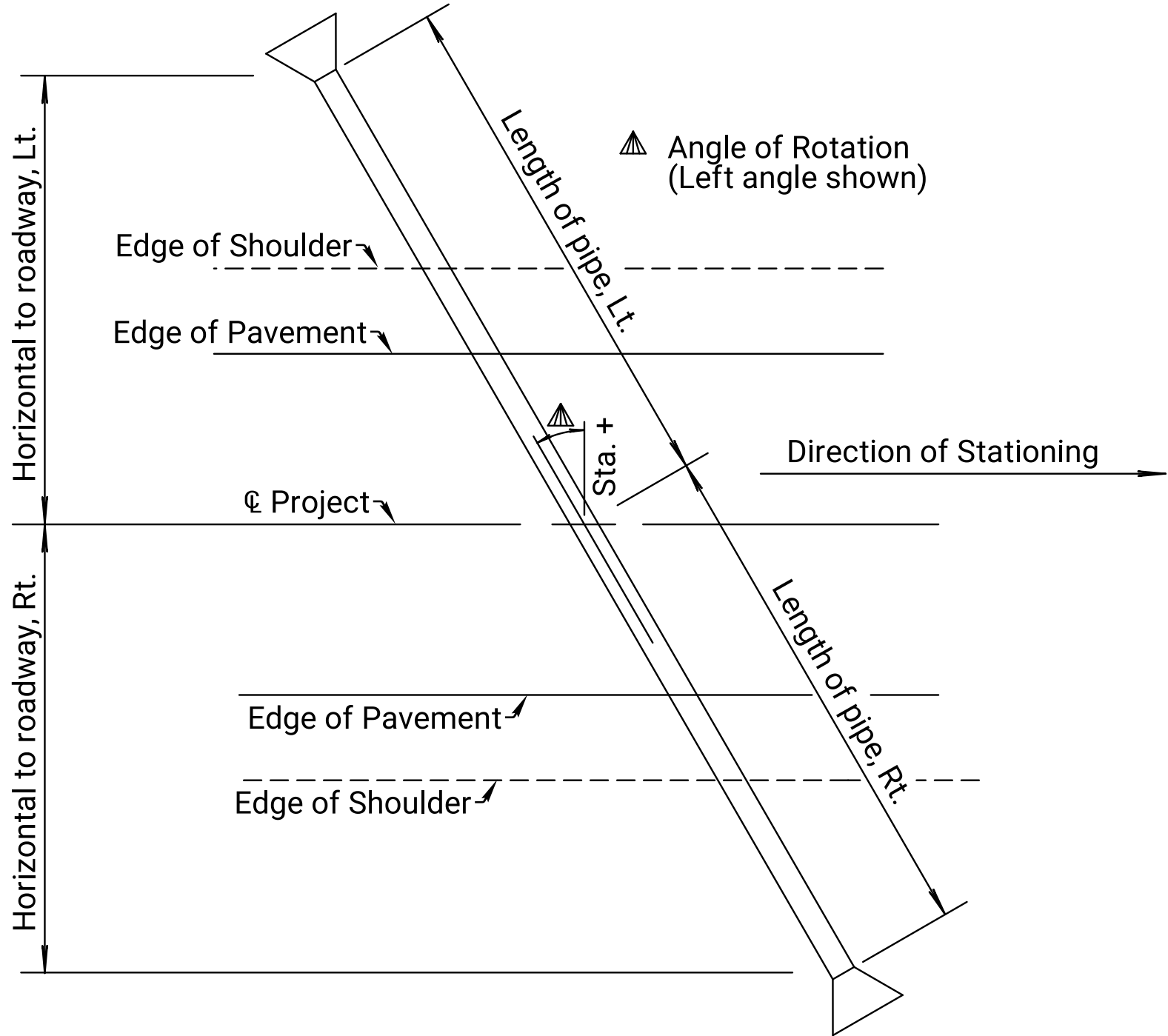
Note to Designer:

KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, PPP SRPE, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	32	141

[illegible]

- Unless otherwise noted, minimum pipe gauge & corrugations to be as shown in RD660. See Summary of Quantities for End Section information.
- ✱ Only include floor elevations for embedded pipes. See RD668 for details. For structures not embedded, the floor elevations may be omitted.



PLAN  
(Showing Rotation about  $\mathbb{C}$ )

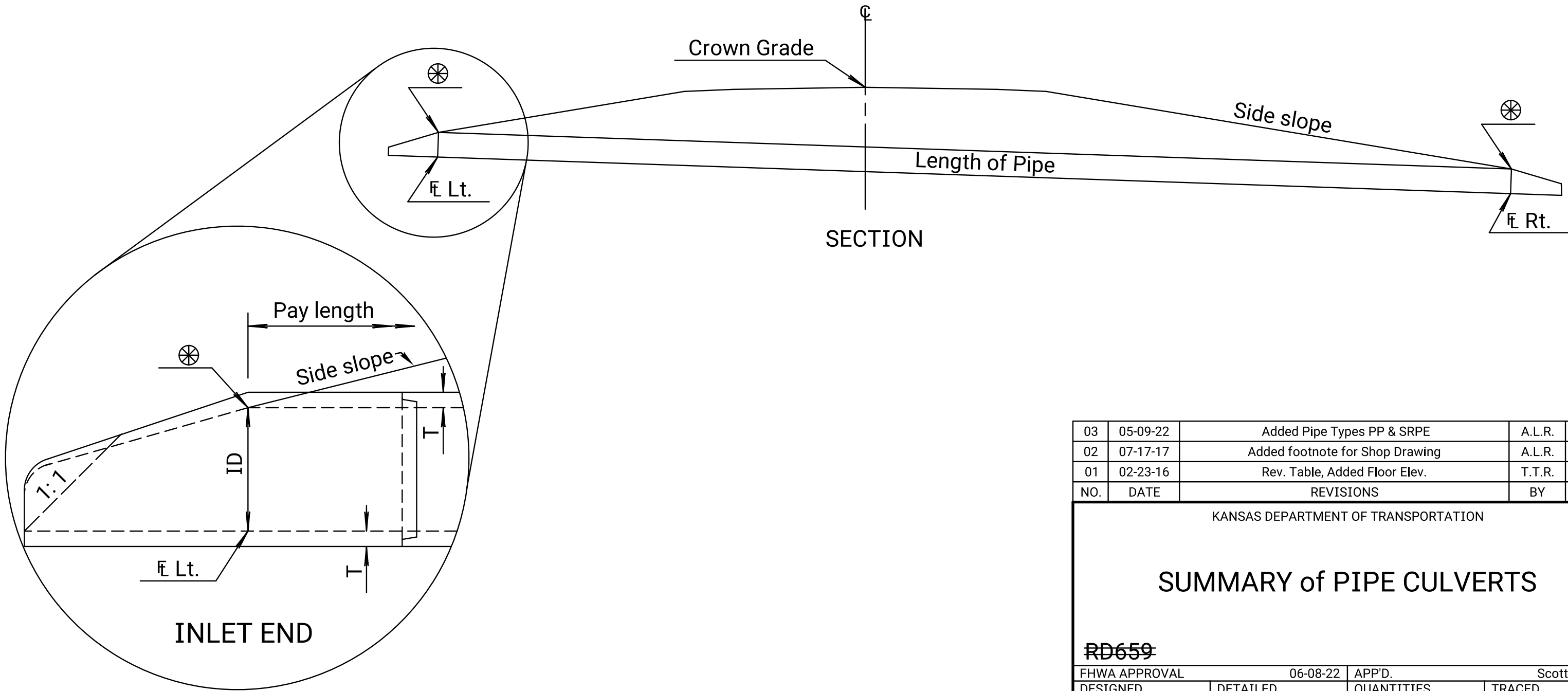
⊗ Design side slope to intersect inside diameter of pipe outside of Clear Zone.

Type	ALLOWABLE LOCATION 			
	Mainline	Side Road	Entrance	Storm Sewer Under ML Not Under ML
 PVCP	X	X	X	X
 PEP	X	X	X	X
 PPP	X	X	X	X
 SRPE	X	X	X	X
CSP				X
ACSP		X	X	X
CAP	X	X	X	X
RCP	X	X	X	X

- ✱ When inside diameter of pipe is 36" or less.
- ⚠ Unless otherwise specified in the plans. Some pipe types may not be allowed at a location if the fill height exceeds the maximum allowable or is less than the minimum allowable cover.
- 📏 When inside diameter of pipe is 60" or less.
- 📏 For inside diameter:  $\geq 30"$

Type	ALLOWABLE END SECTIONS			
	◆ CS	◆ ACS	CA	RC
PVCP	X	X	X	✓
PEP	X	X	X	X
PPP	X	X	X	X
SRPE	X	X	X	X
RCP				X
ACSP CAP CSP	Provide End Sections of the same material and coating type as the pipe.			

- ◆ Type IV End Sections are only made of CS or ACS.
- ψ Submit Shop Drawing of connection for review



03	05-09-22	Added Pipe Types PP & SRPE	A.L.R.	S.W.K.
02	07-17-17	Added footnote for Shop Drawing	A.L.R.	S.W.K.
01	02-23-16	Rev. Table, Added Floor Elev.	T.T.R.	S.W.K.
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
SUMMARY of PIPE CULVERTS				
RD659				
FHWA APPROVAL		06-08-22	APP'D.	Scott W. King
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	



Plotted: J:\ke.P\farmenstf\eks.gblot Location: Bridge  
File: ka572901bbr0048-01.dgn  
Plot Date: 15-JUL-2024 15:55

SUMMARY OF QUANTITIES												
Item  Location	Excavation		Concrete		Reinforcing Steel		Structural Steel			Bearings (Steel Reinf.) (Elast.) Each	Welded Stud Shear Connectors Each	Bridge Deck Grooving  Sq. Yds.
	Class I	Class II	(Grade 4.0) (AE) (SW)	(Grade 4.0) (AE)	Epoxy Coated Grade 60	Grade 60	M270 (Gr. 50WT 3) Lbs.	A709 (Gr. 50W) Lbs.	A709 (Gr. 50) Lbs.			
	Cu. Yds.	Cu. Yds.	Cu. Yds.	Cu. Yds.	Lbs.	Lbs.						
Abutment No. 1	139.3		**	25.8	**							
Pier No. 1	2.3			30.7		5,015				6		
Pier No. 2		1.1		39.7		6,293				6		
Abutment No. 2			**	25.8	**							
Substr. Total	141.6	1.1		122.0		11,310				12		
Superstr. Total			365.5		102,670		256,028	23,415	1,827		3,708	935
Total	142	1	365.5	122.0	102,670	11,310	256,028	23,415	1,827	12	3,708	935

SUMMARY OF QUANTITIES									
Item  Location	Drilled Shaft (48") (Cased) Lin. Ft.	Sonic Test (Drilled Shaft) (Set Price) Each	Core Holes (Investigative)	Piles (Steel) (HPI2x53) Lin. Ft.	Cast Steel Pile Points Each	Bridge Backwall Prof. System Sq. Yds.	Abutment Aggregate Drain Cu. Yds.	Slope Protection (Riprap Stone) ● Cu. Yds.	Temp. Shoring  L.S.
			Lin. Ft.						
Abutment No. 1				448	8	49	63	252	
Pier No. 1	180		180						
Pier No. 2	161		161						
Abutment No. 2				368	8	49	63	369	
Substr. Total	341	1	341	816	16	98	126	621	
Superstr. Total									
Total	341	1	341	† 816	16	98	126	621	L.S.

CONTRACTOR CONSTRUCTION STAKING: Contractor Construction Staking for clear span bridges requires two independent surveys. See KDOT Specifications.

GENERAL NOTES

QUANTITIES: Items not listed separately in the Summary of Quantities are subsidiary to other items in the proposal.

EMBANKMENT: Complete the embankment at the abutments as shown on the Bridge Excavation sheet prior to driving the abutment piling or commencing with the abutment footing excavation.

BRIDGE EXCAVATION: Elevation 1033.50 shall designate the Excavation Boundary Plane of Class I and Class II Excavation; Class I above the plane, Class II below the plane. See the Bridge Excavation sheet for the limits of pay excavation.

PILING SPLICE LOCATION: Integral pile splice locations and weld testing criteria for, Abutments No. 1 & 2 will follow the "Standard Pile Details" Sheet (BR110).

PILING: Drive all piling to penetrate or bear within the Curzon Limestone Member at Abutment 1, and the Du Bois Limestone Member at Abutment 2. Driving shall stop when in the opinion of the Engineer additional driving may damage the piling. Drive all piling to the Pile Driving Formula Load of:

Abutment No. 1 68.5 Tons  
Abutment No. 2 68.5 Tons

As a minimum drive each pile to the load and penetration, but in no case shall the pile be driven to more than 110% of Pile Driving Formula Driving Load. At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Engineer may request that the Pile Driving Analyzer (PDA) equipment be used.

PILING: The limestone bedrock is variable across Abutment 1 and Abutment 2. Please monitor the pile installation carefully to prevent overdriving and pile damage.

PAINT SYSTEM ON EXISTING STRUCTURE: The structural steel has a paint history of:  
1) Original paint system: Unknown Date: 1956  
2) Repaint system: Red Lead/Aluminum Date: 1988  
3) TCLP value is Unknown  
4) Tons of Steel: 79.4  
5) Paint Area: Unknown

ASBESTOS INFORMATION: Samples of this structure were tested to determine the amount of Asbestos Containing Materials (ACM) present in the components. The results are listed below:

Concrete 0%  
Date of Report 11-09-2021

For any result above greater than 1%, abatement shall be performed according to KDOT Specifications. Results less than 1% require no abatement.

GEOTECHNICAL REPORT: The geotechnical report (Dated May 2022) includes soil parameters for retaining wall (sheet pile) design. The report recommends a traffic surcharge of 250 lb/ft<sup>2</sup>. The geotechnical information shown on the plans is the best information available. The report is available for inspection by qualified bidders at the State Bridge Office, KDOT, Eisenhower State Office Building, 700 SW Harrison, Topeka, KS.

BACKFILL COMPACTION: Compact backfill at the abutments.

REMOVAL OF EXISTING STRUCTURES: Removal of existing structure is included in the bid item, "Removal of Existing Structures", Lump Sum. All materials removed from the existing structure shall become the property of the Contractor with the exception of the two existing bridge number plates, which shall be salvaged by the Contractor and remain the property of the State.

TOUCH-UP: Prepare and paint all bolts, nuts, studs, and other small areas of damaged paint (1 square yard or less) requiring touch-up, with an approved organic zinc primer.

NOTE: Only steel pile HPI2X53 shall be used on this project.

† Summary of Piling  
Abutment No. 1 4 @ 56' (Phase II)  
4 @ 56' (Phase III)  
Abutment No. 2 4 @ 46' (Phase II)  
4 @ 46' (Phase III)

\*\* Quantities are included in the Superstr. Total Quantity.

● Gradation shall meet the requirements of 200 Lb. Class.

PAINTING: Blast clean all surfaces of all weathering steel, including all contact surfaces of bolted connections, to meet SSPC-SP6 Specifications (latest Revision). Blast clean to meet SSPC-SP10 Specifications and prime coat the embedded portion of the girders, including the abutment diaphragms; the top flanges, including the shear studs; and the top flange splice plates in accordance with KDOT Specifications.

Top Flanges: (Studs applied in the shop) Apply a 3 mil primer coat of an approved inorganic zinc primer to the tops of the top flanges and to the studs.

(Studs applied in the field) Shop Work - Blast clean the tops of the top flanges to SSPC-SP10 Specifications (latest Revision).

Field Work - Blast clean the tops of the top flanges to SSPC-SP6 Specifications (latest Revision) before the studs are applied. After the studs are applied, blast clean the tops of the top flanges and the studs to SSPC-SP6 Specifications and paint with an approved organic zinc primer to a minimum dry film thickness of 3 mils.

PAINTING: The shop and field coats applied to Structural Steel shall conform to an inorganic zinc primer with a waterborne acrylic finish coat. The finish coat will be Kansas (Brown), this color will match Federal Standard #1(20045).

BRIDGE BACKWALL PROTECTION SYSTEM: See the General Notes on the "Abutment Aggregate Drain" sheet.

ABUTMENT AGGREGATE DRAIN: See the General Notes on the "Abutment Aggregate Drain" sheet.

BRIDGE DECK GROOVING: After the bridge deck has cured, transversely groove the deck in accordance with KDOT Specifications. For phased construction groove each completed phase before opening to traffic. Align the grooves from each adjacent phase across the bridge deck without jogs or discontinuities. For skewed bridges all grooving will be perpendicular to the centerline of the bridge.

LRFR RATING FACTORS		
Design Load	Rating Level	Inventory
HL-93 Loading		1.259
2021 Manual for Bridge Evaluation		

LFD RATING FACTORS		
Truck	Rating Level	Inventory
HS-20 (36T)		1.481
Type HET (110T)		1.209
2002 LFD Rating. 17th Edition AASHTO		

TRAFFIC DATA	
AADT (2024)	1,250
AADT (2044)	1,550
DHV	11%
D	60%
T	22.5%

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	33	141

Index of Bridge Drawings	
Sheet No.	Drawing
33-34	General Notes and Quantities
35	Contour Map
36	Construction Layout
37	Engineering Geology
38-39	Construction Phasing
40-42	Abutment Details
43	Abutment Drainage Details
44	Drilled Shaft Details
45-47	Pier Details
48	Framing Plan
49	Beam Details
50	Bolted Field Splice Details
51	Steel Erection, Fit Up, and Bolting Procedures
52	Diaphragm Details
53	Abutment Frame Details
54	Bearing Device Details
55	Superstructure Details
56-59	Slab Details
60-61	Corral Rail Details
62-63	Bill of Reinforcing Steel and Bending Diagrams
Standards	
64	Bridge Excavation
65	Standard Pile Details
66	Supports and Spacers for Reinforcing Steel

DESIGN DATA

DESIGN SPECIFICATIONS: AASHTO Specifications, 9th Edition. Load and Resistance Factor Design

DESIGN LOADING: HL-93

Design Dead Load includes an allowance of 15 psf for a future wearing surface.

UNIT STRESSES:  
Concrete (Grade 4.0) f'c = 4 ksi  
Concrete (Grade 4.0)(AE) f'c = 4 ksi  
Concrete (Grade 4.0)(AE)(SW) f'c = 4 ksi  
Reinforcing Steel (Grade 60) fy = 60 ksi  
Structural Steel (M270 Gr. 50WT 3) fy = 50 ksi  
Structural Steel (A709 Gr. 50W) Fy = 50 ksi  
Structural Steel (A709 Gr. 50) Fy = 50 ksi  
Steel Pile (Grade 50) Fy = 50 ksi

LRFD DESIGN PILE LOAD:  
Design Loading (Tons/Pile) Strength I Service I Phi  
Abutment No. 1 & 2 68.5 46.7 0.60

LRFD DESIGN DRILLED SHAFT LOAD:  
Design Loading (Tons/Shaft) Strength I Service I Phi  
All Piers 305.9 211.3 0.50

3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) Sta. 62+23.30				
GENERAL NOTES AND QUANTITIES				
K-63 over Turkey Creek				
Proj. 63-66 KA-5729-01			Nemaha Co.	
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.
			JAB	CADD CK.
				JAB



PlotTted: J:\ke.P\farmensfield\eks.gdb\Plot Location: Bridge  
File: ka572901bbr0048-03.dgn  
Plot Date: 15-JUL-2024 15:59

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	34	141

GENERAL NOTES

REINFORCING STEEL: All reinforcing steel dimensions are to the centerline of the bars unless otherwise noted. All reinforcing steel, except the spiral bars, shall conform to the requirements of ASTM A615, Grade 60. Spiral bars may meet the requirements of either ASTM A615 (Gr. 40 or 60) or AASHTO M32, and are included in the bid item "Reinforcing Steel (Grade 60)".

STRUCTURAL STEEL: The rolled beams, flange splice plates, and web splice plates shall meet AASHTO M270 Gr. 50W T3 requirements. All other structural steel shall meet ASTM 709 Gr. 50W, unless noted otherwise. Shop and Field Splices shall be made only where shown on the Contract Plans as a "splice" or as an "optional splice." Elimination of any "splice" may be requested.

SHOP DETAILS: Reference blocking diagrams on the shop details to a level line running the entire length of the girder.

WELDING: Material, Fabrication and Construction shall conform to KDOT Specifications. On the shop drawings, show a code or symbol at the tail of the weld symbol, which refers to an approved prequalified weld procedure.

WELDED STUD SHEAR CONNECTORS: Weld Shear Stud Connectors with automatically timed stud welding equipment connected to a suitable power source. All stud welding shall conform to KDOT Specifications.

ERECTION: Bring each line of girders to the correct line, grade (or relative grade) and camber, and secure in place prior to connection of the girder field splices.

ERECTION PLANS: This is a Category B Structure. Submit detailed Erection Plans to the State Bridge Office (or Bureau of Local Projects) at least 4 weeks before beginning the erection process. Portions of the submitted details shall bear the seal of a licensed Professional Engineer. Identify, on the Erection Plans, the Erection Supervisor required by KDOT Specifications. No structural erection work will begin without approved erection plans.

ERECTION ELEVATION CHECKS: After the abutment and pier beams have been erected and before setting any structural steel, present verification to the Engineer that the elevations at the bearings match plan elevation ( $\pm 1/4$ "). Present verification to the Engineer that the elevations at all field splice locations match the elevations ( $\pm 1/2$ ") in the plans before any connection is fully tightened. (For steel girders that are blocked on the ground, fully tighten the bolted connections prior to erection.)

FALSEWORK PLANS AND SHOP DRAWINGS: Use the U.S. Customary system of units on falsework plans and shop drawing details.

FALSEWORK PLANS: A licensed Professional Engineer shall design the falsework details. Details shall bear the seal of a licensed Professional Engineer. Submit electronic plans conforming to Section 105 of the Standard Specification with details in compliance with KDOT Specifications to the Field Engineer for review.

FALSEWORK INSPECTION: This project has falsework plan requirements which are considered "Category 2" by KDOT specifications. If falsework deficiencies or variations from the approved and sealed plans are found, the falsework design Engineer of Record will provide written approval of the changes. If for the convenience of the Contractor the falsework becomes "Category 1" by the use of non-typical supports; then the inspection and review requirement of "Category 1" will be fully enforced, but at no cost to the State. "Category 2" falsework inspection is not paid for directly, but is subsidiary to other bid items.

BOLTED CONNECTIONS: Girder Connections: Use  $7/8$ " diameter heavy hex structural bolts for the main member connections. Use  $15/16$ " diameter bolt holes. Do not ream during field erection. Accurately align all connections by driving  $15/16$ " diameter drift pins in all corners and in  $1/4$  of the remaining holes in each plate. See KDOT Specifications.

Secondary Member Connections: Use  $3/4$ " diameter heavy hex structural bolts for the secondary member connections. Use  $13/16$ " diameter bolt holes. Oversized and/or slotted holes, as specified in the KDOT Specifications, may be used in only one of the two members connected and must be shown in the approved shop drawings. Oversized and/or slotted holes may require additional standard hardened washers or plate washers. Report to the Engineer prior to any required field reaming that will remove more than  $1/4$ " of material from one ply of the connected parts.

Use Direct Tension Indicators (DTIs) on all high strength bolts. Place the DTI under the bolt head and turn the nut to tighten. This method is preferred whenever possible. Face the protrusions on the DTI to the underside of the bolt head. Place a hardened flat washer under the nut. See KDOT Specifications.

BOLTS: All bolts, nuts and hardened flat washers shall conform to the heavy hex structural requirements of ASTM A325, Type 3, and KDOT Specifications unless otherwise noted. Direct Tension Indicators (DTIs) are to comply with the requirements of the latest edition of ASTM F959. No allowance will be made for high strength bolts used for permanent or temporary connections. This work is subsidiary to the bid item, "Structural Steel". The number of bolts is shown for the convenience of the Contractor.

FABRICATION OF FIELD SPLICES: Prepare joints for the field splices in accordance with KDOT Specifications. Use Type "B" shop laydown.

CONSTRUCTION LOADS: Limited traffic is permitted on the new sub-deck, one-course deck or any concrete overlay during the curing period, keep any exposed deck wet during the curing period. See KDOT Specifications Section 710 Tables 710-1 & 710-2 for additional information.

CONSTRUCTION JOINTS: The construction joints shown are optional with the Contractor, except where noted as required. If used, place the construction joints only at locations shown or at locations approved by the Engineer.

CONCRETE: Superstructure concrete is bid as Concrete (Grade 4.0)(AE)(SW). Substructure concrete is bid as Concrete (Grade 4.0)(AE). If desired, the Contractor may use Concrete (Grade 4.0) in the footings and in the abutments below the construction joint. Bevel all exposed edges of all concrete with a  $3/4$ " triangular molding, except where noted on the plans.

CONCRETE PLACING: Place and hand vibrate all concrete for the abutment above the construction joint to the bottom of deck elevation just prior to the normal paving train operations. Do this work in a manner to avoid cold joints in either the slab or in the abutment.

PREFORMED ANCHOR BOLT HOLES: Preform 3 inch diameter holes using only corrugated polyethylene tubing (Type C) at the locations shown. When temperatures are expected to go below freezing, seal the preformed holes or fill them with a propylene glycol-based antifreeze to prevent expansion damage. The holes will be free of water, antifreeze or other foreign materials at the time of grouting the anchor bolts. The polyethylene tubing may remain in-place. Trim the tubing flush with the top of the concrete. This work shall be subsidiary to Concrete Grade 4.0 (AE).

FILLETS: Construct the finished deck to plan grade by varying the depth of the fillet over the beam to provide for beam profile, concrete dead load deflection and, if necessary, vertical curvature. After the beams are completely erected and the falsework bents are removed, profile each beam. Correct any variation between the actual profile and the concrete dead load deflection shown in the plans by varying the depth of the concrete fillets over the beams so that the finished floor is constructed to the theoretical grade. The minimum depth of the slab over the beam shall be  $10 1/2$ ".

The theoretical amount of concrete required for the fillets is 7.9 C.Y. This amount of concrete is included in the Summary of Quantities. Any additional concrete required to construct the fillets will be subsidiary.

CONCRETE PLACING SEQUENCE: The sequence of placing concrete in the slab and curbs shall be as shown, or the Contractor may submit an alternate placing sequence for review. Submit the alternate placing sequence to the Engineer at the Preconstruction Conference. Include the proposed rate of concrete placement in C.Y./h, the plant capacity, placement direction, construction joint location, a description of the equipment used in placing the concrete, proposed admixtures, and the quantity of concrete in each placing segment. Any additional cost for the Contractor's alternate plan of placing concrete, including admixtures, shall be at the Contractor's expense and shall be considered subsidiary to the bid item, "Concrete (Grade 4.0)(AE)(SW)". Approval of the Contractor's alternate sequence is required prior to placement of concrete in the deck.

TEMPORARY SHORING: The bid item "Temporary Shoring" includes all labor and material necessary to furnish shoring at the location shown on the plans for the temporary bracing of the embankment during excavation. Maintain the temporary shoring until the Engineer authorizes its removal. The temporary shoring plans are to be designed and sealed by a registered Professional Engineer. Submit design calculations and shoring plans to the Field Engineer for review 6 weeks before work is scheduled to begin. Work shall not begin until the Engineer grants approval.

CAUSEWAY: If the Contractor chooses to build a causeway for bridge construction purposes, the Contractor shall obtain any required U.S. Army Corps of Engineers, Section 404 permit, Kansas State Board of Agriculture-Division of Water Resources Permit, Kansas Department of Health and Environment Section 401 Permit, Kansas Department of Wildlife and Parks Endangered Species Permit, or any other permit required by law for causeway construction. Obtain the permit in a timely manner so as not to delay the completion of the project.

DEMOLITION PLANS: This is a Category B Demolition. Submit detailed Demolition Plans to the Field Engineer at least 2 weeks before the demolition meeting. Identify, on the plans, the Demolition Supervisor meeting the requirements of the KDOT Specifications. No Demolition work will begin without approved Demolition Plans. A Licensed Professional Engineer is not required.

BROKEN CONCRETE: Waste the broken concrete from the existing bridge on sites provided by the Contractor and approved by the Engineer.

EXISTING STRUCTURE: Plans of the existing structure are on file and available for inspection by qualified bidders at the State Bridge Office, KDOT, Eisenhower State Office Building, 700 SW Harrison, Topeka, KS.

TEMPERATURE: The design temperature for all dimensions is 60°F.

DIMENSIONS: All dimensions shown on the design plans are horizontal dimensions unless otherwise noted. Make necessary allowances for roadway grade and cross slope.

SLOPE PROTECTION (Riprap Stone): Place Slope Protection (Riprap Stone) to the limits and thicknesses shown on the plans or as directed by the Engineer. Use 200 Lb. Gradation as described in Division 1100.

Place a 10 foot wide mat of geotextile under the rock/rubble embankment on the berm and berm slopes and centered on the drip lines of the slab.

COLUMN CONSTRUCTION: Cure the drilled shaft footing as required by the KDOT Specifications before beginning the column construction (placing resteel or formwork). Do not place cast in place shear bolts, coil inserts or other devices used as falsework support in the column without the approval of the Engineer. Do not remove the column formwork without the approval of the Engineer. Curing shall continue after the formwork is removed as required by the KDOT Specifications.

PIER BEAM CONSTRUCTION: Cure the columns as required by the KDOT Specifications before beginning the pier beam construction (placing resteel or formwork). Do not drill and grout bolts or other devices into the columns used for falsework support unless shown on the plans. Cure the columns as required by the KDOT Specifications before placing pier beam concrete. Do not remove falsework used to support the pier beam until the pier beam concrete has cured as required by the KDOT Specifications. Do not set girders or beams on the pier beam until after the falsework is removed or the pier beam concrete has 0.75f'c strength as tested.

3						
2						
1						
NO.	DATE	REVISIONS			BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION						
Br. No. 63-66-54.90 (048) Sta. 62+23.30						
GENERAL NOTES						
K-63 over Turkey Creek						
Proj. 63-66 KA-5729-01					Nemaha Co.	
SHEET NO.	OF	SCALE		APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB	CADD
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB	CADD CK.
						KMS
						JAB

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	35	141

P.O.T. @ Sta. 57+00.00  
N 815,220.459 E 10,415,033.158  
1. Set ½" rebar 0.3' deep  
2. ½" rebar w/KDOT cap (TP 100)  
3. Mag nail & KDOT wshr. S face power pole  
4. TW of K-63 highway

31.0' SW  
83.5' NW  
0.3' E

P.C. @ Sta. 66+51.38  
N 816,163.030 E 10,414,904.108  
1. Found ½" rebar  
2. Conc. nail & KDOT wshr. N face power pole  
3. @ top N end CMP  
4. TW K-63 highway

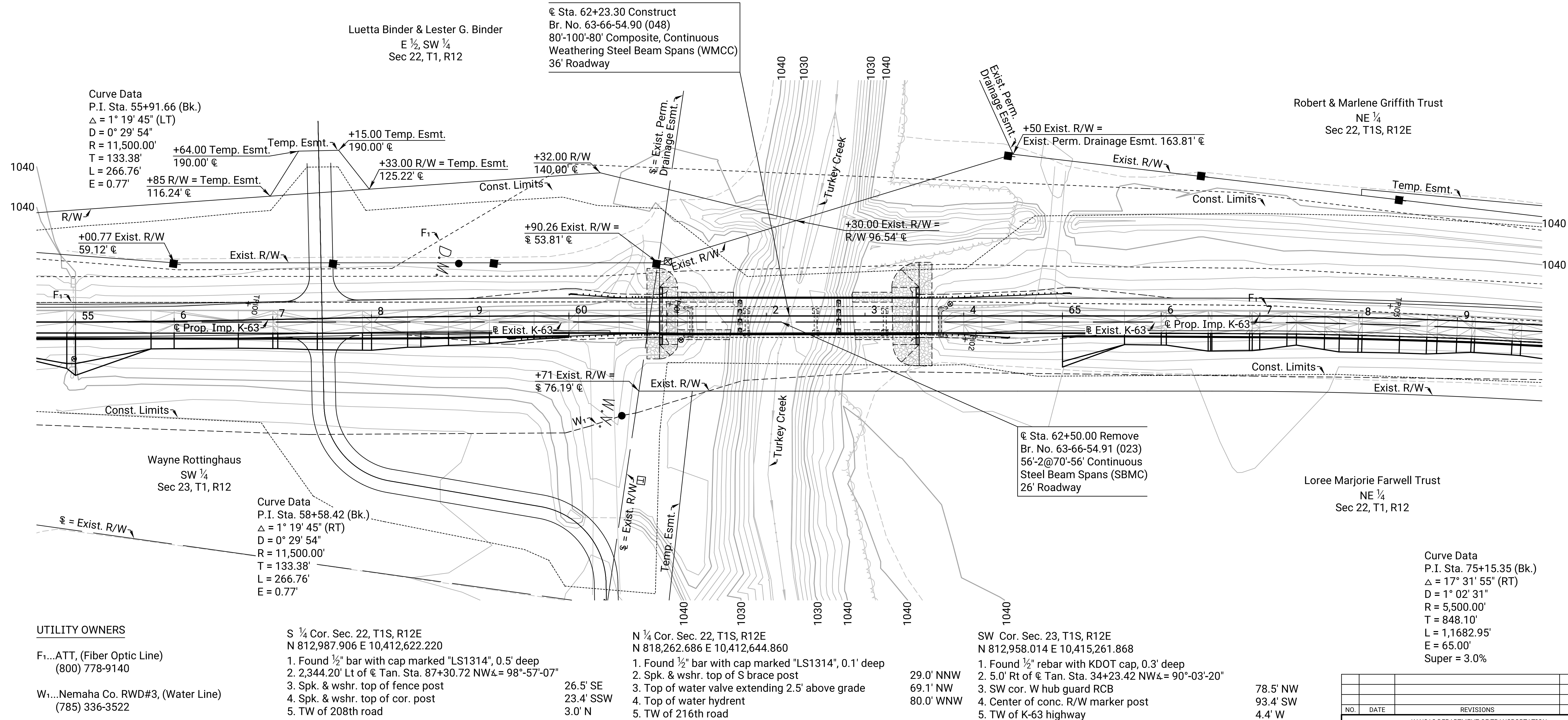
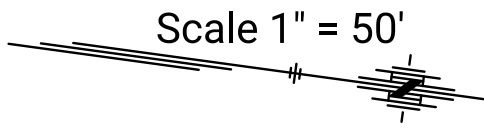
148.7' W  
173.8' SW  
0.3' E

Transverse Point #101  
N 815,616.68 E 10,414,960.44  
1. Set ½" rebar w/ KDOT cap, flush  
2. Nail & KDOT wshr. top guardrail post  
3. SW cor. W hubguard bridge  
4. TW K-63 highway

17.6' SSE  
11.0' N  
18.5' E

Transverse Point #102  
N 815,917.80 E 10,414,955.58  
1. Set ½" rebar w/ KDOT cap, flush  
2. Nail & KDOT wshr. top guardrail post  
3. NE cor. E hubguard bridge  
4. TW K-63 highway

25.8' NNW  
15.0' S  
18.0' E



HISTORIC HIGH WATER INFORMATION

William Holthaus, a KDOT employee with 22 years of knowledge of the project site, says water has not overtopped the roadway, but has been witnessed at pavement edge down stream of the bridge, north of the project near 216th road.  
Elev. = 1047.60

Sea Level Datum NAVD88

NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048)			Sta. 62+23.30	
CONTOUR MAP				
K-63 over Turkey Creek				
Proj. 63-66 KA-5729-01			Nemaha Co.	
DESIGNED	DETAILED	QUANTITIES	CADD	
DESIGN CK.	DETAIL CK.	QUAN CK.	CADD CK.	

CADconform Certify This File

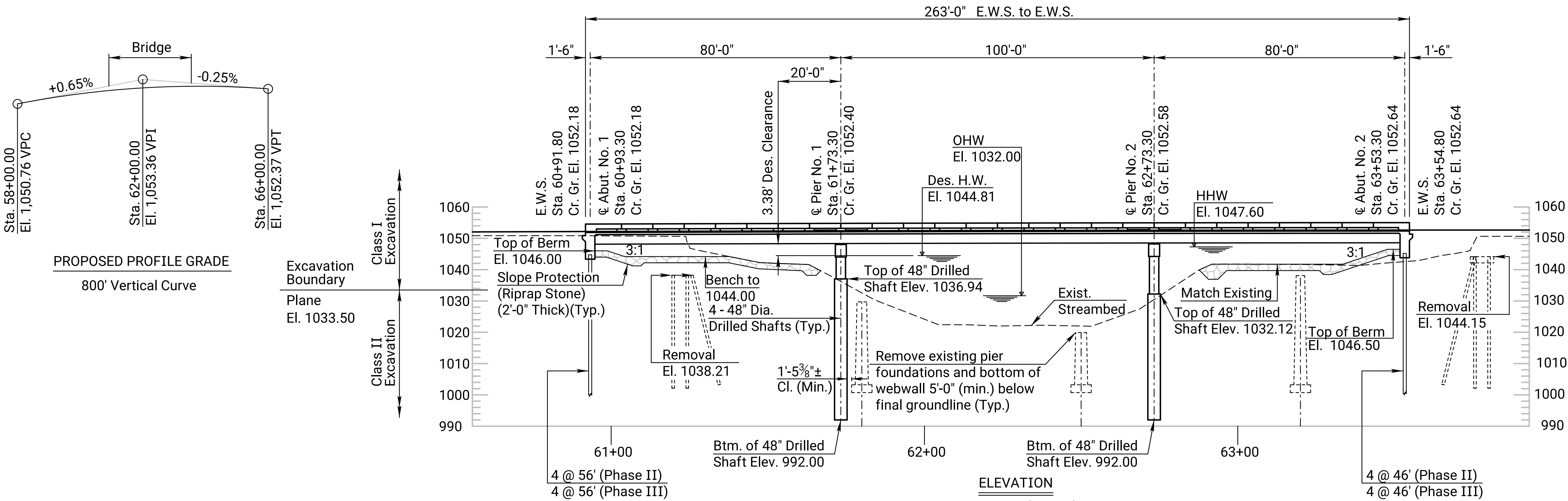
Sh. No. 35



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	36	141

DRAINAGE DATA

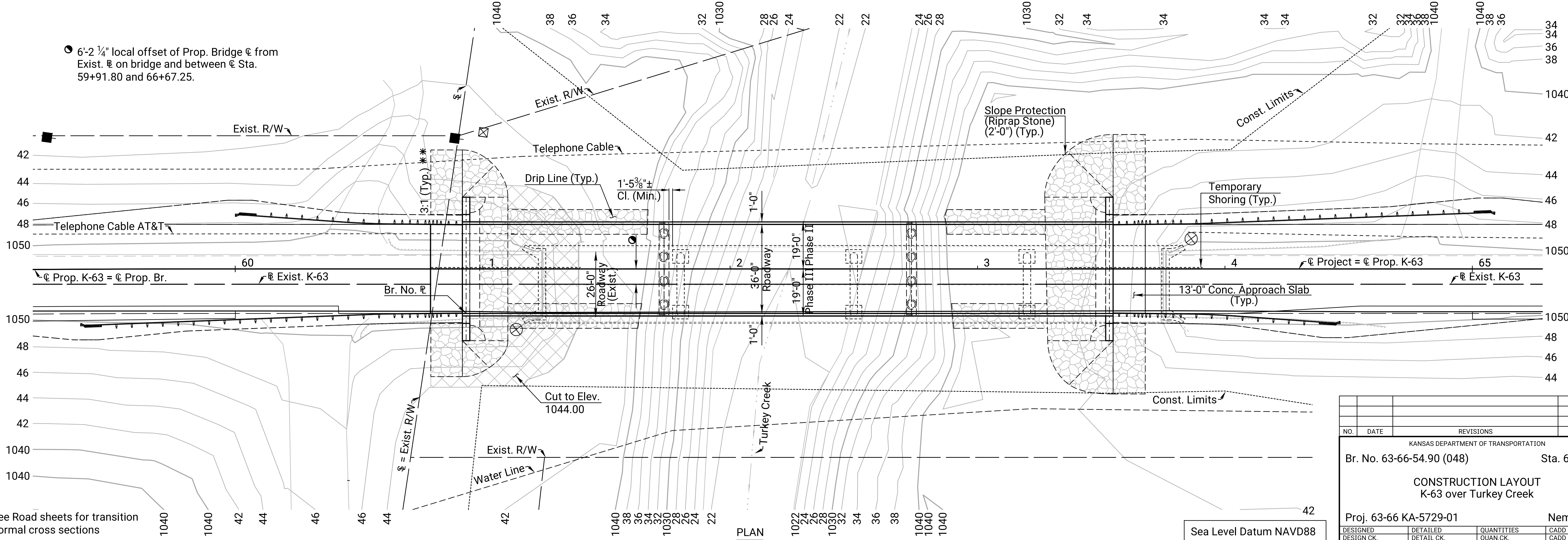
Drainage Area	288.0	sq. mi.
Design Frequency	50	ysrs
Design Discharge (Q50)	24,200	cfs
Design High Water Elevation	1044.81	
Change In Design Backwater	-0.66	ft.
Design Backwater Elevation	1045.31	ft.
Overtopping Elevation (Sta. 55+30)	1049.26	
Overtopping Discharge	65,000	cfs
Overtopping Frequency	>100	ysrs
Discharge at Q100	29,600	cfs
Change in Backwater at Q100	-1.19	ft.
Backwater Elevation at Q100	1045.70	
Historic High Water Elevation	1047.60	
Ordinary High Water Elevation	1032.00	
Total Waterway Provided	3,963	sq. ft.
Design Waterway Provided	3,159	sq. ft.
Estimated Ordinary High Water Discharge	1,853	cfs
Change in Waterway Opening	+ 936	sq. ft.



B.M. #12 Square cut S end E hubguard of bridge  
18.3' Rt. of @ Sta. 61+13.5 Elev. = 1,051.25'

B.M. #13 Square cut NW cor. W hubguard of bridge  
18.3' Lt. of @ Sta. 63+86.3 Elev. = 1,051.29'

Scale: 1" = 20 ft.



\*\*See Road sheets for transition to normal cross sections

NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048)			Sta. 62+23.30	
CONSTRUCTION LAYOUT K-63 over Turkey Creek				
Proj. 63-66 KA-5729-01			Nemaha Co.	
DESIGNED DESIGN CK.	DETAILED DETAIL CK.	QUANTITIES QUAN. CK.	CADD CADD CK.	

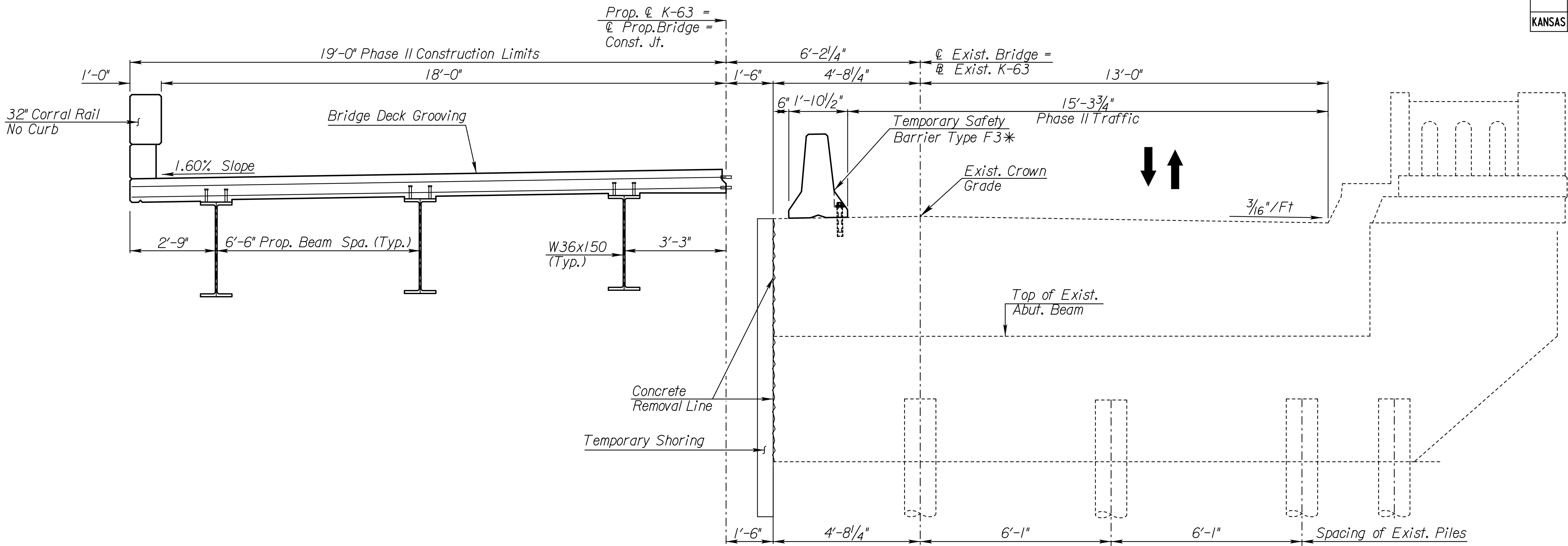
CADconform Certify This File

Sh. No. 36

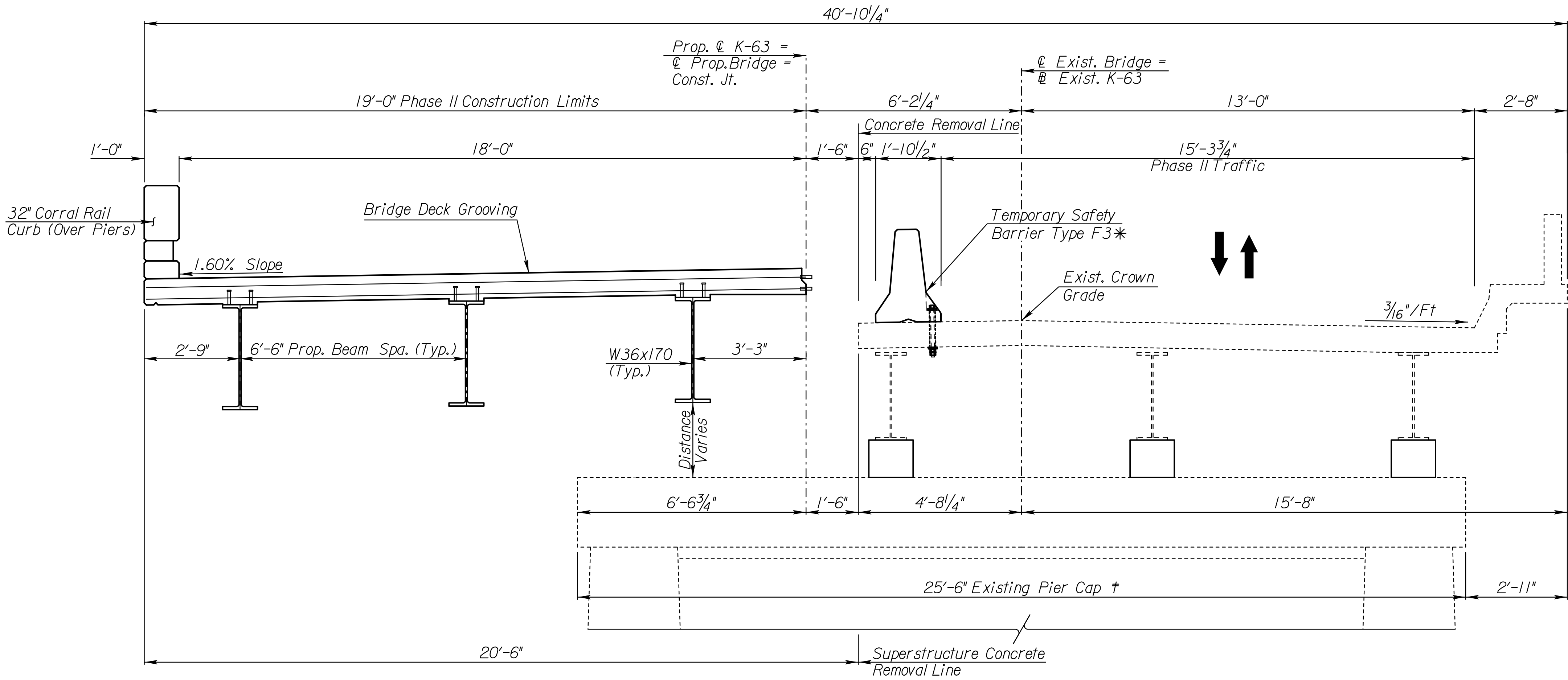




STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	38	141



TYPICAL SECTION AT EXISTING ABUTMENTS DURING PHASE II  
(Abut. No. 1 Shown, Looking Upstation, Abut. No. 2 similar)

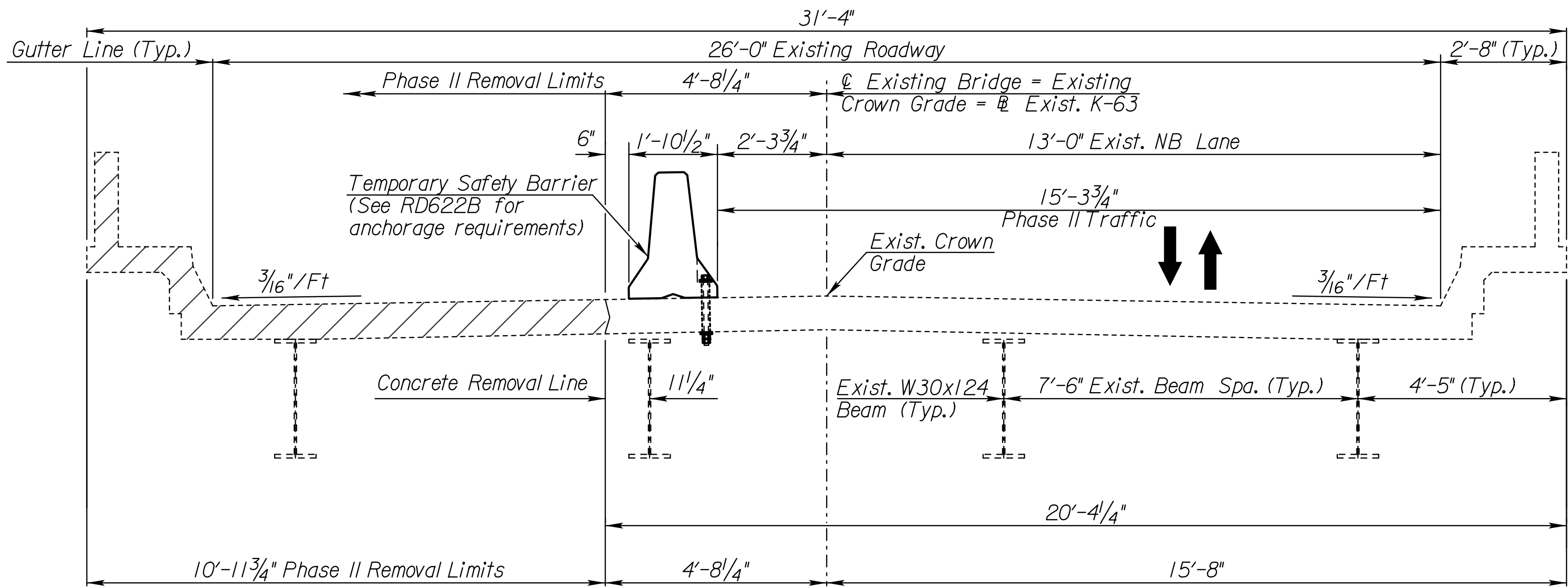


TYPICAL SECTION NEAR EXIST. PIER DURING PHASE II  
(Looking Upstation)

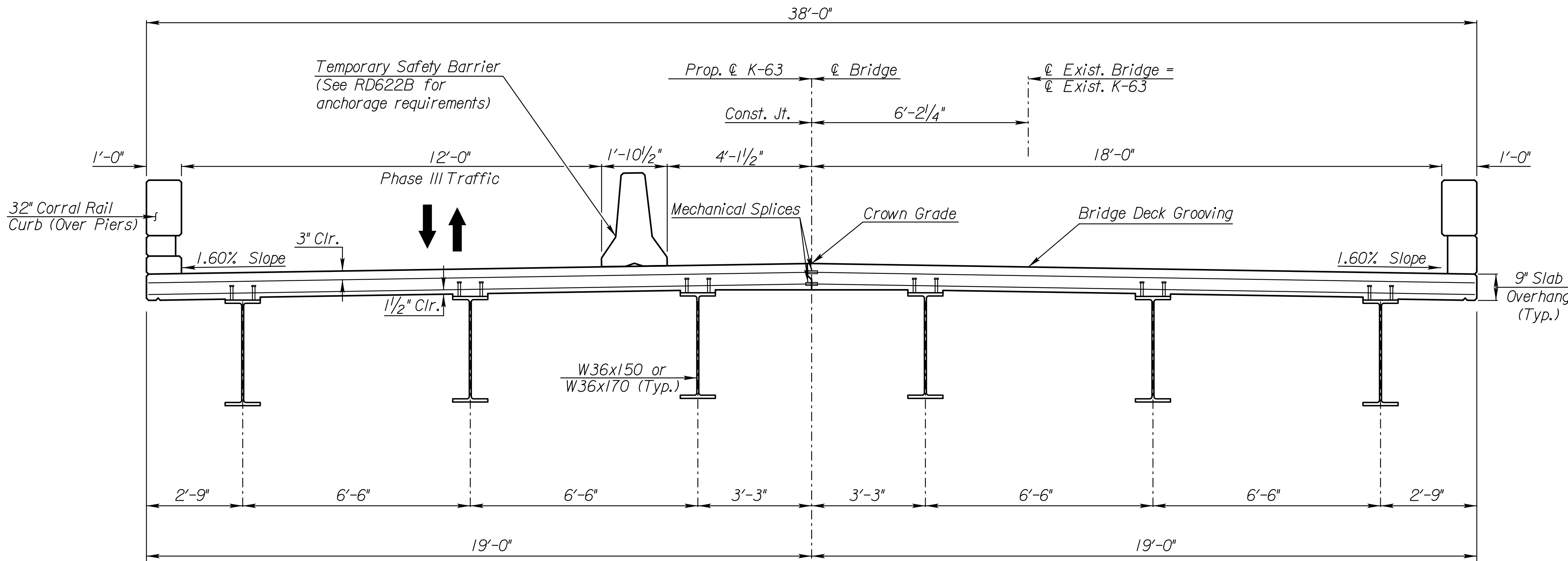
\* See RD622B for anchorage requirements  
† Demolish exist. pier during Phase III construction.  
See "Construction Layout" sheet for removal limit details

3					
2					
1					
NO.	DATE	REVISIONS			BY APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) Sta. 62+23.30					
PHASING DETAILS SHEET I					
K-63 over Turkey Creek					
Proj. 63-66 KA-5729-01				Nemaha Co.	
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB
				CADD	KMS
				JAB	JAB

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	39	141



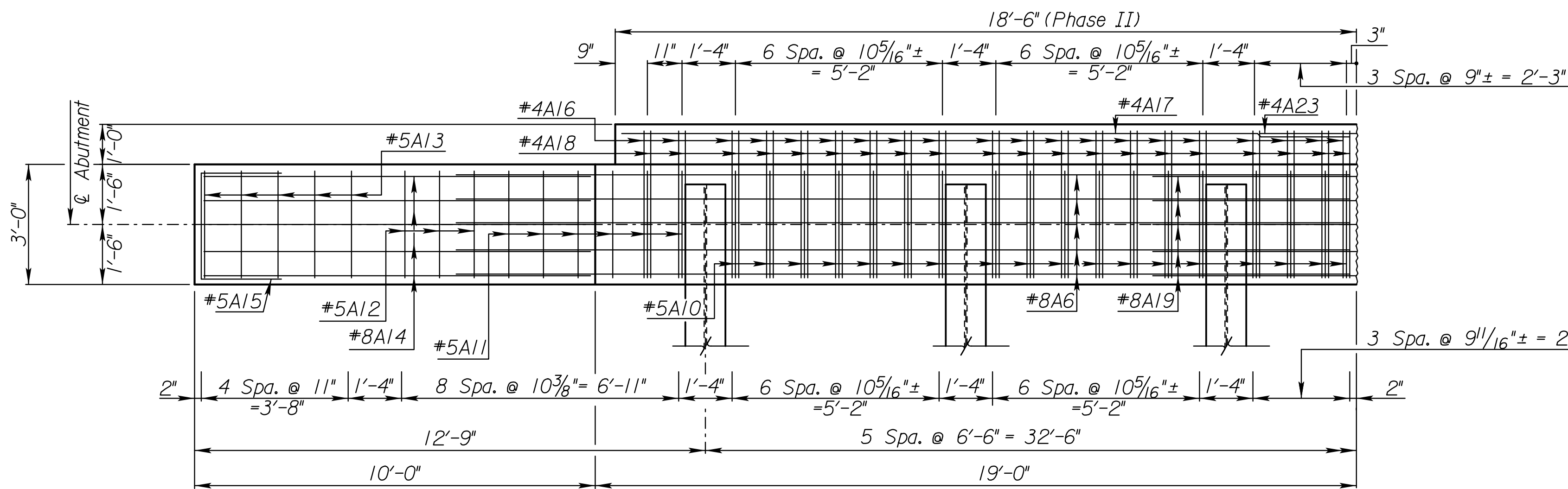
EXIST. BRIDGE TYPICAL SECTION DURING PHASE II  
(Looking Upstation)



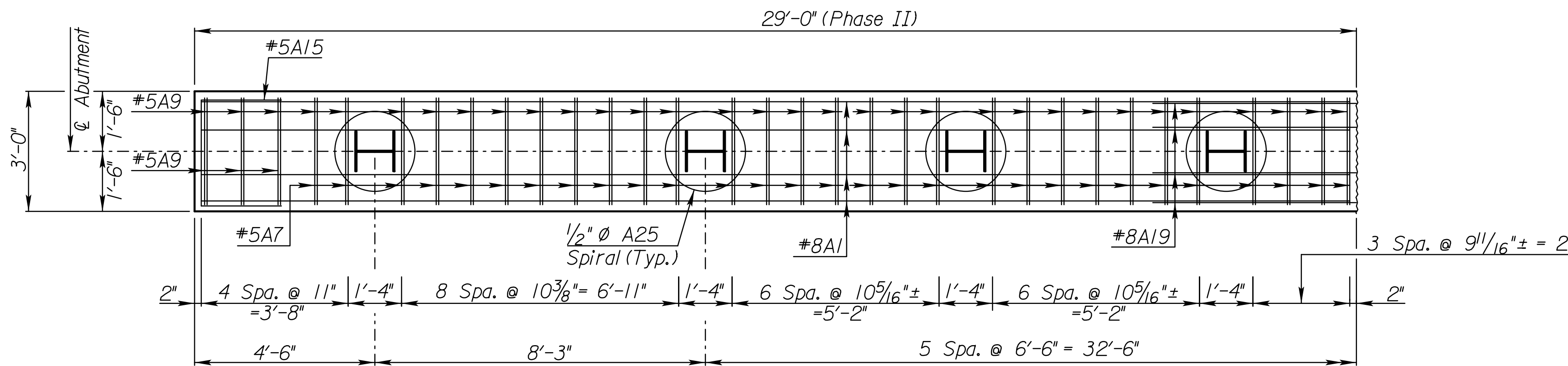
PROP. BRIDGE TYPICAL SECTION DURING PHASE III  
(Looking Upstation)

3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) Sta. 62+23.30					
PHASING DETAILS SHEET 2					
K-63 over Turkey Creek					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB

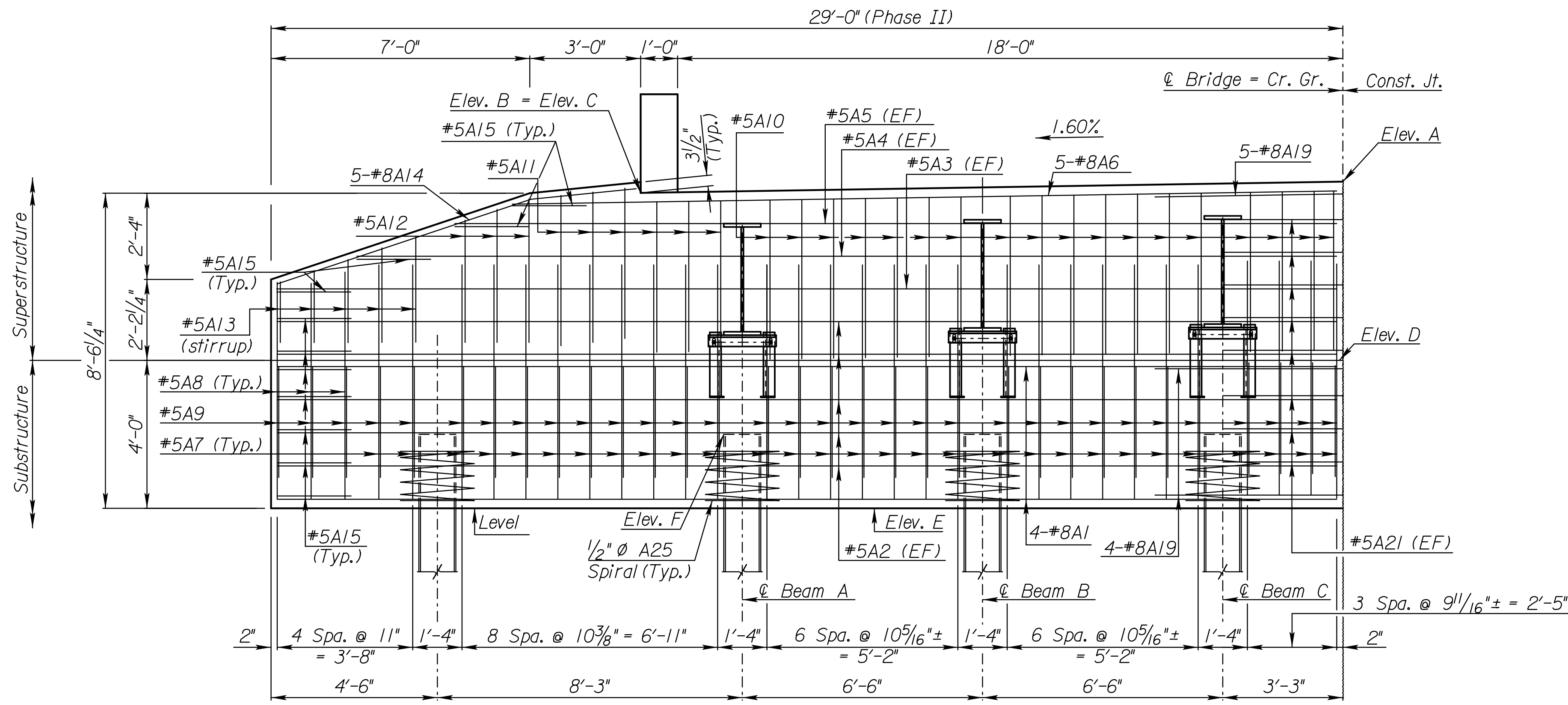
Plotted: J:\ke.P\pennstie\kgs.gdb\Plot Location: Bridge  
File: ka572901bbr0048-17.dgn  
Plot Date: 15-JUL-2024 15:59



TOP PLAN  
(Abutment #2 Shown, Abutment #1 Similar)



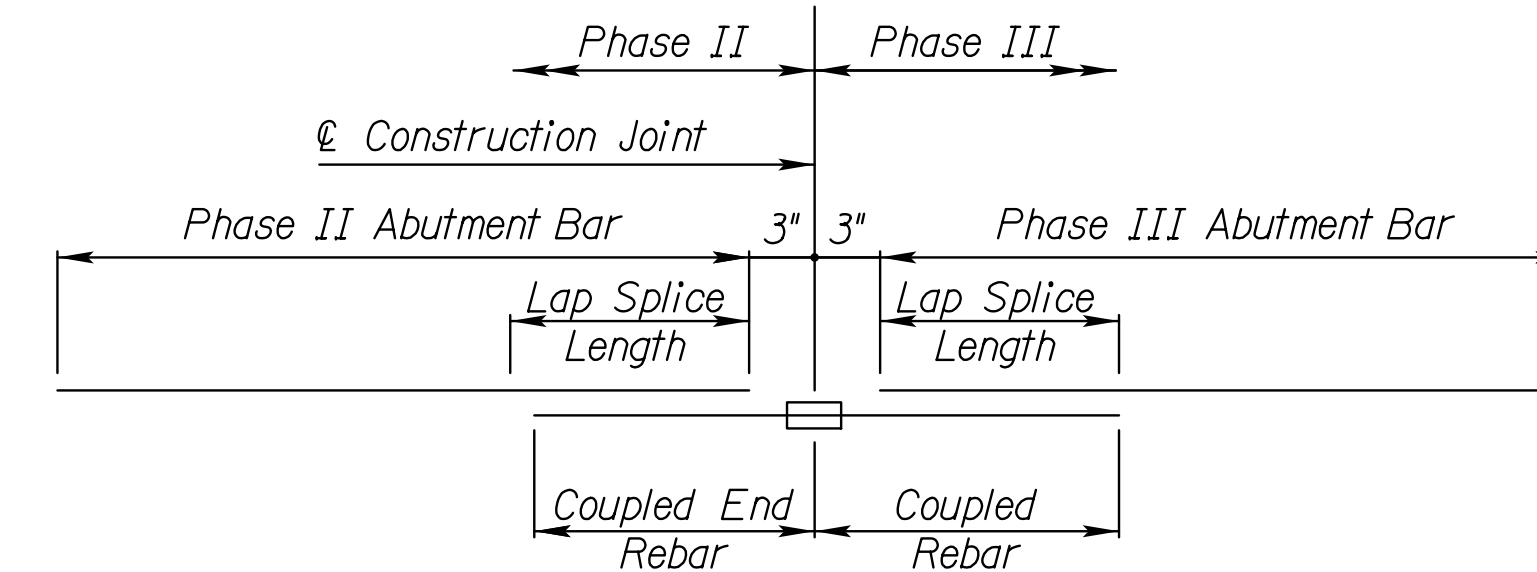
BOTTOM PLAN



ABUTMENT ELEVATION  
(Looking Upstation @ Abutment #2, Abutment #1 Similar)

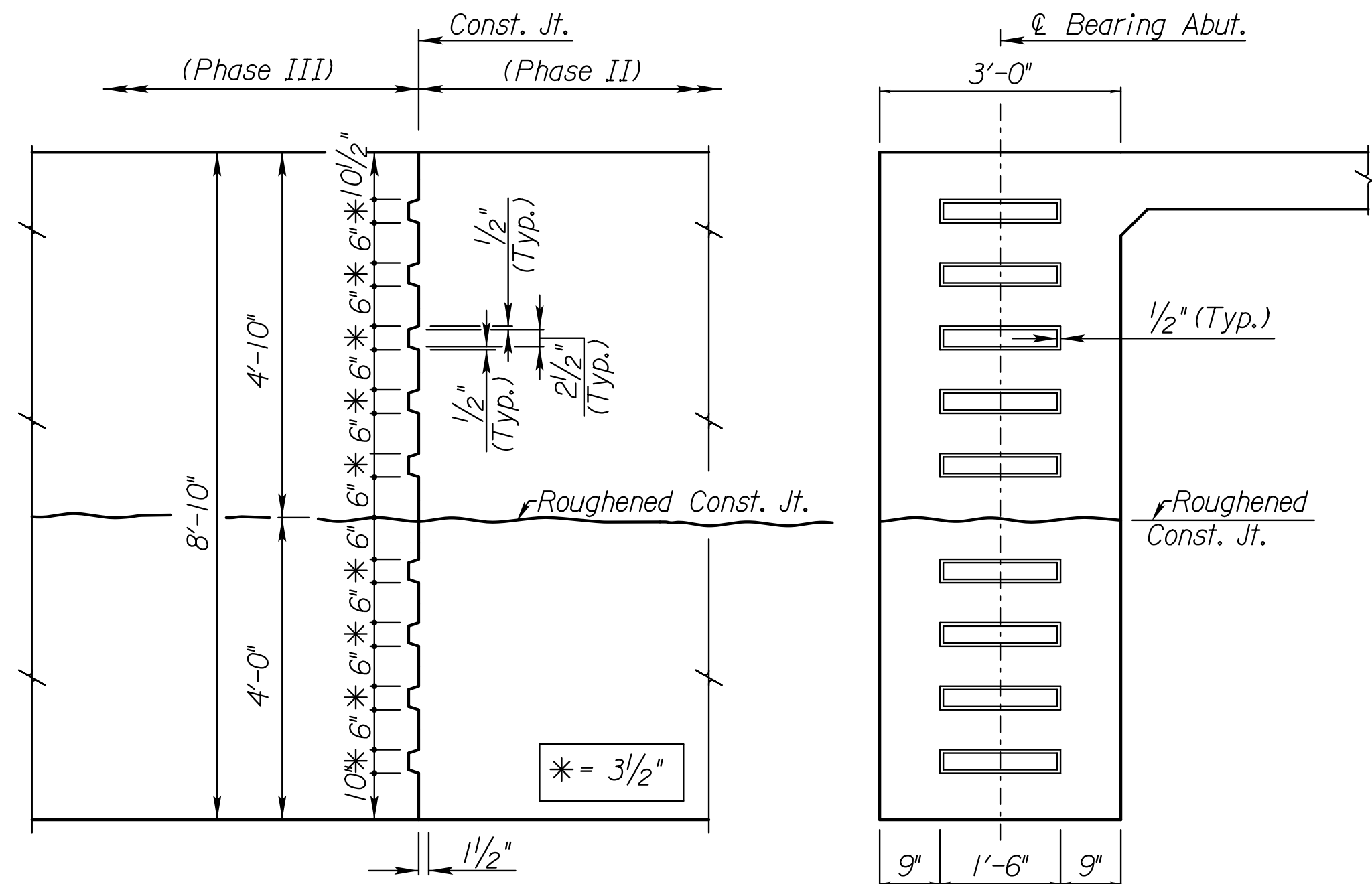
NOTE: Abutment bars are to be spliced from Phase II to Phase III utilizing mechanically spliced rebar. See the "Splice/Coupler Reinforcing Detail" below.

Mechanical splice bars not shown in Plan and Elevation views for clarity.



### SPLICE/COUPLER REINFORCING DETAIL

Note: See the "AUXILIARY ABUTMENT DETAILS" sheet for "Table of Abutment Reinforcing Splices".



### ELEVATION CONSTRUCTION JOINT

Legend  
EF= Each Face

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	40	141

ELEVATIONS		
	Abut. #1	Abut. #2
Elev. A	1052.18	1052.64
Elev. B	1051.88	1052.34
Elev. C	1051.88	1052.34
Elev. D	1047.35	1047.81
Elev. E	1043.35	1043.81
Elev. F	1045.35	1045.81

NOTE: Elevations are located along the Abutment C.

BRIDGE BEARING SEAT ELEVATION		
	Abut. #1	Abut. #2
Beam A	1048.06	1048.51
Beam B	1048.16	1048.62
Beam C	1048.27	1048.72

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) Sta. 62+23.30				
ABUTMENT DETAILS I				
(Phase II Construction)				
Proj. 63-66 KA-5729-01 Nemaha Co.				
SHEET NO. OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	41	141

ELEVATIONS		
	<i>Abut. #1</i>	<i>Abut. #2</i>
<i>Elev. A</i>	<i>1052.18</i>	<i>1052.64</i>
<i>Elev. B</i>	<i>1051.88</i>	<i>1052.34</i>
<i>Elev. C</i>	<i>1051.88</i>	<i>1052.34</i>
<i>Elev. D</i>	<i>1047.35</i>	<i>1047.81</i>
<i>Elev. E</i>	<i>1043.35</i>	<i>1043.81</i>
<i>Elev. F</i>	<i>1045.35</i>	<i>1045.81</i>

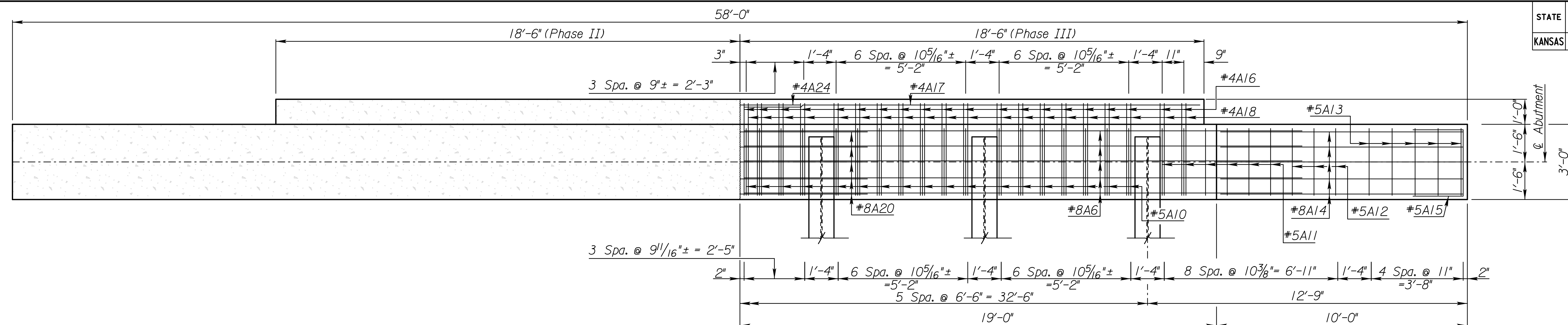
NOTE: Elevations are located  
along the Abutment C.

BRIDGE BEARING SEAT ELEVATION		
	<i>Abut. #1</i>	<i>Abut. #2</i>
<i>Beam D</i>	1048.27	1048.72
<i>Beam E</i>	1048.16	1048.62
<i>Beam F</i>	1048.06	1048.51

NOTE:  
See the "AUXILIARY ABUTMENT DETAILS"  
sheet for Abutment sections.

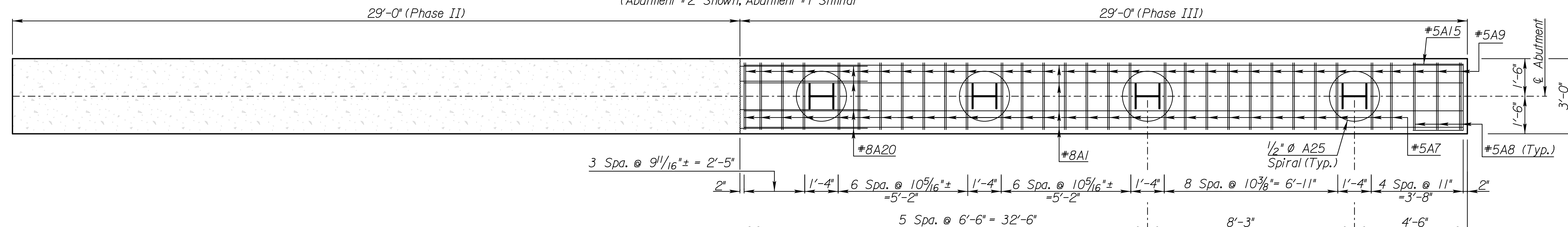
**NOTE:**  
Abutment bars are to be spliced from Phase II to Phase III utilizing mechanically spliced rebar. See the "ABUTMENT DETAILS 1" sheet for bar splice details.

Mechanical splice bars are not shown  
Plan and Elevation views for clarity.

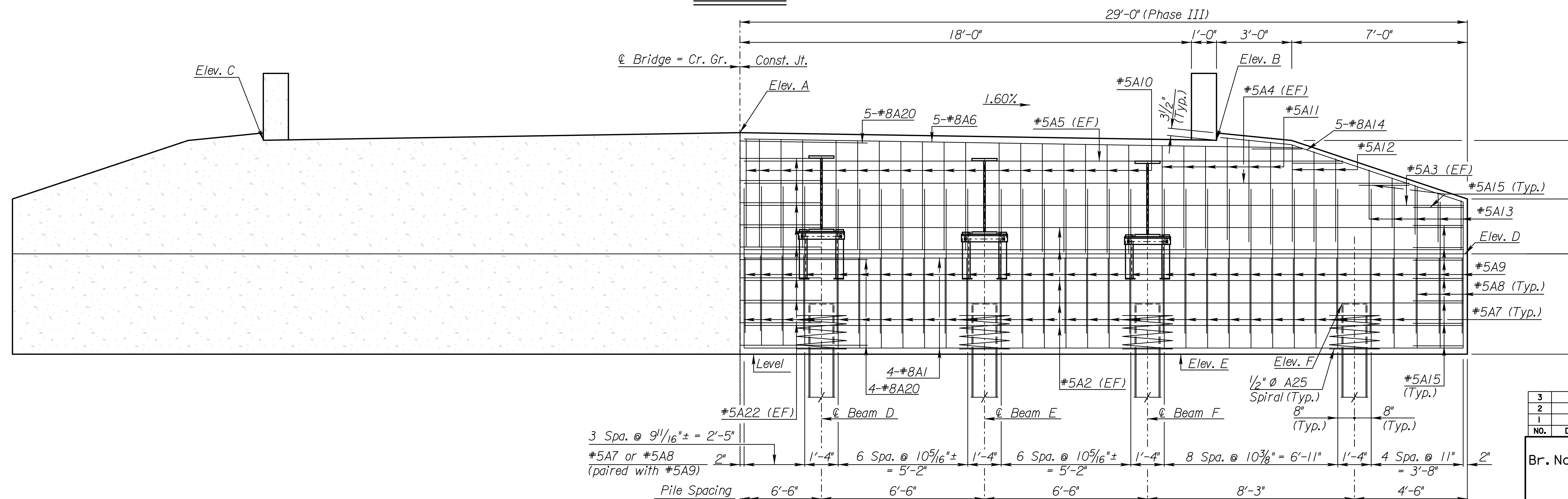


TOP PLAN

(Abutment #2 Shown, Abutment #1 Similar)



BOTTOM PLAN



ABUTMENT ELEVATION

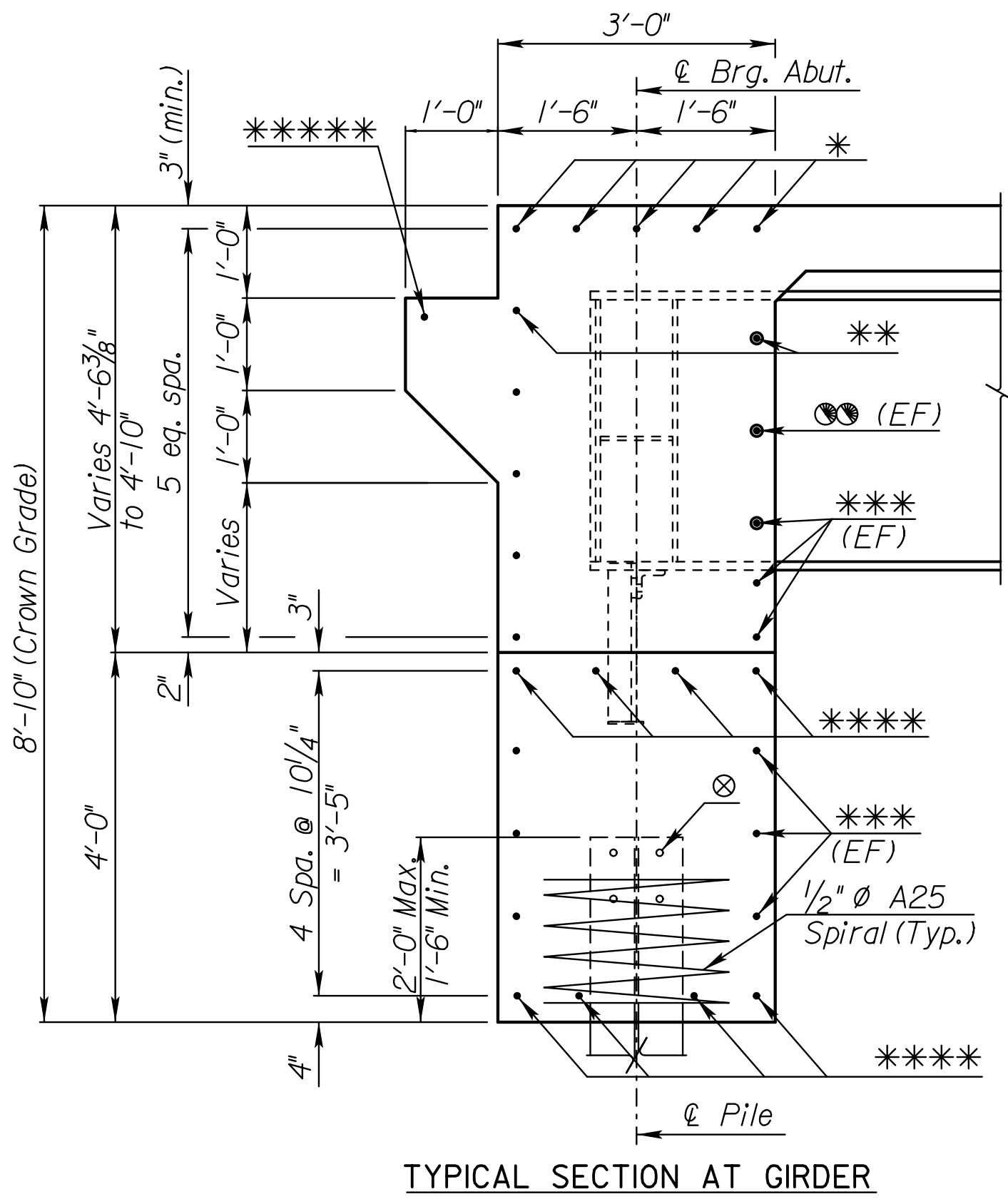
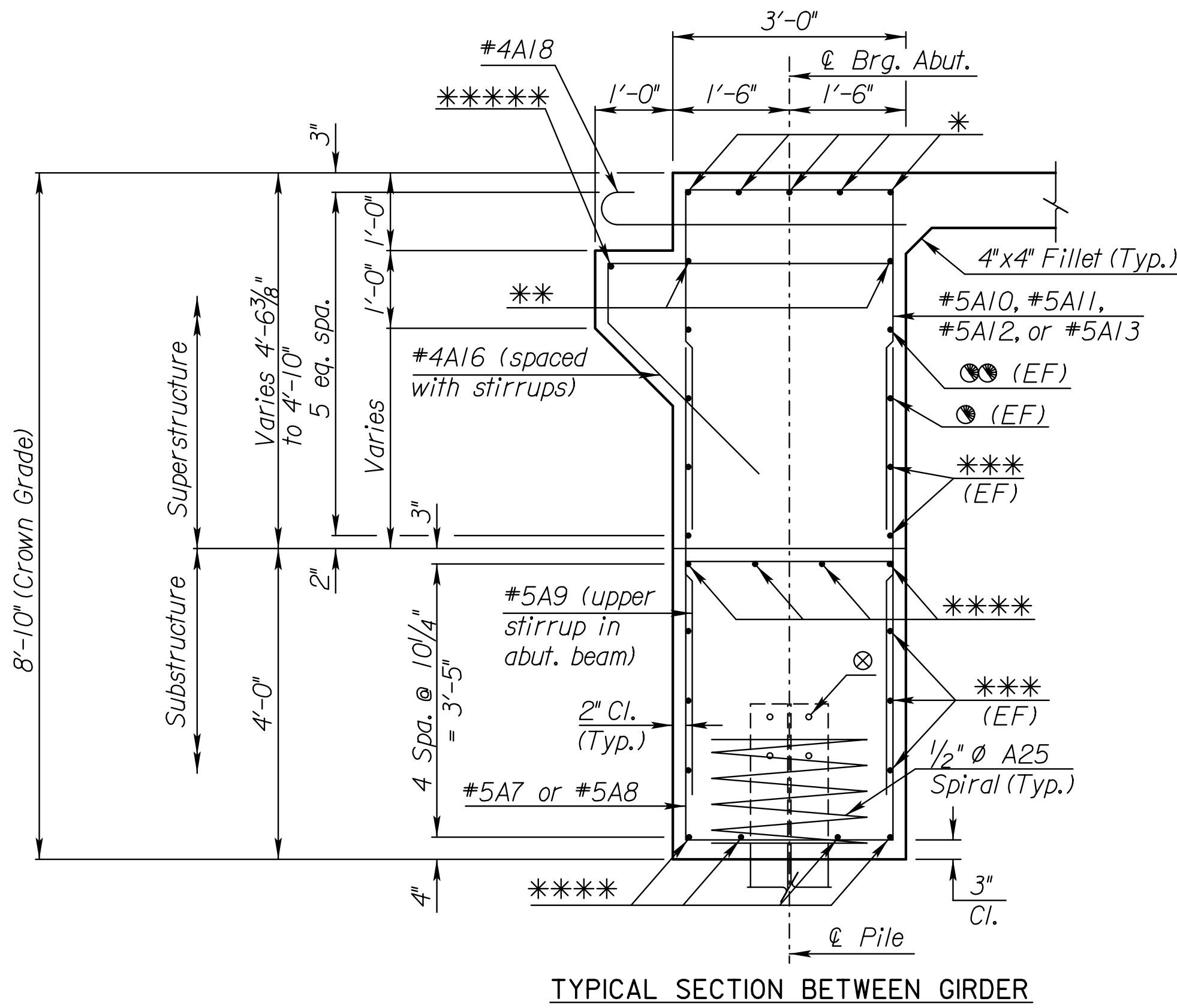
(Looking Upstation @ @ Abutment #2, Abutment #1 Similar)

3						
2						
1						
NO.	DATE	REVISIONS			BY	APP'D
<p align="center"><b>KANSAS DEPARTMENT OF TRANSPORTATION</b></p> <p align="center">Br. No. 63-66-54.90 (048) Sta. 62+23.30</p> <p align="center"><b>ABUTMENT DETAILS 2</b> (Phase III Construction)</p> <p align="center">Proj. 63-66 KA-5729-01</p> <p align="right">Nemaha Co.</p>						
SHEET NO.	OF	SCALE		APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB	CADD
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB	CADD CK.
						KMS
						JAB

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	42	141

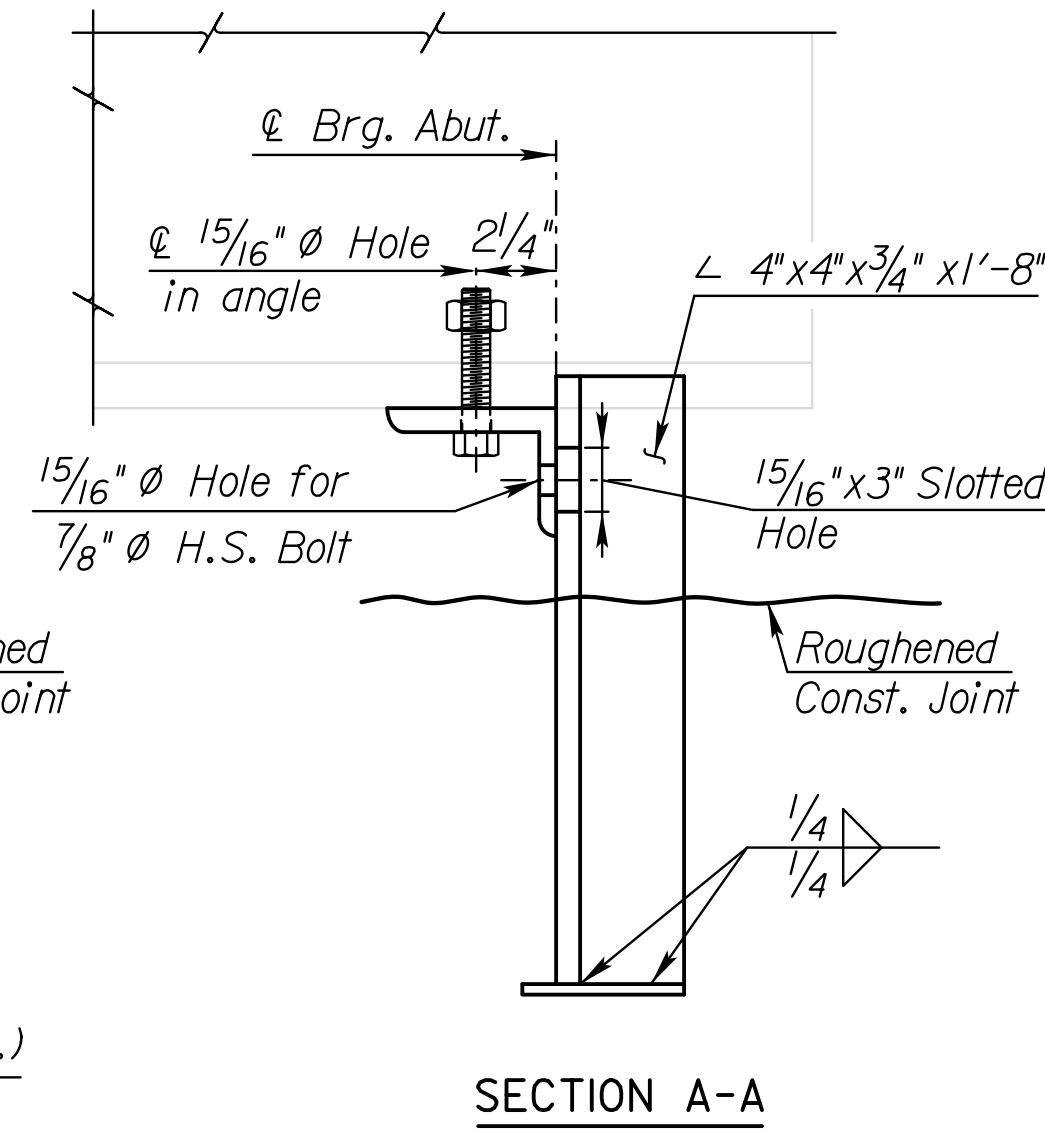
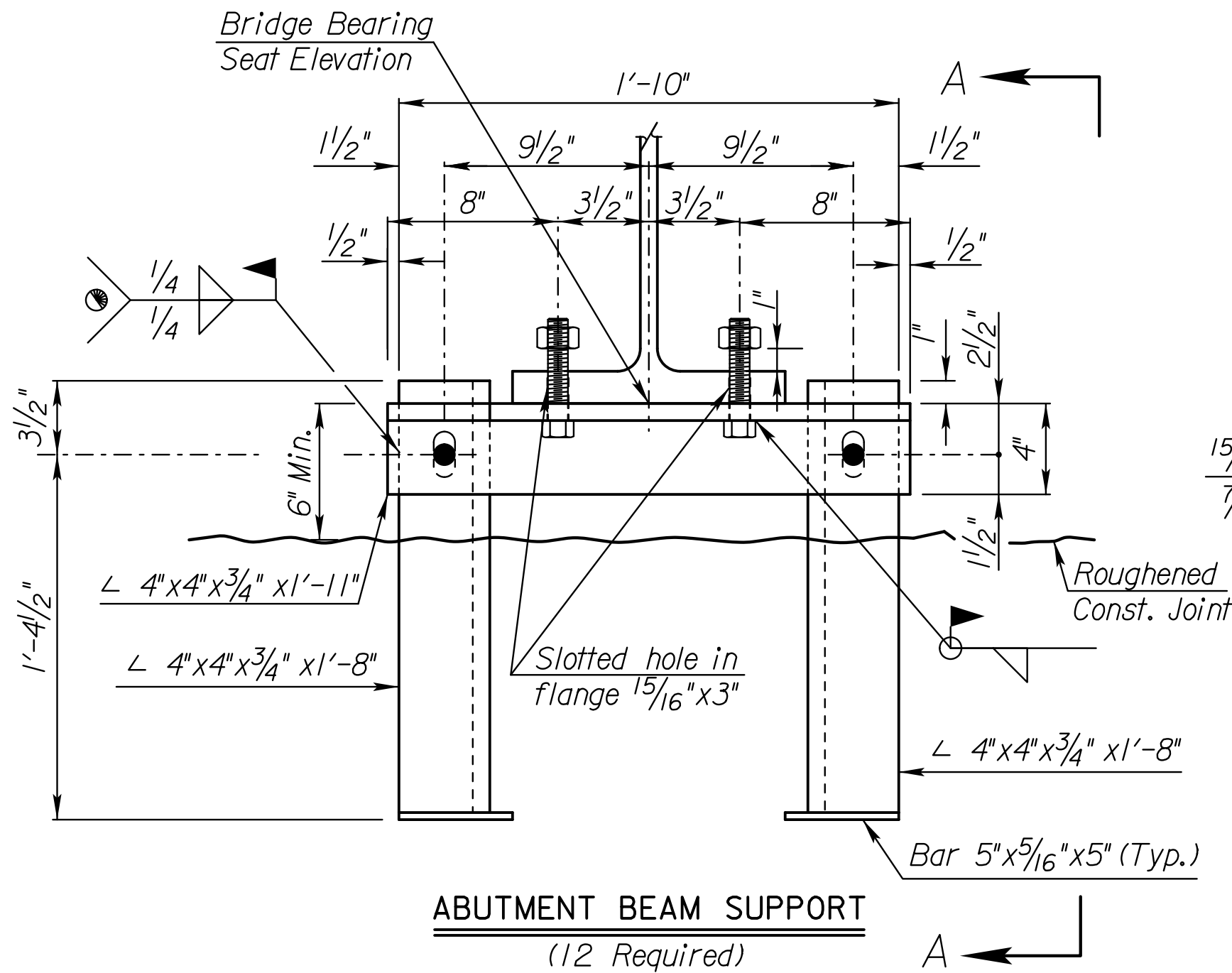
Symbol	Phase II	Phase II Splice	Phase III	Phase III Splice
⊙	#5A3	#5A2I	#5A3	#5A22
⊗	#5A4	#5A2I	#5A4	#5A22
*	#8A6	#8A19	#8A6	#8A20
**	#5A5	#5A2I	#5A5	#5A22
***	#5A2	#5A2I	#5A2	#5A22
****	#8A1	#8A19	#8A1	#8A20
*****	#4A17	#4A23	#4A17	#4A24

BRIDGE BEARING SEAT ELEVATION			
	Abut. #1	Abut. #2	
Beam A	1048.06	1048.51	
Beam B	1048.16	1048.62	
Beam C	1048.27	1048.72	
Beam D	1048.27	1048.72	
Beam E	1048.16	1048.62	
Beam F	1048.06	1048.51	



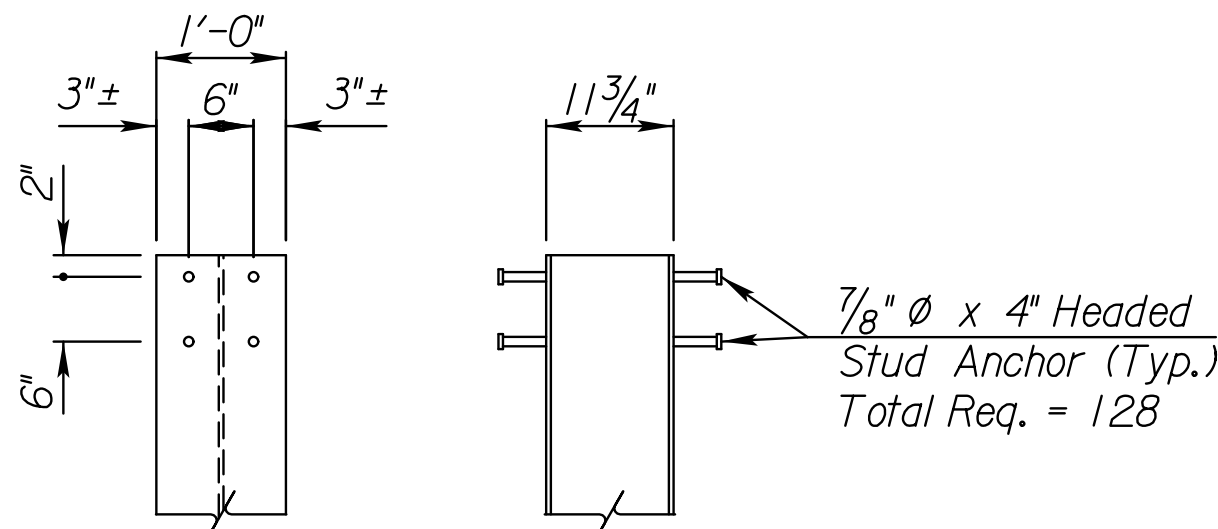
⊗ Note: Stud weld 8 - 7/8" ∅ Welded Stud Shear Connectors to each Pile. See Detail.

Note: Longitudinal Deck Reinforcing Steel not shown for clarity.



#### H.S. BOLT DIMENSIONS

(1 Beam Support)	
Girders	2 - 7/8" x 4 1/2"
Support Legs	2 - 7/8" x 2 1/2"



#### PILE ANCHOR SPACING DETAIL

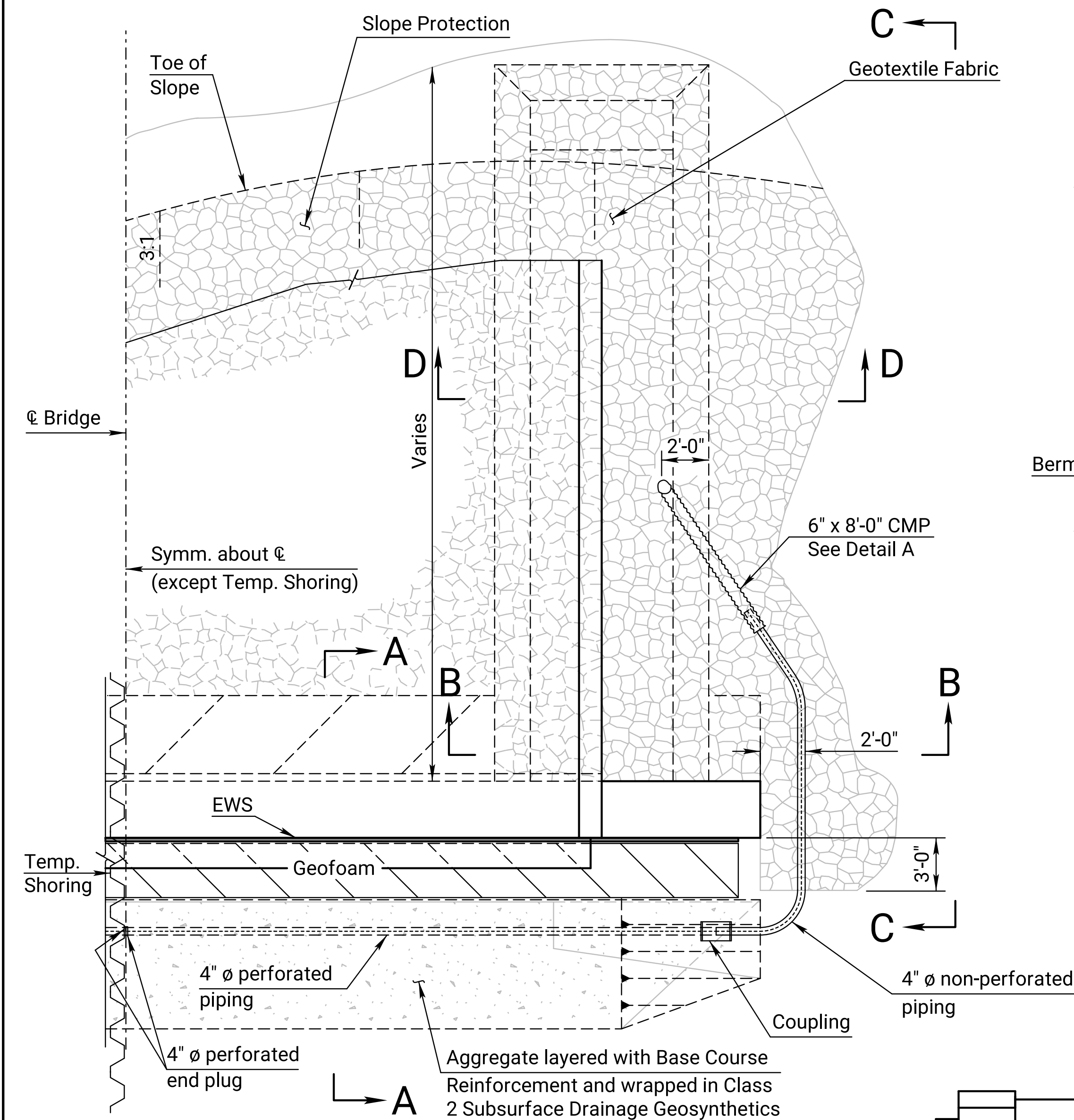
Note: Headed Stud Anchors to be subsidiary to the bid item "Piles (Steel) (HPI2x53)".

LRFD DESIGN PILE LOAD:  
Design Loading (Tons/Pile)    Strength I    Service I    Phi  
Abutment    68.5    46.7    0.60

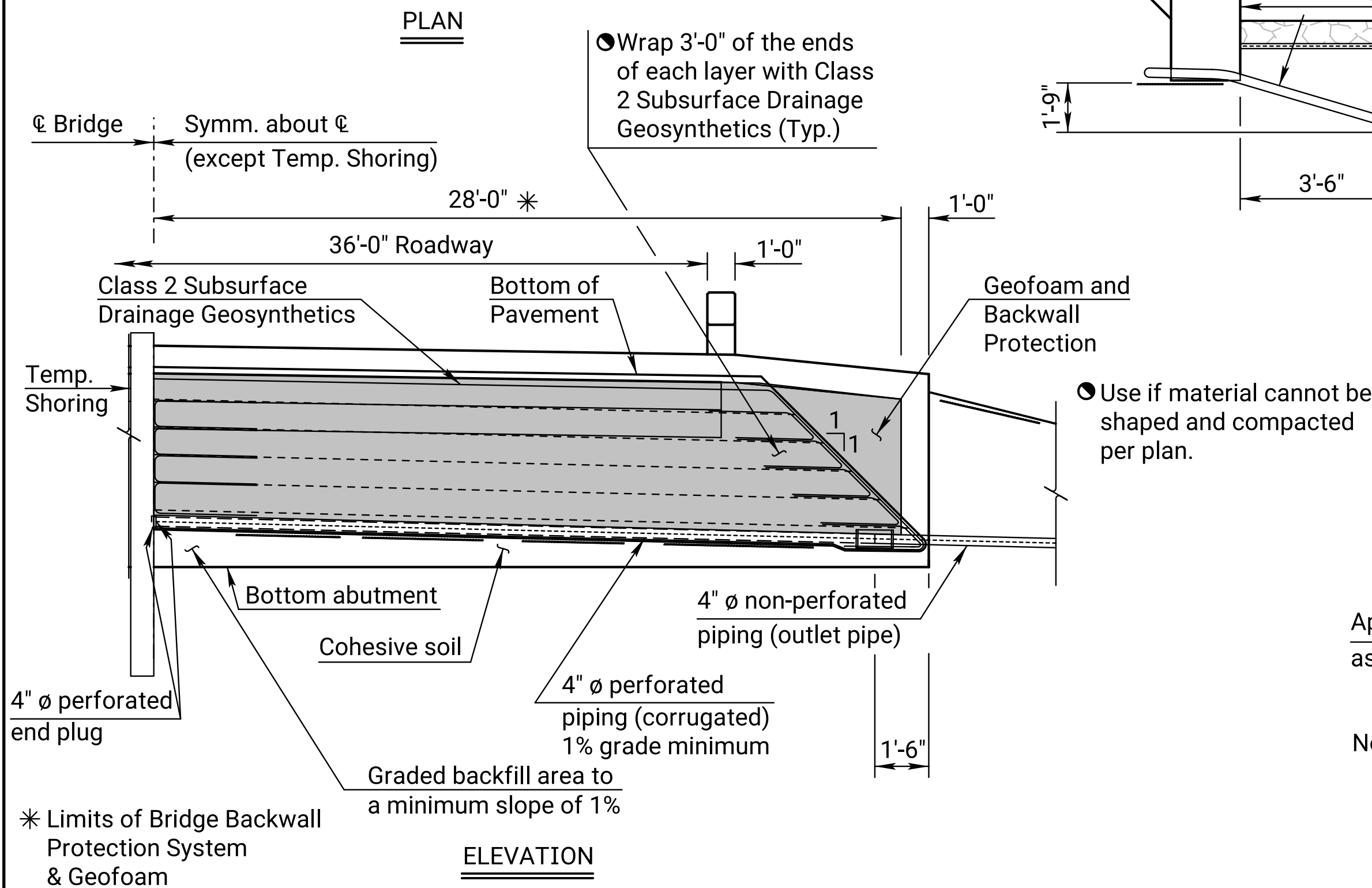
Legend  
EF= Each Face

3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048)    S+a. 62+23.30					
ABUTMENT AUXILIARY DETAILS					
Proj. 63-66 KA-5729-01    Nemaha Co.					
SHEET NO. OF	SCALE	APP'D			
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB

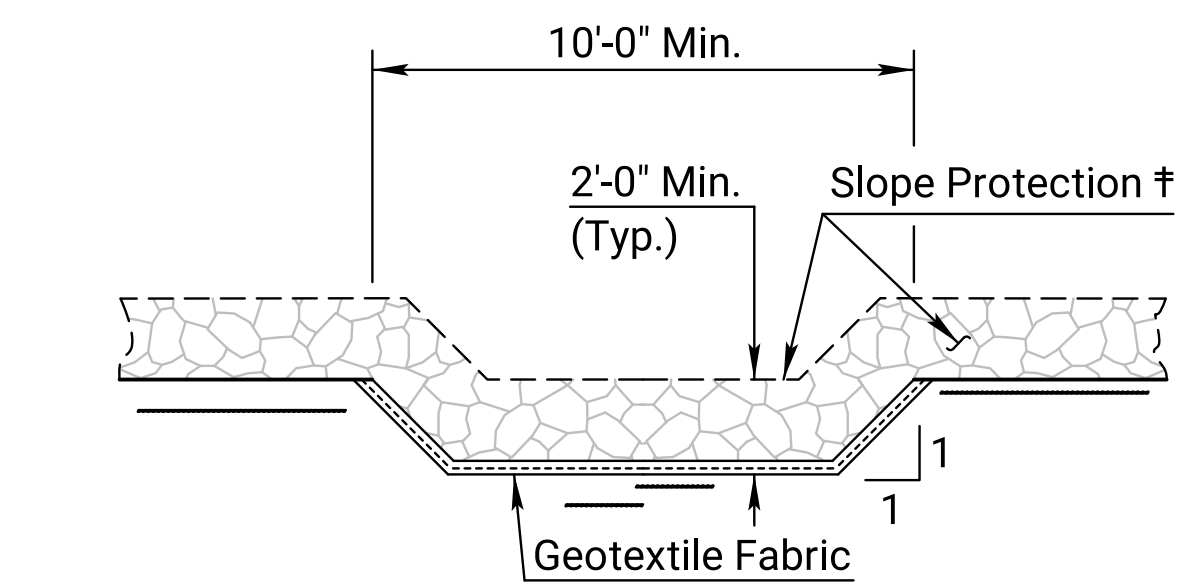
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	43	141



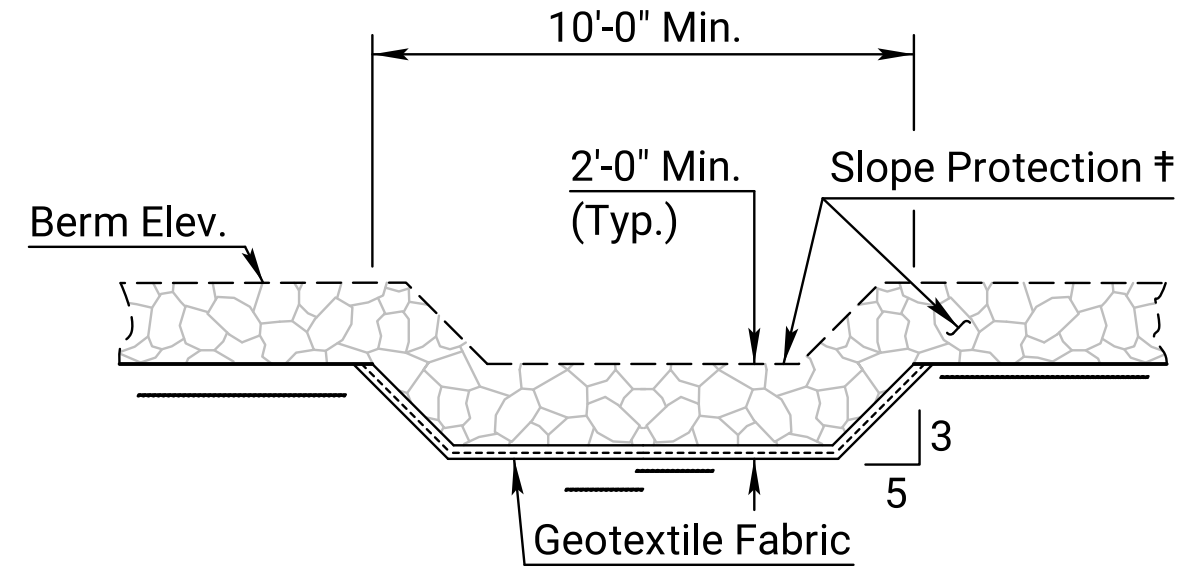
## PLAN



## ELEVATION



## SECTION D-D



## SECTION B-B

**GEOSYNTHETICS:** Use material that complies with KDOT Specification Section 1710 Class 2 subsurface drainage fabric. Place the Class 2 subsurface drainage fabric on graded and compacted material shaped as shown. Allow for enough material so that the sides can be overlapped and that the ends fully separate the aggregate drain from the embankment soils. Place the perforated drain pipe and couple to non-perforated pipe as shown. Allow the non-perforated pipe to pass through a hole carefully cut in fabric. Place aggregate within fabric to just leave the top of the pipe visible. Verify the slope of the pipe, that it is not damaged or displaced and that the couplers are firmly coupled. Continue to backfill to the elevation and shape shown.

AGGREGATE: Use aggregates that complies with KDOT Specifications for SB-1 or SB-2.

**BASE COURSE REINFORCEMENT:** Use "Base Course Reinforcement" that complies with KDOT Specification Division 1700 or approved material. Place this material in uniform layers without gaps or sags per the manufacturer's recommendations.

GEOFOAM: Use "Geofoam" that complies with ASTM D6817 EPS 12. Acceptance according to Type "C" certification. Bond this material to the backwall protection using materials recommended by the manufacturer.

GEOTEXTILE FABRIC: Use "Geotextile Fabric" that conforms with KDOT Specification 1710 and found on the Prequalified Materials List.

PIPE: Place perforated pipe within the limits and use non-perforated pipe outside the limits of the Abutment Aggregate Drain.

‡ All "Slope Protection (Riprap Stone)" on this sheet shall meet gradation requirements of 1/4 Ton Class.

**ABUTMENT AGGREGATE DRAIN:** The Bridge Contractor shall excavate to the limits shown on the Bridge Excavation Sheet. Backfill, compact & grade the cohesive soil to the limits shown. Place the bridge backwall protection, geofoam, geotextile, perforated pipe, alternating layers of aggregate and base course reinforcement as shown. Place the outlet pipe, the CMP, and the backfill. Separate as shown the entire Abutment Aggregate Drain with the geotextile.

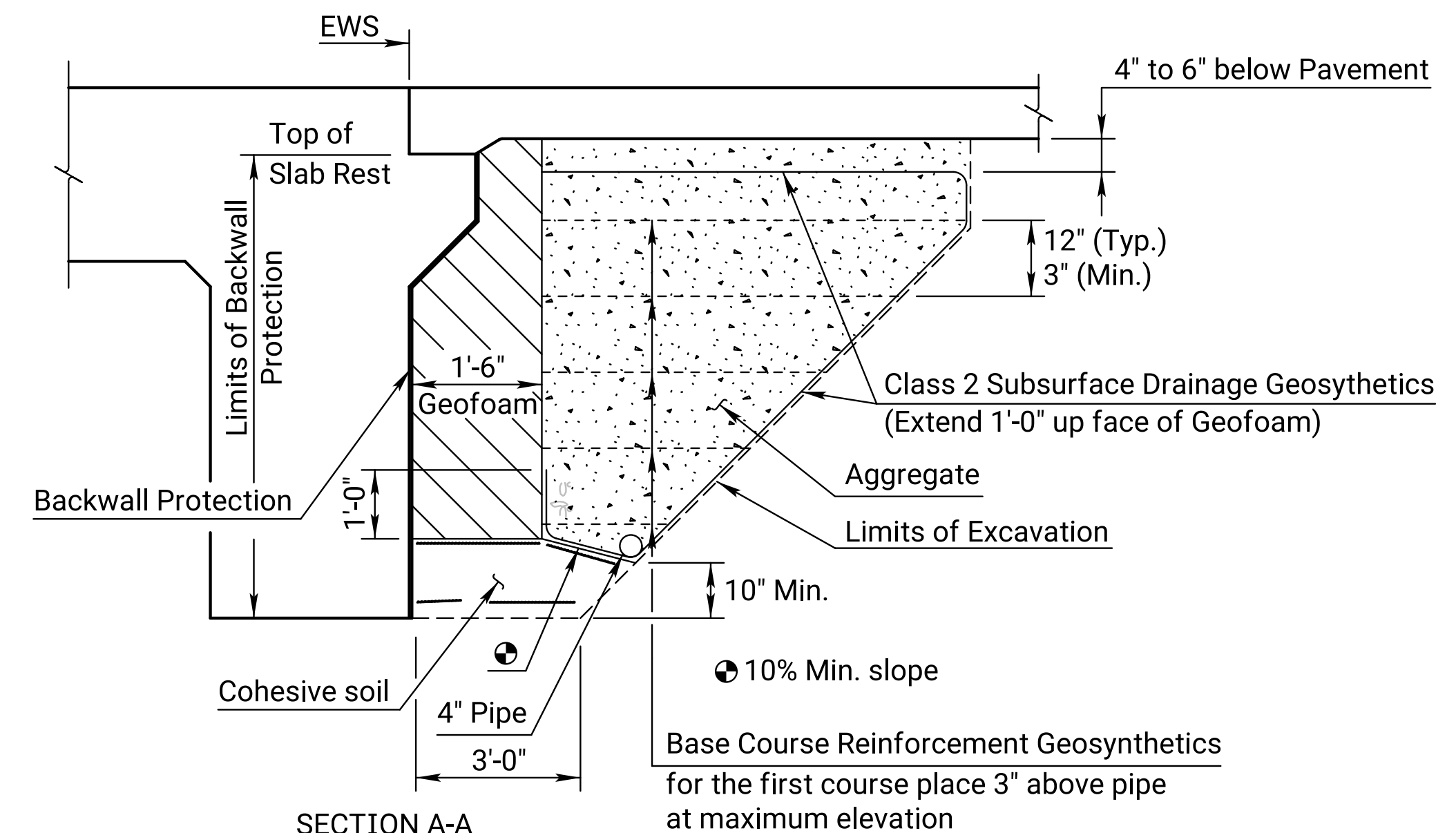
**BRIDGE BACKWALL PROTECTION SYSTEM:** Apply a non coal-tar Bridge Backwall Protection System to the approach side of the abutments and the wings in accordance with KDOT Specifications and the manufacturer's recommendations. Cover the abutments and wings to the limits shown on the details. Repair any damage done at no charge to the state.

Compact the abutment backfill. See the KDOT Specifications.

Perforated pipe and non-perforated outlet pipe shall be corrugated polyethylene tubing conforming to the KDOT Specifications.

Fit the CMP end section with ¼" galvanized mesh screen to prevent the entrance of rodents. Seal the joint between the outlet pipe and the end section with a joint sealer.

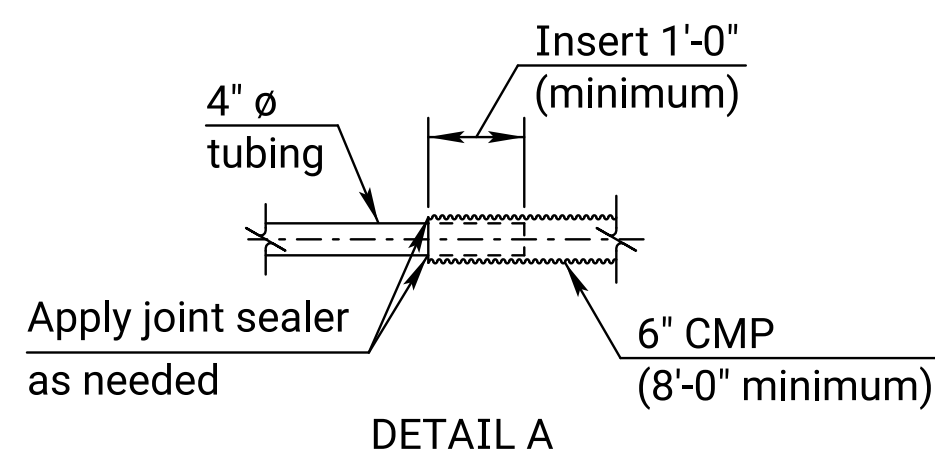
**COHESIVE SOILS:** Grade the bottom surface of the excavated area to drain as shown. Backfill this area with a cohesive type of soil. The soil will have a Unified Soil Classification of CL, CH, ML or MH according to ASTM D2487 Classification System with a minimum plasticity index of 13. Compact the material to Type A, MR-90 specifications. If the plasticity index cannot be met, add and mix Bentonite to the soil prior to placement and compaction so that the  $PI \geq 13$ .



SECTION A-A  
(Abutment Aggregate Drain)  
Taken at C

SUMMARY OF QUANTITIES (2 Abutments)	
Abutment Aggregate Drain	126 Cu. Yds.
Bridge Backwall Protection System	98 Sq. Yds.
Items subsidiary to Abutment Aggregate Drain	
4" ø Perforated Pipe	112 Lin. Ft.
4" ø Outlet Pipe	57 Lin. Ft.
6" ø CMP	32 Lin. Ft.
Geosynthetics (Class 2 Subsurface Drainage)	356 Sq. Yds.
Geosynthetics (Base Course Reinforcement)	320 Sq. Yds.
Geofoam	35 Cu. Yds.
Items subsidiary to Slope Protection	
Geotextile Fabric	102 Sq. Yds.

Note: The toe shall extend the entire width of the Slope Protection.



### DETAIL A

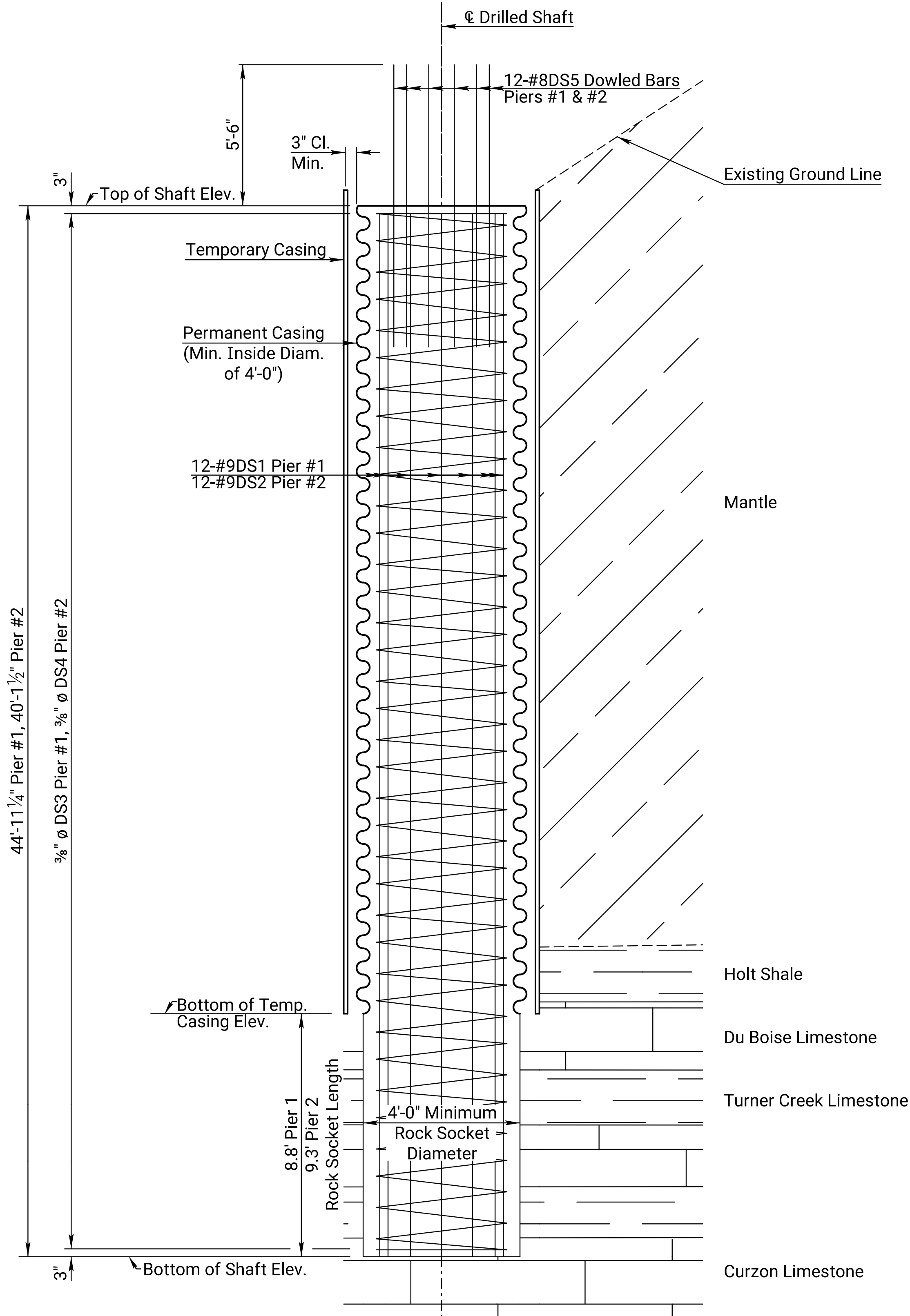
Note: The 1'-0" lap and joint sealer may be replaced by a reducing coupler at the junction of the CMP and the 4" round tubing. For stream crossings place outlet on downstream side of bridge.

08	05-05-23	Removed soil cap; Geofoam bottom flat			M.L.L.	M.A.H.														
07	12-11-18	Corrected std. base file name			M.L.L.	J.P.J.														
06	02-04-15	Modified Per 2015 Specification			J.P.J.	C.E.R.														
NO.	DATE	REVISIONS			BY	APP'D														
<p style="text-align: center;">KANSAS DEPARTMENT OF TRANSPORTATION</p> <p>Br. No. 63-66-54.90 (048) <span style="float: right;">Sta. 62+23.30</span></p> <p style="text-align: center;"><b>ABUTMENT DRAINAGE DETAILS</b></p> <p style="text-align: center;">Proj. 63-66 KA-5729-01 <span style="float: right;">Nemaha Co.</span></p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 15%;">DESIGNED</td> <td style="width: 15%;">J.P.J.</td> <td style="width: 15%;">DETAILED</td> <td style="width: 15%;">J.P.J.</td> <td style="width: 15%;">QUANTITIES</td> <td style="width: 15%;">CADD</td> <td style="width: 10%;">M.L.L.</td> </tr> <tr> <td>DESIGN CK.</td> <td></td> <td>DETAIL CK.</td> <td></td> <td>QUAN. CK.</td> <td>CADD CK.</td> <td></td> </tr> </table>							DESIGNED	J.P.J.	DETAILED	J.P.J.	QUANTITIES	CADD	M.L.L.	DESIGN CK.		DETAIL CK.		QUAN. CK.	CADD CK.	
DESIGNED	J.P.J.	DETAILED	J.P.J.	QUANTITIES	CADD	M.L.L.														
DESIGN CK.		DETAIL CK.		QUAN. CK.	CADD CK.															

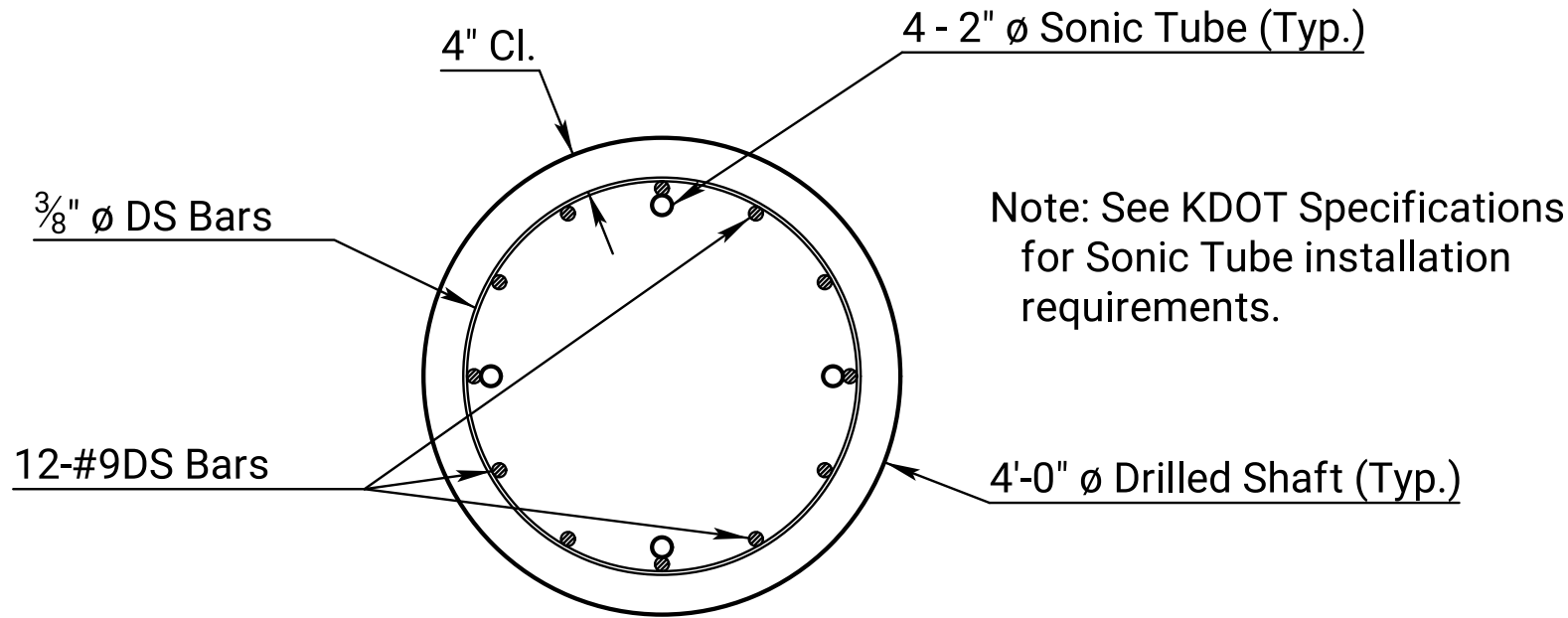


Plotted by : Jake.Pfannenstiel@ks.gov 15-JUL-2024 15:57  
File : ka572901bbr0048-25.dgn

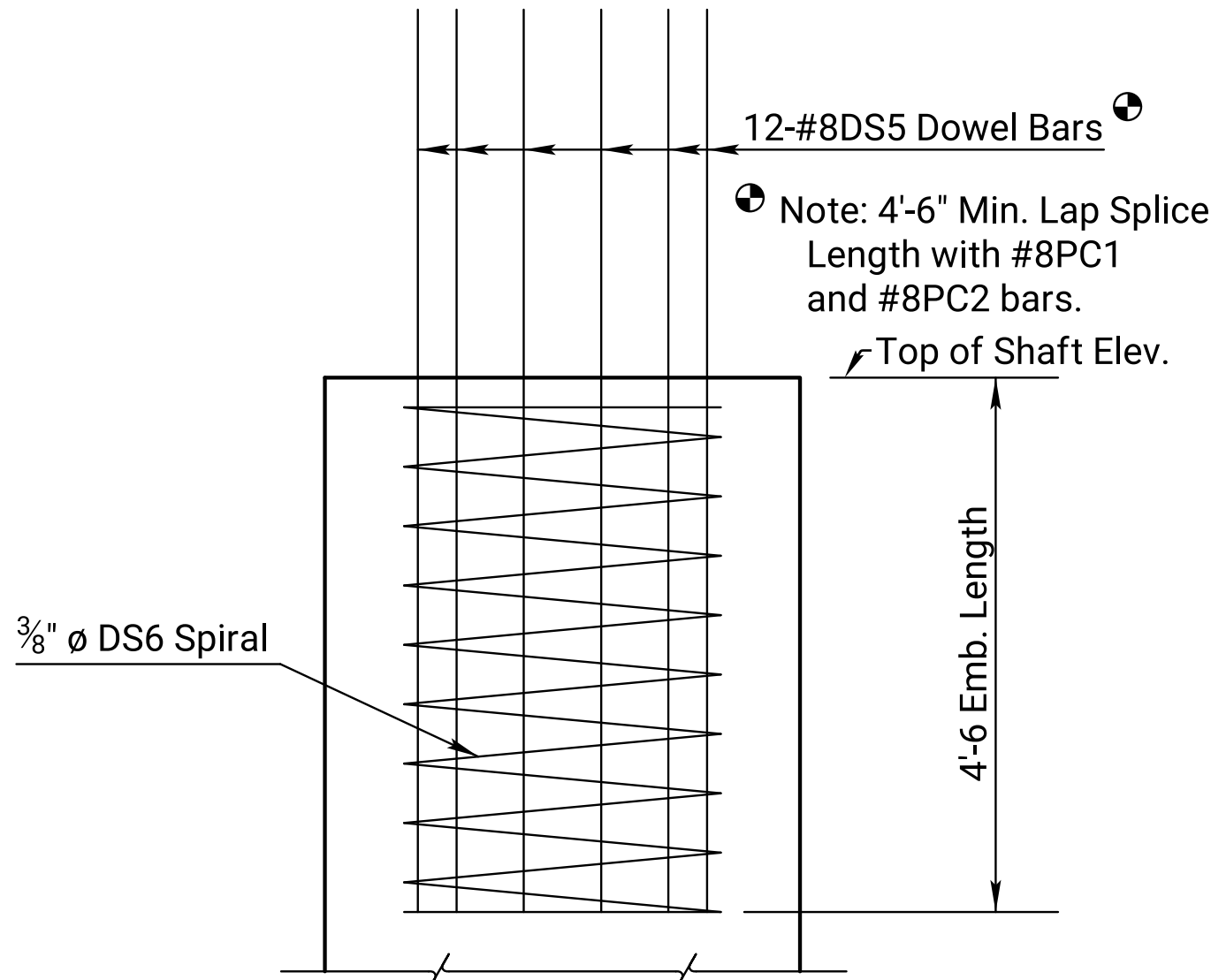
Note: Contact the KDOT Topeka Regional Geology Office prior to the construction of the drilled shafts.



TYPICAL ELEVATION VIEW OF DRILLED SHAFT



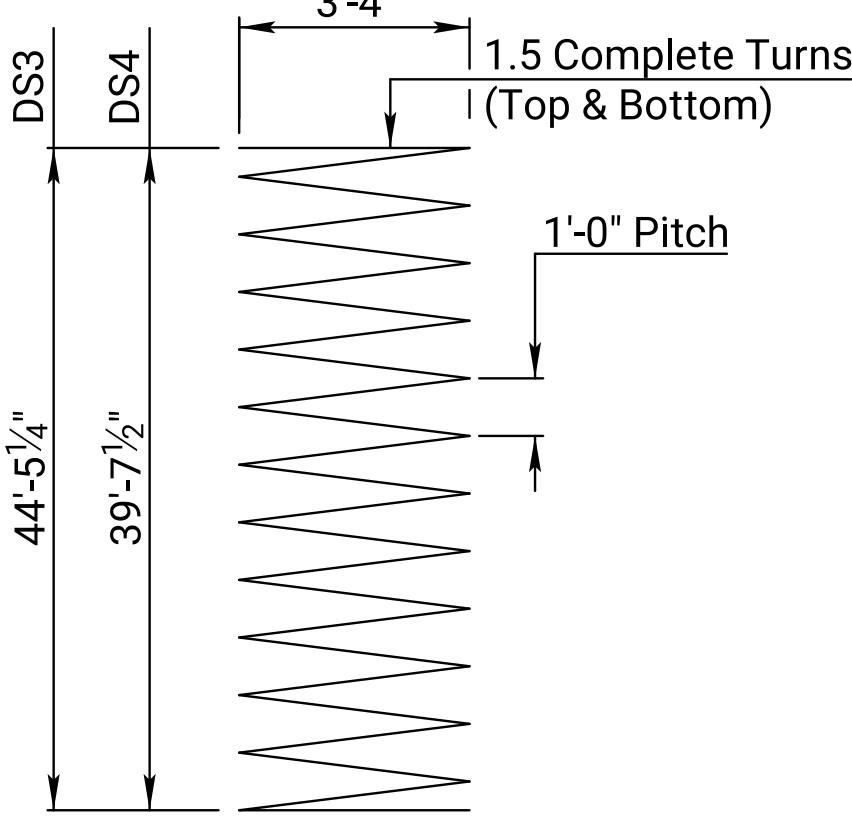
TYPICAL SECTION OF DRILLED SHAFT



DETAIL OF DRILLED SHAFT DOWEL BARS

BILL OF REINFORCING STEEL			
Grade 60 Non-Epoxy			
Total for Pier #1 & #2			
Subsidiary to Drilled Shafts			
Shown for Information Only			
Mark	Size	Number	Length
DS1	9	48	44'-8 1/4"
DS2	9	48	39'-10 1/2"
DS5	8	96	10'-0"
DS3	3/8" DS	4	††
DS4	3/8" DS	4	††
DS6	3/8" DS	8	††

†† See Bending Diagram

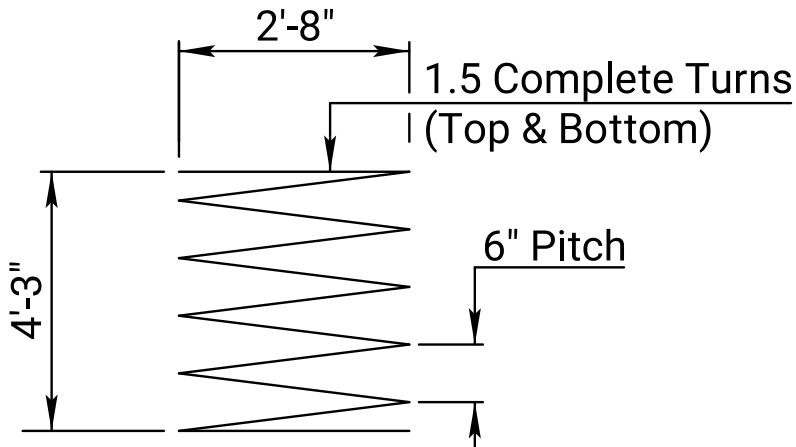


DS3, DS4

(3/8" DS smooth or deformed bar)

Spiral reinforcing shall meet the requirements of ASTM A615 Grade (60 or 40) or ASTM A82.

Spiral Spacer Bars:  
1) Minimum section modulus = 0.008 in.<sup>3</sup>  
2) 4 required per spiral



DS6

(3/8" DS smooth or deformed bar)

Spiral reinforcing shall meet the requirements of ASTM A615 Grade (60 or 40) or ASTM A82.

Spiral Spacer Bars:  
1) Minimum section modulus = 0.008 in.<sup>3</sup>  
2) 4 required per spiral

Pier	Station	Top of Shaft Elev.	Exist. Ground Line Elev.	Top of Rock Elev.	Bottom of Casing Elev.	Bottom of Shaft Elev.	Drilled Shaft Length (ft)
Pier No. 1	61+73.30	1036.94	1036.23±	1001.3	1000.8	992.0	44'-11 1/4"
Pier No. 2	62+73.30	1032.12	1030.45±	1003.8	1001.3	992.0	40'-1 1/2"

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	44	141

DRILLED SHAFTS: Construct the drilled shafts using the cased method. A permanent casing is required. All excavation, concrete, reinforcing steel, pipes for Sonic Testing, casings, labor, and incidentals necessary to complete the shaft as shown on the details and as directed by KDOT Specifications shall be included in the bid item "Drilled Shafts (48") (Cased)". Use Grade 4.0 Concrete in the drilled shaft. In no case shall the bottom of the drilled shaft be placed higher than the elevation shown unless otherwise directed by the KDOT Geologist.

Drill an Investigative Core Hole at the location(s) shown on the plans. See KDOT Specifications.

If the location of the top of shaft is such that the casing cannot be overtopped to remove concrete impurities, provide extra casing length to over-pour the concrete in the shaft and chip back to the plan elevation of the top of the shaft.

If the permanent casing is to be corrugated metal pipe (CMP) then it will be galvanized.

PERMANENT CASING: See KDOT Specifications

SONIC TESTING: Equip all drilled shafts with piping to allow sonic testing to be done. Install pipes at locations shown on the plans. All wet pours will be tested. Also, the Engineer has the option to require sonic, non-destructive, integrity testing at any location of concern. Sonic testing shall be paid for at the unit price set for "Sonic Test" (Drilled Shaft) (Set Price). If the sonic testing indicates defective concrete in the shaft, the Engineer will measure the first sonic test for payment, and the Contractor is responsible for subsequent sonic testing of that shaft. Report test results directly to KDOT's Chief Geologist. No work will be done above the top of drilled shaft without the approval of the Chief Geologist.

DRILLED SHAFT BACKFILL: Backfill the annular space between the temporary casing and the permanent casing with granular material as defined in the KDOT Specifications.

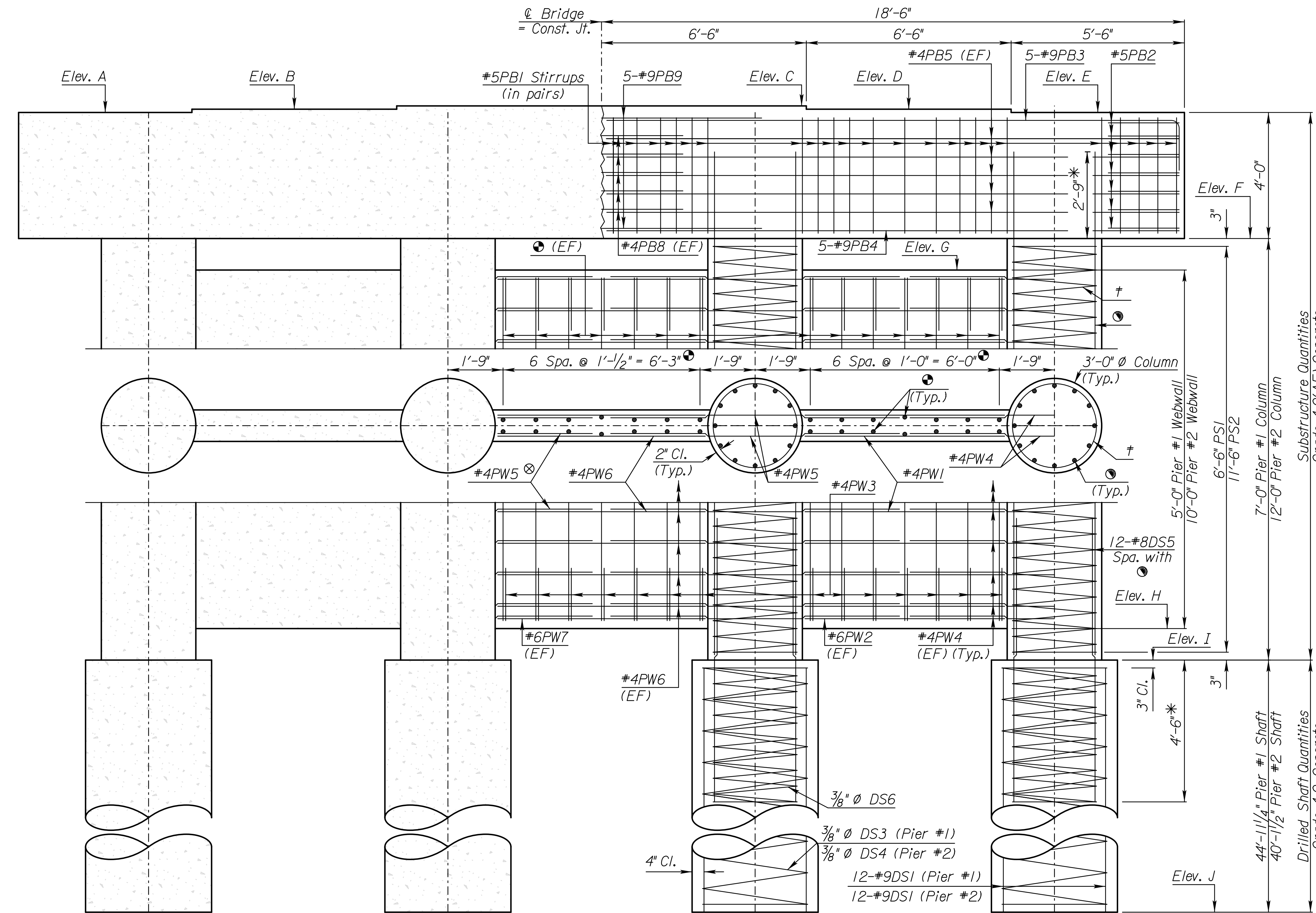
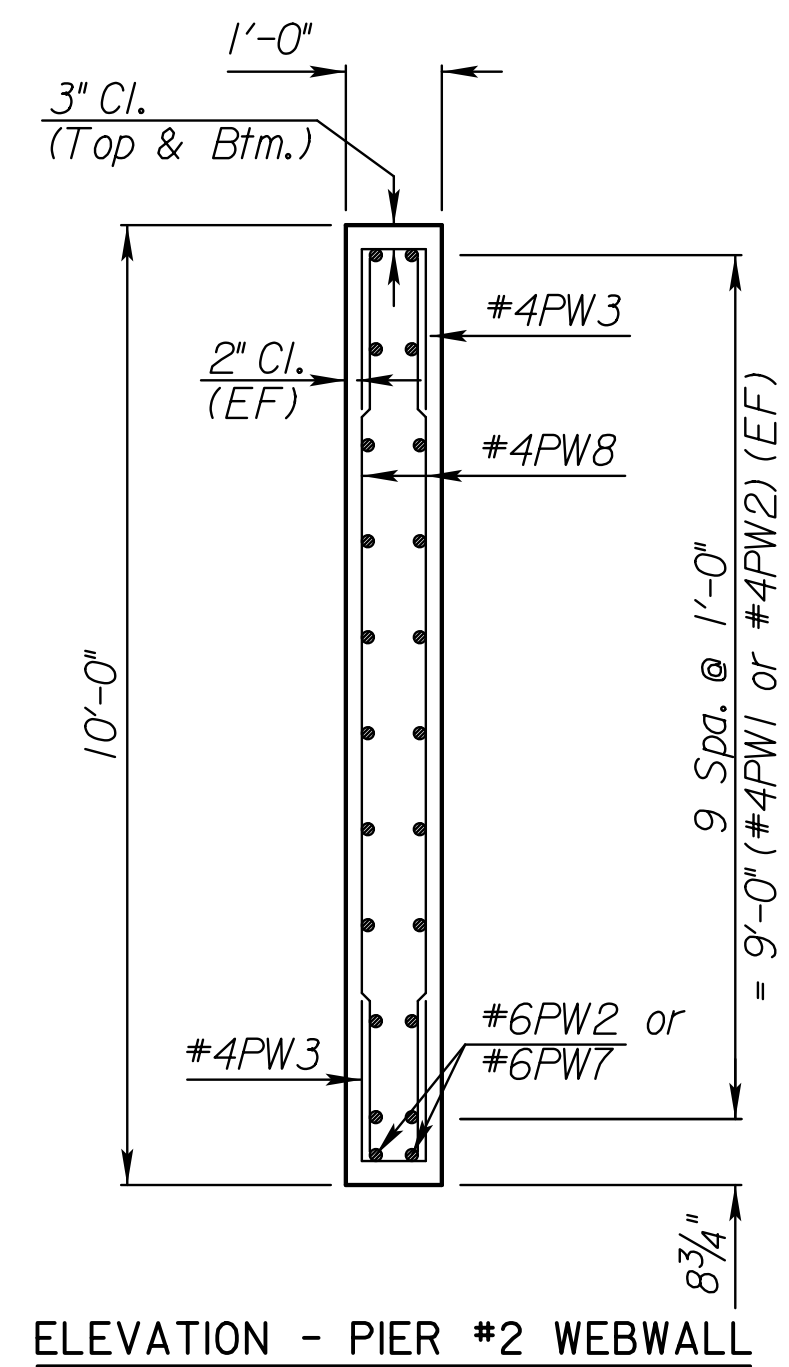
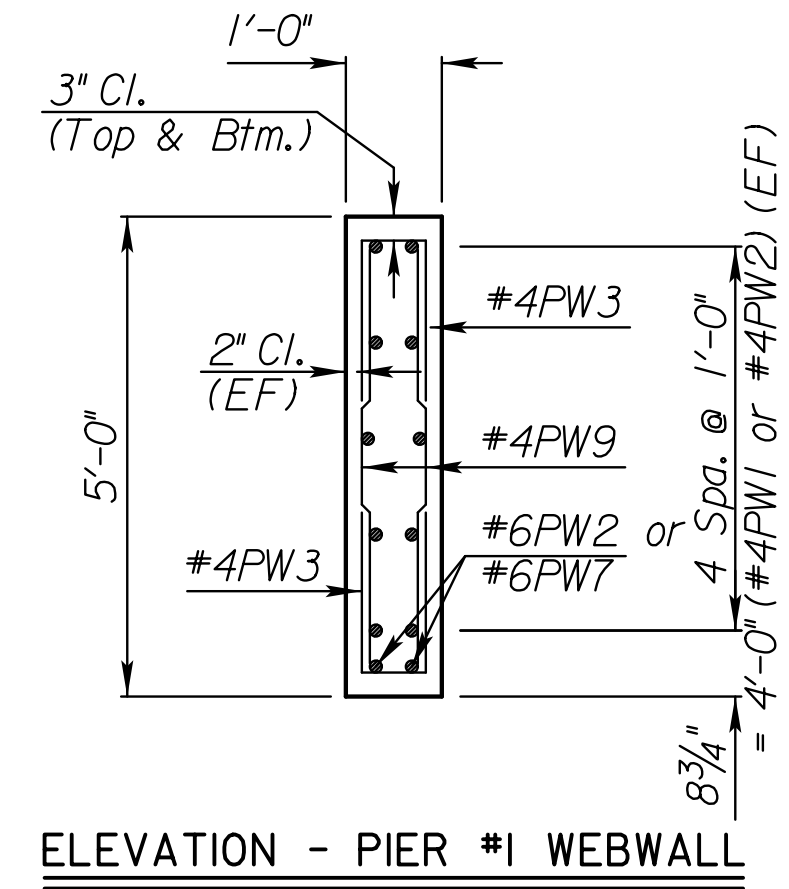
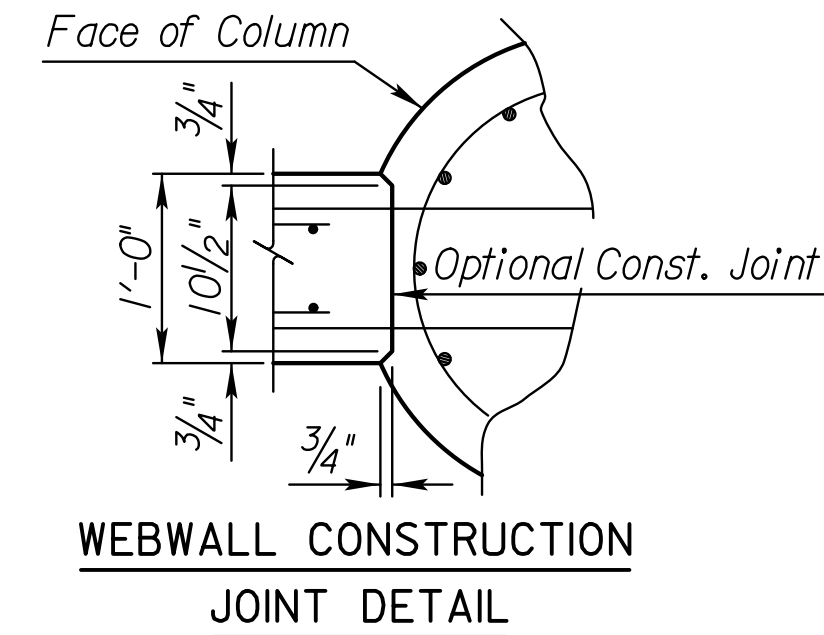
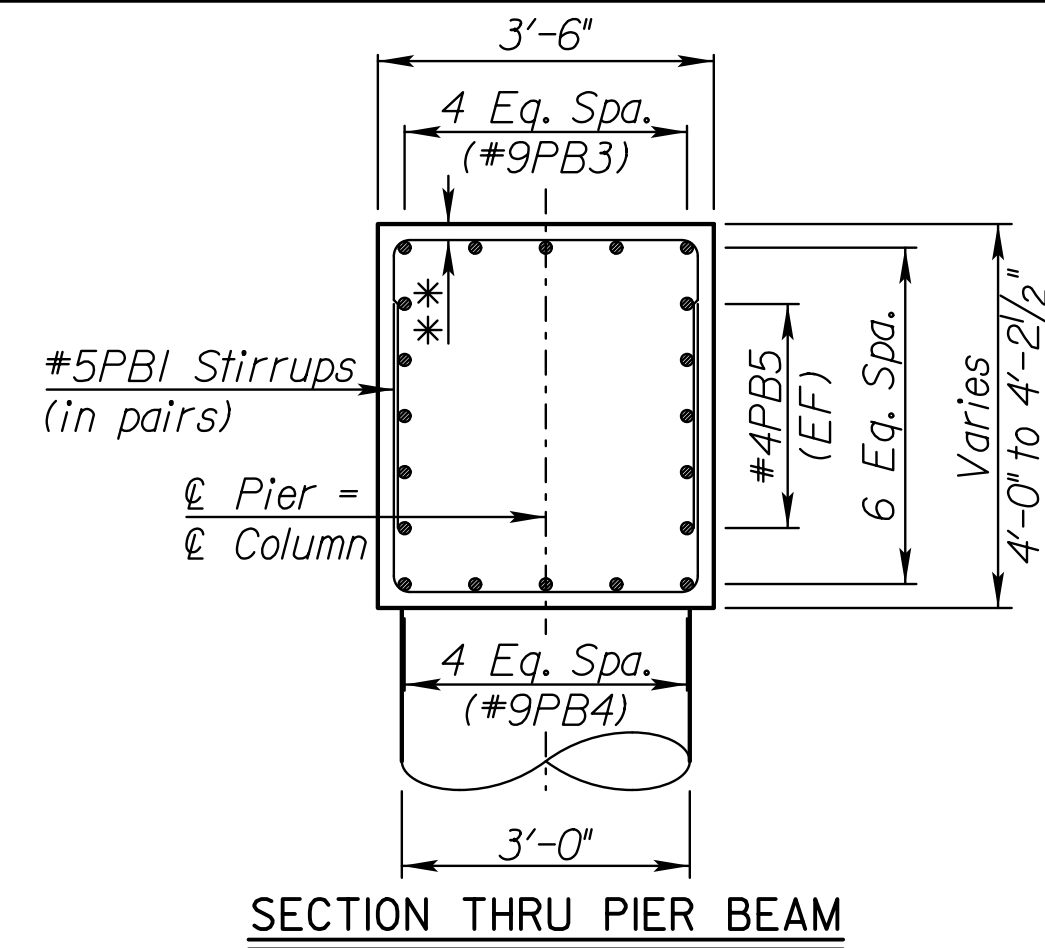
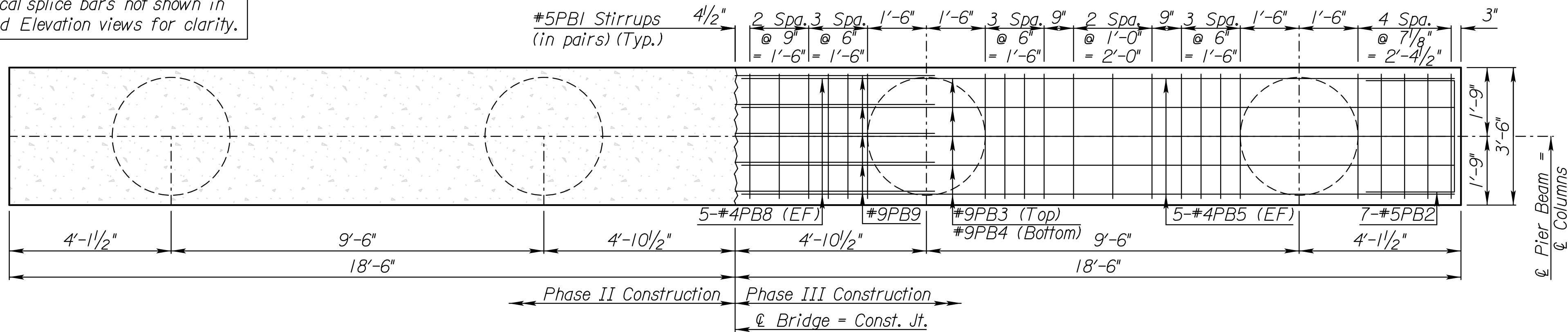
ROCK SOCKET LENGTH: Provide a minimum Rock Socket Length below the bottom of the casing as shown. Contact the KDOT Bridge Design Section if not able to provide the minimum Rock Socket Length.

NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048)			Sta. 62+23.30	
DRILLED SHAFT DETAILS				
Proj. 63-66 KA-5729-01			Nemaha Co.	
DESIGNED	DETAILED	QUANTITIES	CADD	
DESIGN CK.	DETAIL CK.	QUAN CK.	CADD CK.	





Mechanical splice bars not shown in Plan and Elevation views for clarity.



ELEVATION  
(Looking North)

TABLE OF ELEVATIONS		
Elevation	Pier #1	Pier #2
A	1047.94	1048.12
B	1048.04	1048.22
C	1048.15	1048.33
D	1048.04	1048.22
E	1047.94	1048.12
F	1043.94	1044.12
G	1042.94	1043.12
H	1037.94	1033.12
I	1036.94	1032.12
J	992.00	992.00

Note: Elevations are located along the Pier C/L.

Note: See "AUXILIARY PIER DETAILS" sheet for locations of Preformed Anchor Bolt Holes.

Note: Either cast the columns and pier web monolithically, or cast the columns separately using a keyed joint as shown. If the columns are cast separately, use threaded deformed bars in lieu of the #4PW4 & #4PW5 dowel bars. Bar diameter and embedment length into the web wall shall be as designated. The inserts shall develop the full yield strength of bars. No change in compensation is allowed with the use of inserts. Coil inserts are not allowed.

\*\* The clearance from the top of the Pier beam to the #5PBI stirrups will vary with a minimum of 2".

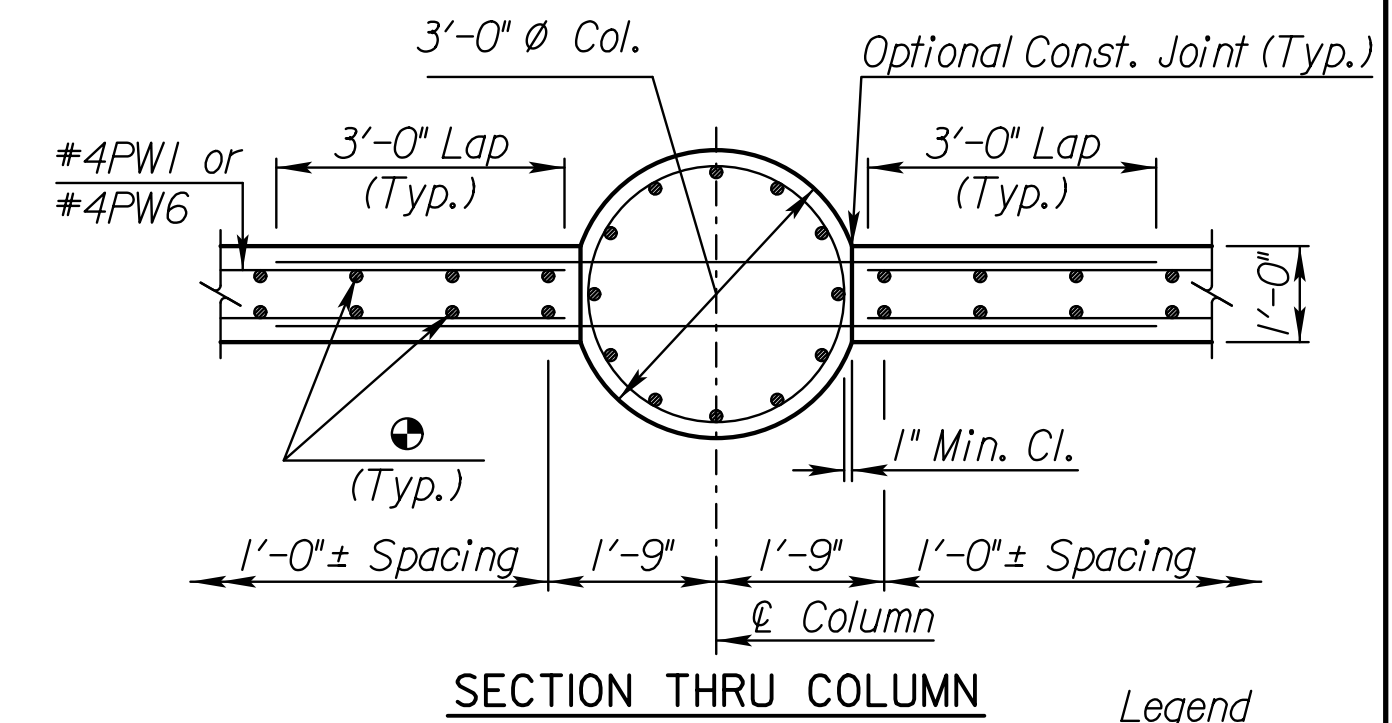
\* Minimum Embedment

● 12-#8PCI for Pier #1 & 12-#8PC2 for Pier #2

† 3/8"Ø PSI for Pier #1 & 3/8"Ø PS2 for Pier #2

⊗ #4PW9 for Pier #1 & #4PW8 for Pier #2

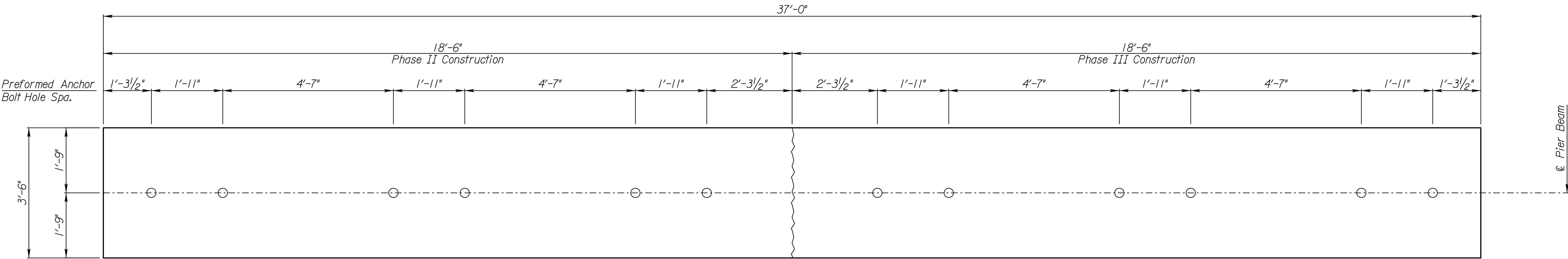
⊗ Splice the #4PW6 and #6PW7 bars to the #4PW5 bars from Phase II



Legend  
EF = Each Face

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) Sta. 62+23.30				
PIER DETAILS 2				
(Phase III Construction)				
Proj. 63-66 KA-5729-01			Nemaha Co.	
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	47	141



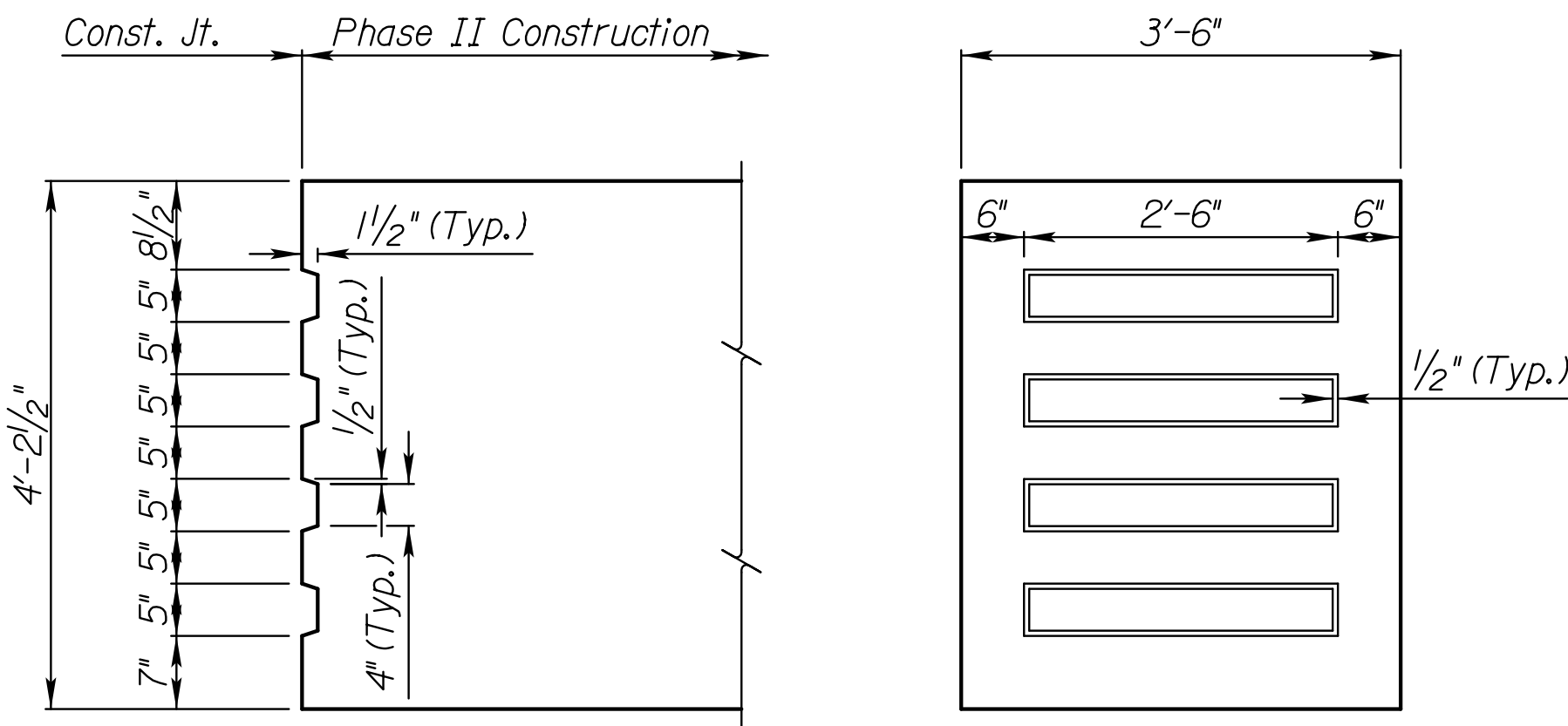
PLAN

NOTE: Spacing shown to centerline of Preformed Anchor Bolt Holes.

ANCHOR BOLTS: Anchor bolts will adhere to KDOT Standard Specification Division 1600 (Grade 55) with the following exception. The threads may be rolled or cut. The bolts and nuts shall be galvanized.

Place the pier beam reinforcing bars to clear the anchor bolts.

PREFORMED ANCHOR BOLT HOLES: Preform 3 inch diameter holes at the locations shown. When temperatures are expected to go below freezing, seal the preformed holes or fill them with a propylene glycol-based antifreeze to prevent expansion damage. The holes will be free of water, antifreeze or other foreign materials at the time of grouting the anchor bolts. The polyethylene tubing may remain in place. Trim the tubing flush with the top of the concrete. This work shall be subsidiary to Concrete Grade 4.0 (AE).

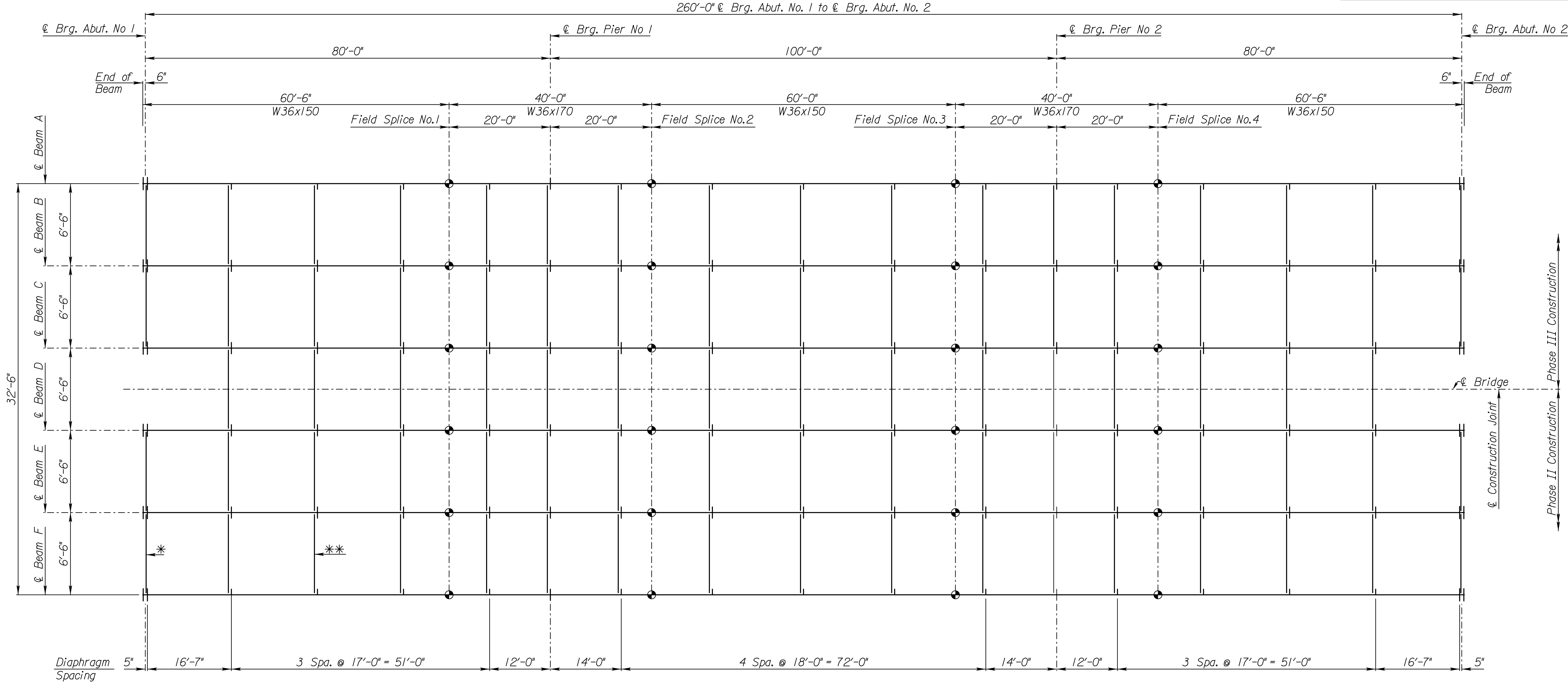


PIER BEAM CONSTRUCTION JOINT

Phase II	Phase II Splice	Phase III	Phase III Splice
#9PB3	#9PB7	#9PB3	#9PB9
#9PB4	#9PB7	#9PB4	#9PB9
#4PB5	#4PB6	#4PB5	#4PB8

3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) Sta. 62+23.30				
AUXILIARY PIER DETAILS				
Proj. 63-66 KA-5729-01 Nemaha Co.				
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.
			CFB	CADD
			JAB	CADD CK.
				KMS
				JAB





\* See "Abutment Frame Details" sheet.  
\*\* See "Diaphragm Details" sheet.

Note: Attach the intermediate diaphragms between Beams C and D after the placement of Phase III deck concrete. Field drill holes in the Beam D connections stiffeners using holes in the diaphragms as a template.

Note: All longitudinal dimensions are horizontal. All transverse web stiffeners are perpendicular to the top flange of the beam, except bearing stiffeners, which shall be vertical. Cut beam ends vertical. Correct beam length for grade.

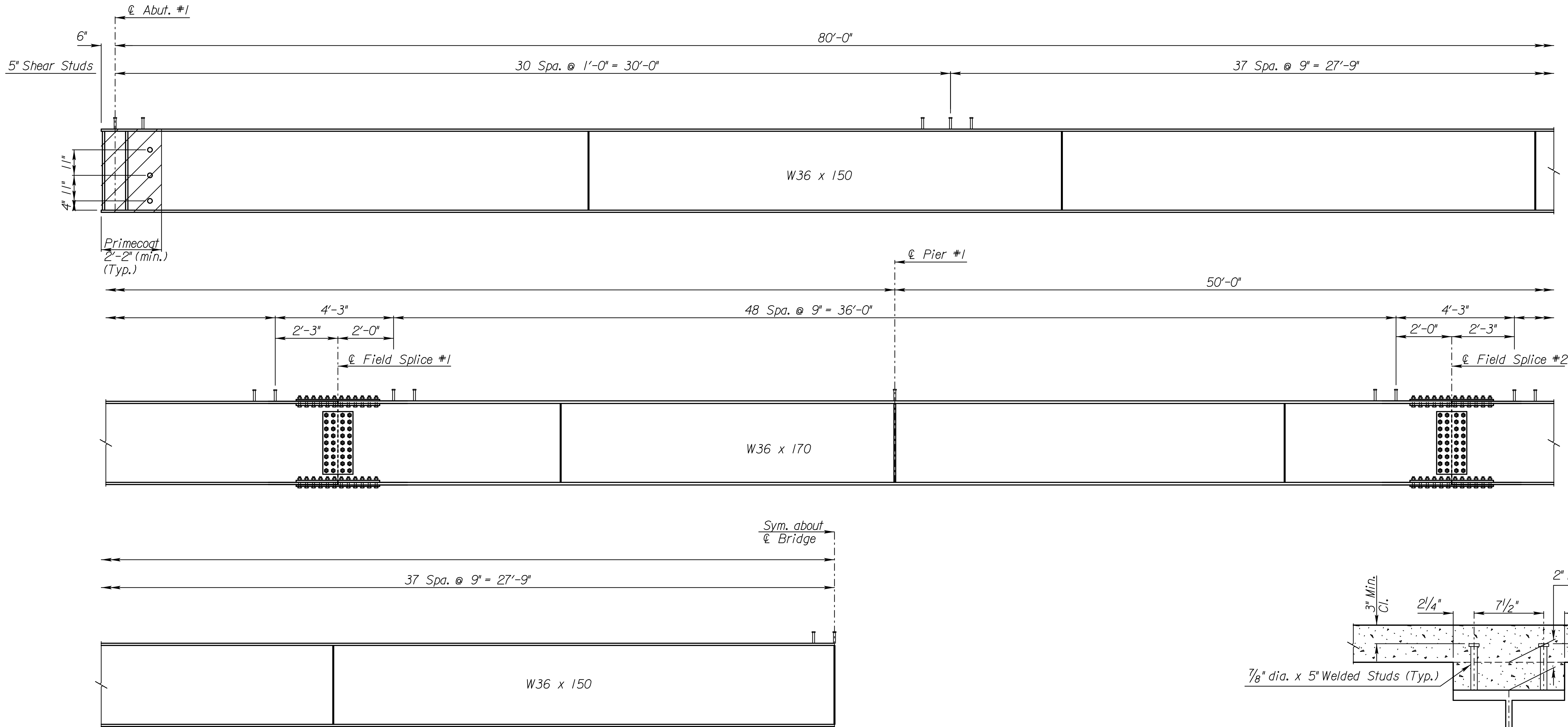
Note: Blast clean all bent plate diaphragms and stiffeners at bolted connections to a surface condition of SSPC-SP6 or better to satisfy Class B Surface requirements.

FRAMING PLAN

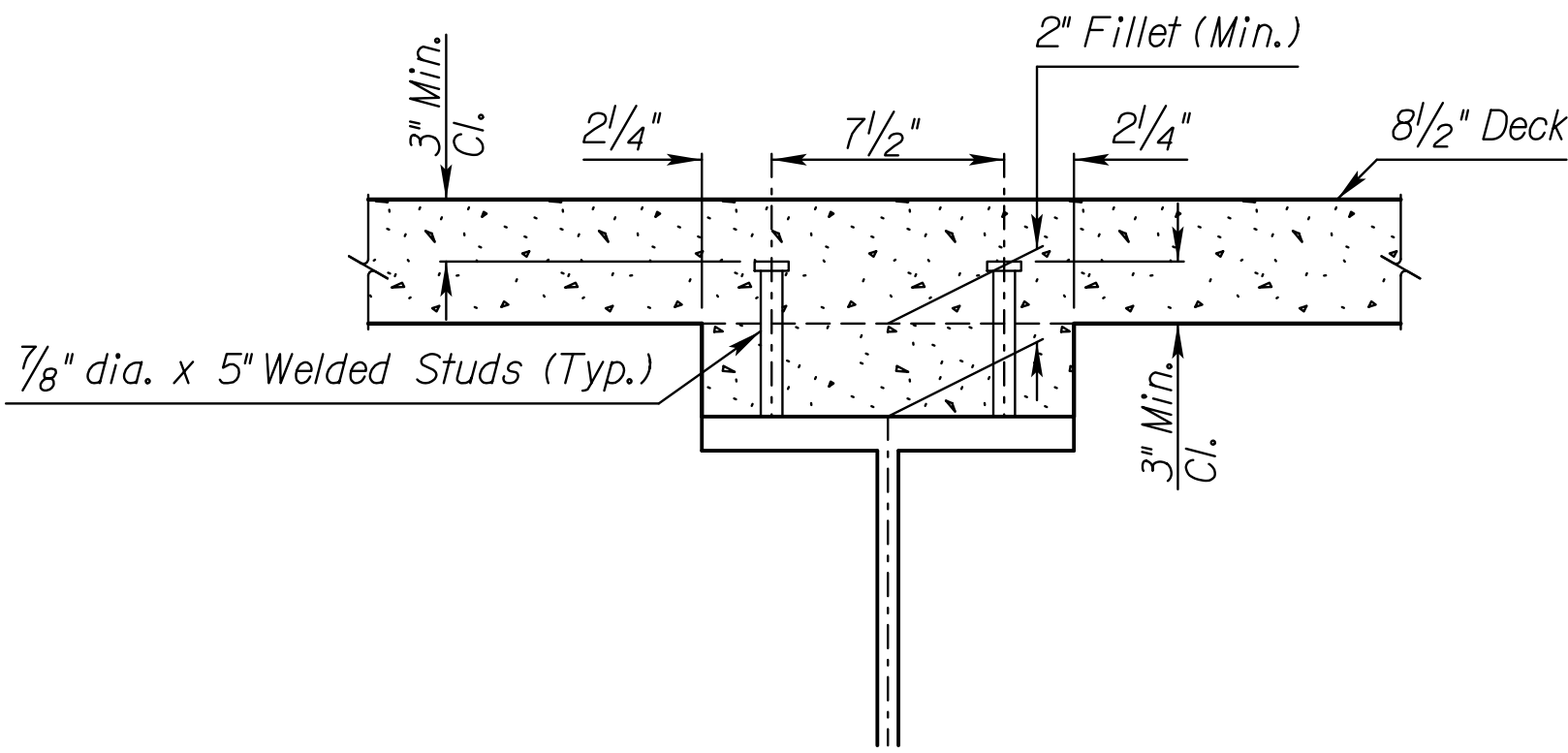
STRUCTURAL STEEL SUMMARY			
Item	ASTM A709 Gr. 50 (lb)	ASTM A709 Gr. 50W (lb)	AASHTO M270 Gr. 50W T3 (lb)
Beams			244,500
Flange Splice Plates			7,887
Web Splice Plates			3,641
Stiffeners		4,928	
Fill Plates	608		
Bearing Stiffeners (Pier only)		938	
Abutment Frames		1,040	
Bent Plate Diaphragm		16,509	
Beam Supports	1,219		
Total Structural Steel	1,827	23,415	256,028

3					
2					
1					
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) S+a. 62+23.30					
FRAMING PLAN					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB
				CADD	KMS
				CK.	JAB

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	49	141



LOCATION OF WELDED SHEAR CONNECTORS  
(618 - 7/8" Ø x 5" studs Req'd per Beam)



SHEAR STUD CONNECTORS DETAIL

Plotted: J:\ke.P\arnenstie\kgs\Plot Location: Bridge  
File: ka572901bbr0048-34.dgn  
Plot Date: 15-JUL-2024 15:55

3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) Sta. 62+23.30					
BEAM DETAILS					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB

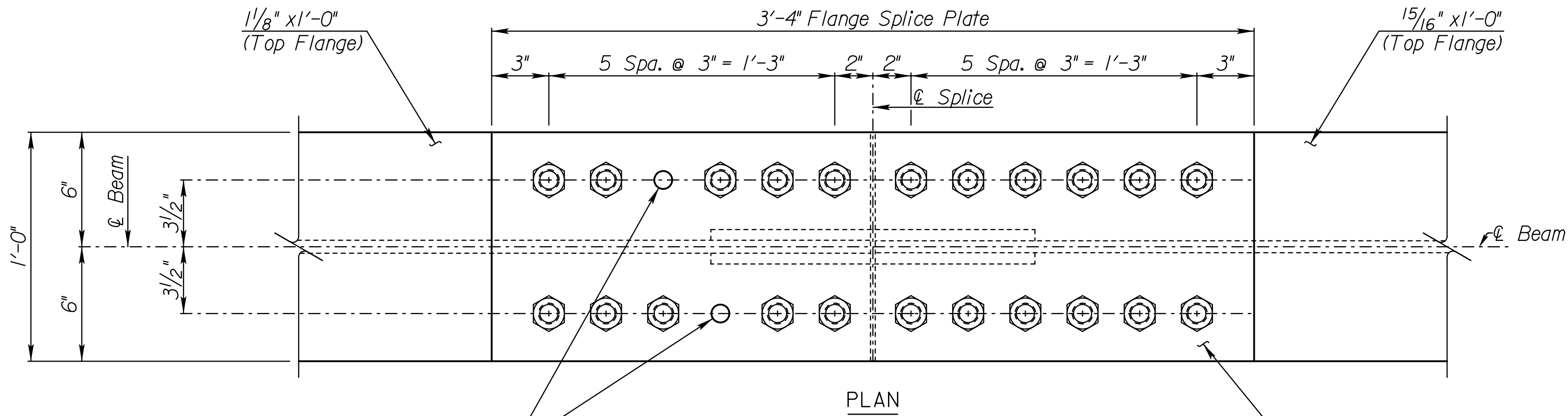
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	50	141

Field Splice Notes:

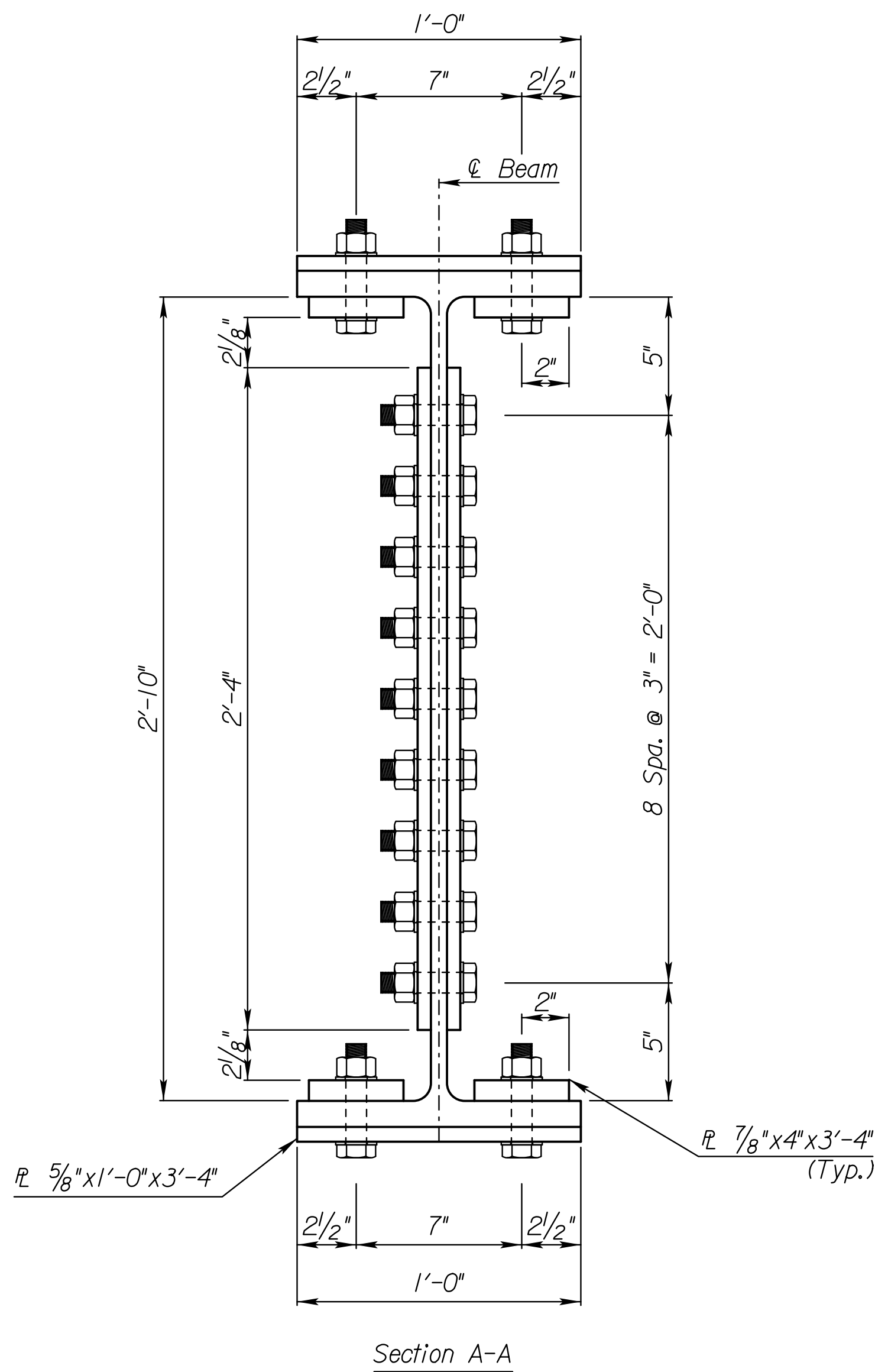
Fasteners for field splices shall be  $\frac{7}{8}$ " high strength steel bolts. Use ASTM F3125 Grade 325 (Type 3) with DTI washers and hardened washers.

Flange splice plates and web splice plates shall be AASHTO M270 Grade 50W T3 Structural Steel.

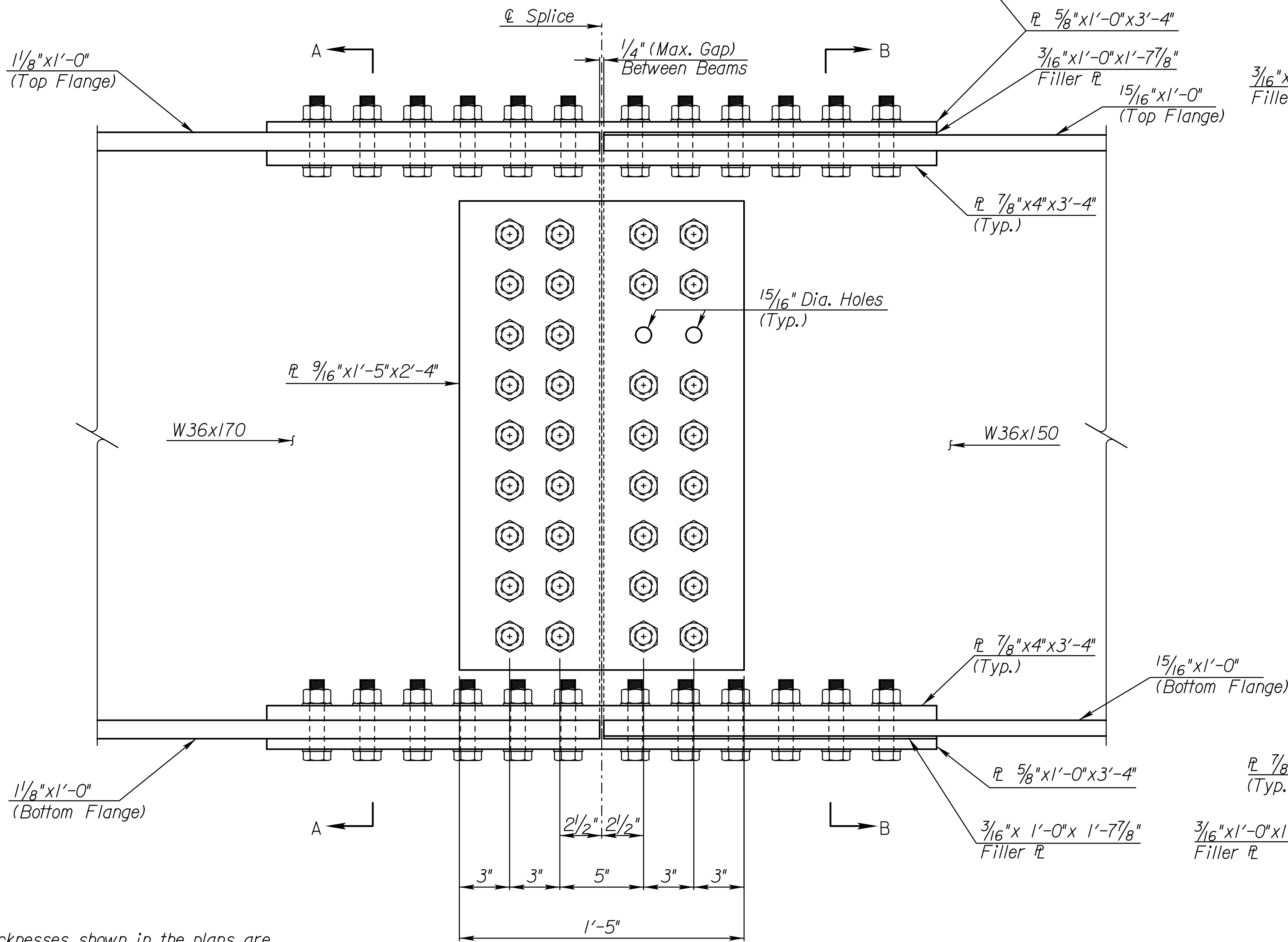
Filler plates shall be ASTM A709 Gr. 50 Structural Steel.



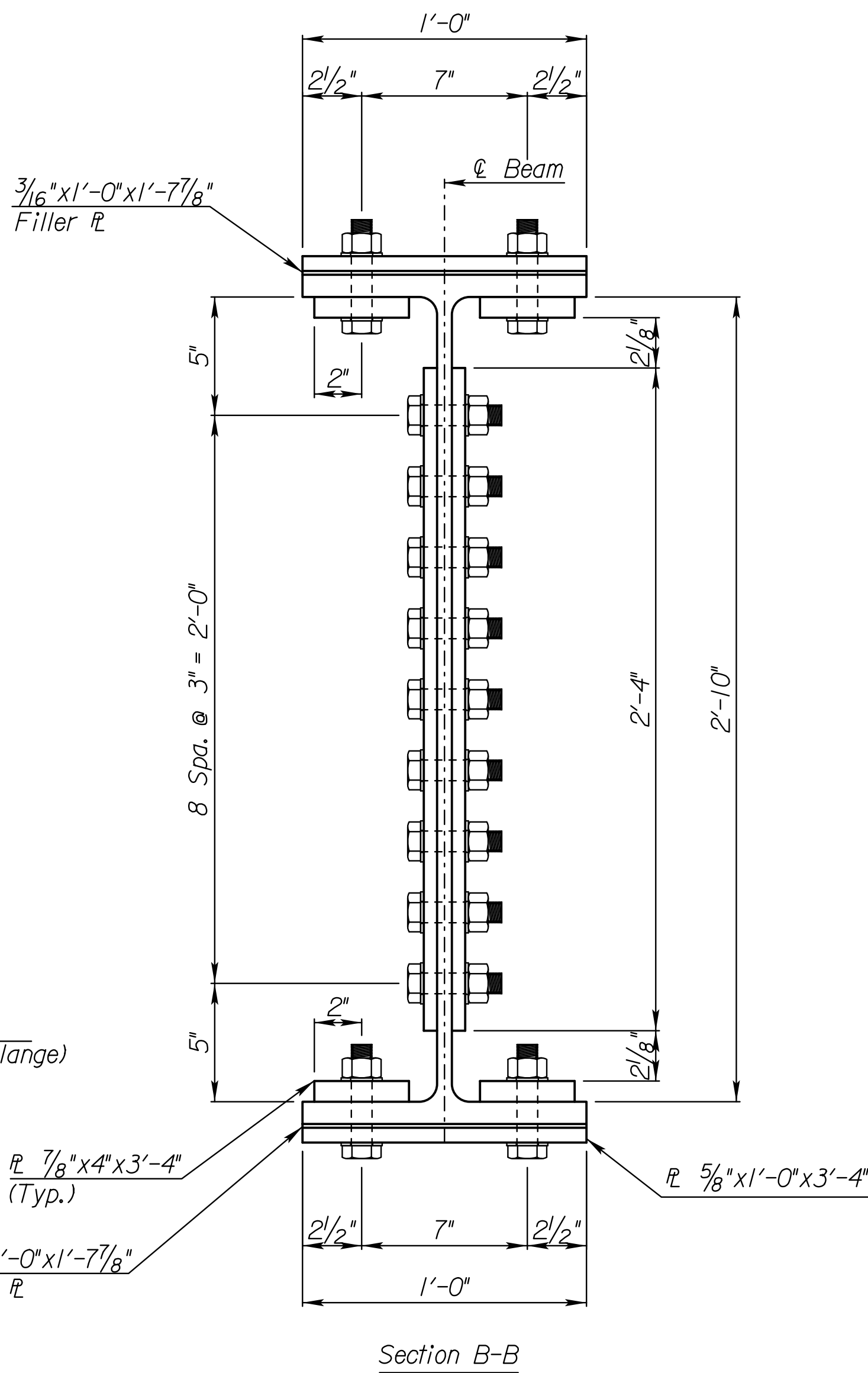
PLAN



Section A-A



ELEVATION



Section B-B

FILL PLATES: Fill plate thicknesses shown in the plans are based on nominal AISC dimensions. Fabricators are required to revise plan fill plate thicknesses as necessary to account for as-rolled variations in flange and web thickness or overall beam depth. Minimum fill plate thickness =  $\frac{1}{8}$ ". By itself, a change in fill plate thickness does not require a resubmittal of shop drawings.

BOLTED FIELD SPLICE DETAILS

(24 splices required)  
(84 -  $\frac{7}{8}$ "  $\emptyset$  F3125 Grade 325 HS Bolts Req'd per Splice)

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) S+a. 62+23.30				
BOLTED FIELD SPLICE DETAILS				
Proj. 63-66 KA-5729-01			Nemaha Co.	
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	51	141

Production Bolt Tightening

1. Install bolts and tighten to "snug tight" in a pattern, starting at the center of the splice and working toward the edge. On large girders this may have to be done twice, as the center bolts will become loose as plates are "Ironed out". This step is important because typically, any variation in results during production bolting is the result of a change in the materials, lubricant or equipment used to take the bolts to a "snug tight" condition during the calibration process.
2. Mark all of the bolts, nuts and the plate as shown in the marking detail. Mark the socket with a start and stop point. The stop point corresponds to the target rotation determined earlier.
3. Align the start mark on the socket with the line on the plate. While the bolt is being backed up, turn the nut until the stop mark on the socket lines up with the start mark on the plate.
4. Repeat with all bolts of the same length in the splice.

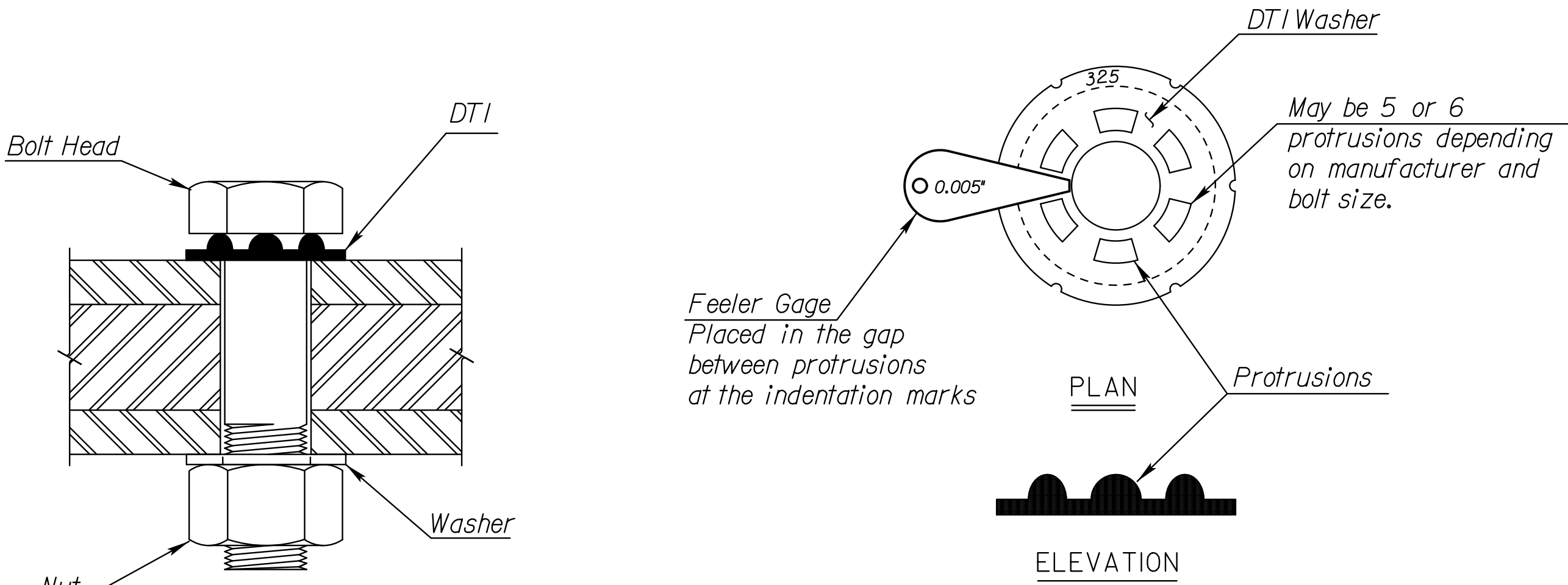
Acceptance and Rejection of Bolts

1. The Engineer will check all bolts with a feeler gage.
2. All nuts must be turned at least the target rotation beyond "snug tight".
3. All DTI's must have at least 3 refusals of the 0.005" gage.
4. If all gaps refuse the 0.005" gage, and the nut, plate and bolt are not marked, reject the bolt.
5. If all gaps refuse the 0.005" gage, and the turned element has not been rotated more than 45° beyond the calibrated turn, accept the bolt.
6. If all gaps refuse the 0.005" gage, and the turned element has been rotated more than 45° beyond the calibrated turn, reject the bolt.

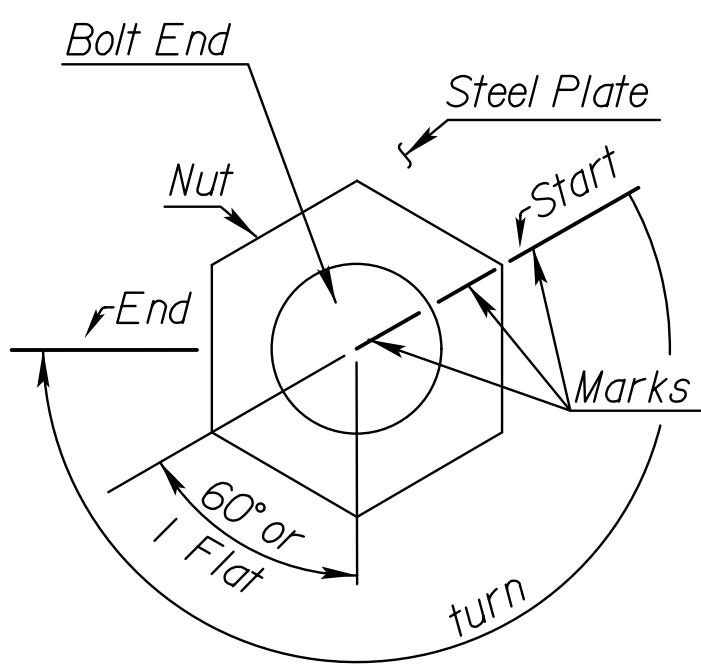
For additional information see the structural steel section of the Bridge Construction Manual.

Suggested Impact wrench models:  
CP 611  
iR 2940  
Cleco WS2110  
ATP 1011/1040  
Norbar PT1500

3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) S+a. 62+23.30					
STEEL ERECTION FIT-UP AND BOLTING PROCEDURE					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB



TYPICAL DIRECT TENSION INDICATOR (DTI)

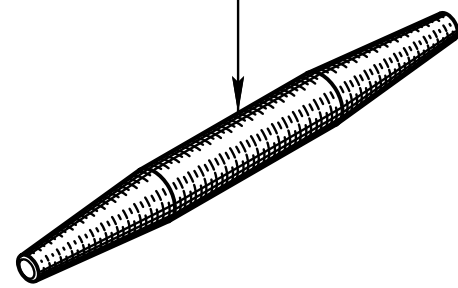


REQUIRED MARKING DETAIL

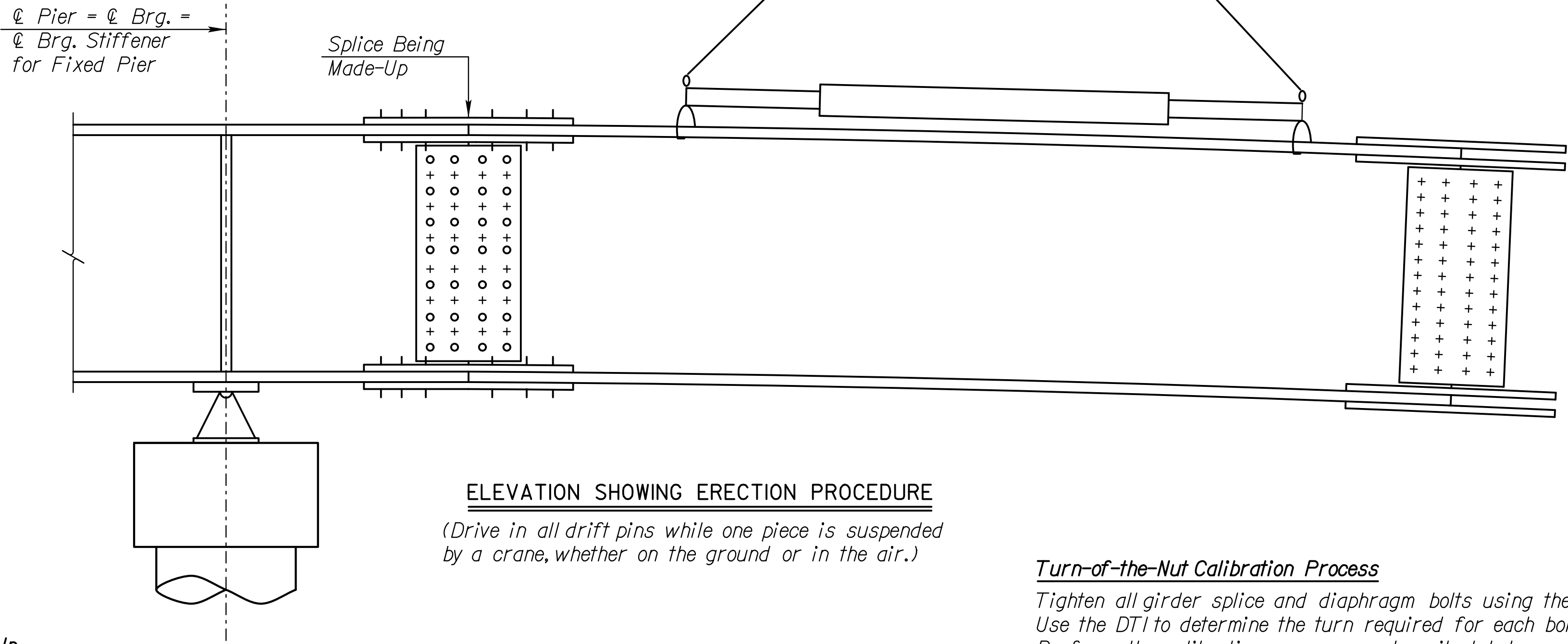
(shows calibrated turn = 3 1/2 flats from snug tight condition). Example only, calibrated turn may be more or less than shown.

PROPER FIT-UP (DRIFTING)

Drift Pin Diameter = Hole Diameter  
Hole Ø = 15/16" for girder splice  
Hole Ø = 13/16" for X-frame or diaphragm connection.



DRIFT PIN



ELEVATION SHOWING ERECTION PROCEDURE

(Drive in all drift pins while one piece is suspended by a crane, whether on the ground or in the air.)

Fit Up

During the fit up, install drift pins in all corner bolt holes, plus 25 percent of the bolt holes (as a min.), evenly distributed throughout the splice. Fill at least 25 percent of the bolt holes with high strength bolts. Fully tighten these bolts by the calibrated turn-of-the-nut method before removing any drift pins or moving the members. These bolts may be either erection bolts or production bolts. Erection bolts are used during fit up, to compress the plies of the splice to achieve a snug condition. Erection bolts are the property of the Contractor and do not remain in the bridge permanently. Erection bolts must be A325, and can be reused. Erection bolts are required when the abutting plates are of different thickness and no fill plate is provided. This situation usually results in a slight bending of the splice plates. If erection bolts are not used, the DTI's may fully compress before the plates are in firm contact. This would be cause for rejecting the splice. Clearly mark the erection bolts so that they are not left in the splice.

Erection

Two independent crews will survey the bearing seat elevations. The Engineer will verify that the results of those surveys show that the bearing seat elevations are within ± 1/4 inch of the plan elevations before erection begins. Use the blocking diagram, as shown on the shop drawings, when erecting the beams/girders on the ground. Do not lift the assembled pieces into position until at least 25 percent of the holes are filled with fully tightened bolts. Locate the centerline of the bearing stiffener with the centerline of bearing device. Secure the beams/girders to the top of the pier cap prior to placement of the bearing device anchor bolts.

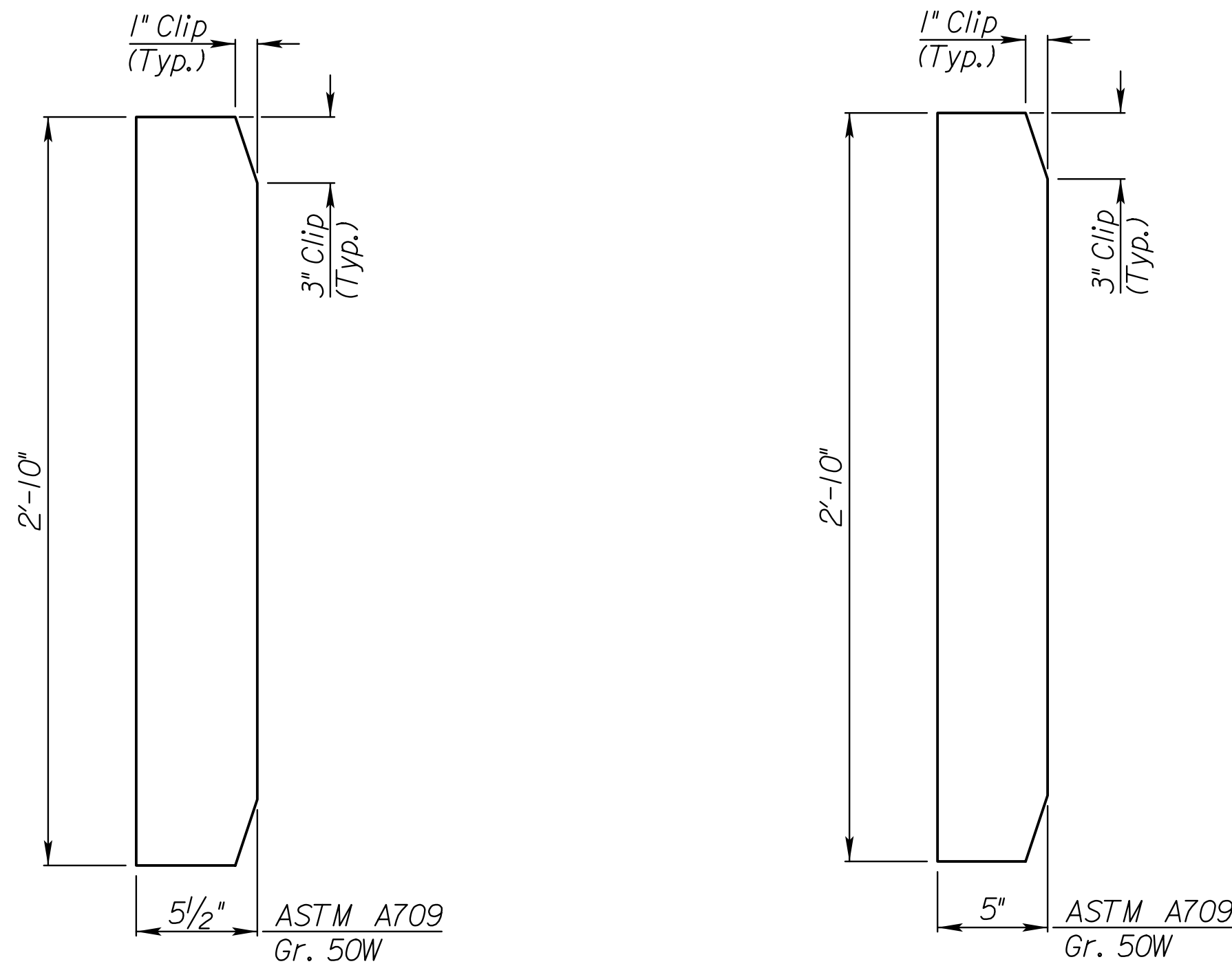
Turn-of-the-Nut Calibration Process

Tighten all girder splice and diaphragm bolts using the calibrated turn-of-the-nut method. Use the DTI to determine the turn required for each bolt diameter & length. Perform the calibration process as described below on the actual beam splice or using 3 plies of steel plate with the same thickness as the actual splice.

1. Bring at least 25 percent of the bolts in the splice to a "snug-tight-condition". "Snug tight condition" is defined as (with all plies in firm contact) "the full effort of a man on a spud wrench". Usually a smaller impact gun (1/2" drive) is used to snug the splice and a larger impact gun (1" drive) is used for final tightening. This is preferred over the use of a spud wrench. Production bolting and calibration must use the same tools and lubricating procedures. If an impact wrench is used to "iron the plates" and snug the bolts for calibration, then an impact wrench must be used during the snugging process during production bolting.
2. See "Required Marking Detail" (choose a bolt at the center of the splice and recheck snug on adjacent bolts)
  - a. Mark the outside of the socket at one of the corners.
  - b. Mark the bolt, plate, and nut at a corner with a start line.
  - c. Align the mark on the socket with the start mark on the bolt end.
  - d. While holding a backup wrench on the head of the bolt, turn the nut turn (3 flats).
  - e. Record the number of refusals.
  - f. If all of the gaps refuse, go to another bolt and turn the nut 2 flats (1/3 turn).
  - g. If there are fewer than 3 refusals turn the nut an additional 1/4
  - h. Repeat step g, turning the nut 1/3 of a flat or less each time, until all of the gaps refuse the feeler gage. Record the amount required to cause all of the gaps to refuse the feeler gage. This is the target rotation.
3. Repeat this process for each bolt diameter and length.

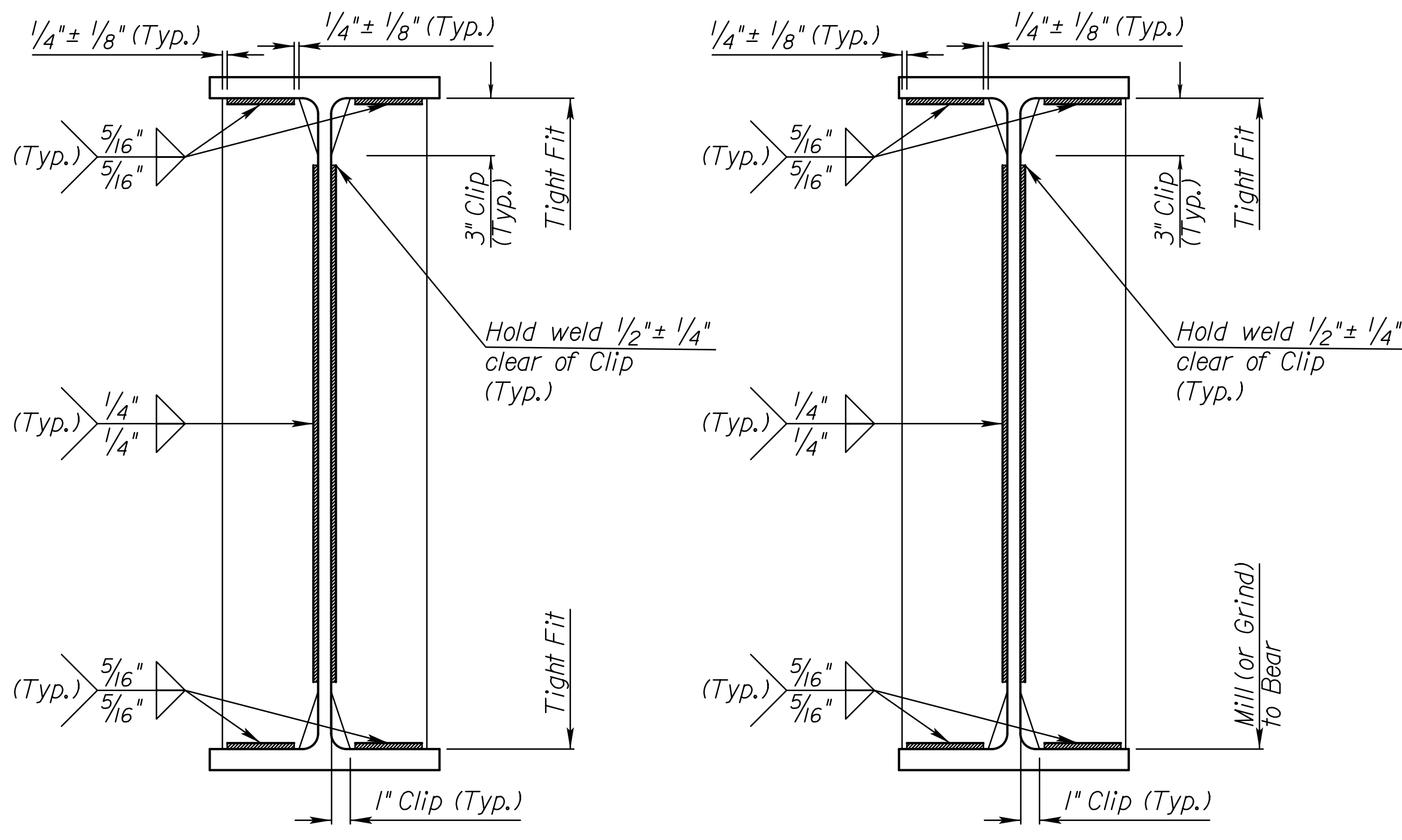


STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	52	141



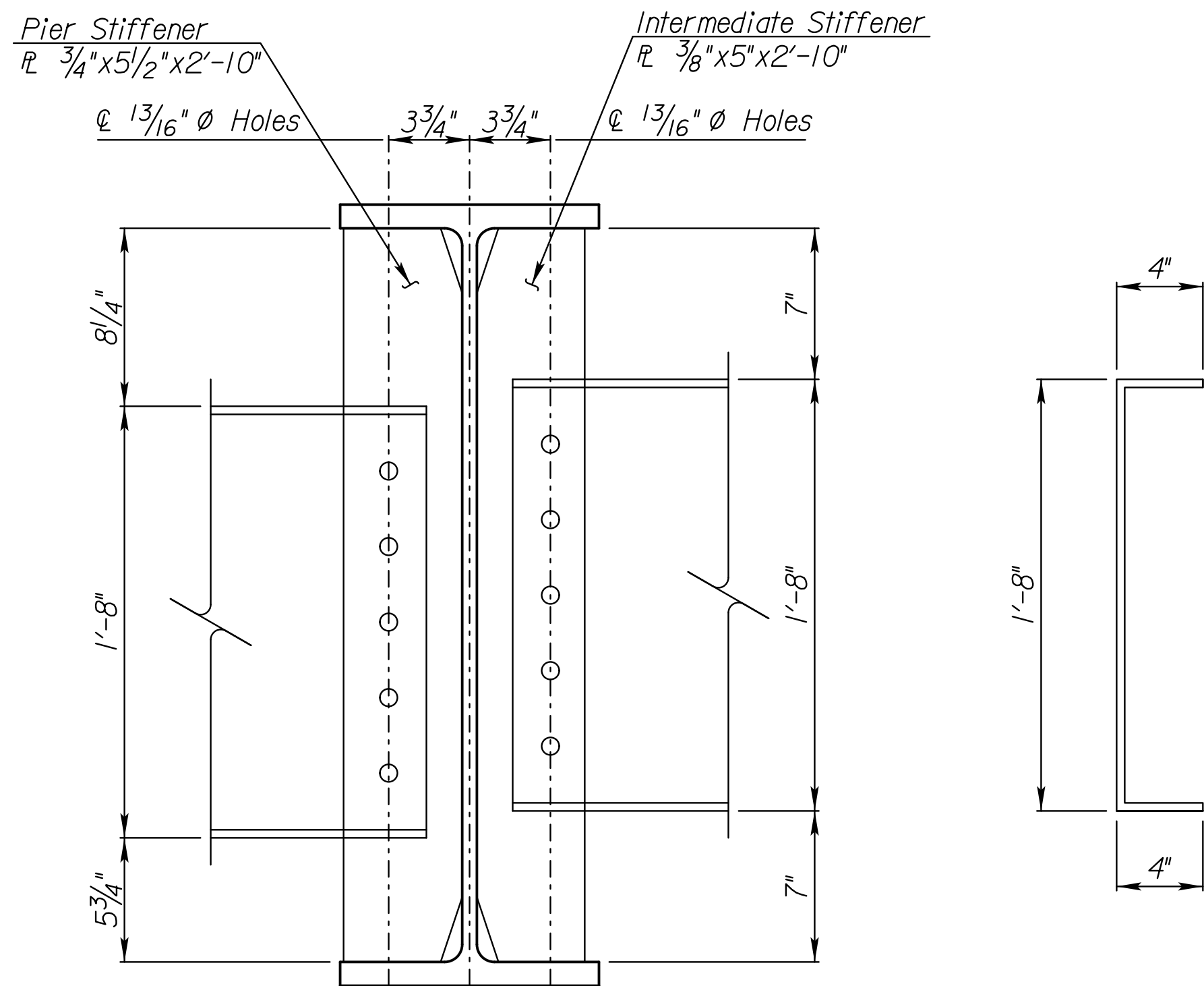
PIER STIFFENERS  
PL 3/4"x5 1/2"x2'-10"  
(ASTM A709 Gr. 50W)  
(24 Req'd)

INTERMEDIATE STIFFENERS  
PL 3/8"x5"x2'-10"  
(ASTM A709 Gr. 50W)  
(130 Req'd)

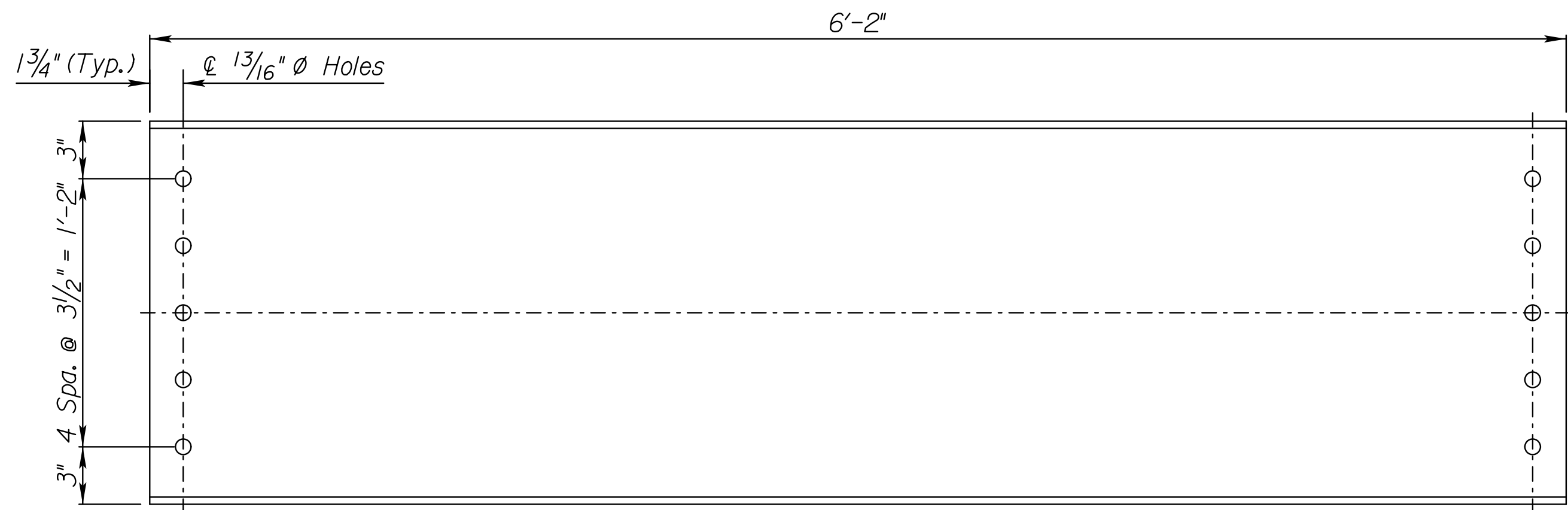


CONNECTION STIFFENER WELDS

PIER STIFFENER WELDS



PIER AND INTERMEDIATE DIAPHRAGM CONNECTIONS



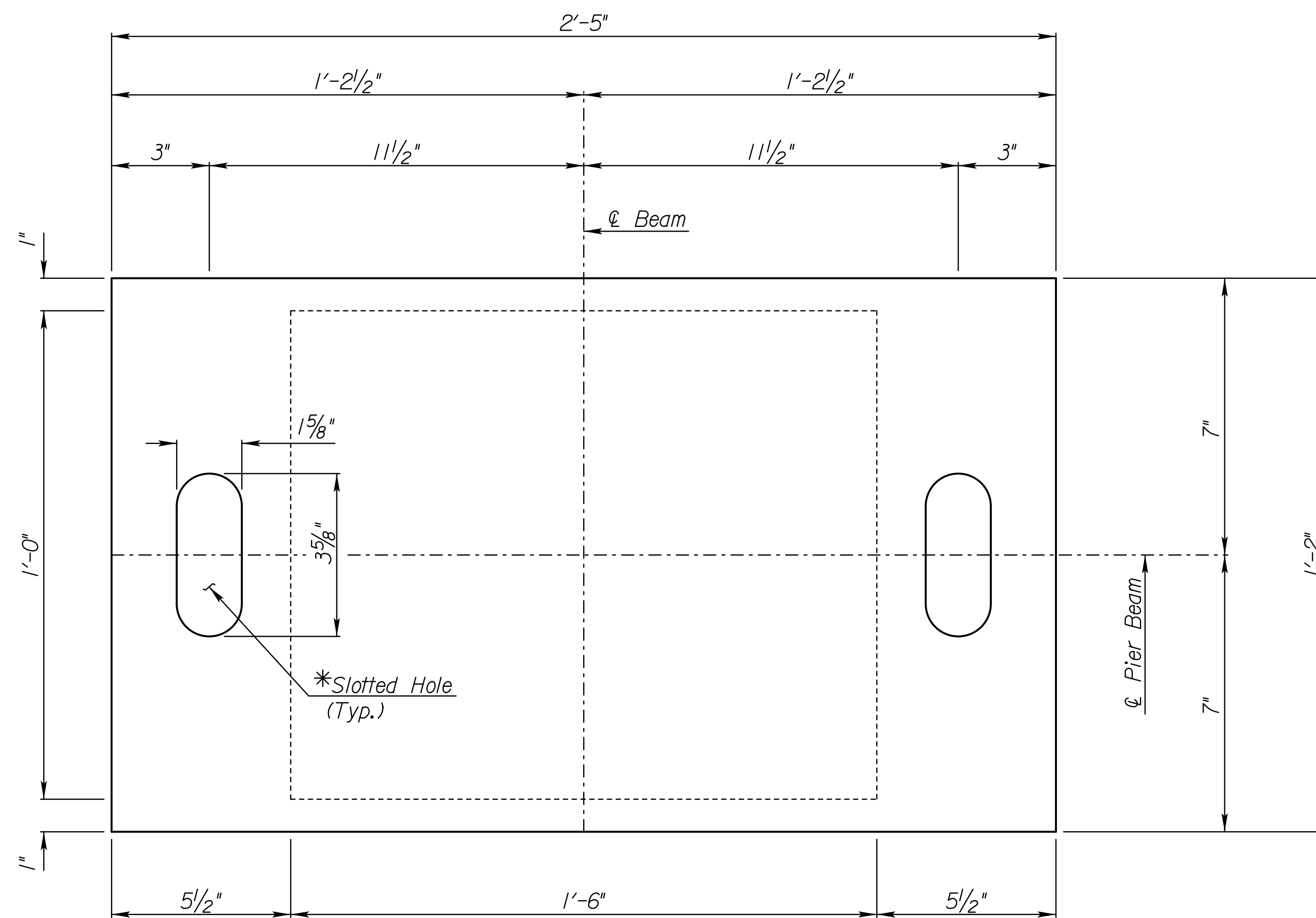
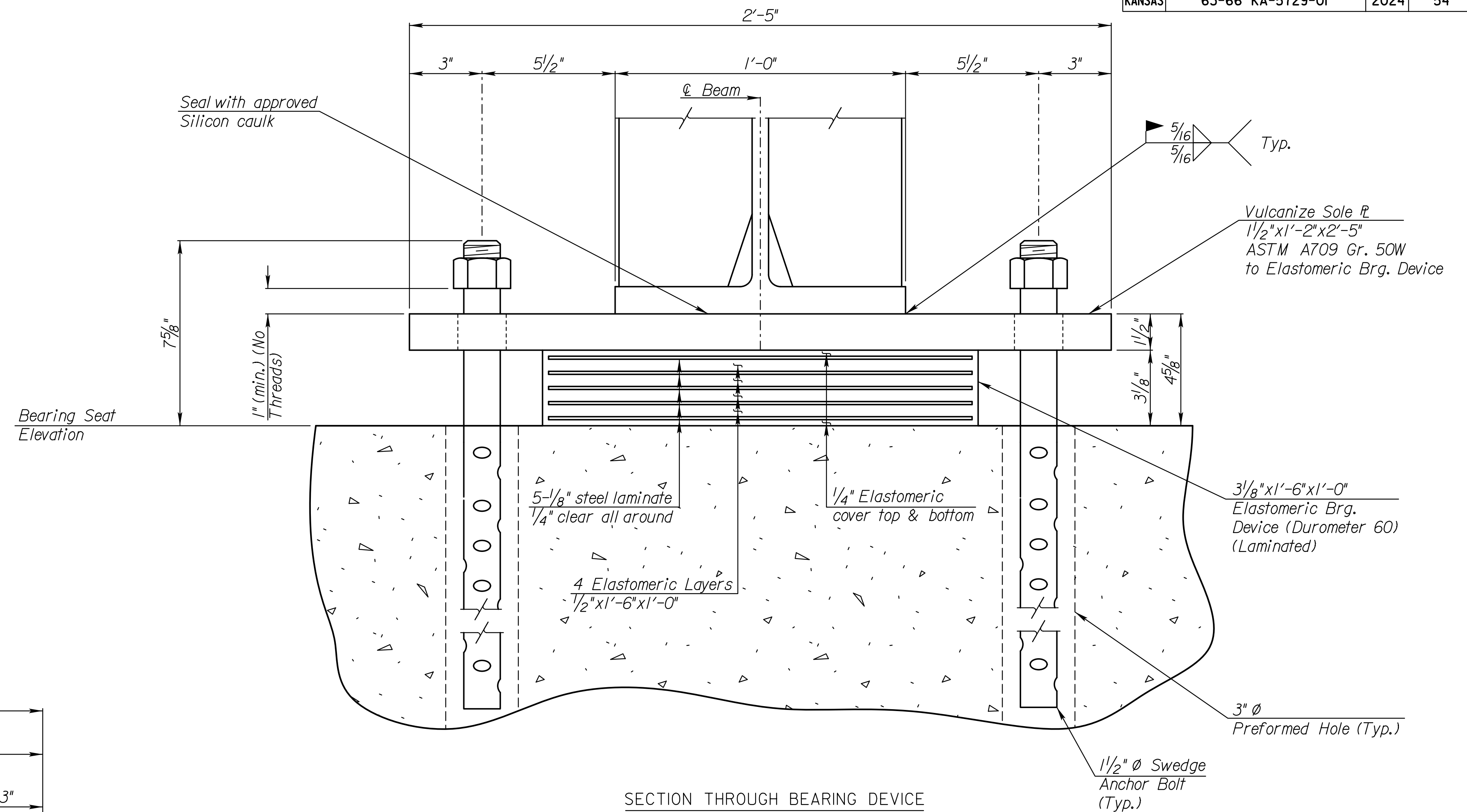
INTERMEDIATE & PIER DIAPHRAGM MEMBER  
(3/8"x2'-4"x6'-2")  
(75 Req'd.)

3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) S+a. 62+23.30				
DIAPHRAGM DETAILS				
Proj. 63-66 KA-5729-01			Nemaha Co.	
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.
			CFB	CADD
			JAB	CADD CK.
				KMS
				JAB



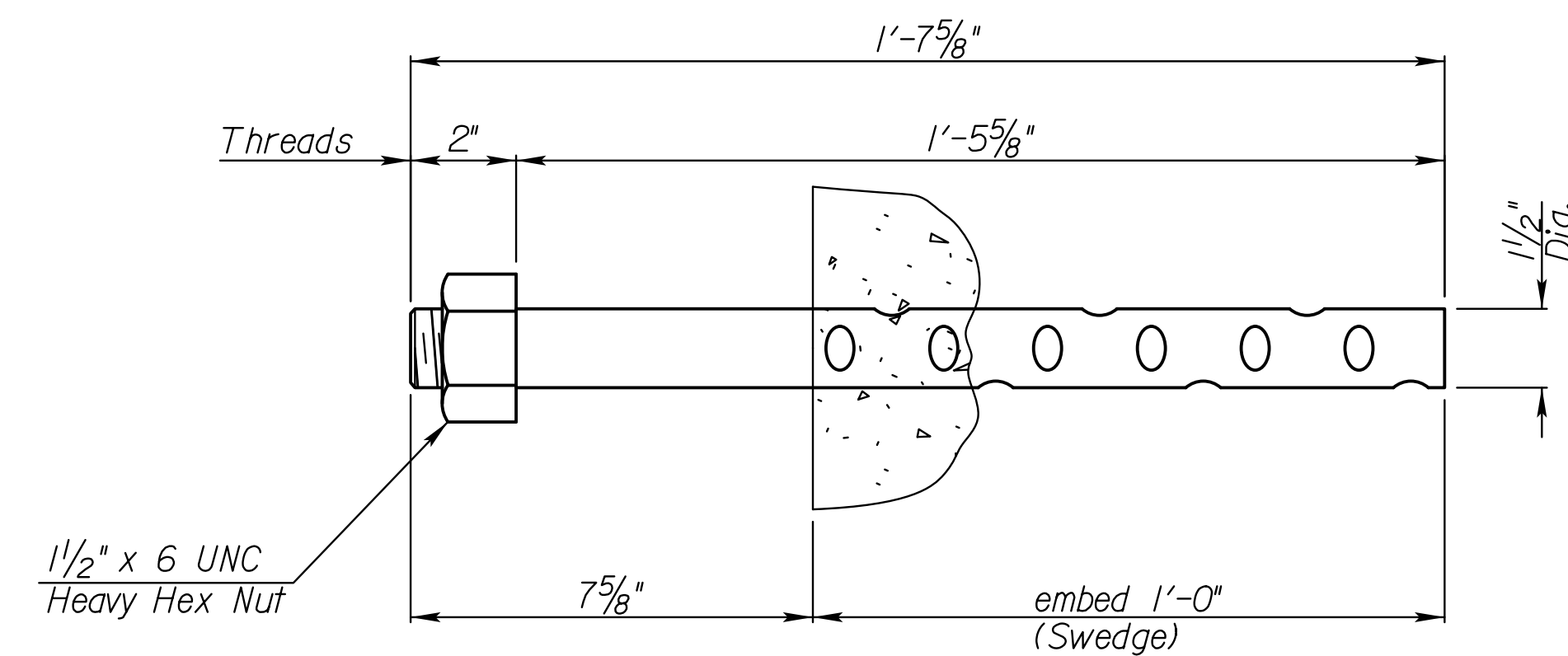
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	54	141

*The sole plate shall be ASTM A709 Gr. 50W structural steel.*



PLAN OF BEARING DEVICE FOR PIER NO.1 AND 2  
(12 required)

*\*Note: Center the anchor bolts in the center of the slotted holes regardless of installation temperature.*



SWEDGE ANCHOR BOLT  
(24 Required)

*ANCHOR BOLTS: Anchor bolts and nuts will adhere to KDOT Standard Specification Section 1600 (Grade 55) with the following exception: The threads may be rolled or cut. The bolts and nuts shall be galvanized.*

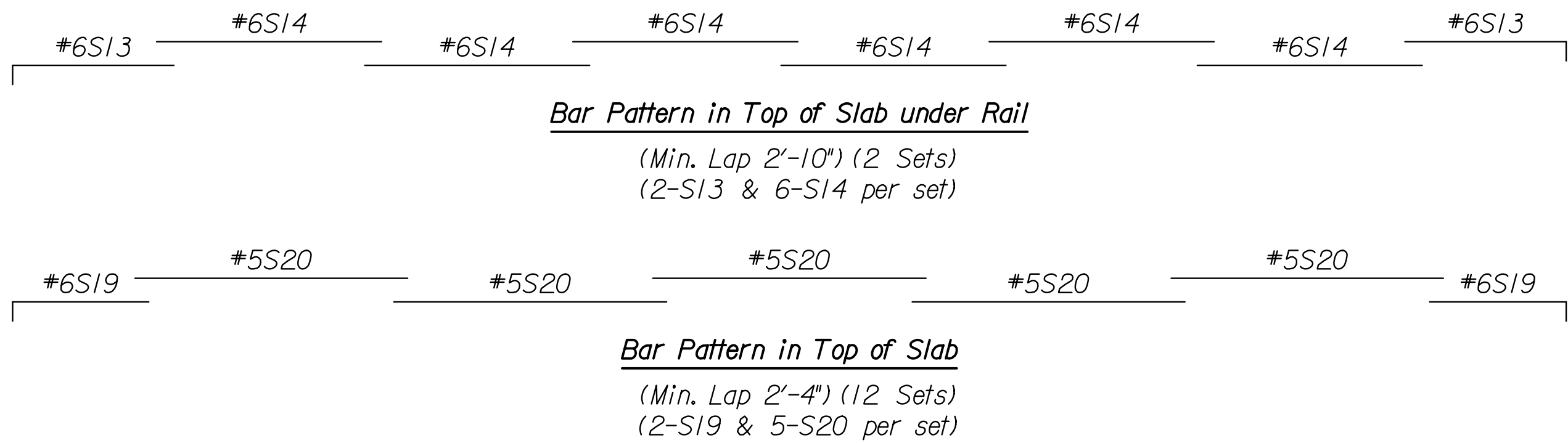
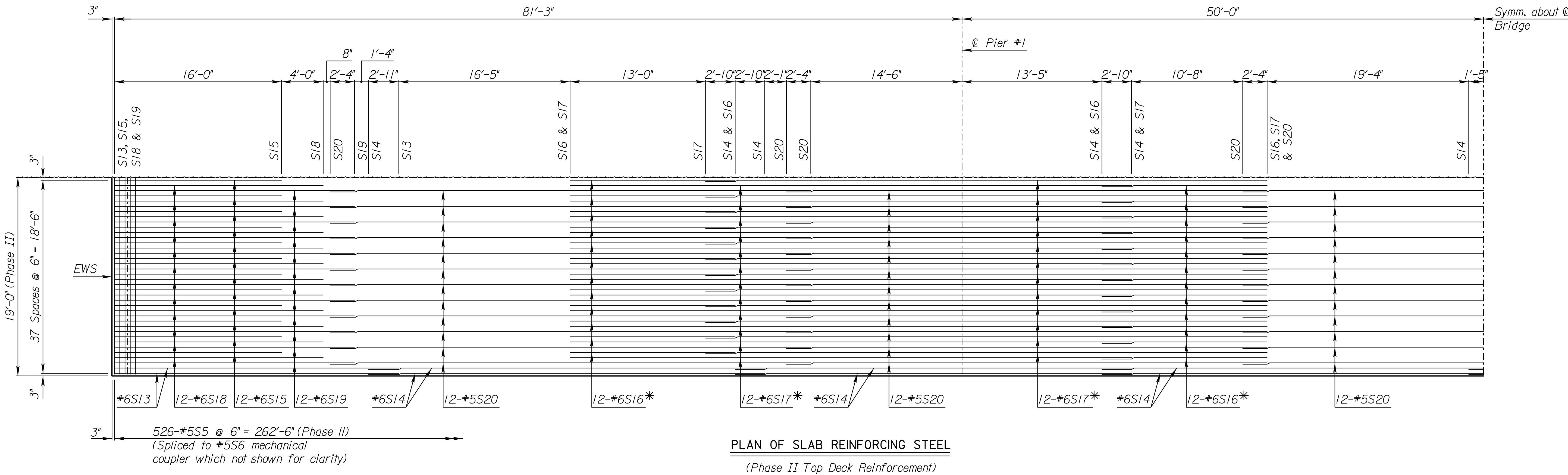
3					
2					
1					
NO.	DATE	REVISIONS			BY APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048)			Sta. 62+23.30		
BEARING DEVICE DETAILS					
Proj. 63-66 KA-5729-01			Nemaha Co.		
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB CADD KMS
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB CADD CK. JAB

Plotted: Jake.Pfannenstiel@ks.gov | Location: Bridge  
 File: ka57290lbb0048-44.dgn  
 Plot Date: 15-JUL-2024 15:59





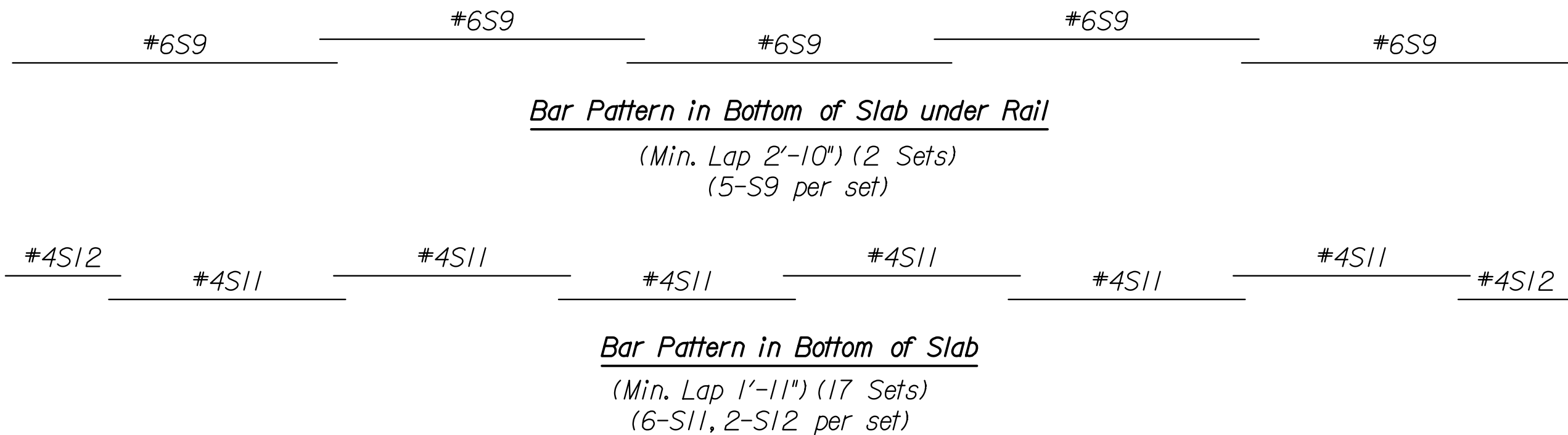
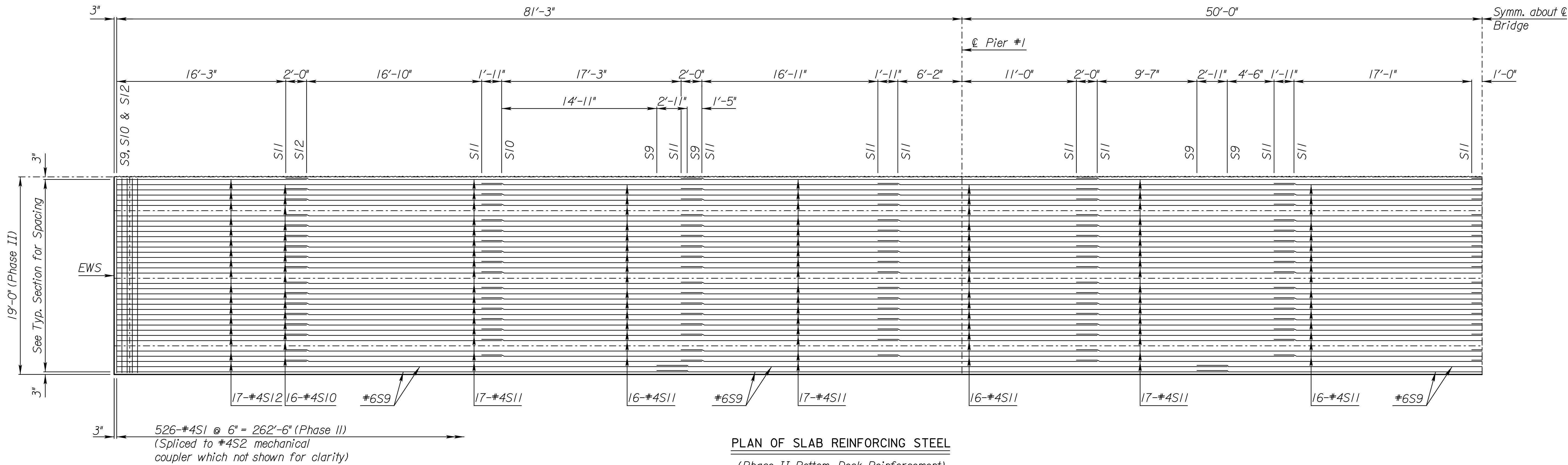
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	56	141



\*NOTE: #6S16 spliced to #6S17. (Min. lap 2'-10")

3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) Sta. 62+23.30					
SLAB DETAILS I					
Phase II Construction					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	57	141

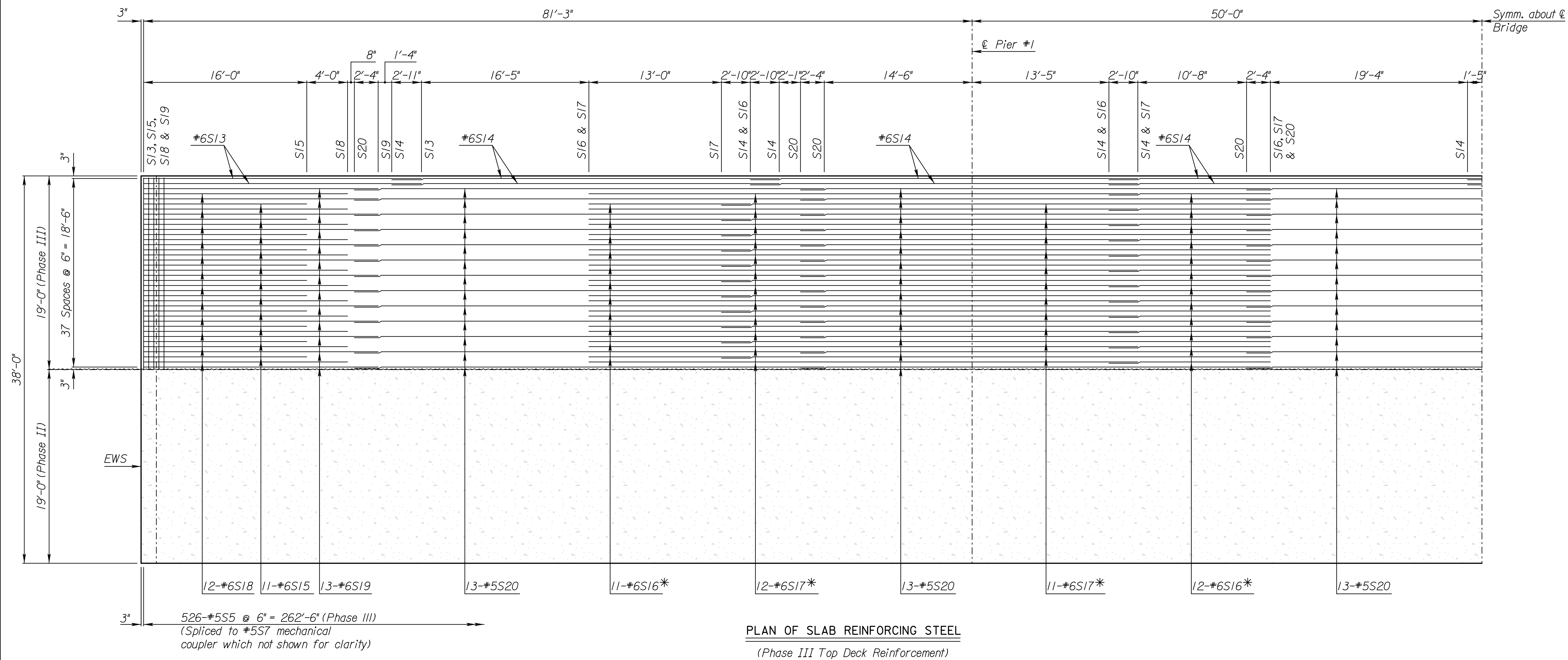


Bar Pattern in Bottom of Slab  
(Min. Lap 1'-11") (16 Sets)  
(5-S11, 2-S10 per set)

3					
2					
1					
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) Sta. 62+23.30					
SLAB DETAILS 2					
Phase II Construction					
Proj. 63-66 KA-5729-01				Nemaha Co.	
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB
				CADD	KMS
				CADD CK.	JAB



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	58	141



PLAN OF SLAB REINFORCING STEEL  
(Phase III Top Deck Reinforcement)

**Bar Pattern in Top of Slab under Rail**  
 (Min. Lap 2'-10") (2 Sets)  
 (2-S13 & 6-S14 per set)

**Bar Pattern in Top of Slab**  
 (Min. Lap 2'-4") (13 Sets)  
 (2-S19 & 5-S20 per set)

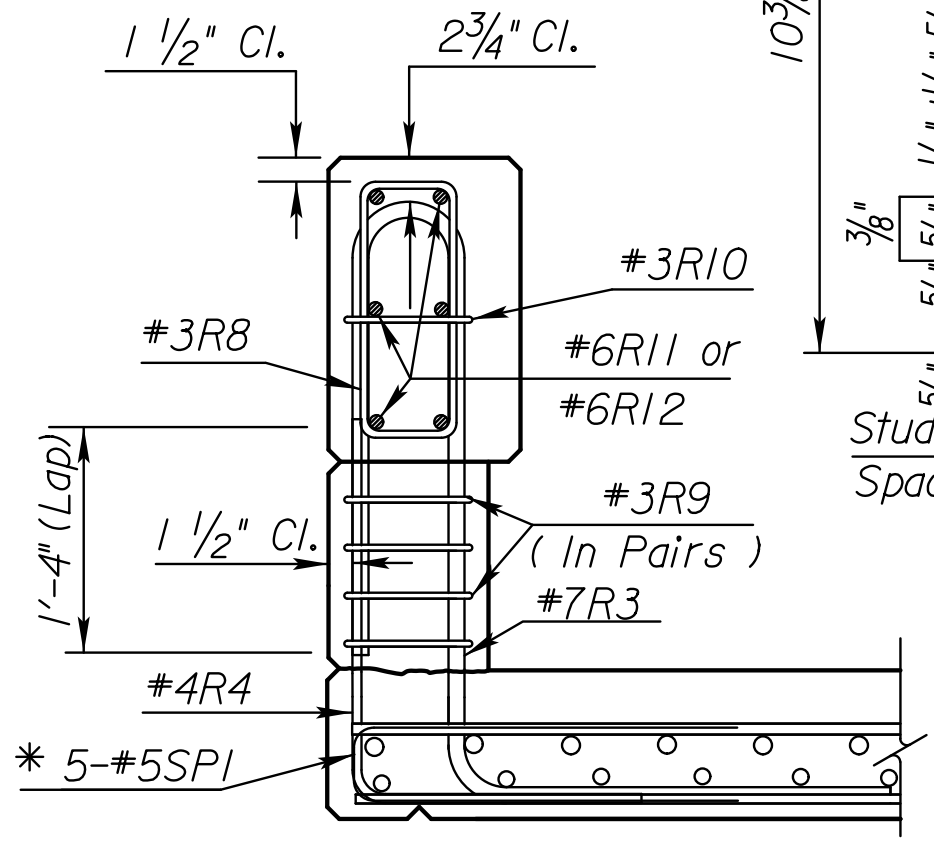
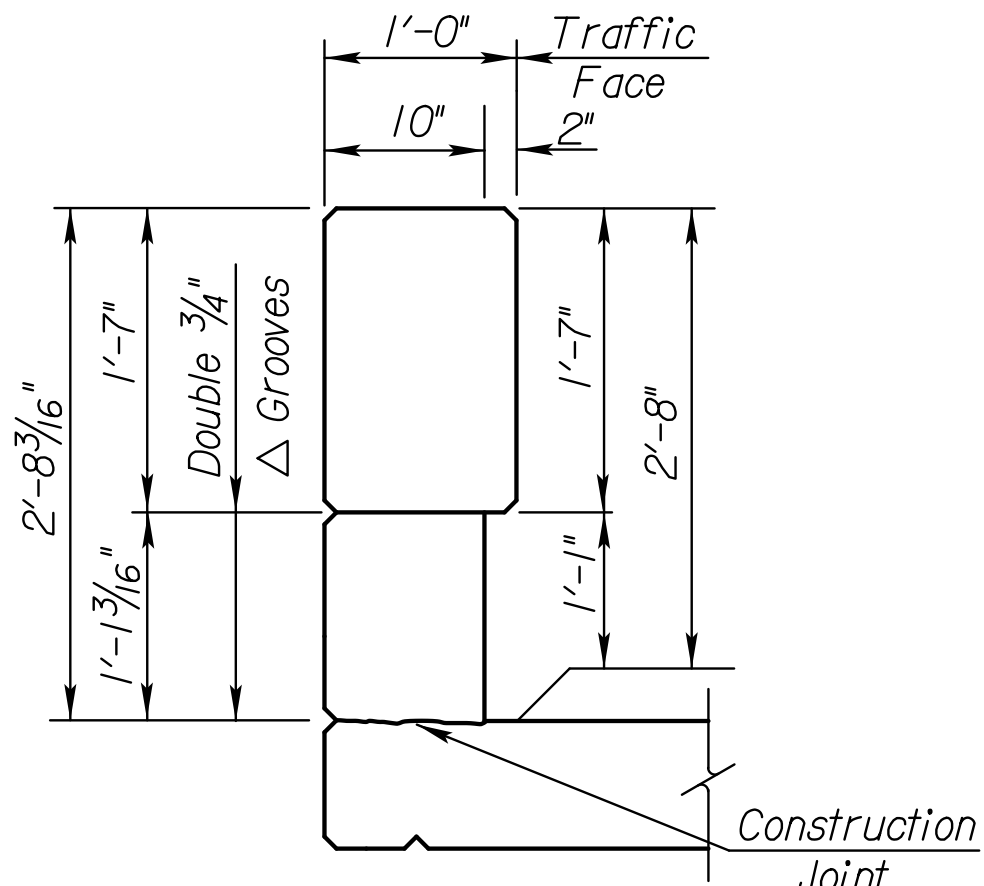
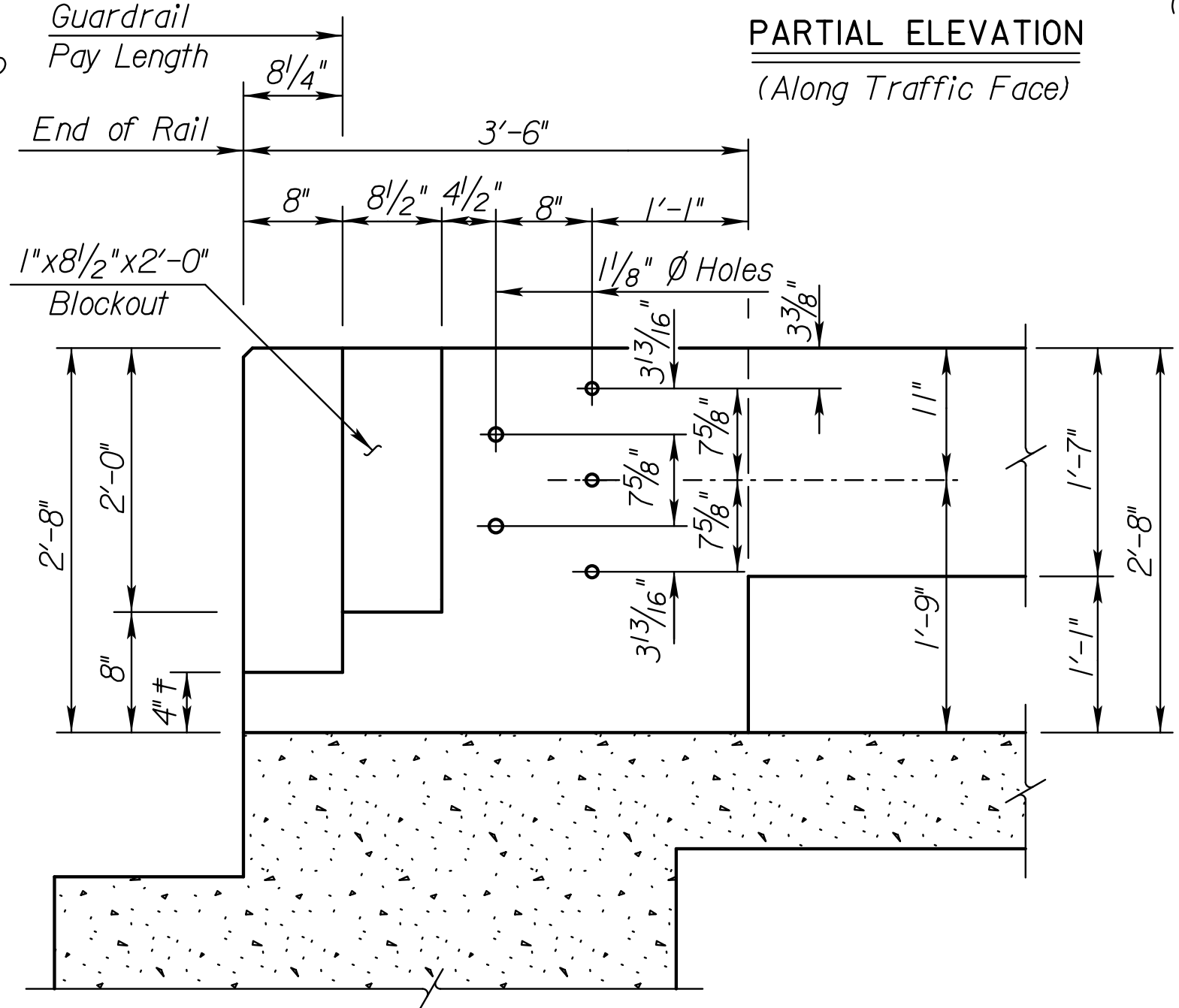
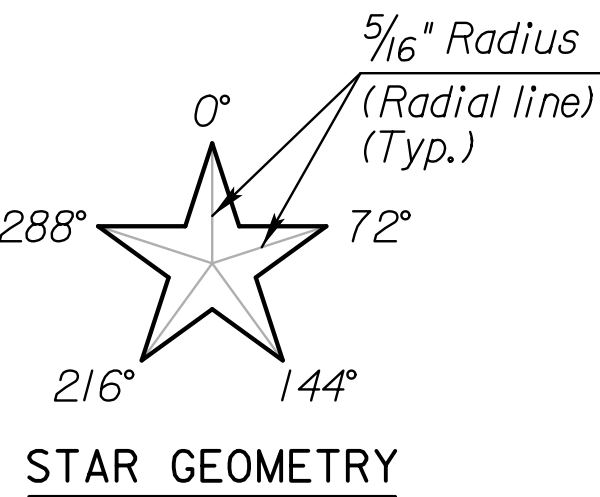
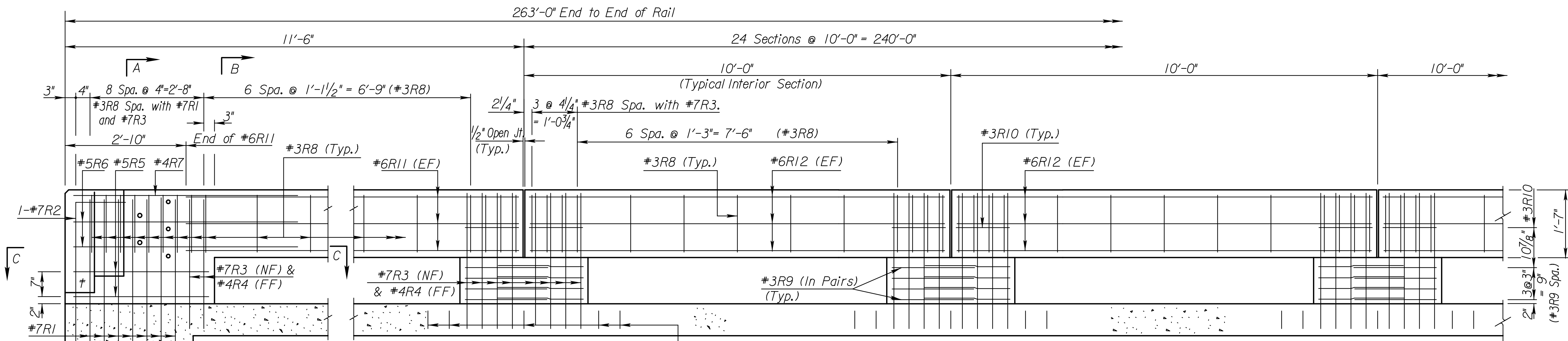
\* NOTE: #6S16 spliced to #6S17. (Min. lap 2'-10")

3						
2						
1						
NO.	DATE	REVISIONS			BY	APP'D
<p align="center"><b>KANSAS DEPARTMENT OF TRANSPORTATION</b></p> <p align="center">Br. No. 63-66-54.90 (048)      Sta. 62+23.30</p> <p align="center"><b>SLAB DETAILS 3</b></p> <p align="center">Phase III Construction</p> <p align="center">Proj. 63-66 KA-5729-01      Nemaha Co.</p>						
SHEET NO.	OF	SCALE		APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB	KMS
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB	CADD CK. JAB

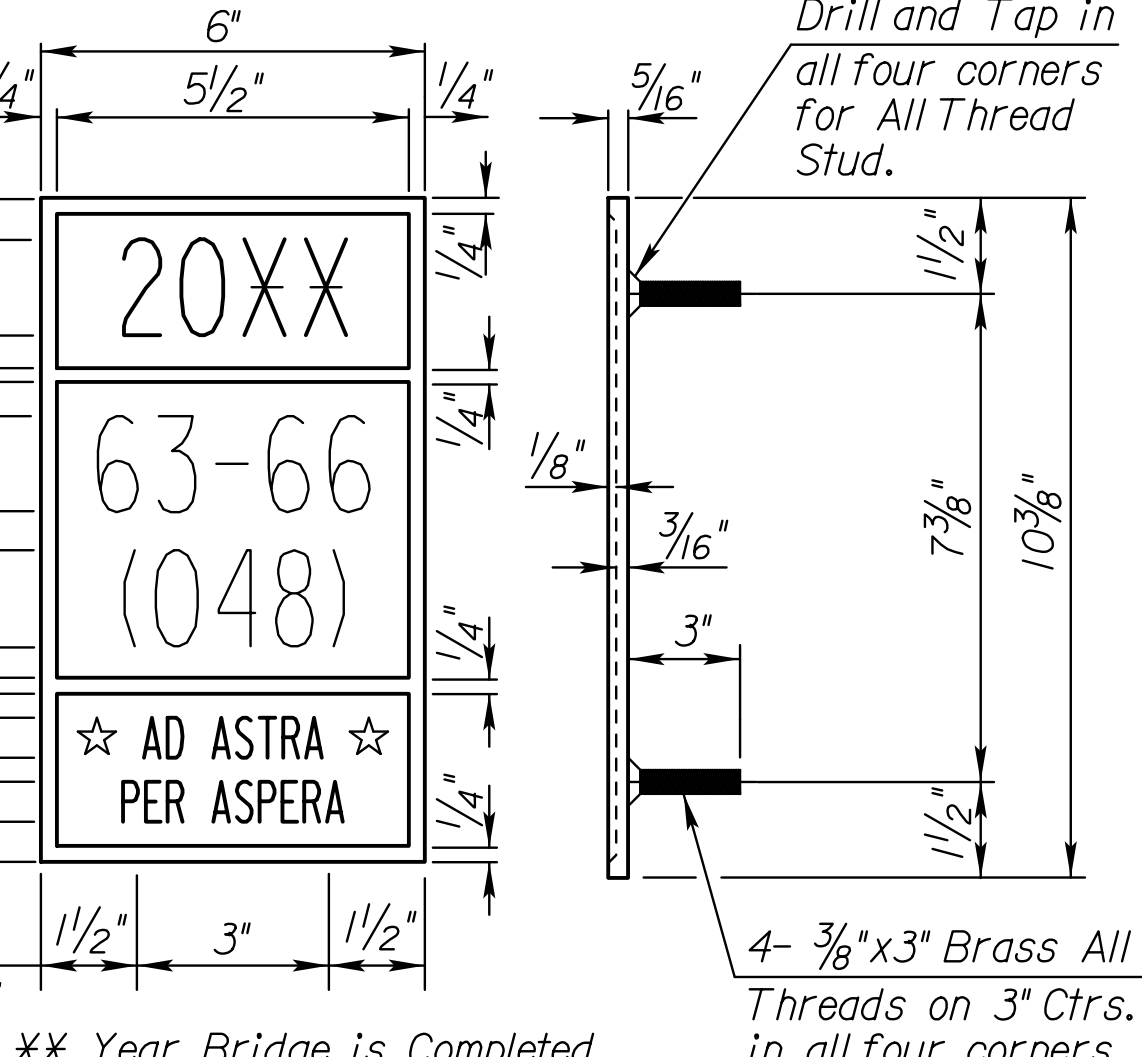
Plotted: Jake.Pfannenstiel@ks.gov	Plot Location: Bridge
File: ka57290lbb0048-53.dgn	
Plot Date: 15-JUL-2024 16:00	



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	60	141

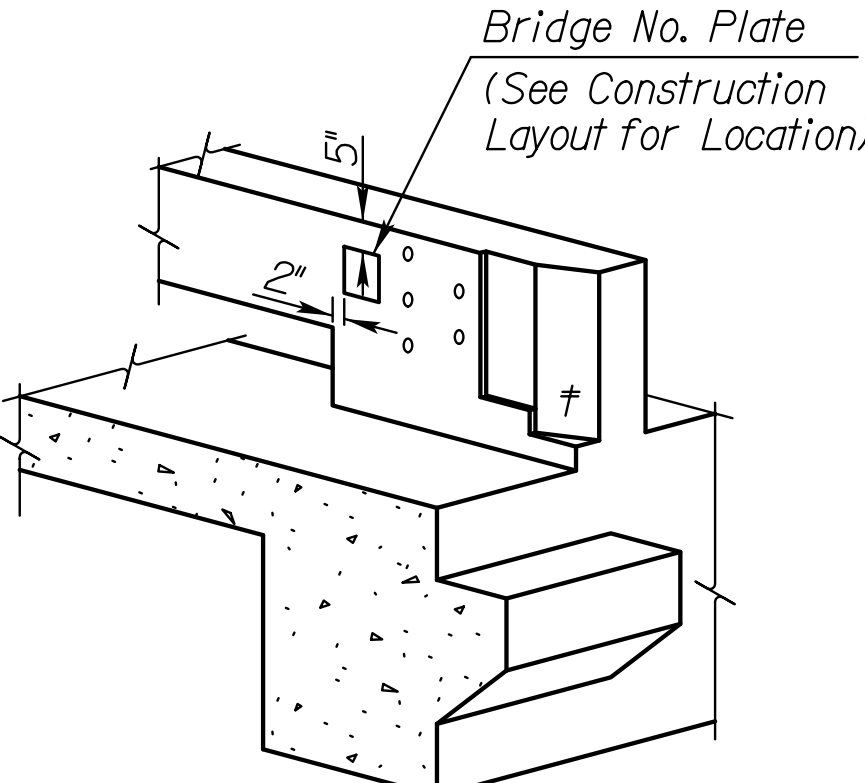


\* Note:  
The hook may be canted  
to provide clearance  
and/or fit between  
reinforcing.

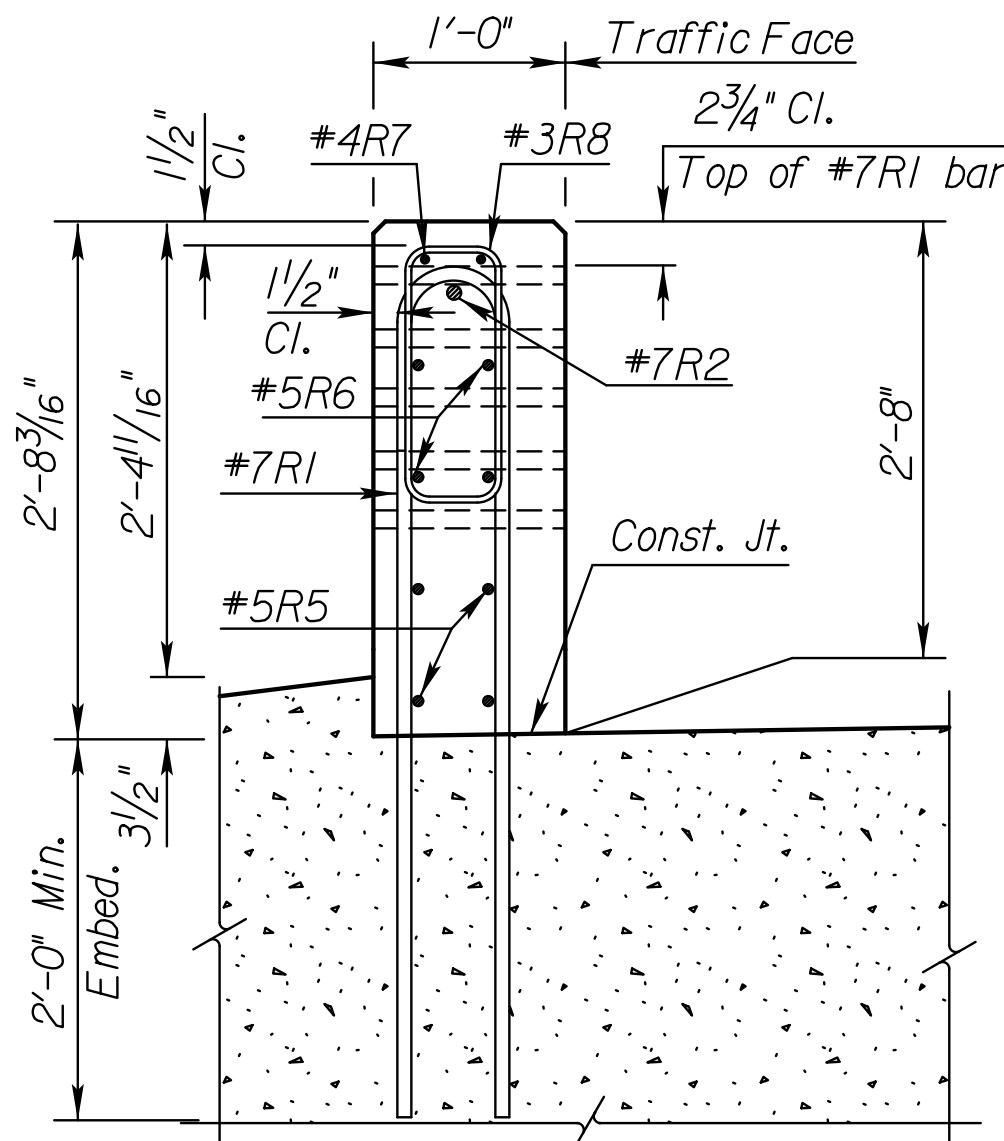


BRIDGE NUMBER PLATE

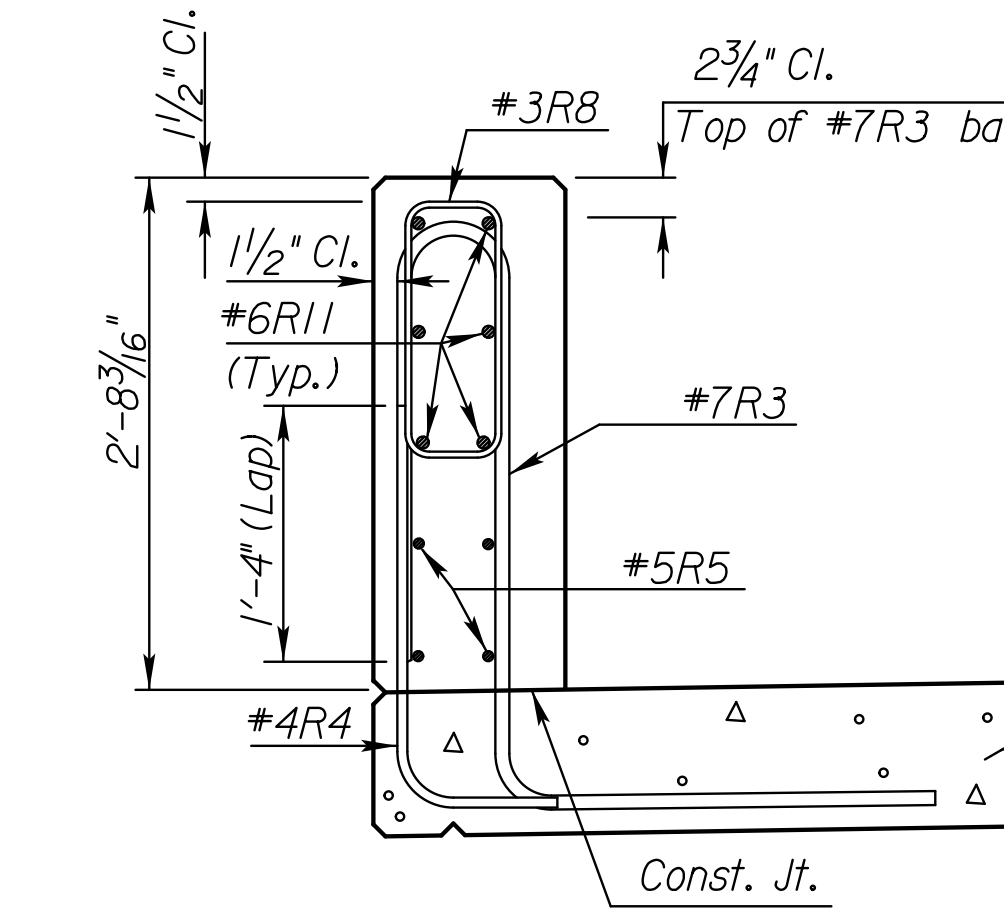
( 1 Required )  
( See Construction Layout for Location )



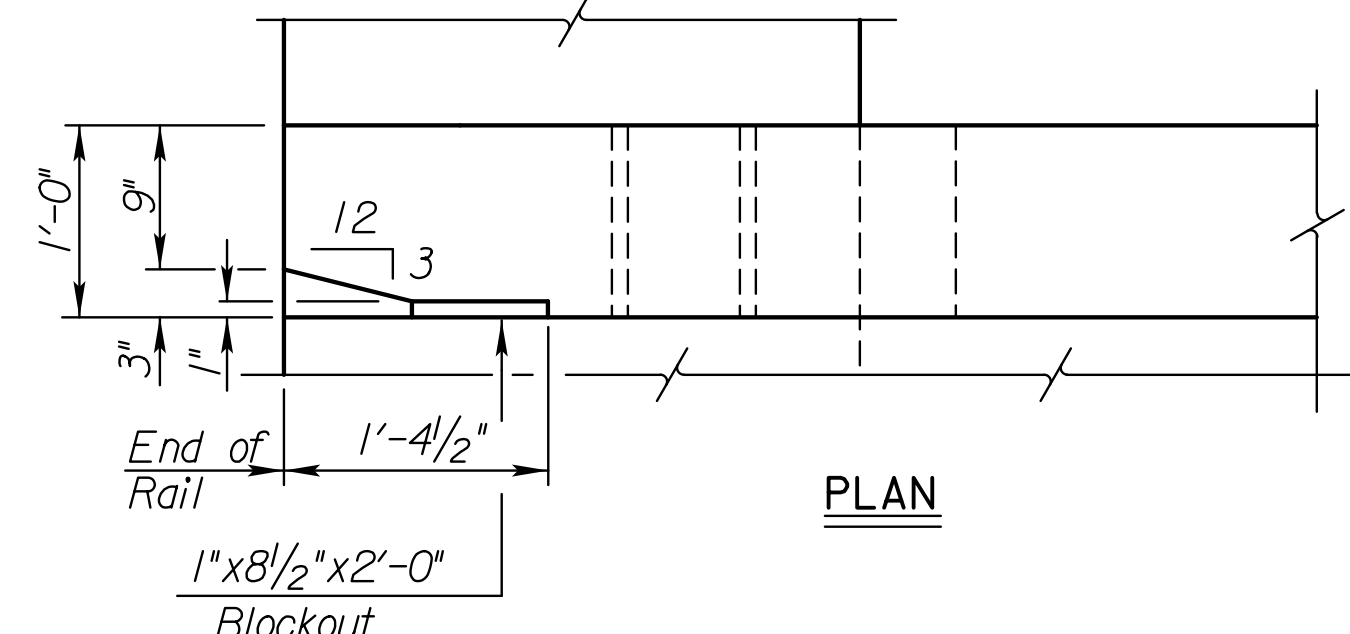
BRIDGE NUMBER PLATE PLACEMENT DETAIL



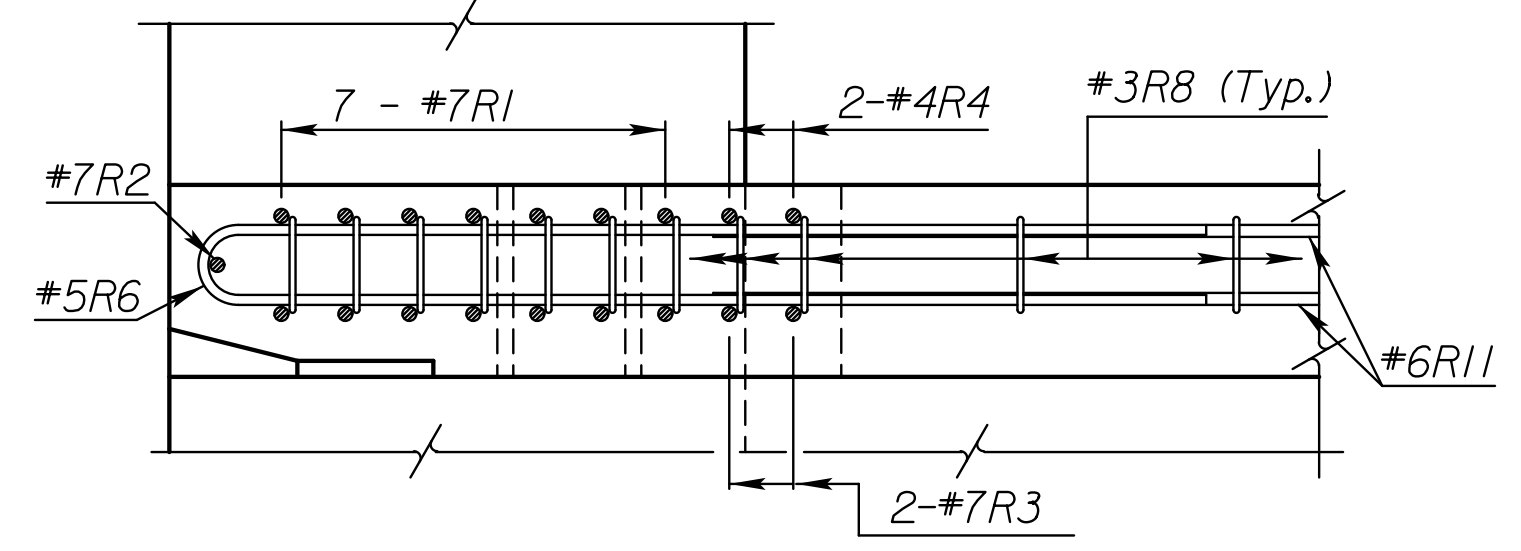
SECTION A-A



SECTION B-B



PLAN



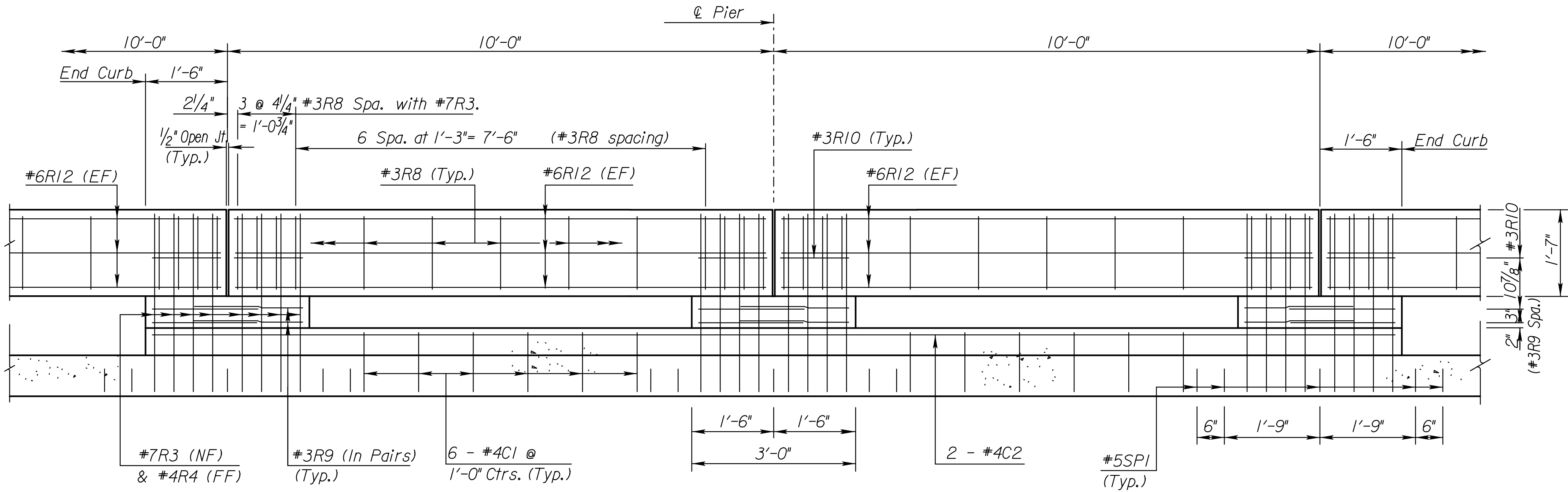
SECTION C-C

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 63-66-54.90 (048) S+a. 62+23.30				
CORRAL RAIL DETAILS I				
Proj. 63-66 KA-5729-01 Nemaha Co.				
SHEET NO. OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.

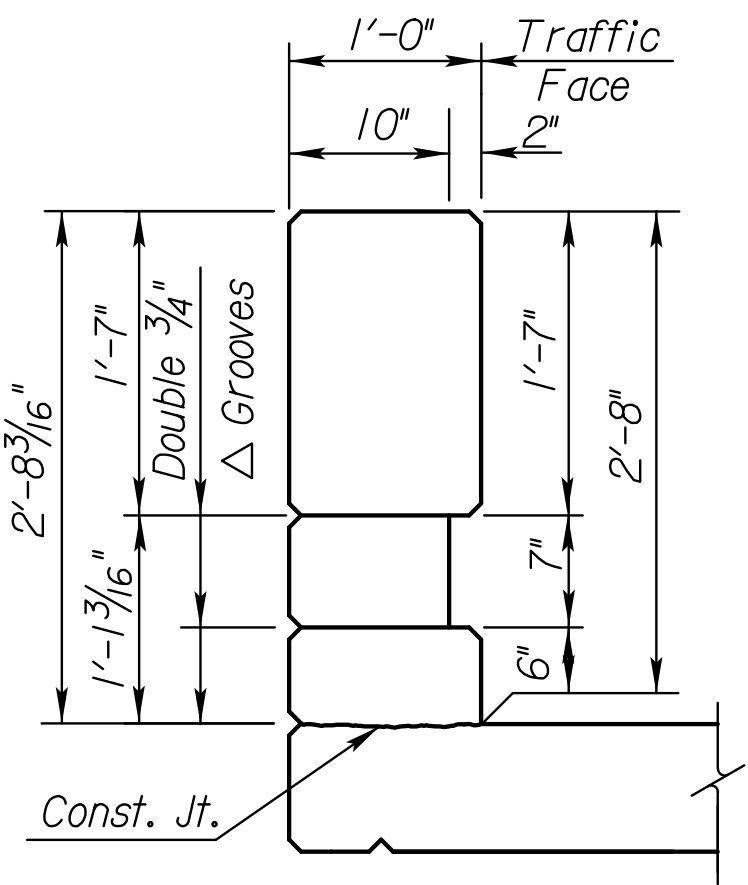
Plotted: J:\ke.P\amensfield\ks.gdb\ot Location: Bridge  
File: ka572901bbr0048-56.dgn  
Plot Date: 15-JUL-2024 15:59



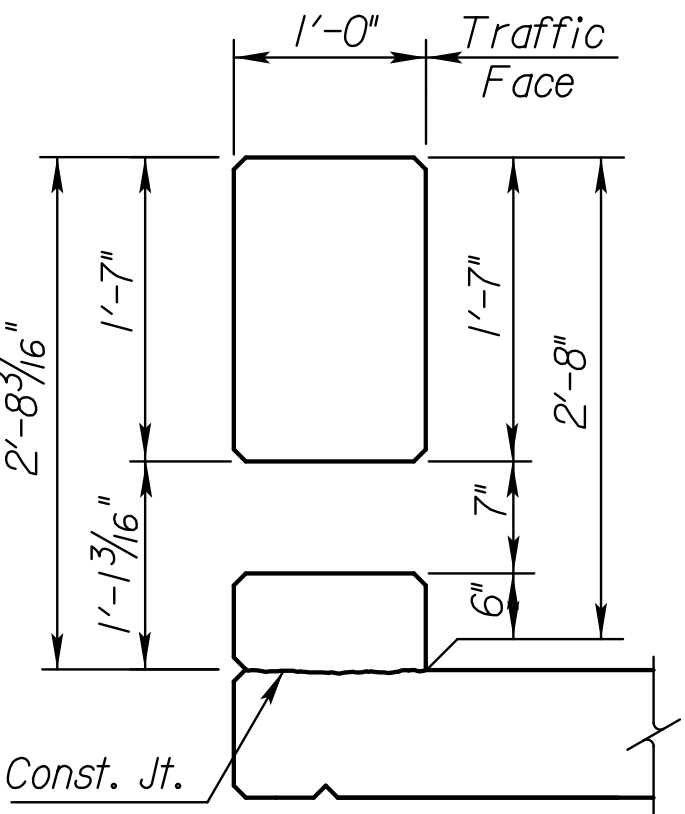
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	61	141



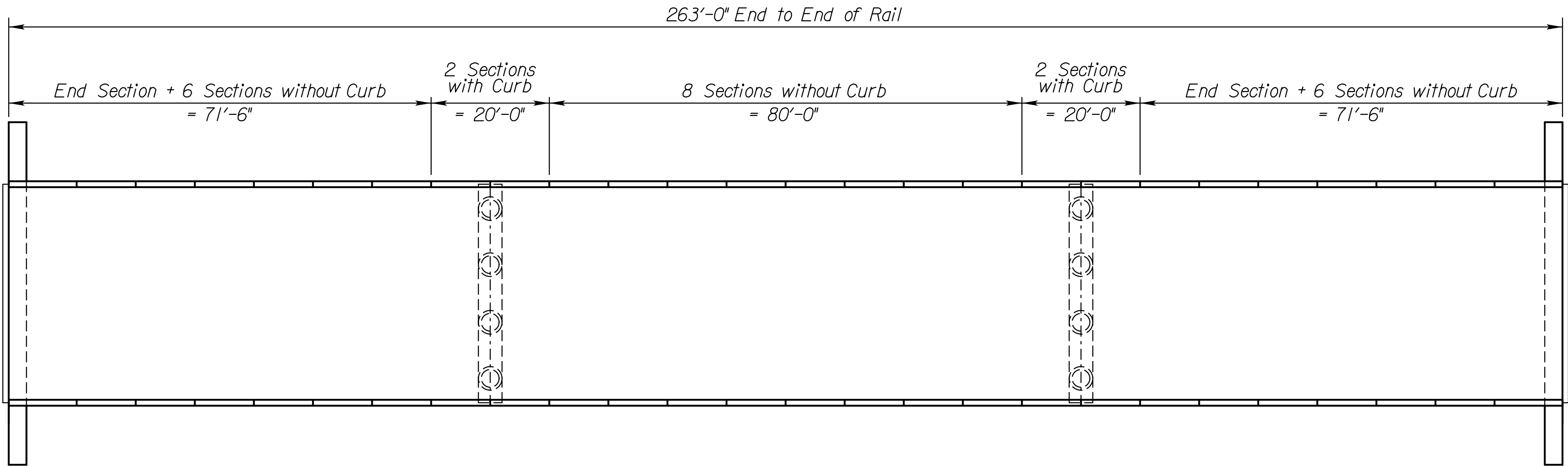
PARTIAL ELEVATION AT PIERS  
(Along Traffic Face)



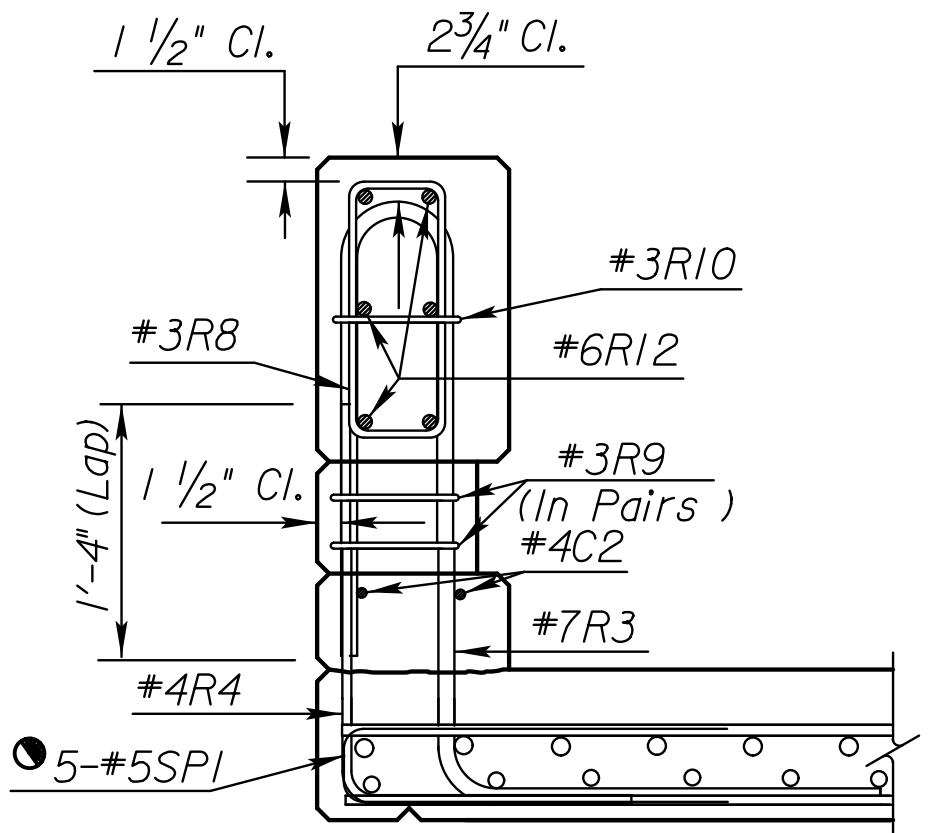
TYPICAL INTERIOR POST  
(With Curb)



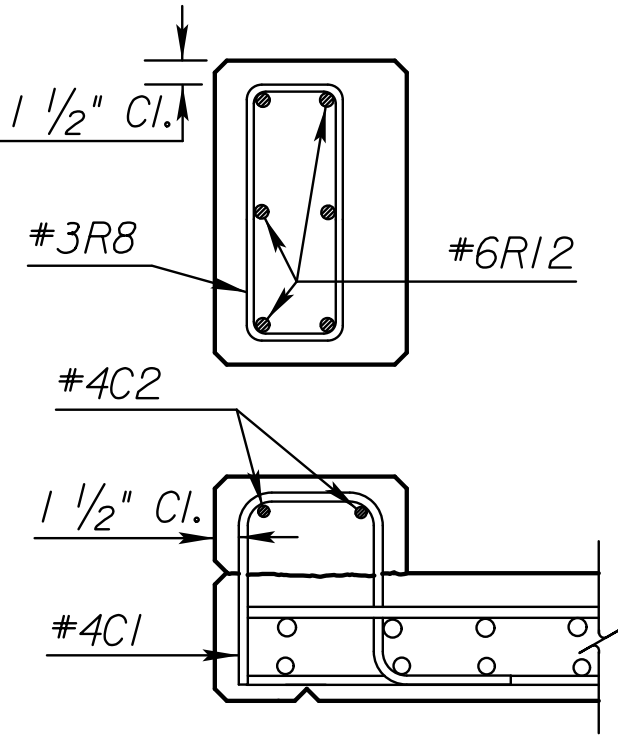
TYPICAL SECTION BETWEEN POSTS  
(With Curb)



RAIL SECTION LAYOUT



SECTION THRU POSTS  
(With Curb)



TYPICAL SECTION BETWEEN POSTS  
(With Curb)

The hook may be canted to provide clearance and/or fit between reinforcing.

Legend  
EF= Each Face  
FF= Far Face  
NF= Near Face

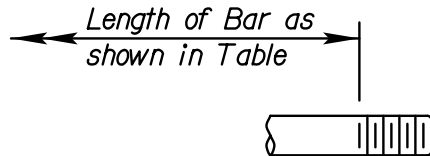
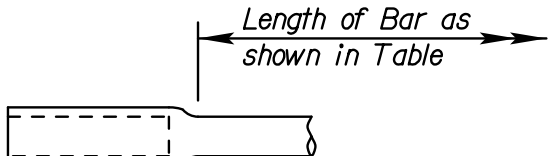
3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) S+a. 62+23.30					
CORRAL RAIL DETAILS 2					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB

Plotted: Coper.Bettiseks.gov | Plot Location: Bridge  
File: ka572901bbr0048-61.dgn  
Plot Date: 15-JUL-2024 20:52

BILL OF REINFORCING STEEL Epoxy Coated - Grade 60							
Straight Bars				Bent Bars			
Mark	Size	Number	Length	Mark	Size	Number	Length
A1	#8	32	28'-8"	A14	#8	20	10'-0"
A6	#8	20	21'-6"				
				R1	#7	28	9'-5"
R11	#6	24	8'-6"	R2	#7	4	5'-8"
R12	#6	288	9'-8"	R3	#7	408	7'-7"
S9	#6	20	54'-10"				
S14	#6	24	38'-0"	S13	#6	8	29'-9"
S16	#6	94	15'-10"	S15	#6	48	18'-6"
S17	#6	94	53'-10"	S18	#6	48	22'-6"
				S19	#6	48	25'-6"
A2	#5	40	28'-8"				
A3	#5	8	28'-2"	A7	#5	116	15'-4"
A4	#5	8	25'-6"	A8	#5	12	⊗
A5	#5	8	23'-6"	A9	#5	128	8'-8"
S5	#5	1052	18'-8"	A10	#5	72	11'-2"
S20	#5	120	46'-1"	A11	#5	24	10'-8"
				A12	#5	12	⊗
A17	#4	4	18'-2"	A13	#5	20	⊗
C2	#4	8	22'-8"	A15	#5	44	6'-6"
S1	#4	1052	18'-8"	R5	#5	8	6'-6"
S10	#4	64	37'-0"	R6	#5	8	10'-8"
S11	#4	363	40'-0"	SPI	#5	250	4'-5"
S12	#4	66	18'-3"				
				A16	#4	80	7'-0"
				A18	#4	80	4'-3"
				A25	#4	16	⊗
				C1	#4	48	3'-5"
				R4	#4	408	4'-2"
				R7	#4	4	10'-8"
				R8	#3	696	4'-4"
				R9	#3	352	4'-6"
				R10	#3	100	4'-6"

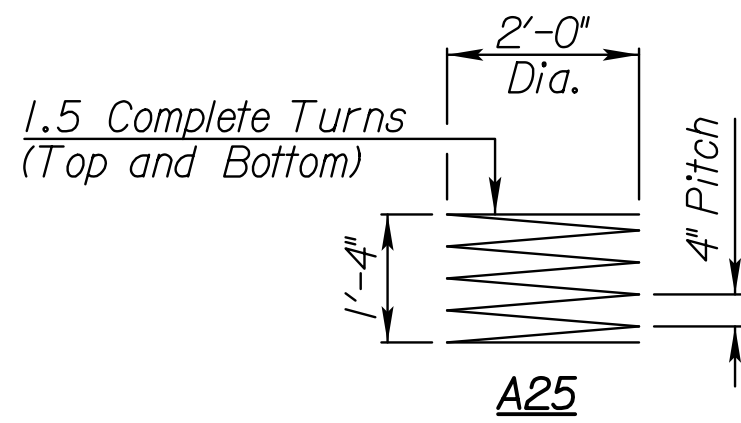
⊗ See Bending Diagram

NOTE: S4 and S8 bar marks were intentionally skipped.

	
<i>Coupled Rebar (Place in Phase III Construction)</i>	<i>Coupled End Rebar (Place in Phase II Construction)</i>
THREADED REBAR SPLICE SYSTEM	

Epoxy Coated Threaded Rebar Splices (Grade 60)							
Threaded Bars				Coupled Bars			
Mark	Size	Number	Length	Mark	Size	Number	Length
A20	#8	26	5'-1"	A19	#8	26	5'-1"
A22	#5	32	3'-3"	A21	#5	32	3'-3"
S7	#5	526	2'-7"	S6	#5	526	2'-7"
A24	#4	2	2'-7"	A23	#4	2	2'-7"
S3	#4	526	2'-1"	S2	#4	526	2'-1"

NOTE: The weight of the Threaded and Coupled rebars will be included in the weight of reinforcing steel. The Mechanical splice system shall meet the requirements of the KDOT specifications for "Fatigue Resistant" mechanical couplers. The additional material & labor shall be subsidiary to the bid item "Reinforcing Steel (Gr. 60) Epoxy Coated."

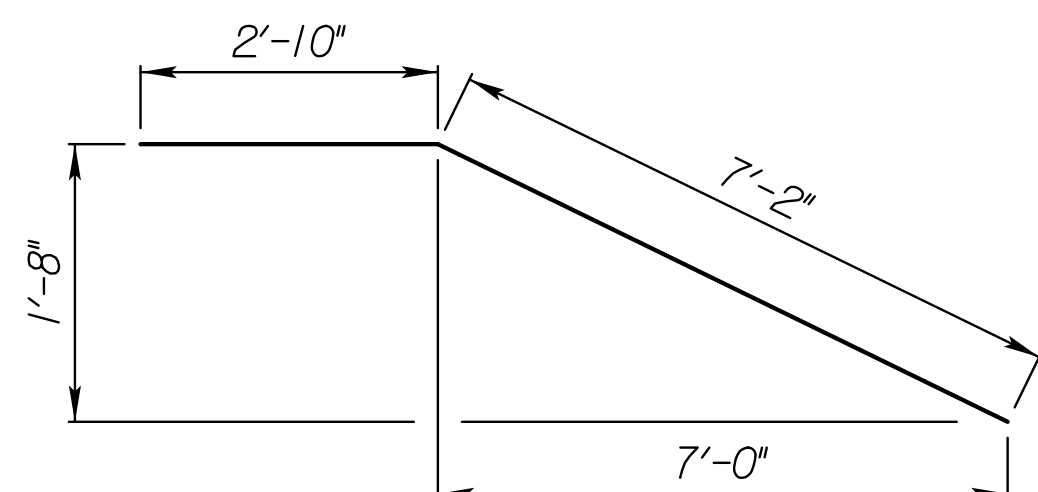


(1/2" ∅ smooth or deformed bar)

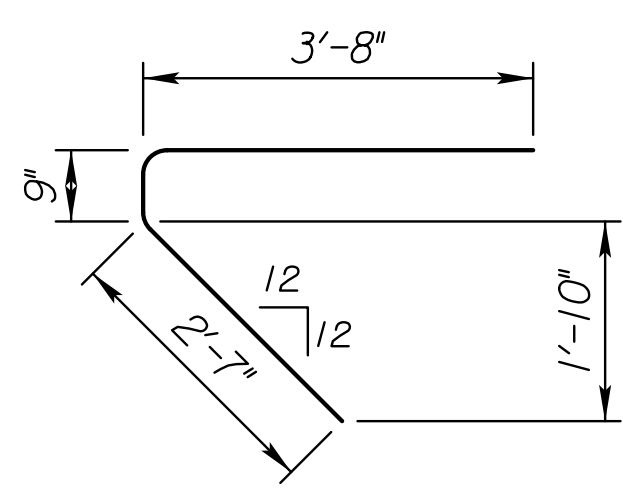
Spiral reinforcing shall meet the requirements of ASTM A615 Grade (60 or 40) or ASTM A82.

Spiral Spacer Bars:

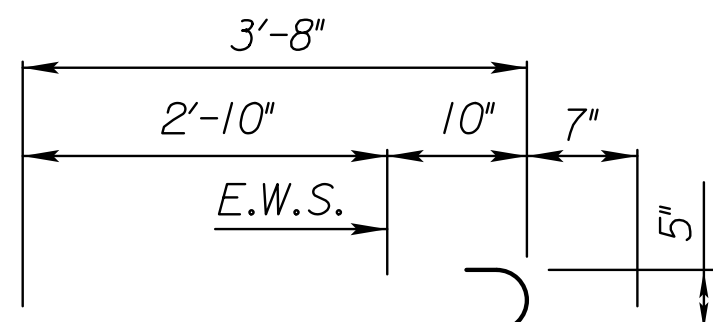
- 1) Are included in the mass of reinforcing steel
- 2) Minimum section modulus = 0.030 in<sup>3</sup>
- 3) 3 required per spiral



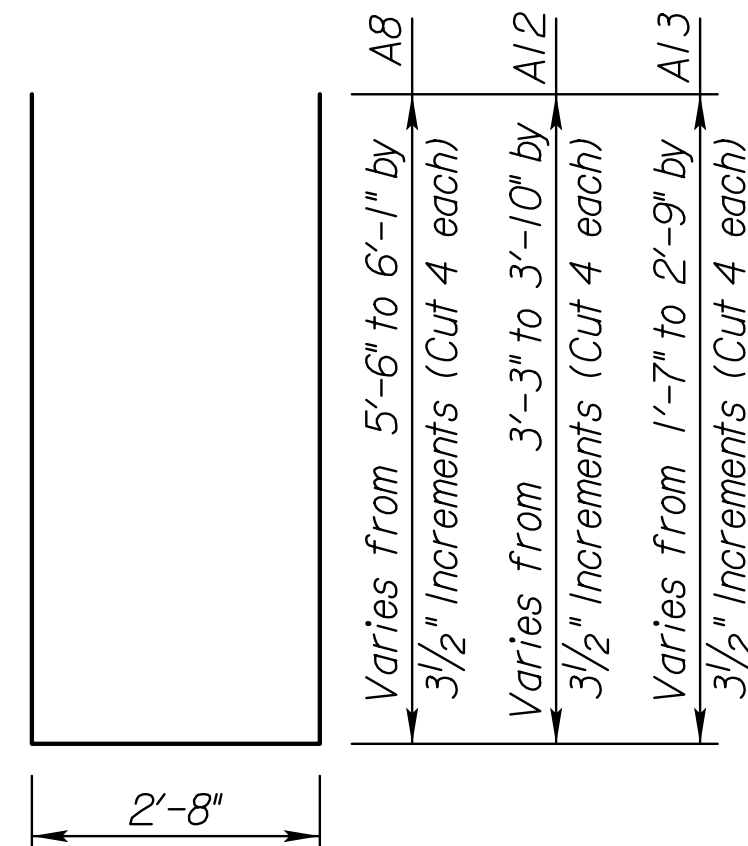
A14



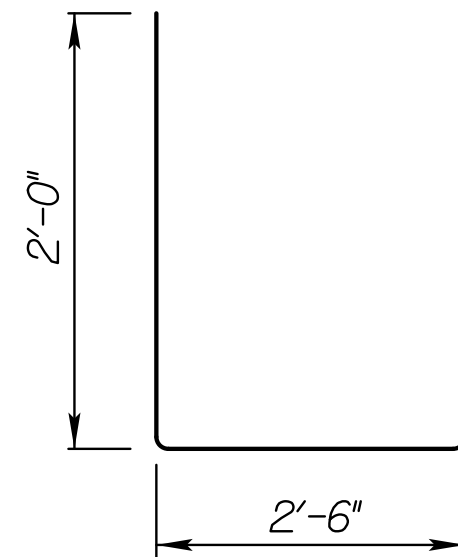
A16



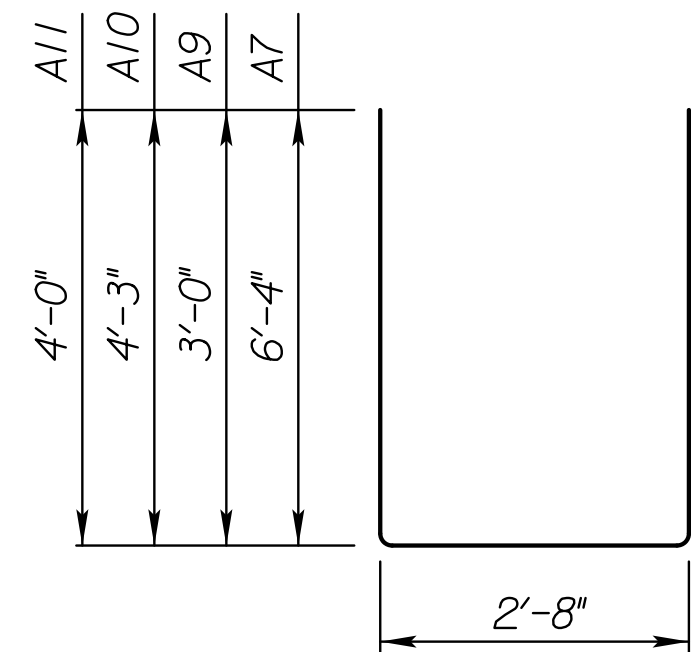
A18



A8, A12 & A13

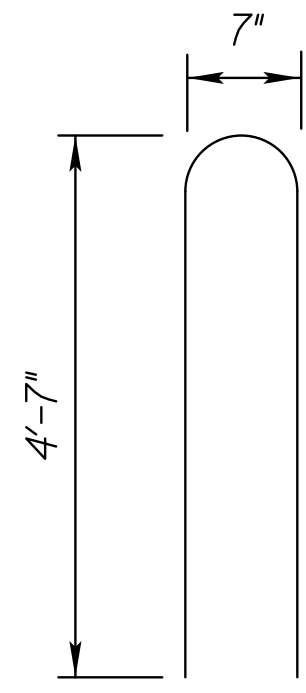


A15

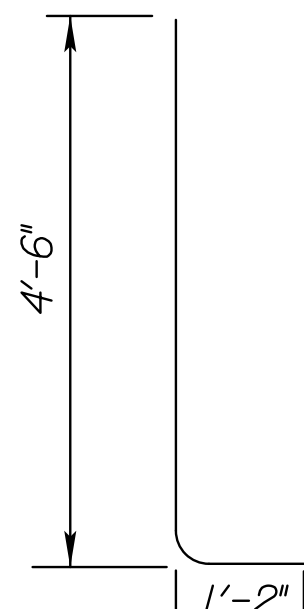


A7, A9, A10 & A11

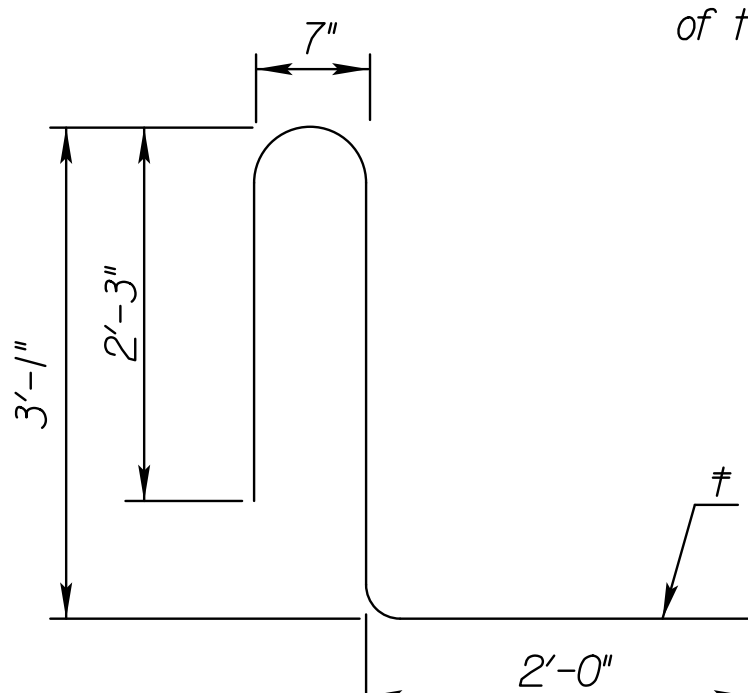
\* Bend this leg to match the slope of the roadway.



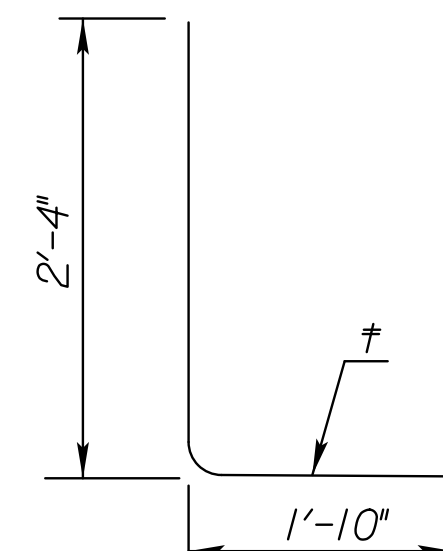
R1



R2

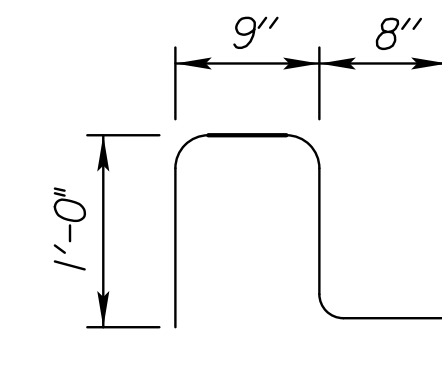


R3

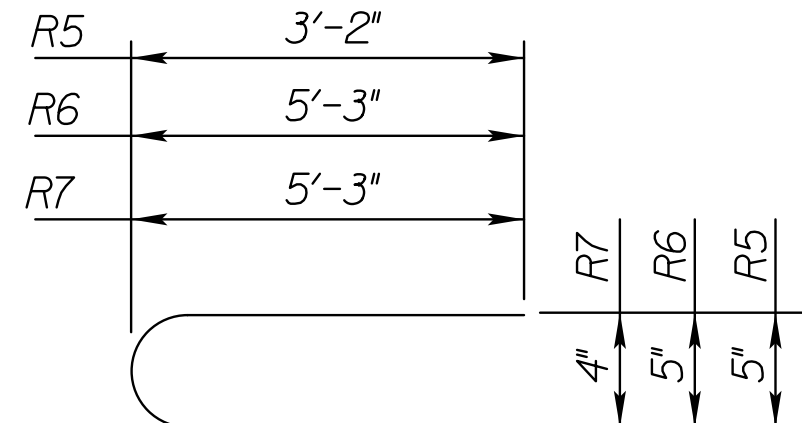


R4

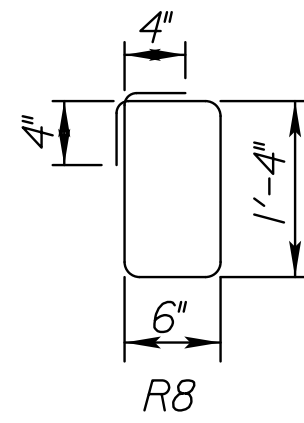
(Lap 1'-4" with R3)



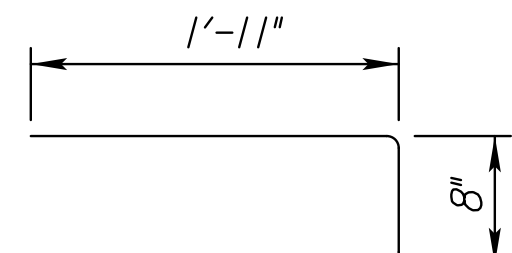
C1



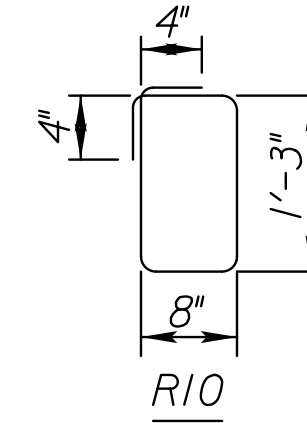
R5, R6 & R7



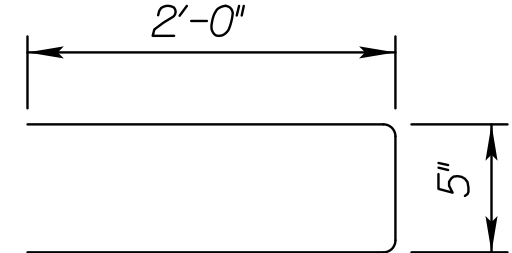
R8



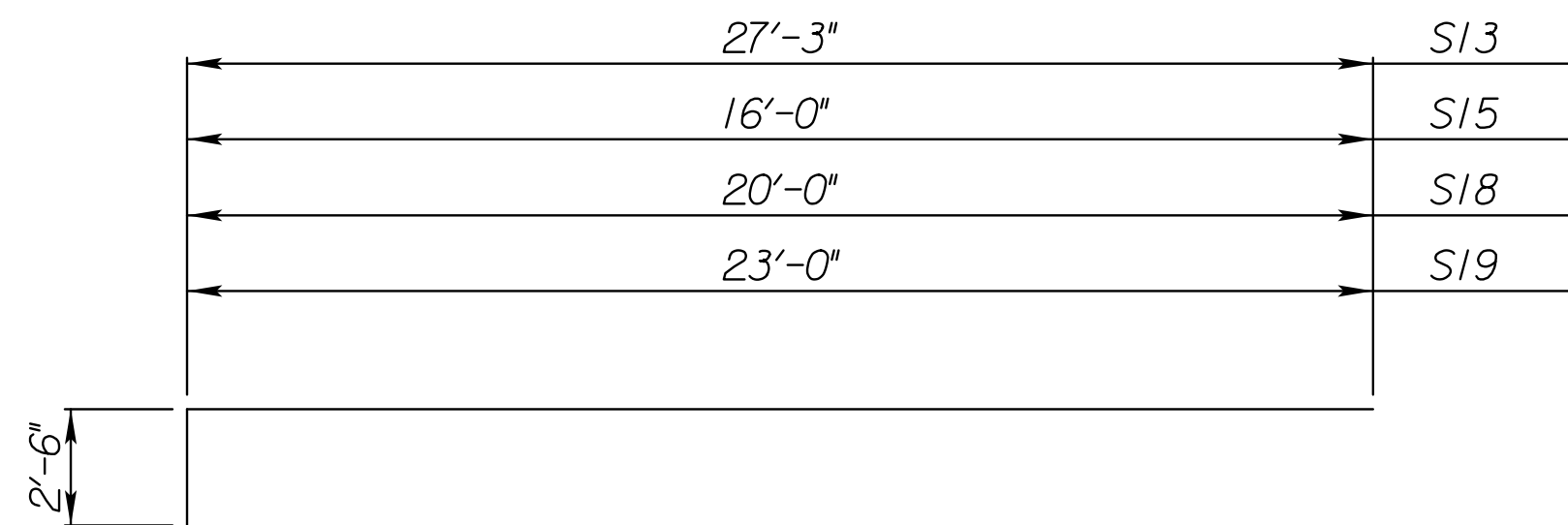
R9



R10



SPI



S13, S15, S18 & S19

BENDING DIAGRAMS

(All dimensions are out to out of bars.)

3					
2					
1					
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) S+a. 62+23.30					
BILL OF REINFORCING STEEL & BENDING DIAGRAMS					
Proj. 63-66 KA-5729-01					Nemaha Co.
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB
				CADD	KMS
				CADD CK.	JAB

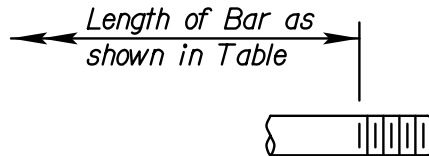
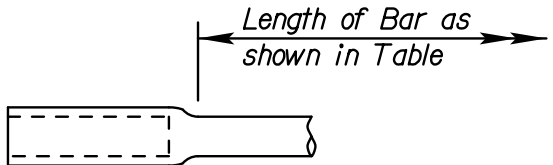
KDOT Graphics Certified 07-15-2024

Sheet No. 62

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	63	141

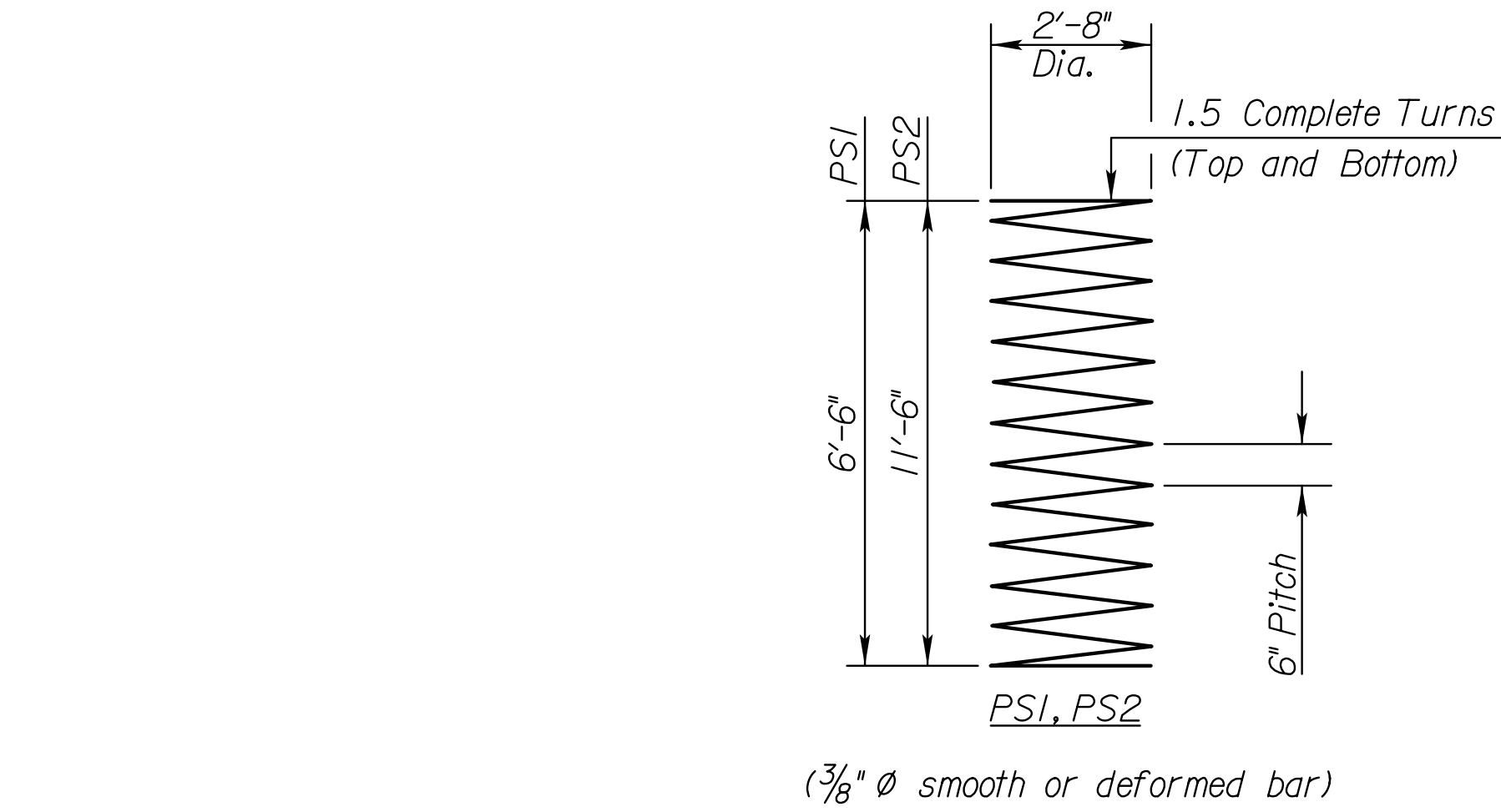
BILL OF REINFORCING STEEL Non-Epoxy Coated - Grade 60							
Straight Bars				Bent Bars			
Mark	Size	Number	Length	Mark	Size	Number	Length
PB4	9	20	18'-2"	PB3	9	20	19'-9"
PC1	8	48	9'-7"	PB1	5	176	9'-2"
PC2	8	48	14'-7"	PB2	5	28	7'-7"
PW2	6	8	6'-4"	PW3	4	84	4'-0"
PW7	6	4	6'-7"				
				PS1	3	4	⊗
PB5	4	40	18'-2"	PS2	3	4	⊗
PW1	4	60	6'-4"				
PW4	4	68	4'-7"				
PW5	4	68	9'-2"				
PW6	4	30	6'-7"				
PW8	4	42	9'-6"				
PW9	4	42	4'-6"				

⊗ See Bending Diagram

	
<i>Coupled Rebar (Place in Phase III Construction)</i>	<i>Coupled End Rebar (Place in Phase II Construction)</i>
THREADED REBAR SPLICE SYSTEM	

Threaded Rebar Splices (Grade 60)							
Threaded Bars				Coupled Bars			
Mark	Size	Number	Length	Mark	Size	Number	Length
PB9	9	20	5'-1"	PB7	9	20	5'-1"
PB8	4	20	2'-7"	PB6	4	20	2'-7"

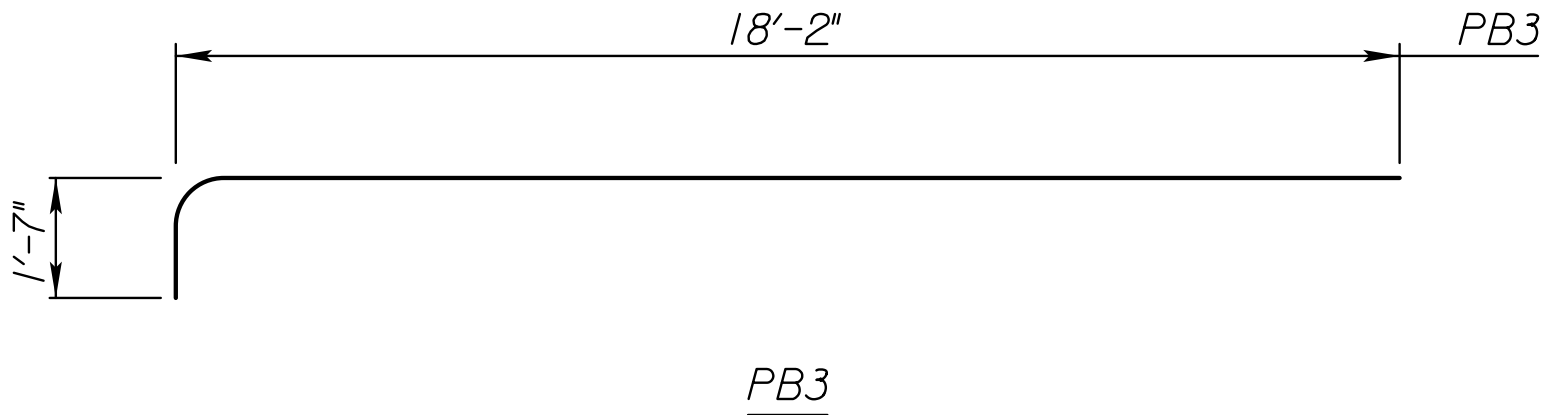
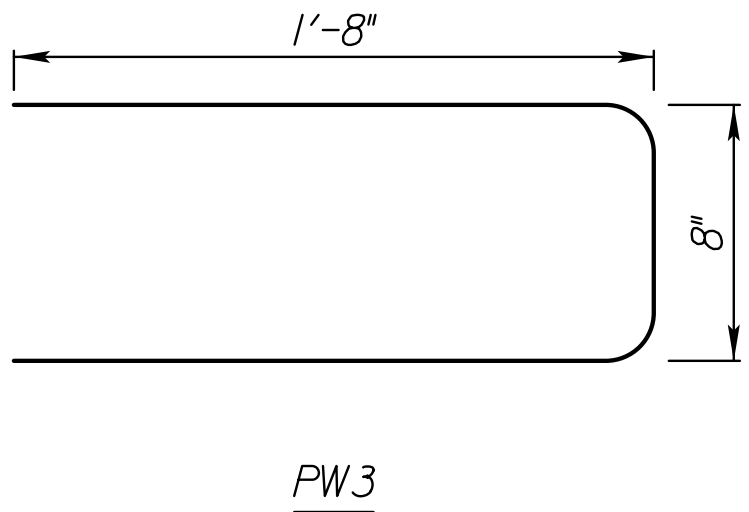
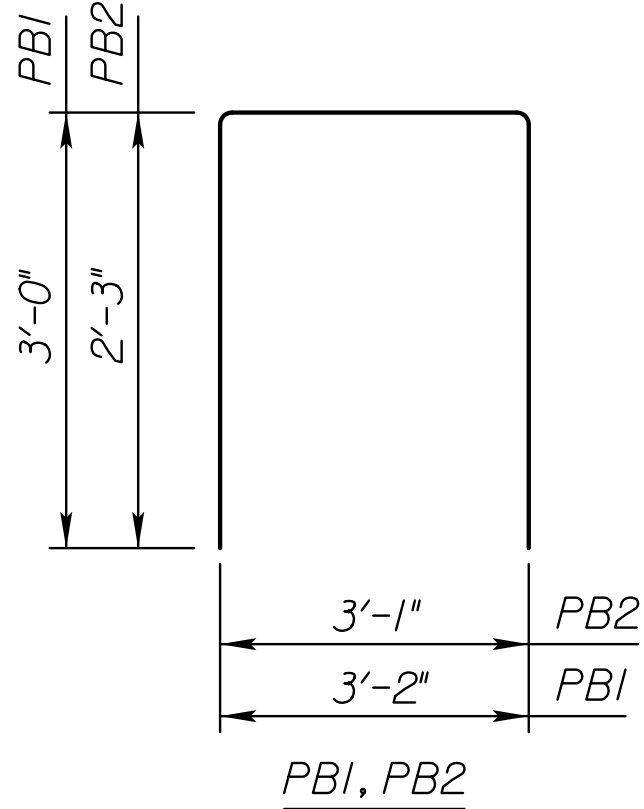
NOTE: The weight of the Threaded and Coupled rebars will be included in the weight of reinforcing steel. The Mechanical splice system shall meet the requirements of the KDOT specifications for "Fatigue Resistant" mechanical couplers. The additional material & labor shall be subsidiary to the bid item "Reinforcing Steel (Gr. 60) Epoxy Coated."



Spiral reinforcing shall meet the requirements of ASTM A615 Grade (60 or 40) or ASTM A82.

Spiral Spacer Bars:

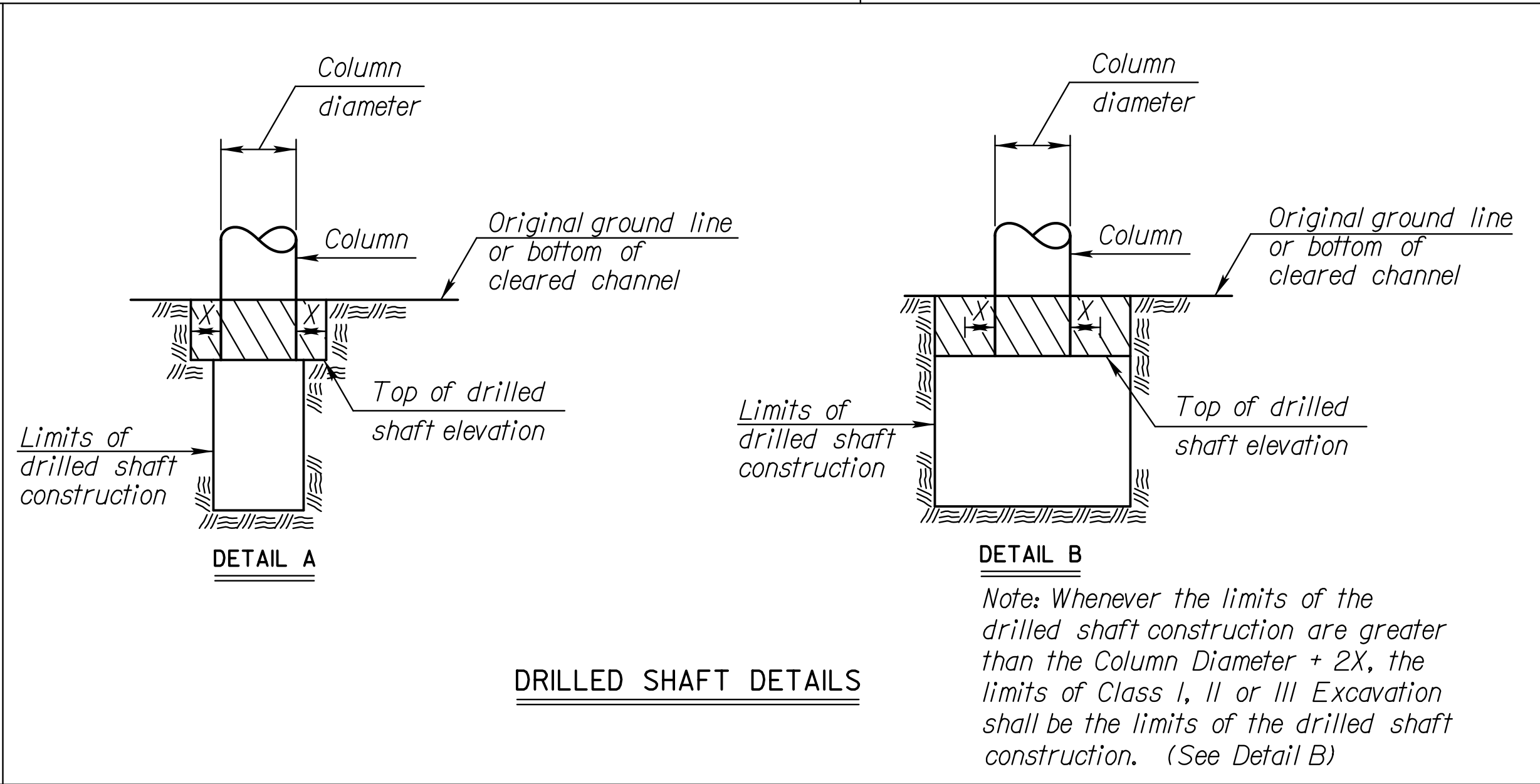
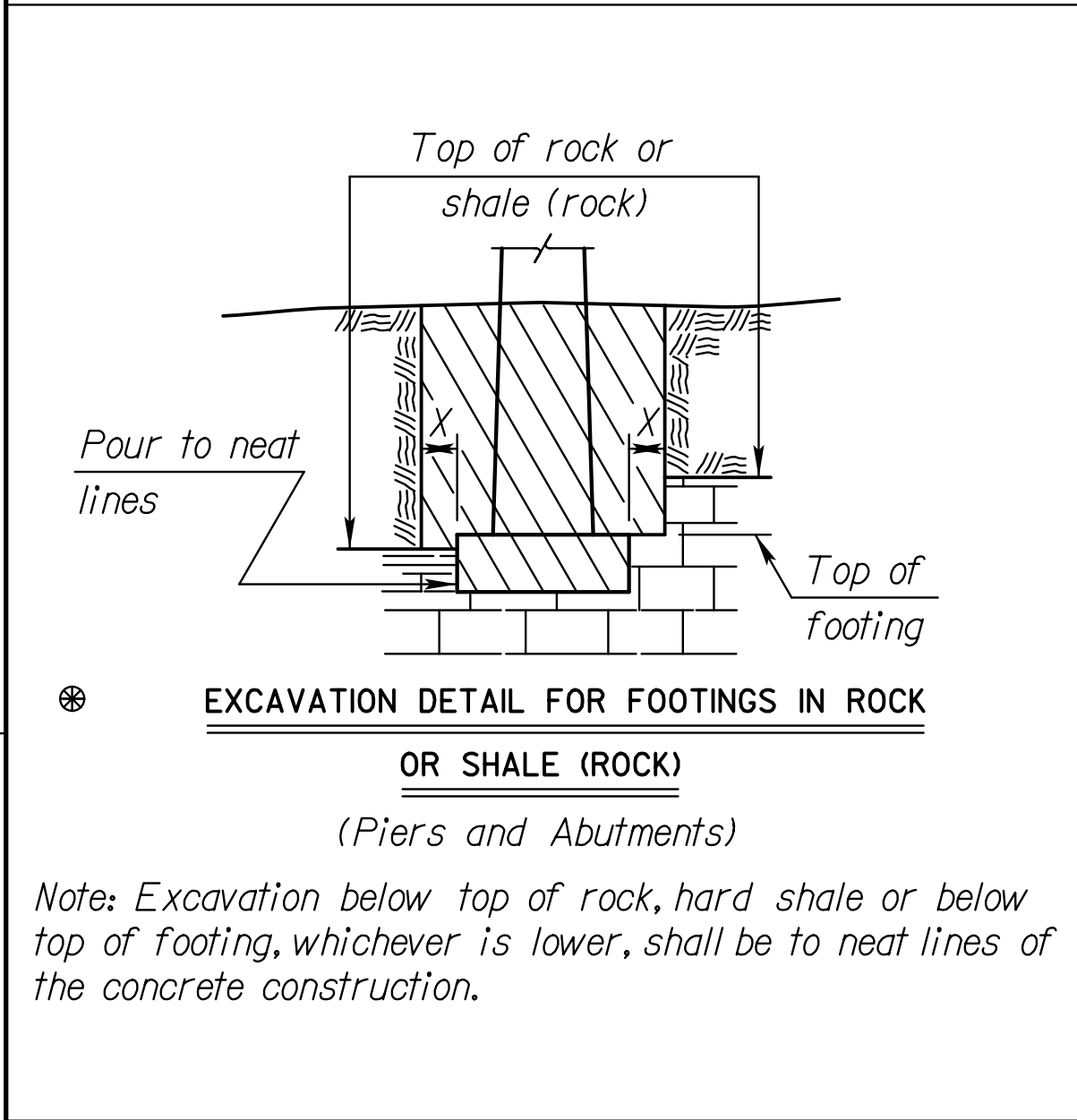
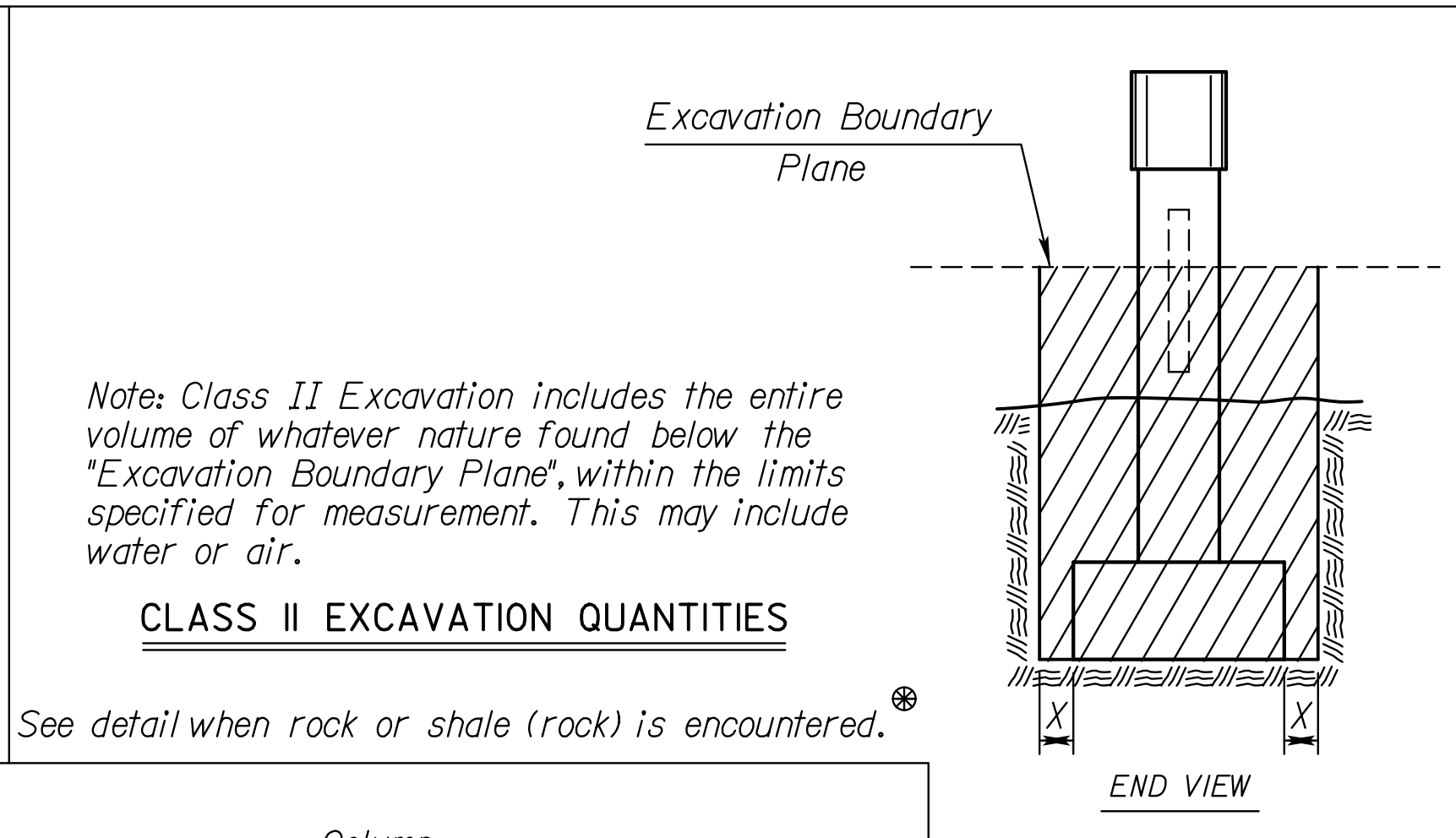
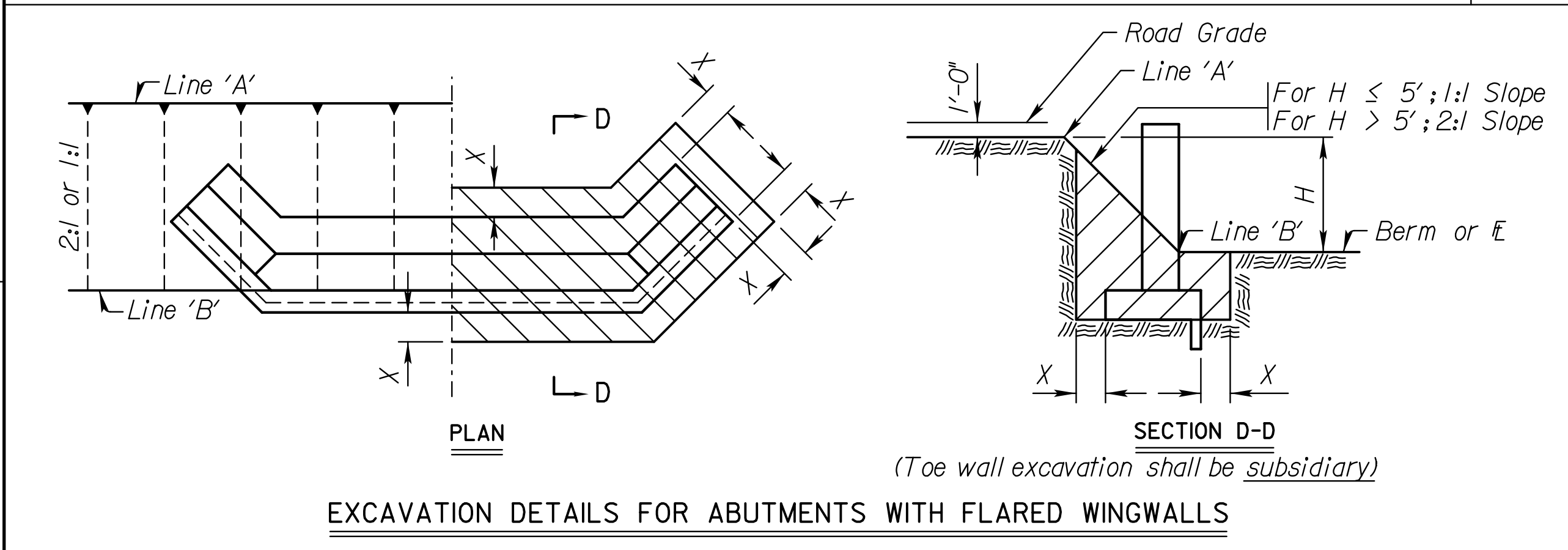
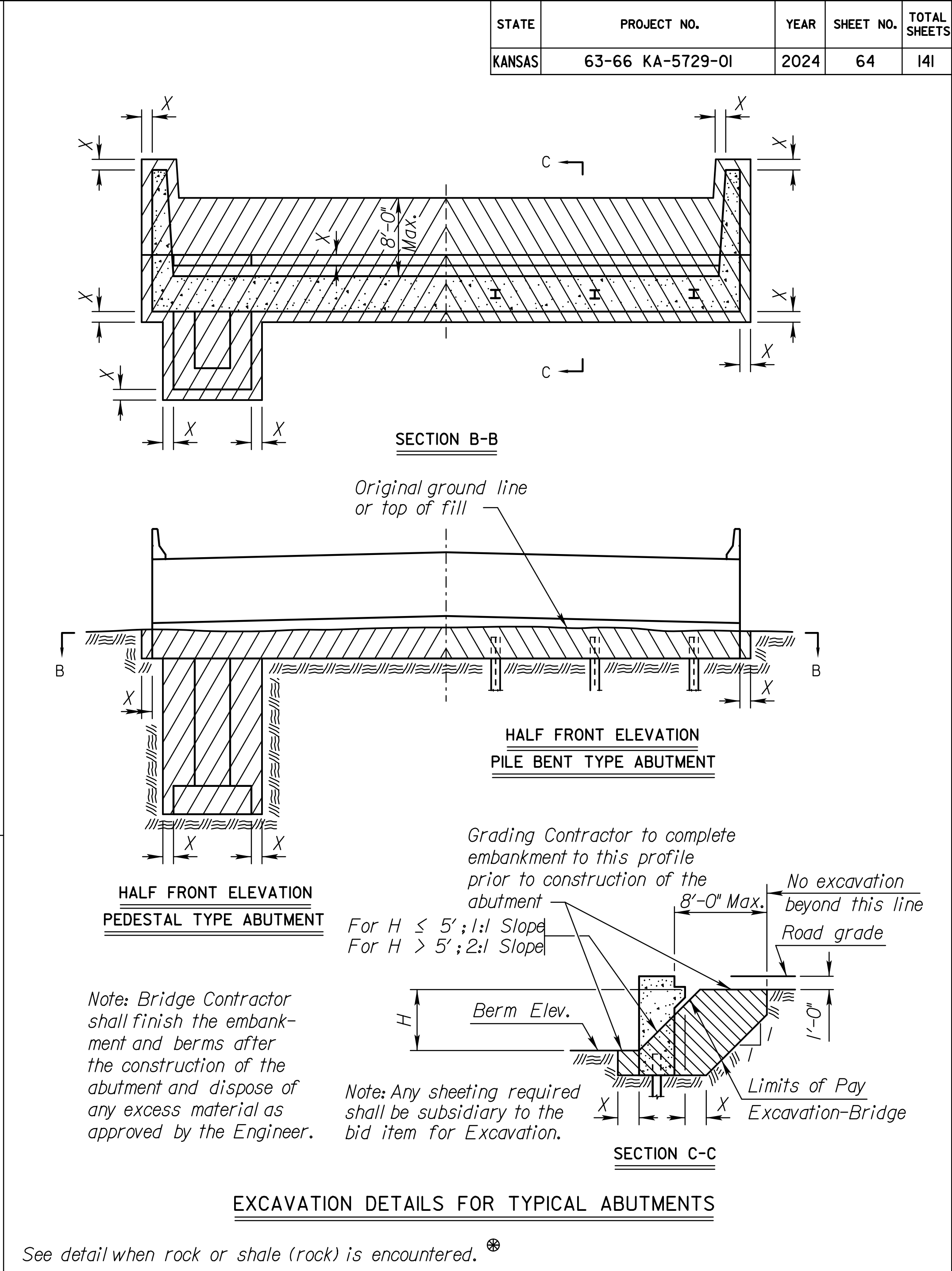
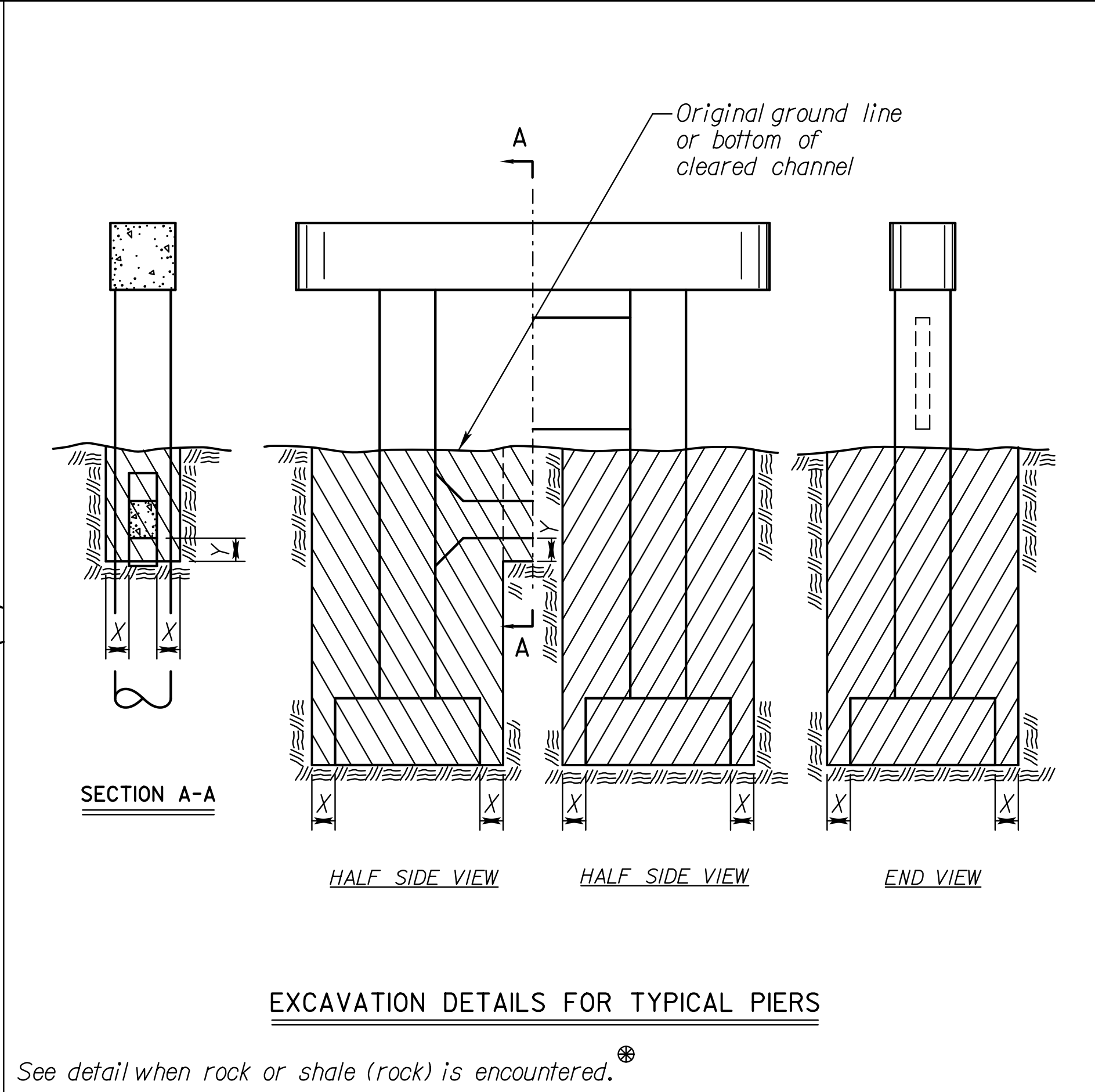
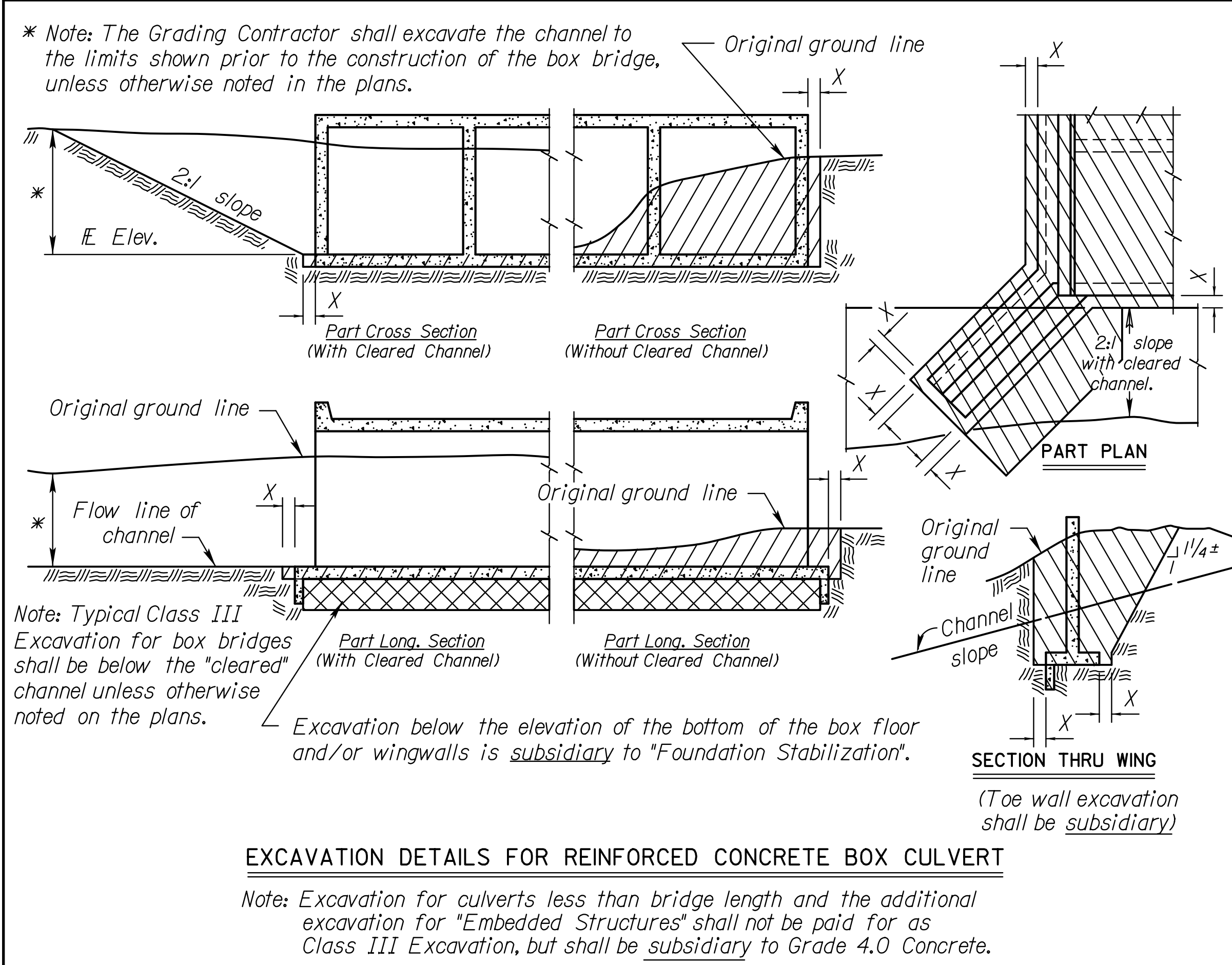
- 1) Are included in the mass of reinforcing steel
- 2) Minimum section modulus = 0.008 in<sup>3</sup>
- 3) 4 required per spiral in PSI & PS2



BENDING DIAGRAMS  
(All dimensions are out to out of bars.)

3					
2					
1					
NO.	DATE	REVISIONS			BY APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 63-66-54.90 (048) Sta. 62+23.30					
BILL OF REINFORCING STEEL & BENDING DIAGRAMS					
Proj. 63-66 KA-5729-01 Nemaha Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	CFB	DETAILED	CFB	QUANTITIES	CFB
DESIGN CK.	JAB	DETAIL CK.	JAB	QUAN. CK.	JAB
				CADD	KMS
				JAB	JAB

Std. Base File: br100.dgn  
Plotted: Jake.Pfannenstiel@ks.gov  
File: ka572901bss0048-01  
Plot Date: 15-JUL-2024 16:00



Note: All bridge excavation shall be computed on the basis of the cross-hatch areas and boundary lines indicated on this sheet and the Excavation Boundary Plane on the Construction Layout.

Sides of trenches in hard or compacted soil including embankments shall be shored, sheeted, braced or otherwise supported when the trench is more than 5 feet in depth and 8 feet or more in length. In lieu of the shoring, the sides of the trench above the 5 foot level may be sloped to preclude collapse. The slope for average soils shall be 1:1. If the angle of repose of the soil is less, flatter slopes shall be required.

Dimension "X" shall be 2'-0" unless indicated otherwise on the general plans.

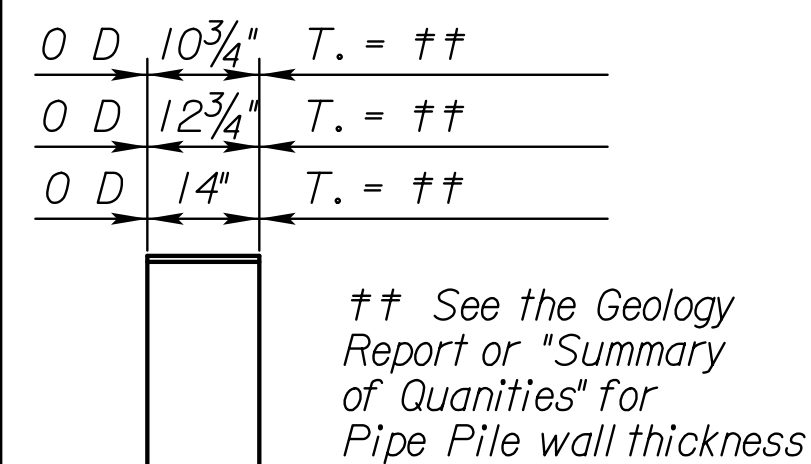
Dimension "Y" shall be 1'-6" unless indicated otherwise on the general plans.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	64	141

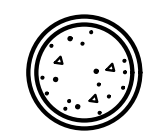
NO.	DATE	REVISIONS	BY	APP'D
7				
6	8-15-12	Embedment Excavation Subsidiary	JPJ	TLF
5	5-15-12	Revised Wing Excavation	JPJ	TLF
4	3-3-10	Revised Wing Excavation	JPJ	TLF
3	10-16-06	Revised 'Foundation Stab.' Note	JPJ	KFH
2	10-19-04	Concrete - Class to Grade	RAM	KFH
1	4-10-02	Added 'Foundation Stab.' Note	RAM	KFH

KANSAS DEPARTMENT OF TRANSPORTATION				
BRIDGE EXCAVATION (LRFD)				
BR100B				
FHWA APPROVAL		4/17/10 APP'D		TERRY L. FLECK
DESIGNED	DETAILED	RDR QUANTITIES	CADD	
DESIGN CK.	DETAIL CK.	LRRI QUAN. CK.	CADD CK.	

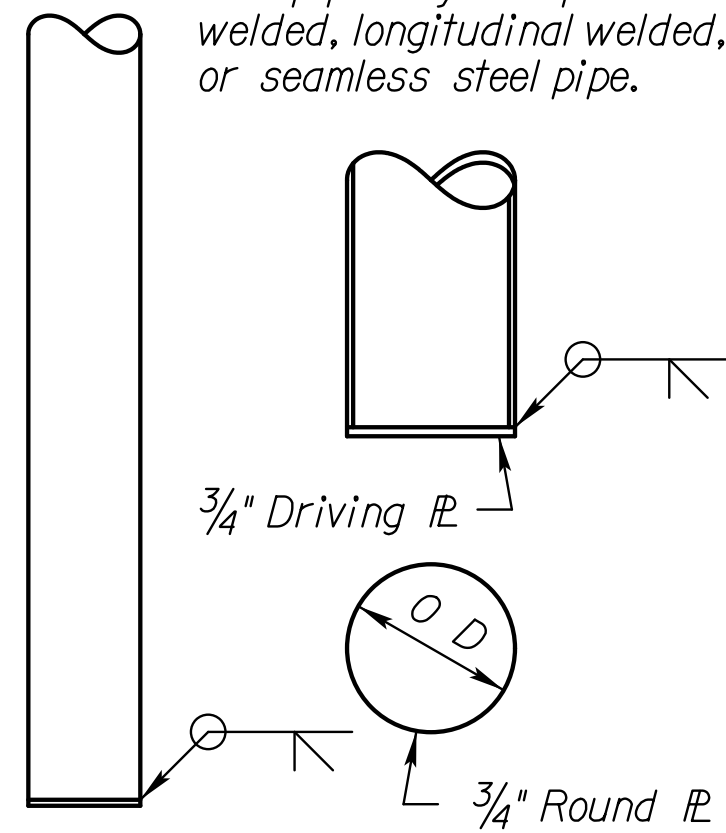




See the Geology Report or "Summary of Quantities" for Pipe Pile wall thickness



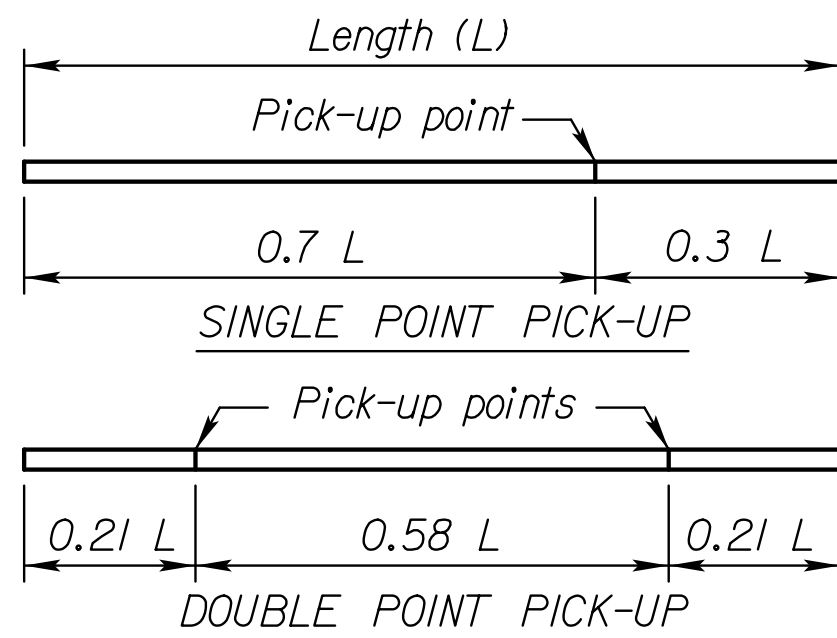
Note: Pile shall be driven with a steel head having a projecting ring fitting inside the pipe. Clearance between ring and pipe should be 1/4".



### PLAIN ROUND CAST-IN-PLACE CONCRETE PILES

#### CAST STEEL PILE POINT

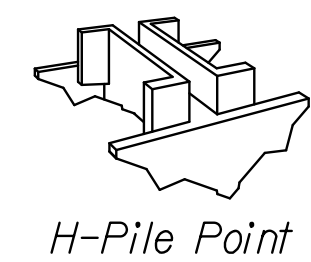
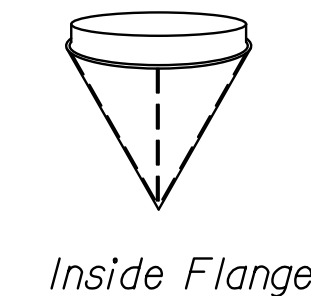
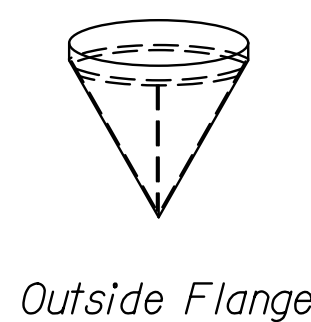
The pile point shall be a one-piece unit of cast steel. Weld pile points in accordance with manufacturers recommendations to each steel pile before driving.



#### PICK-UP POINTS FOR PRESTRESSED PILING

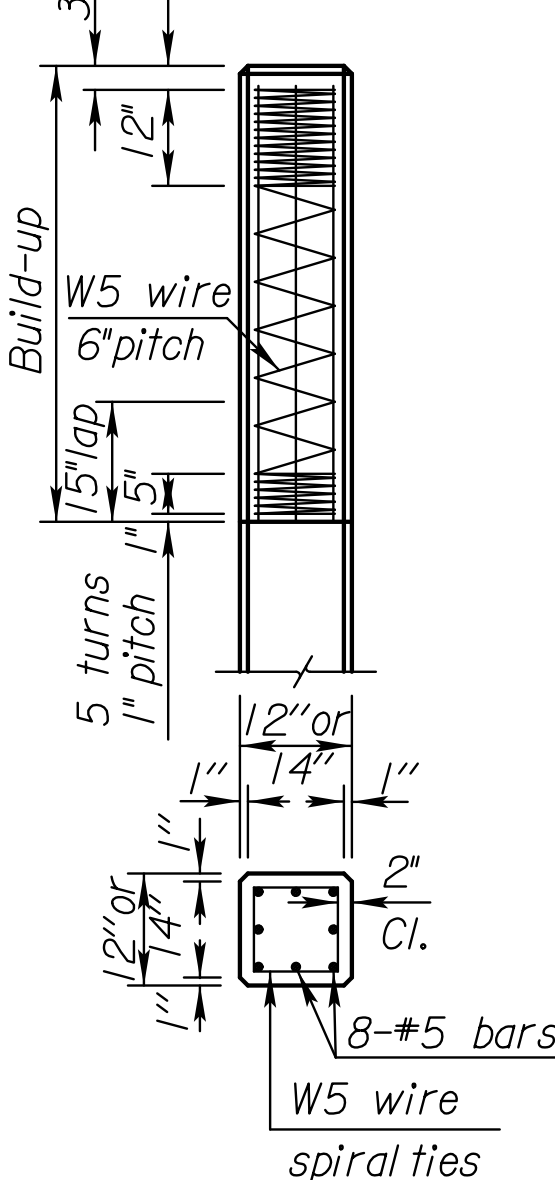
Max. length - 55' single point pick-up  
Max. length - 80' double point pick-up

Note: Piles shall be marked at Pick-up points to indicate proper points for attaching handling lines.



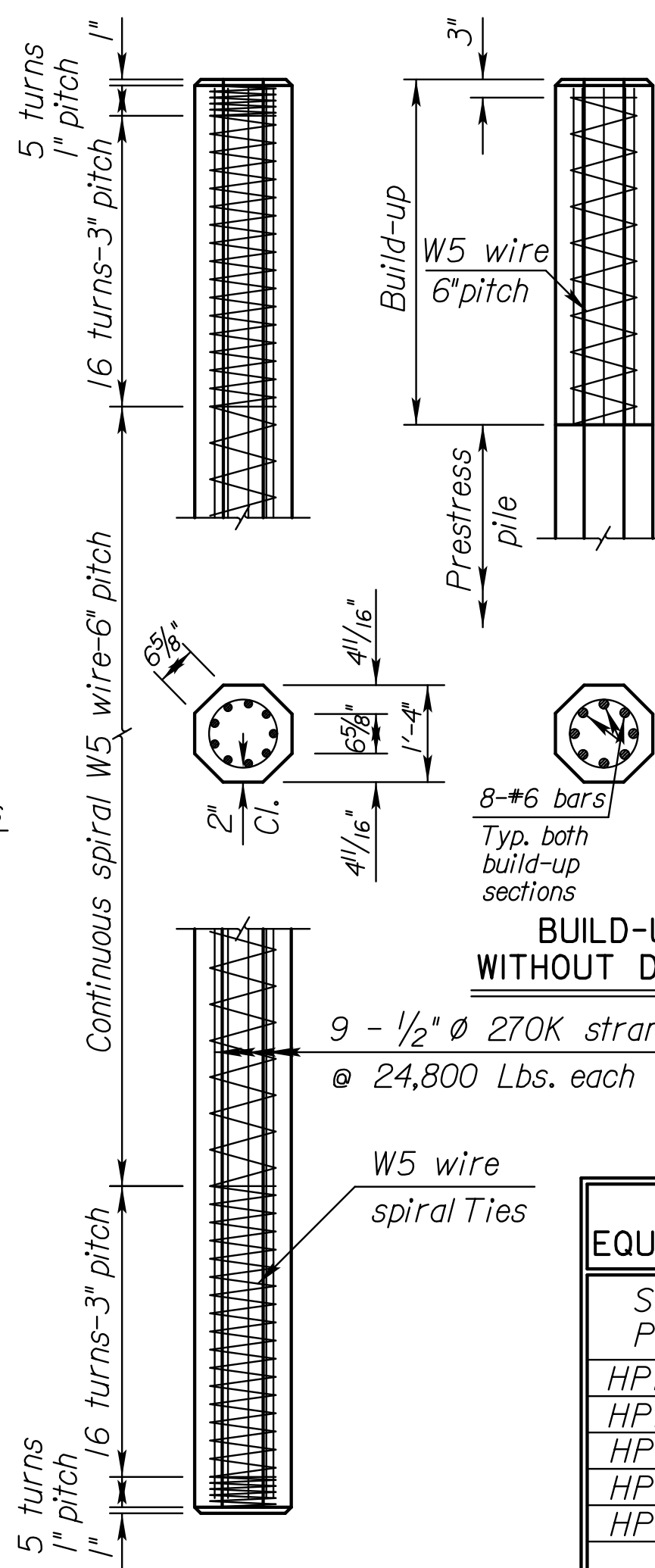
#### PIPE PILE POINT

Note: If additional driving is required, use 1" pitch as shown.



#### BUILD UP SECTION

8 - 3/8" Ø 270K strands  
@ 16,000 Lbs. each  
12" x 12" piles  
8 - 1/2" Ø 270K strands  
@ 22,700 Lbs. each  
14" x 14" piles



#### BUILD-UP WITHOUT DRIVING

9 - 1/2" Ø 270K strands  
@ 24,800 Lbs. each

#### FOR INFORMATION ONLY EQUIVALENT POINT BEARING PILES

STEEL PILES	CONCRETE PILES	
	Pipe	Pre-stress
HPI10x42	10 3/4	
HPI2x53	12 3/4	
HPI4x73	14	12
HPI4x102		14
HPI4x117		16

SPLICES: Splices for steel piles and shell piling shall be in accordance with details shown on this sheet and the Standard Specifications.

For integral pile bent abutments and piers, if a pile splice is required, do not locate the pile splice within a region extending 2'-0" above and 10'-0" below the bottom of the concrete web wall. For abutments, locate the pile splice at least 10'-0" below top of fill.

With the approval of the Engineer, one splice per bent may be allowed in the region described above without testing. If additional splices are anticipated, based on the geology, the Contractor prior to driving, will locate the splice so that the splice will not fall within the regions described above.

\* For integral pile bent abutments and piers, if a splice is located within the regions described above, then the Contractor will test the welds by Radiograph (RT) test methods. Repair and retest any welds not passing the test(s). Each weld tested will have written confirmation of results. Report these results to the Engineer. This work is not paid for directly, but is subsidiary to "Piles".

\* Minimum as required by welding process.

BG = Backgauge

#### PILE SPlice DETAILS

#### GENERAL NOTES

PRESTRESSED PILES: Fabricate prestressed concrete pile splices in accordance with the Manufacturer's recommendations subject to the approval of the Engineer.

Method of attachment of pile to build-up may be by any of the methods given in the notes on "Alternate Methods. If mild reinforcing steel is used for attachment, the area shall be no less than that used in the build-up.

ALTERNATE METHODS: Method of attachment of a pile to build-up may be by any of the following methods:

1. Cut off at least 2'-0" of pile and expose a minimum of 2'-0" of strands.
  2. Cast 8-#6, or 8-#5 bars (equally spaced) into pile head. All bars shall extend into pile head and project from pile head a minimum of 2'-0".
  3. Drill 8 holes in pile head (equally spaced) for installation of 8 grouted dowel bars of same size and length as in 2.
  4. Provide cored holes for bars as in 3.
- No bars or strands are to extend from head of pile or build-up into footing or pile cap unless approved by the Engineer.

TEST PILES: Drive test piles where called for on the bridge plans. The test piles located within the limits of the substructure will become a part of the bridge pile system.

DRIVING FORMULA: Driving formula shall conform to the Standard Specifications.

MEASUREMENT AND PAYMENT: Measurement and payment for all piles shall comply with the Standard Specifications.

The following items are covered in Division 1000 of the Standard Specifications:

REINFORCEMENT: Use reinforcing steel conforming to ASTM A615, Grade 60. Hoops and spirals may be either plain or deformed bars.

PRESTRESSING STEEL: Use uncoated seven-wire stress relieved or low relaxation prestressing strand conforming to ASTM A416, Gr. 270.

SPECIFICATIONS: Standard Specifications for State Road and Bridge Construction as currently used by the Kansas Department of Transportation. The following items are covered in Division 700 of the Standard Specifications:

CONCRETE: Concrete for cast-in-place shall be f'c = 3,500 PSI.. Concrete for prestressed shall be f'c = 5,000 PSI.

WELDING: All field welding shall meet the requirements of the Standard Specifications.

Use only Shielded Metal Arch Welding SMAW (stick welding) for pile splices.

Use only low hydrogen E7018, 7016, or 7015 series welding rod (electrode) for all welding applications during pile splicing. See General Notes or proper storage of welding rod, welding filler rod (electrode) for field welding of splices.

New electrode are to be purchased for each KDOT project. The electrode shall arrive on the project in factory hermetically sealed containers opened and labeled with indelible ink in front of the engineer. The label shall include the current date and the project number. If the container seal is questionable or shows signs of damage the electrode is to be dried in an oven at least one hour at a temperature of 700°F to 800°F.

Upon removal from intact hermetically sealed factory packaging or the drying oven the electrode is to be placed in a storage oven with a minimum temperature of 250°F.

When electrodes are removed from the hermetically sealed container or storage oven and exposed to the atmosphere for less than 4 hours place into the storage oven for at least 4 hours before removing for use.

If electrode is exposed to the atmosphere for 4 hours or more (or 9 hours for moisture resistant electrodes designated with an R in their labeling) then electrode can be dried in a drying oven at a temperature of 450°F to 550°F.

If the electrode is exposed to the atmosphere for 4 hours or more a second time or the rod becomes wet discard rod.

CAST-IN-PLACE SHELLS: Steel shells for cast-in-place piles shall conform to the requirements of the Standard Specifications.

All piles driven without a mandrel shall be of the minimum thicknesses shown. Piles driven with a mandrel shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion and/or buckling due to soil pressure after the mandrel is removed.

Remove, replace or correct to the satisfaction of the Engineer improperly driven, broken or otherwise defective pipe piles. Otherwise drive an additional pile at no extra cost.

The Contractor shall maintain a light suitable for visual inspection of the pile on the job at all times prior to and during the filling of the pipe.

STEEL PILE: Steel pile shall conform to the requirements of the Standard Specifications.

PILE POINTS: Pile points shall conform to the dimensions shown and to requirements of the Standard Specifications.

PAINT: All paint shall comply with the Standard Specifications, or as specified on the plans.

MILL TEST REPORTS: Steel piles test reports and steel shell test reports shall comply with the Standard Specifications.

NO.	DATE	REVISIONS	BY	APP'D
4	08-16-18	Add splice web section, clarify note	MLL	JPJ
3	09-15-15	Clarify Notes	JPJ	CER
2	06-18-12	Clarify fg, rod type, use and weld	JPJ	TLF
1	1-5-09	Pile Splice Location and Weld Test	JPJ	KFH

#### KANSAS DEPARTMENT OF TRANSPORTATION

#### STANDARD PILE DETAILS

BRIIO	DESIGNED	JPJ	DETAILED	QUANTITIES	CADD	RAA
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.			

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	66	141

GENERAL NOTES

Reference is made to the latest edition of the CRSI "Manual of Standard Practice" for recommended industry practices concerning reinforcing steel.

Use only the following types of bar supports:

- 1) Wire Bar Supports:
- a) Epoxy coated reinforcing: Class 1 Protection
- b) Non-epoxy coated reinforcing: Class 1, 2, or 3 Protection
- 2) Plastic Bar Supports
- 3) Supplementary bars

When securing epoxy coated reinforcement, use tie wires or metal clips that are epoxy or plastic coated.

Do not weld reinforcing steel to bar supports or to other reinforcing steel. Shop weld spacer frames for haunched slabs.

Tie bars at all intersections around the perimeter of each mat and at not less than 2'-0" centers or at every intersection, whichever is greater.

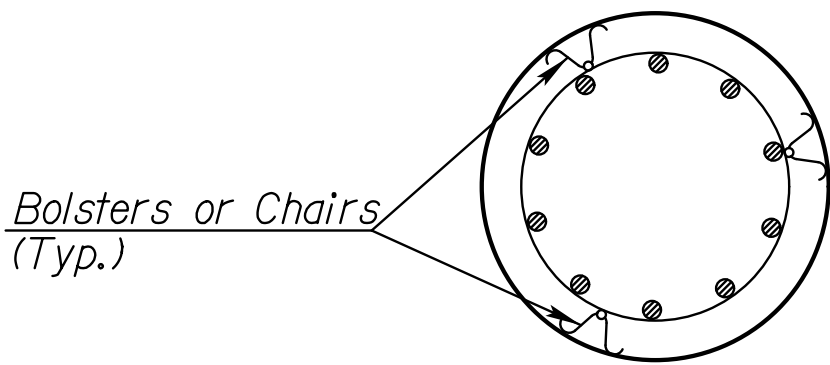
Where more than one length of bar support is required, lap the end legs so they are locked or tied together.

Use proper height supports to maintain the distance between the reinforcing and the formed surface or the top surface of deck slabs within 1/4" of that indicated on the plans.

Spacings shown are maximums. Use sufficient supports, as determined by the Engineer, to retain the reinforcing steel in position.

Construct any platforms, required for the support of workers and/or equipment during concrete placement, directly on the forms and not on the reinforcing steel.

Designs and arrangements of Supports or Spacers other than as shown on this sheet, may be used with the permission of the Engineer.



SECTION A-A

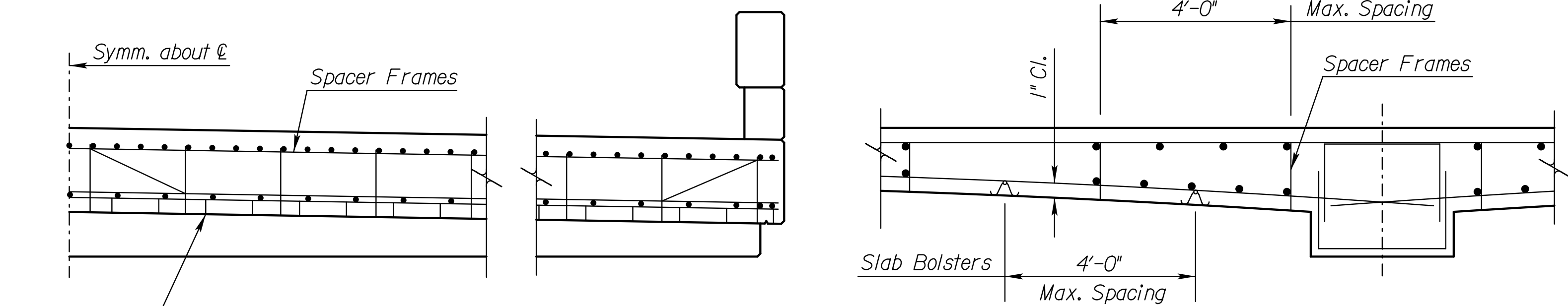
5	11-10-10	Column Bar Supports Req'd	JPJ	TLF
4	12-01-05	Drilled Shaft Spiral Steel Placement	JPJ	KFH
3	8-21-00	Added Pre-Cast Panel Detail	RAM	KFH
2	12-20-99	Added Haunched Slab Bolsters	RAM	KFH
1	12-09-99	Revised Drilled Shaft Clearance	RAM	KFH
NO.	DATE	REVISIONS	BY	APP'D

SUPPORTS AND SPACERS FOR REINFORCING STEEL

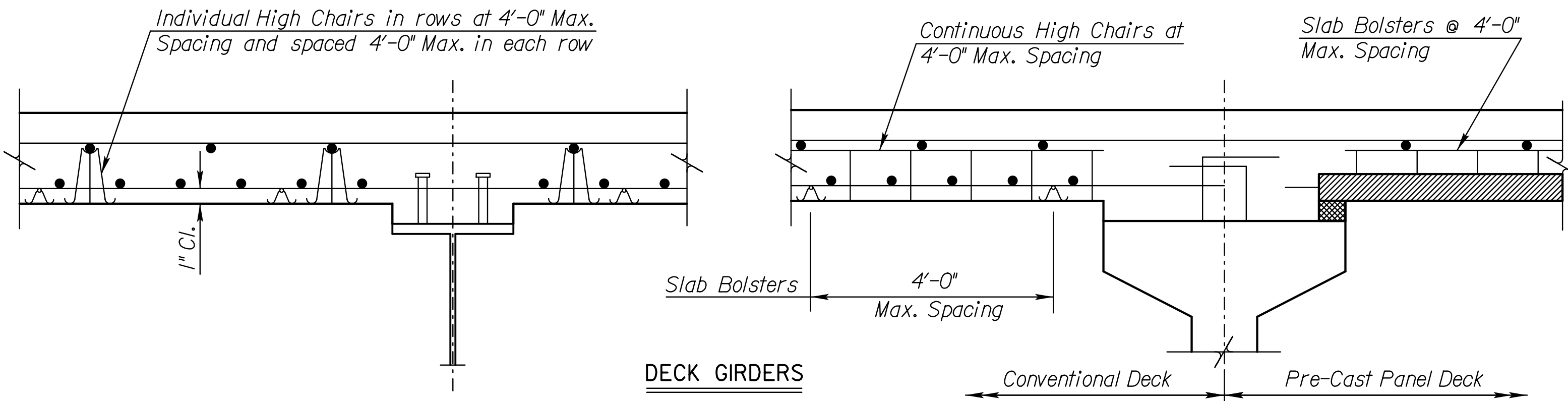
FHWA APPROVAL		11-17-10   APP'D		Terry L. Fleck	
DESIGNED	RAM	DETAILED	RAM	QUANTITIES	CADD
DESIGN CK.	LRRI	DETAIL CK.	RAM	QUAN. CK.	CADD CK.

KDOT Graphics Certified 03-15-2024

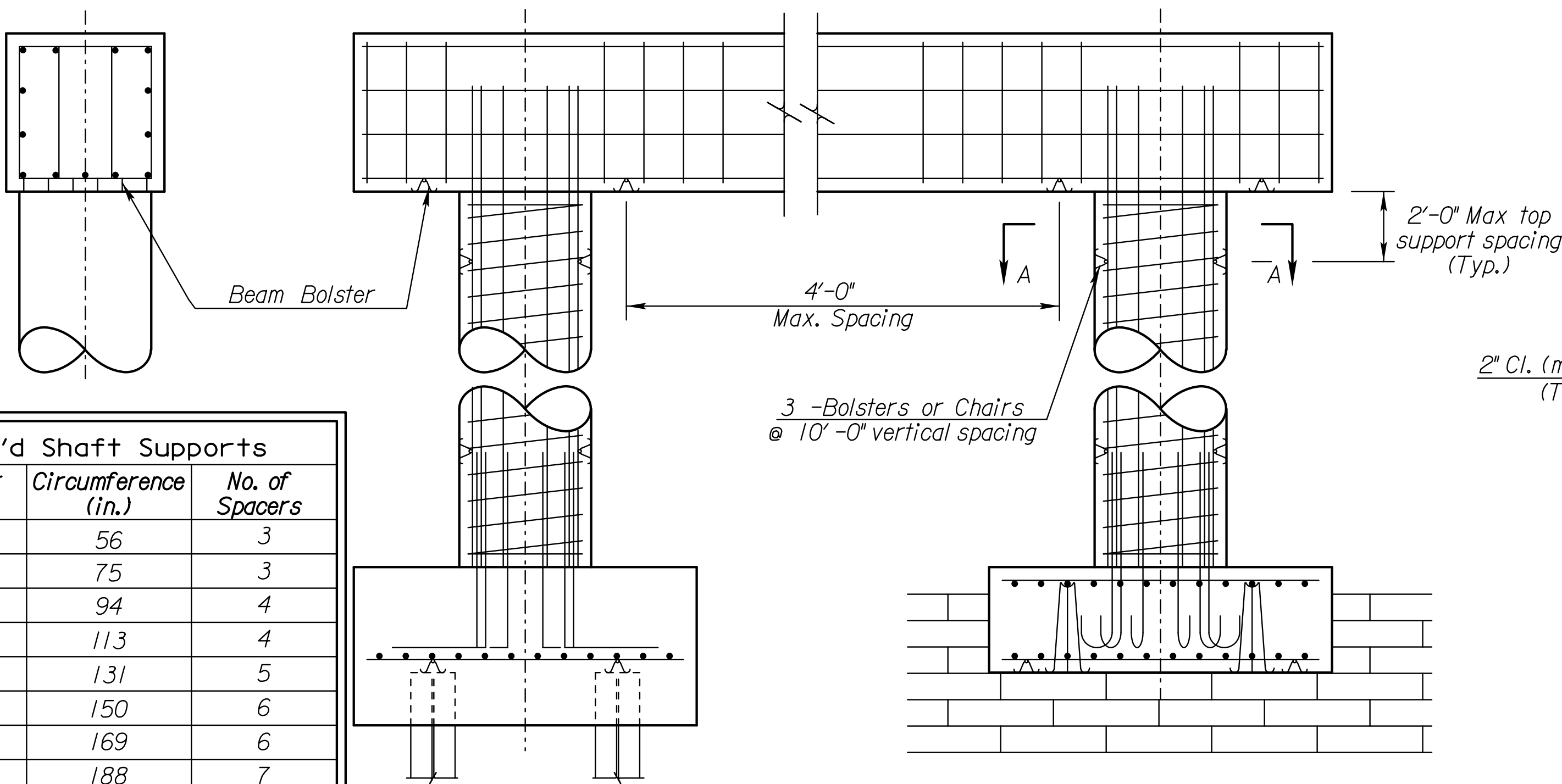
Sheet No. 66



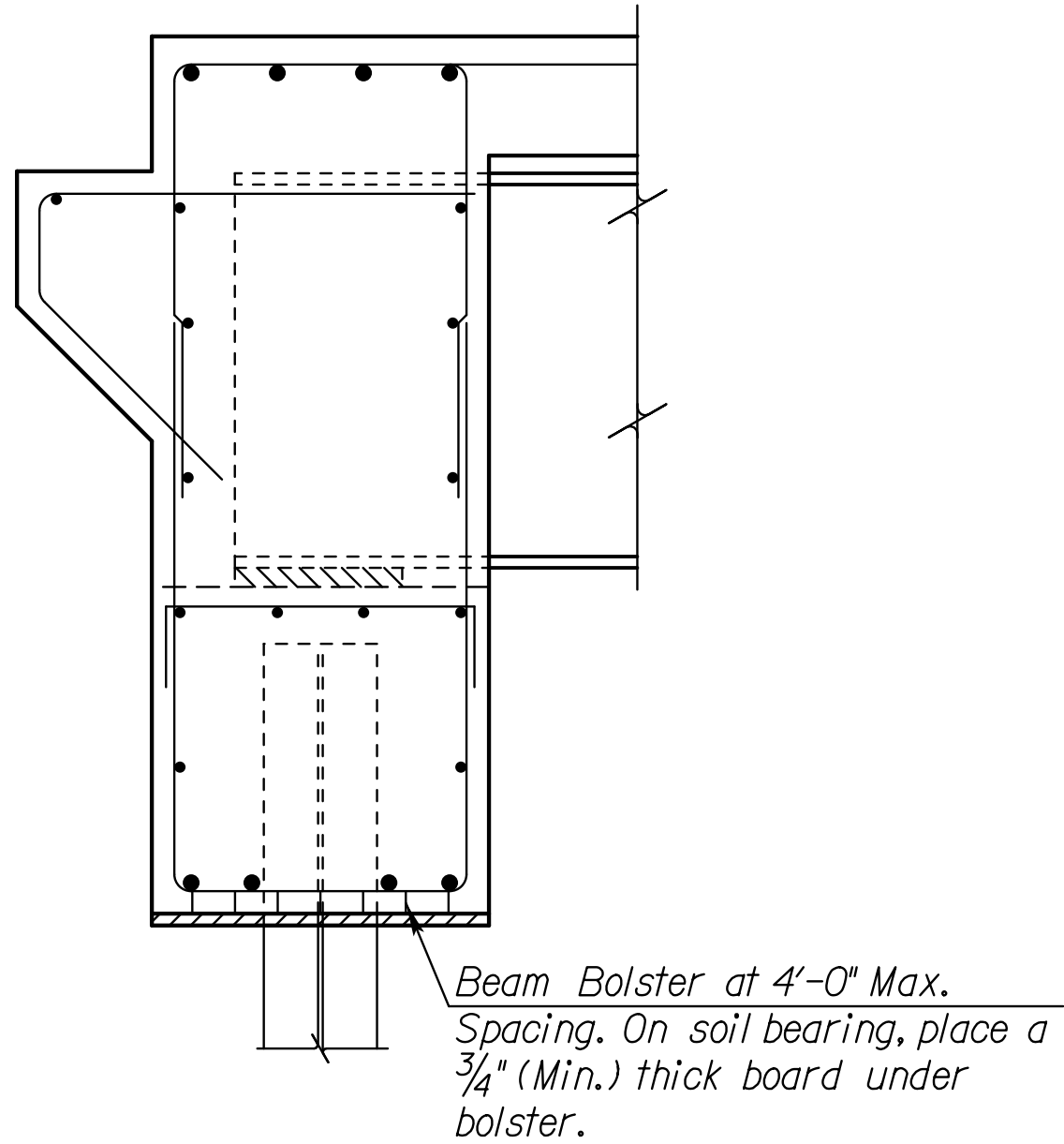
CONTINUOUS HAUNCHED SLAB



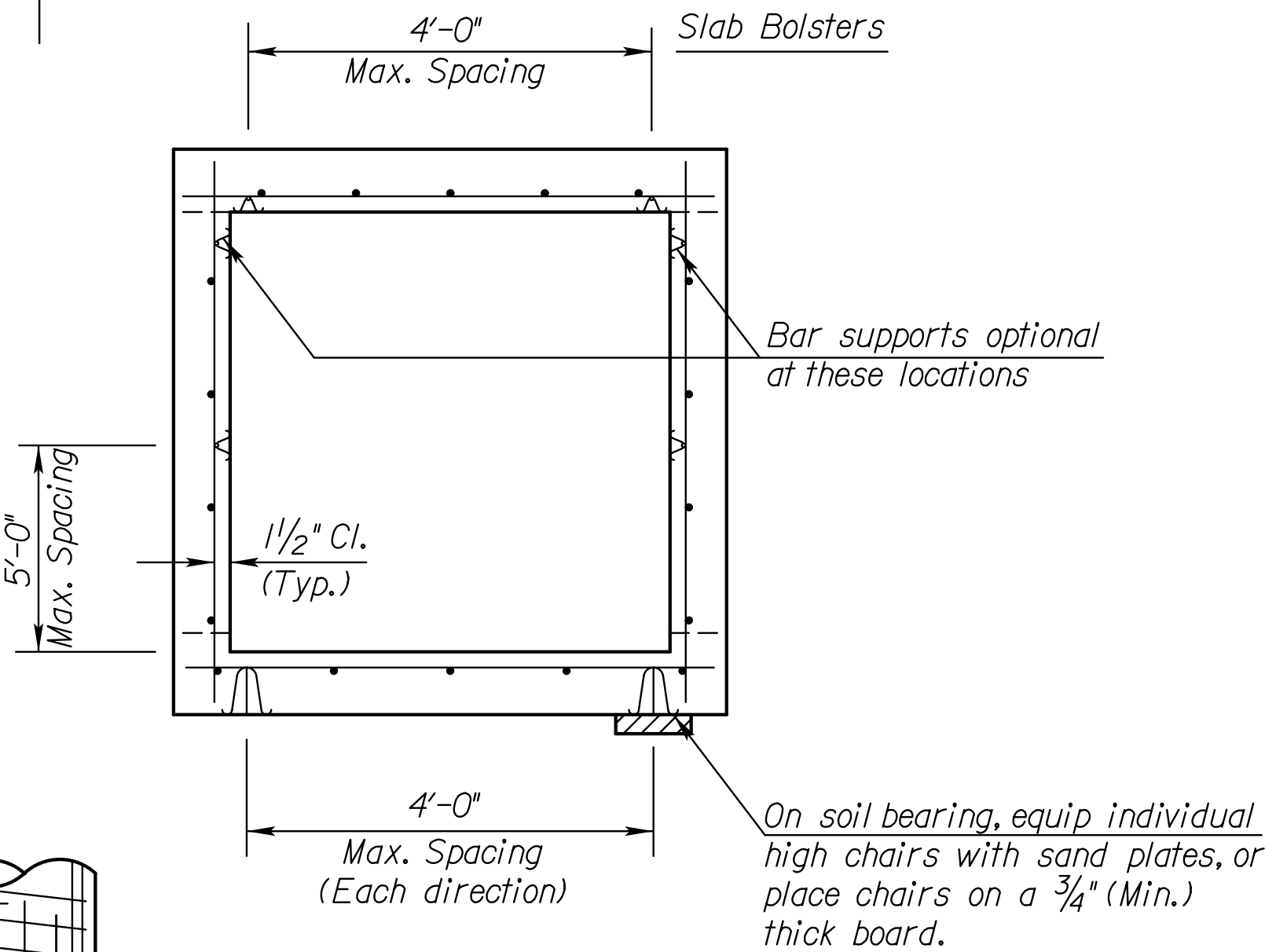
DECK GIRDERS



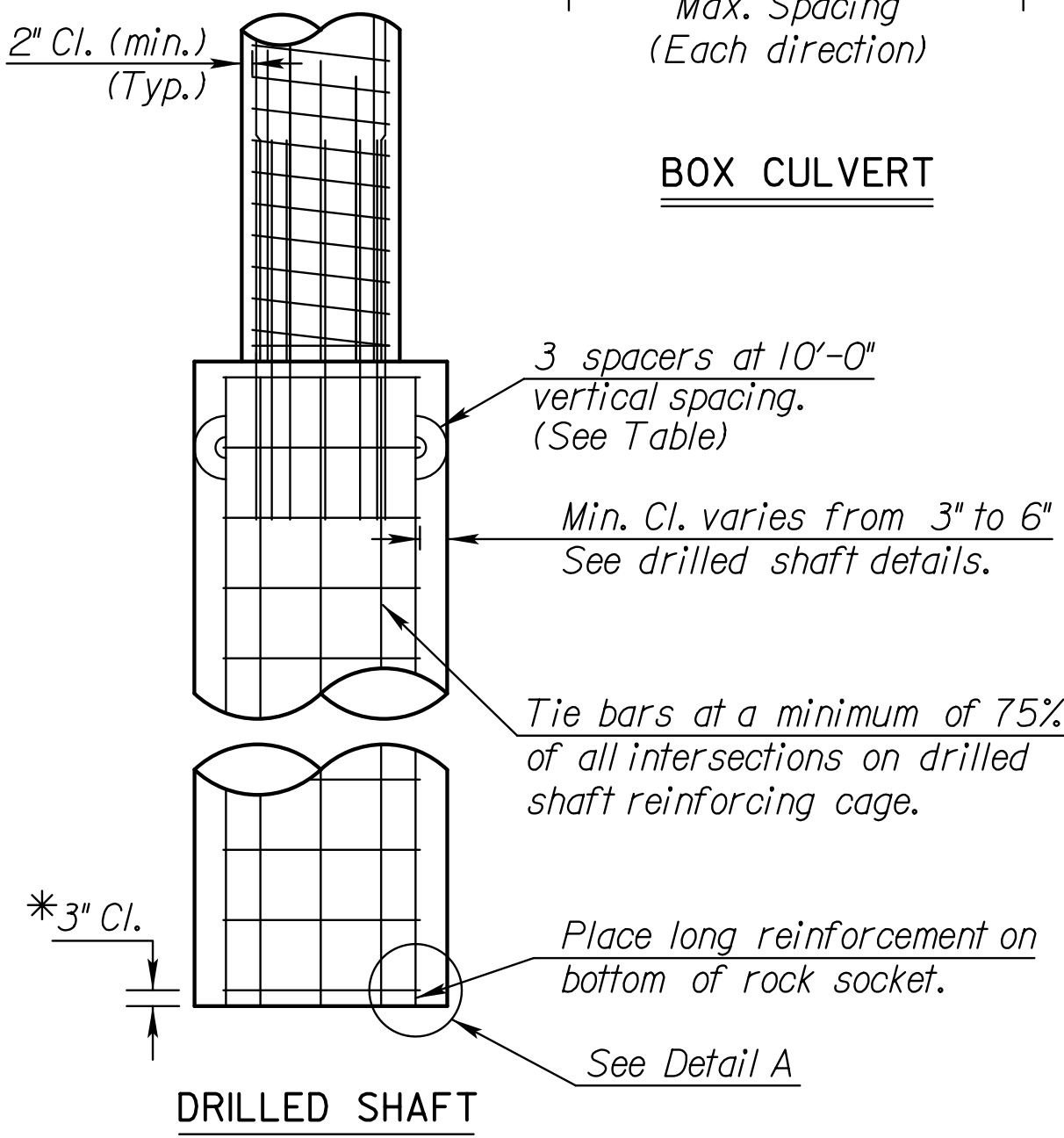
PIER



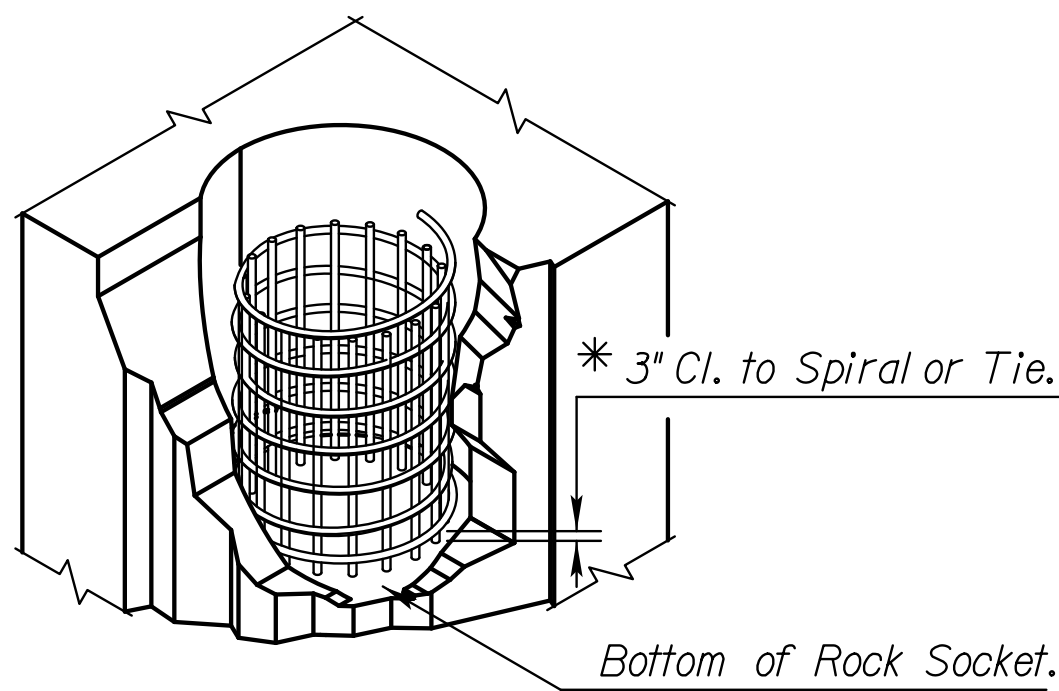
ABUTMENT



BOX CULVERT



DRILLED SHAFT



DETAIL A

\* Note: Longitudinal reinforcing steel is placed on the bottom of the rock socket. Maintain 3" clearance from the bottom of rock socket to the first spiral or tie bar.

Req'd Shaft Supports		
Diameter (in.)	Circumference (in.)	No. of Spacers
18	56	3
24	75	3
30	94	4
36	113	4
42	131	5
48	150	6
54	169	6
60	188	7
66	207	7
72	226	8
78	244	9
84	263	9
90	282	10
96	301	11
102	320	11
108	339	12

Std. Base File: bri20.dgn  
Plotted: Jake.Pfannenstiel@ks.gov  
File: ka572901bss0048-03  
Plot Date: 15-JUL-2024 15:58



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	67	141

DRAINAGE STRUCTURES																	
LOCATION	STATION	OFFSET	SIDE	SIZE	TYPE	ENTRANCE PIPES (FT.)					END SECTIONS (EACH)				REMARKS		
						RCP	PVCP, PEP, PPP, ACSP, CAP, RCP		PVCP, PEP, PPP, SRPE, ACSP, CAP, RCP		PEP, PPP, SRPE, ACSP, CAP, RCP		Ø				
							24"	18"	24"	30"		42"		18"		24"	30"
Field Ent.	1+20		℄	30"	EP					92			2				
Field Ent.	2+80		℄	24"	EP			86				2					
Field Ent.	6+20		℄	18"	EP		48					2					
K-63	73+00	78'	Lt.	42"	EP					60				2			
Ditch Realign.	3+00	2.5'	Lt.	24"	EP	36						2			Low Water Crossing		
Ditch Realign.	3+00	2.5'	Rt.	24"	EP	36						2			Low Water Crossing		
TOTAL						72	48	86		92	60	2	6	2	2		

Ø See Summary of Pipe Culverts Sheet No. 32 for Allowable End Section Types

STEEL PLATE GUARDRAIL (MGS)							
ROUTE	STATION to STATION	SIDE	LENGTH (FT)	END TERMINALS		MGS THRIE BEAM BRIDGE CONNECTION (INFO ONLY)	REMARKS
				FLARED			
				MGS SRT Alt. 1	MGS FLEAT Alt. 2		
K-63	59+39.34 to 60+91.80	Rt.	112.50	1	1	1	
K-63	60+02.00 to 60+91.80	Lt.	50	1	1	1	
K-63	63+54.80 to 64+44.60	Rt.	50	1	1	1	
K-63	63+54.80 to 65+07.15	Lt.	112.50	1	1	1	
	TOTAL		325.00	4	4	4	

APPROACH SLAB PAVEMENT QUANTITIES							
LOCATION	BRIDGE	STATION TO STATION		WIDTH FEET	LENGTH FEET	CONCRETE PAVEMENT (10" UNIFORM) (AE)(BR APP) SQ.YD.	REMARKS
K-63	63-66-54.90 (048)	60+78.80	60+91.80	36	13	53.4	
K-63	63-66-54.90 (048)	63+54.80	63+67.80	36	13	53.4	
TOTAL						106.8	

TEMPORARY CONCRETE SAFETY BARRIER							
LOCATION	STATION TO STATION		TYPE F3 (TEMPORARY) (LIN. FT.)	TYPE F3 (TEMPORARY - RELOCATE) (LIN. FT.)	IMPACT ATTENUATOR (TEMPORARY) (TL-3)	REPLACEMENT MODULES (IMPACT ATTENUATOR)	REMARKS
PHASE 2: K-63 LT	47+25	86+75	3,800		2	10	
PHASE 3: K-63 RT	48+00	76+00		3,800			Impact Attenuator and Replacement Modules relocate to Phase 3
TOTAL			3,800	3,800	2	10	

GUARDRAIL, REMOVAL OF STEEL PLATE				
ROUTE	STATION	SIDE	LENGTH (FT)	REMARKS
K-63	60+50.56 to 61+23.82	Rt.	76.04	Leave on Site
K-63	60+61.87 to 61+23.82	Lt.	62.86	Leave on Site
K-63	63+76.38 to 64+64.57	Lt.	87.82	Leave on Site
K-63	63+76.38 to 64+64.77	Rt.	87.78	Leave on Site
	TOTAL		314.50	

EARTHWORK													
LOCATION	STATION to STATION	EXCAVATION						COMPACTION		NOT SUBGRADED THROUGH CUTS			SALVAGE TOPSOIL SQ.YD.
		COMMON		ROCK		CONTR. FURN. CU.YDS.	TYPE AA MR-5-5 CU.YDS.	TYPE A MR-5-5 CU.YDS.	COMM. CU.YDS.	TYPE AA CU.YDS.	TYPE A CU.YDS.		
		CU.YDS.	VMF	CU.YDS.	VMF								
Phase I: Temporary Widening													
@ K-63	46+00 to 88+00	547	0.82	—		1,149		1,391					
Phase II: Lt.													
@ K-63	49+00 to 85+00	10,391	0.82	—		13,325	2,543	16,905	462	462		34,226.7	
Phase III: Rt.													
@ K-63	49+00 to 85+00	12,239	0.82	—		2,829	2,721	9,634	679	679		38,153.9	
TOTALS		23,117				17,303	5,264	27,930	1,141	1,141		72,380.6	

[illegible][illegible]

\*Non- Participating

For Low Water Crossing Quantities, See Sh. No. 13  
For Bridge Quantities, See Sh. No. 33  
For Surfacing Quantities, See Sh. No. 68  
For Temporary Erosion and Pollution Control Quantities, See Sh. No. 69  
For Seeding Quantities, See Sh. No. 80  
For Signing Quantities, See Sh. No. 94  
For Traffic Control Quantities, See Sh. No. 118

02	01-14-08	Rem. Drainage Structure summary	S.W.K.	J.O.B.
01	01-09-91	Detailed on CADD	R.J.S.	J.O.B.
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
SUMMARY OF QUANTITIES				
RD050				
FHWA APPROVAL		05-28-08	APP'D.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	S.W.K.

Plotted by : Brogan.Andrews@ks.gov 17-JUL-2024 15:46  
File : KA572901rsq-01.dgn

GENERAL NOTE:

On surfacing projects, the 6" of Compaction Type AA, shown for the center portion on the roadbed, is for the purpose of restoring the original Compaction Type AA which may have been lost since grading operations. The exact locations of this Compaction Type AA, which will be required, is to be determined by the Engineer at the time of construction. This work shall be paid under the bid item "Compaction of Earthwork (Type AA)(MR-5-5)".

Over all structures, unless otherwise directed by the Engineer, where the top of the hubguard is level with or above the finished shoulder grade, the earth cover over the structure slab shall be removed and backfilled with material as directed by the Engineer. The removal of this material will be subsidiary.

The material used to backfill over the structure shall be paid for at the prices shown in the contract.

The earth shoulders shall be compacted full depth (Type MR ) except, when ordered by the Engineer, the top 3" shall be left uncompacted for seeding.

All side roads and house entrances shall be surfaced with to the R/W line as indicated on the detail. All side roads and house entrances with existing asphalt surface shall be surfaced with at least to the

R/W line or to the end of construction, as directed by the Engineer. Each mailbox turnout (ON PROJECTS WHERE STABILIZED SHOULDERS ARE NOT SPECIFIED) shall be surfaced with to the limits shown on the detail.

Surfacing material (SA ) shall be used for surfacing house entrances and side roads ( C.Y./SQ. YD.) beyond the limits of the asphalt surface to the limits of construction as determined by the Engineer.

The thickness of side road and entrance surfacing may be increased to the same thickness as the stabilized shoulder within the approximate limits of the shoulder.

On projects which specify both asphalt base and surface course materials, side roads, house entrances and mailbox turnouts may be surfaced with both materials at the contractors option, with the approval of the Engineer.

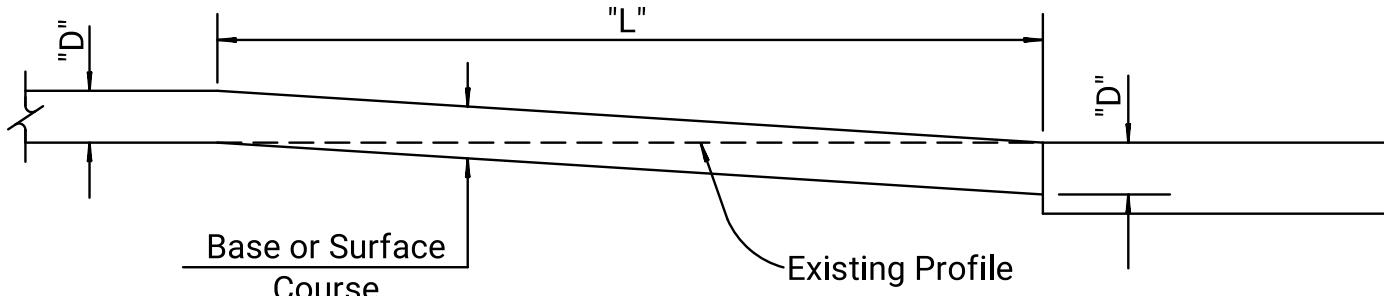
Quantities for aggregate for shoulders, AS-1, are calculated on the basis of 150 lbs. per cu. ft. Quantities for stabilized base course, AB-3, are calculated on the basis of 1 56 lbs. per cu. ft. Weight/cu. ft. includes moisture allowed by specification.

The base course shall be constructed to the plan thickness as shown.

Thicknesses indicated for all construction which is paid for on a weight or volume basis are approximate and may vary to correct for unevenness in the foundations or for other normal unevenness encountered in placement operations.

A tack coat of SS-1HP shall be provided between each lift of all base courses and surface courses and under the first lift of base or surface courses when they are placed on an existing asphalt, brick, or concrete surface, when so ordered by the Engineer and at the rate designated by him. Quantities are included for these tacks calculated at the rate of 0.06 gal. /sq. yd.

Asphalt Material quantities are calculated on the basis of 8.328 lbs. per gal. Shoulder rumble strips will not be constructed as part of this project.



TYPICAL PROFILE AT GRADE CONTROL POINTS

The Contractor shall cut the subgrade in accordance with this profile at all grade control points, i.e. ; existing pavements, grade bridges and R.R. crossings, also at changes in thickness of base or surface courses. Corresponding dimensions of "D" and "L" shall be as given in the table below.

The work of cutting the subgrade and disposing of excess excavated material shall be subsidiary to other items in the contract.

TABLE OF DIMENSIONS											
D	L	D	L	D	L	D	L	D	L	D	L
1"	25'	3"	75'	5"	125'	7"	175'	9"	225'	11"	275'
2"	50'	4"	100'	6"	150'	8"	200'	10"	250'	12"	300'

SUMMARY OF QUANTITIES											
LOCATION		STATION TO STATION		SIDE	* HMA-COMMERCIAL GRADE (CLASS A) (TONS)			AGGREGATE BASE (AB-3) (6.0") (SQ.YD.)	AGGREGATE SHOULDER (AS-1) (6.0") (SQ.YD.)	Φ CALCIUM CHLORIDE (TONS)	REMARKS
					4.0"	6.0"	9.0"				
Phase I:											
℄ K-63	46+00	49+00	Lt.		36.9		128.4				Temporary Widening
℄ K-63	47+00	87+00	Rt.		542.0		1,821.1				Temporary Widening
℄ K-63	84+98.44	88+00	Lt.		36.9		128.4				Temporary Widening
℄ K-63	86+86.90	87+82.07	Lt.					83.6	0.04		216th Rd
Phase II:											
℄ K-63	49+00	60+78.80	Lt.			1,060.0	2,198.4				Mainline
℄ K-63	49+00	57+00	Lt.						0.23		Shoulder
℄ K-63	57+13.02	57+86.77	Lt.					417.6	0.53		Field Entrance
℄ K-63	59+96.95	60+91.80	Lt.	2.9				62.1	0.08		Guardrail Pad
℄ K-63	63+54.80	65+12.15	Lt.	2.9				110.5	0.14		Guardrail Pad
℄ K-63	63+67.80	85+00	Lt.			1,842.8	3,823.7				Mainline
℄ K-63	67+00	85+00	Lt.					418.2	0.53		Shoulder
℄ K-63	72+63.27	73+36.73	Lt.					279.9	0.35		Field Entrance
Phase III:											
℄ K-63	49+00	60+78.80	Rt.			988.0	2,051.4				Mainline
℄ K-63	49+00	60+78.80	Rt.					295.8	0.37		Shoulder
℄ K-63	57+12.52	57+87.22	Rt.					1,233.6	1.55		Field Entrance
℄ K-63	59+34.42	60+91.80	Rt.	2.9				109.9	0.14		Guardrail Pad
℄ K-63	63+54.80	64+49.57	Rt.	2.9				62.2	0.08		Guardrail Pad
℄ K-63	63+67.80	85+00	Rt.			1,778.2	3,692.4				Mainline
℄ K-63	67+00	85+00	Rt.					516.0	0.65		Shoulder
℄ K-63	72+62.73	73+37.27	Rt.					212.1	0.27		Field Entrance
TOTALS					11.6	615.8	5,669.0	13,843.8	3,985.7	5.0	

\* Computed at the rate of 145 lbs./Cu. Ft.

φ Computed at the rate of 0.003726 tons per Cu. Ft. of AS-1

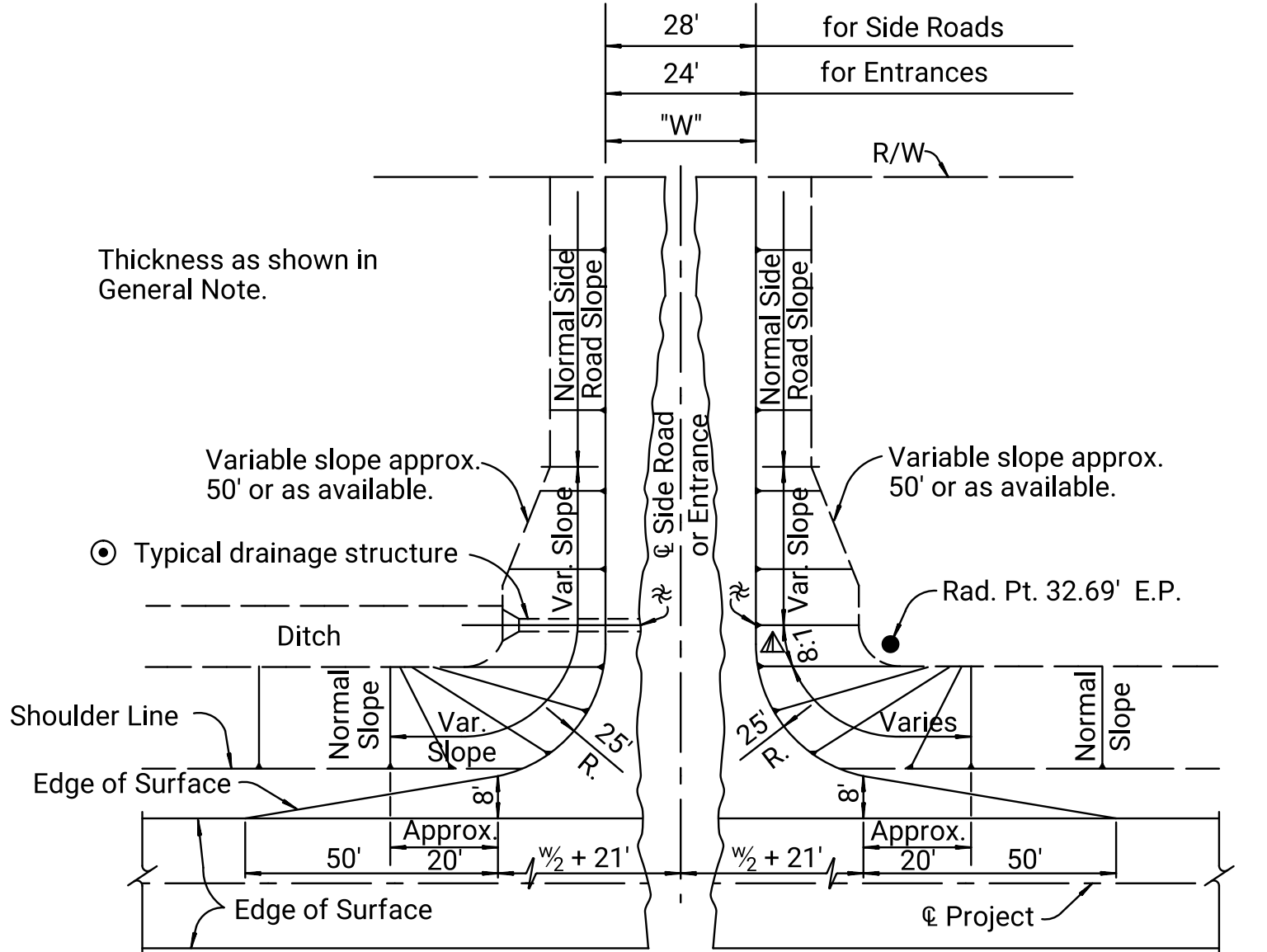
MILLING		
LOCATION	MILLING (TONS) †	REMARKS
Phase II: Sta. 49+00 to Sta. 85+00	2,056	Exist. Lt. Lane
Phase III: Sta. 49+00 to Sta. 85+00	2,656	Exist. Rt. Lane & Temp. Pavement
TOTAL	4,712	

† Computed at the rate of 145 lbs/cu.ft.

TRANSPORTING SALVAGEABLE MATERIAL			
ITEM	TOTAL	UNIT	DELIVERY LOCATION
Millings	4,712	Tons	KDOT Mixing Strip 1.6 miles N. of Seneca on K-63

RECAPITULATION OF QUANTITIES		
ITEM	TOTAL	UNIT
HMA-Commerical Grade (Class A)	6,296	Tons
Field office and Laboratory (Type A)	1	Each
Aggregate Base (AB-3)(6.0")	13,844	SQ. YD.
Aggregate Shoulder (AS-1)(6.0")	3,986	SQ. YD.
Water (Aggregate Base) (Set Price)	1	MGal.
Water (Aggregate Shoulder) (Set Price)	1	MGal.
Calcium Chloride	5	Tons
Milling	4,712	Tons
⊗Transporting Salvageable Material (5-8MI)	4,712	Tons

⊗ Non-Participating



WITH DRAINAGE STRUCTURE MOUND ENTRANCE OR SIDE ROAD

DETAIL FOR SURFACING OF SIDE ROADS & HOUSE ENTRANCES

▲ 8:1 Slope at the appropriate clear zone shall apply to all mound entrances and mound side roads to 10' fill height. Normal Slope (but not steeper than 6:1) for over 10' fill height.

⊙ Normal Slope (but not steeper than 6:1) at approximate ℄ Structure or appropriate clear zone width.

≈ On side roads and entrances which slope toward the highway, a low point approx. 6" deep shall be constructed to divert surface drainage into the highway ditch, unless otherwise shown on the plans.

12	01-10-07	Changed bituminous to asphalt	S.W.K.	J.O.B.
11	08-30-06	Changed tack type/rate	S.W.K.	J.O.B.
10	03-24-05	Revised compaction, tack type/rate	S.W.K.	J.O.B.

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
SUMMARY OF QUANTITIES (Surfacing)				
RD051				
FHWA APPROVAL		09-06-06	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	



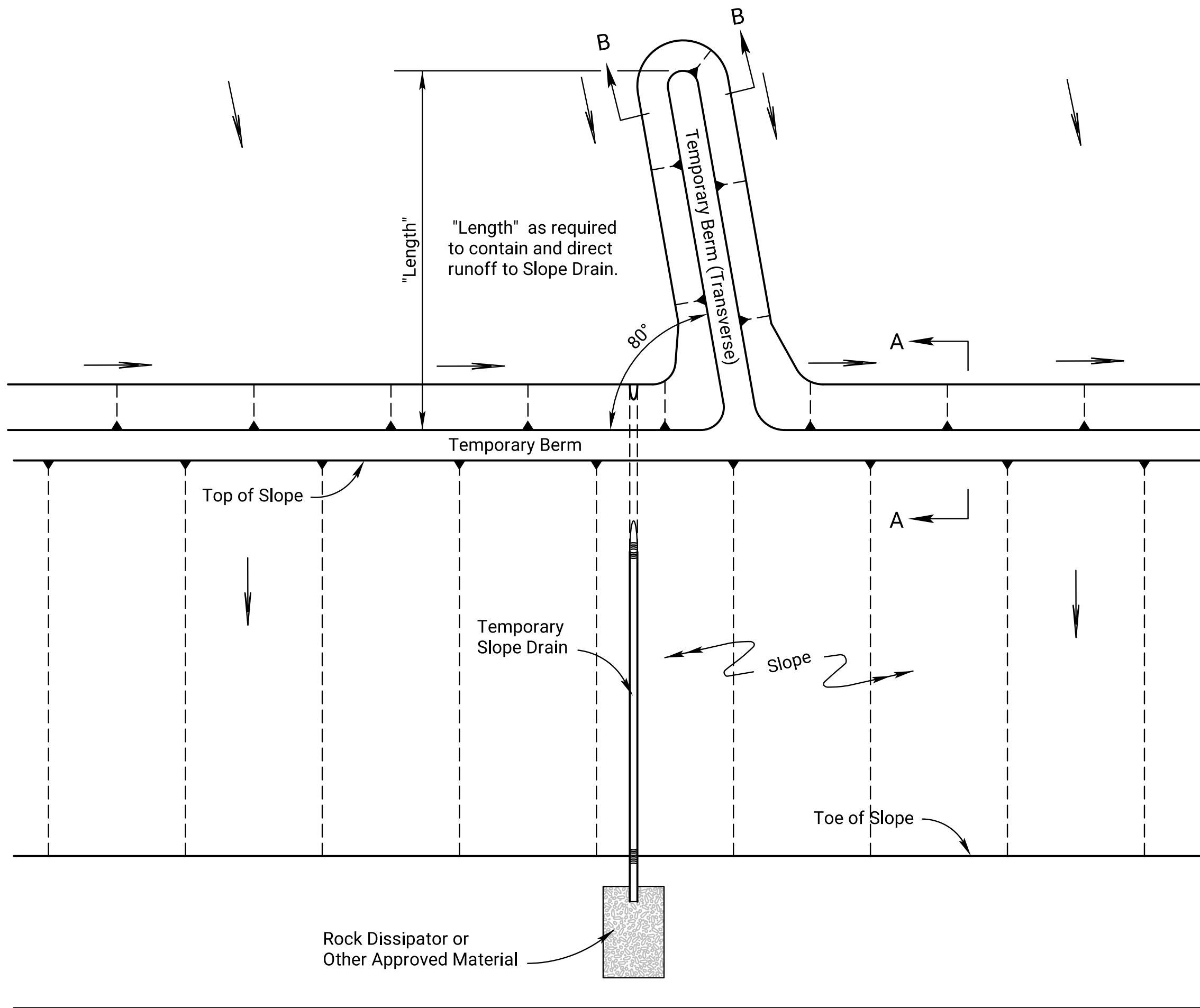




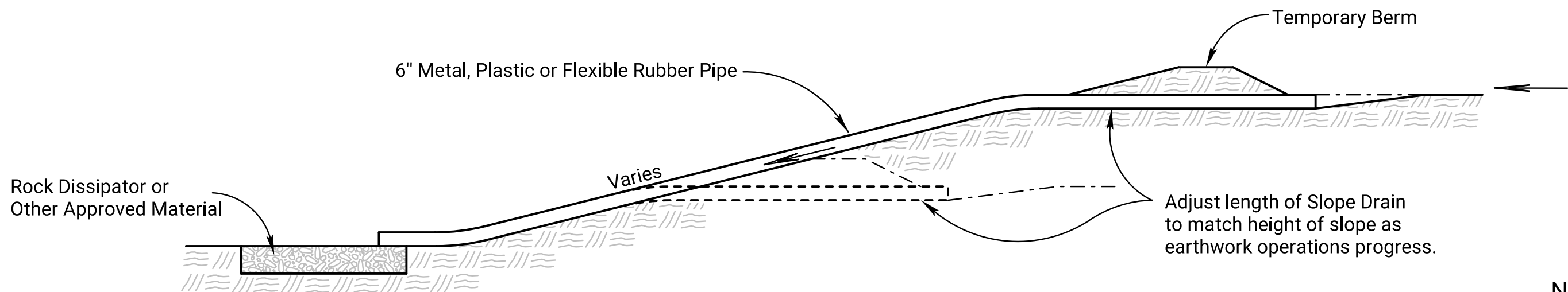




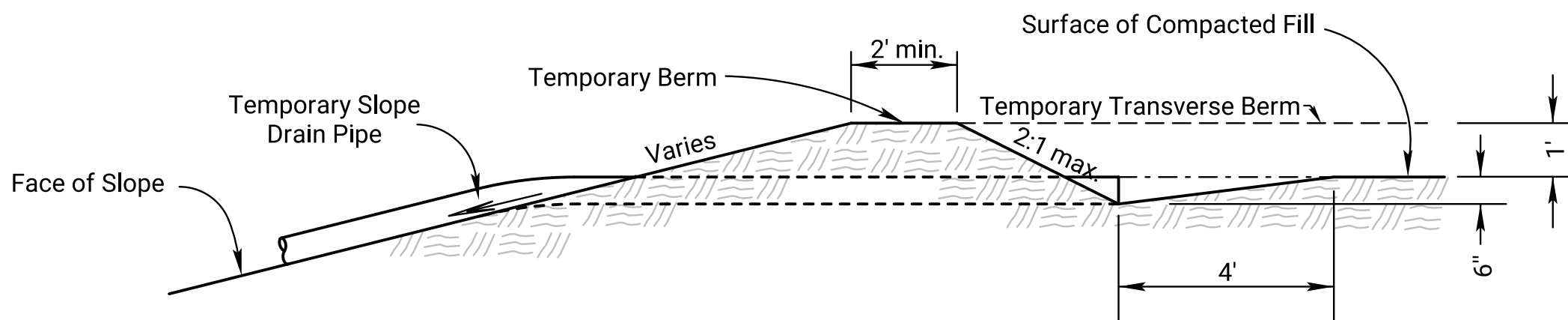
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	72	141



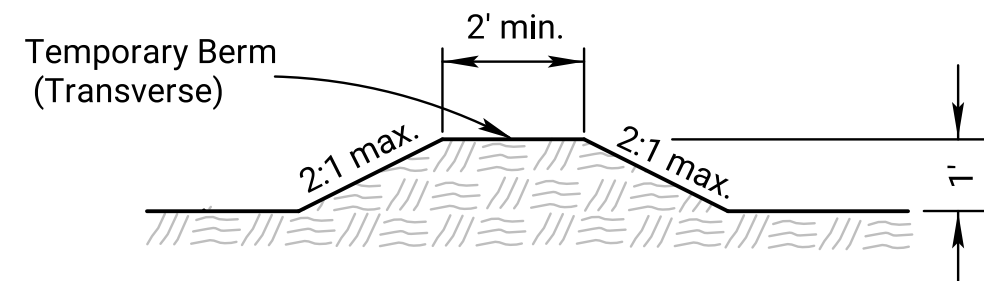
TYPICAL PLAN VIEW OF  
TEMPORARY BERM AND  
TEMPORARY SLOPE DRAIN  
NO SCALE



TYPICAL PROFILE OF TEMPORARY SLOPE DRAIN  
NO SCALE

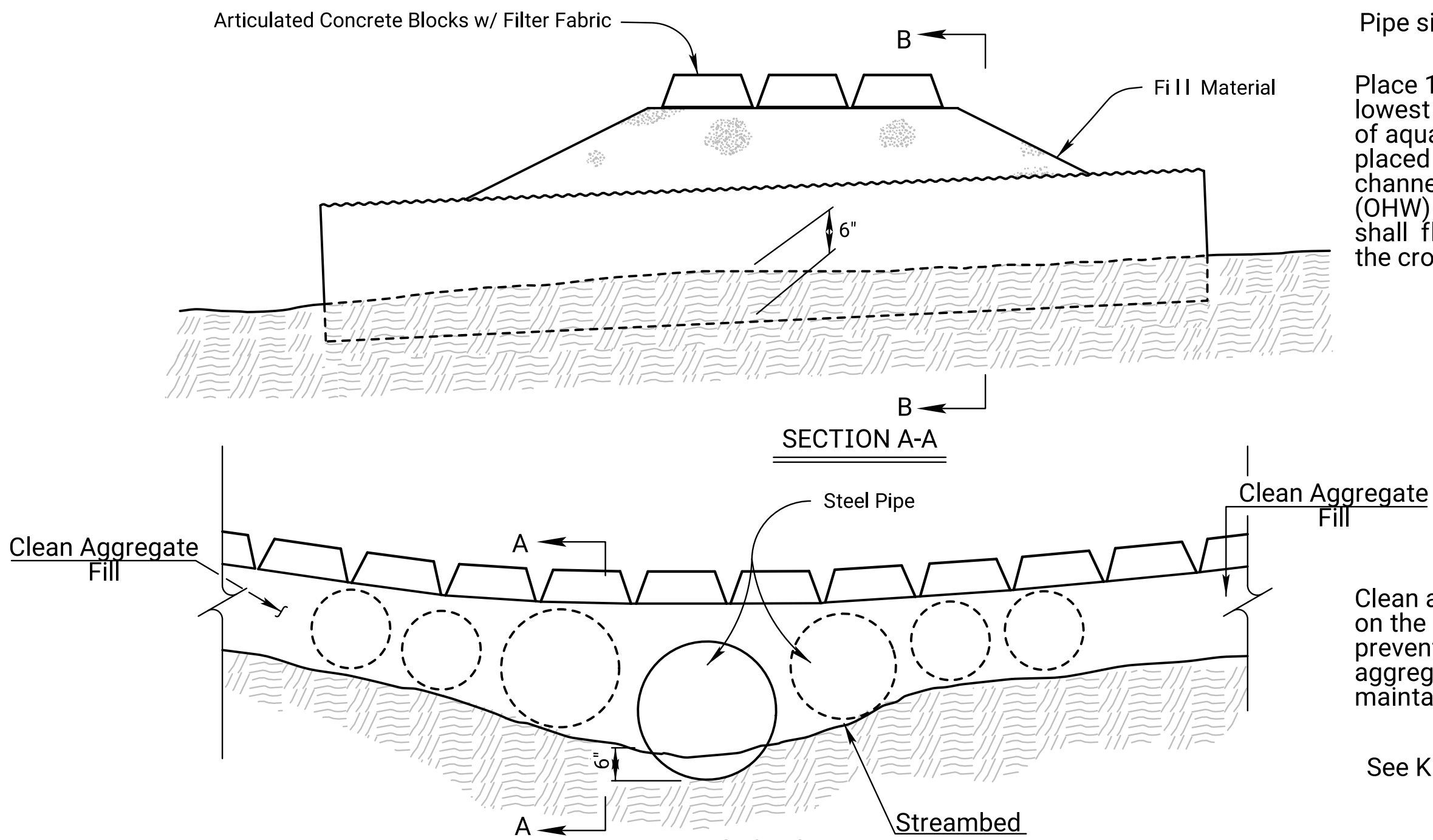


SECTION A-A  
NO SCALE



SECTION B-B  
NO SCALE

TYPICAL PROFILE OF TEMPORARY BERM  
NO SCALE



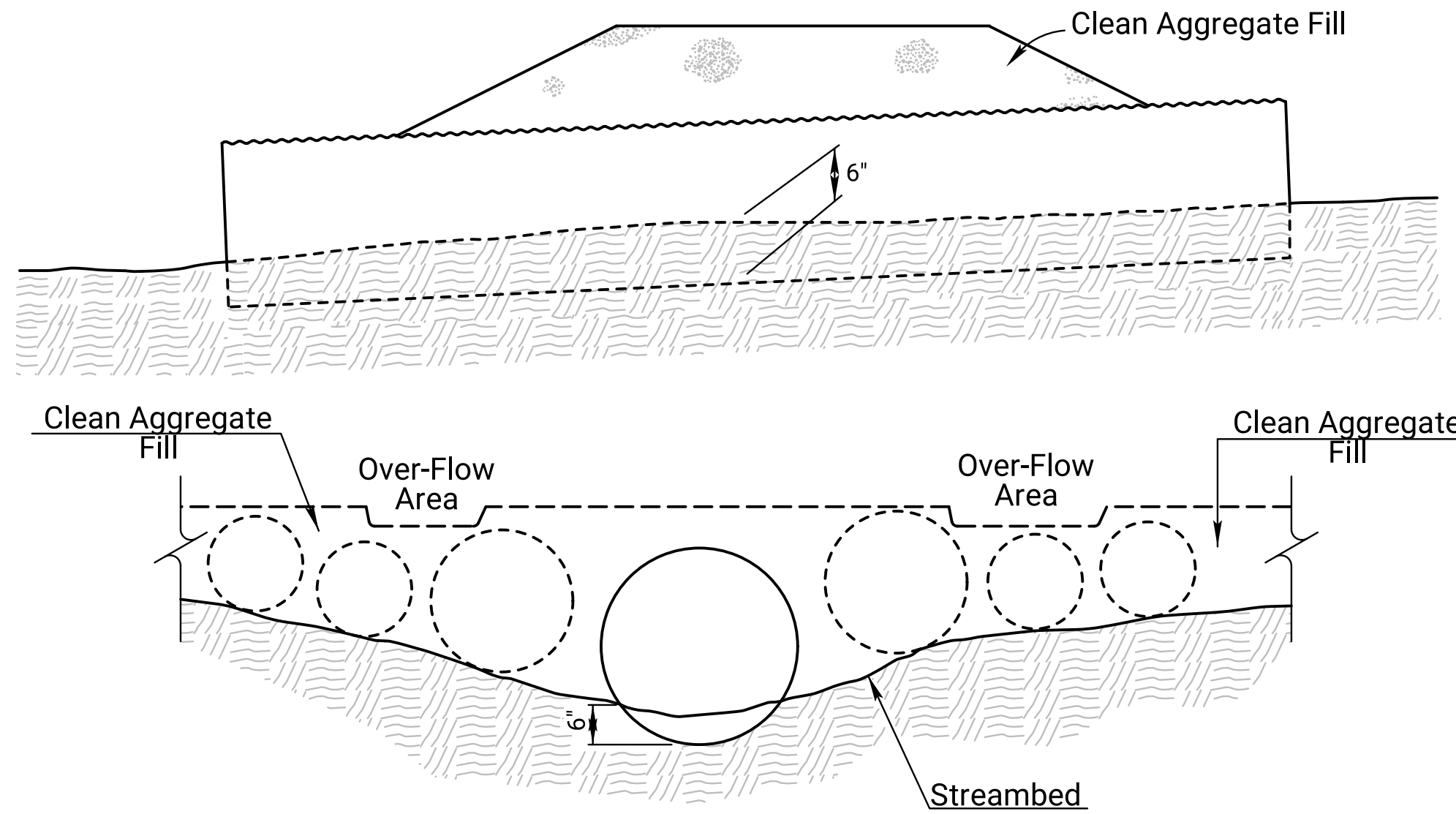
TEMPORARY STREAM CROSSING (ARTICULATED CONCRETE BLOCKS)  
NO SCALE

Pipe size may vary.

Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

Clean aggregate fill will extend a minimum of 50' on the entrance and exit side of the crossing to prevent tracking. The aggregate shall be clean aggregate and a minimum of 6" thick and will be maintained through the use of the crossing.

See KDOT Specifications for more information.



SECTION B-B  
TEMPORARY STREAM CROSSING (AGGREGATE)  
NO SCALE

Pipe size may vary.

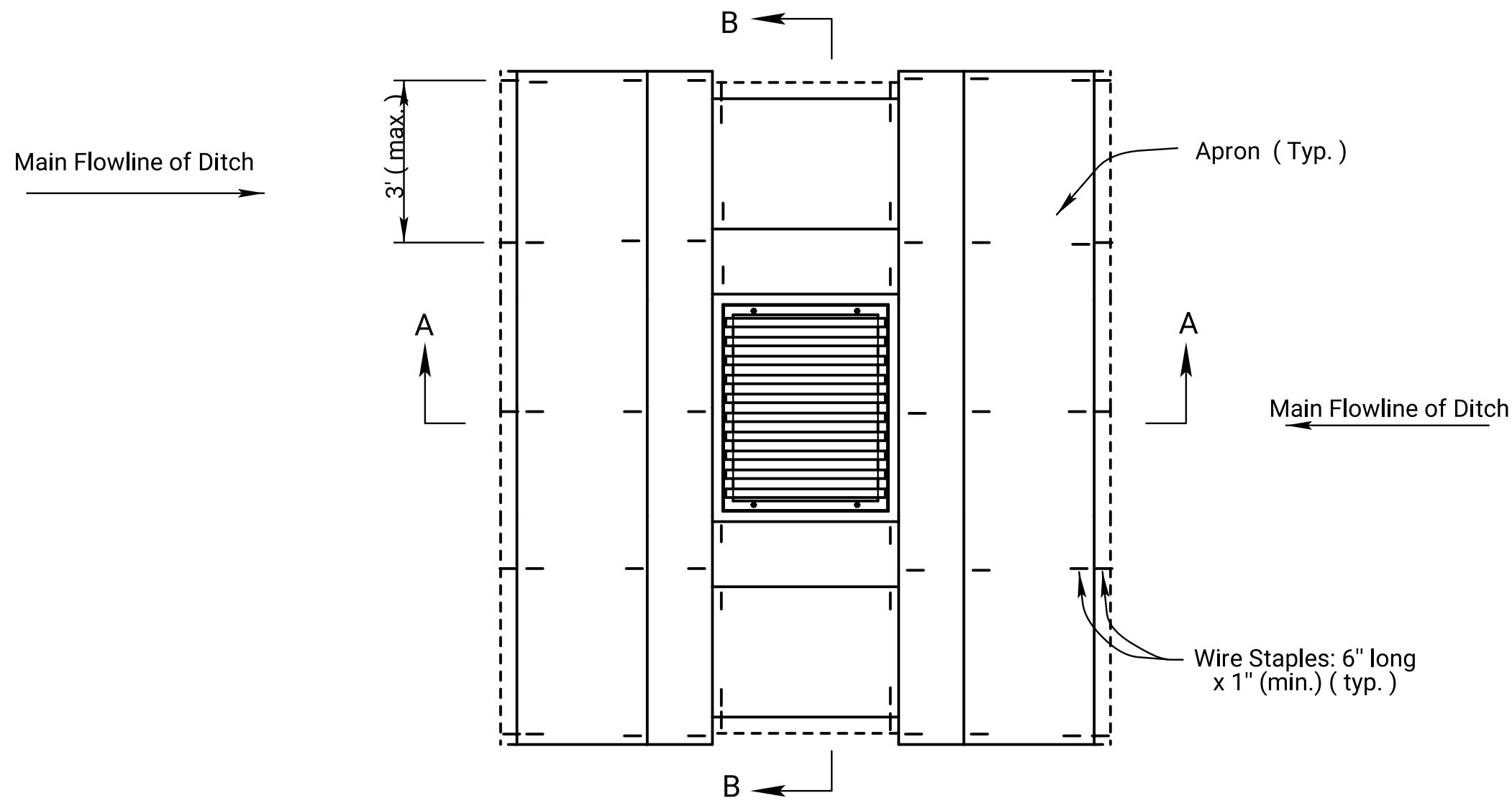
Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

Clean aggregate fill will extend a minimum of 50' on the entrance and exit side of the crossing to prevent tracking. The aggregate shall be clean aggregate and a minimum of 6" thick and will be maintained through the use of the crossing.

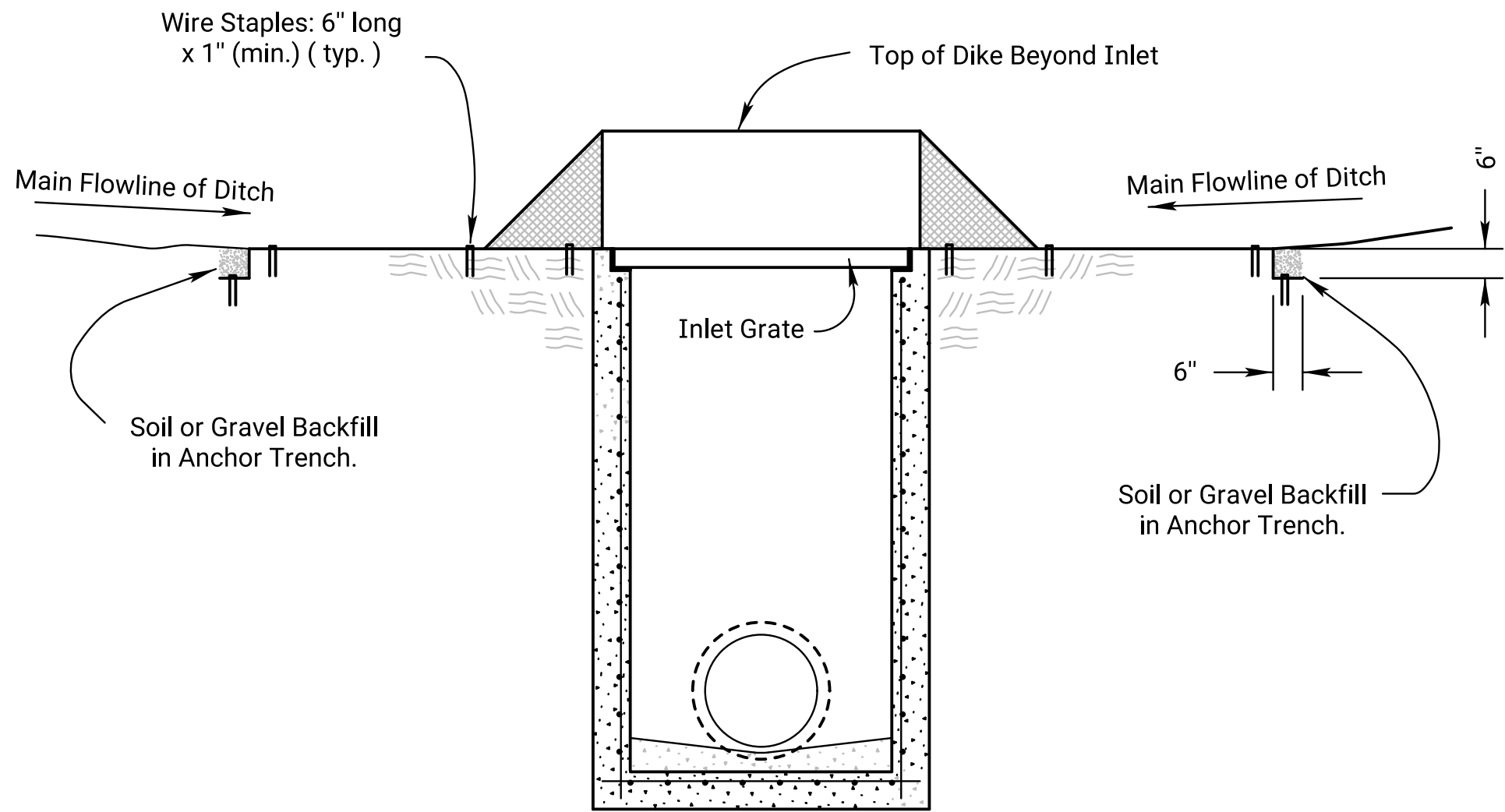
See KDOT Specifications for more information.

KANSAS DEPARTMENT OF TRANSPORTATION					
TEMPORARY EROSION AND POLLUTION CONTROL					
TEMPORARY SLOPE DRAIN, TEMPORARY STREAM CROSSING (AGGREGATE)					
LA852B					
DESIGNED	01-21-22	APPD.	M.R.D.	M.L.	
DESIGN CK.	08-24-21	QUANTITIES	M.R.D.	M.L.	
	06-11-13	QUAN. CK.	M.R.M.	S.H.S.	
NO.	DATE	REVISIONS	BY	APPD	
KDOT Graphics Certified					

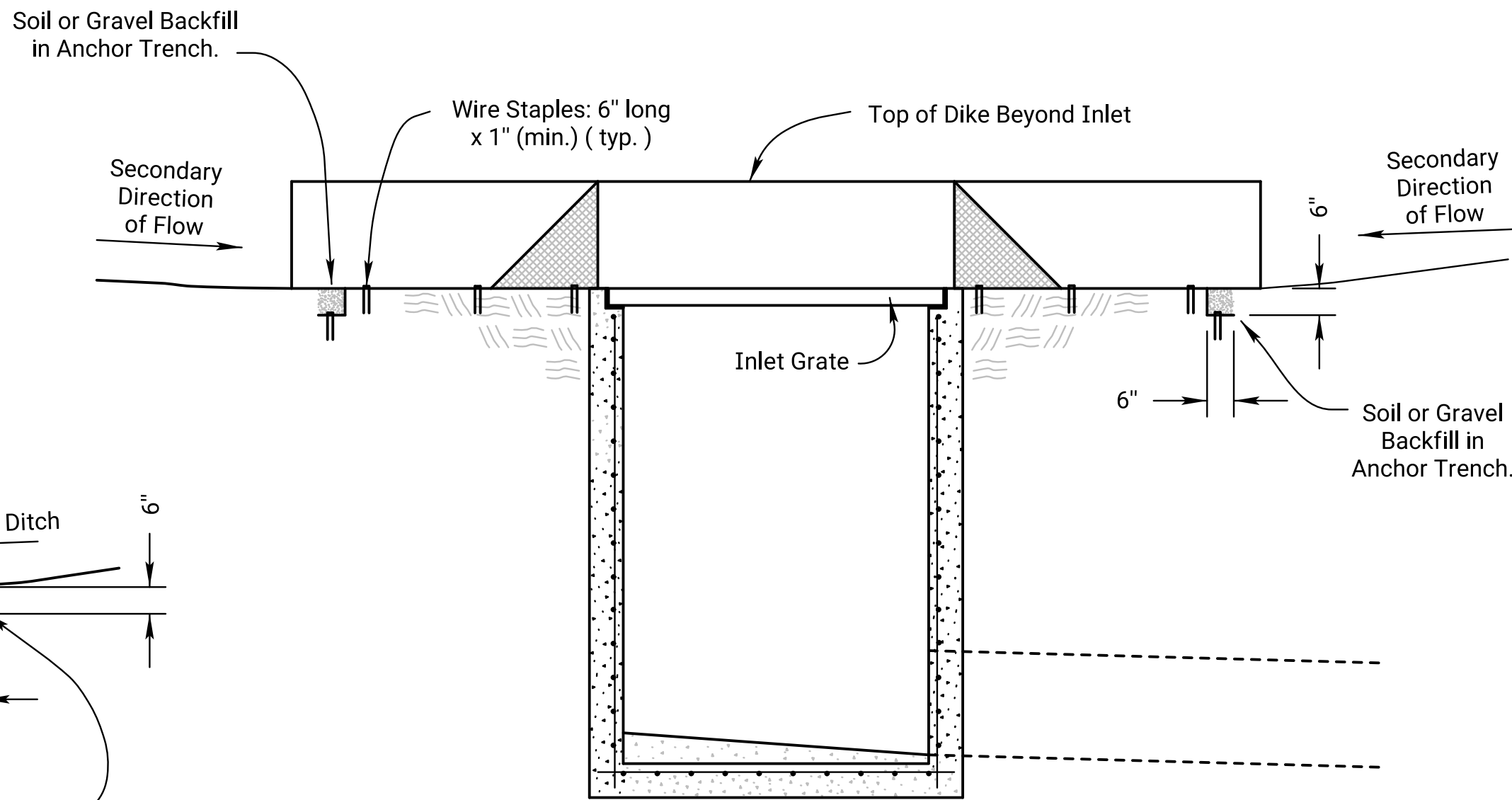
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	73	141



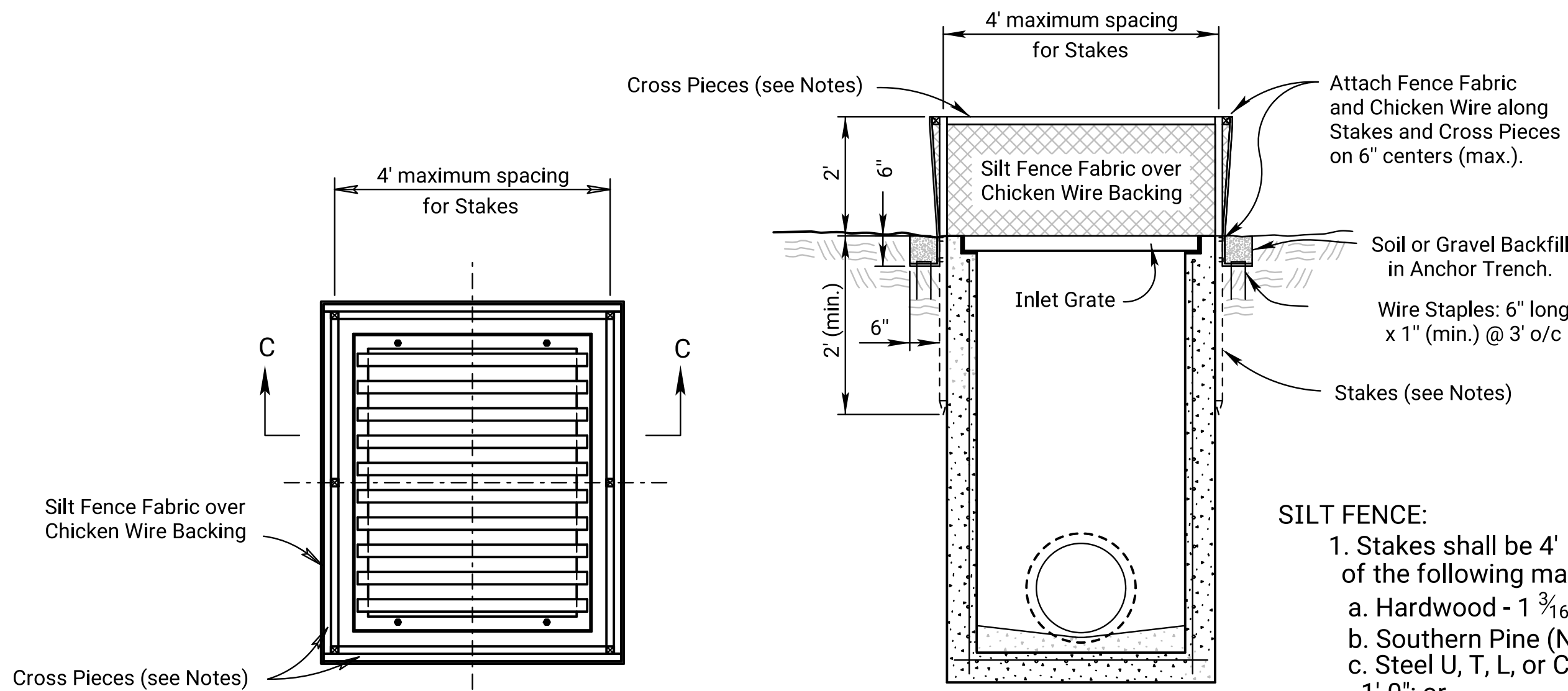
PLAN  
TEMPORARY I NLET SEDI MENT BARRI ER  
(TRI ANGULAR SI LT DI KE METHOD)  
NO SCALE



SECTION A - A



SECTION B - B

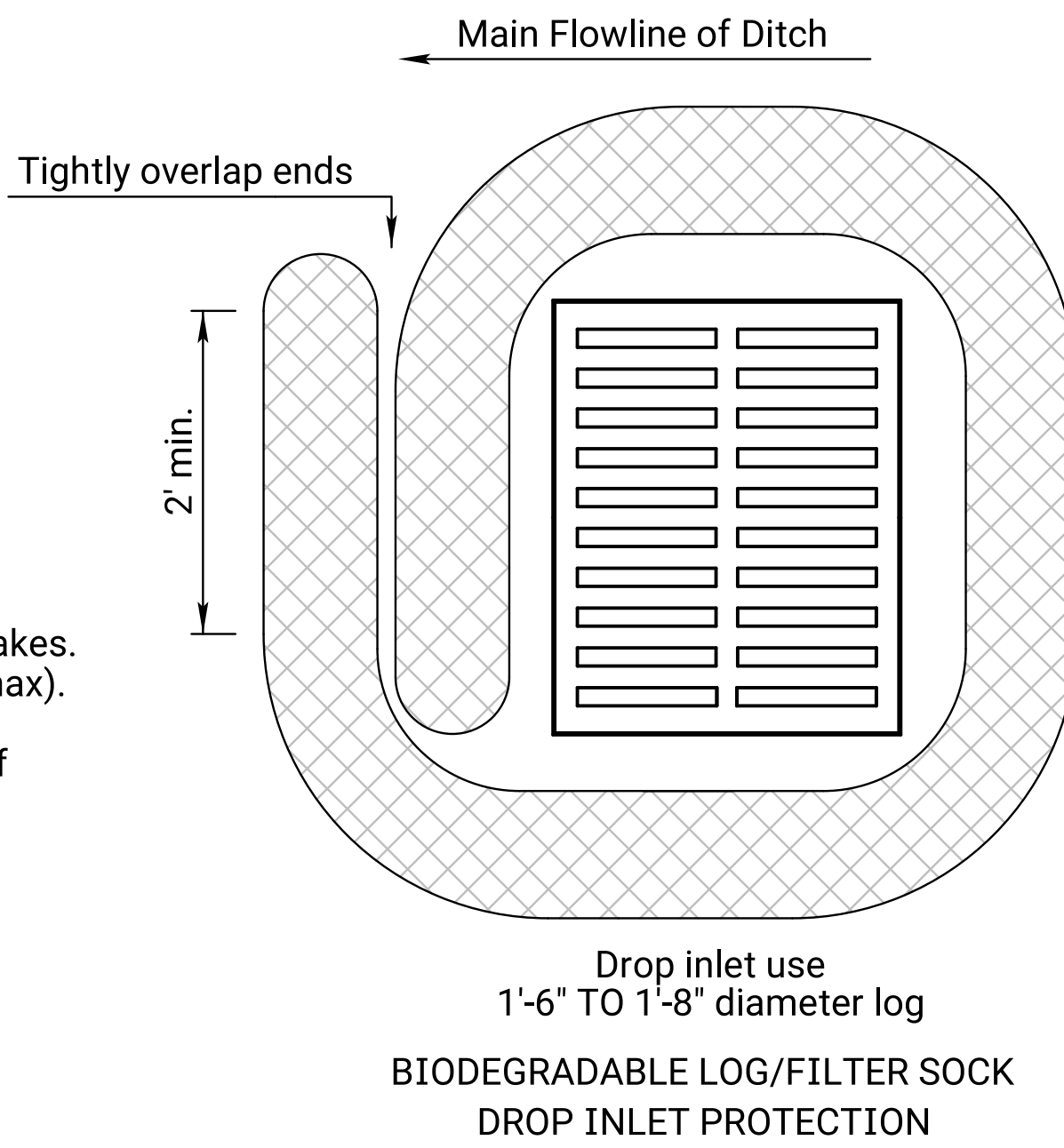


PLAN  
TEMPORARY I NLET SEDI MENT BARRI ER  
(SI LT FENCE METHOD)  
NO SCALE

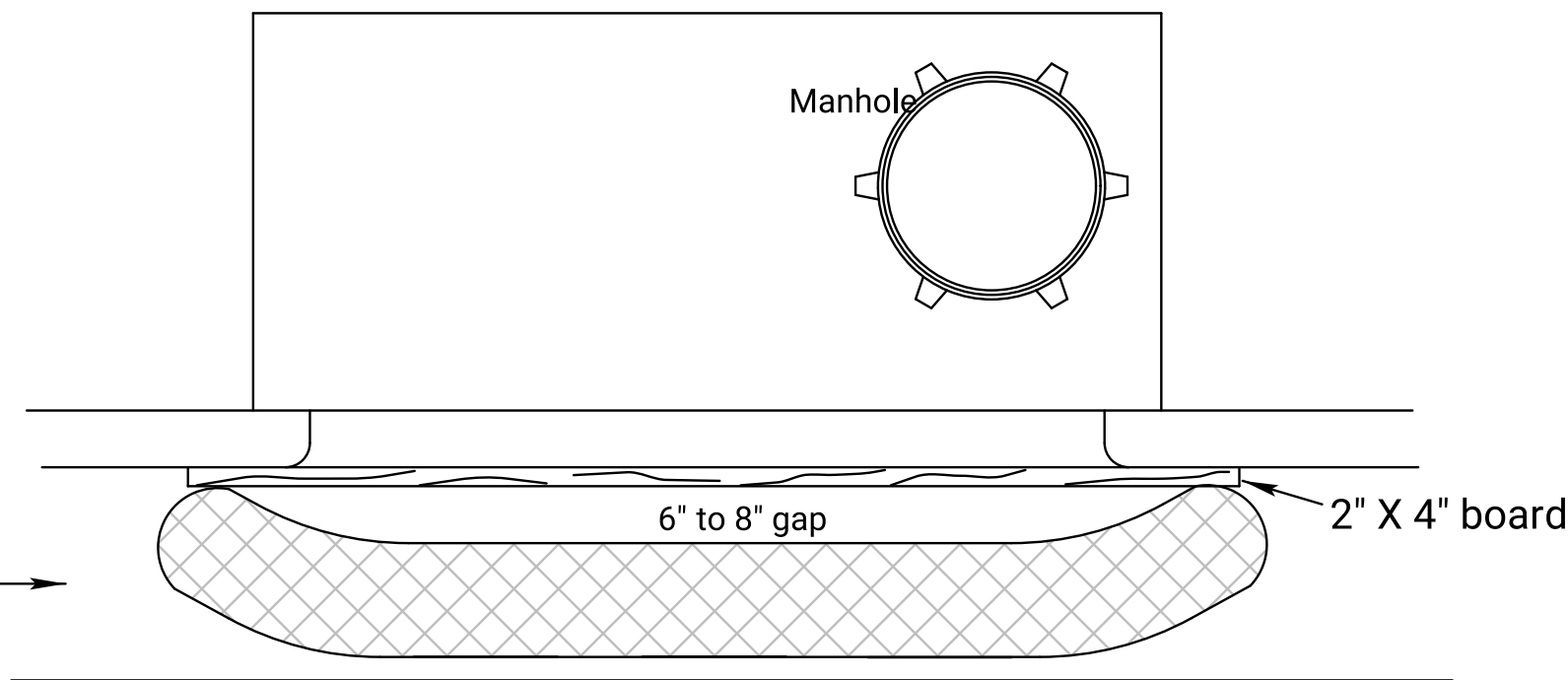
SECTION C - C

- SILT FENCE:
1. Stakes shall be 4' (min.) long and of one of the following materials:
    - a. Hardwood - 1 3/16" x 1 3/16";
    - b. Southern Pine (No. 2) - 2 5/8" x 2 5/8";
    - c. Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
    - d. Synthetic - same strength as wood stakes.
  2. Cross pieces shall be of same material as stakes.
  3. Attach fence fabric securely on 6" centers (max).
  4. Use of high flow material is acceptable.
  5. Refer to plan sheets to estimate the length of silt fence required.

Bags = synthetic net (3mm mesh) or burlap bags  
Rock = approximately 1" to 2" diameter



Drop inlet use  
1'-6" TO 1'-8" diameter log  
BIODEGRADABLE LOG/FILTER SOCK  
DROP INLET PROTECTION



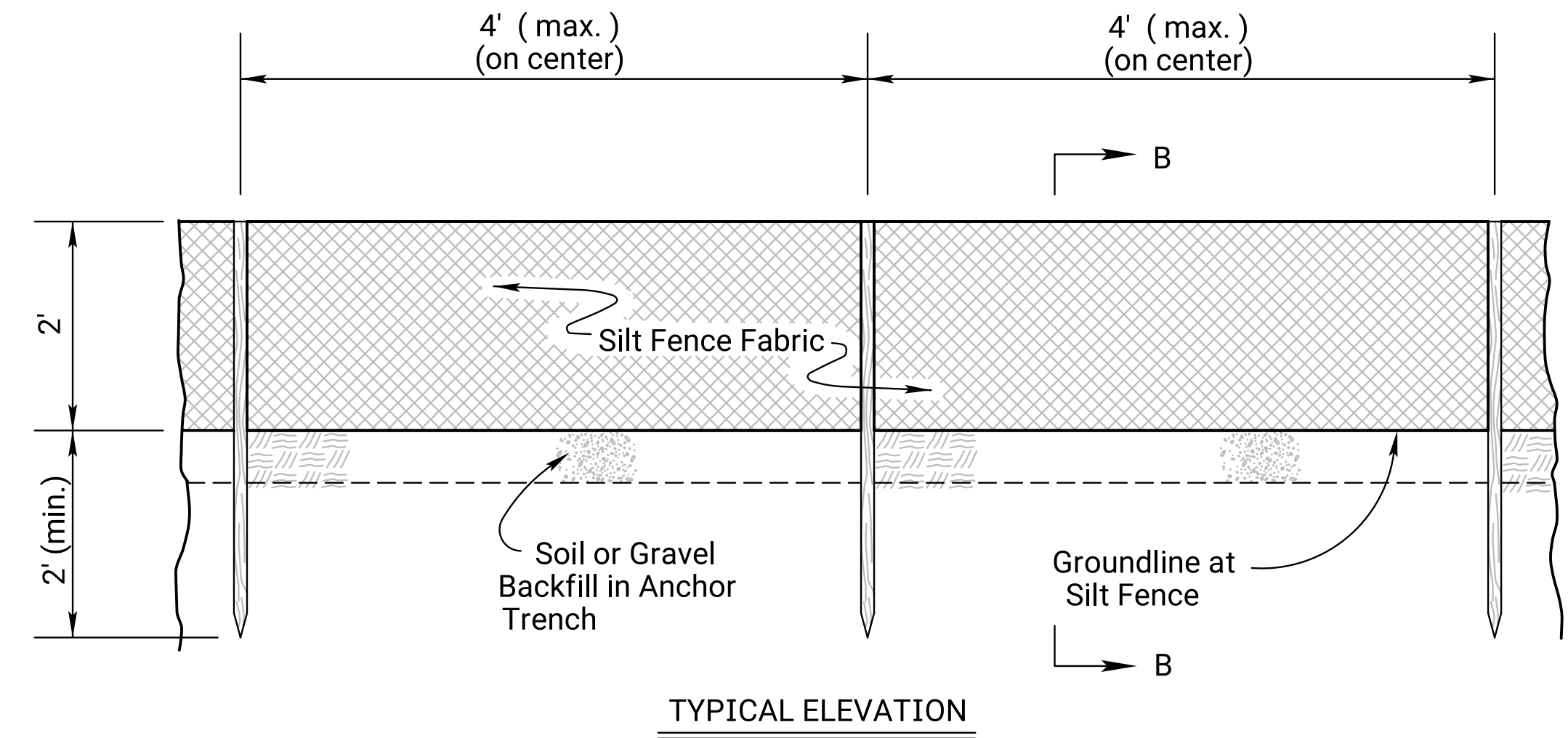
CURB INLET PROTECTION

1. If multiple gravel bags are required, place them in such a way that no gaps are evident.
2. Height of bags (8" minimum diameter) must not be above top of curb.
3. Alternative products may be used other than gravel bags such as the "Gutter Buddy". Products must be approved by the Engineer.
4. Curb inlet protection will be measured and paid for as Filter Sock.

Material Requirements	
Use 100% shredded mulch or other non-compost biodegradable material as fill for logs.	
No compost or fines.	
No hay or straw.	
Do not use material which prohibits water infiltration.	
Log Mesh: Use mesh with 1/4" openings or larger. Mesh must allow water infiltration but also hold fill material in place.	

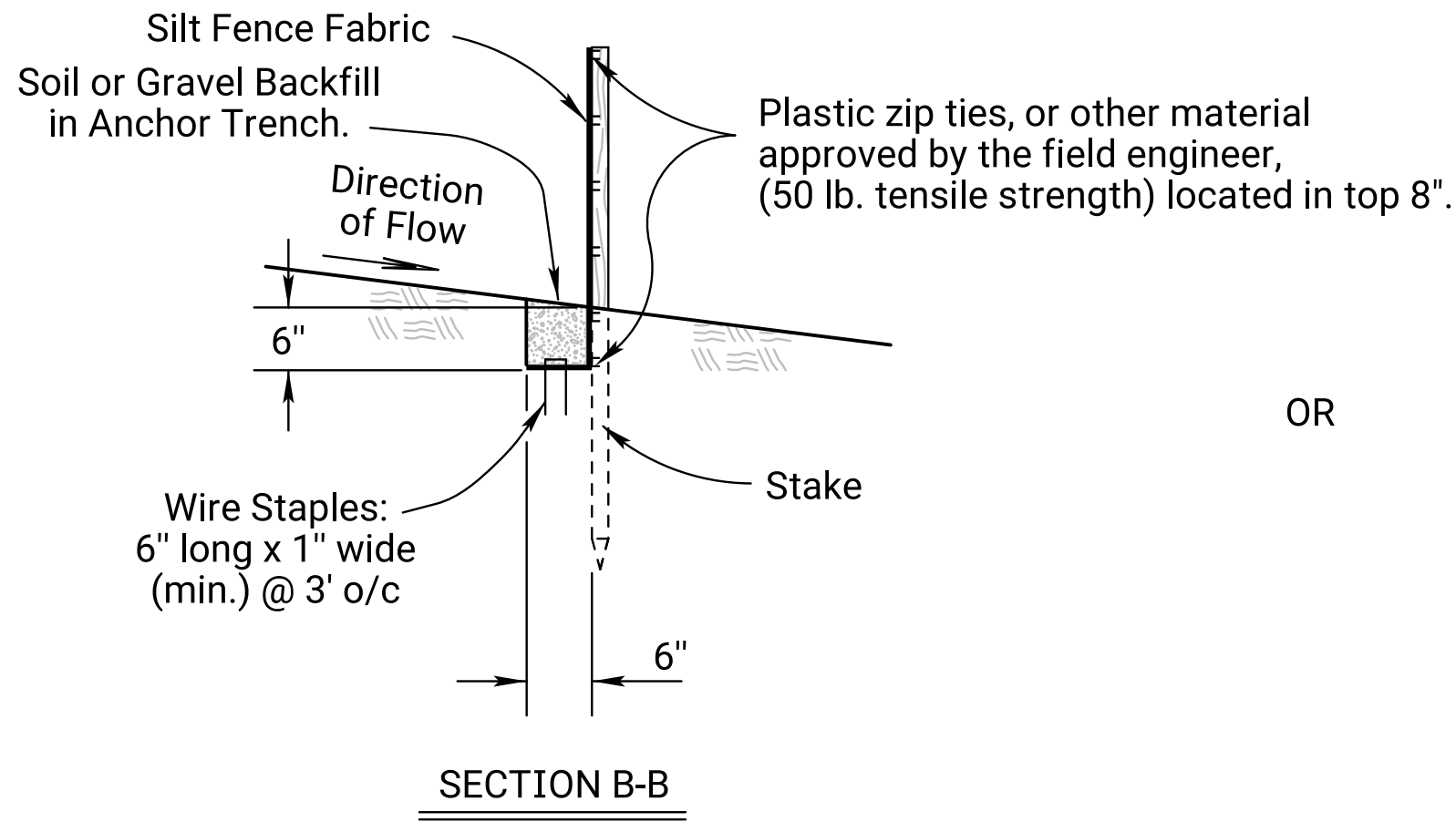
03	09-26-19	Changed Direction of Main Flowline of Ditch Arrow	M.R.D.	S.H.S.
02	03-10-15	Revised Standard	R.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL, TEMPORARY INLET SEDIMENT BARRIER (SILT FENCE) TEMP. INLET SEDIMENT BARRIER (T.S.D.) LA852C				
FHWA APPROVAL		03-10-15	APPD.	Scott H. Shields
DESIGNED	R.A.	DETAILED	R.A.	QUANTITIES
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.
		TRACED		TRACE CK.

Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:36  
File : la852d.dgn



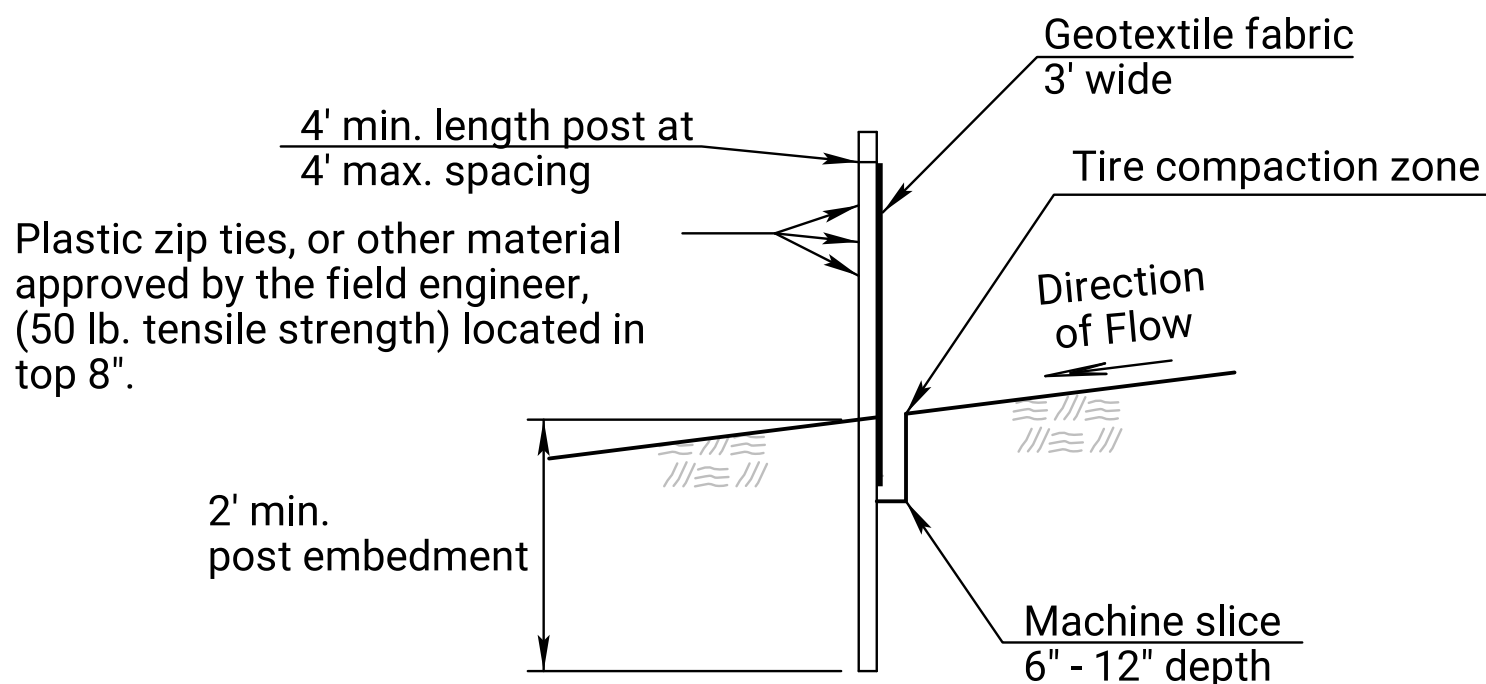
TYPICAL ELEVATION

SILT FENCE BARRIER  
NO SCALE

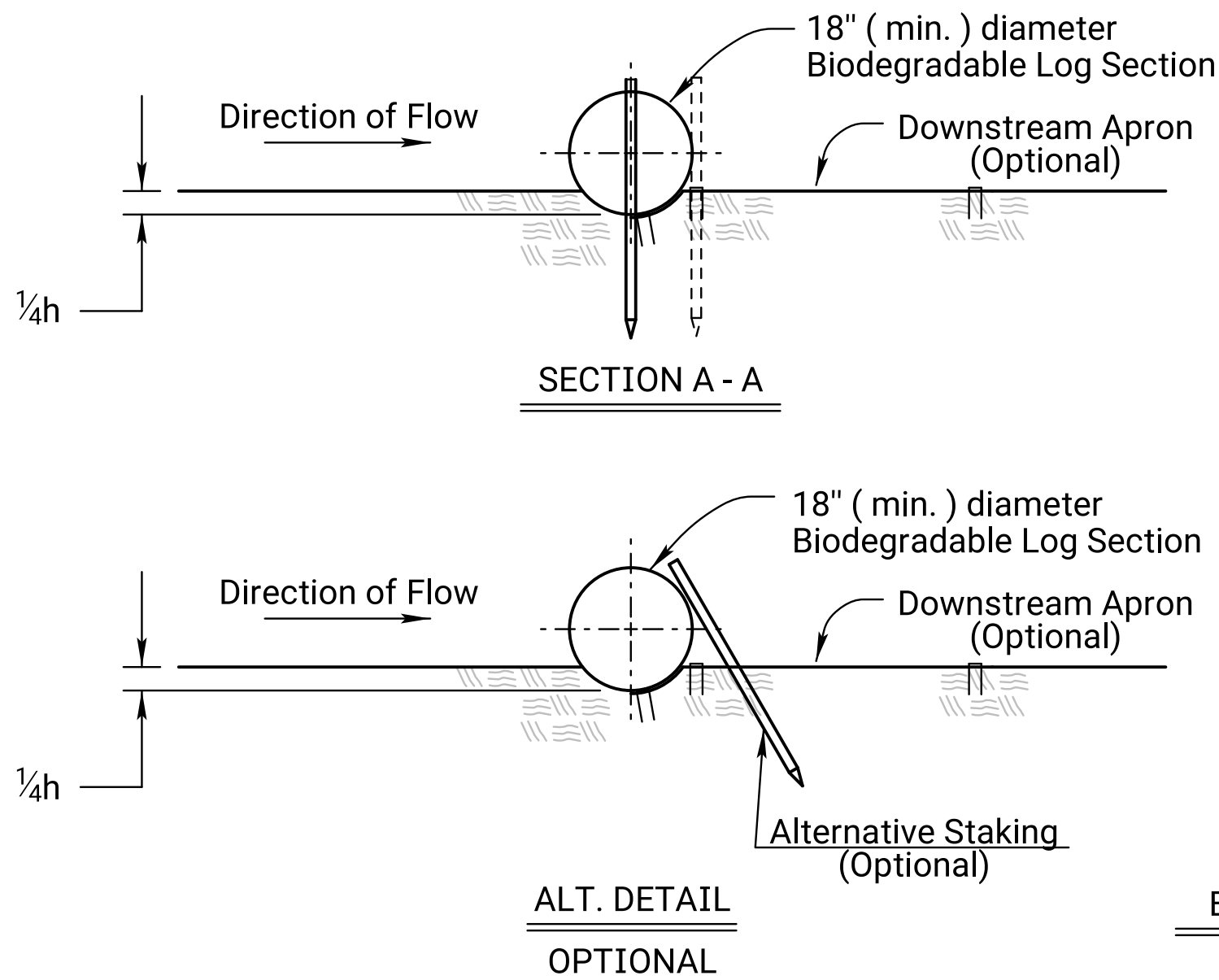


SECTION B-B

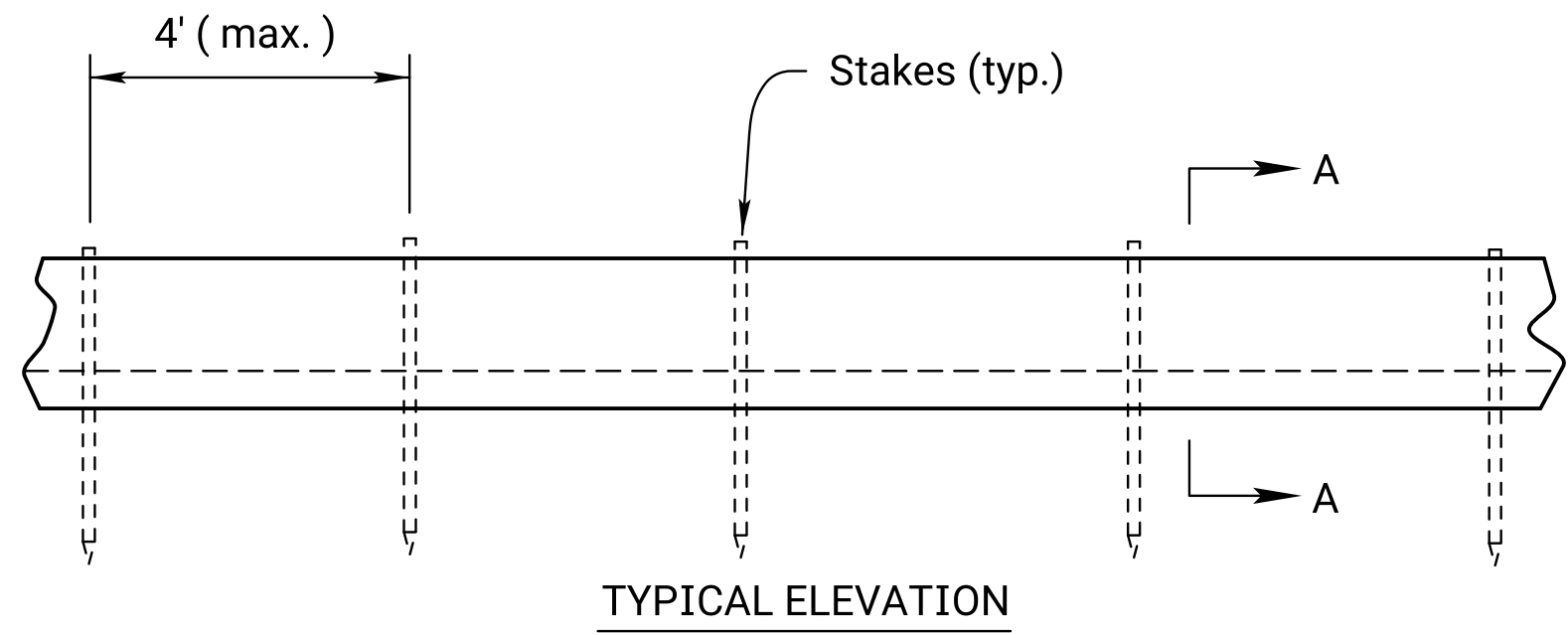
OR



SECTION B-B



ALT. DETAIL  
OPTIONAL



TYPICAL ELEVATION

BIODEGRADABLE LOG SLOPE INTERRUPTIONS  
OR Filter Sock

INSTALLATION NOTES

- SILT FENCE:
- Stakes shall be 4' (min.) long and of one of the following materials:
    - Hardwood - 1 3/16" x 1 3/16";
    - Southern Pine (No. 2) - 2 5/8" x 2 5/8";
    - Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
    - Synthetic - same strength as wood stakes.
  - Attach fence fabric with 3 zip ties within the top 8" of the fence  
Alternate attachment methods may be approved by the Engineer on a performance basis.
  - Use of high flow material is acceptable.
  - Refer to plan sheets to estimate the length of silt fence required.

BIODEGRADABLE LOG OR FILTER SOCK

- Place biodegradable logs or filter sock tightly together minimum overlap of 18".
- Wood stakes shall be 2" x 2" (nom.).
- Refer to plan sheets to estimate length of biodegradable log and filter sock required.
- Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.
- Length of stakes should be 2 times the height of the log at a minimum with minimum ground embedment equal to the height of the log / sock.

Biodegradable Log or Filter Sock Slope Interruptions

		PRODUCT		
		9" Sediment Log or 8" Filter Sock (ft)	12" Sediment Log or 12" Filter Sock (ft)	20" Sediment Log or 18" Filter Sock (ft)
Slope Gradient	≤4H:1V	40	60	80
	3H:1V	30	45	60

Deviations should be approved by the Field Engineer.

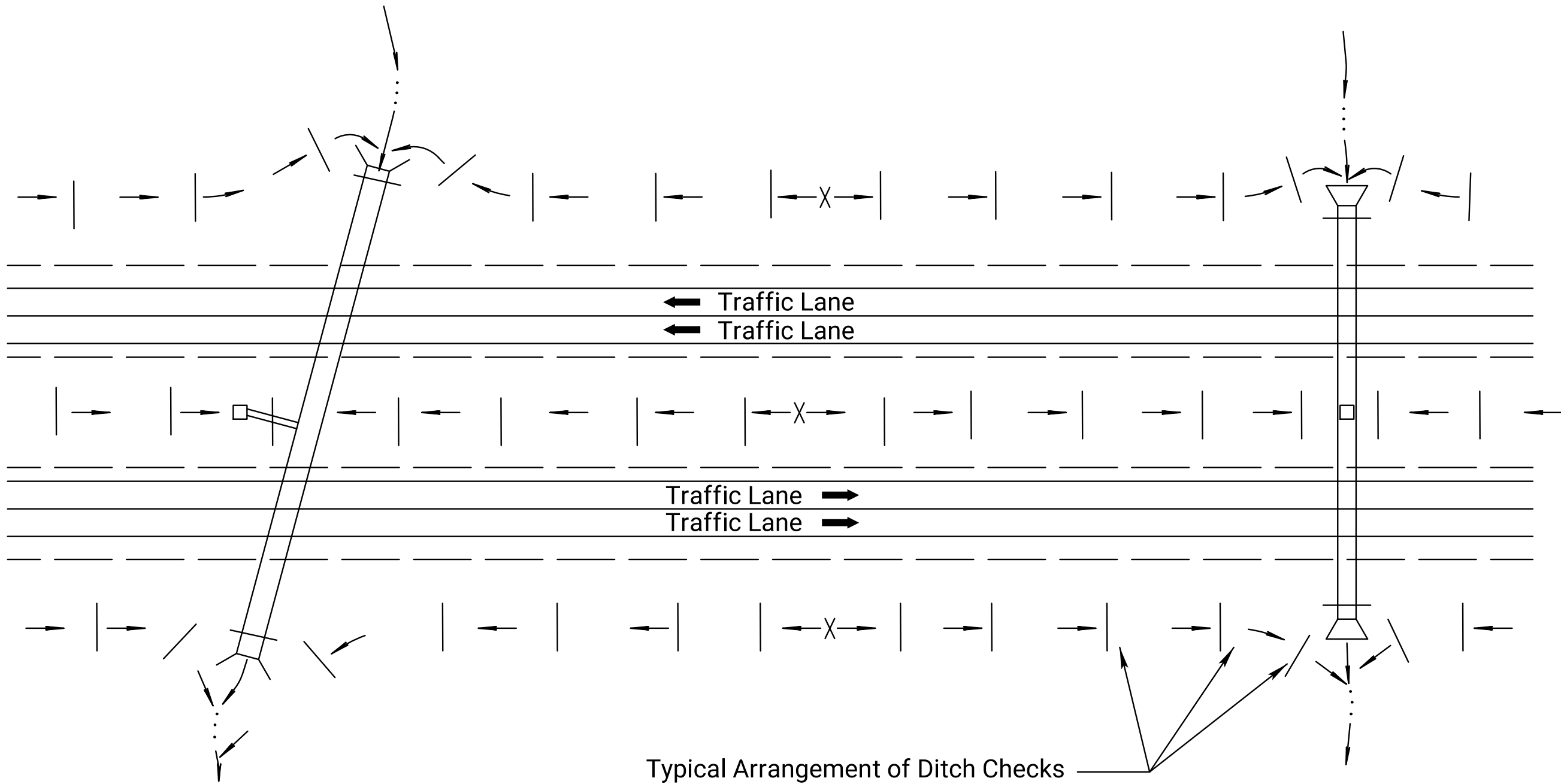
BIODEGRADABLE LOG MATERIAL		
	LOW FLOW	HIGH FLOW
9"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
12"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
18"-20"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber

GENERAL NOTES

- Slope interruptions shall be placed along contour lines, with a short section turned upgrade at each end of the barrier.
- The maximum length of the slope interruptions shall not exceed 250 feet, and the barrier ends need to be staggered.
- Interruptions damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired immediately by Contractor at no additional cost to KDOT.
- Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

03	06-28-16	Revised Standard		R.A.	S.H.S.
02	03-01-15	Revised Standard		R.A.	S.H.S.
01	06-01-13	Revised Standard		M.R.M.	S.H.S.
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
TEMPORARY EROSION AND POLLUTION CONTROL SLOPE INTERRUPTIONS BIODEGRADABLE LOG / SILT FENCE LA852D					
FHWA APPROVAL		09-14-16		APP'D: Scott H. Shields	
DESIGNED	S.H.S.	DETAILED	R.A.	QUANTITIES	TRACED
DESIGN CK.	S.H.S.	DETAIL CK.		QUAN.CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	75	141



TYPICAL DITCH CHECK LAYOUT PLAN  
NO SCALE

20" BIOLOG CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
1.0	125
2.0	60
3.0	40
4.0	30
5.0	25
NOTE: Use this spacing for all except Rock Ditch Checks.	

18" FILTER SOCK CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
1.0	110
2.0	55
3.0	35
4.0	25
5.0	20
NOTE: Use this spacing for all except Rock Ditch Checks.	

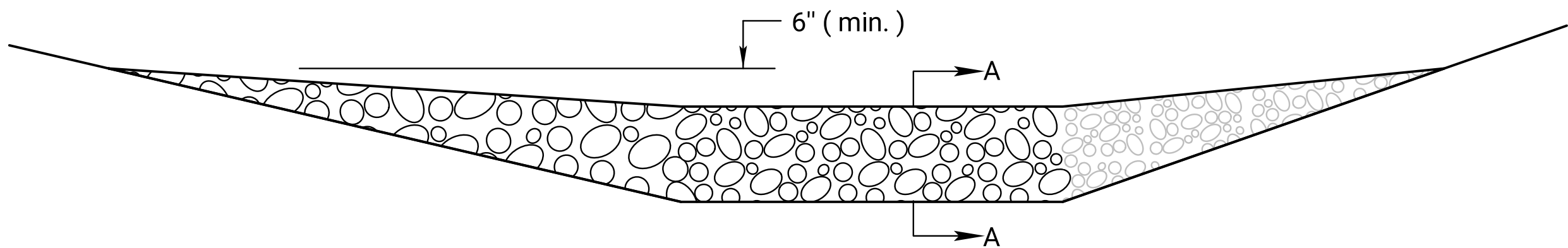
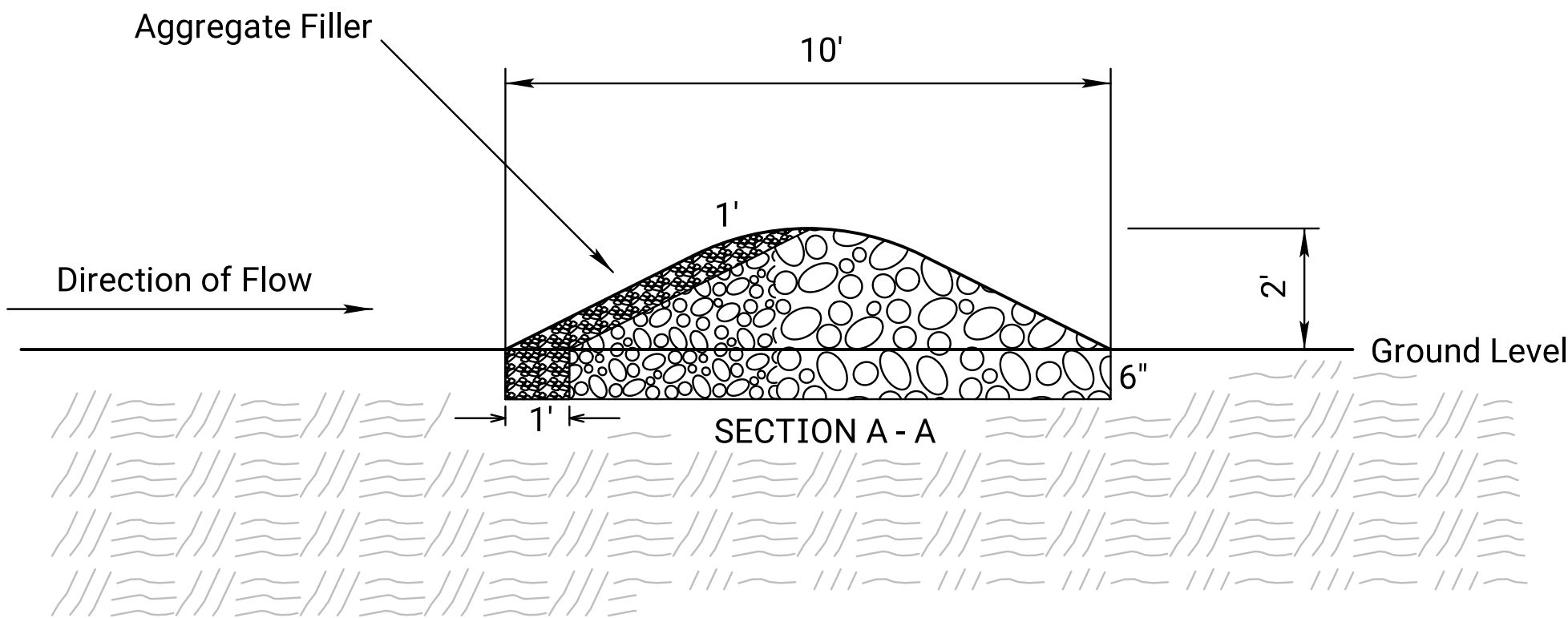
GENERAL NOTES

- 1) The choice of ditch check methods is at the option of the Contractor.
- 2) Use only rock checks in situations where the ditch slope is 6 percent or greater.
- 2) Ditch checks damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired by Contractor at no extra cost to KDOT.

03	08-10-16	Revised Standard			R.A.A.	S.H.S.
02	06-28-16	Revised Standard			R.A.A.	S.H.S.
01	06-01-13	Revised Standard			M.R.M.	S.H.S.
NO.	DATE	REVISIONS			BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION						
TEMPORARY EROSION AND POLLUTION CONTROL DITCH CHECKS						
LA852E						
FHWA APPROVAL		09-14-16		APP'D.		Scott H. Shields
DESIGNED	S.H.S.	DETAILED	R.A.A.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN.CK.	TRACE CK.	S.H.S.



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	76	141



TYPICAL ELEVATION

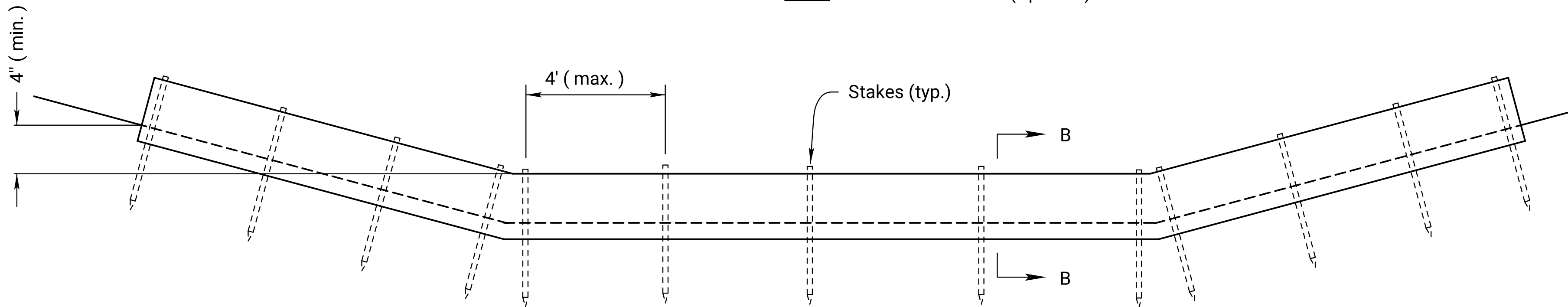
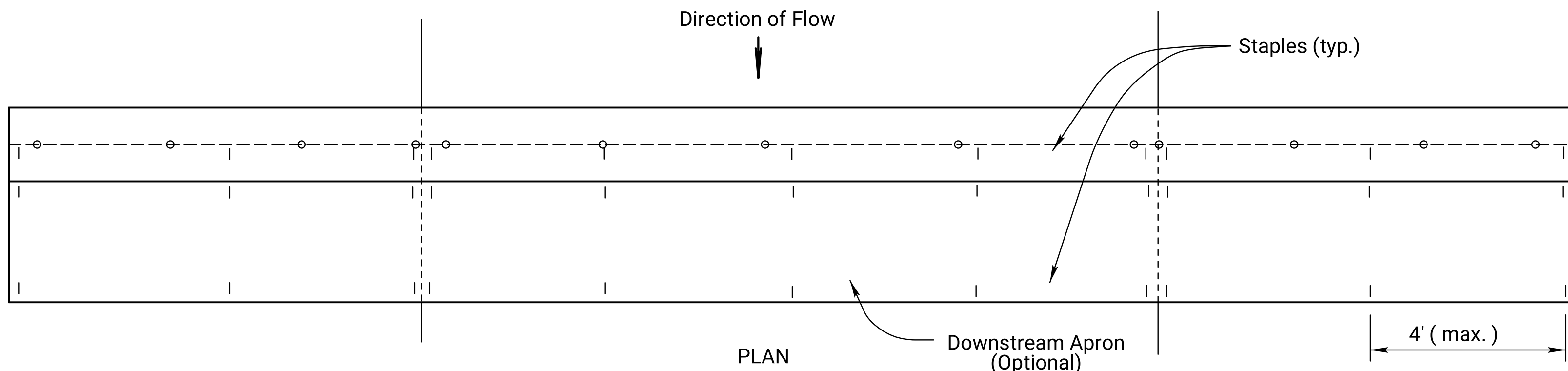
ROCK DITCH CHECK

NO SCALE

TEMPORARY ROCK DITCH CHECK SPACING	
DITCH $\phi$ SLOPE (%)	SPACING INTERVAL (FEET)
5.0	60
6.0	50
7.0	43
8.0	36
9.0	33
10.0	29
NOTE: Use this spacing for Rock Ditch Checks only.	

ROCK DITCH CHECK NOTES

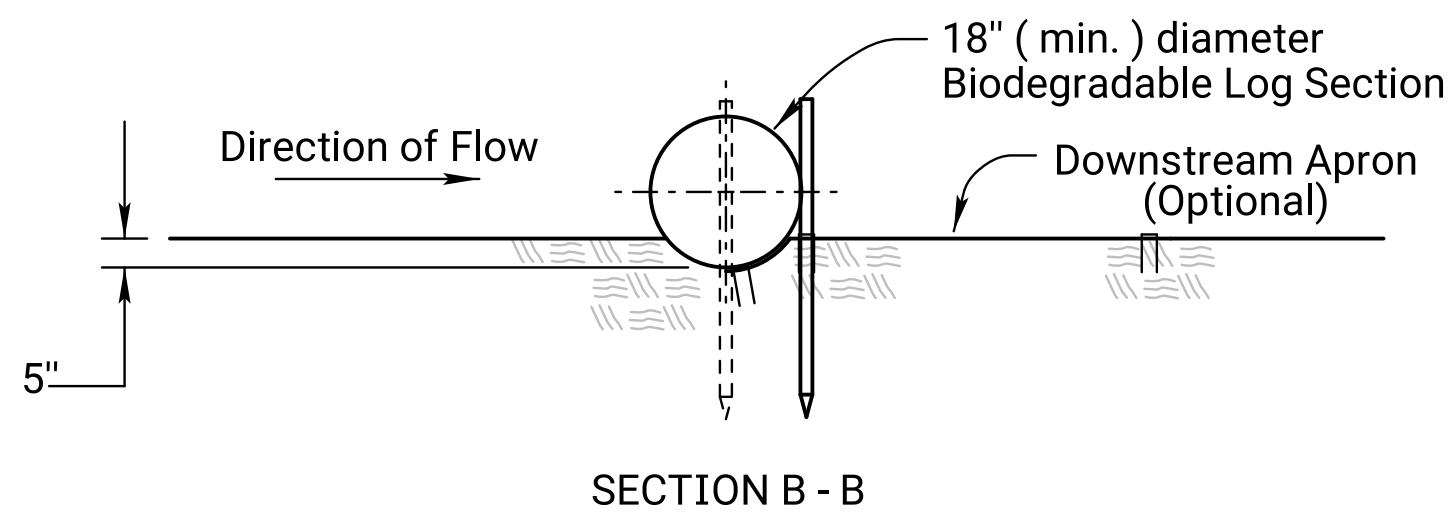
1. Rock shall be clean aggregate, D50-6" and aggregate filler.
2. Place rock in such manner that water will flow over, not around ditch check.
3. Do not use rock ditch checks in clear zone.
4. Excavation: The ditch area shall be reshaped to fill any eroded areas. Prior to placement of the rock, the ditch shall be excavated to the dimensions of the Rock Ditch Check and to a minimum depth of 6" (150mm). After placement of the rock, backfill and compact any over-excavated soil to ditch grade. This work shall be subsidiary to the bid item Temporary Ditch Check (Rock).
5. Aggregate excavated on site may be used as an alternate to the 6" rock, if approved by the Engineer.
6. The Engineer may approve the use of larger aggregates for the downstream portion of the check when conditions warrant their use.
7. When the use of larger rock is approved, D50-6" rock will be placed between the larger aggregate and the aggregate filler.
8. Aggregate filler will be placed on the upstream face of the ditch check. Aggregate filler will comply with Filter Course Type I, Division 1114.



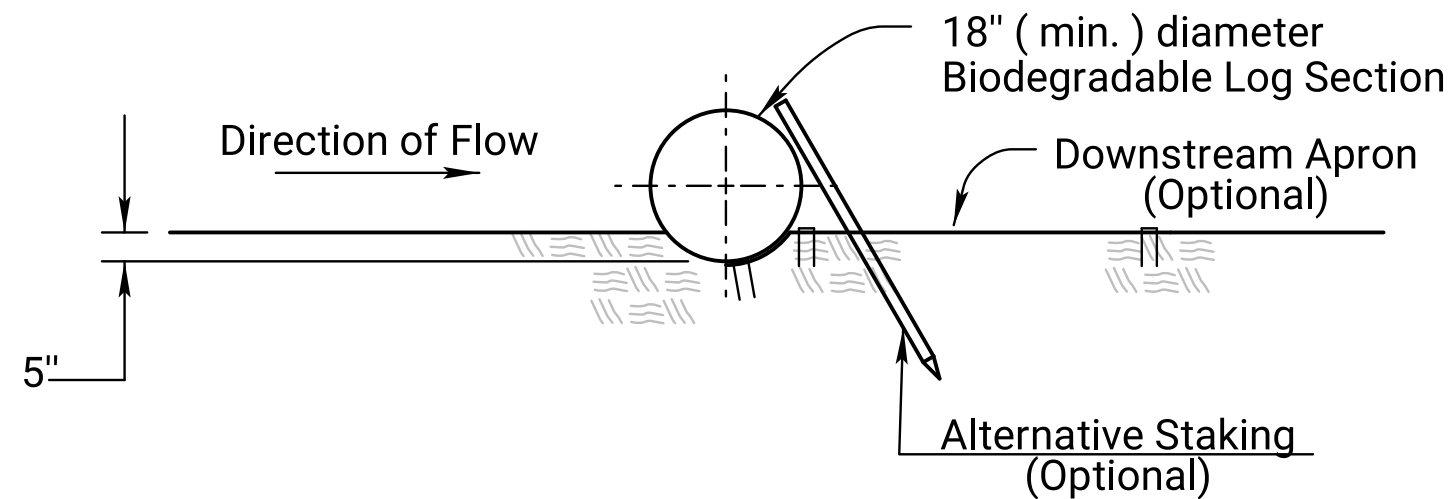
TYPICAL ELEVATION

BIODEGRADABLE LOG DITCH CHECK

OR Filter Sock Ditch Check  
NO SCALE



SECTION B - B



ALT. DETAIL  
OPTIONAL

BIODEGRADABLE LOG DITCH CHECK NOTES

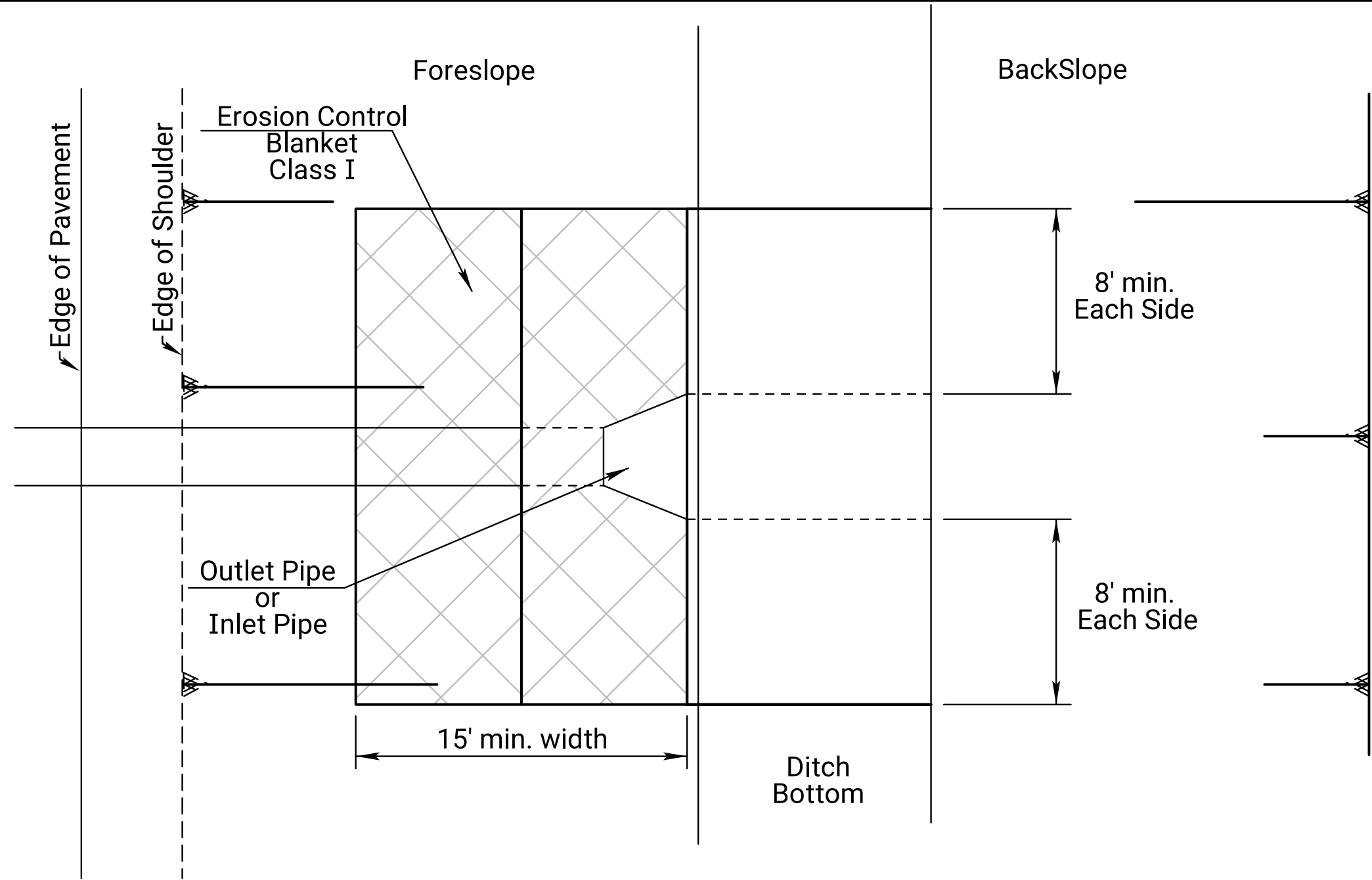
1. Use as many biodegradable log sections as necessary to ensure water does not flow around end of ditch check.
2. Overlap sections a minimum of 18".
3. Stakes shall be wood or steel according to Section 2114 of the Standard Specifications. Length of stakes shall be a minimum of 2 x the diameter of the log.
4. Use Erosion Control (Class 1) (Type C) as the downstream apron when required.
5. A downstream apron is required when directed by the Engineer. Apron material will be paid at the contract unit price.
6. Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.

03	11-19-20	Revised Standard	M.R.D.	M.L.
02	08-10-16	Revised Standard	R.A.A.	S.H.S.
01	10-21-15	Revised Standard	R.A.A.	S.H.S.

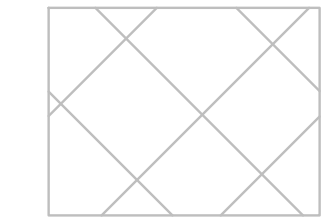
KANSAS DEPARTMENT OF TRANSPORTATION						
TEMPORARY EROSION AND POLLUTION CONTROL						
ROCK DITCH CHECKS						
BIODEGRADABLE LOG DITCH CHECKS						
LA852G						
FHWA APPROVAL		11-19-20		APP'D.		Mervin Lare
DESIGNED	M.L.	DETAILED	D.K.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	M.L.	DETAIL CK.	M.L.	QUAN.CK.	TRACE CK.	R.A.A.



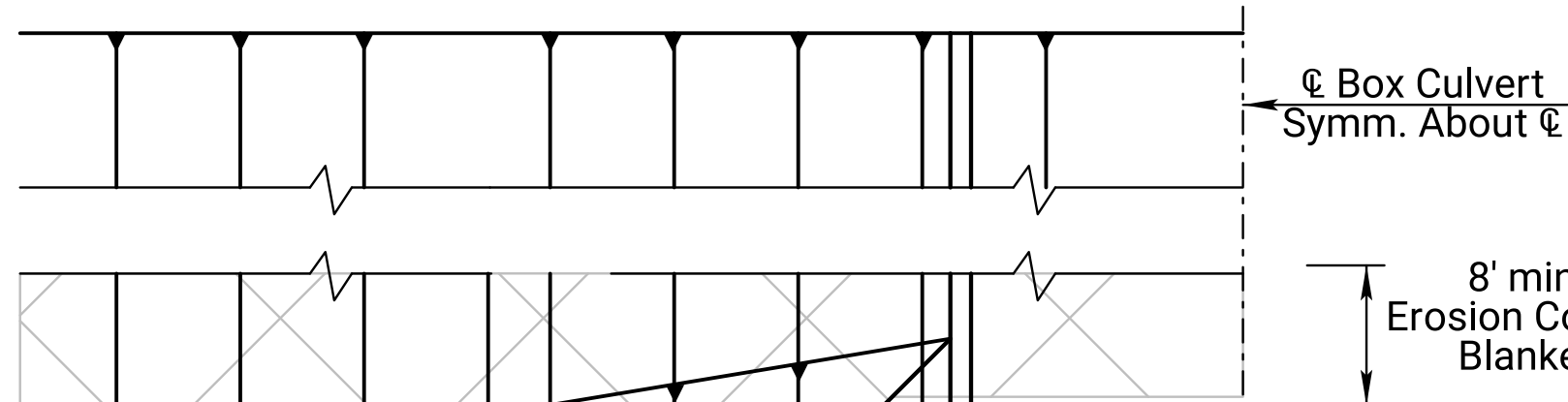
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	78	141



PARTIAL PLAN PIPE



Limits of Erosion Control Blanket



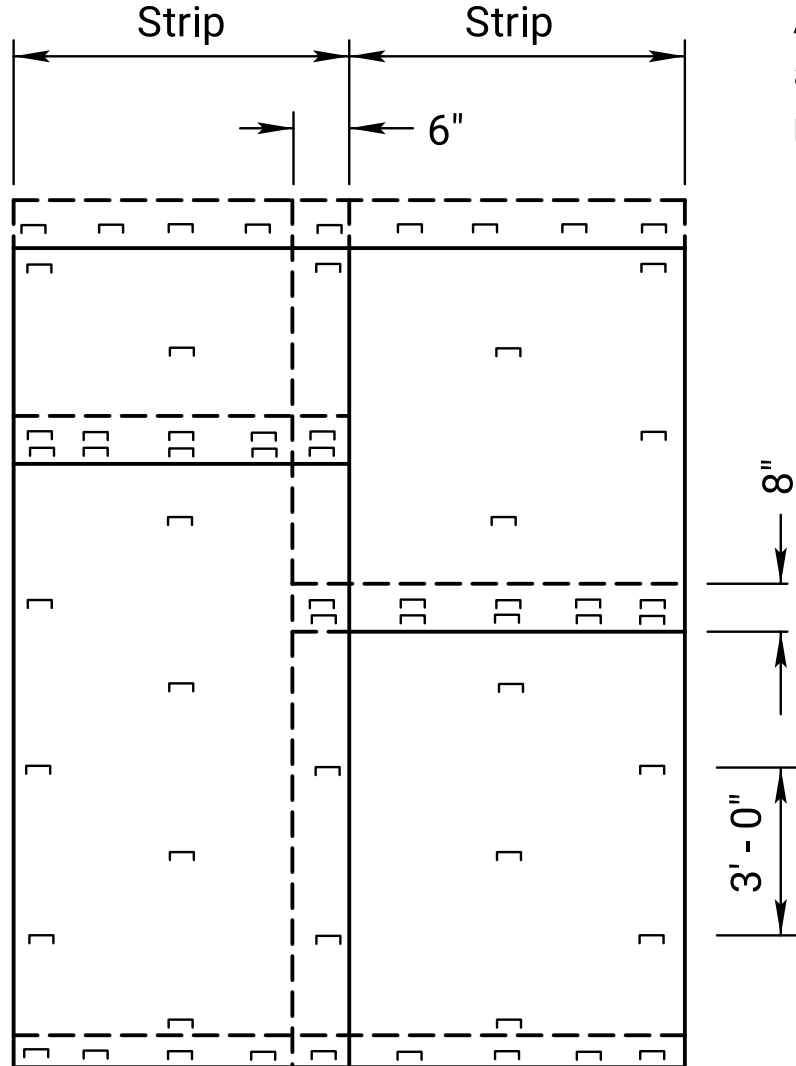
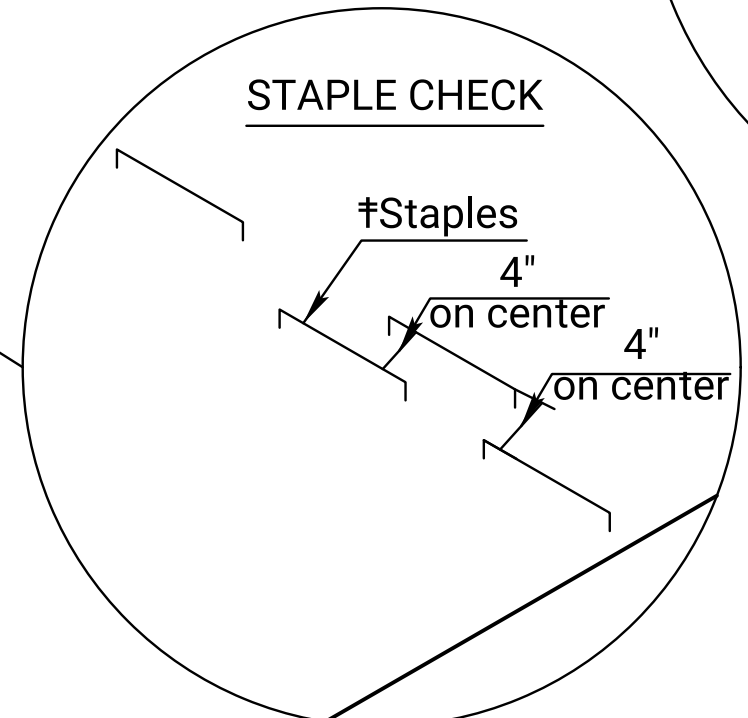
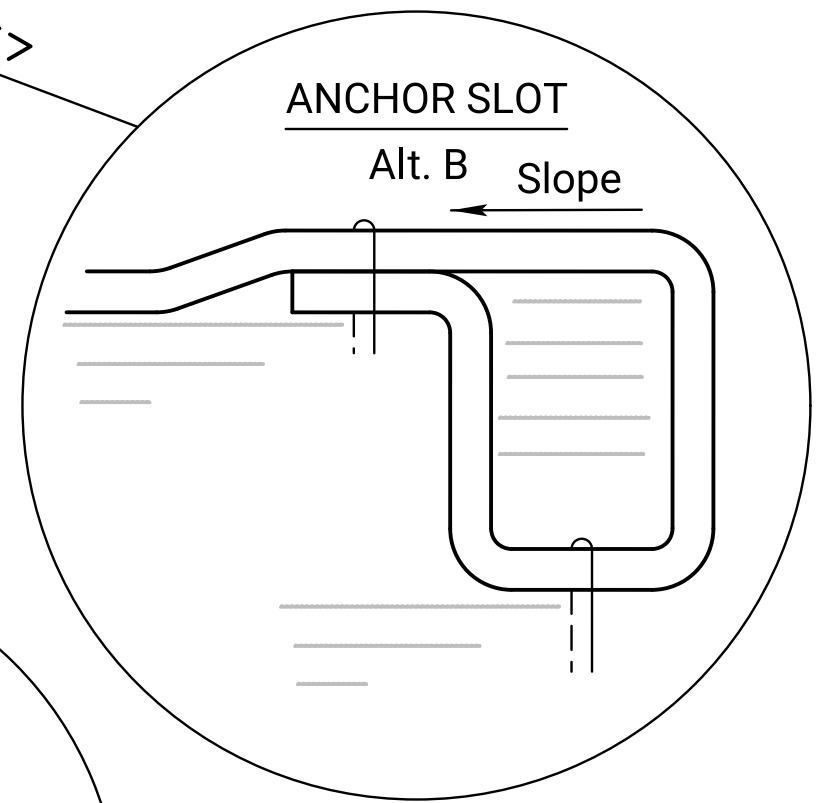
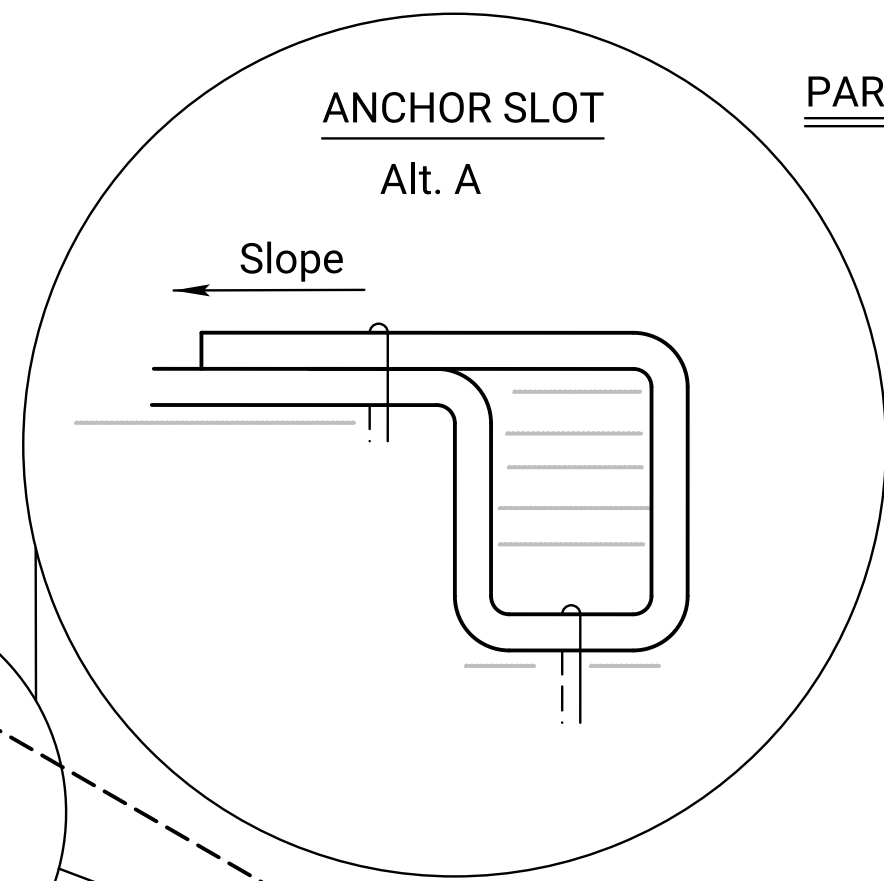
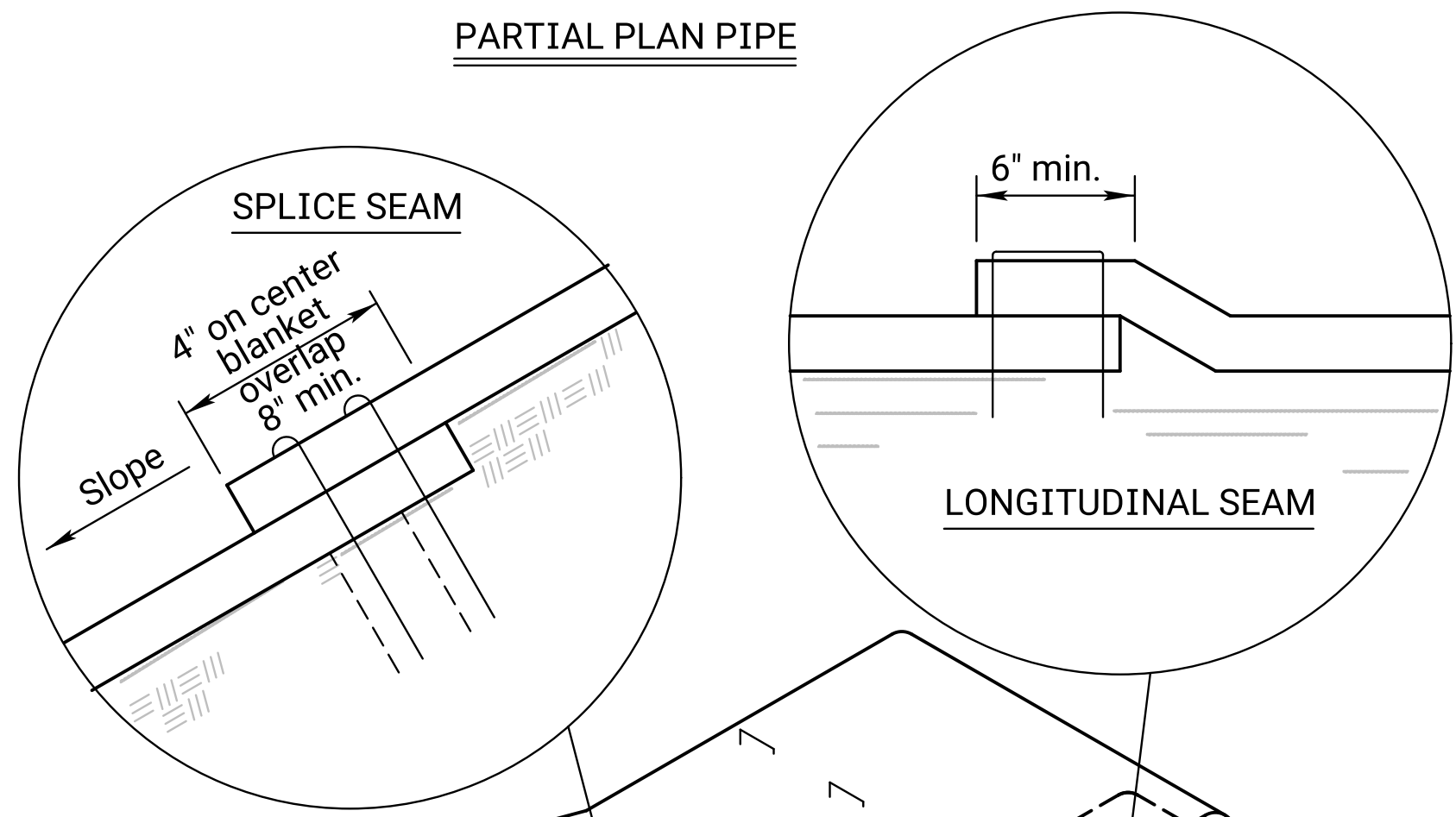
PARTIAL PLAN BOX CULVERT

INSTALLATION DETAILS FOR EROSION CONTROL CLASS 1

Erosion Control Blankets shall be laid loosely in the direction of the slope, beginning at the bottom of the slope. In order for blanket to be in contact with the soil, lay blanket loosely, avoiding stretching.

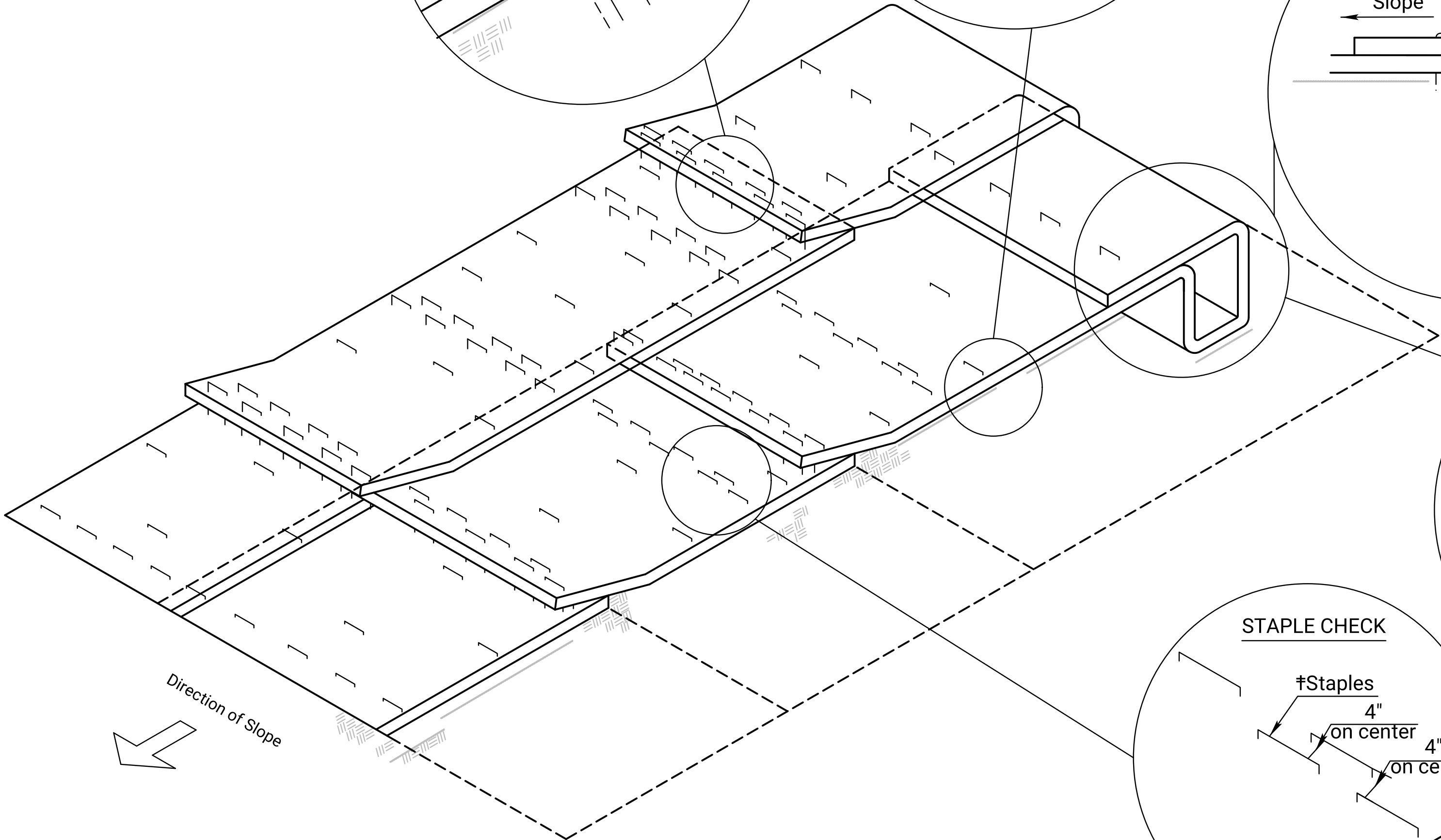
- ANCHOR SLOTS:** The top of the blanket should be "slotted in" at the top of the slope and anchored in place with anchors 6 inches apart. The slots should be 6 inches wide x 6 inches deep with the blanket anchored in the bottom of the slot, then backfilled, tamped and seeded.
- LONGITUDINAL SEAMS:** The edges of the blanket should overlap each other a minimum of 6 inches, with anchors catching the edges of both blankets.
- SPLICE SEAM:** When splices are necessary, overlap end a minimum of 8 inches in direction of water flow. Stagger splice seams.
- TERMINAL FOLD:** The bottom edge of the blanket shall be turned under a minimum of 4 inches, then anchored in place with anchors 9 inches apart.
- TYPICAL ANCHORS:** Anchor design shall be as recommended by the manufacturer.
- STAPLE CHECK:** Establish Staples in 2 rows 4" on center apart. Staple Checks - shall be 30' apart.

● Erosion Control Class I may be omitted if the area is immediately covered by permanent slope protection (where directed by the plans).



PLAN VIEW - ANCHORING DIAGRAM

**NOTE:**  
Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.  
Single post ring and shank staple is acceptable.



ISOMETRIC VIEW

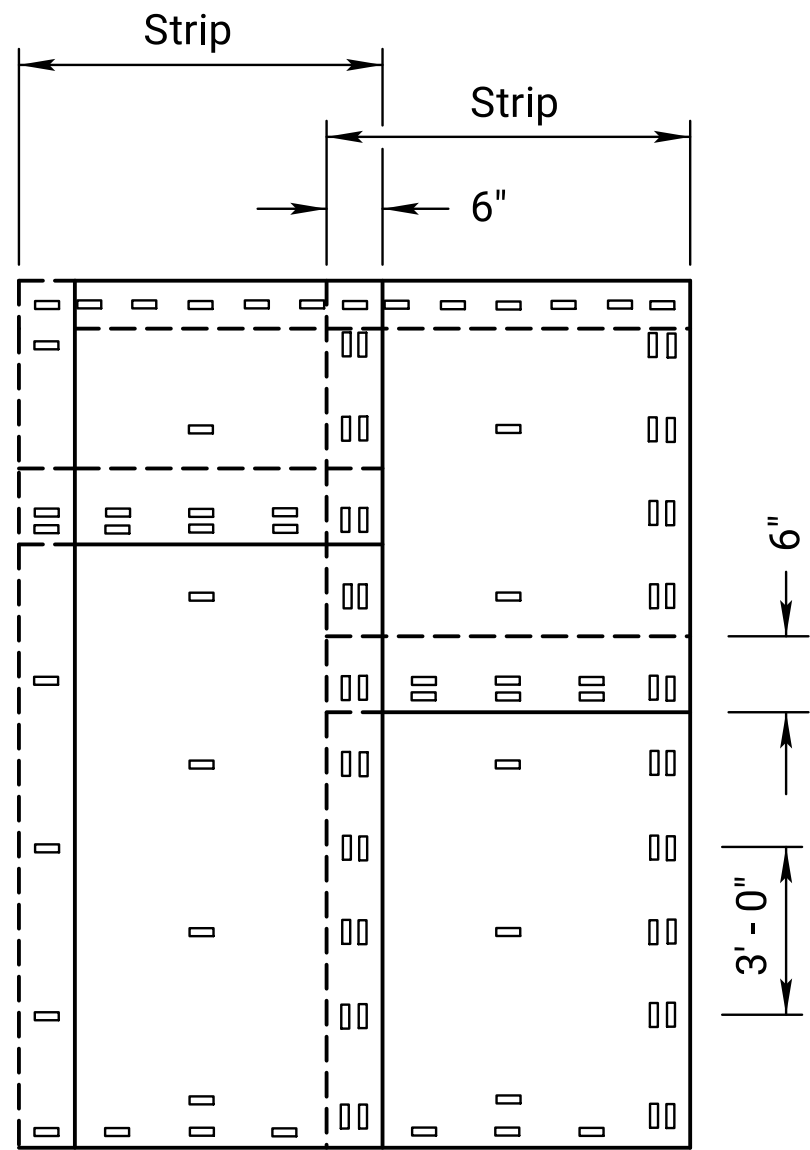
KANSAS DEPARTMENT OF TRANSPORTATION					
INSTALLATION DETAIL EROSION CONTROL CLASS 1 SLOPE PROTECTION					
LA855					
FHWA APPROVAL		03-10-15		APPD.	
DESIGNED	R.A.A.	DETAILED	R.A.A.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	R.A.A.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	79	141

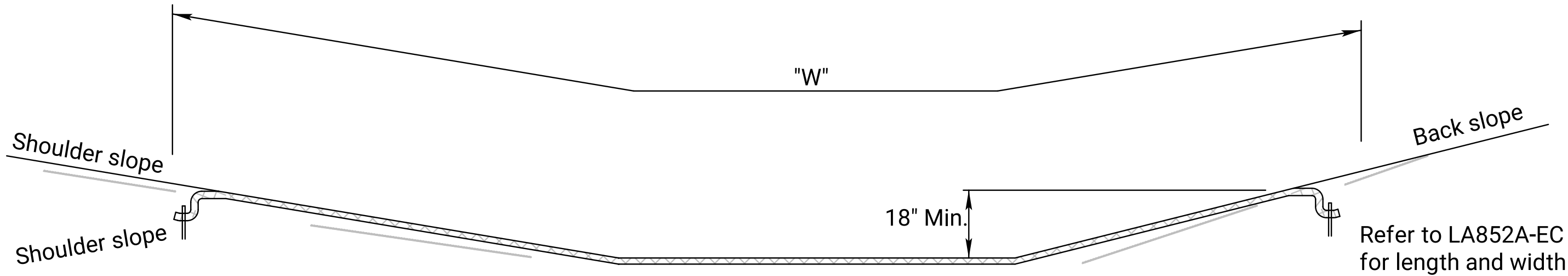
INSTALLATION DETAILS FOR EROSION CONTROL CLASS 2

Erosion Control Mats shall be laid loosely in the direction of the flow, with the first course at the centerline of channel, where applicable. In order for the mat to be in contact with the soil, lay the mat loosely, avoiding stretching.

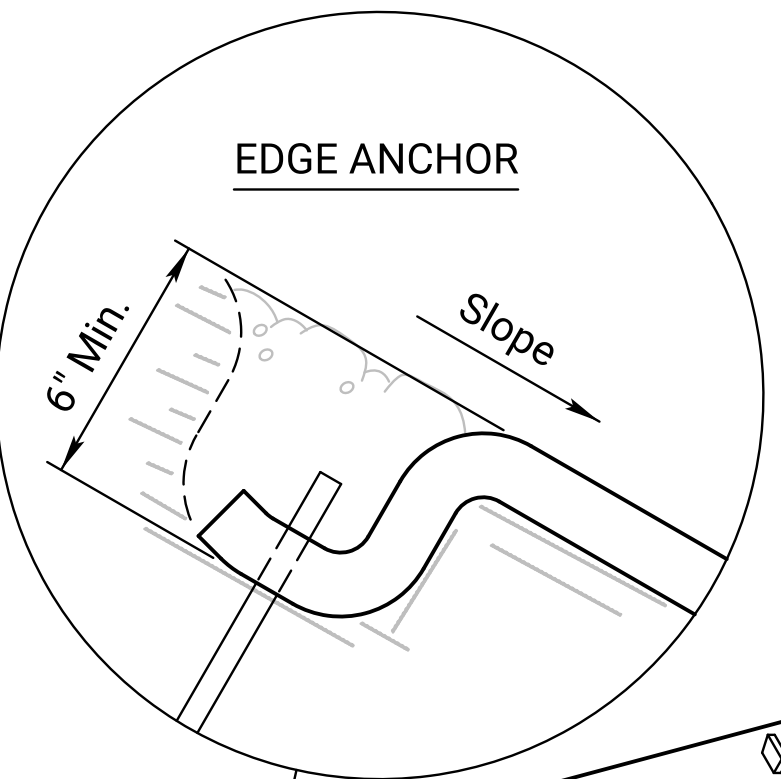
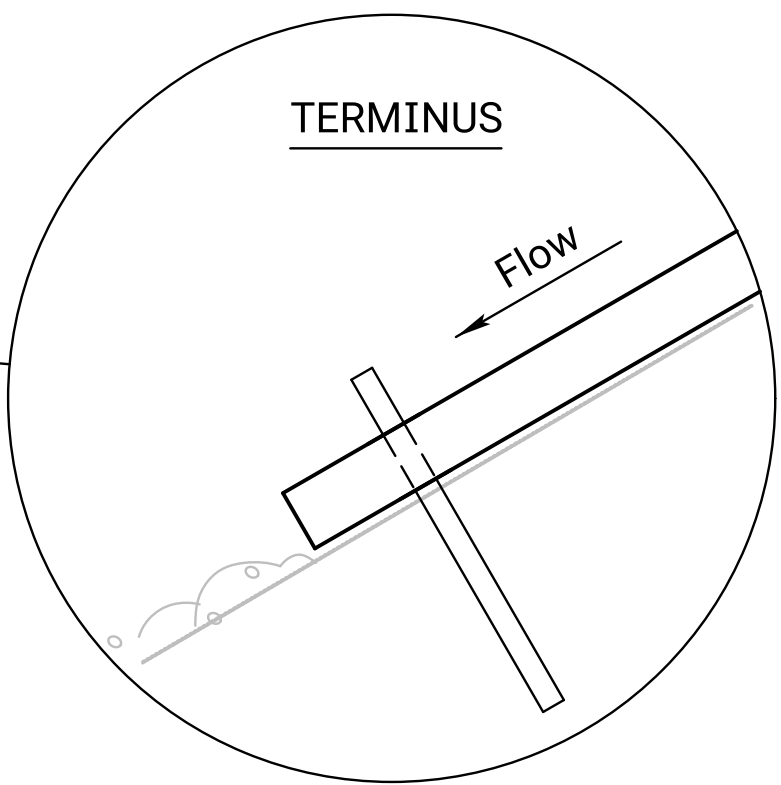
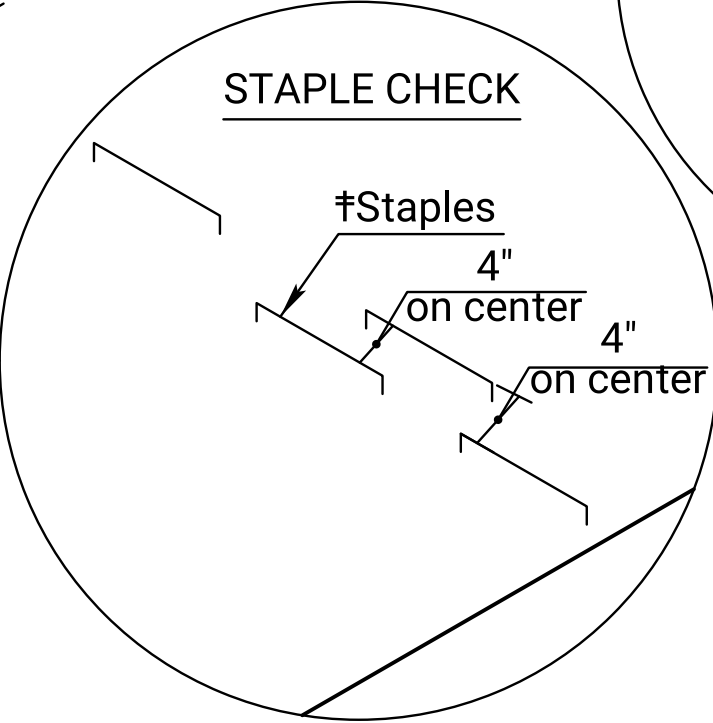
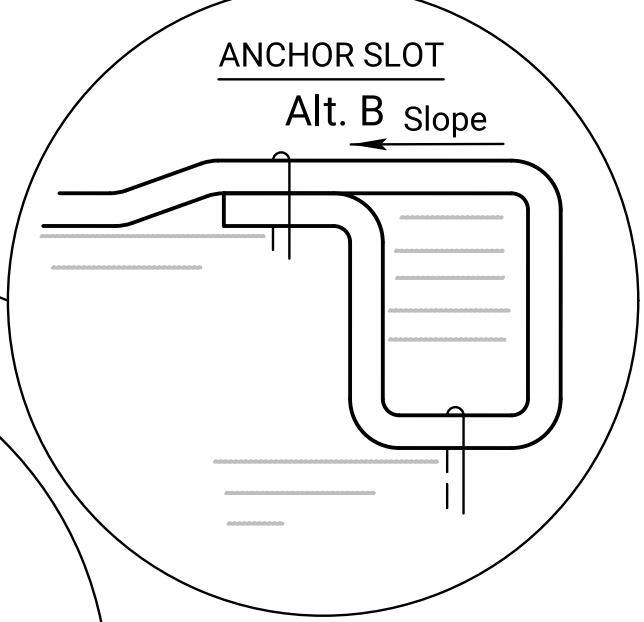
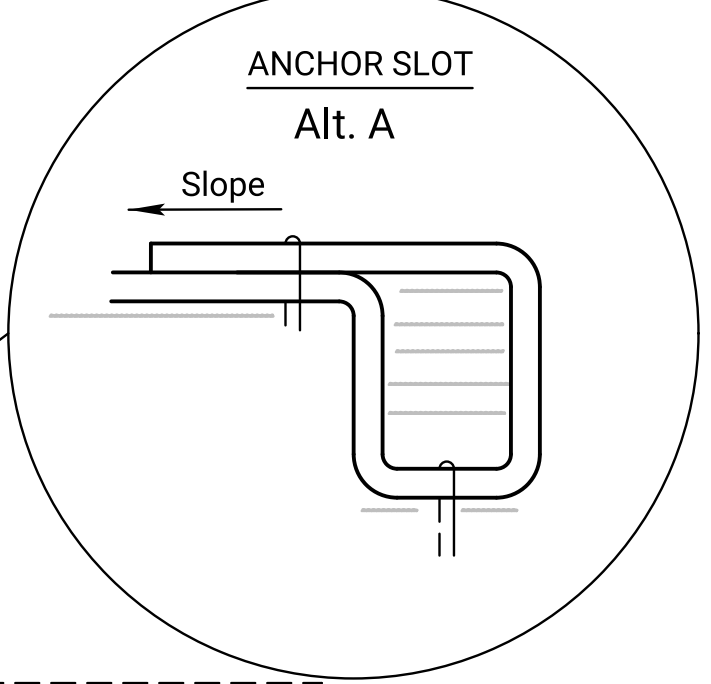
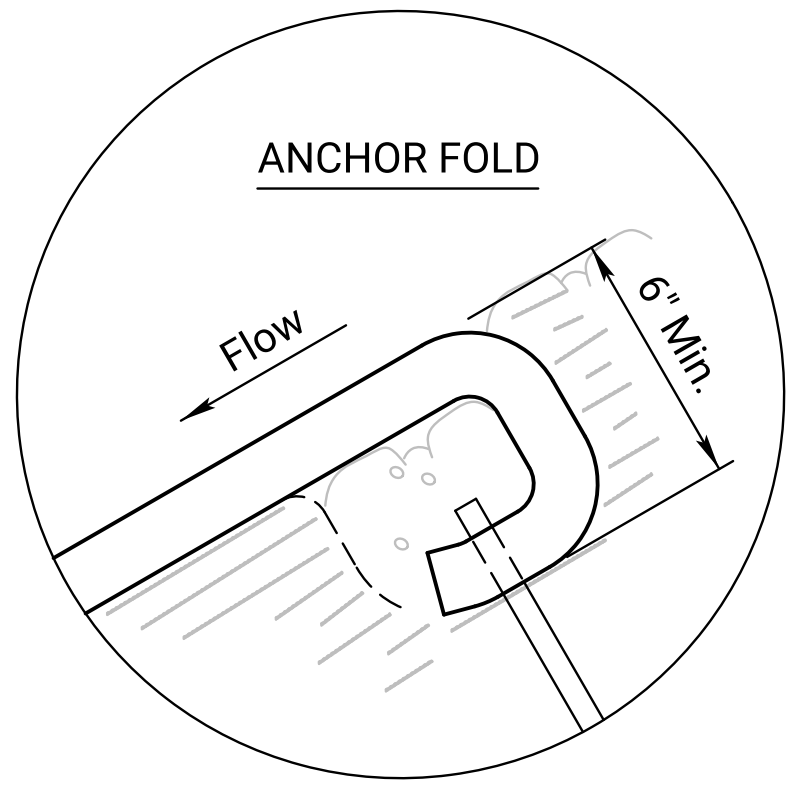
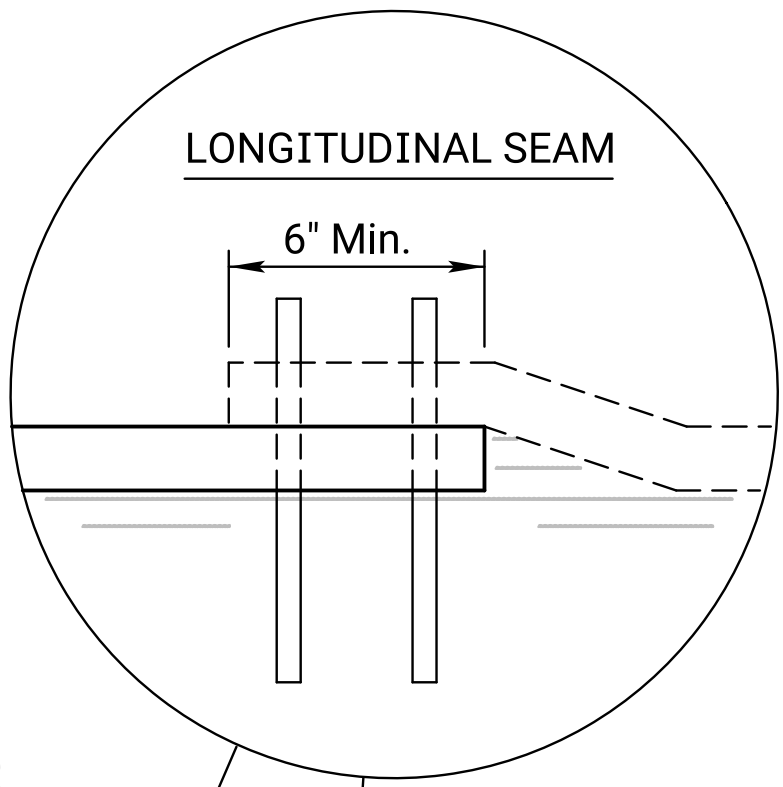
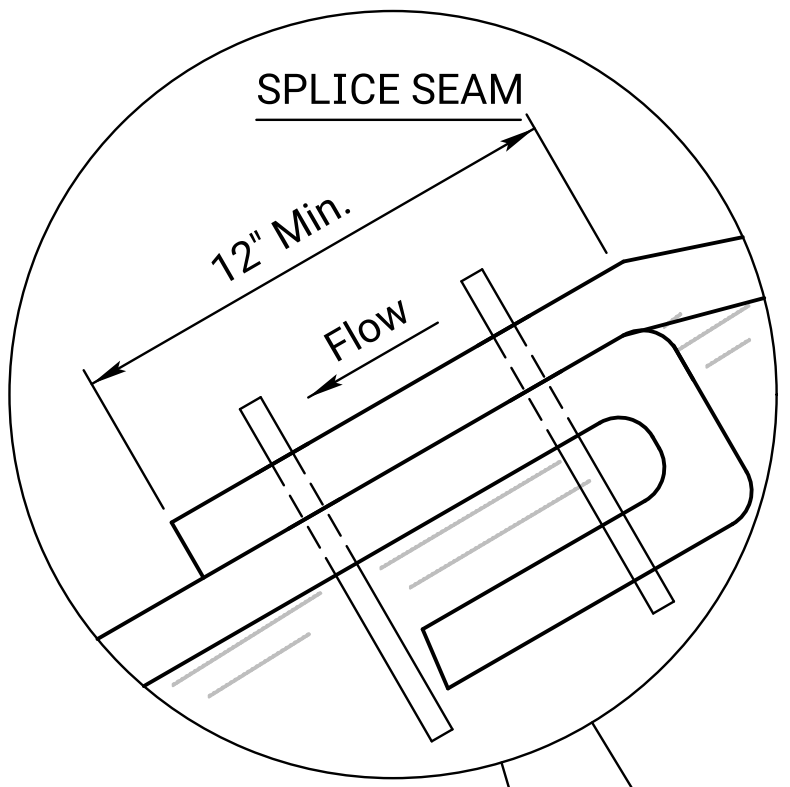
- ANCHOR FOLD:** The top of the mat should be folded under, buried and secured with approved anchors placed 6 inches apart. The top edge of the mat should be buried in a slot, 6 inches wide x 6 inches deep; anchored in the bottom of the slot, backfilled, and the mat folded over the top as shown in detail.
- LONGITUDINAL SEAMS:** The adjacent edges of the mat should overlap a minimum of 6 inches, with anchors catching the edges of both mats.
- SPLICE SEAM:** When splices are necessary, overlap end a minimum of 12 inches in direction of water flow. Stagger splice seams.
- STAPLE CHECK:** †Establish Staples in 2 rows 4" on center apart. Staple Checks - shall be 30' apart.
- EDGE ANCHOR:** Lay outside edge of mat into trench at top of side slope. Anchor at 3 foot intervals along trench.
- TERMINUS:** The bottom edge of the mat shall be anchored in place with anchors spaced at 9 inch intervals along the terminating edge.
- TYPICAL ANCHORS:** Anchor design shall be as recommended by the manufacturer.



PLAN VIEW - ANCHORING DIAGRAM



CROSS SECTION (Ditch Lining)



Direction of Water Flow

ISOMETRIC VIEW

04	09-25-15	Modified Staple Check	R.A.A.	S.H.S.
03	09-15-14	Revised Standard	R.A.A.	S.H.S.
02	03-01-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
INSTALLATION DETAIL EROSION CONTROL CLASS 2 FLEXIBLE CHANNEL LINER				
LA856				
FHWA APPROVAL		11-02-15	APPD.	Scott H. Shields
DESIGNED	R.A.A.	DETAILED	R.A.A.	QUANTITIES
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.
DESIGNED		TRACED	R.A.A.	
DESIGN CK.		TRACE CK.		





Plotted by : austin.loyd@ks.gov  
File : KA572901pss402-01.dgn

16-JUL-2024 20:37

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	81	141

## SYMBOL KEY

	REMOVE SIGN
	REMOVE POST
	REMOVE FOOTING
	REMOVE SIGN & POST
	REMOVE POST & FOOTING
	REMOVE SIGN, POST, & FOOTING
	MOUNT ON WOOD POST IN CONCRETE FOOTING
	MOUNT ON WOOD POST IN SOIL
	MOUNT ON STEEL BEAM BREAKAWAY POST
	MOUNT ON STEEL U-POST
	MOUNT ON PSST POST
	MOUNT ON EXISTING POST
	MOUNT ON VERTICAL SUPPORT
	SHOULDER MOUNTED INSTALLATION
	OFFSET MOUNTED INSTALLATION
	EXISTING SIGN
	EXISTING SIGN TO BE OVERLAID
	SIGN IS NOT PART OF PROJECT
	TYPE 'A' DELINEATOR (RIGID)
	TYPE 'A' DELINEATOR (RIGID) (BK-BK)
	TYPE 'B' DELINEATOR (RIGID)
	TYPE 'A' DELINEATOR (FLEXIBLE)
	TYPE 'A' DELINEATOR (FLEXIBLE) (BK-BK)
	TYPE 'B' DELINEATOR (FLEXIBLE)
	TYPE 2 OBJECT MARKER
	TYPE 3 OBJECT MARKER
	TYPE 3 OBJECT MARKER (BK-BK)

## GENERAL NOTES

In order to expedite the completion of the project for traffic service, the signing and delineator work shall be sequenced with any other contract work such that the phases of construction may proceed and be completed at the same time.

New signs erected on the project which are in conflict with existing signing are to be completely covered until the existing signs are removed or the new signing is applicable. The existing signs that are being replaced, removed, or do not follow the current MUTCD signing standards are to be removed when the project is completed or as determined by the Engineer.

The Contractor shall exercise caution at all times when installing sign supports in and around areas where utilities exist, either underground or overhead, and will be held responsible for any damage incurred to the system. The installation of sign supports shall include the excavation, drilling, or driving the support footing and the erection of the sign support. The contractor shall exercise caution when working around any existing signs that are to remain and will be held responsible for any damage to the signs, supports, or footings. The Contractor shall exercise care when working around shrubbery while removing or installing signs or sign supports.

An existing sign post installation shall be plumb and the compaction of the backfill soil shall comply with the specifications after the removal and resetting of a sign, the removal and replacement of a sign, or the installation of a new sign.

The Contractor shall provide mounting bolts that are of a length that does not extend more than a nominal 1 inch beyond the sign post. The Contractor shall not make any field modifications to the mounting bolt prior to or after the sign is installed.

Specific service (LOGO) signs that are to be removed shall have the business logo plaques removed and transported to location determined by KDOT, at which time the plaques become the property of KDOT. The Contractor will be assessed a replacement cost for any damage to a business logo plaque prior to the plaque becoming the property of KDOT.

The materials and fabrication for signing and delineation work shall conform to the Standard Specifications for State Road and Bridge Construction (2015 edition) and Special Provisions.

## INDEX OF SHEETS

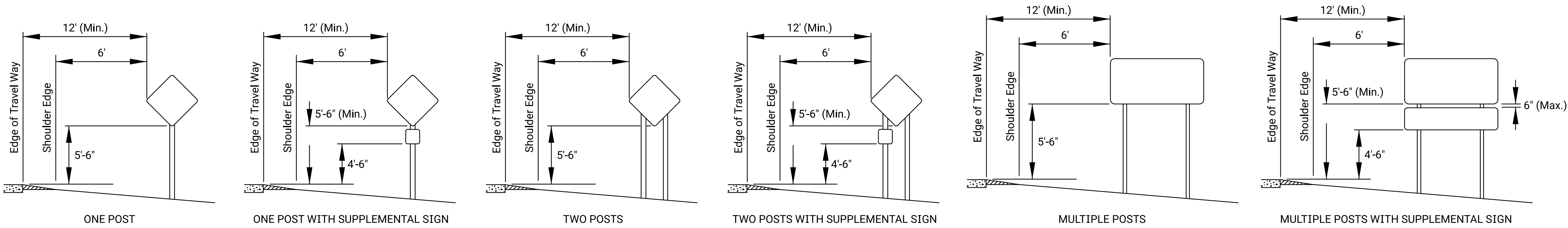
81	SIGNING INDEX, SYMBOLS, & GENERAL NOTES
82	HEIGHT & LATERAL DISTANCE FOR ERECTION
83-85	POSITIONING, DESIGN, & MOUNTING FOR OBJECT MARKERS (TYPE 2 & 3)
86-87	PLAN SHEETS (INSTALLATIONS)
88-89	PLAN SHEETS (REMOVALS)
90	QUANTITIES SHEETS (INSTALLATIONS)
91	QUANTITIES SHEET (DELINEATORS & OBJECT MARKERS)
92	SUMMARY SHEET (INSTALLATIONS & REMOVALS)
93	RECAPITULATION SHEET
94-95	STANDARD STRUCTURAL SIGN SUPPORTS (WOOD & STEEL POSTS)
96	MOUNTING OF SIGNS ON WOOD POSTS
97-99	DETAILS FOR FLAT SHEET SIGN BLANKS
100	DETAILS FOR GUIDE SIGNS
101	DETAILED SIGN SPECIFICATIONS

02	10-01-19	Changed symbol, notes, & index		D.D.G.	E.W.N.
01	07-23-10	Changed General Notes and Spec Book Date		D.D.G.	D.B.
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
SIGNING SYMBOL KEY GENERAL NOTES AND INDEX					
TE402				07-01-03	
FHWA APPROVAL		10-01-19	APP'D.	Steven A. Buckley	
DESIGNED	D.D.G.	DETAILED	W.S.B.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.

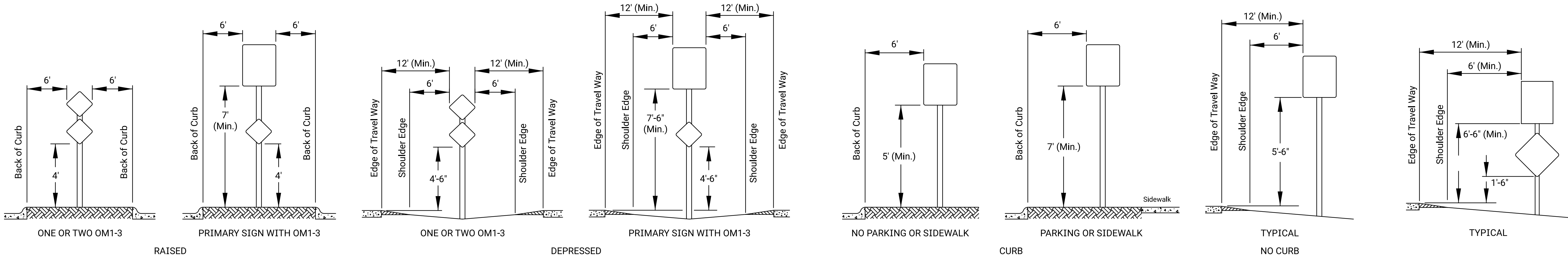
Plotted by : austin.loyd@ks.gov  
File : KA572901pss407-01.dgn

16-JUL-2024 20:36

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	82	141



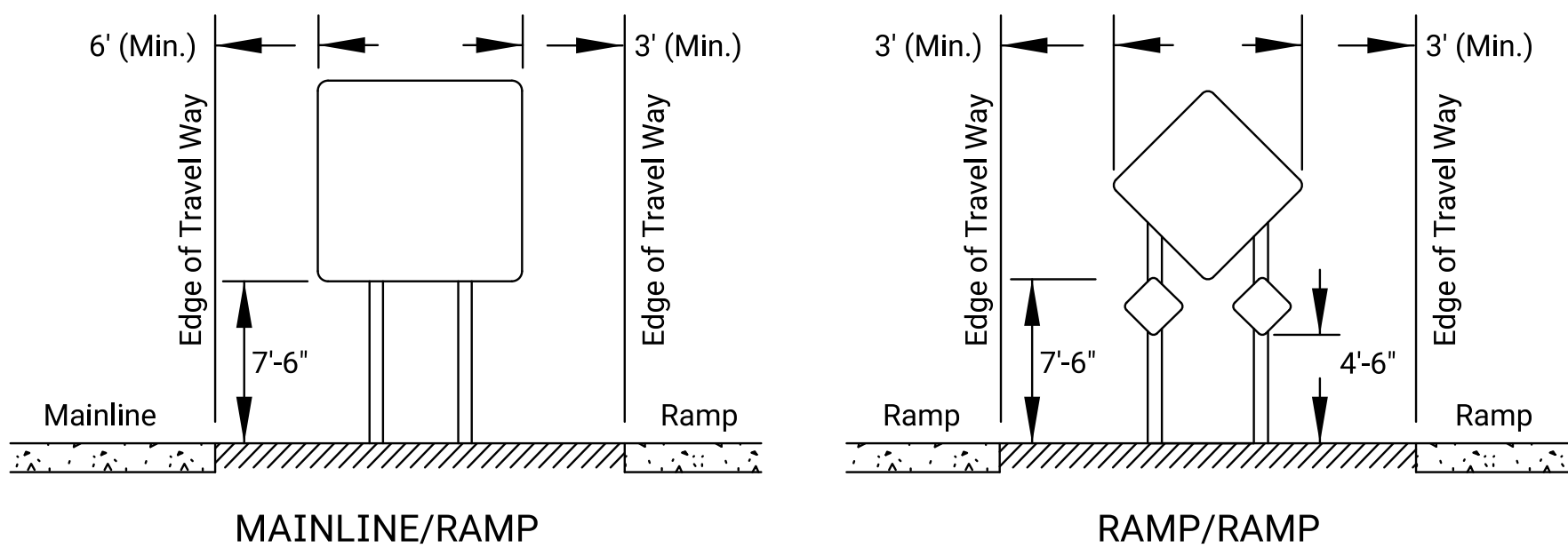
### CONVENTIONAL HIGHWAY AND SIDE ROADS



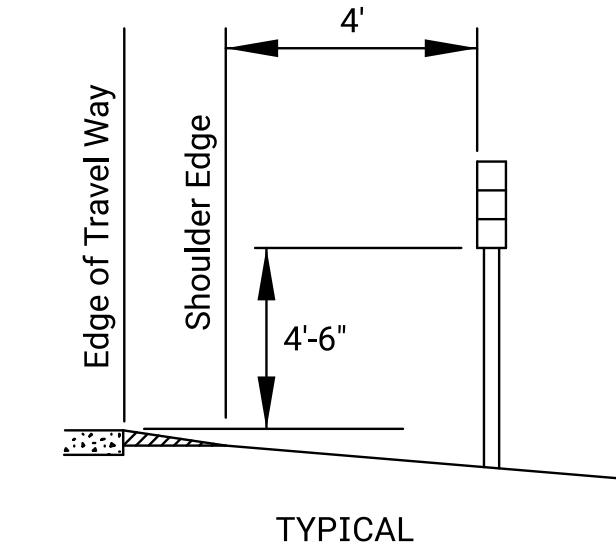
### MEDIANS AND ISLANDS

### URBAN ROADWAYS

### ADOPT A HIGHWAY

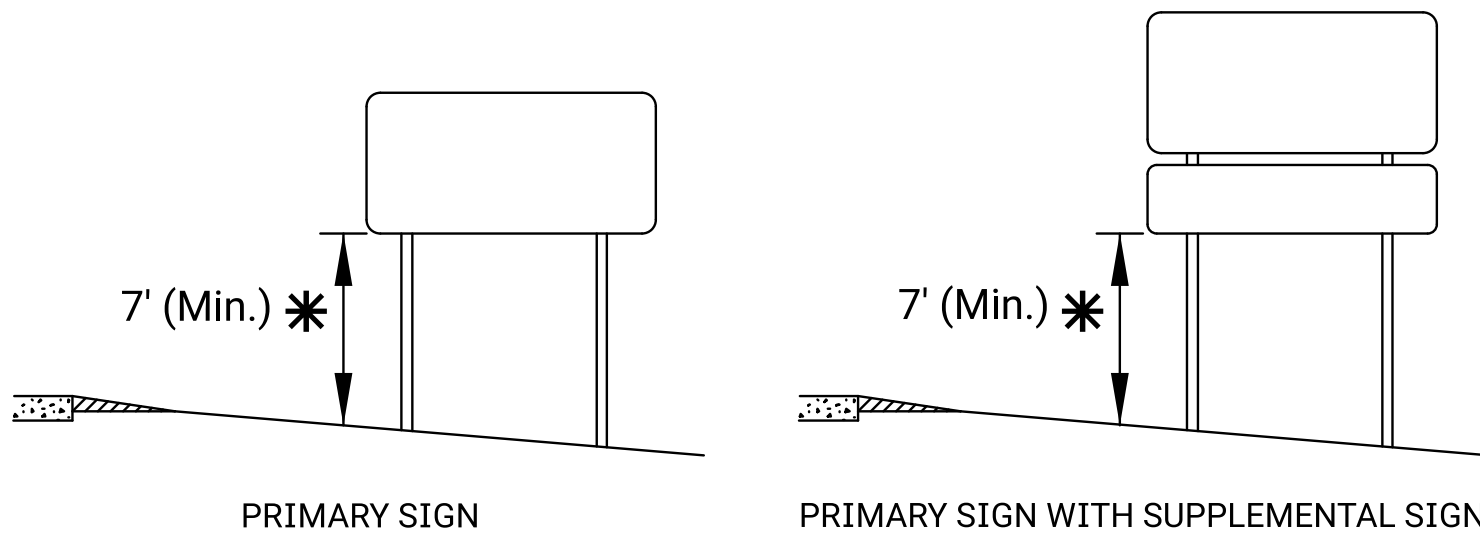


### HIGHWAY GORES



Reference Marker Post: 2 Lb/Ft "U" Post

### REFERENCE MARKERS



### GROUND CLEARANCE FOR STEEL BEAM POSTS

#### NOTES

The "Edge of Travel Way" is the edge line or the edge of the driving lane.

The outer edge of the sign shall not extend beyond the right of way line.

A minimum lateral clearance of 6' from pavement edge may be used where lateral offsets are limited.

In business, commercial, or residential districts where with limited lateral offsets, a minimum lateral clearance of 2' with a 7'-6" minimum mounting height may be used.

When signs are behind guardrail, the near edge of the sign shall not extend beyond the back side of the guardrail and the nearest sign post shall be a minimum of 5' from the face of the guardrail. Shoulder mounted signs shall not be located between 100' in advance of and 50' beyond the nose of the guardrail.

When the median or island is too narrow for the typical lateral placement, the sign may be placed a minimum of 2' from the back of the curb. In no case shall the sign edge extend beyond the back edge of the curb.

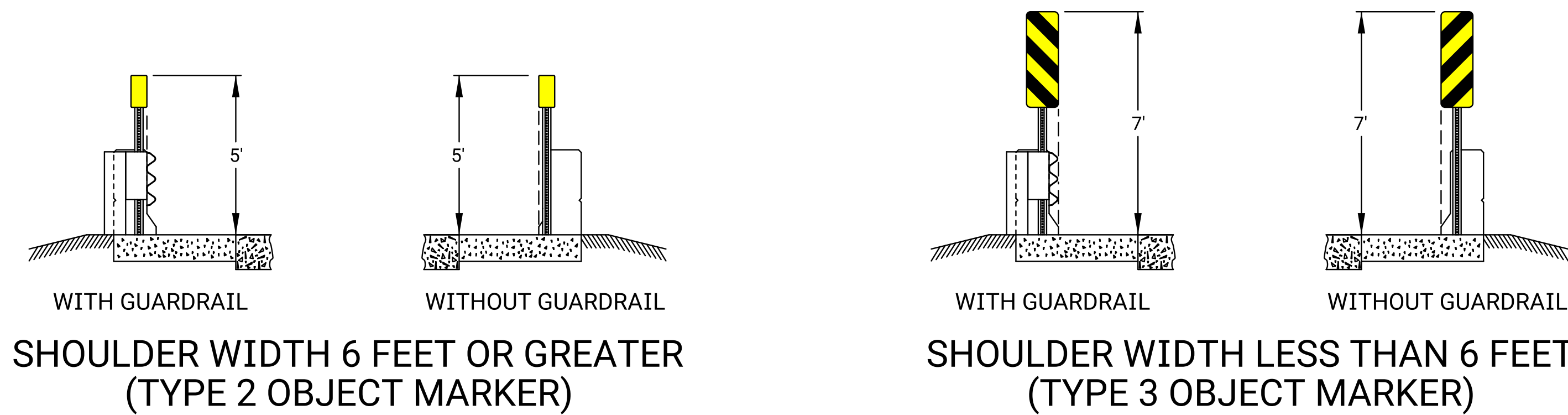
The gore sign shall be installed in the paved gore area. The edges of the gore sign shall not extend beyond the shoulder edge. The minimum distance from the centerline of the posts to the back of the paved gore area is 2'.

Signs may be moved laterally or longitudinally if it will improve visibility of the sign or other signs or if it will protect the sign more. The maximum allowable longitudinal adjustment is 100', with the exception of the reference marker which is 50'.

The minimum spacing between signs, excluding reference markers is 100'.

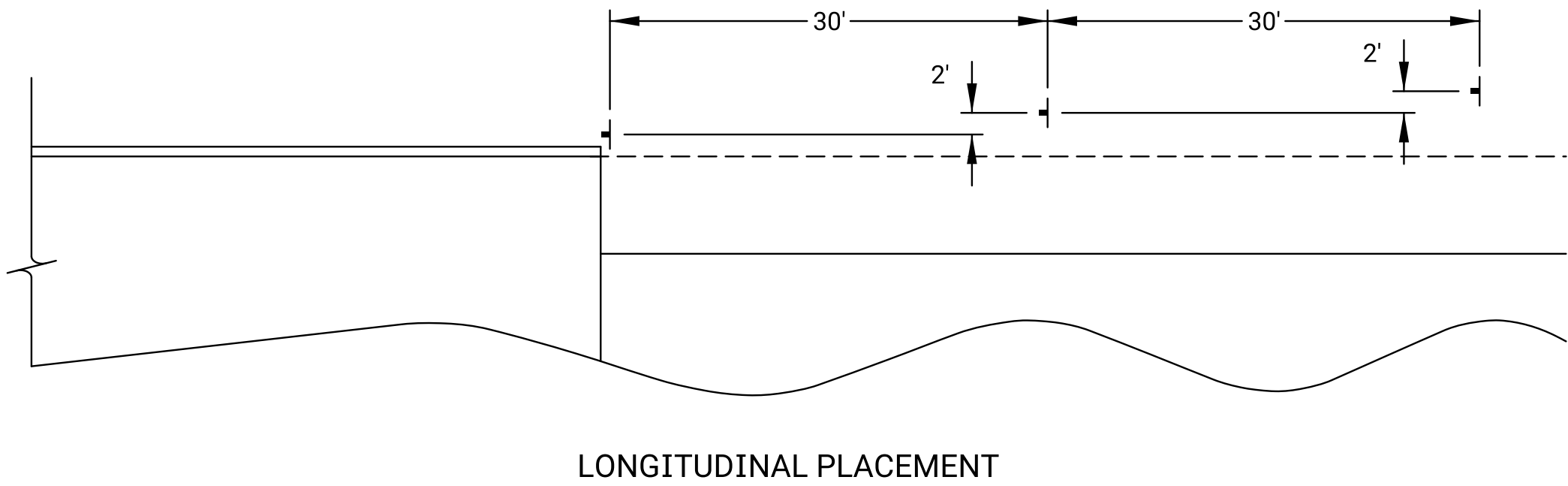
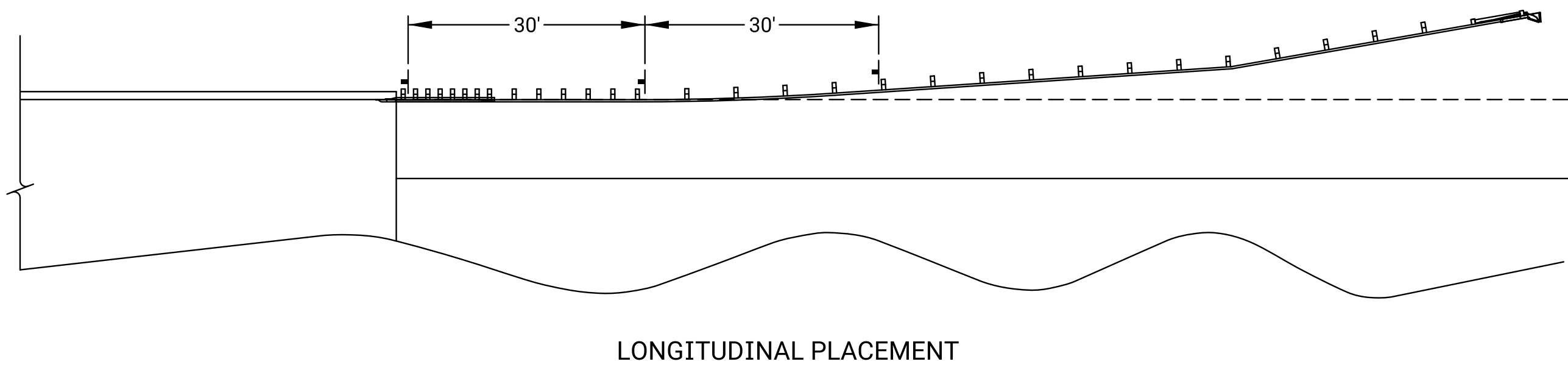
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
MOUNTING HEIGHT & LATERAL OFFSET					
FOR CONVENTIONAL HIGHWAYS,					
SIDE ROADS, MEDIANS, ISLANDS,					
GORES, AND URBAN ROADWAYS					
TE407				10-01-19	
FHWA APPROVAL		10-01-19		APP'D: Eric W. Nichol	
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES	TRACED
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN.CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	83	141

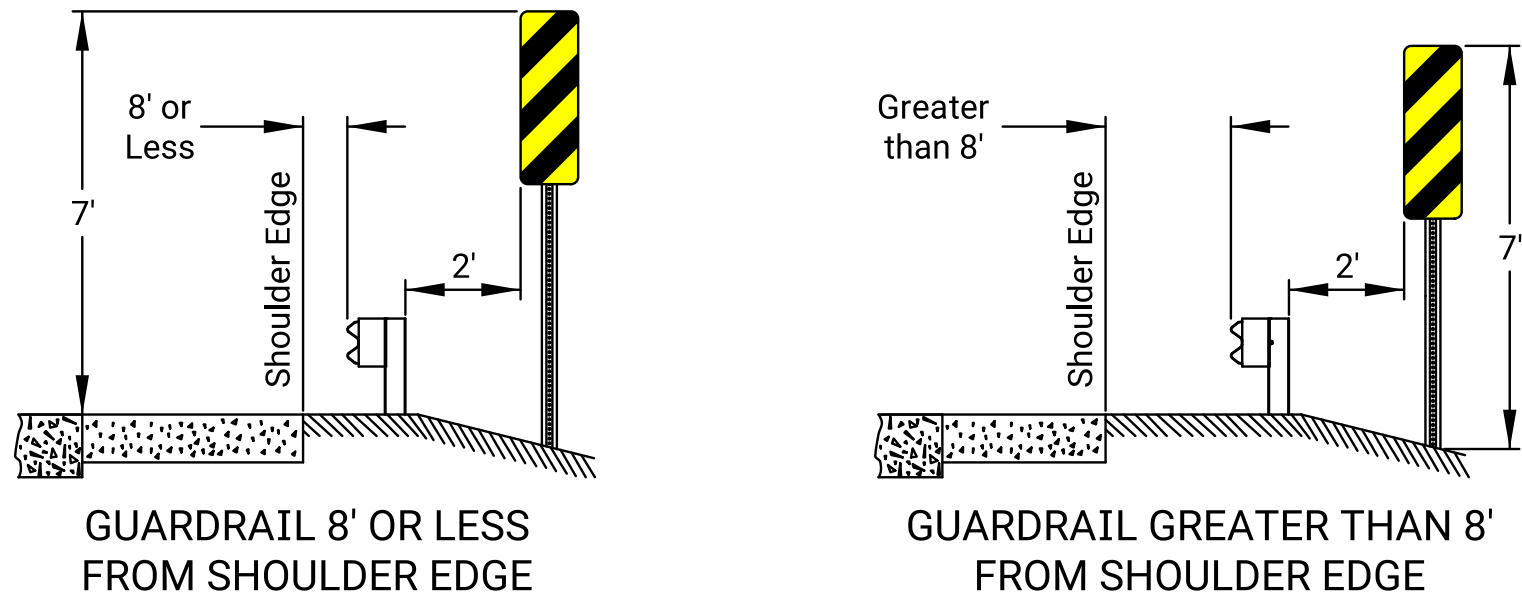


NOTE:  
The longitudinal location of the object markers from the structure end shall be a maximum spacing of 42".

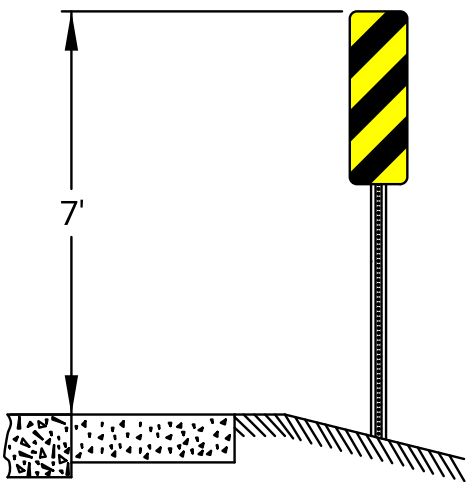
END OF STRUCTURE



NOTE:  
The lateral offset is measured from the centerline of the object markers.



STRUCTURE APPROACH  
GUARDRAIL WITHOUT MARKERS



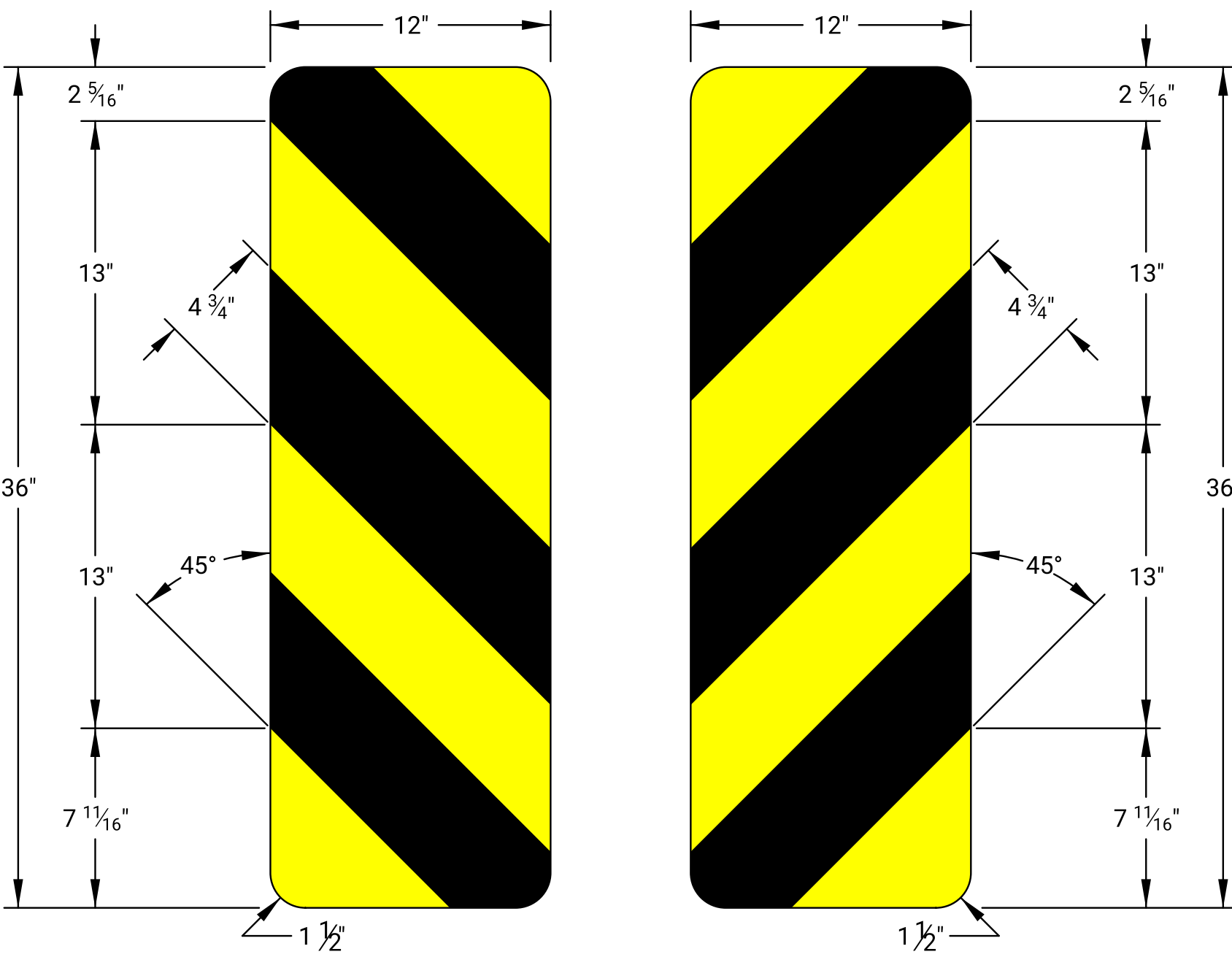
STRUCTURE APPROACH  
WITHOUT GUARDRAIL

NO.	DATE	REVISIONS	BY	APPD.	
KANSAS DEPARTMENT OF TRANSPORTATION					
DESIGN DETAILS FOR OBJECT MARKERS (TYPE 2 & 3) FOR STRUCTURES WITH PARAPETS					
TE415				10-01-19	
FHWA APPROVAL		10-01-19		APPD.	
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES	TRACED
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN. CK.	TRACE CK.



Plotted by : austin.loyd@ks.gov  
File : KA572901pss416-01.dgn

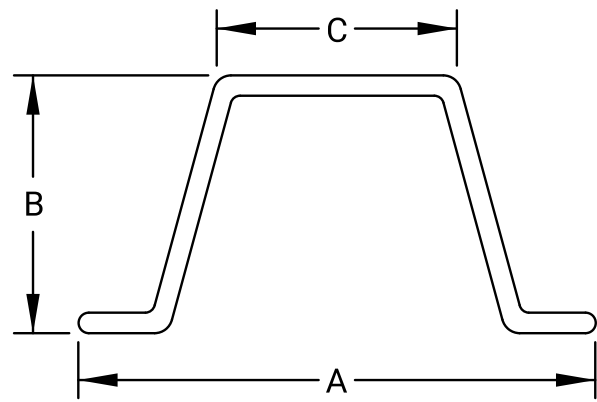
16-JUL-2024 20:36



OM3-L

OM3-R

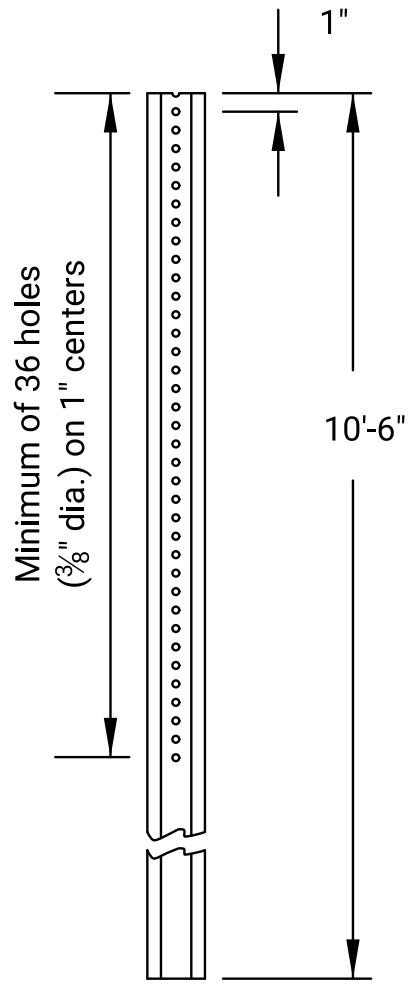
COLORS:  
Yellow Background (Reflective)  
Black Stripes (Non-reflective)



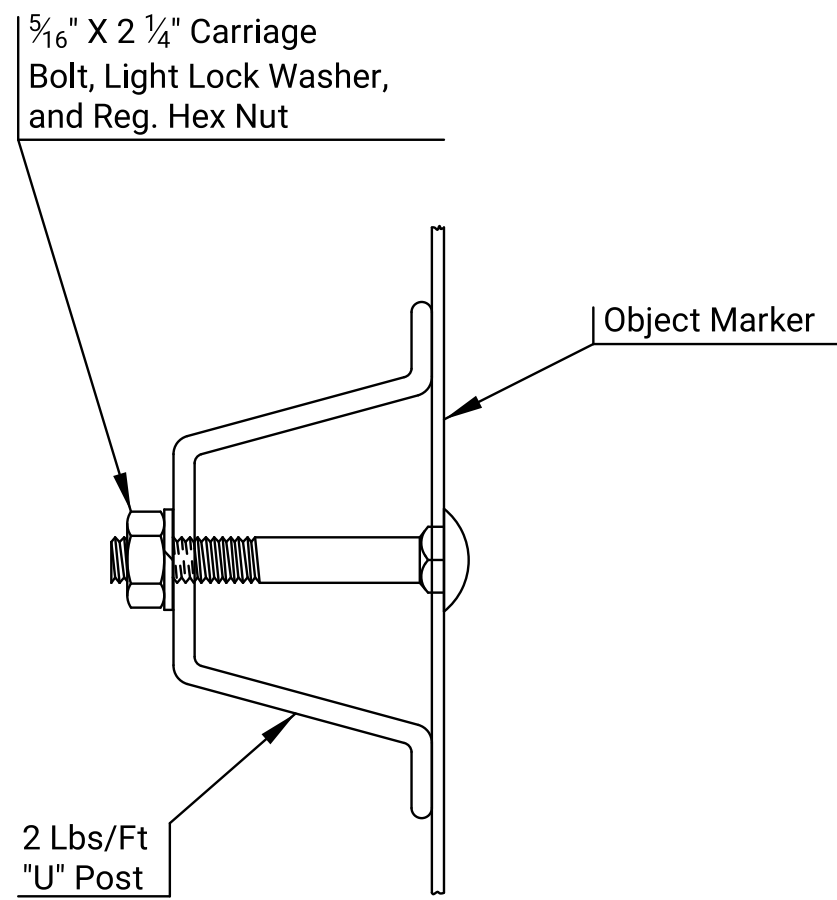
DIMENSIONS	
A	3 1/8
B	1 17/32
C	1 1/4

(Dimensions are nominal)

2 lb/ft "U" POST



PUNCHING DETAILS

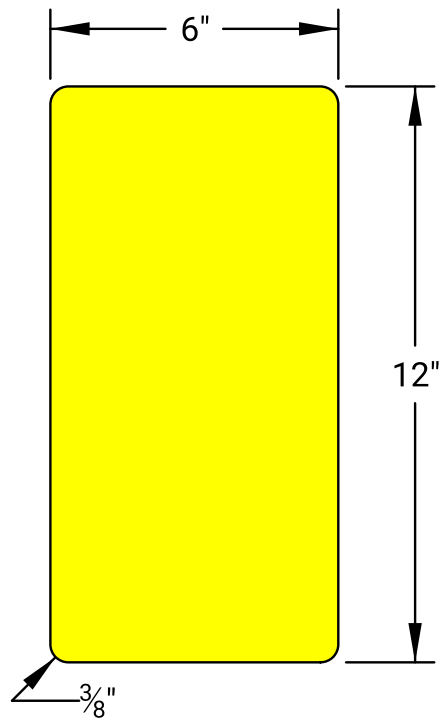


MOUNTING DETAILS

TYPE 3 OBJECT MARKER

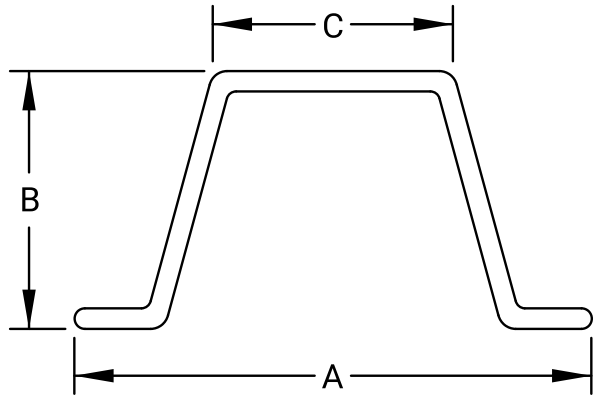
GENERAL NOTE:  
See flat sheet sign blank standard sheets for the 6" x 12" and 12" x 36" sign blank details.

The object markers shall be covered with Type XI High Intensity yellow retroreflective sheeting.



OM2

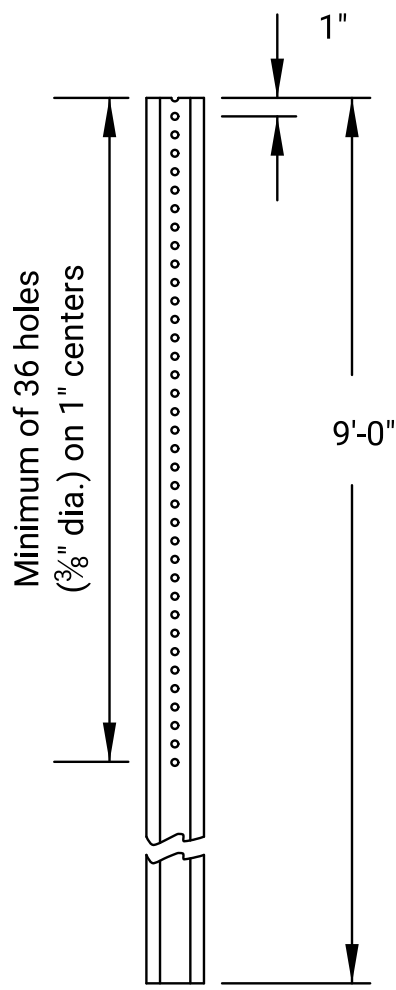
COLOR:  
Yellow Background (Reflective)



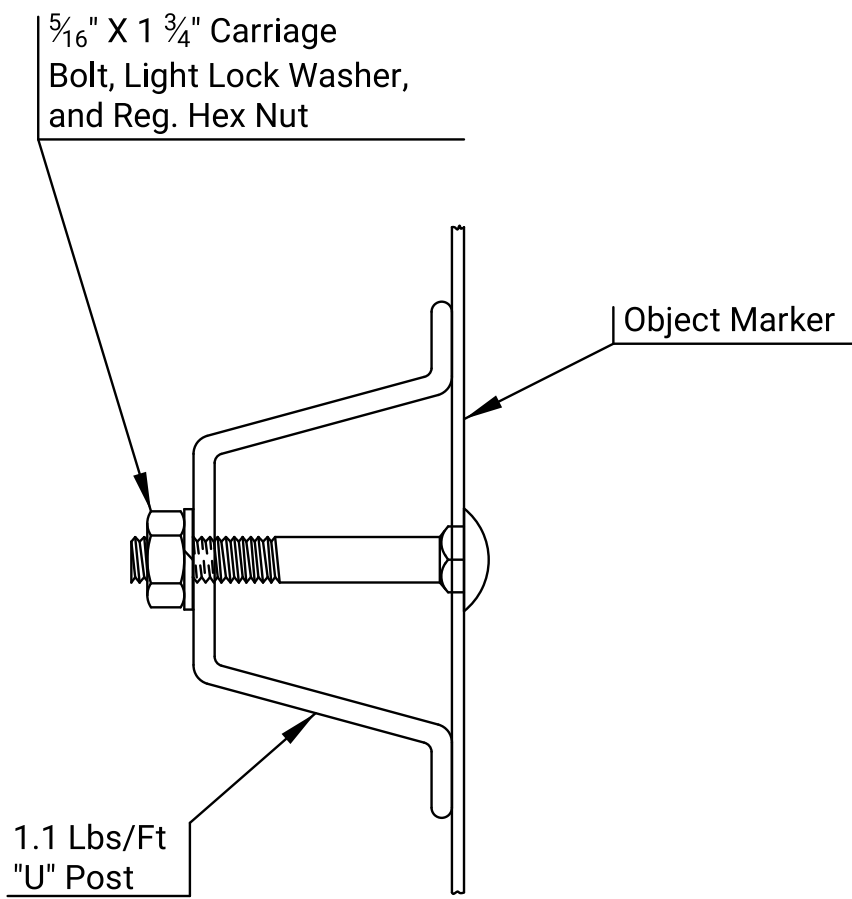
DIMENSIONS	
A	2 1/16
B	7/8
C	1 3/16

(Dimensions are nominal)

DELINEATOR POST  
(1.1 lb/ft "U" Post)



PUNCHING DETAILS



MOUNTING DETAILS

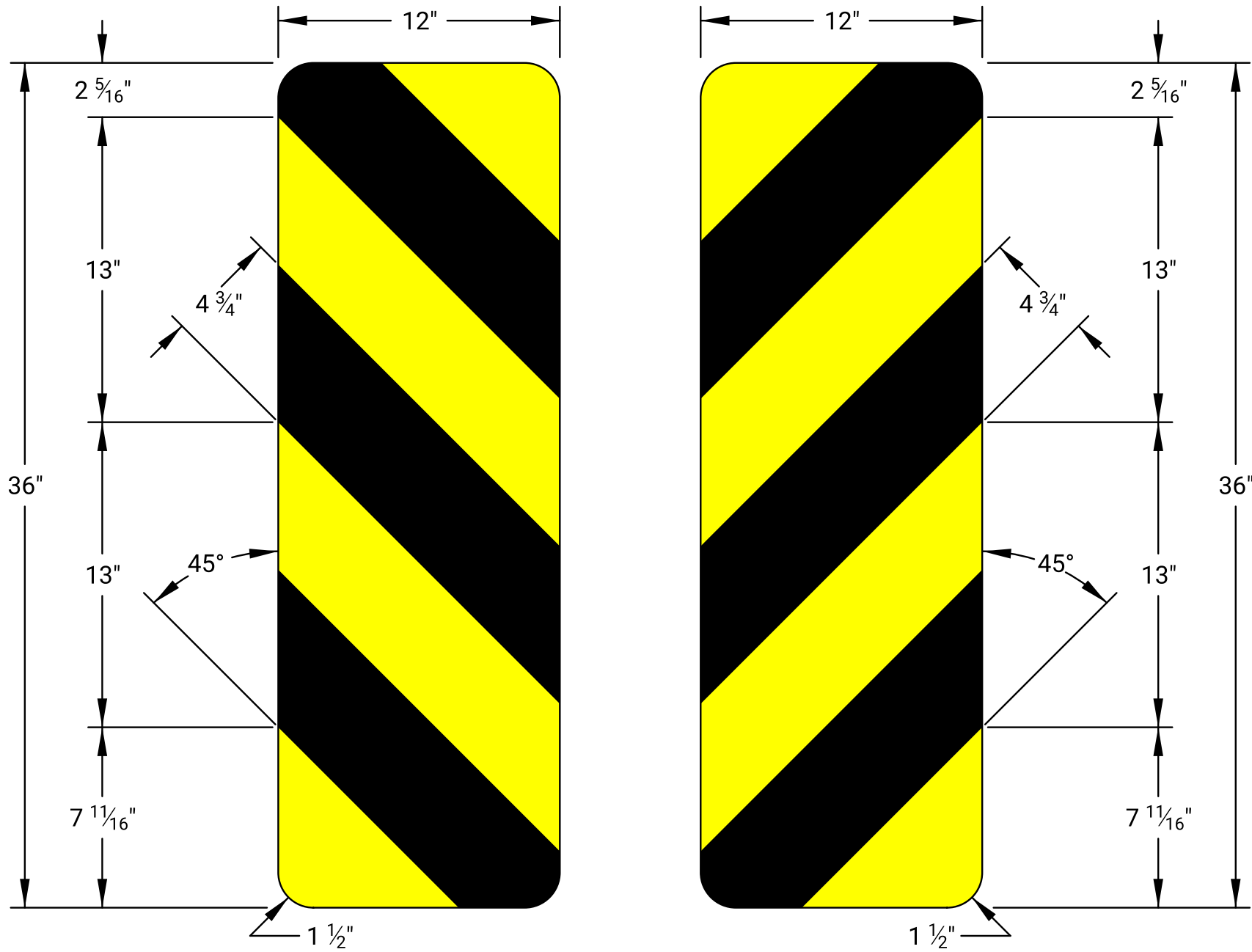
TYPE 2 OBJECT MARKER

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	84	141

All dimensions are in inches unless otherwise noted.  
See standard plan sheet TE590 for detailed specifications.

NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
DESIGN DETAILS FOR OBJECT MARKERS TYPE 2 AND TYPE 3					
TE416		10-01-19		Eric W. Nichol	
FHWA APPROVAL		10-01-19	APP'D		
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES	TRACED
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN. CK.	TRACE CK.

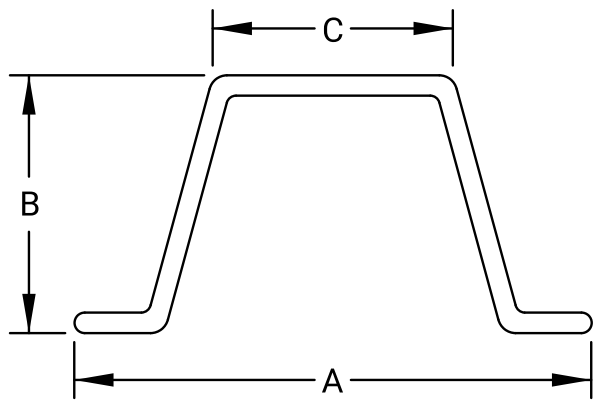
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	85	141



OM3-L

OM3-R

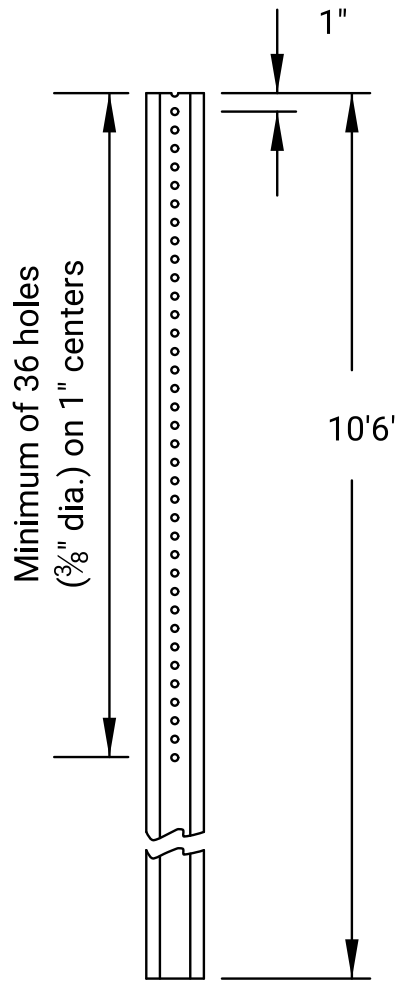
COLORS:  
Yellow Background (Reflective)  
Black Stripes (Non-reflective)



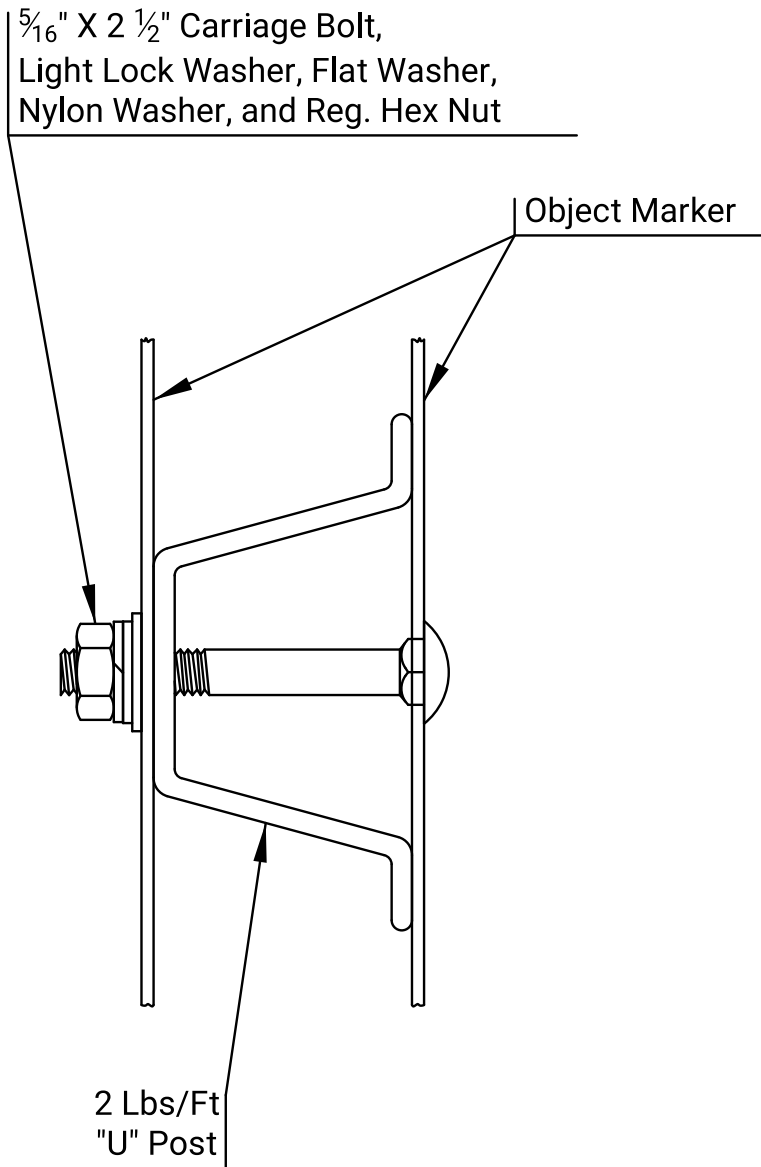
DIMENSIONS	
A	3 1/8
B	1 17/32
C	1 1/4

(Dimensions are nominal)

2 lb/ft "U" POST



PUNCHING DETAILS



MOUNTING DETAILS

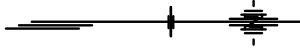
GENERAL NOTE:  
See flat sheet sign blank standard sheets for the 12" x 36" sign blank details.  
  
The object markers shall be covered with Type IV or better High Intensity yellow retroreflective sheeting.

TYPE 3 OBJECT MARKER

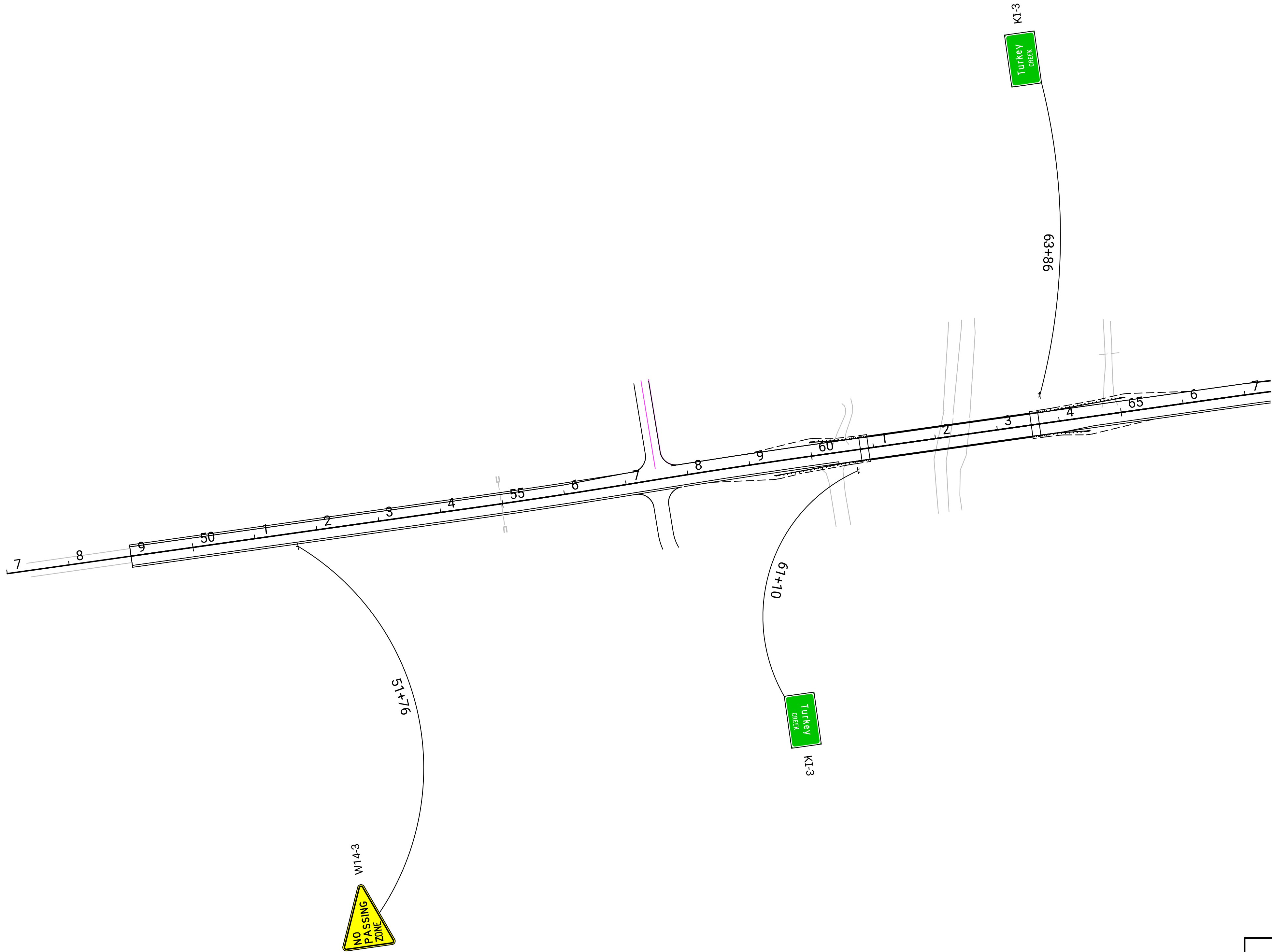
All dimensions are in inches unless otherwise noted.  
See standard plan sheet TE590 for detailed specifications.

NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
DESIGN DETAILS FOR TYPE 3 OBJECT MARKERS (BACK TO BACK)					
TE417			10-01-19		
FHWA APPROVAL		10-01-19	APP'D.	Eric W. Nichol	
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES	TRACED
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN. CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	86	141



SCALE 1" = 100'

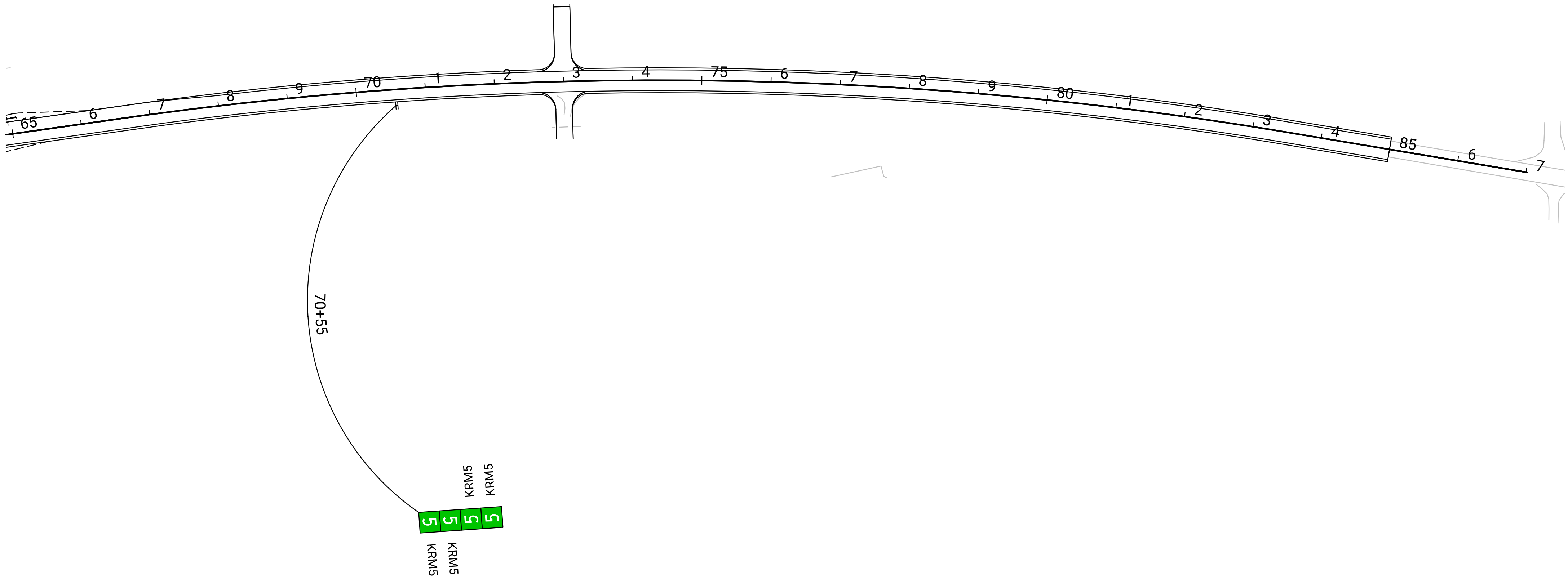


NOTE: SIGNS AND LEGEND NOT TO SCALE

KANSAS DEPARTMENT OF TRANSPORTATION			
K-63 MAINLINE STA. 47+00 TO STA. 66+00			
APP'D DESIGNED	DETAILED	DESIGN CK.	DETAIL CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	87	141

SCALE 1" = 100'



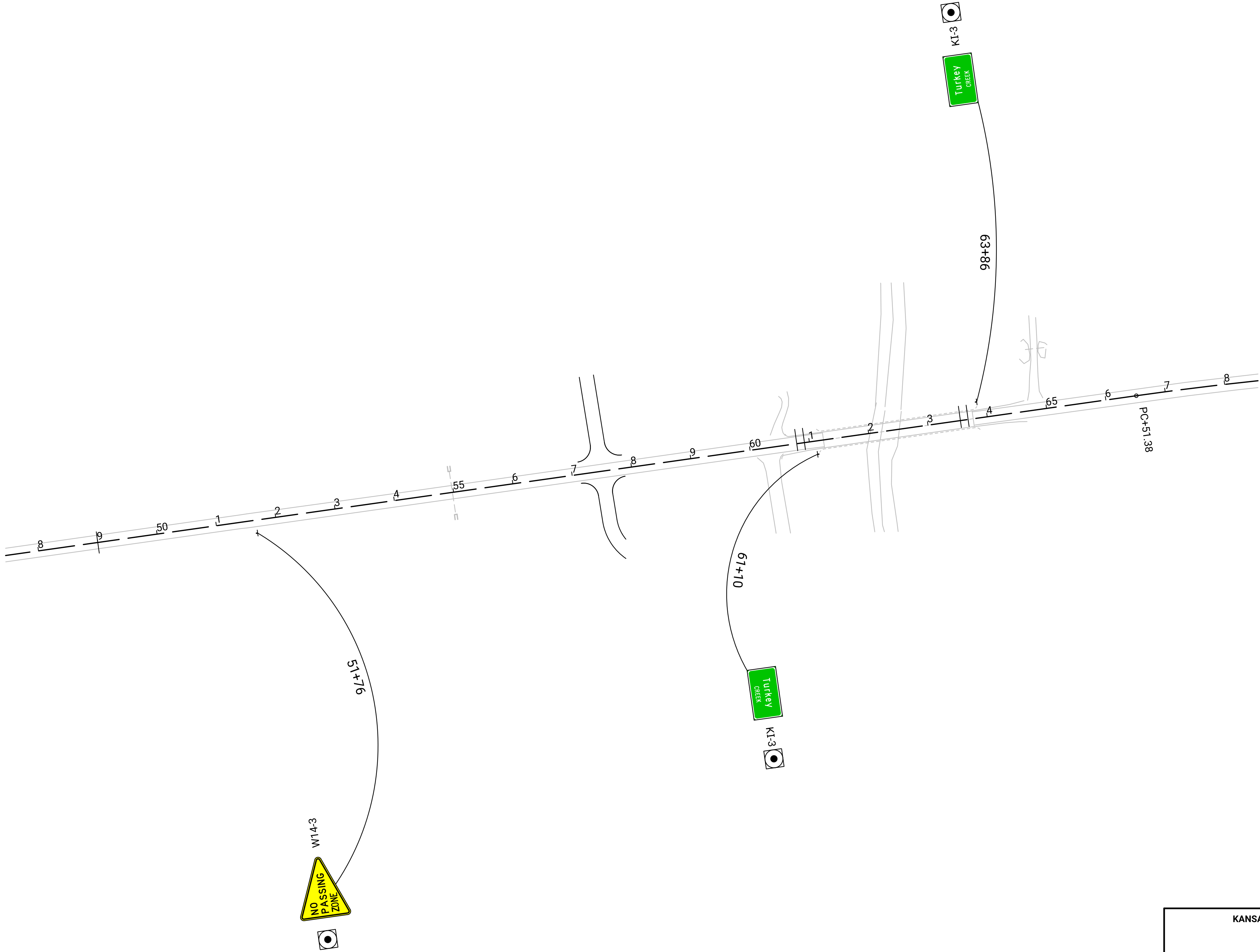
NOTE: SIGNS AND LEGEND NOT TO SCALE

KANSAS DEPARTMENT OF TRANSPORTATION			
K-63 MAINLINE			
STA. 66+00 TO STA. 87+00			
APP'D			
DESIGNED	DETAILED	DESIGN CK.	DETAIL CK.



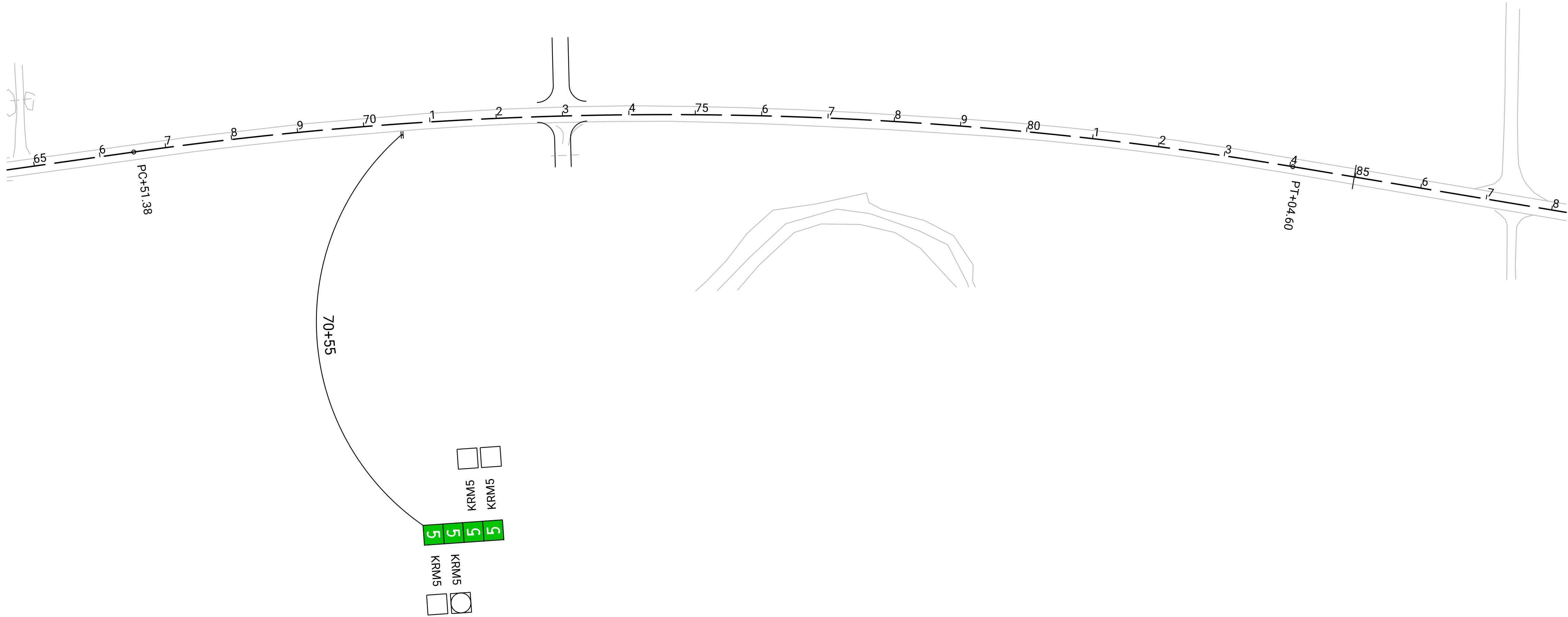
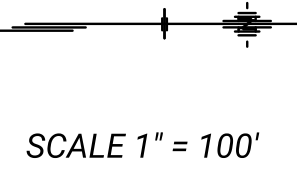
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	88	141

SCALE 1" = 100'



KANSAS DEPARTMENT OF TRANSPORTATION			
K-63 MAINLINE STA. 48+00 TO STA. 66+00 REMOVALS			
APP'D DESIGNED	DETAILED	DESIGN CK.	DETAIL CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	89	141



NOTE: SIGNS AND LEGEND NOT TO SCALE

KANSAS DEPARTMENT OF TRANSPORTATION			
K-63 MAINLINE STA. 66+00 TO STA. 87+00 REMOVALS			
APP'D DESIGNED	DETAILED	DESIGN CK.	DETAIL CK.



## QUANTITIES SHEET

## DELINEATORS AND OBJECT MARKERS

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	91	141

[illegible]

01	10-09-21	Added delineator & object marker types					D.D.G.	E.W.N.
NO.	DATE	REVISIONS					BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION								
QUANTITIES SHEET								
DELINEATORS & OBJECT MARKERS								
TE436							07-01-03	
FHWA APPROVAL		10-01-19		APP'D.		Steven A. Buckley		
DESIGNED	D.D.G.	DETAILED	K.D.S.	QUANTITIES		TRACED		
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.		TRACE CK.		



Plotted by : austin.loyd@ks.gov  
File : KA572901pss439-01.dgn

16-JUL-2024 20:36

SIGNS		
TYPE	NUMBER	SQUARE FEET
FLAT SHEET	7	18.562
REINFORCED PANEL		
OVERLAY		

DELINEATORS				
TYPE	FLEXIBLE DELINEATOR		RIGID DELINEATOR	
	TYPE I ANCHOR	TYPE III ANCHOR	"U" POST	BRACKET MOUNT
TYPE 'A' WHITE				
TYPE 'A' YELLOW				
TYPE 'B' WHITE				
TYPE 'B' YELLOW				
TYPE 'A' WHITE (BACK TO BACK)				
TYPE 'A' YELLOW (BACK TO BACK)				

OBJECT MARKERS			
TYPE			NUMBER
TYPE 2 ("U" POST)			4
TYPE 3 ("U" POST)			
INFORMATION ONLY	OM3-L		<div></div>
	OM3-R		
	OM3-C		
TYPE 3 ("U" POST) (BACK TO BACK)			

NUMBER & LENGTHS OF POSTS & ALUMINUM BEAMS (INFORMATION ONLY)																
LENGTH OF POST OR BEAM	4" x 6" POST			312.25 ALUMINUM BEAM	"U" POST		GALVANIZED STEEL BEAM POST						PERFORATED SQUARE STEEL TUBE (PSST)			
	WOOD		STEEL				W6x9		W10x12		W10x22					
	FLAT SHEET SIGN	REINFORCED PANEL SIGN	STRUCTURAL TUBING		2 LBS/FT	3 LBS/FT	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	1-3/4"	2"	2-1/4"	2-1/2"
	2.1' - 4'															
4.1' - 6'																
6.1' - 8'																
8.1' - 10'																
10.1' - 12'					1											
12.1' - 14'	4															
14.1' - 16'	1															
16.1' - 18'																
18.1' - 20'																
20.1' - 22'																
22.1' - 24'																
24.1' - 26'																
26.1' - 28'																
28.1' - 30'																
30.1' - 32'																

SUMMARY OF QUANTITIES

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	92	141

POSTS AND ALUMINUM BEAMS																
	4" x 6" POST			312.25 ALUMINUM BEAM	"U" POST		GALVANIZED STEEL BEAM POST						PERFORATED SQUARE STEEL TUBE (PSST)			
	WOOD		STEEL				W6x9		W10x12		W10x22					
	FLAT SHEET SIGN	REINFORCED PANEL SIGN	STRUCTURAL TUBING		2 LBS/FT	3 LBS/FT	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	1-3/4"	2"	2-1/4"	2-1/2"
	NUMBER	5				1										
FEET	71.0				10.5											

POST FOOTINGS AND BRACKETS											
	CONCRETE FOOTING (DIA.)					PERFORATED SQUARE STEEL					
	WOOD	A36 STEEL		A572 STEEL (ALT)		TUBE FOOTING				BRACKET	
	18"	24"	30"	24"	30"	1-3/4"	2"	2-1/4"	2-1/2"	1-3/4"	2"
NUMBER											
FEET											

BASE PLATES AND STUB POSTS						
	W6x9		W10x12		W10x22	
	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)
BREAKAWAY BASES						
BASE PLATE (TOP)						
STUB POST WITH BASE PLATE						
NON-BREAKAWAY BASES						
BASE PLATE						

REMOVALS	
TYPE	NUMBER
SIGNS	7
POSTS	4
FOOTINGS	2
SIGN STRUCTURES	

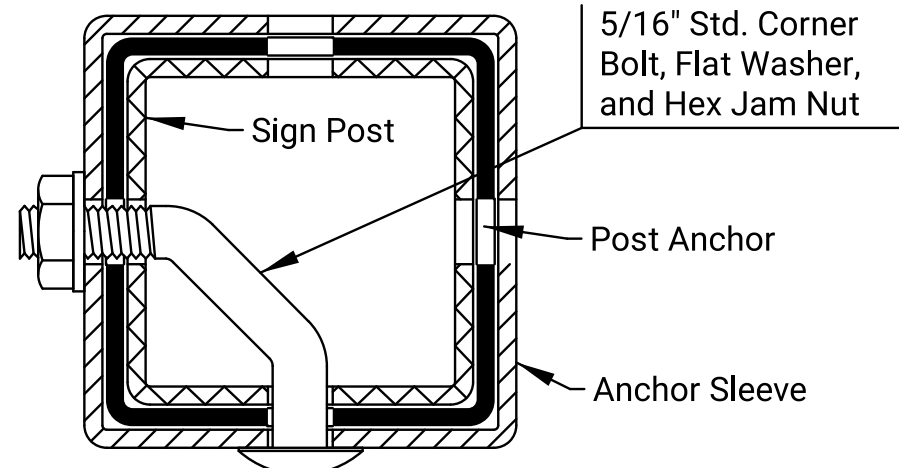
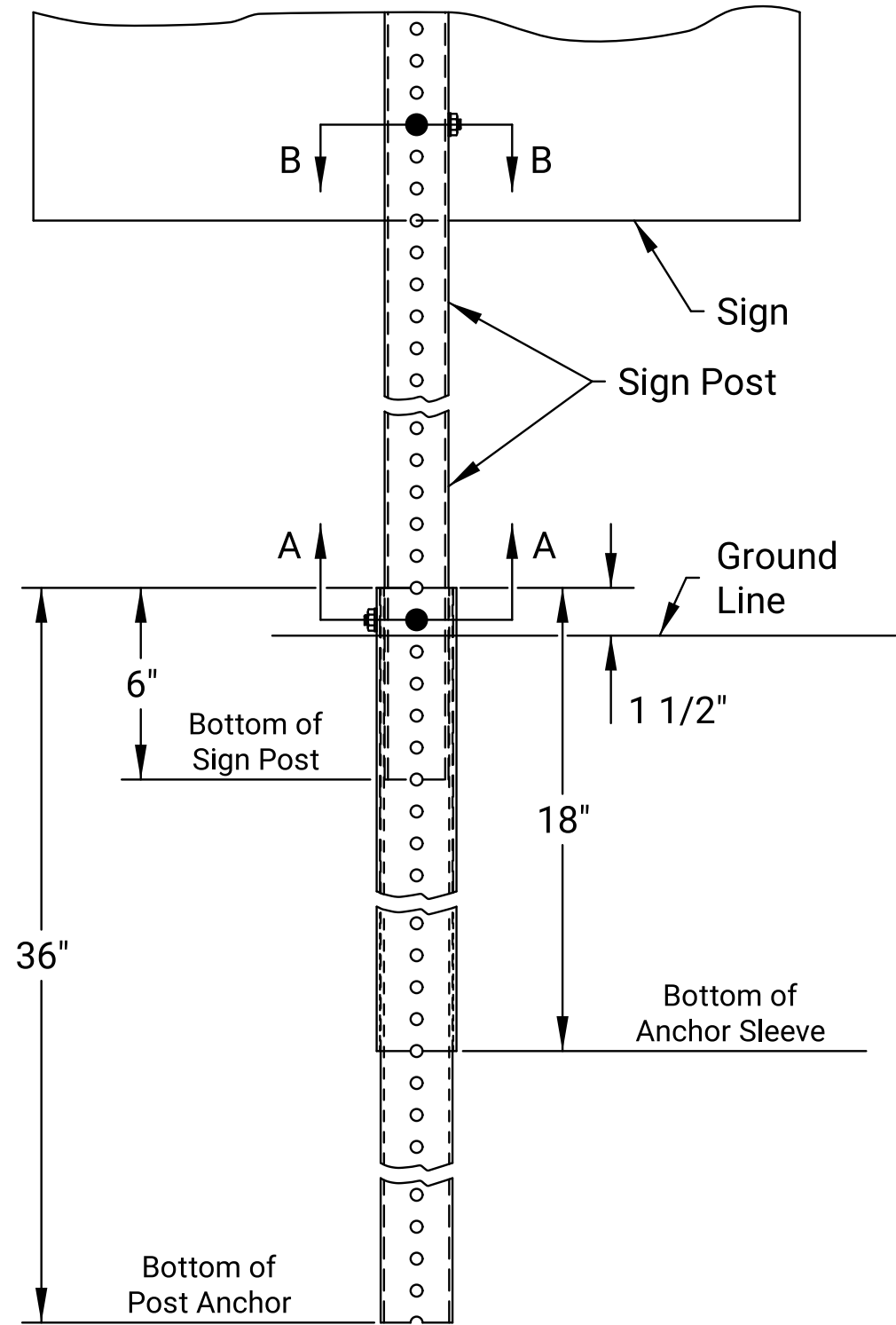
SIGN STRUCTURES				
TYPE	NEW	MODIFIED	REMOVE AND RESET	RESET
OVERHEAD STRUCTURE				
CANTILEVER STRUCTURE				
BUTTERFLY STRUCTURE				
BRIDGE MOUNT ATTACHMENT				
MAST ARM SIGN SUPPORT				
SINGLE TAPERED TUBE SIGN SUPPORT				

02	10-01-19	Revised Tables		D.D.G.	E.W.N.
01	07-23-10	Revised Tables		D.D.G.	D.B.
NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
SUMMARY OF QUANTITIES FOR INSTALLATIONS AND REMOVALS					
TE439				07-01-03	
FHWA APPROVAL		10-01-19	APP'D.	Steven A. Buckley	
DESIGNED	D.D.G.	DETAILED	K.D.S.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.



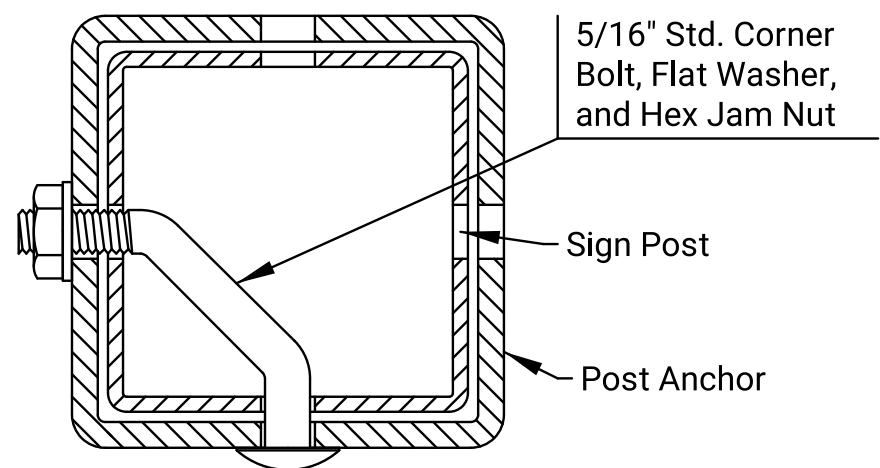
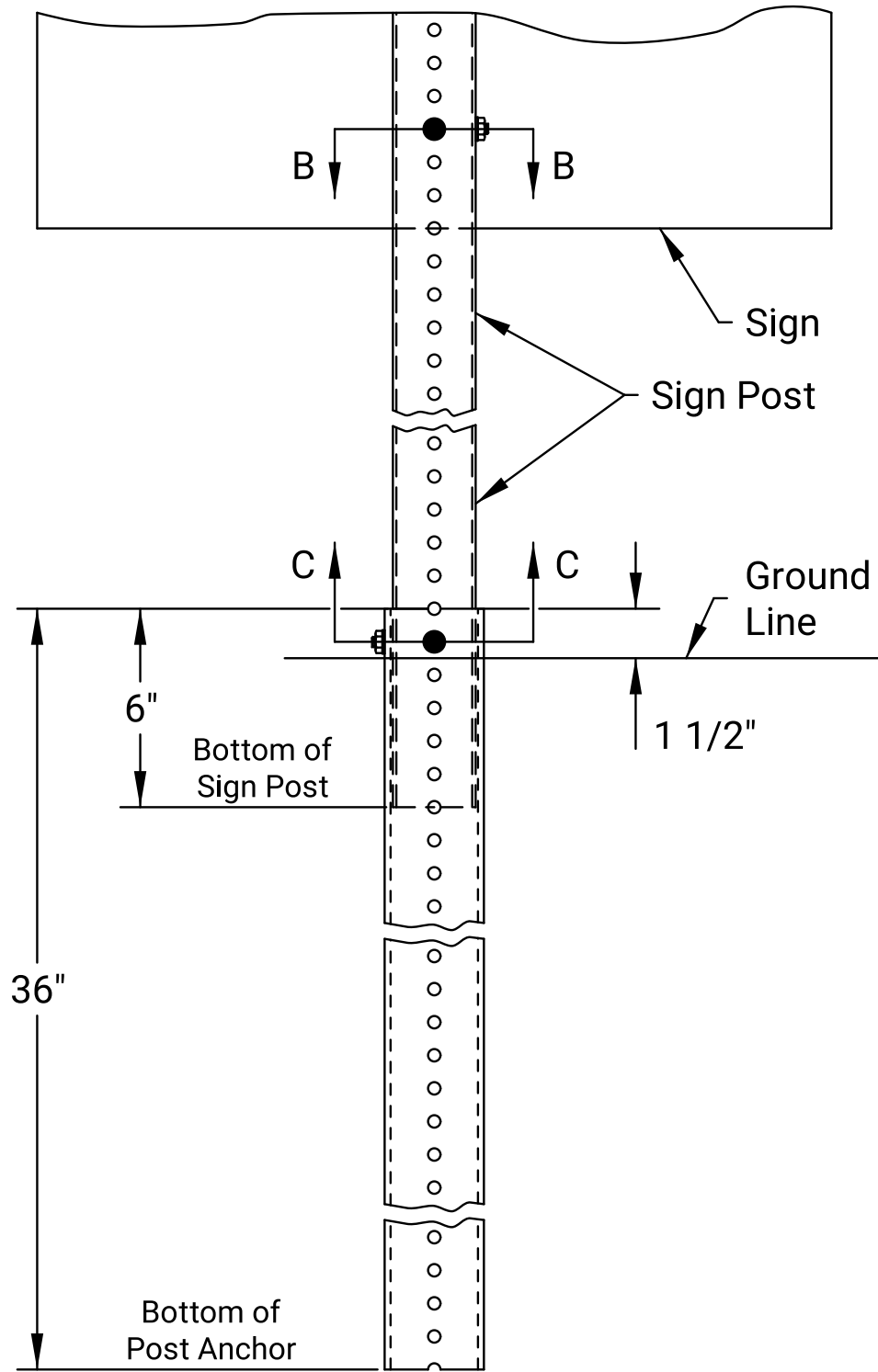


STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	95	141



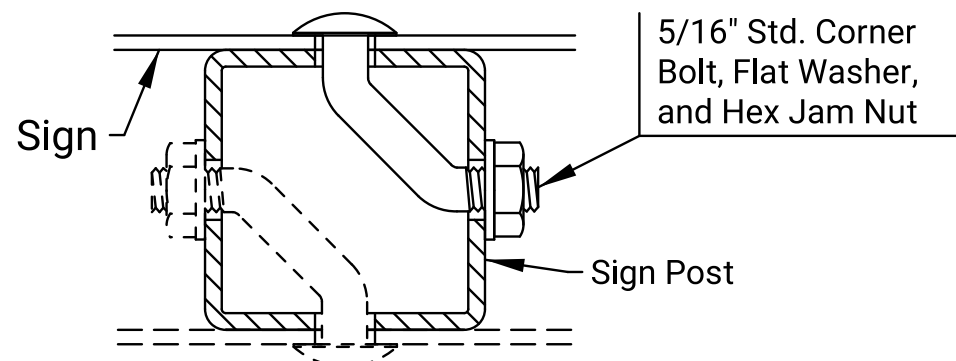
SECTION A-A

1 3/4", 2", OR 2 1/4" PSST SIGN POST



SECTION C-C

2 1/2" PSST SIGN POST



SECTION B-B

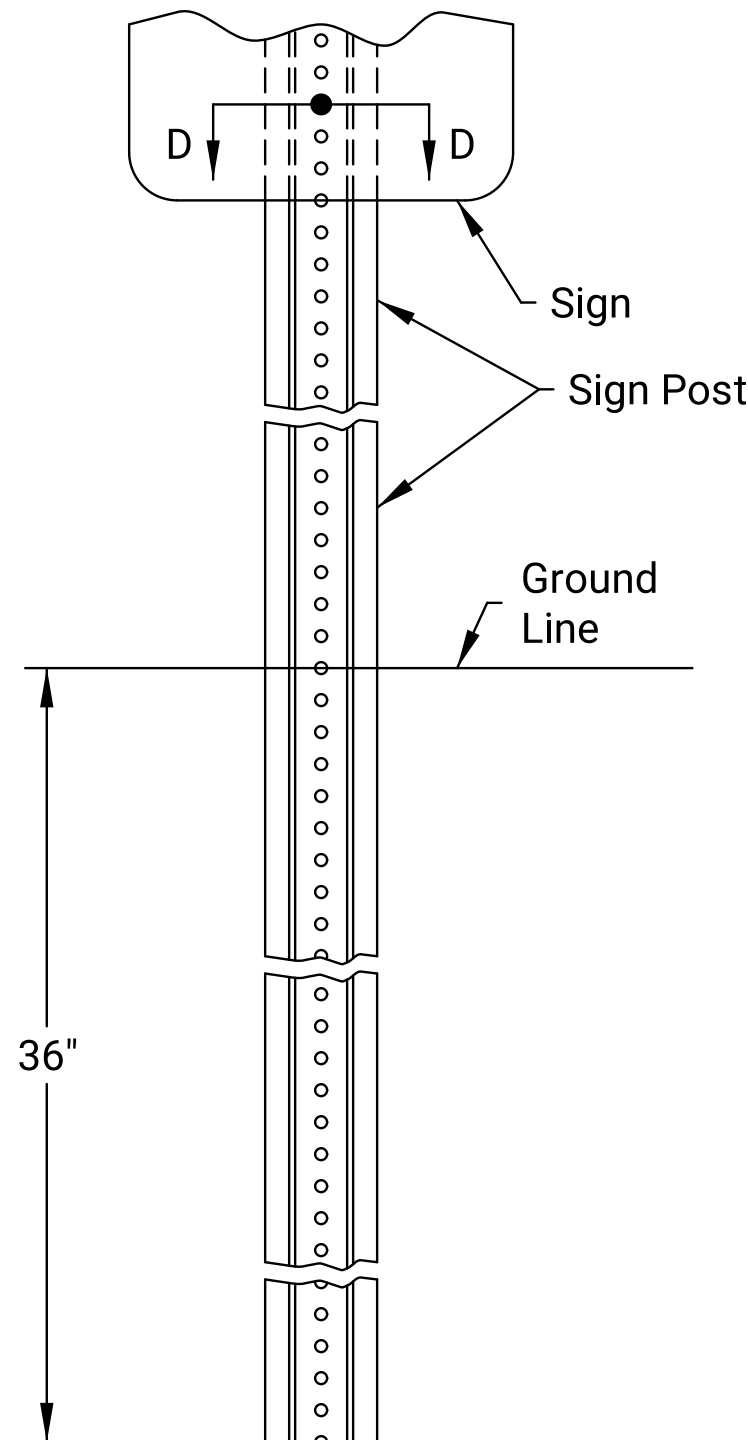
MATERIALS TABLE FOR SIGN POST AND FOOTING		
SIGN POST 12 GA. OR 14 GA.	FOOTING	
	POST ANCHOR	ANCHOR SLEEVE
1 3/4" X 1 3/4"	2" X 2" X 12 GA.	2 1/4" X 2 1/4" X 12 GA.
2" X 2"	2 1/4" X 2 1/4" X 12 GA.	2 1/2" X 2 1/2" X 12 GA.
2 1/4" X 2 1/4"	2 1/2" X 2 1/2" X 12 GA.	3" X 3" X 7 GA.
2 1/2" X 2 1/2"	3" X 3" X 7 GA.	Not Required

NOTE: 14 ga. posts must meet a certified minimum yield strength of 60,000 p.s.i.

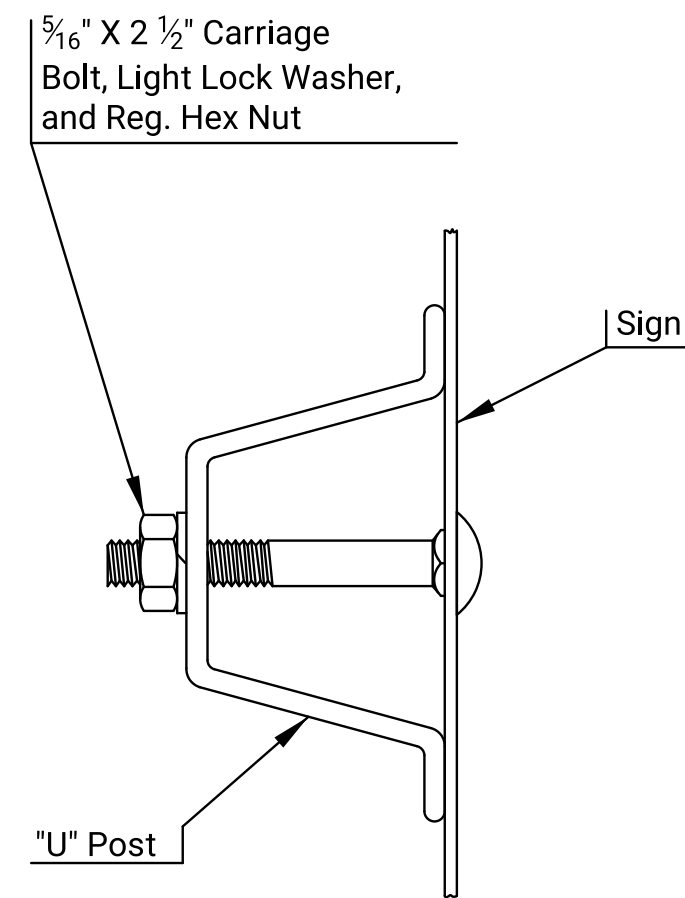
### INSTALLATION PROCEDURES

1. Plumb and drive post anchor into the ground 18", if anchor sleeve is required, or to the specified height above the ground line.
2. Install anchor sleeve (if required) on the post anchor and align the first holes above the ground line. Plumb and drive post anchor with anchor sleeve into the ground to the specified height above the ground line.
3. Install sign post into the post anchor.

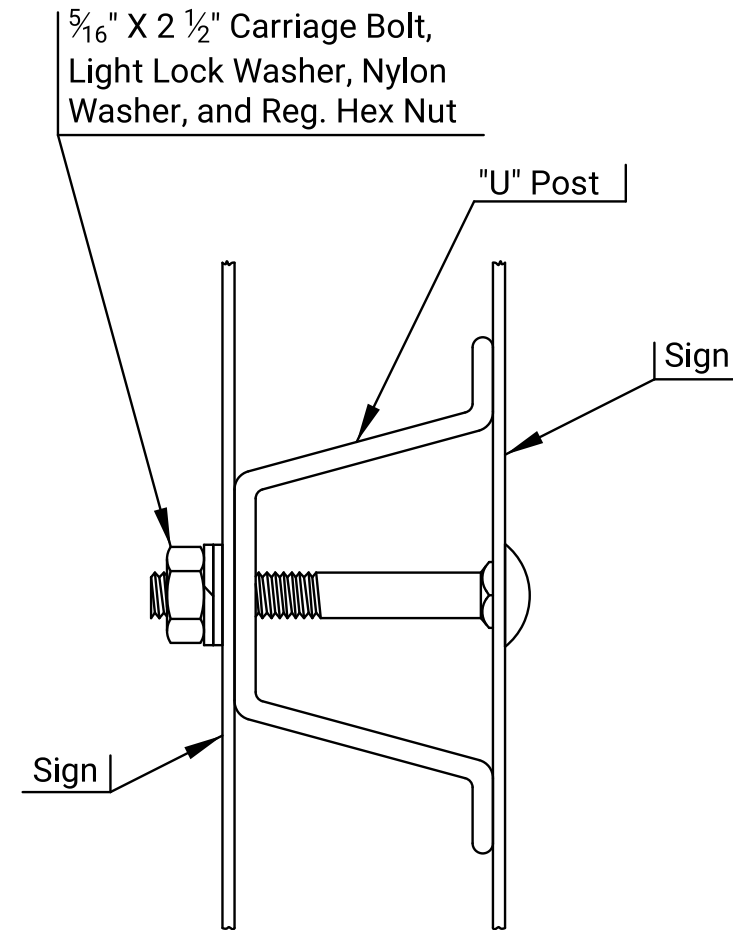
### PERFORATED SQUARE STEEL TUBE POST (PSST)



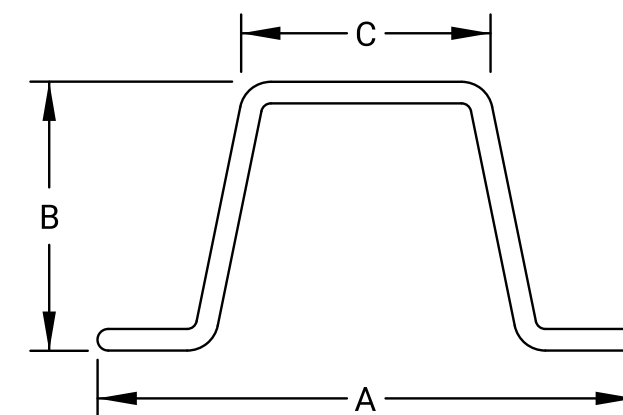
TYPICAL



SECTION D-D  
(TYPICAL)



SECTION D-D  
(BACK TO BACK)



DIM.	2 LBS/FT	3 LBS/FT
A	3 1/8 "	3 1/2 "
B	1 17/32 "	1 3/4 "
C	1 1/4 "	1 5/8 "

(Dimensions are nominal)

"U" POST

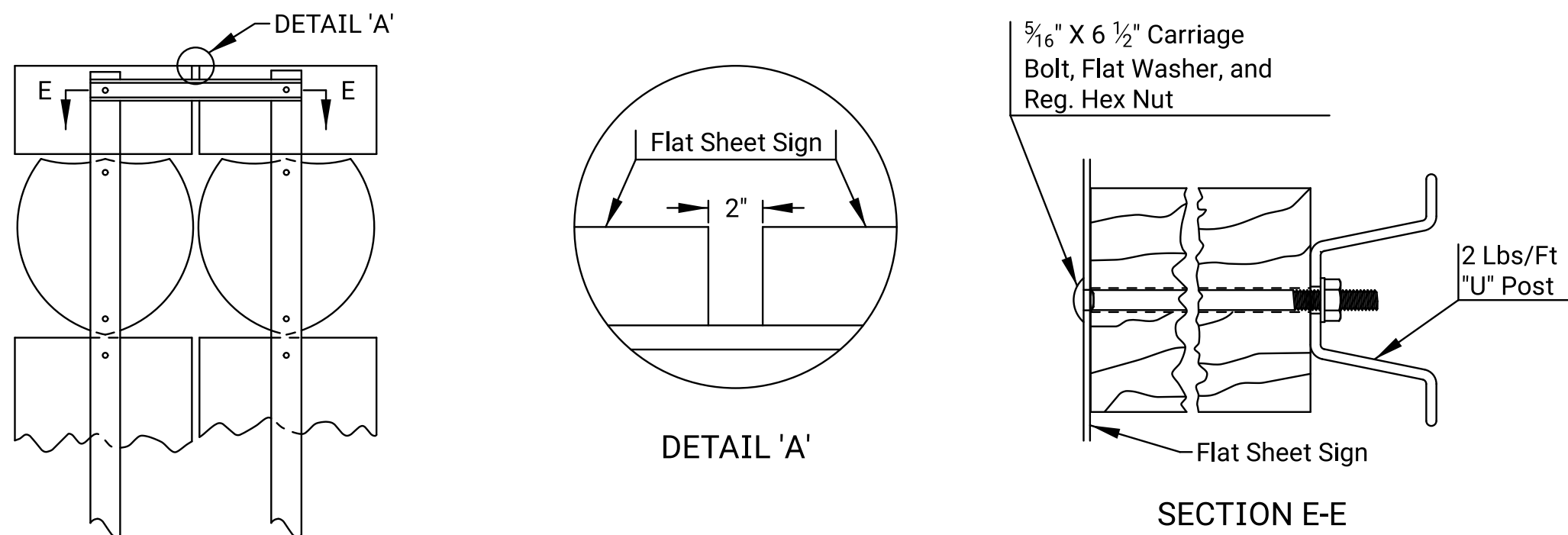
### STEEL "U" POST

NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
DETAILS FOR PERFORATED SQUARE STEEL TUBE POSTS (PSST) AND STEEL "U" POSTS					
TE466		10-01-19		Eric W. Nichol	
FHWA APPROVAL		10-01-19		APP'D	
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES	TRACED
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN. CK.	TRACE CK.

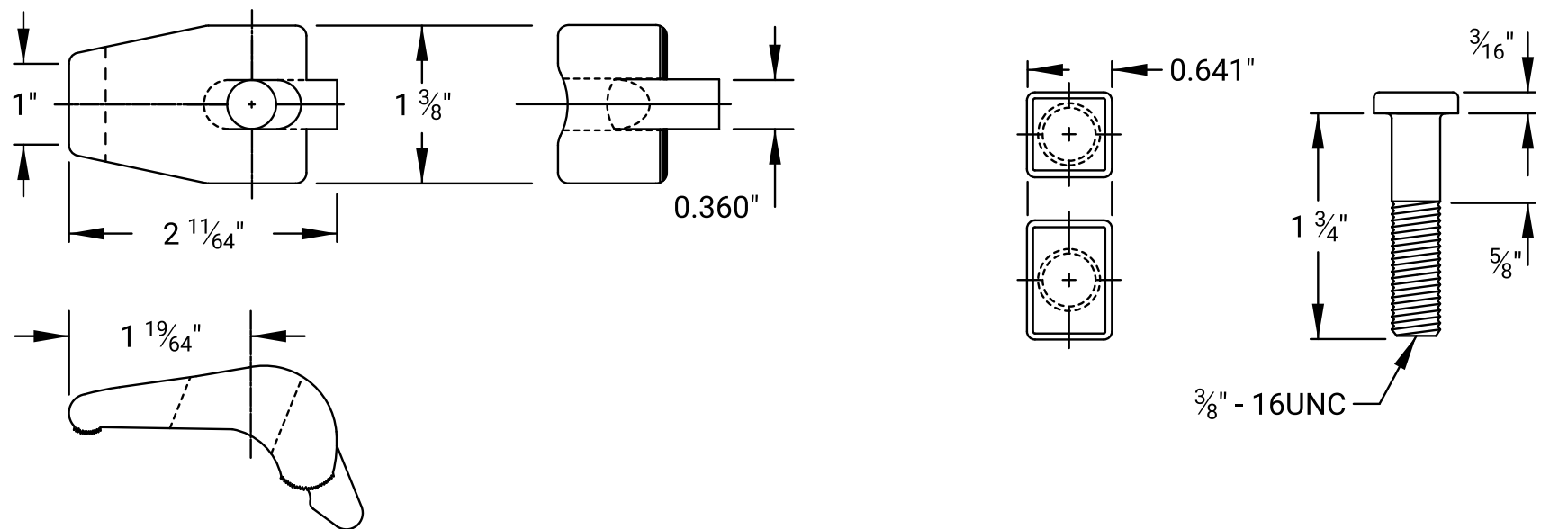


Plotted by : austin.loyd@ks.gov  
File : KA572901pss481-01.dgn  
16-JUL-2024 20:36

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	96	141



ROUTE MARKER ASSEMBLIES ATTACHMENT



ALUMINUM POST CLIP AND POST CLIP BOLT

NOTES:  
The top of the post shall not extend above the top of the sign.

When signs are mounted back to back, the signs shall be mounted at their prescribed height. In general installations, the bottom holes of the signs should be aligned. In order to prevent having to drill holes in the signs or posts, the sign on the back should be raised and positioned such that the holes are aligned. When a sign is mounted on the back of the R1-1 (Stop) sign, that sign is to be centered vertically on the R1-1 sign. When a sign is mounted on the back of the R1-2 (Yield) sign, the top holes of the signs should be aligned.

The primary sign and supplemental sign are to be mounted at their prescribed height, but under no circumstances shall the signs overlap each other. If the primary sign cannot be mounted without overlapping, then it shall be raised above the supplemental sign.

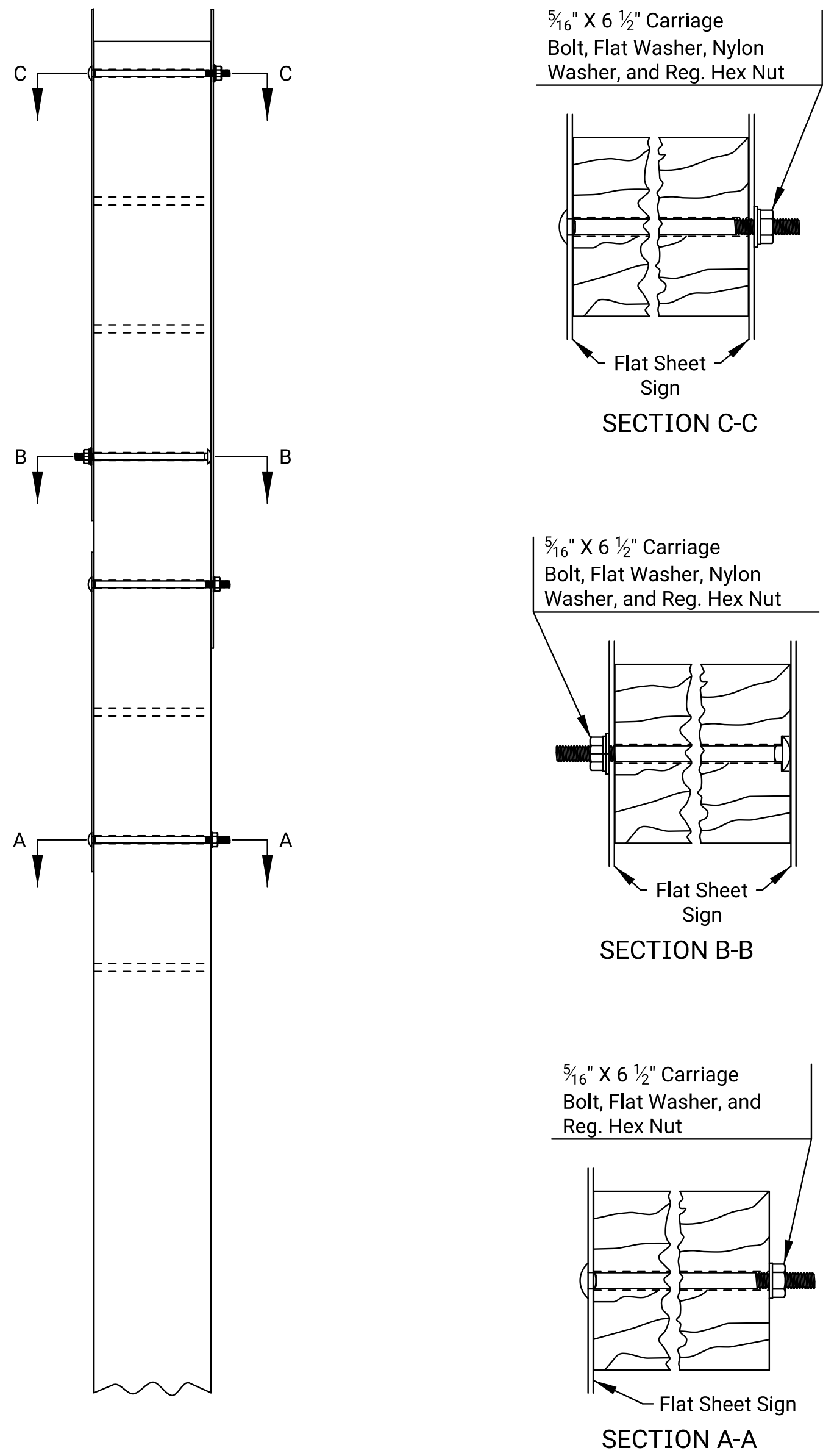
Any additional mounting holes, either through the sign or post, shall be drilled by the contractor. All holes drilled in the post shall be treated with a perservative. All holes drilled in the sign shall be free of any defects and the sheeting around the hole shall not be damaged.

A nylon washer shall be placed against the sheeting when a nut is to be tightened against the sign face.

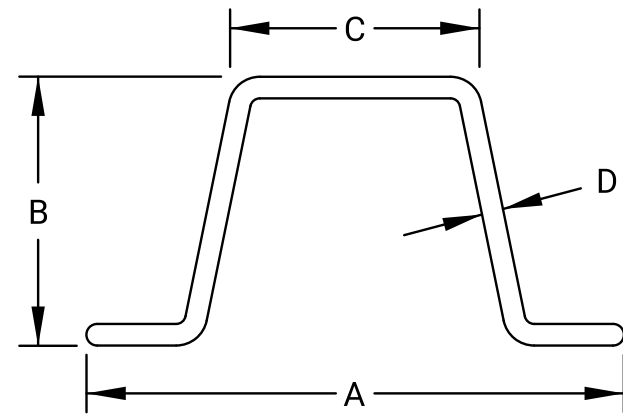
The 3 lb/ft steel "U" post used for reinforced panel sign installations is to be included in the bid item 'SIGN POST (4" x 6" WOOD) (REINFORCED PANEL SIGN)'.

When the 2 lb/ft steel "U" post is used for the route marker assemblies attachment, it shall be subsidiary to the bid item 'SIGN POST (4" x 6" WOOD) (FLAT SHEET SIGN)'.

The aluminum post clip bolt may have a rectangular head if the smaller dimension is equal to the square head dimension.



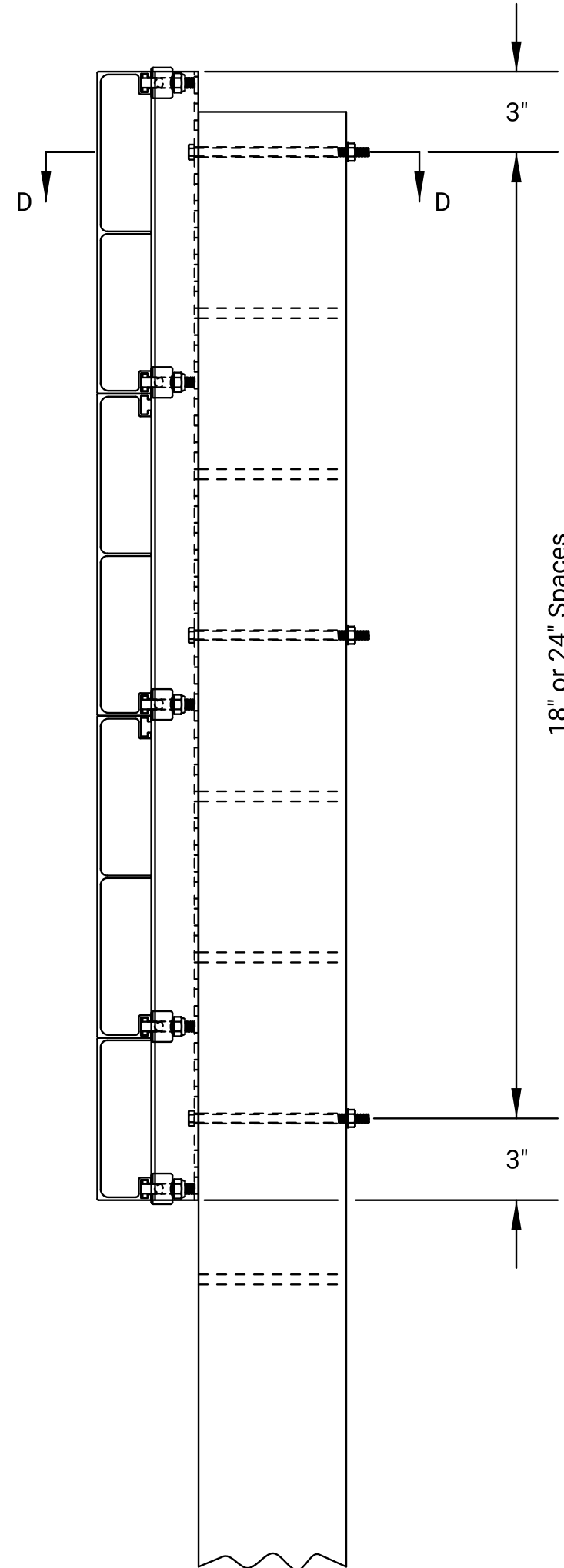
TYPICAL MOUNTING OF FLAT SHEET SIGNS



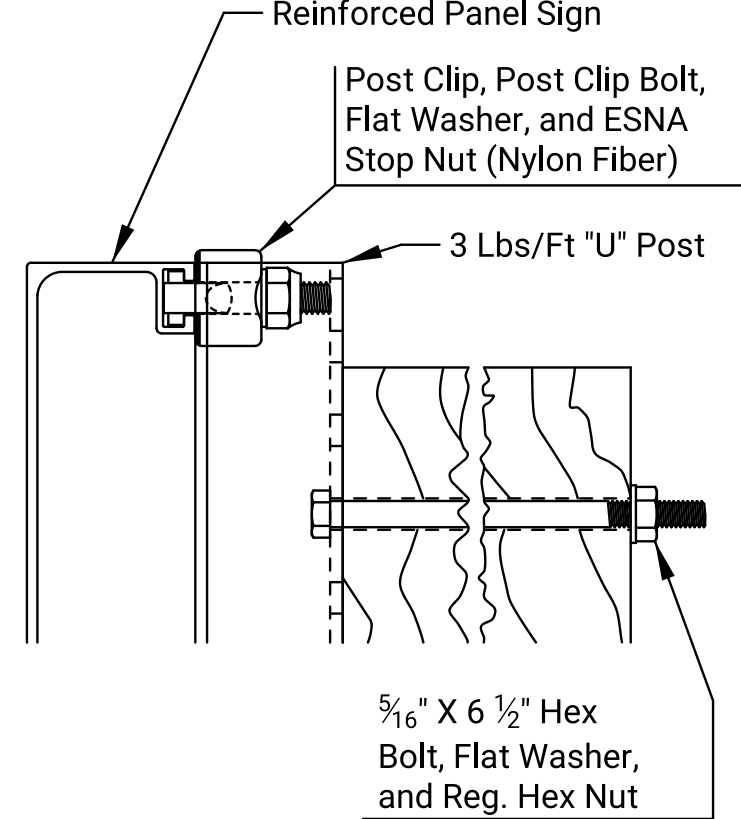
DIM.	2 LBS/FT	3 LBS/FT
A	3 1/8 "	3 1/2 "
B	1 17/32 "	1 3/4 "
C	1 1/4 "	1 5/8 "
D	1/8 "	9/64 "

(DIMENSIONS ARE NOMINAL)

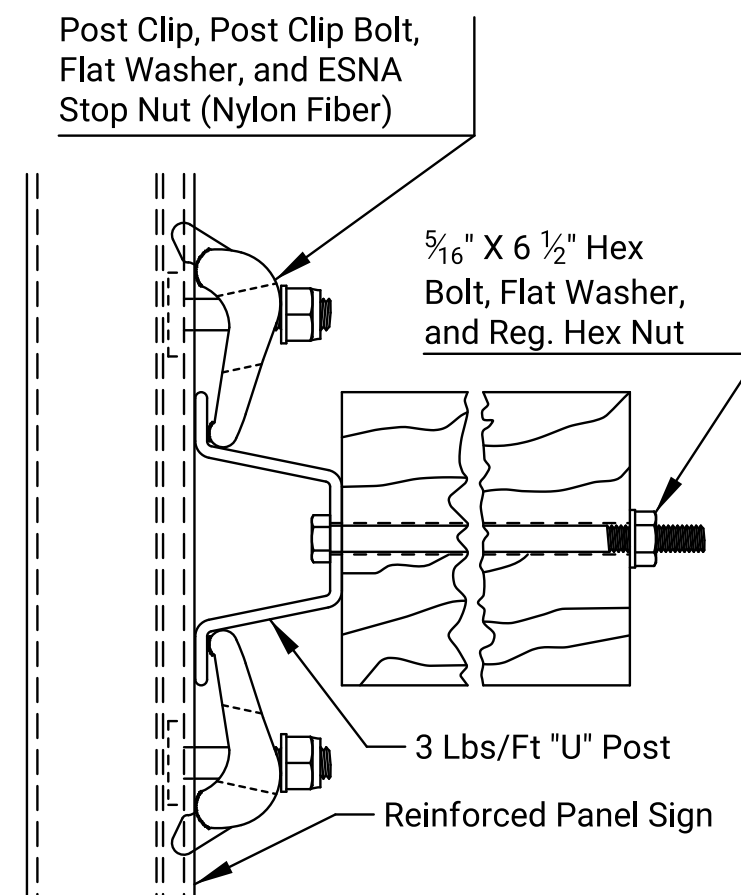
"U" POST



TYPICAL MOUNTING OF REINFORCED PANEL SIGNS



SECTION D-D

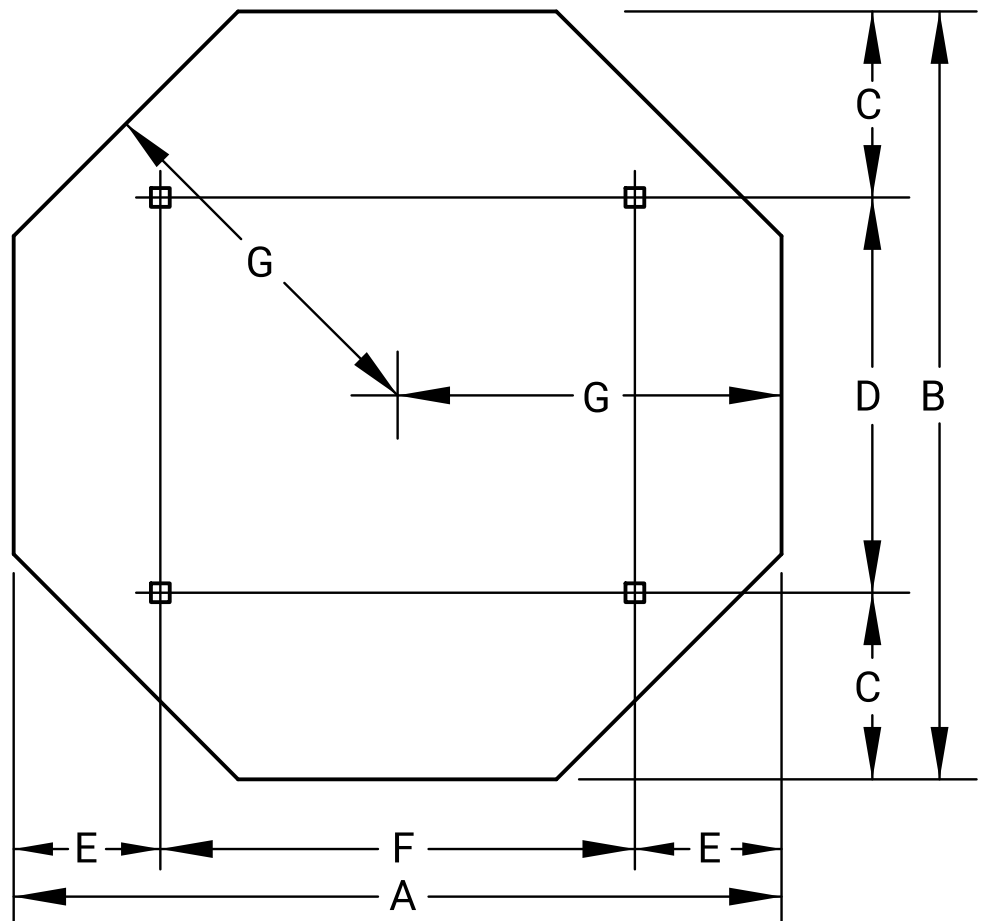


TOP VIEW

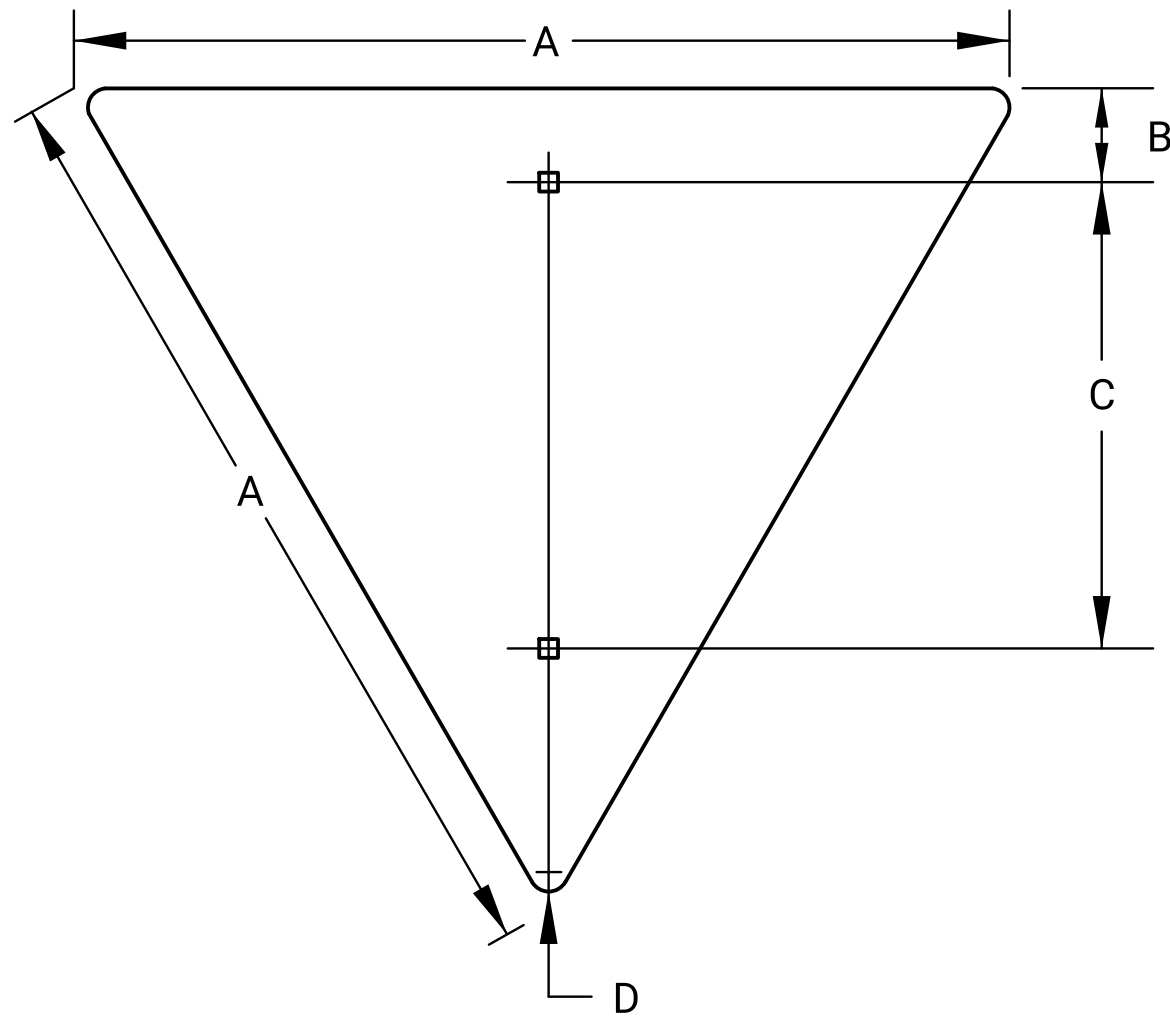
All dimensions are in inches

NO.	DATE	REVISIONS	BY	APPD
01	10-01-19	Revised drawings and notes	D.D.G.	E.W.N.
KANSAS DEPARTMENT OF TRANSPORTATION				
DETAILS FOR MOUNTING SIGNS ON WOOD POST FLAT SHEET AND REINFORCED PANEL				
TE481				
07-01-03				
FHWA APPROVAL				
DESIGNED D.D.G. DETAILED A.A.D. QUANTITIES TRACED				
DESIGN CK. S.A.B. DETAIL CK. D.D.G. QUAN. CK. TRACE CK.				

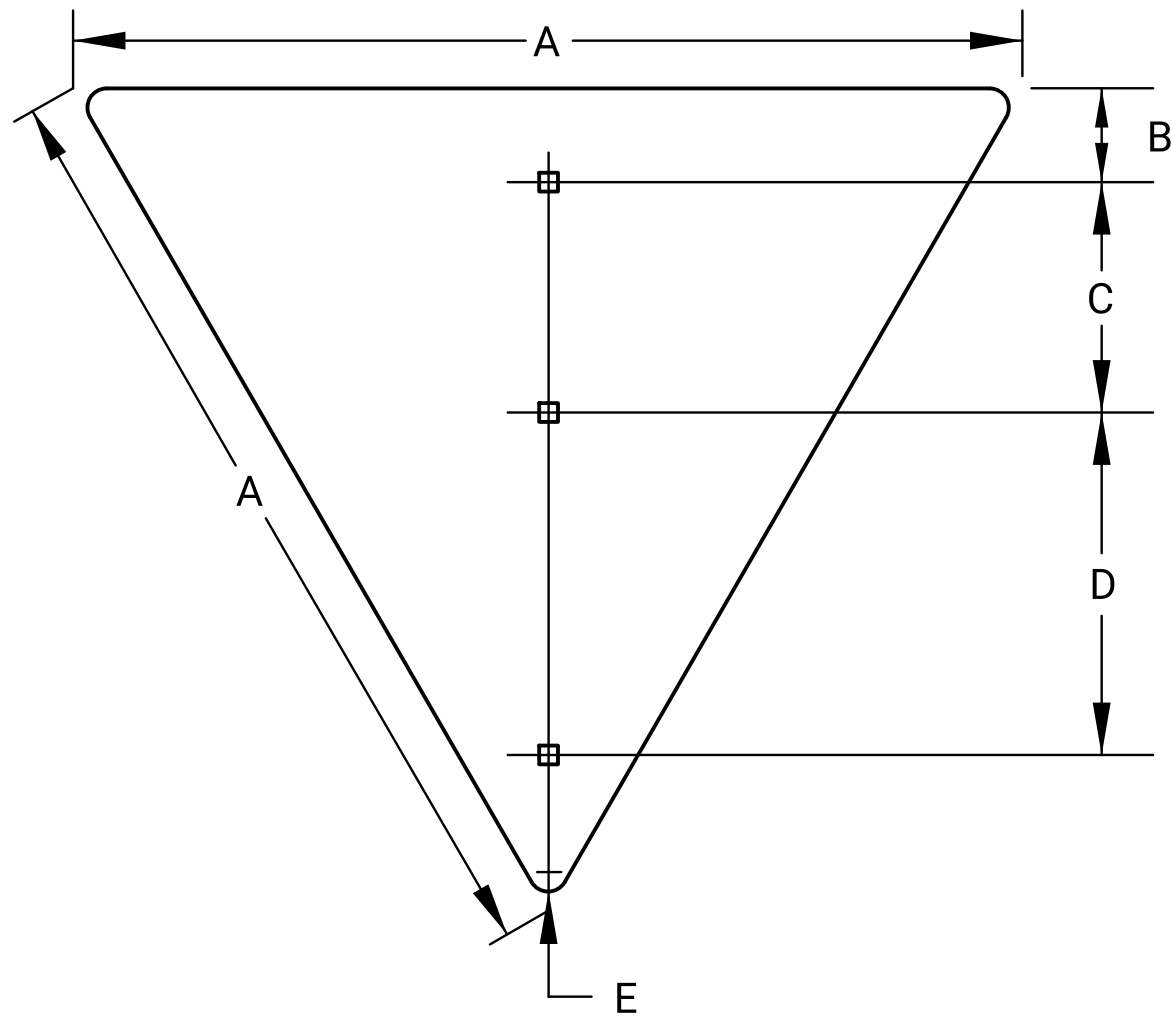
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	97	141



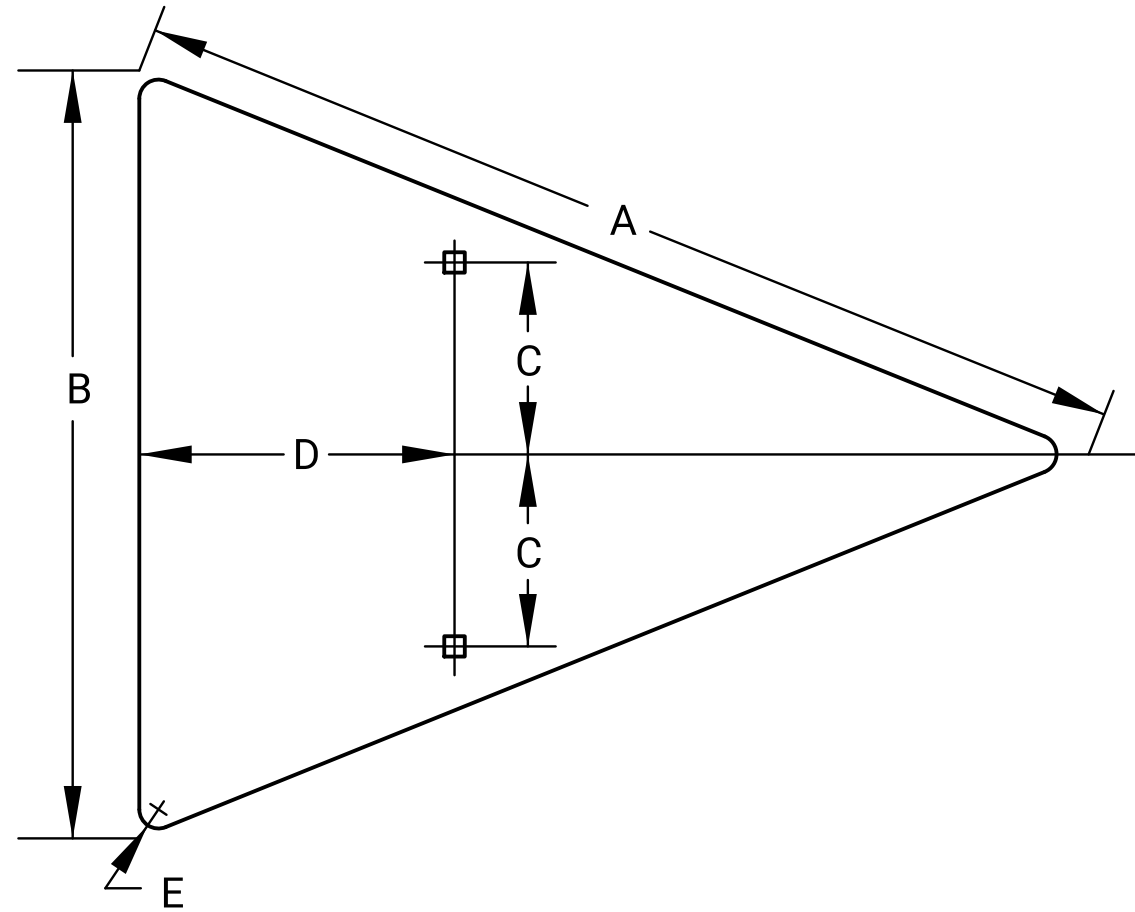
SIGN SIZE	A	B	C	D	E	F	G	T	AREA
48 X 48	48	48	12	24	9	30	24	0.100	13.25



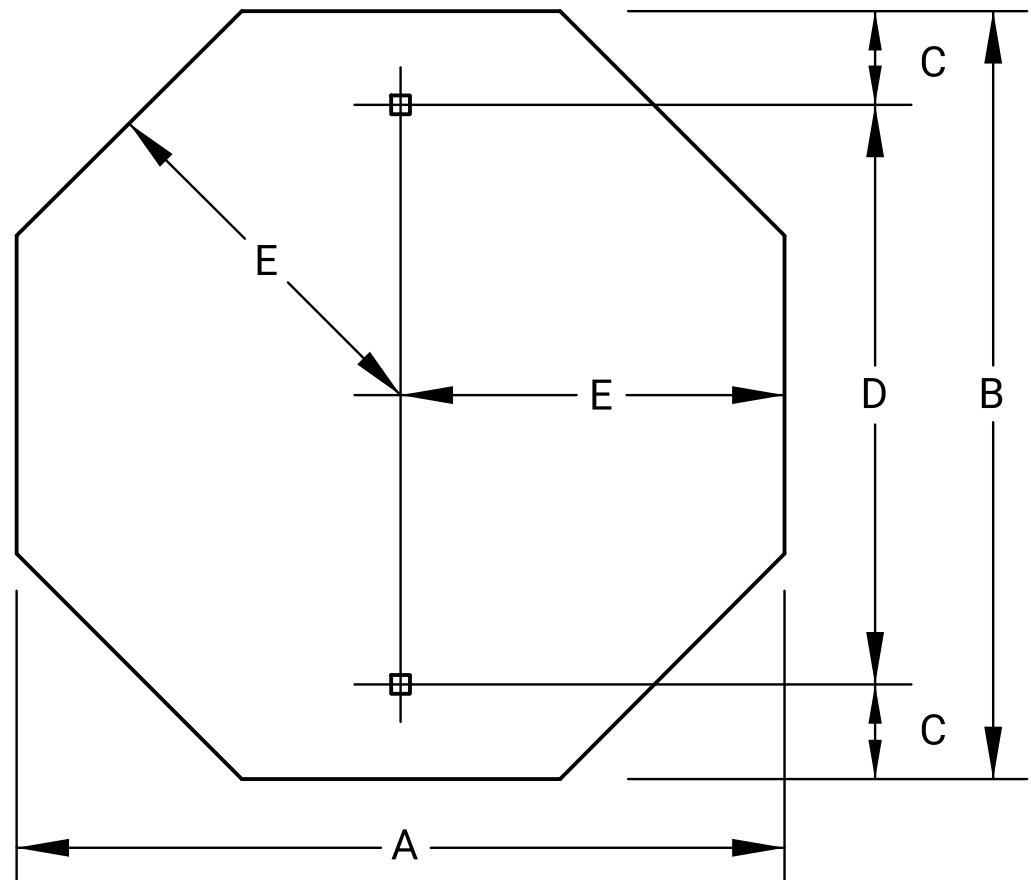
SIGN SIZE	A	B	C	D	T	AREA
36 X 36	36	3	18	2	0.080	3.90



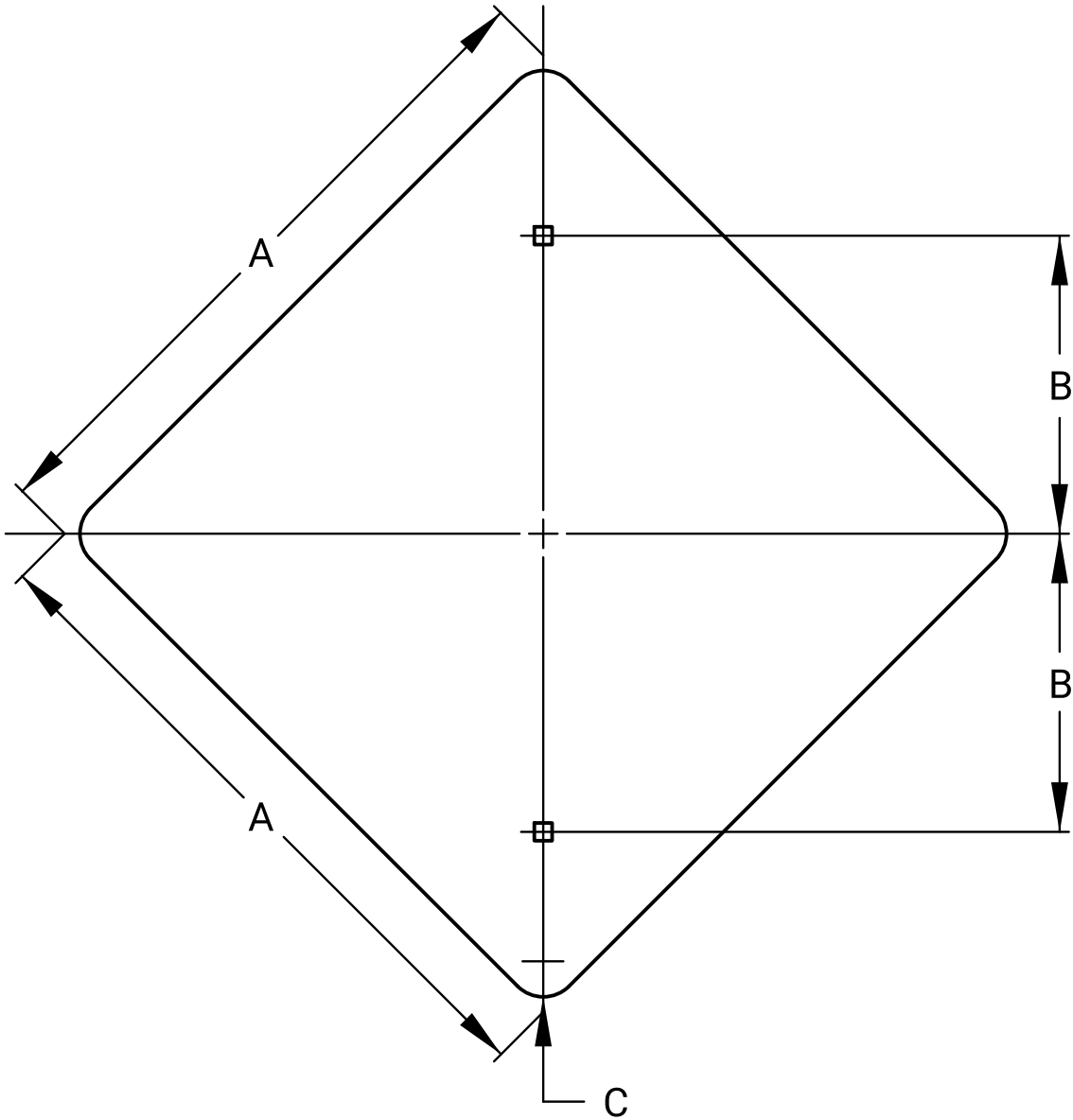
SIGN SIZE	A	B	C	D	E	T	AREA
48 X 48	48	3	12	18	3	0.080	6.93
60 X 60	60	3	18	18	4	0.100	10.83



SIGN SIZE	A	B	C	D	E	T	AREA
48 X 36	48	36	9	14 3/4	2 1/4	0.125	5.56

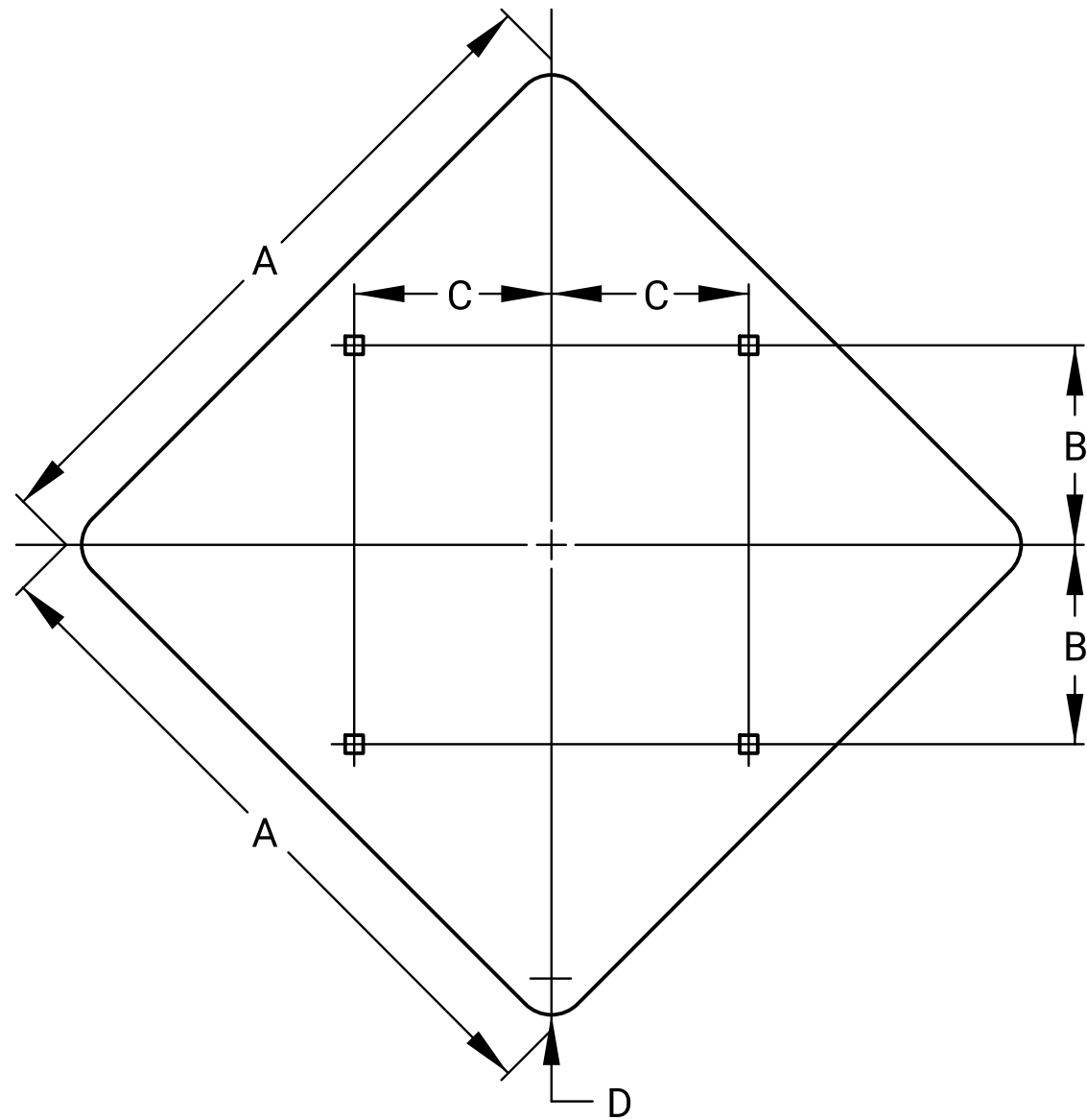


SIGN SIZE	A	B	C	D	E	T	AREA
30 X 30	30	30	3	24	15	0.080	5.18
36 X 36	36	36	6	24	18	0.080	7.46



①

SIGN SIZE	A	B	C	T	AREA
18 X 18	18	6	1 1/2	0.080	2.25
24 X 24	24	12	1 1/2	0.080	4.00
30 X 30	30	12	1 7/8	0.080	6.25
36 X 36	36	18	2 1/4	0.080	9.00



SIGN SIZE	A	B	C	D	T	AREA
48 X 48	48	12	15	3	0.100	16.00

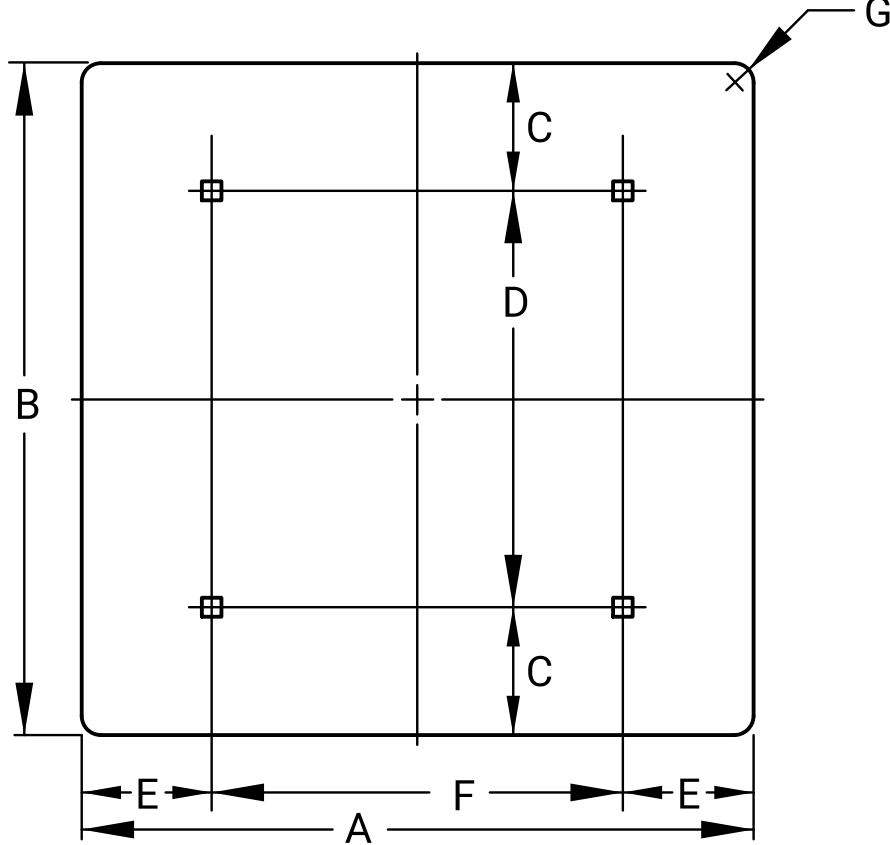
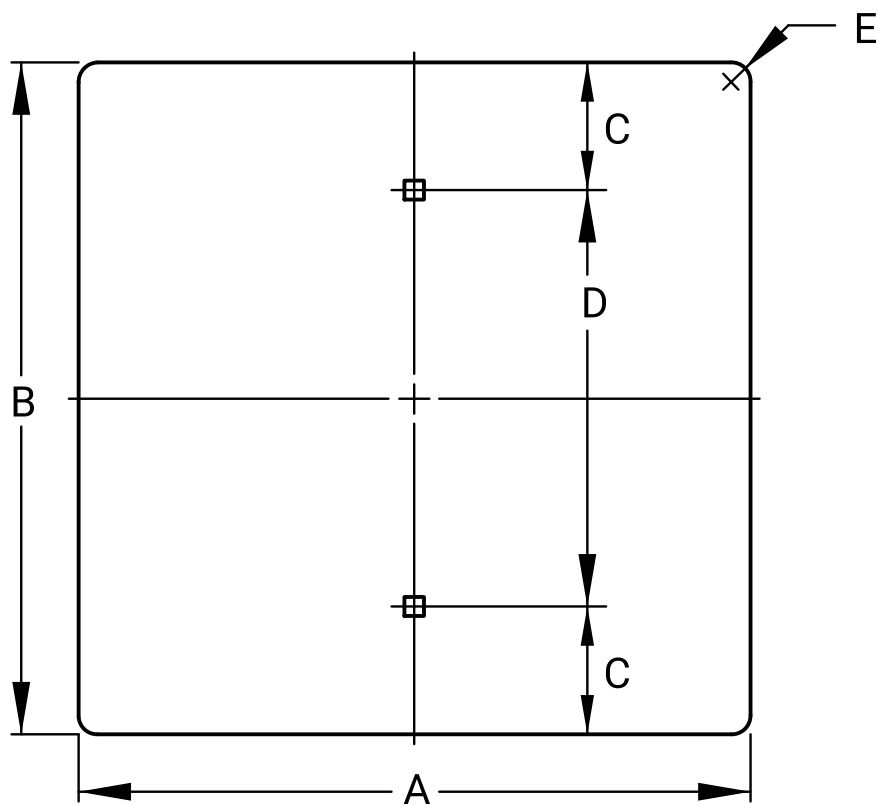
NOTE:  
All holes are 3/8 " square unless otherwise noted.

The dimension "t" is the thickness of the aluminum blank.

① Center hole is required.

All dimensions are in inches.

01	10-01-19	Update sign blank details and dimensions	D.D.G.	E.W.N.	
NO.	DATE	REVISIONS	BY	APPD	
KANSAS DEPARTMENT OF TRANSPORTATION					
SIGN BLANK DETAILS FOR FLAT SHEET SIGNS					
TE503			07-01-03		
FHWA APPROVAL		10-01-19		APPD.	
DESIGNED	D.D.G.	DETAILED	A.A.D.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.



	SIGN SIZE	A	B	C	D	E	T	AREA
①	3 X 8	3	8	1	6	$\frac{3}{8}$	0.040	0.17
①	6 X 12	6	12	3	6	$\frac{3}{8}$	0.063	0.50
	12 X 6	12	6	$1\frac{1}{2}$	3	$\frac{3}{4}$	0.063	0.50
	12 X 9	12	9	$1\frac{1}{2}$	6	$1\frac{1}{2}$	0.063	0.75
	12 X 18	12	18	3	12	$1\frac{1}{2}$	0.063	1.50
	12 X 24	12	24	3	18	$1\frac{1}{2}$	0.080	2.00
	12 X 36	12	36	6	24	$1\frac{1}{2}$	0.080	3.00
	12 X 48	12	48	6	36	$1\frac{1}{2}$	0.080	4.00
	18 X 6	18	6	$1\frac{1}{2}$	3	$1\frac{1}{2}$	0.063	0.75
	18 X 18	18	18	3	12	$1\frac{1}{2}$	0.063	2.25
	18 X 30	18	24	3	24	$1\frac{1}{2}$	0.080	3.75
	18 X 36	18	24	6	24	$1\frac{1}{2}$	0.080	4.50
	18 X 42	18	24	6	30	$1\frac{1}{2}$	0.080	5.25
	18 X 48	18	24	6	36	$1\frac{1}{2}$	0.080	6.00
	21 X 15	21	15	$1\frac{1}{2}$	12	$1\frac{1}{2}$	0.080	2.19
	24 X 6	24	6	$1\frac{1}{2}$	3	$1\frac{1}{2}$	0.080	1.00
	24 X 12	24	12	3	6	$1\frac{1}{2}$	0.080	2.00
	24 X 18	24	18	3	12	$1\frac{1}{2}$	0.080	3.00
	24 X 24	24	24	3	18	$1\frac{1}{2}$	0.080	4.00
	24 X 30	24	30	3	24	$1\frac{1}{2}$	0.080	5.00
	24 X 36	24	36	6	24	$1\frac{1}{2}$	0.080	6.00
	30 X 12	30	12	3	6	$1\frac{7}{8}$	0.080	2.50
	30 X 15	30	15	$1\frac{1}{2}$	12	$1\frac{7}{8}$	0.080	3.13
	30 X 18	30	18	3	12	$1\frac{7}{8}$	0.080	3.75
	30 X 21	30	21	$1\frac{1}{2}$	18	$1\frac{1}{2}$	0.080	4.38
	30 X 24	30	24	3	18	$1\frac{7}{8}$	0.080	5.00
	30 X 30	30	30	3	24	$1\frac{7}{8}$	0.080	6.25
	30 X 36	30	36	6	24	$1\frac{7}{8}$	0.080	7.50
	36 X 12	36	12	3	6	$1\frac{1}{2}$	0.080	3.00
	36 X 18	36	18	3	12	$1\frac{1}{2}$	0.080	4.50
	36 X 24	36	24	3	18	$1\frac{1}{2}$	0.080	6.00
	36 X 30	36	30	3	24	$2\frac{1}{4}$	0.080	7.50
	36 X 36	36	36	6	24	$2\frac{1}{4}$	0.080	9.00
③	45 X 36	45	36	3	30	$2\frac{1}{4}$	0.100	11.25

	SIGN SIZE	A	B	C	D	E	F	G	T	AREA
	36 X 12	36	12	3	6	3	30	$1\frac{1}{2}$	0.080	3.00
	36 X 30	36	30	3	24	3	30	$2\frac{1}{4}$	0.080	7.50
	36 X 48	36	48	9	30	6	24	0	0.100	12.00
	36 X 60	36	60	12	36	6	24	0	0.100	15.00
②	36 X 72	36	72	6	60	6	24	0	0.100	18.00
	42 X 12	48	12	3	6	6	30	$1\frac{1}{2}$	0.080	3.50
	42 X 18	48	18	3	12	6	30	$1\frac{1}{2}$	0.080	5.25
	42 X 24	48	24	6	12	6	30	$1\frac{7}{8}$	0.080	7.00
	42 X 36	48	36	6	24	6	30	0	0.100	10.50
	48 X 12	48	12	3	6	9	30	$1\frac{1}{2}$	0.080	4.00
	48 X 18	48	18	3	12	9	30	$1\frac{1}{2}$	0.080	6.00
	48 X 24	48	24	6	12	9	30	$1\frac{7}{8}$	0.080	8.00
	48 X 30	48	30	6	18	9	30	0	0.100	10.00
	48 X 36	48	36	6	24	9	30	0	0.100	12.00
	48 X 42	48	42	6	30	9	30	0	0.100	14.00
	48 X 48	48	48	9	30	9	30	0	0.100	16.00
②	48 X 60	48	60	12	36	9	30	0	0.100	20.00
②	48 X 72	48	72	6	60	9	30	0	0.100	24.00
②	48 X 96	48	96	12	72	9	30	0	0.100	32.00
	60 X 12	60	12	3	6	12	36	0	0.100	5.00

	SIGN SIZE	A	B	C	D	E	F	G	T	AREA
	60 X 18	60	18	3	12	12	36	0	0.100	7.50
	60 X 24	60	24	6	12	12	36	0	0.100	10.00
	60 X 30	60	30	6	18	12	36	0	0.100	12.50
	60 X 36	60	36	6	24	12	36	0	0.100	15.00
	60 X 42	60	42	6	30	12	36	0	0.100	17.50
	60 X 48	60	48	9	30	12	36	0	0.100	20.00
	72 X 12	72	12	3	6	15	42	0	0.100	6.00
	72 X 18	72	18	3	12	15	42	0	0.100	9.00
	72 X 24	72	24	6	12	15	42	0	0.100	12.00
	72 X 30	72	30	6	18	15	36	0	0.100	15.00
	72 X 36	72	36	6	24	15	42	0	0.100	18.00
	72 X 42	72	42	6	30	15	42	0	0.100	21.00
	72 X 48	72	48	9	30	15	42	0	0.100	24.00
	84 X 12	84	18	3	6	18	48	0	0.100	7.00
	84 X 18	84	18	3	12	18	48	0	0.100	10.50
	84 X 24	84	24	6	12	18	48	0	0.100	14.00
	84 X 30	84	30	6	18	18	48	0	0.100	17.50
	84 X 36	84	36	6	24	18	48	0	0.100	21.00
	84 X 42	84	42	6	30	18	48	0	0.100	24.50
	84 X 48	84	48	9	30	18	48	0	0.100	28.00

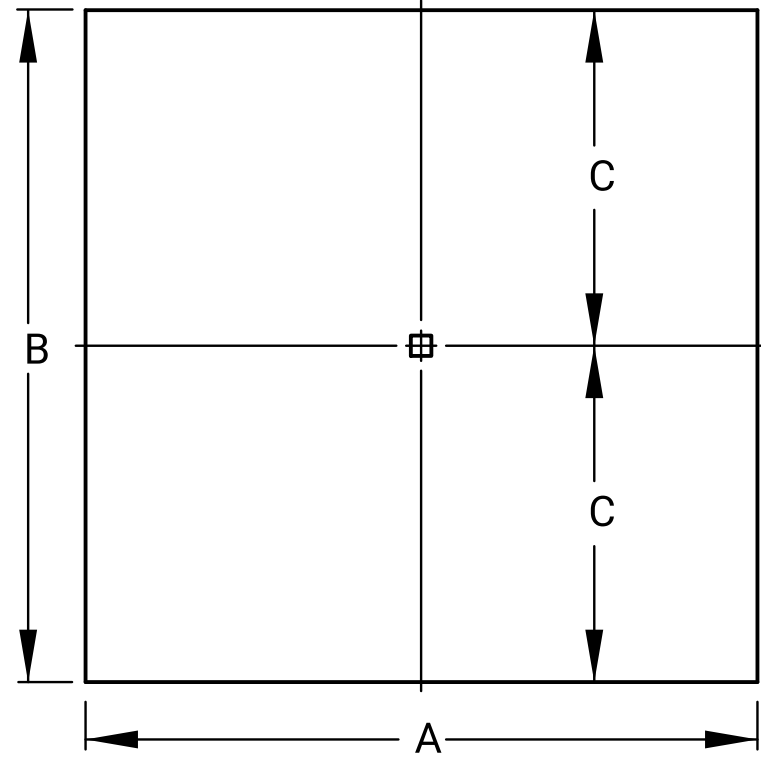
- NOTE:
- All holes are  $\frac{3}{8}$ " square, unless otherwise noted.
- The dimension "T" is the thickness of the aluminum blank.
- ① Holes shall be  $\frac{5}{16}$ " diameter.
- ② Dimension "D" requires a center hole.
- ③ Additional hole 12" below top hole.

All dimensions are in inches.

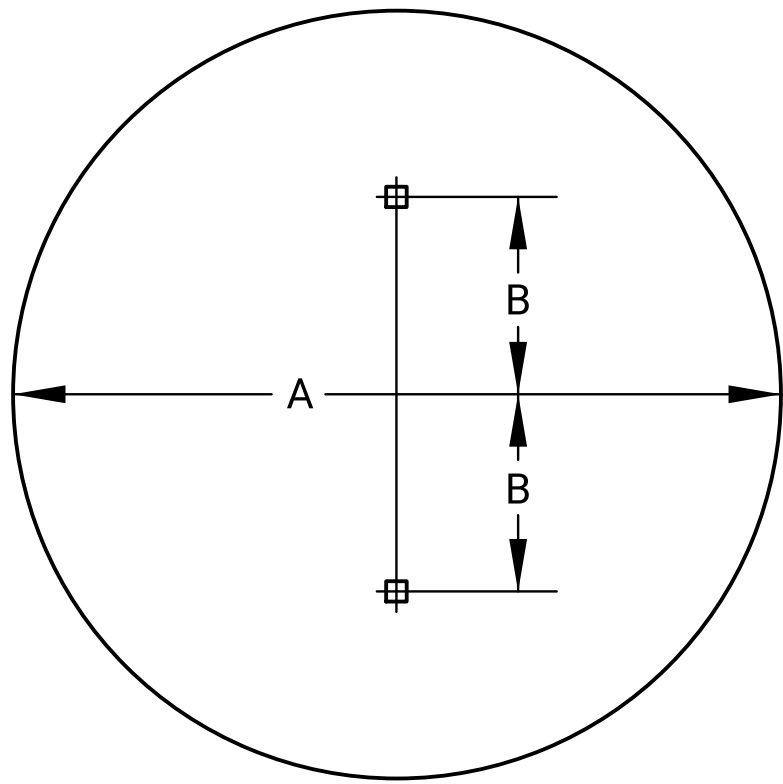
01	10-01-19	Updated sign blank details and dimensions			D.D.G. E.W.N.
NO.	DATE	REVISIONS			BY APPD
KANSAS DEPARTMENT OF TRANSPORTATION					
SIGN BLANK DETAILS FOR FLAT SHEET SIGNS					
TE506				07-01-03	
FHWA APPROVAL		10-01-19	APP'D.	Steven A. Buckley	
DESIGNED	D.D.G.	DETAILED	A.A.D.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.

Plotted by : austin.loyd@ks.gov  
File : KA572901pss509-01.dgn  
16-JUL-2024 20:37

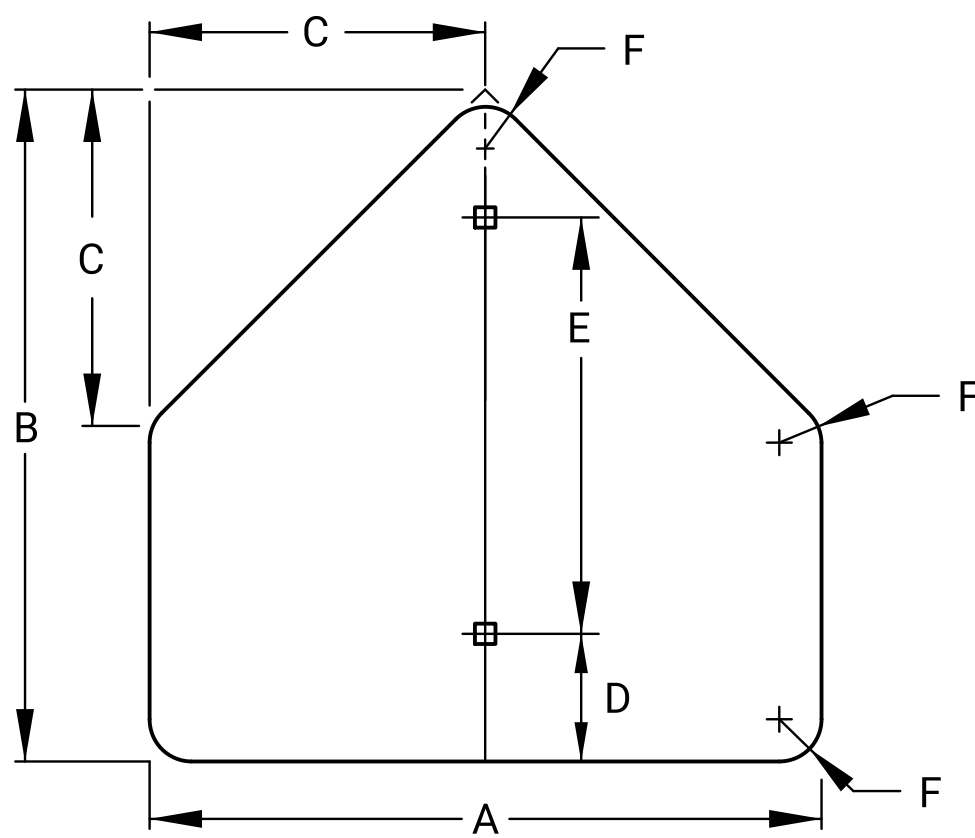
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	99	141



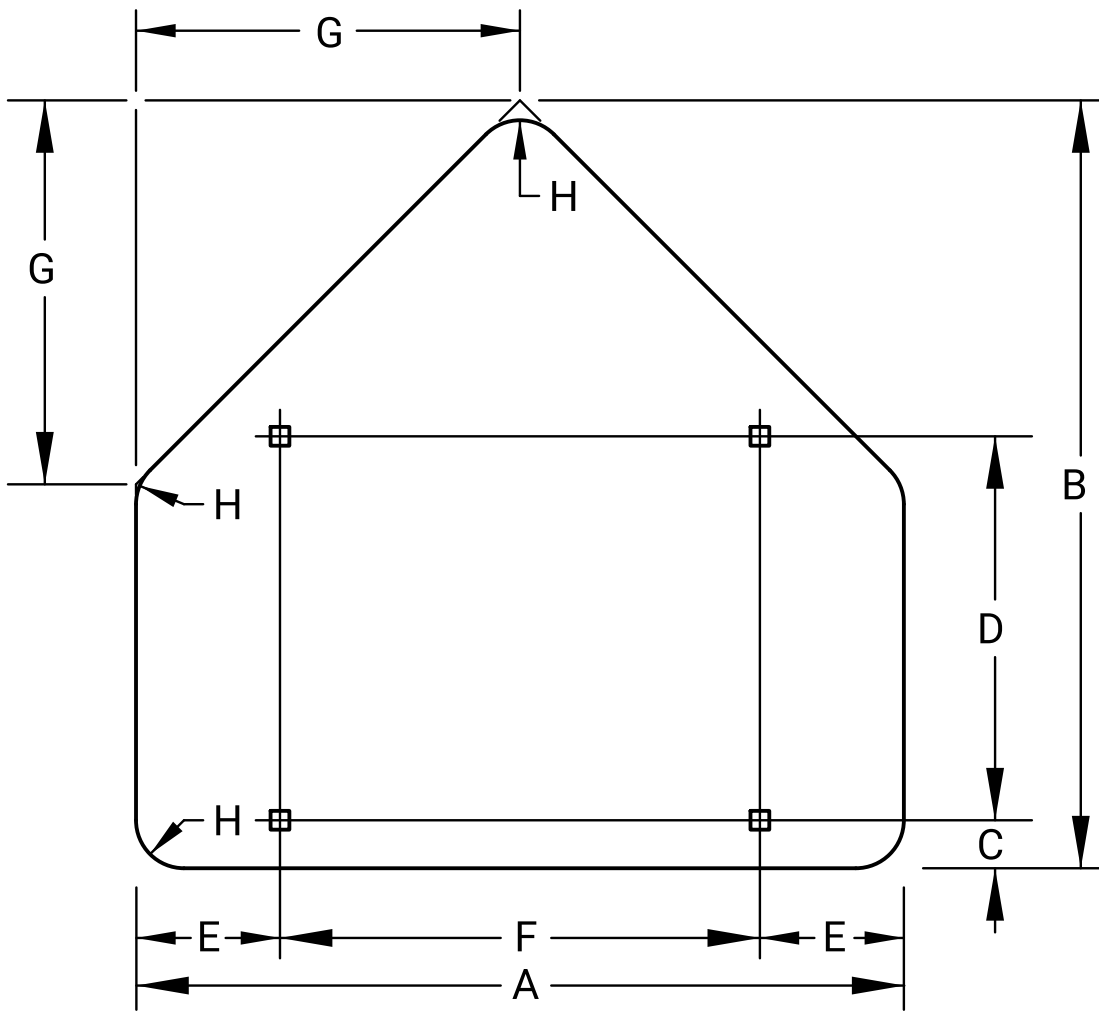
SIGN SIZE	A	B	C	T	AREA
6 X 6	6	6	3	0.063	0.25



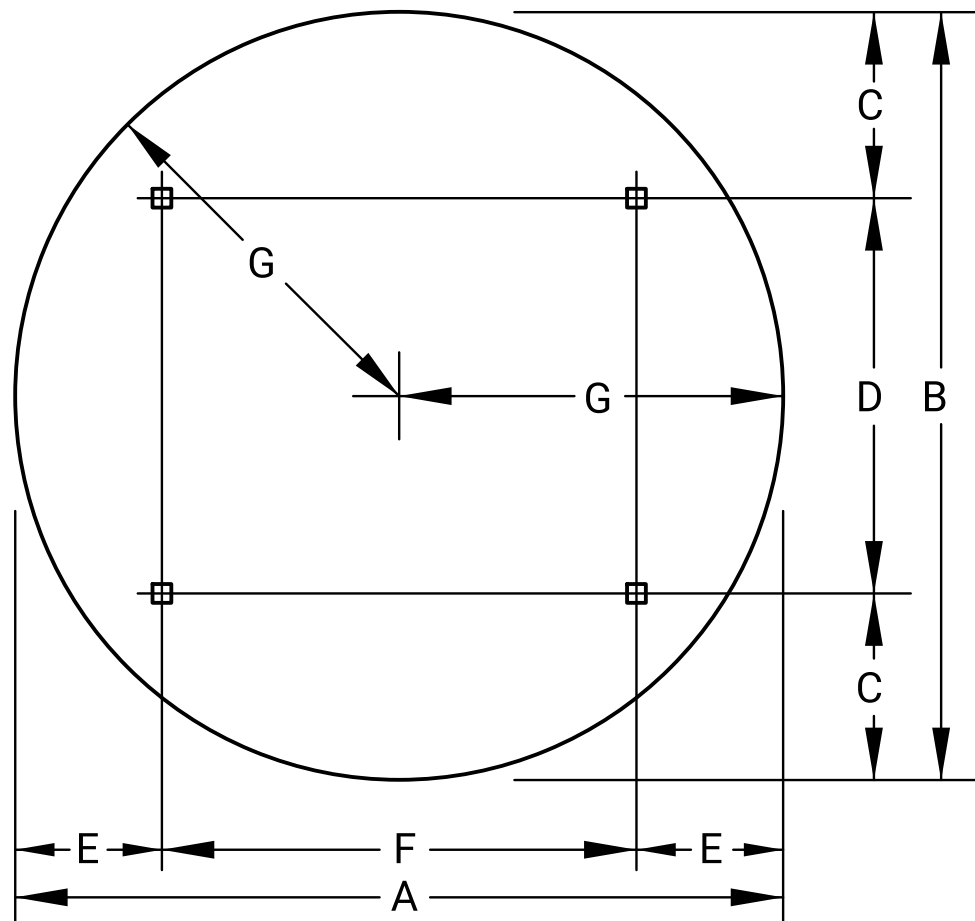
SIGN SIZE	A	B	T	AREA
36 DIA	36	12	0.080	7.07



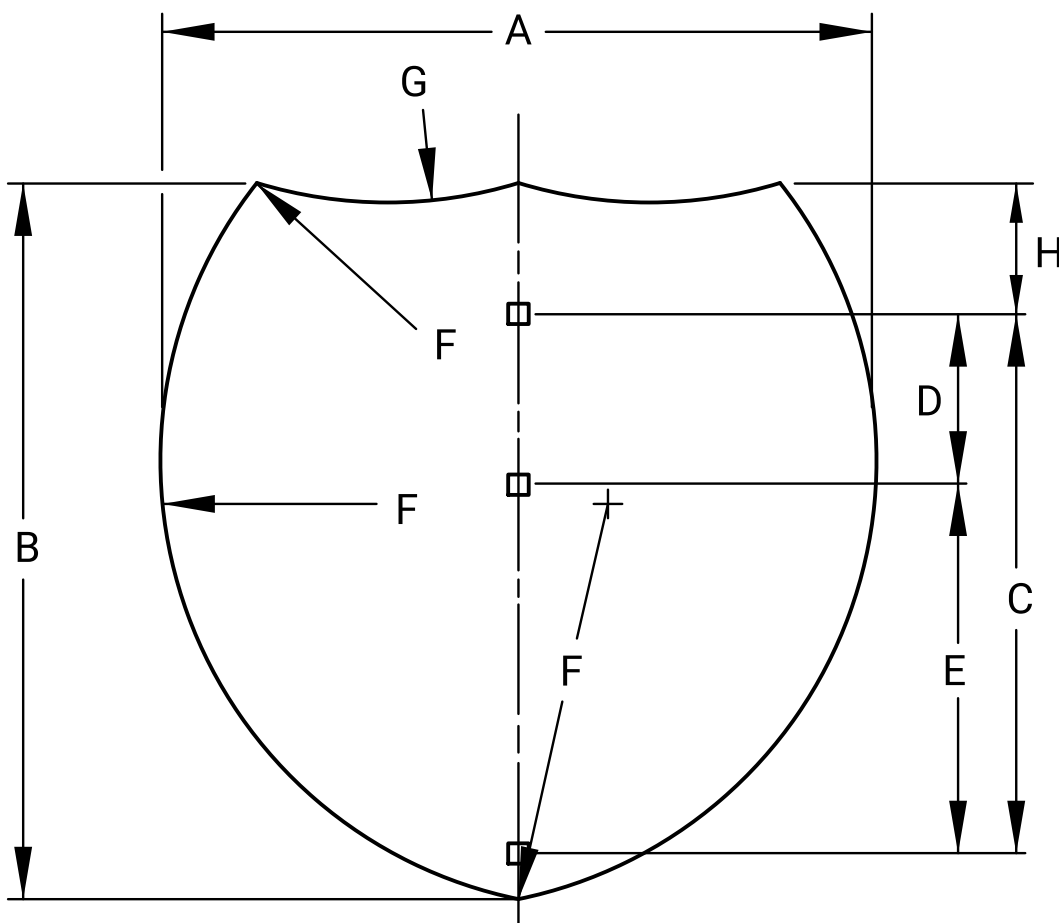
SIGN SIZE	A	B	C	D	E	F	T	AREA
30 X 30	30	30	15	3	24	1 7/8	0.080	4.69
36 X 36	36	36	18	6	24	2 1/4	0.080	6.75



SIGN SIZE	A	B	C	D	E	F	G	H	T	AREA
48 X 48	48	48	3	24	9	30	24	3	0.100	12.00



SIGN SIZE	A	B	C	D	E	F	G	T	AREA
48 X 48	48	48	12	24	9	30	24	0.100	12.57



### INDEPENDENT USE

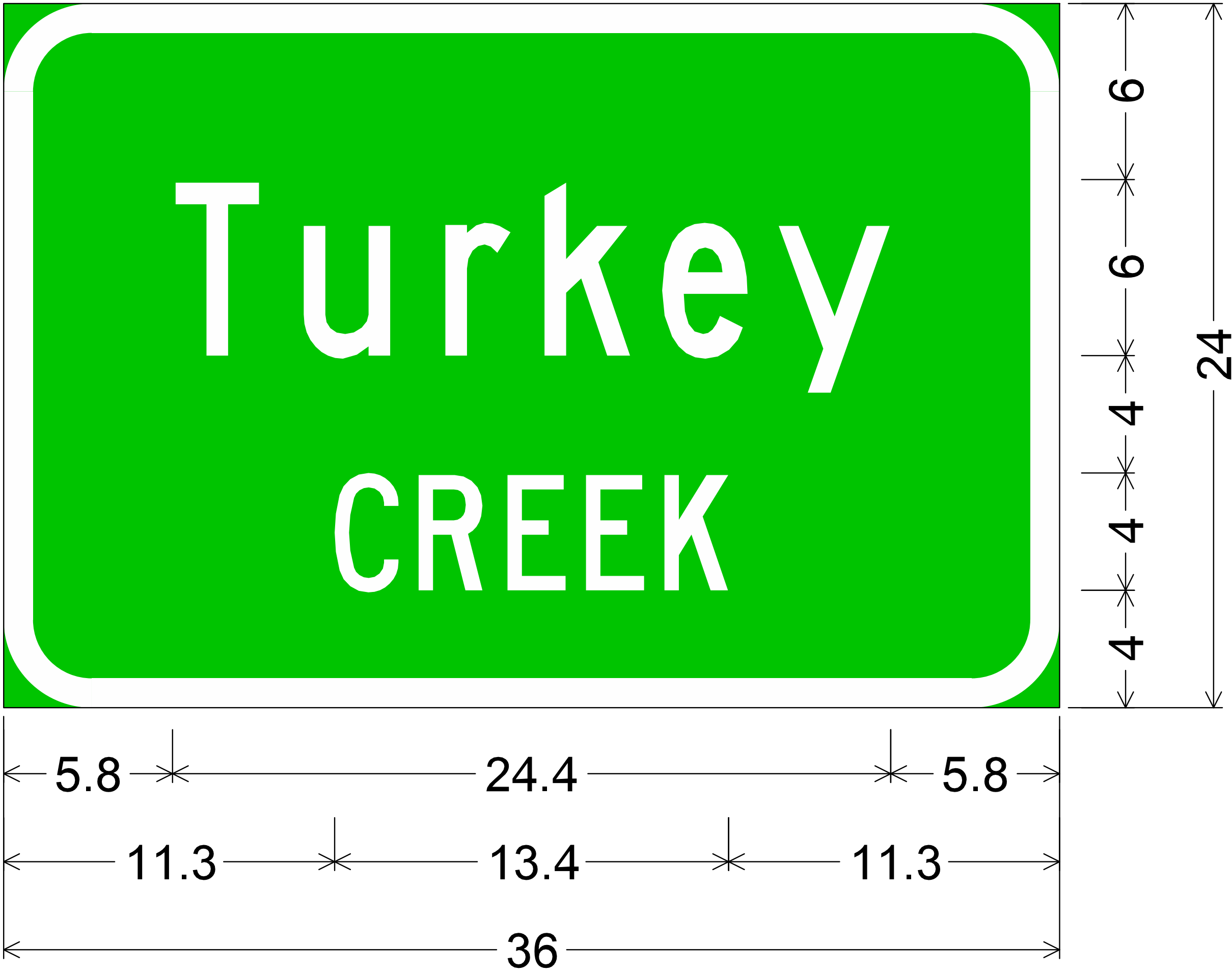
DIMENSIONS										
SIZE	A	B	C	D	E	F	G	H	T	AREA
24 X 24	24	24	18	-	-	15	15	3	0.080	3.20
36 X 36	36	36	30	12	18	22 1/2	22 1/2	3	0.080	7.20
30 X 24	30	24	18	-	-	17	24	3	0.080	3.99
45 X 36	45	36	30	12	18	25 1/2	36	3	0.100	8.99

NOTE:  
All holes are 3/8" square, unless otherwise noted.  
Dimension "T" is the thickness of the aluminum blank.

All dimensions are in inches.

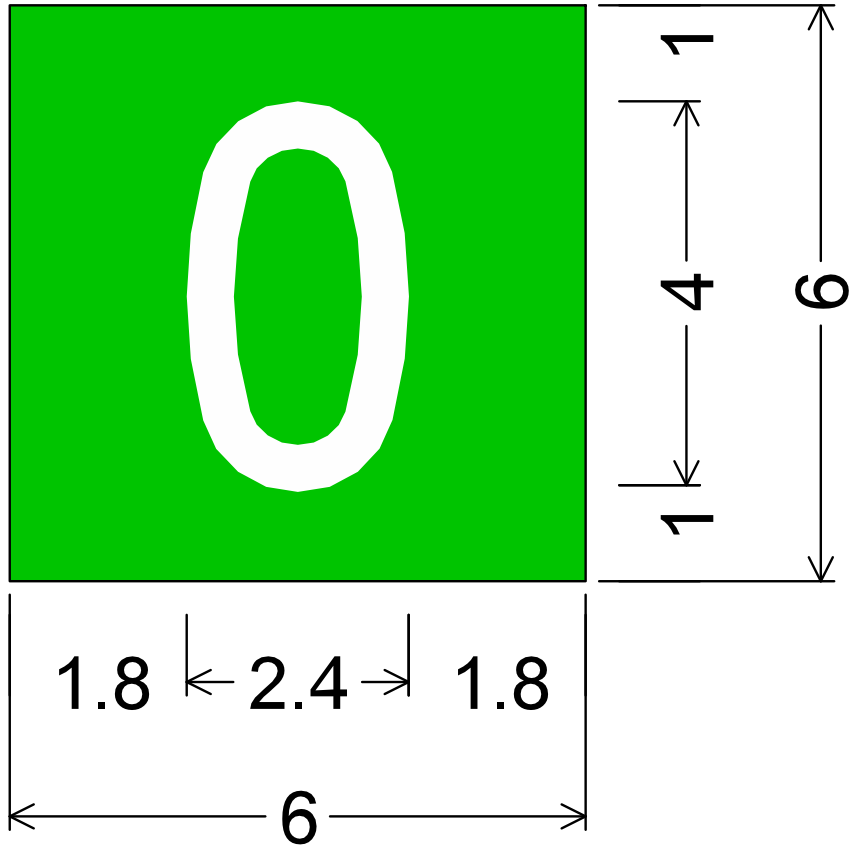
01	10-01-19	Updated sign blank details and dimensions			D.D.G. E.W.N.
NO.	DATE	REVISIONS			BY APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
SIGN BLANK DETAILS FOR FLAT SHEET SIGNS					
TE509				07-01-03	
FHWA APPROVAL		10-01-19	APP'D.		Steven A. Buckley
DESIGNED	D.D.G.	DETAILED	A.A.D.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.





KI-3 Turkey Creek;  
3.0" Radius, 1.0" Border, White on Green;  
"Turkey", C; "CREEK", C;  
Table of letter and object lefts

T	u	r	k	e	y
5.8	10.2	15.1	18.4	22.4	26.4
C	R	E	E	K	
11.3	14.1	17.2	19.9	22.5	



KRM0 ;  
No border, None on Green;  
"0" White, C;  
Table of distances between letter and object lefts

0
1.8 2.4 1.8

NOTE: SIGNS AND LEGEND NOT TO SCALE

Plotted by : austin.loyd@ks.gov  
File : KA572901pss590-01.dgn  
16-JUL-2024 20:36

DETAILED SPECIFICATIONS FOR FLAT SHEET SIGNS AND OVERLAY PANELS

All new flat sheet sign blanks shall be of the fabrication and thickness shown on the flat sheet blank detail sheets, unless other details are shown in the plans.

Flat sheet blanks shall be used for signs that are less than or equal to 7'-0" in length and/or less than or equal to 4'-0" in height, unless other details are shown in the plans. Flat sheet blanks shall also be used for signs that are 4'-0" in length and less than or equal to 8'-0" in height, unless other details are shown in the plans.

The design details for signs (color, letter height, and letter series) shall be as shown in the FHWA Standard Highway Signs and Markings book (2004 edition and supplements), unless other details are shown in the plans.

All sign faces shall be covered with Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The sheeting used for the direct applied legend and borders shall be Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The school warning signs, the "SCHOOL" portion of the S5-1 sign, S4-3p plaque, and any supplemental plaques used with these warning signs shall have a fluorescent yellow-green background, unless otherwise noted in the plans.

The type of adhesive used for retroreflective sheeting or lettering film shall be heat activated or pressure sensitive.

DETAILED SPECIFICATIONS FOR REINFORCED PANEL SIGNS

All new reinforced sign panels shall be of the fabrication and thickness shown on the reinforced panel detail sheets. If extrusheet fabricated sign panels are used, they shall be of the length, width and in the position shown. If extrusheet fabricated panel dimensions are not shown, a line of legend should be placed entirely on one panel. If extruded fabricated sign panels are used, either 1'-0" or 6" panels shall be used. The 6" panels shall be used only at the top or bottom of signs.

Reinforced panels shall be used for signs that are greater than 7'-0" in length or greater than 4'-0" in height, unless other details are shown in the plans.

All sign faces shall be covered with Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The sheeting used for the direct applied legend and borders shall be Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The type of adhesive used for retroreflective sheeting or lettering film shall be heat activated or pressure sensitive.

Letters and numbers on reinforced panel signs are modified Series "E" unless otherwise shown.

Spacing table dimensions are in inches.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	101	141

02	10-01-19	Changed notes	D.D.G.	E.W.N.	
01	07-23-10	Changed Notes and Sheeting Type	D.D.G.	D.B.	
NO.	DATE	REVISIONS	BY	APPD	
KANSAS DEPARTMENT OF TRANSPORTATION					
DETAILS SPECIFICATIONS FOR REINFORCED SIGN PANELS AND FLAT SHEET SIGNS					
TE590				07-01-03	
FHWA APPROVAL		10-01-19	APPD.	Steven A. Buckley	
DESIGNED	D.D.G.	DETAILED	K.D.S.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.



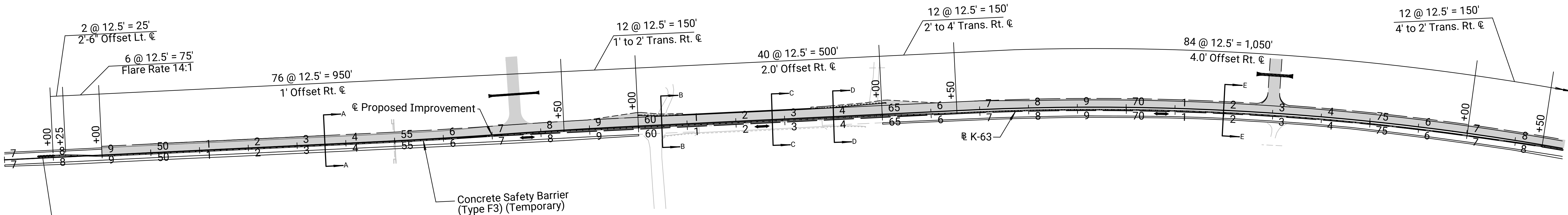
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	103	141

CONSTRUCTION NOTES

2. Place temporary concrete safety barrier from  $\text{Ct. Sta. } 48+00$  to  $\text{Ct. Sta. } 86+00$ .  
Construct proposed southbound K-63 to accomodate traffic during Phase 3.

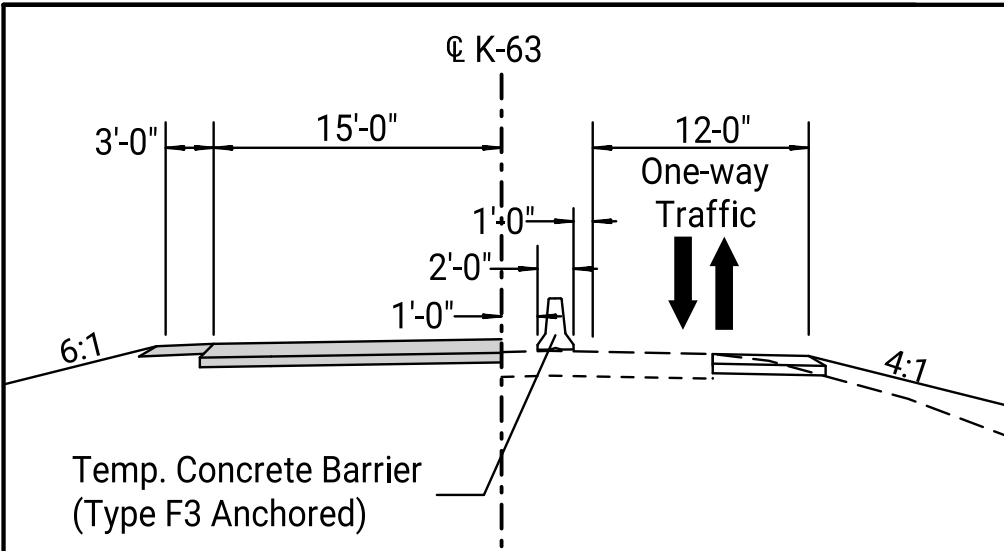
TRAFFIC HANDLING

2. Shift traffic to the widened northbound lane, using temporary signals to accomodate alternating one-way traffic.

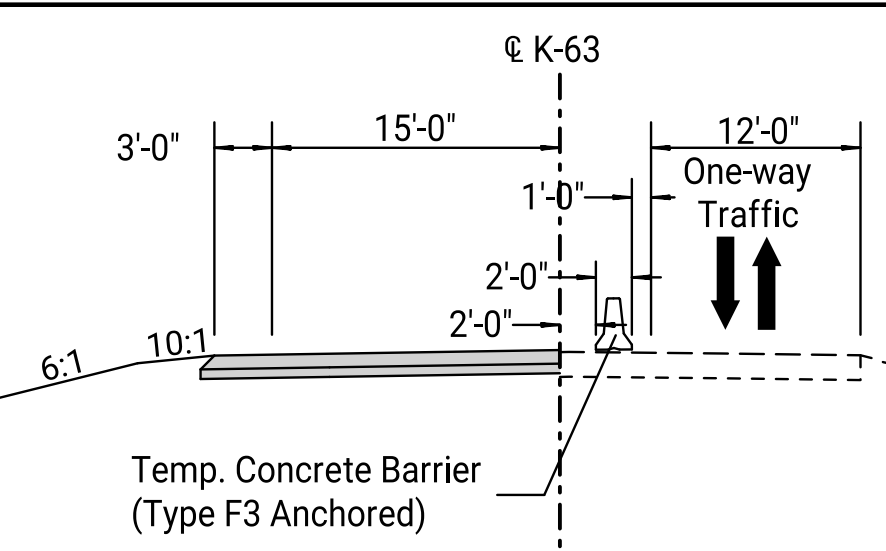


Impact Attenuator (TL-3) (Temporary)  
See Sh. No. 67, 123-124

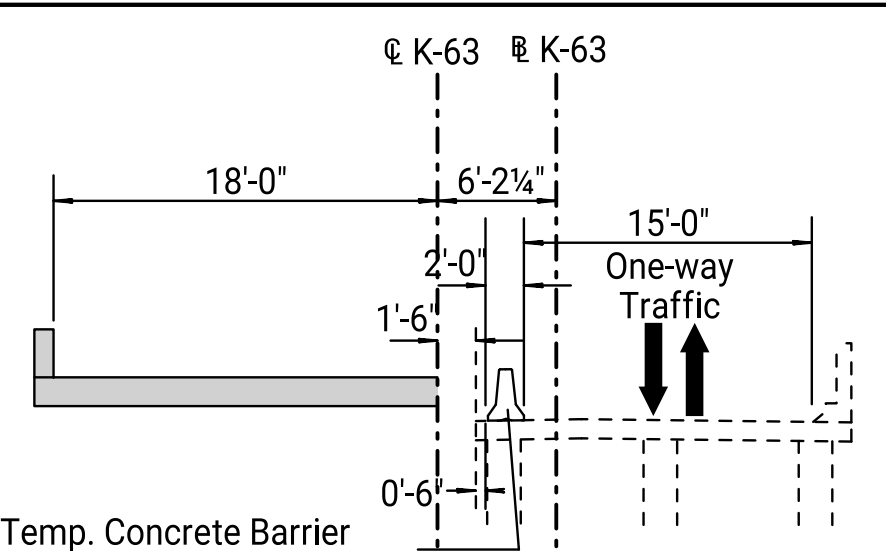
Concrete Safety Barrier (Type F3) (Temporary)



SECTION A-A



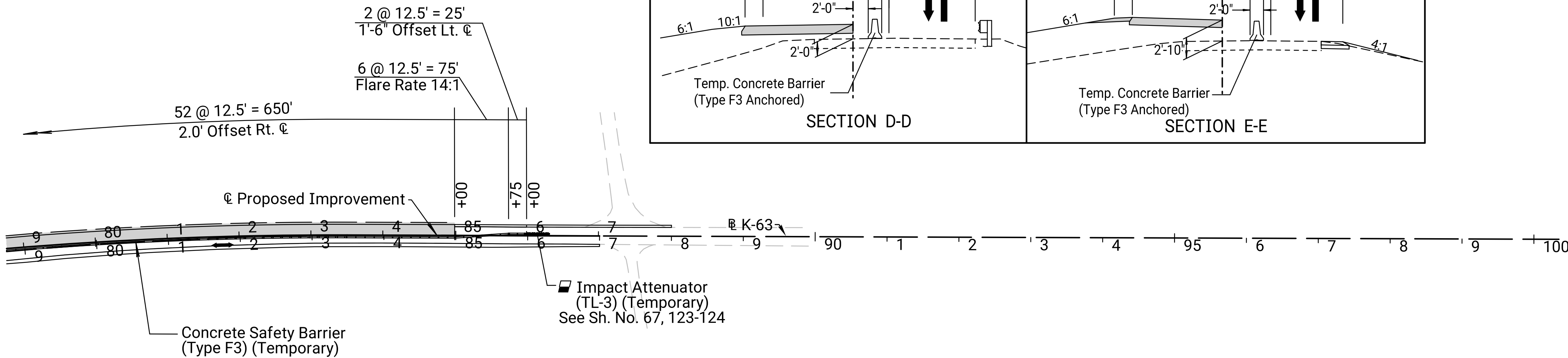
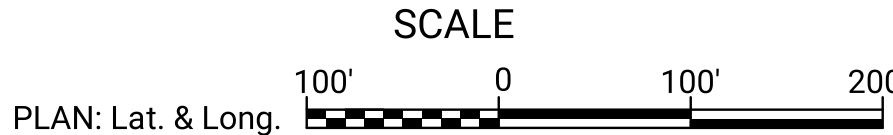
SECTION B-B



SECTION C-C  
See Sh. No. (38-39)

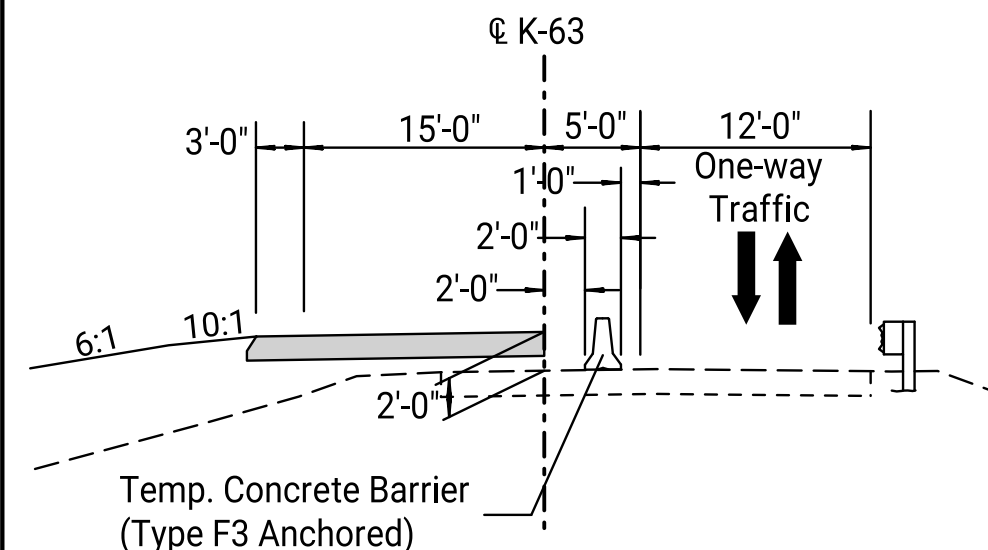
LEGEND

Construction During Phase

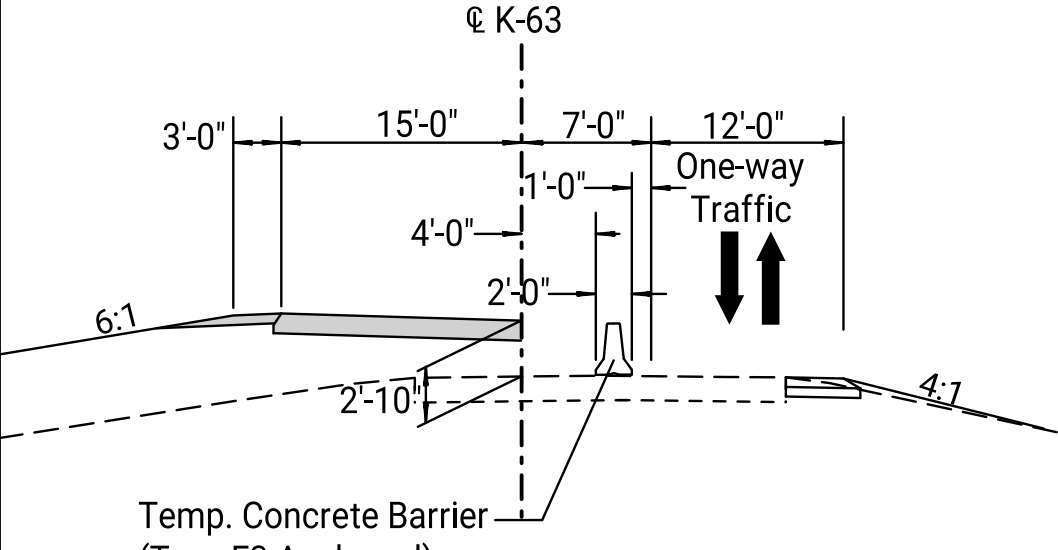


Concrete Safety Barrier (Type F3) (Temporary)

Impact Attenuator (TL-3) (Temporary)  
See Sh. No. 67, 123-124



SECTION D-D



SECTION E-E

Concrete Safety Barrier (Type F3) (Temporary) Phase 2				
Station to Station	Design Speed	Side	Type F3 (Free Standing)	Type F3 (Anchored)
48+00 to 86+00	60 mph	Rt.	0'	3,800

For information only  
To be relocated to Phase 3  
See Temporary Concrete Safety Barrier (Type F3) standards for anchorage and transition details.



KANSAS DEPARTMENT OF TRANSPORTATION  
CONSTRUCTION SEQUENCING  
PHASE 2



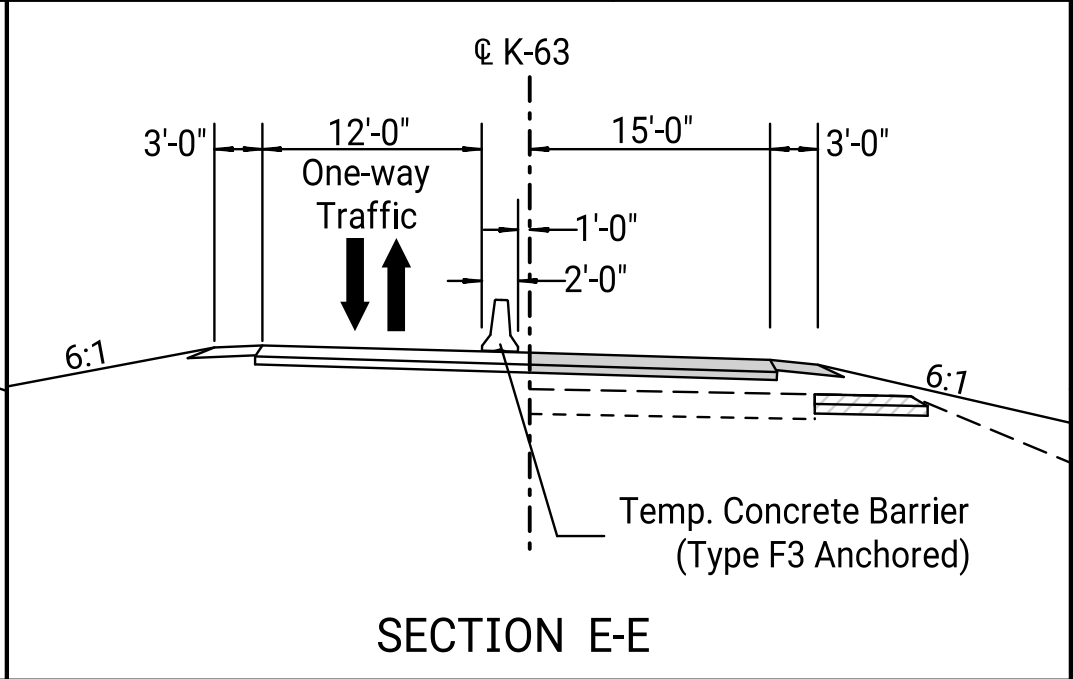
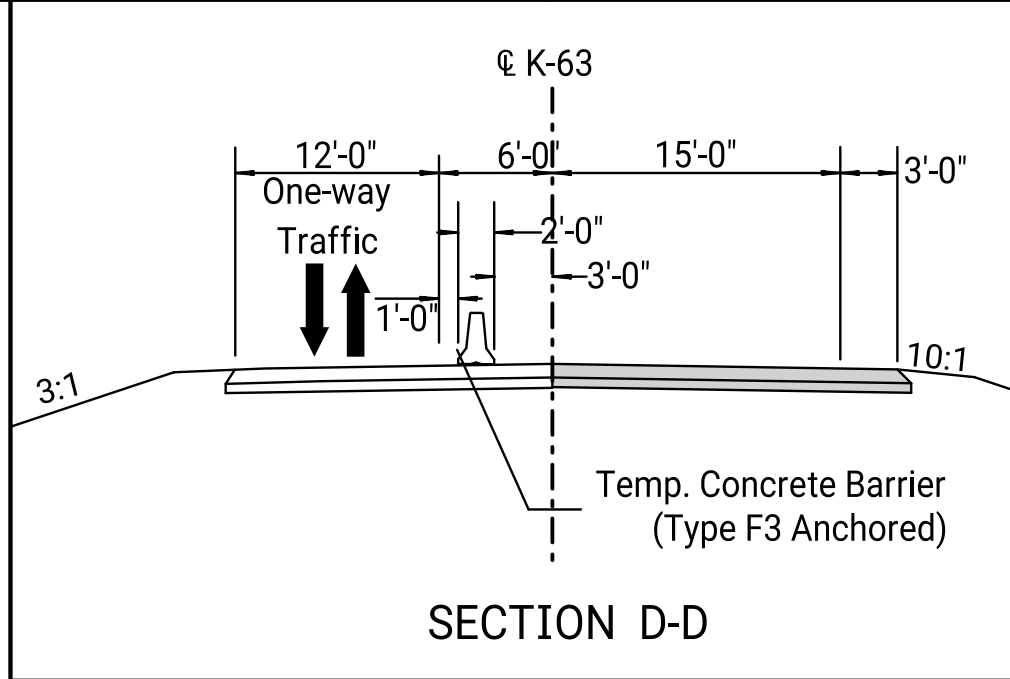
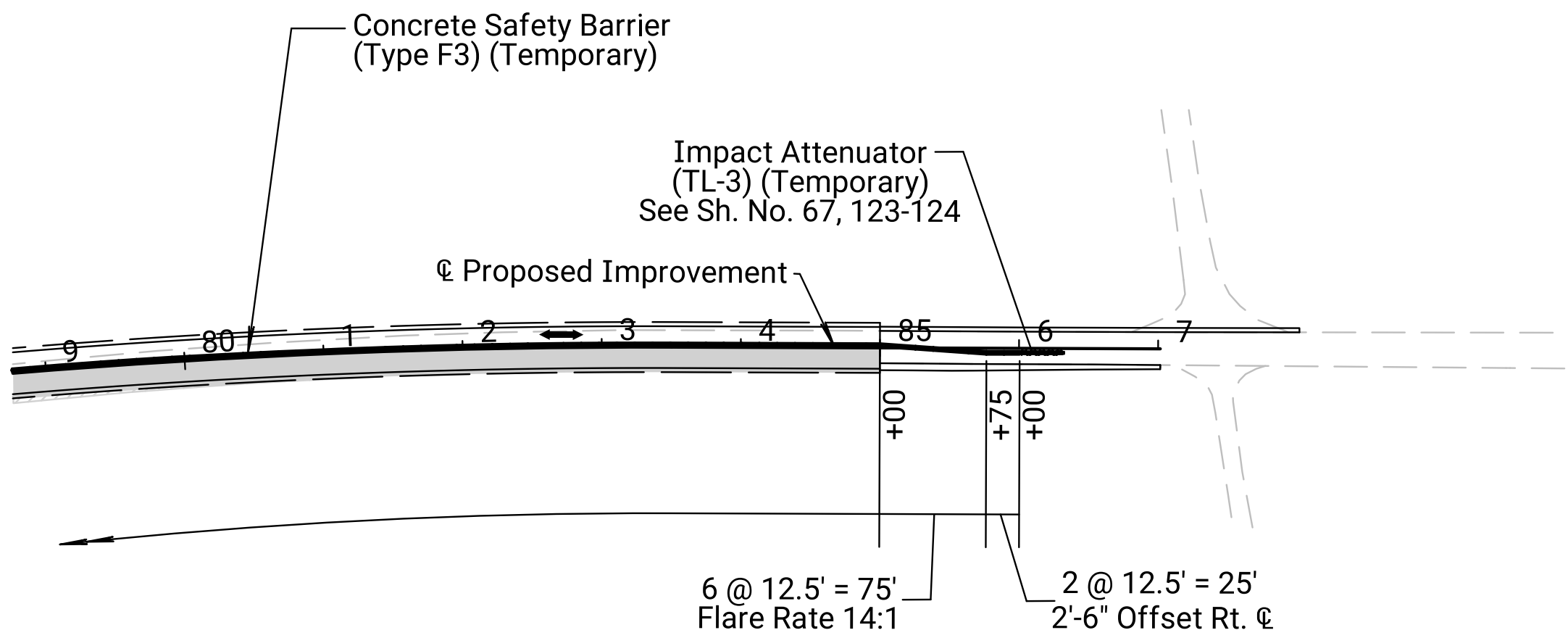
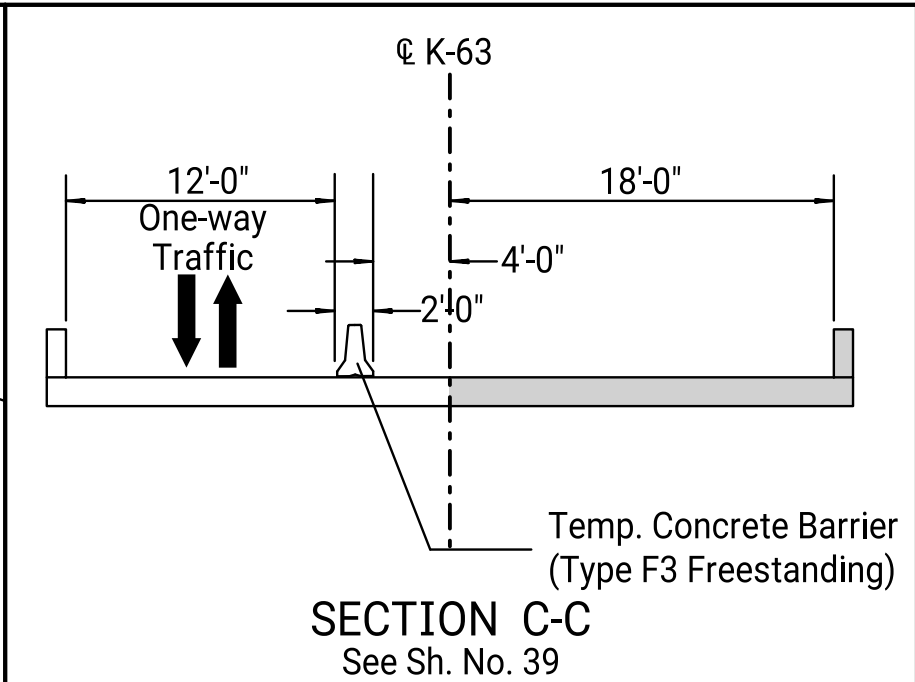
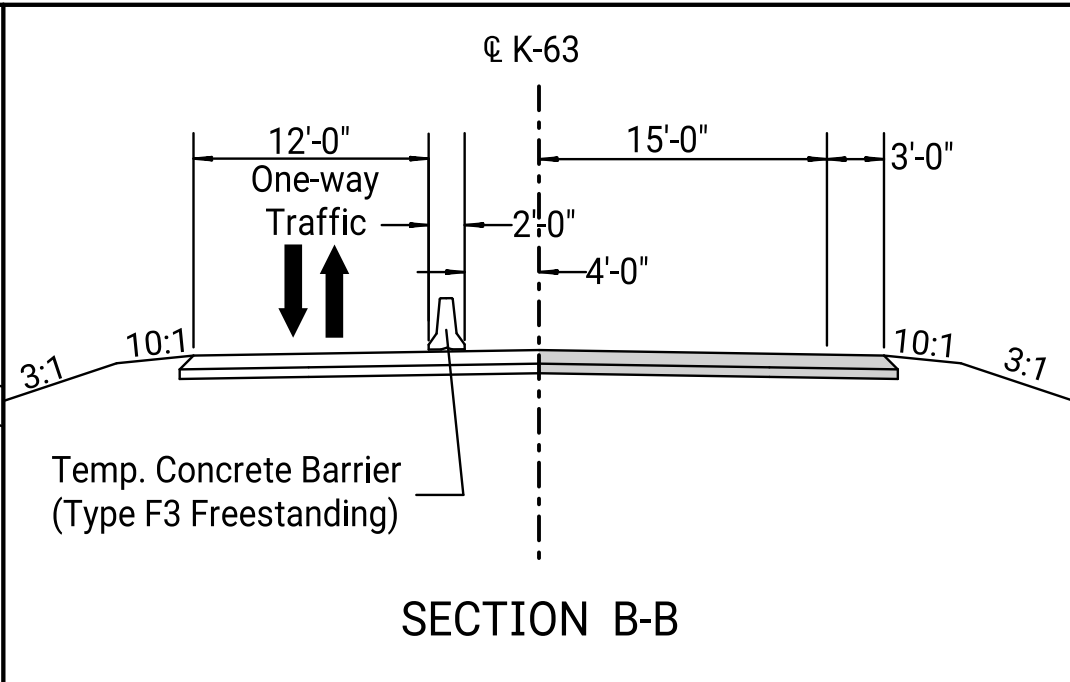
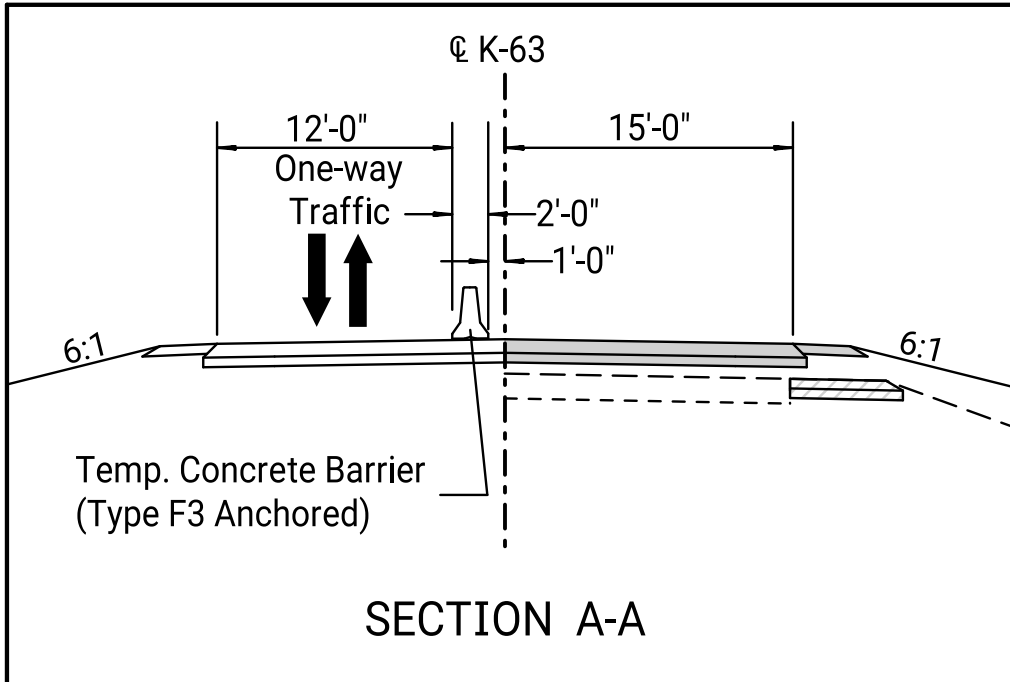
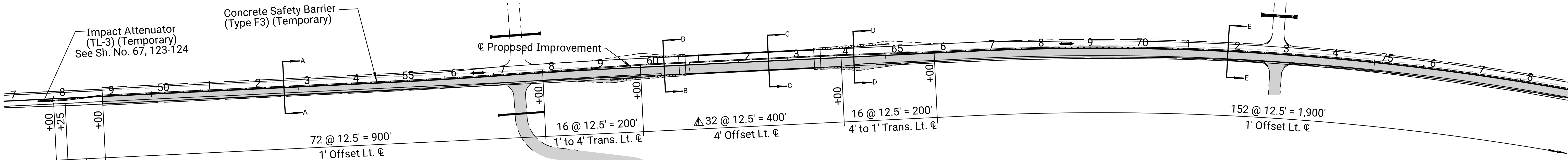
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	104	141

CONSTRUCTION NOTES

3. Relocate temporary concrete safety barrier and impact attenuators from Phase 2 and place barriers from  $\text{C Sta. } 48+00$  to  $\text{C Sta. } 86+00$ . Mill the temporary pavement from  $\text{C Sta. } 49+00$  Rt. to  $\text{C Sta. } 60+00$  Rt. and  $\text{C Sta. } 65+00$  Rt. to  $\text{C Sta. } 85+00$  Rt. Construct proposed northbound K-63. Temporary pavement from  $\text{C Sta. } 46+00$  to  $\text{C Sta. } 49+00$  and  $\text{C Sta. } 85+00$  to  $\text{C Sta. } 88+00$  shall remain.

TRAFFIC HANDLING

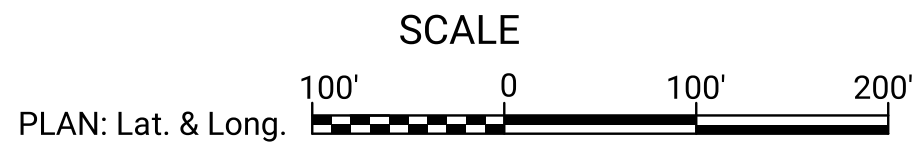
3. Shift traffic to the proposed southbound lane, using temporary signals to accomodate alternating one-way traffic.



LEGEND	
	Construction During Phase
	Removal During Phase

Concrete Safety Barrier (Type F3) (Temporary - Relocate) Phase 3				
Station to Station	Design Speed	Side	Type F3 (Free Standing)	Type F3 (Anchored)
48+00 to 86+00	60 mph	Lt.	400'	3,400'

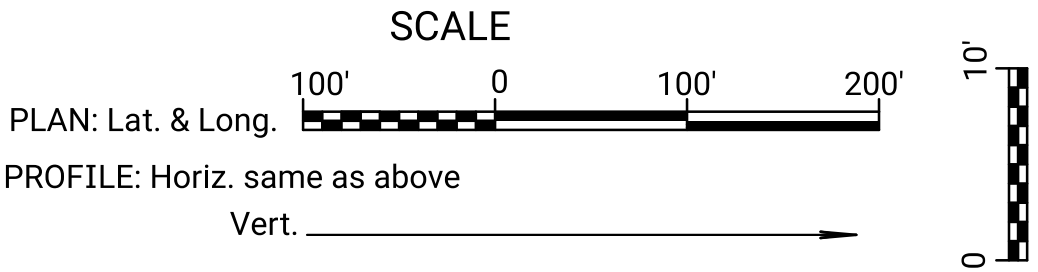
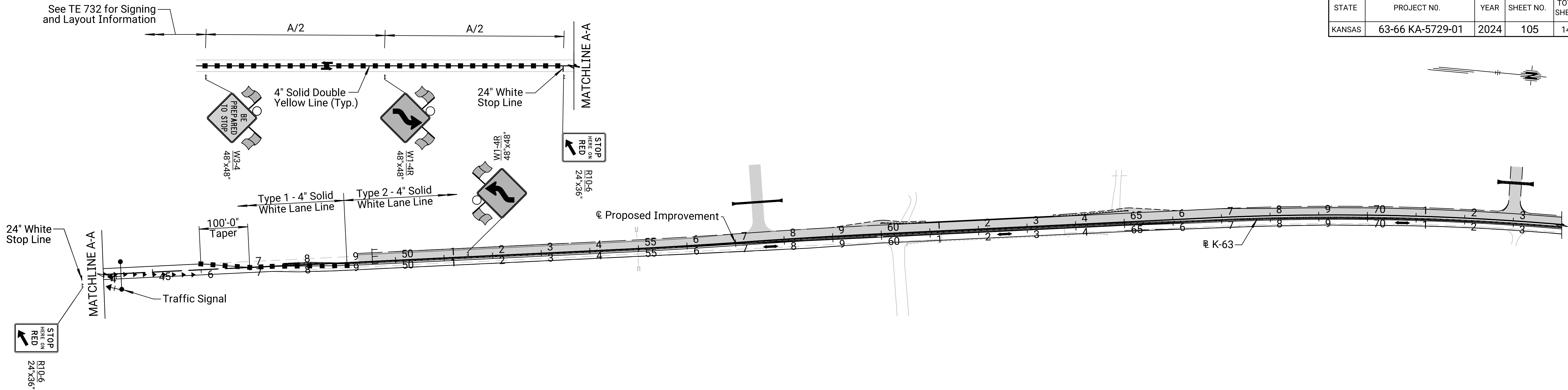
For information only  
Type F3 Concrete Safety Barrier (Freestanding)  
See Temporary Concrete Safety Barrier (Type F3) standards for anchorage and transition details.



KANSAS DEPARTMENT OF TRANSPORTATION  
CONSTRUCTION SEQUENCING  
PHASE 3

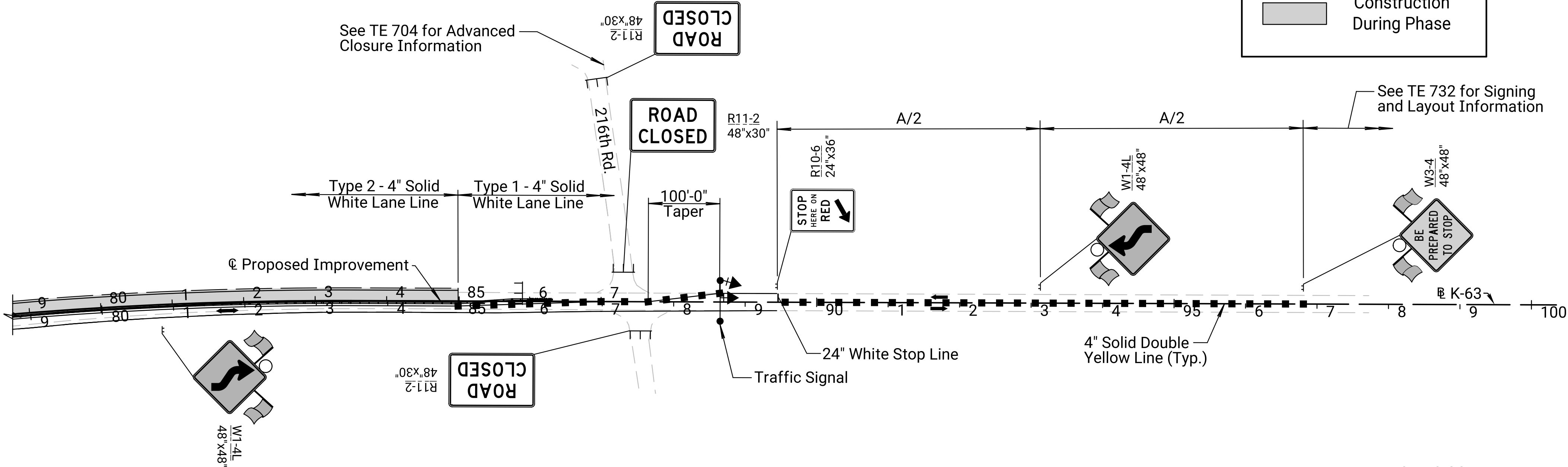
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	105	141

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED

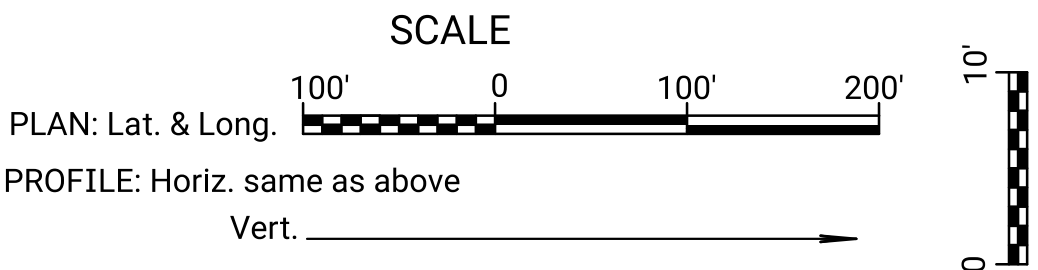


LEGEND

Construction During Phase



- NOT TO SCALE
- TYPE III BARRICADES
  - CHANNELIZING DEVICE
  - UNI-DIRECTIONAL TEMPORARY RAISED PAVEMENT MARKER
  - TYPE "A" LOW INTENSITY WARNING LIGHT



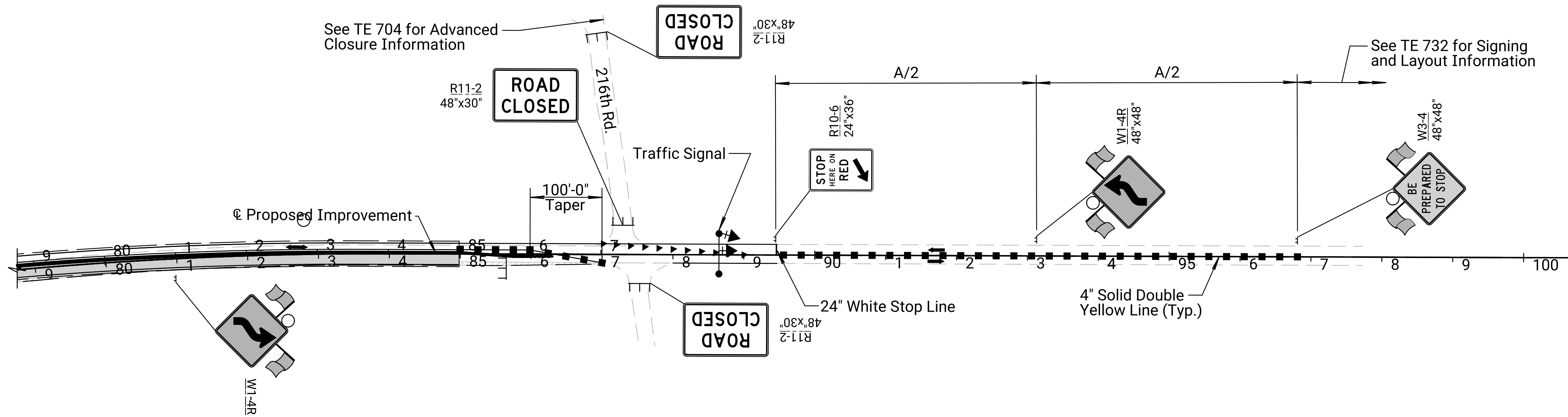
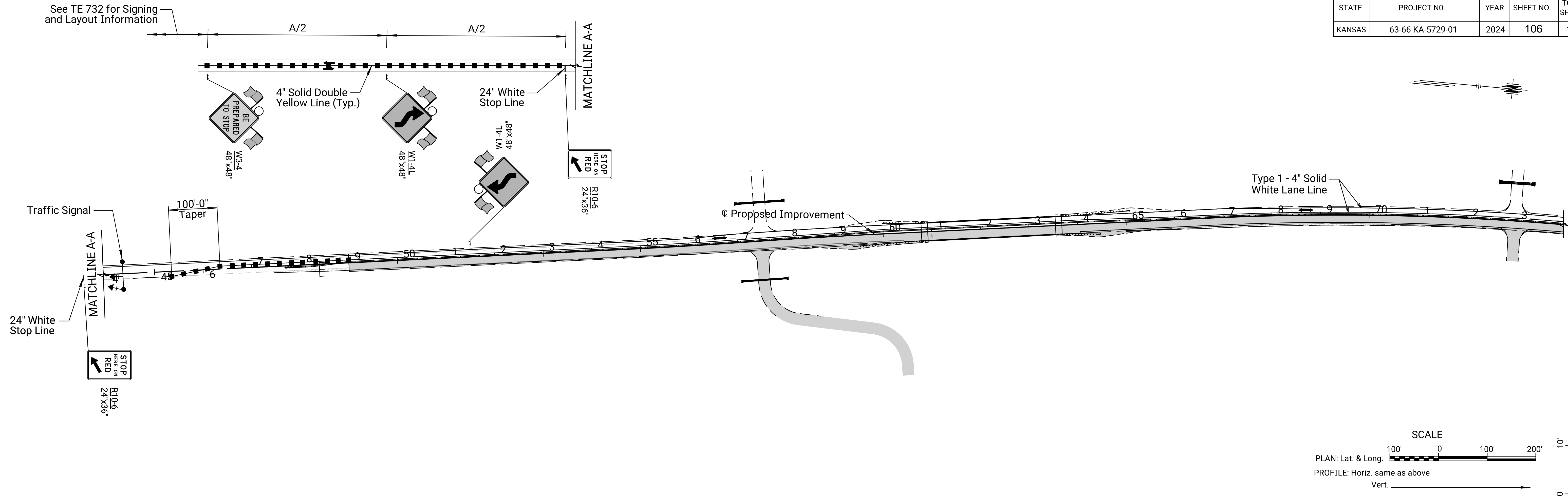
KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL

PHASE 2

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	106	141

BY	DATE
REFERENCES NOTED	REFERENCES CHECKED

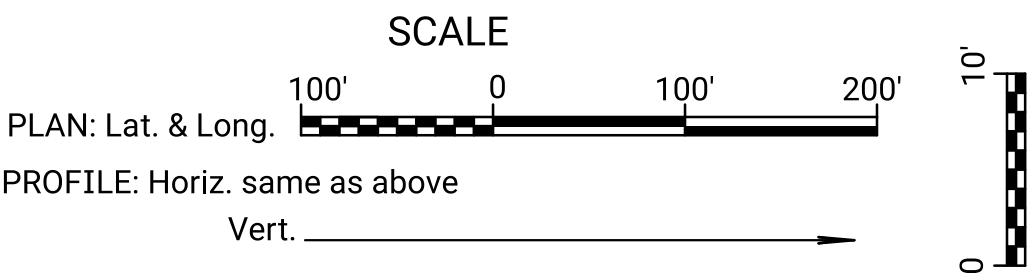


LEGEND

Construction During Phase

Removal During Phase

- NOT TO SCALE
- TYPE III BARRICADES
  - CHANNELIZING DEVICE
  - UNI-DIRECTIONAL TEMPORARY RAISED PAVEMENT MARKER
  - TYPE "A" LOW INTENSITY WARNING LIGHT



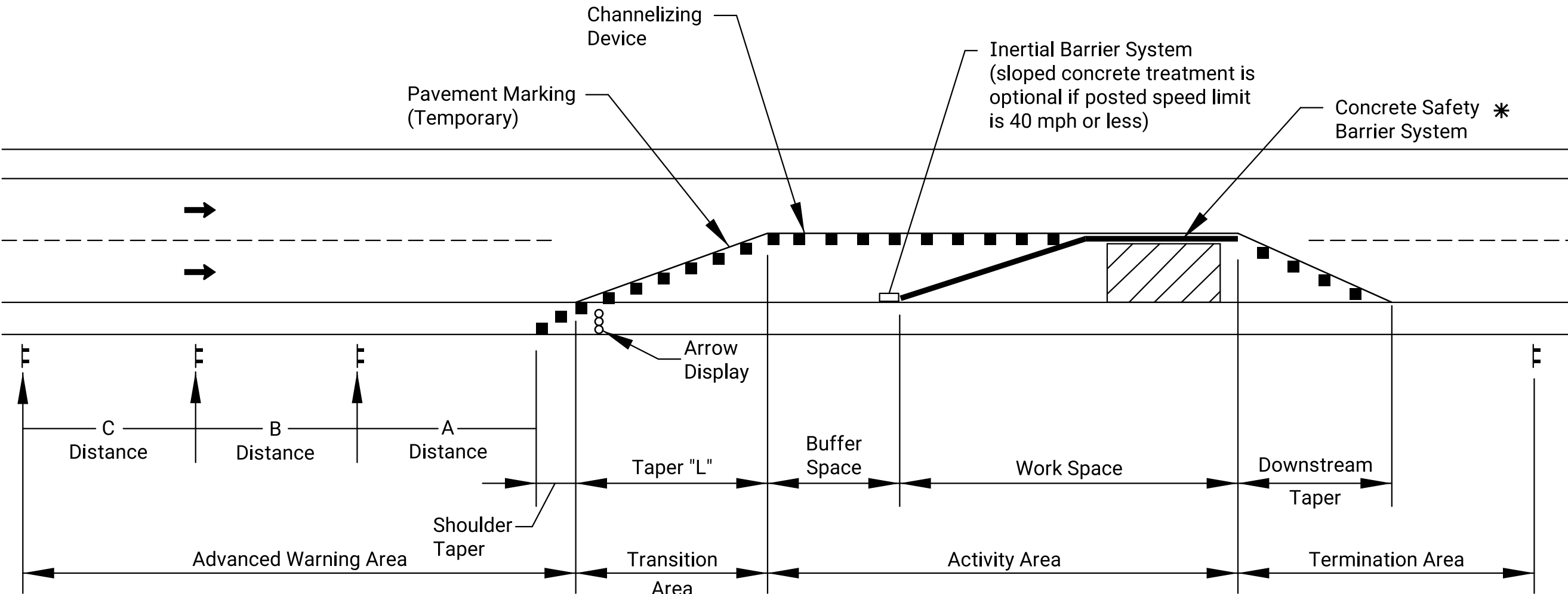
KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL

PHASE 3

Plotted by : JLDrawing 17-JAN-2024 16:55  
File : te700.dgn

- 1) Design Speed: Those items delegated to temporary traffic control should be designed and installed using the posted/legal speed of the roadway prior to work starting.
- 2) Minimum Lane Width: Lane widths shall be a minimum of 11' (measured between centerlines of pavement markings) or as shown on the plans, or as directed by the engineer. A lane width less than 11' may require restricted roadway width signing.
- 3) Consideration should be made to separate pedestrian and, if needed, bicycle movements from both work site activity and vehicular traffic. Unless a reasonable safe route that does not involve crossing the roadway can be provided, pedestrians should be appropriately directed with advance signing that encourages them to cross to the opposite side of the roadway. In urban and suburban areas with high vehicular traffic volumes, these signs should be placed at intersections (rather than midblock locations) so that pedestrians are not confronted with midblock work sites that will induce them to attempt skirting the work site or making a midblock crossing.
- 4) When existing pedestrian facilities are disrupted, closed, or relocated, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility.
- 5) When the driving surface open to traffic is milled or is a temporary surface made of loose material, or when directed by the engineer a W8-15 (Grooved Pavement) or W8-7 (Loose Gravel) sign shall be used on mainline approaches. This sign should be placed a "C" distance after the W20-1 (Road Work Ahead) sign. A W8-15p motorcycle plaque shall be used to supplement the W8-15 or W8-7 signs. All signs shall be displayed as long as the condition is present.
- 6) Alternative temporary rumble strip options may be available. Please contact the Temporary Traffic Control Unit for more information at 785-296-1179 or 785-296-1183.



TYPICAL WORK ZONE COMPONENTS

✱ When concrete barrier system is used, portable channelizing devices are not needed along the tangent barrier section.

Minimum advance warning sign spacing (in feet):

SPEED (MPH) ✱	A	B	C
URBAN (40 MPH OR LOWER)	100	100	100
URBAN (45 MPH OR HIGHER)	350	350	350
RURAL (55 MPH OR LOWER)	500	500	500
RURAL (60 MPH OR HIGHER)	750	750	750
EXPRESSWAY/FREEWAY	1000	1500	2640

- ✱ Posted speed prior to work starting
- The minimum spacing between signs shall be no less than 100', unless directed by the engineer.
- The spacing between any signs may be increased beyond the minimum values in the table above as approved by the engineer in order to maximize visibility.

Taper Formulas:

L = WS for speeds of 45 MPH or more

L = WS<sup>2</sup>/60 for speeds of 40 MPH or less

Where: L = Minimum length of taper in feet  
S = Numerical value of posted speed prior to work starting in MPH  
W = Width in offset feet

Shifting Taper=1/2 L  
Shoulder Taper=1/3 L

Channelizer Placement:

- (1) The spacing between devices in transition area (taper) should not exceed a distance in feet equal to 1/2 the posted speed limit in mph prior to work starting.
- (2) The spacing between devices in the advanced warning area and the activity area should not exceed a distance in feet equal to two times the posted speed limit in mph prior to work starting.
- (3) Channelizing devices shall be placed for optimum visibility, normally at right angles to the traffic flow.
- (4) Place directional indicator barricades in series to direct traffic onto the new path. The arrow sign should not be visible to opposing traffic.
- (5) Alternating diagonal orange and white striping must slope downward in the direction traffic is expected to pass.

Buffer Space

SPEED (MPH) ✱	20	25	30	35	40	45	50	55	60	65	70	75
LENGTH (ft)	115	155	200	250	305	360	425	495	570	645	730	820

- ✱ Posted speed prior to work starting

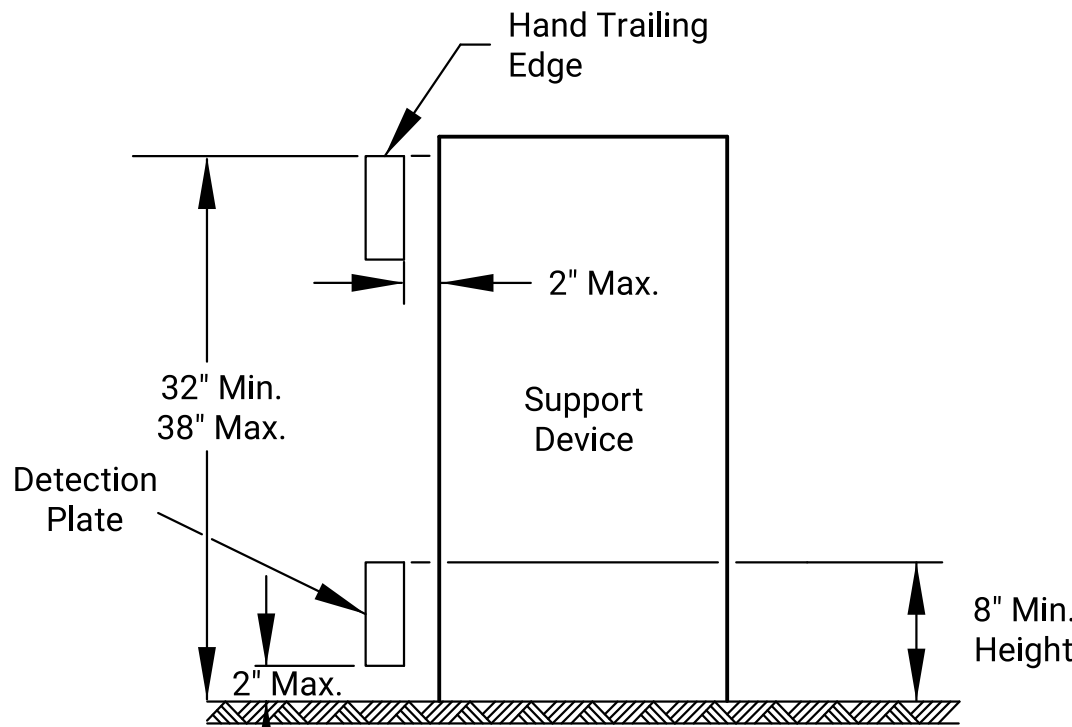
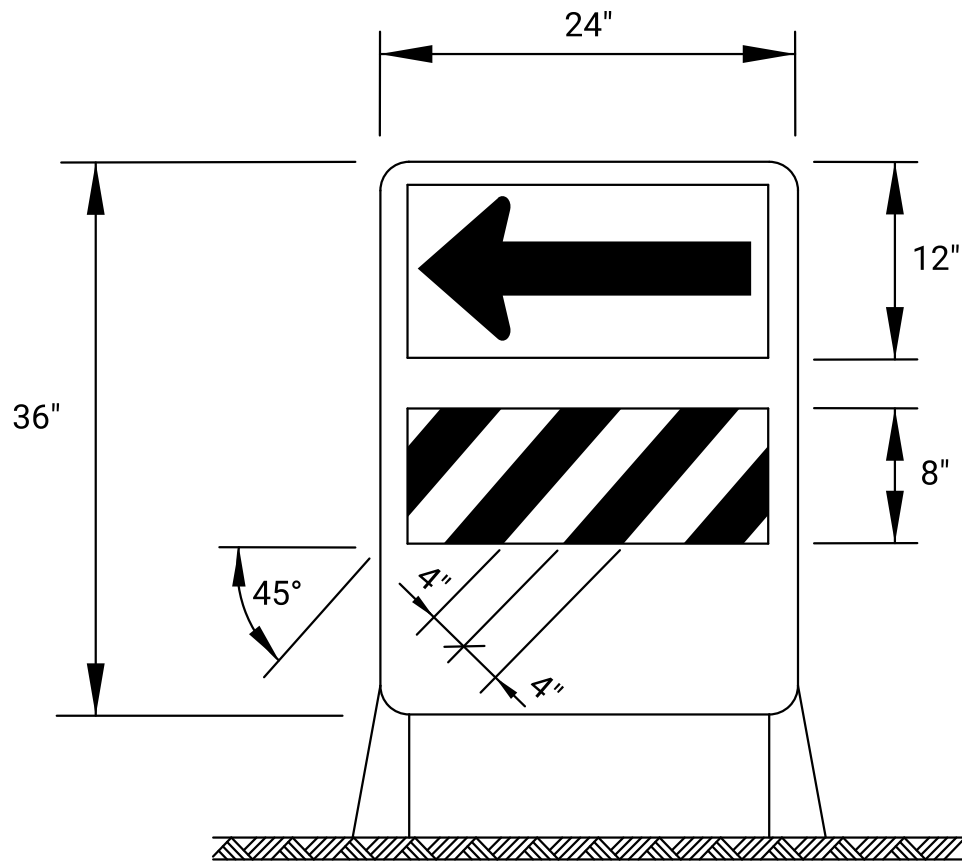
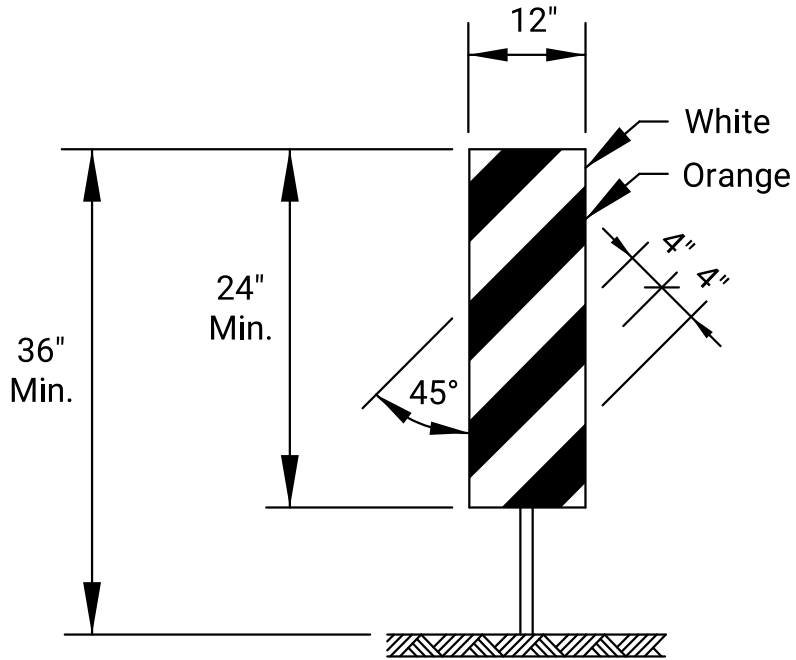
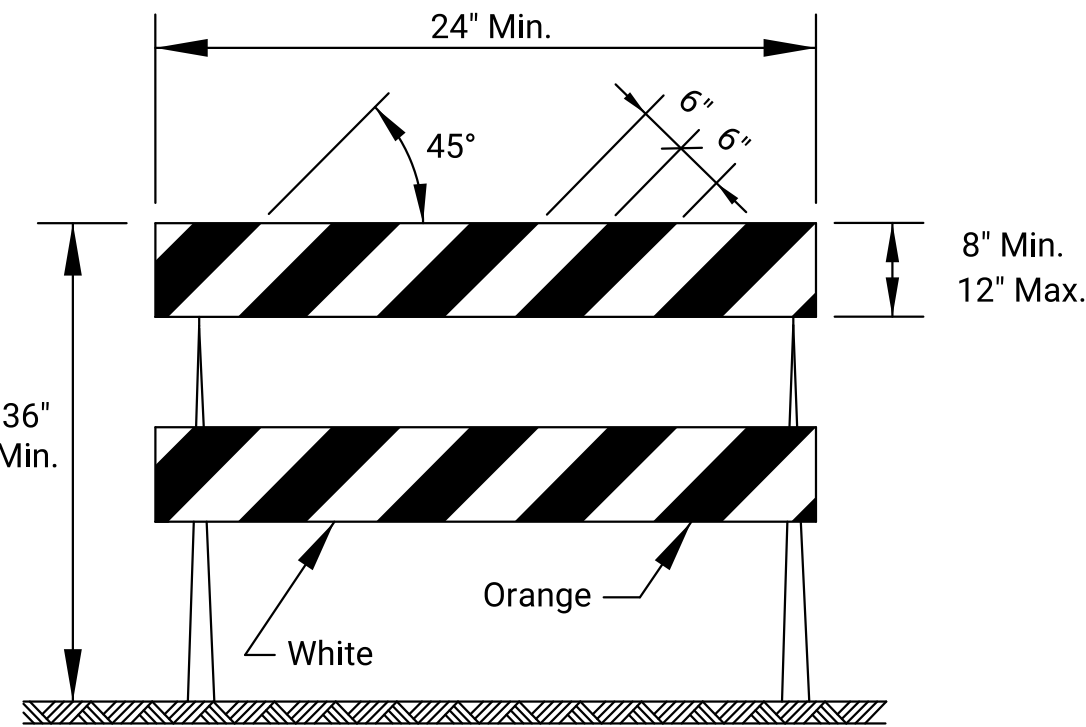
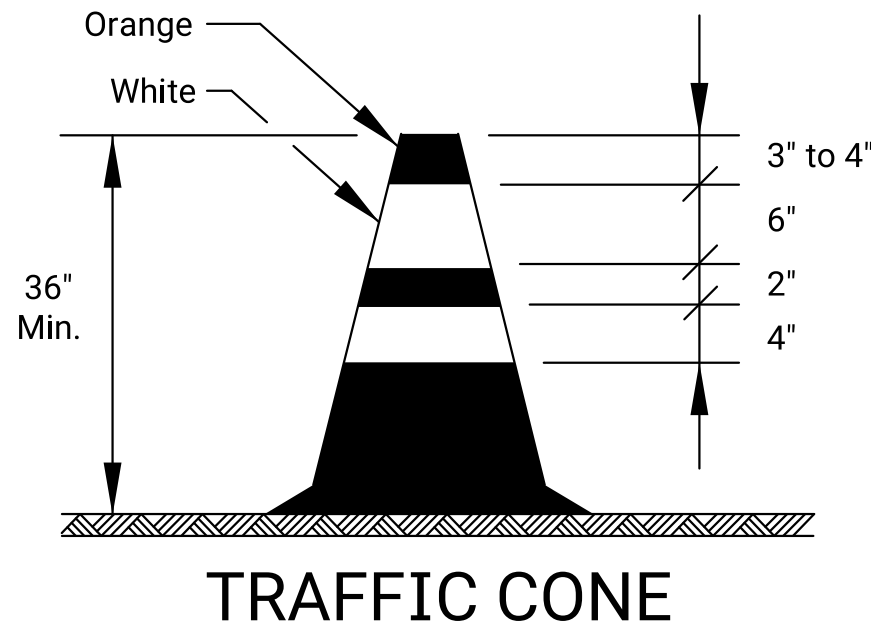
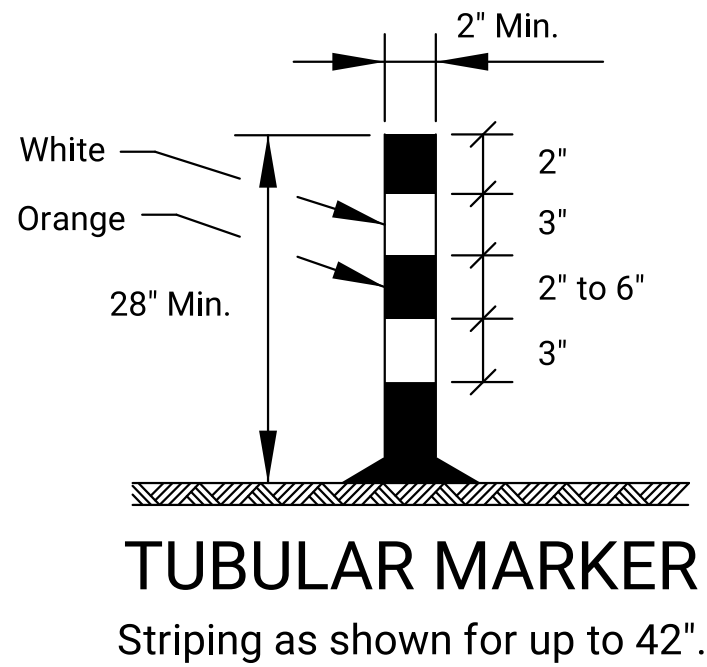
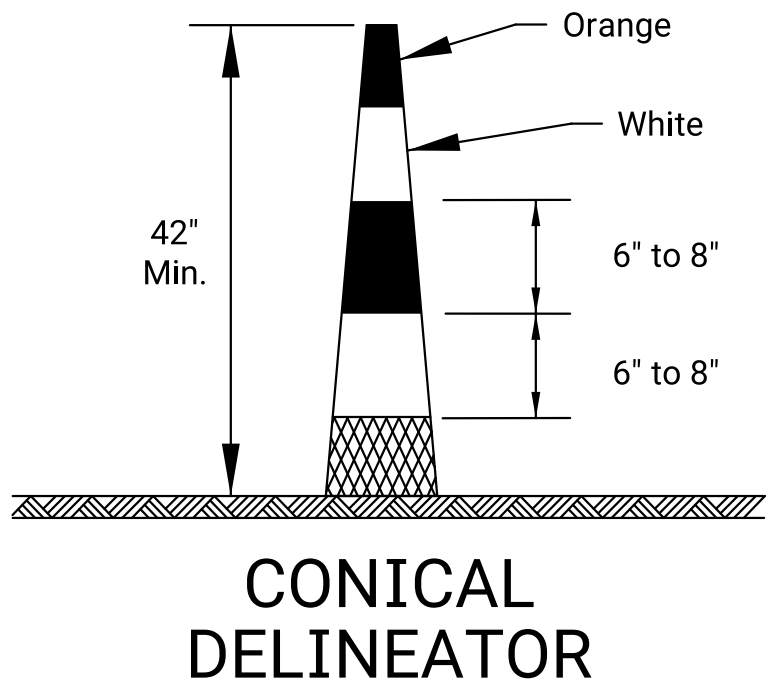
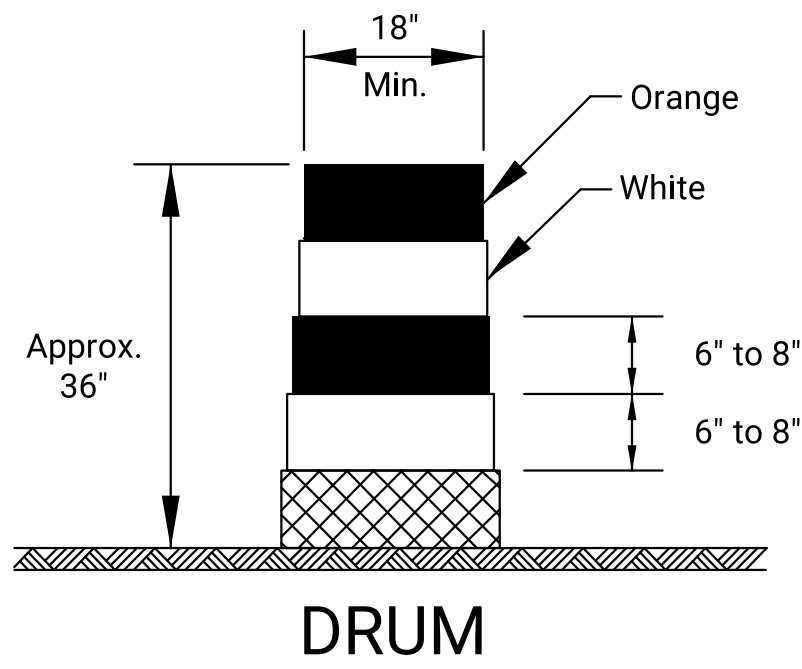
Neither work activity nor storage of equipment, vehicles, or material should occur in the buffer space. When a protection vehicle is placed in advance of the work space, only the space upstream of the vehicle constitutes the buffer space.

If temporary concrete safety barrier system is used to separate approaching traffic from the work space, the barrier system shall be considered part of the activity area. A full lane width should be available throughout the length of the buffer space. See typical work zone components above.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	107	141

02	03-13-18	W8-15p usage changed to Shall	R.W.B.	E.K.G.
01	08-18-15	Channelizer spacing info	R.W.B.	K.E.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL GENERAL NOTES				
TE700				
FHWA APPROVAL		03-13-18	APPD.	Eric Kocher
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	





### TYPE 2 BARRICADE

For rails less than 36" long, 4" wide stripes may be used.  
All stripes shall slope downward to the traffic side for channelization.

### VERTICAL PANEL

The stripes shall slope downward to the traffic side for channelization.

### DIRECTION INDICATOR BARRICADE

The stripes shall slope downward in the direction traffic is to pass.  
The direction indicator barricade shall be used in series to direct the motorist into the intended lane of travel.

### PEDESTRIAN CHANNELIZER

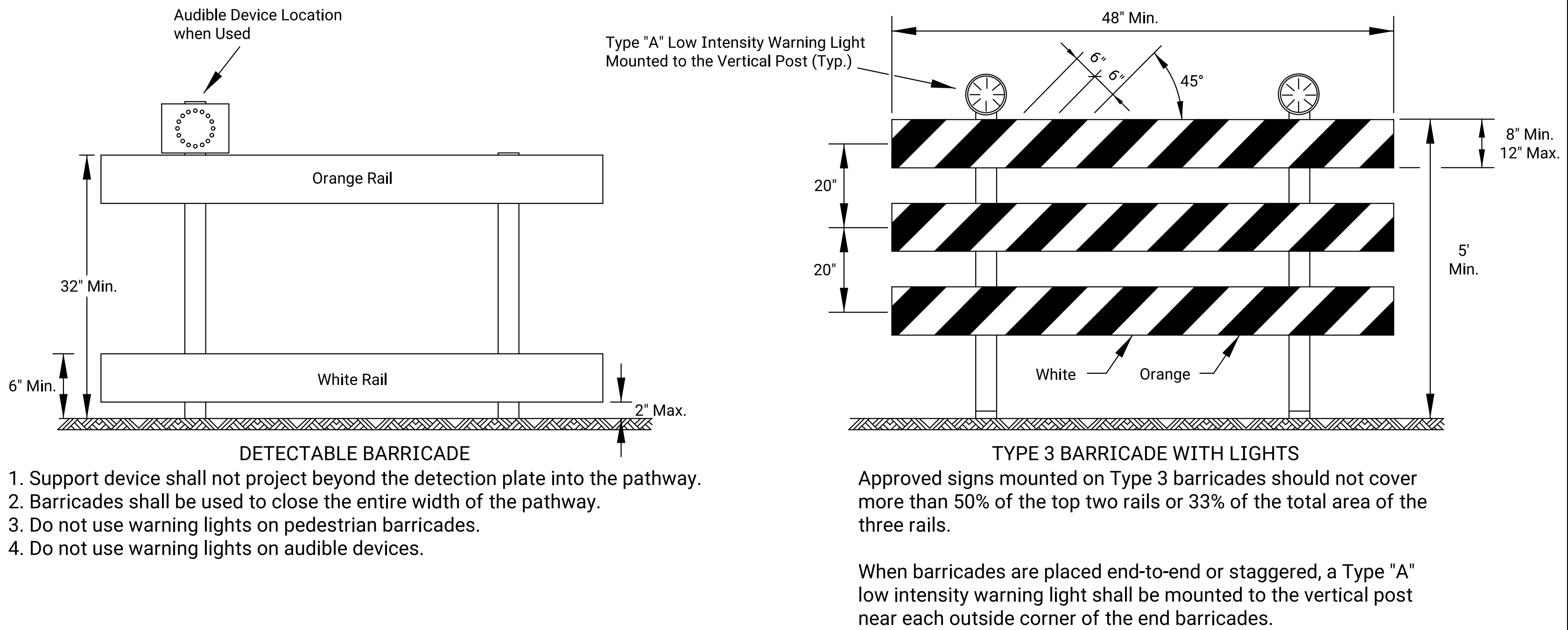
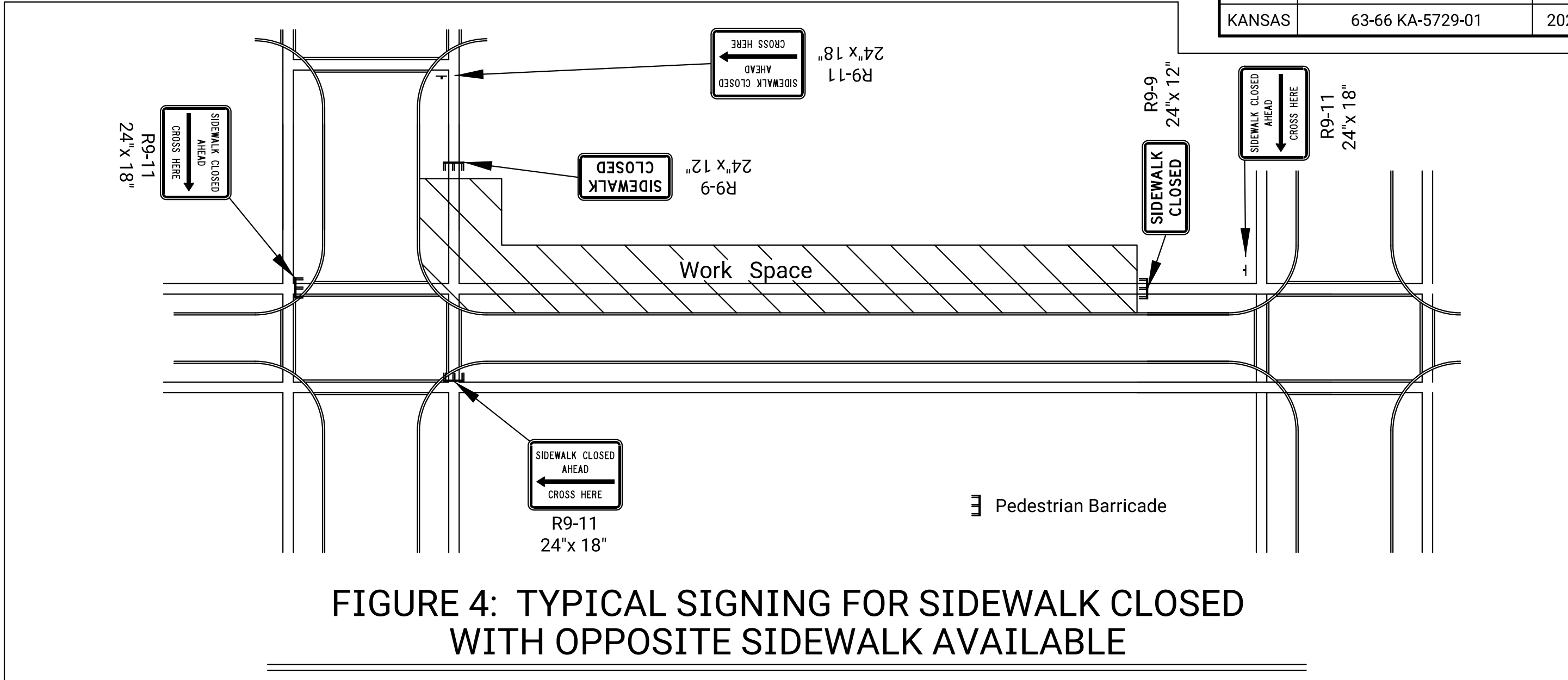
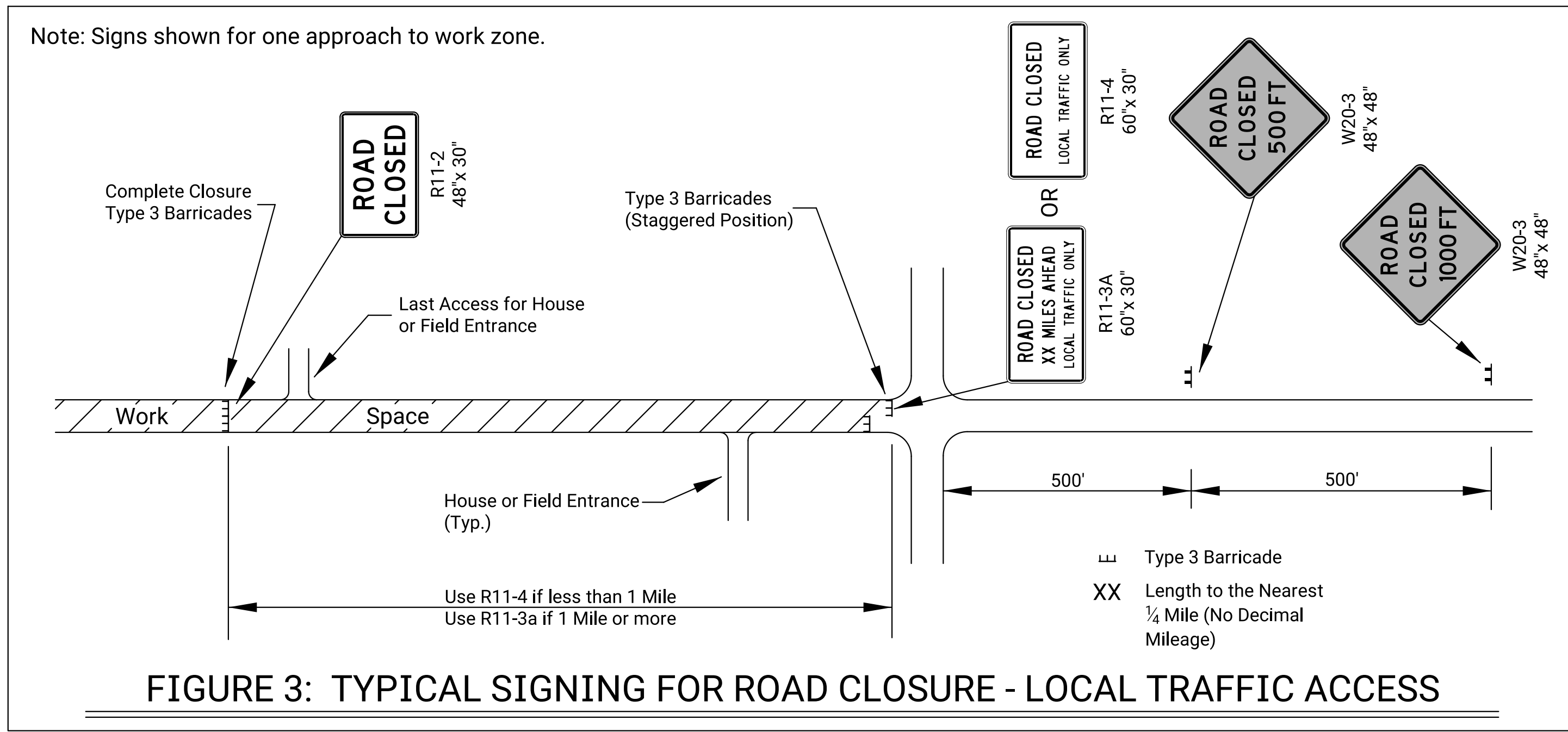
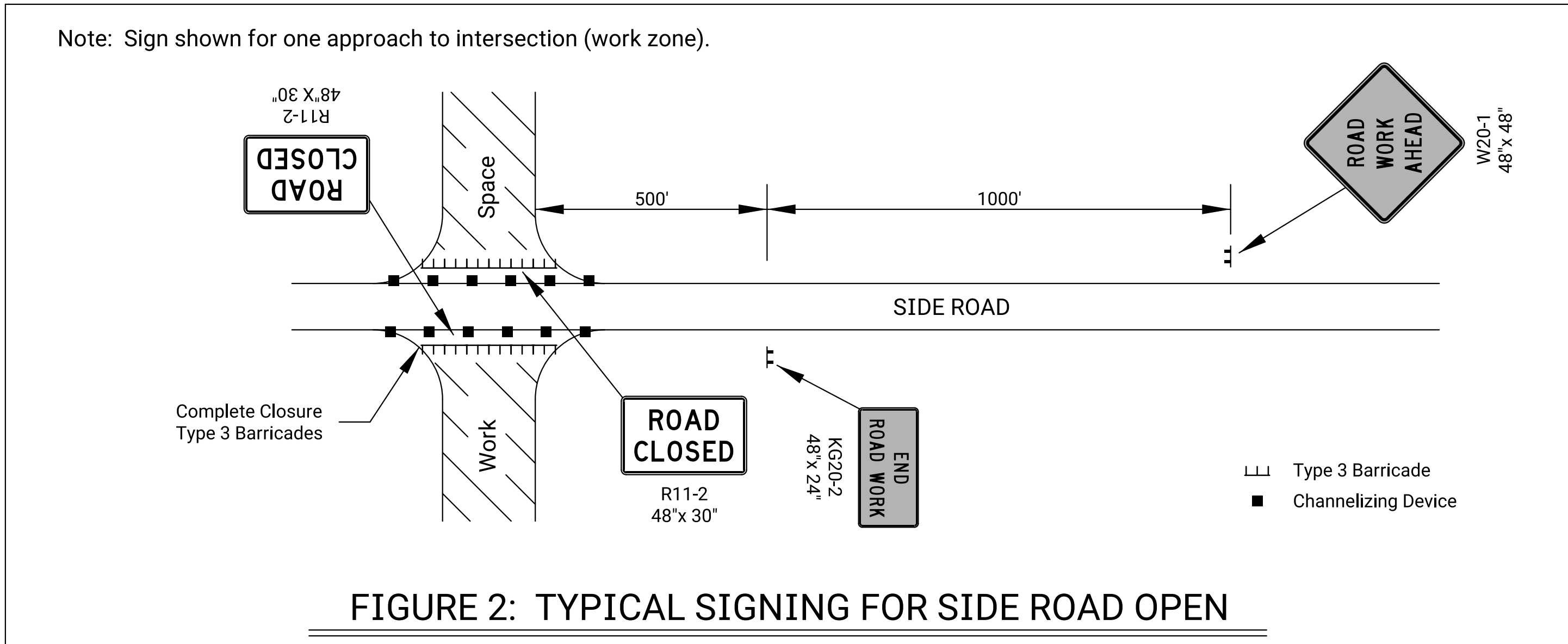
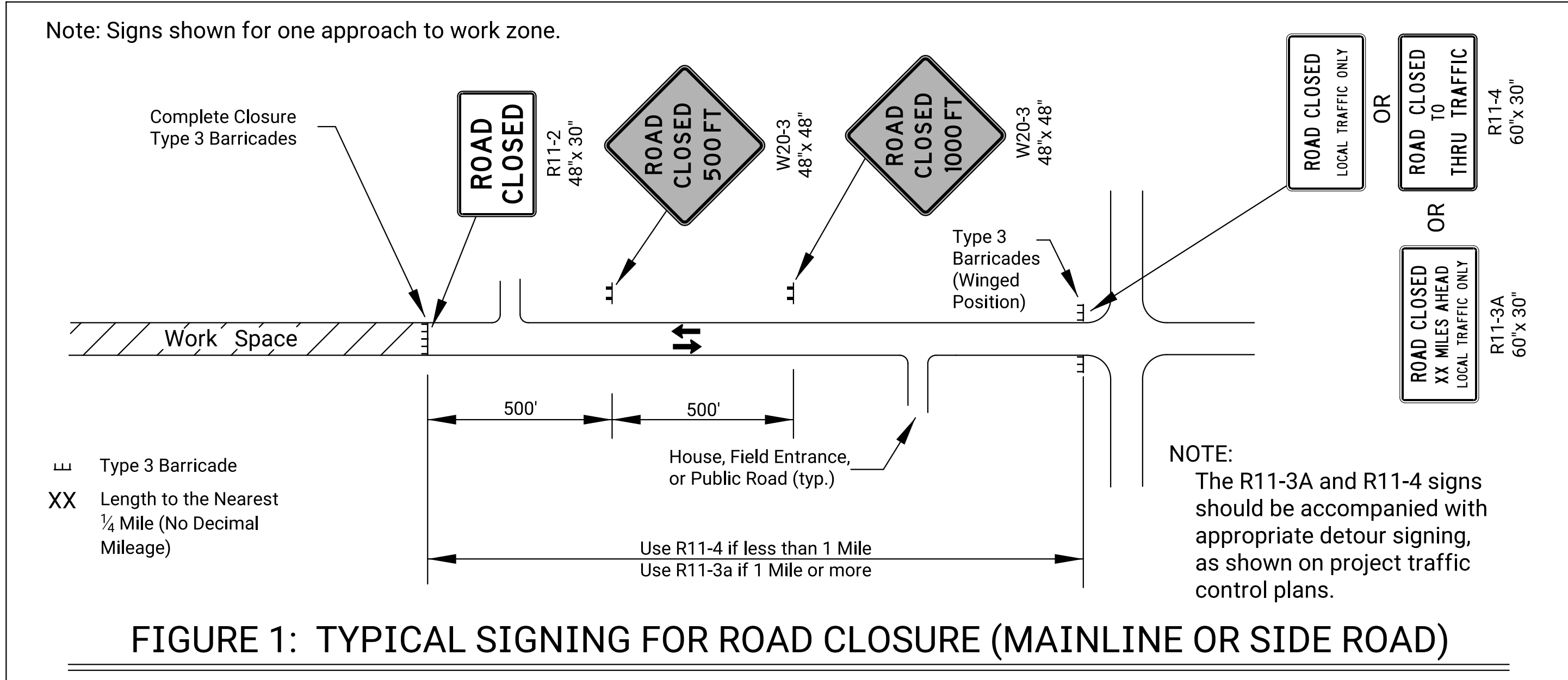
- Support device shall not project beyond the detection plate into the pathway.
- Hand trailing edges and detection plates are optional for continuous walls.
- Interconnect pedestrian channelizers to prevent displacement and to provide continuous guidance through or around work.
- Alternate pathways shall be firm, stable, and slip resistant.
- Treat height differentials > 1/2" in the surfaces of alternate paths with a firm, stable, and slip resistant temporary ramp having a slope of 12:1 or flatter and having a width equal to the alternate path.
- Use alternating orange/white on interconnected devices.

Location		Cross-overs	Shoofly Divisions	Tangents	Tapers	Ramps	Head to Head	Object Identifier	Lead-in Devices	Gores
Portable	Drums	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes
	Conical Delineators	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes
	Vertical Panels	(2)	(2)	(2)	(2)	(2)	(1,2)	Yes	(2)	(2)
	Direction Indicator Barricade	No	No	No	Yes	No	No	No	No	No
	Type 2 Barricade	(2)	(2)	(2)	(2)	No	No	Yes	No	No
	Traffic Cones	No	No	(4)	(4)	(4)	No	(4)	(4)	(4)
Fixed										
	Tubular Markers	(3)	(3)	(3)	No	(3)	Yes	No	Yes	Yes
	Vertical Panels	(3)	(3)	(3)	(3)	(3)	(3)	Yes	(2,3)	(2)

- (1) Not allowed on centerline delineation along freeways or expressways.  
(2) The stripes shall slope downward to the traffic side for channelization.  
(3) May be used upon the approval of the engineer.  
(4) Daytime operations only.

NO.	DATE	REVISIONS			BY APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL CHANNELIZING DEVICES					
TE702					
FHWA APPROVAL		06-01-15	APP'D.	Kristina Erickson	
DESIGNED	L.E.R.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	109	141



#### ROAD CLOSED GENERAL NOTES

As shown in Figure 1, at the point where thru traffic must detour and local traffic can proceed to the location where the roadway is completely closed, the R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) or R11-4 (ROAD CLOSED LOCAL TRAFFIC ONLY or ROAD CLOSED TO THRU TRAFFIC) sign shall be used with Type 3 barricades (winged position), placed on the shoulders of roadway.

As shown in Figure 3, when local traffic must be allowed access into the work zone, Type 3 barricades shall be longitudinally staggered to maintain the appearance of a closed roadway. A second line of end-to-end Type 3 barricades shall be placed just beyond the last access point in the work zone, to completely close the roadway.

The R11-4 (ROAD CLOSED TO THRU TRAFFIC or ROAD CLOSED LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is less than 1 mile.

The R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is 1 mile or greater.

The words "BRIDGE OUT" (or BRIDGE CLOSED) may be substituted for the words "ROAD CLOSED" on the R11-3a or R11-4 sign where applicable.

NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL CLOSURES					
TE704					
FHWA APPROVAL		06-01-15		APP'D. Kristina Ericksen	
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	110	141

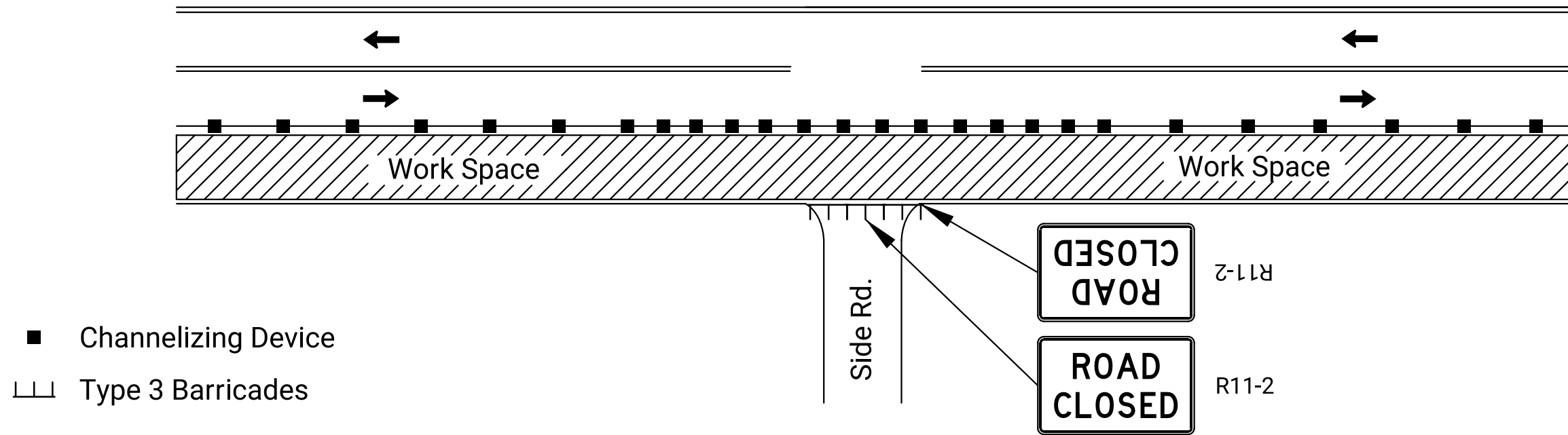


FIGURE 1: SIDE ROAD OR ENTRANCE CLOSED THROUGH WORK AREA

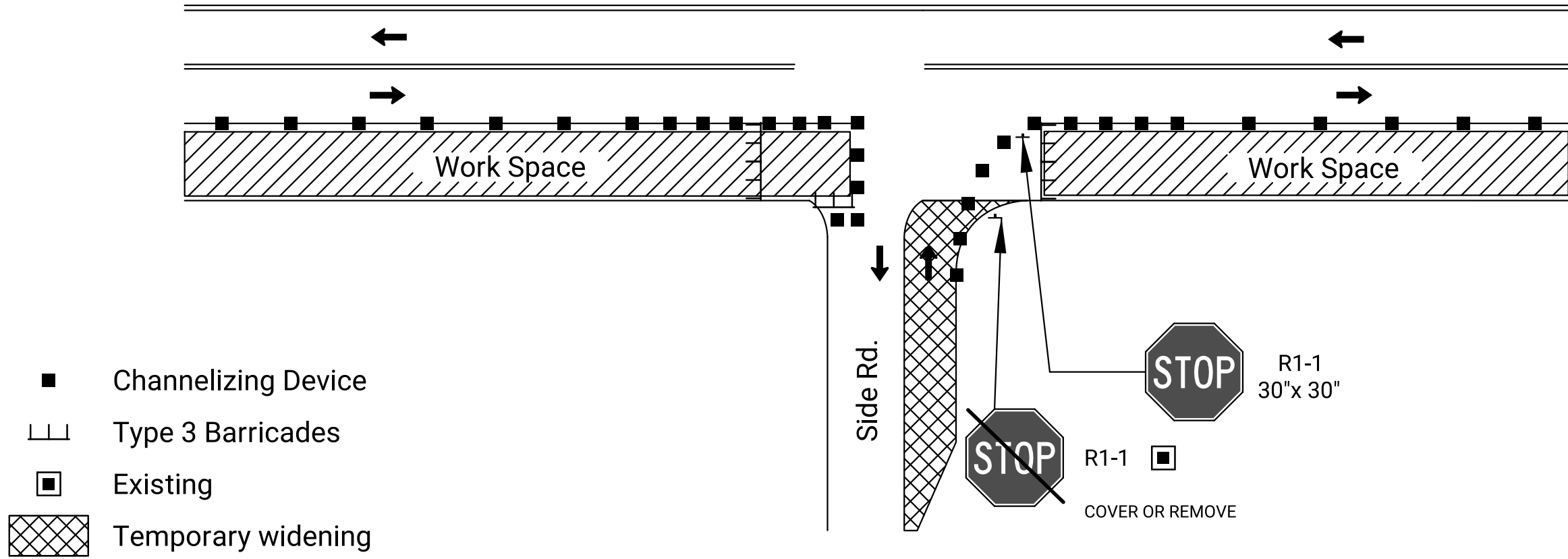


FIGURE 4: SIDE ROAD OR ENTRANCE CONSTRUCTED HALF AT A TIME:  
TWO WAY TRAFFIC REQUIRED

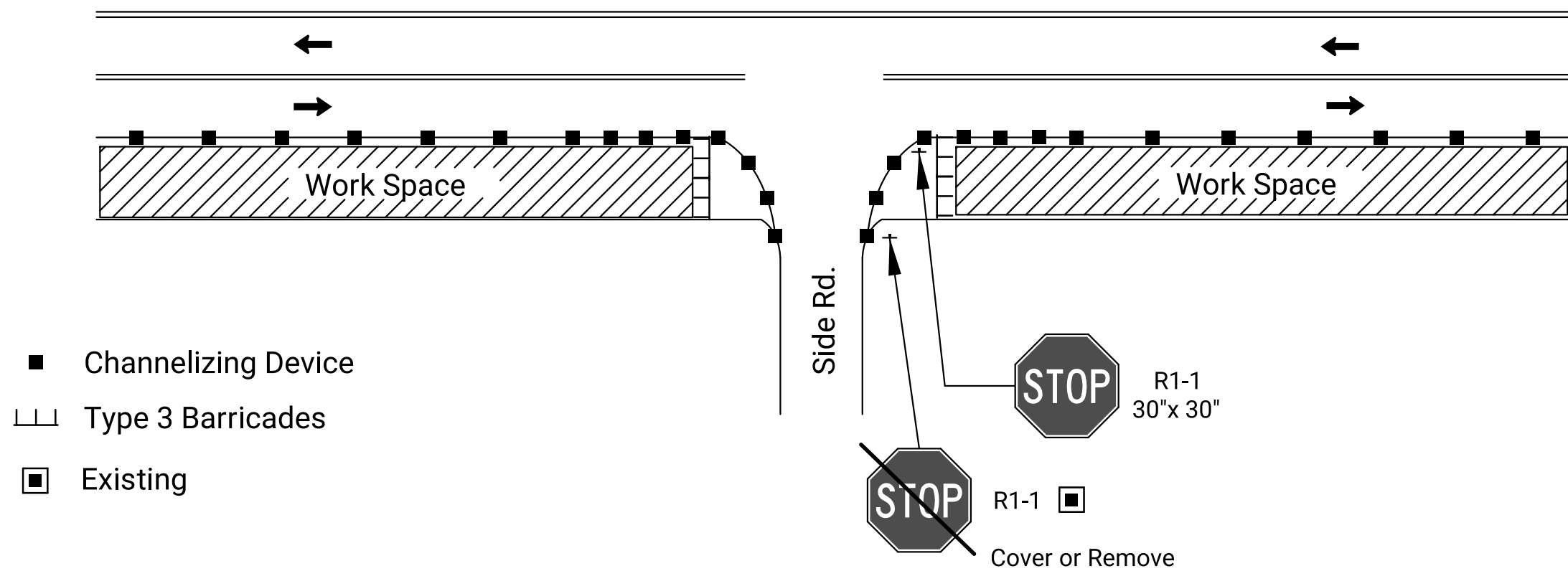


FIGURE 2: SIDE ROAD OR ENTRANCE OPEN THROUGH WORK AREA

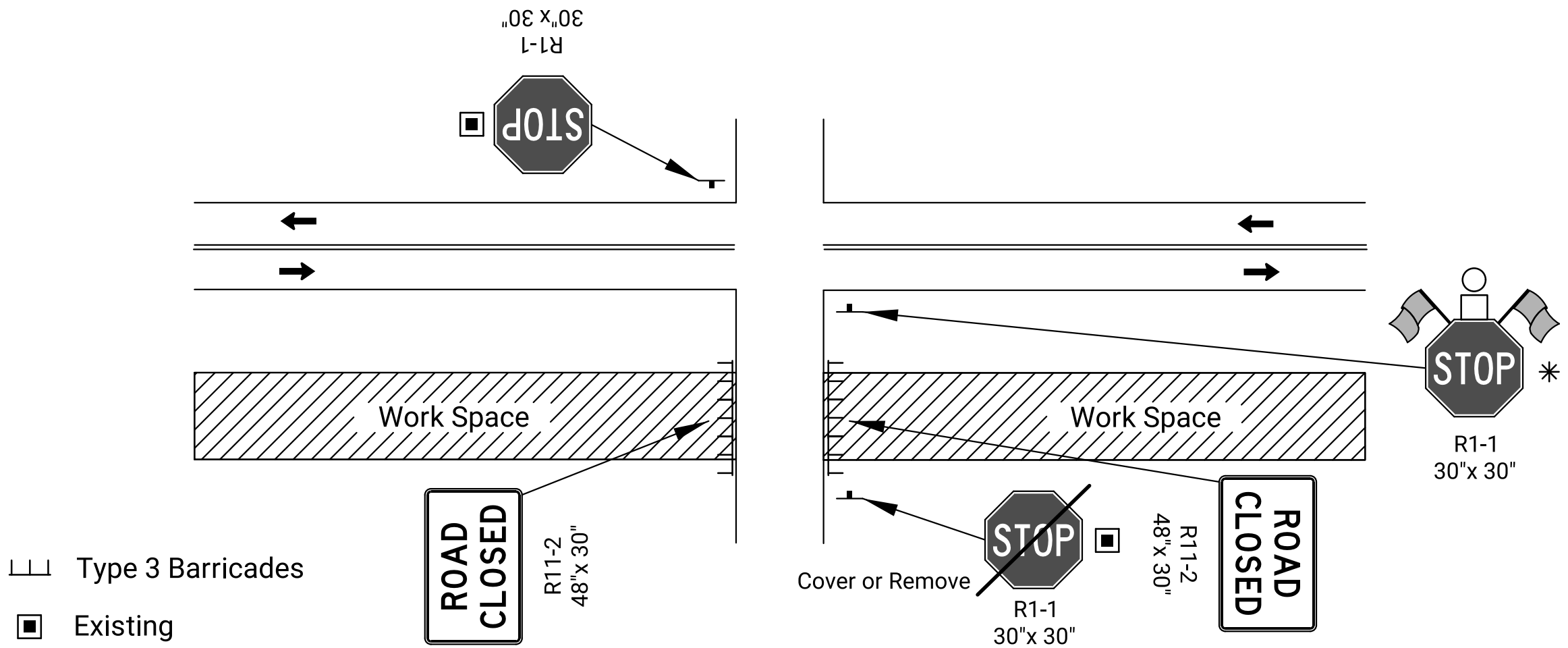


FIGURE 5: SIDE ROAD OPEN THROUGH WORK AREA ON DIVIDED ROADWAY

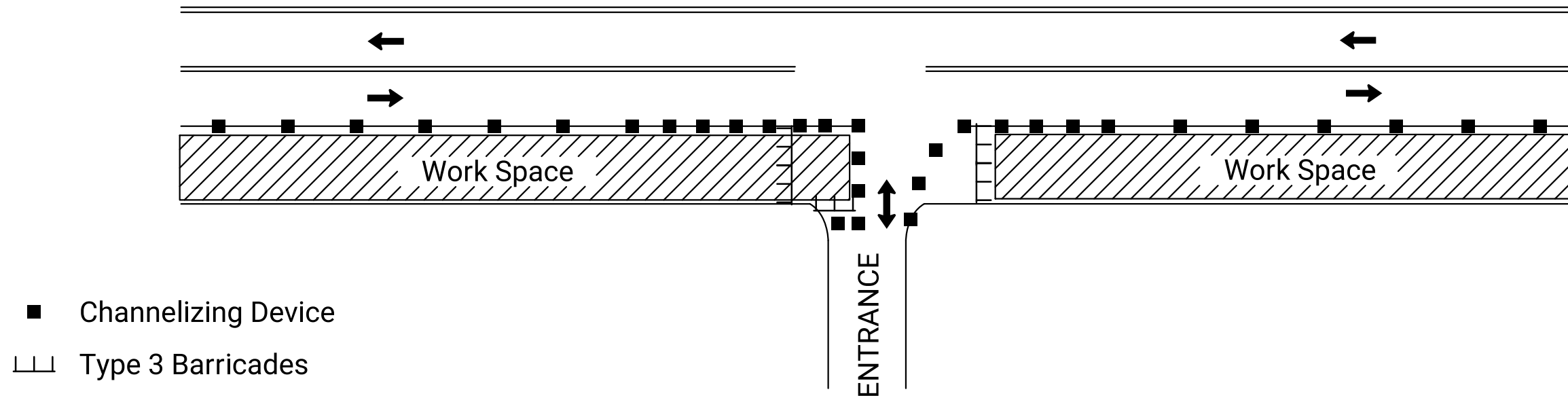


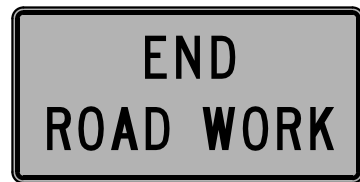
FIGURE 3: LOW VOLUME ENTRANCE CONSTRUCTED HALF AT A TIME

Note: Consider large vehicles making right turns into and out of entrance  
and use figure 4 as needed

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL ACCESS THROUGH THE WORK AREA				
TE705				
FHWA APPROVAL 06-01-15 APPD. Kristina Ericksen				
DESIGNED R.W.B.	DETAILED R.W.B.	QUANTITIES R.W.B.	TRACED R.W.B.	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

Plotted by: JLDrawing 17-JAN-2024 16:56  
File : te710.dgn

### SIGN LAYOUT INFORMATION



KG20-2

Std. Size  
Expwy/Freeway

6" C  
48"x 24"



KG20-5

Std. Size  
Expwy/Freeway

6" C  
48"x 24"

WORK ZONE

KM4-20

Std. Size

3" C  
24"x 6"

Expwy/Freeway

6" C  
48"x 12"



W7-3a

Mileage to be Determined  
by the Engineer.



W8-17

Std. Size  
Expwy/Freeway

48"x 48"



W8-17P  
(Optional)

Std. Size  
Expwy/Freeway

30"x 24"



W8-15

Std. Size  
Expwy/Freeway

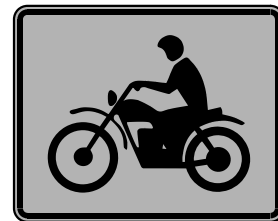
8" D  
48"x 48"



W8-7

Std. Size  
Expwy/Freeway

8" D  
48"x 48"



W8-15p

Std. Size  
Expwy/Freeway

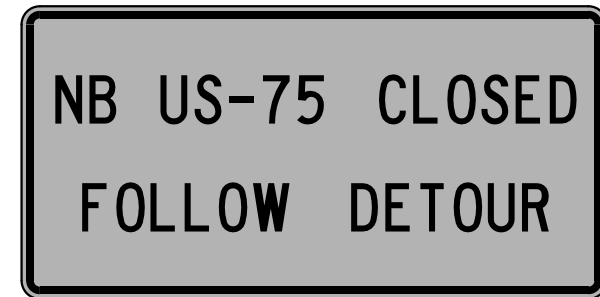
30"x 24"



W8-11

Std. Size  
Expwy/Freeway

8" D  
48"x 48"



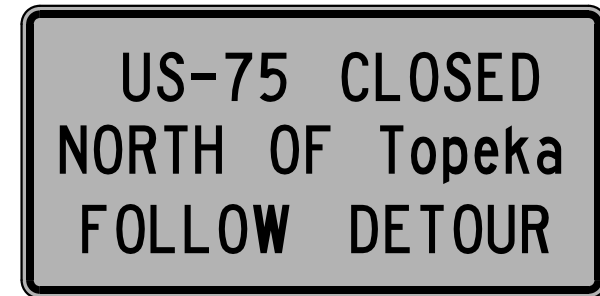
SP-01  
(Special Sign)

Std. Size

6" C

Expwy/Freeway

10" D



SP-02  
(Special Sign)

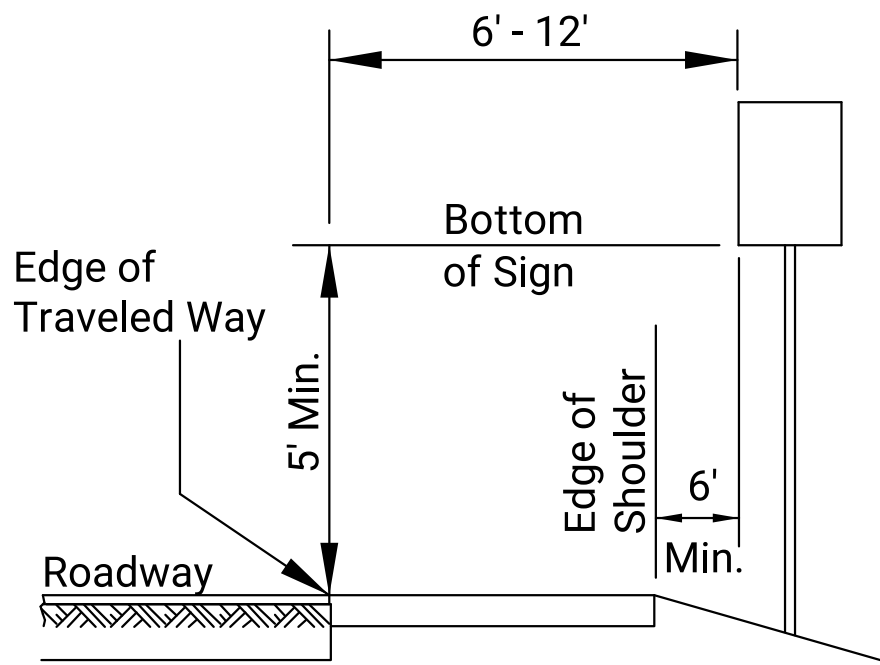
Std. Size

Uppercase: 6" C  
Lowercase: 4.5" C

Expwy/Freeway

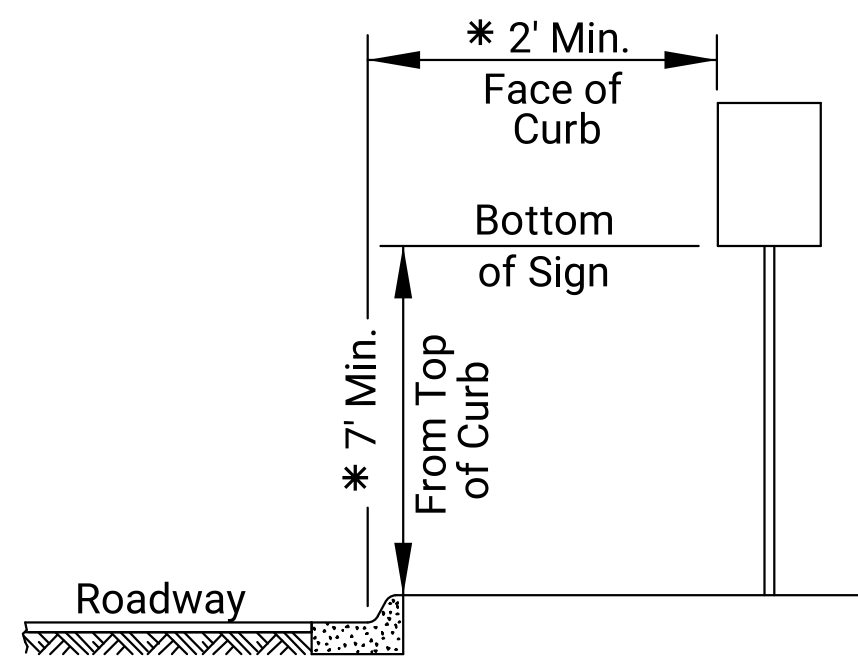
Uppercase: 10" D  
Lowercase: 8" D

All city names and street names on special signs and destination signs  
must have upper and lower case letters.



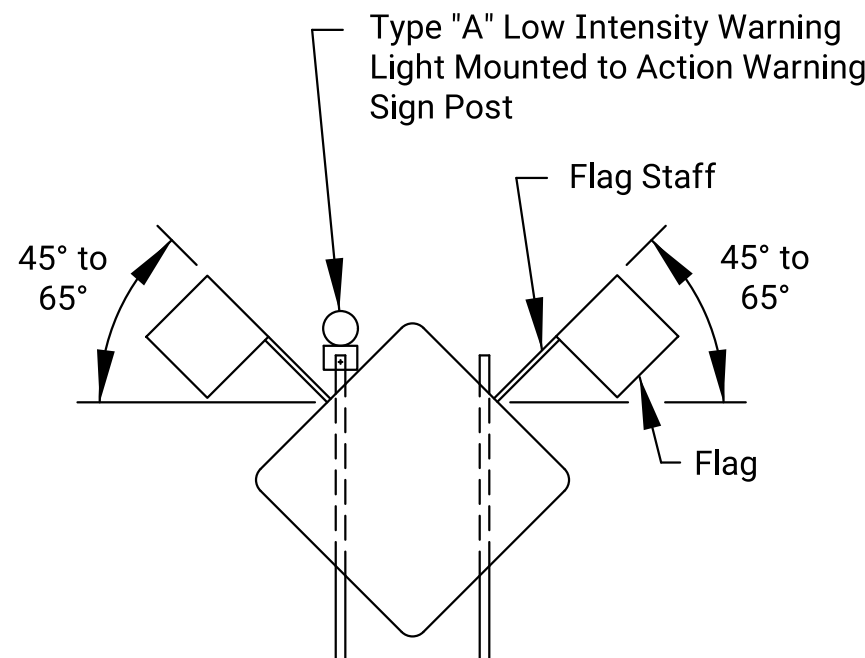
#### RURAL

- 1) Ground-mounted signs shall be mounted at a minimum height of 5' measured from the bottom of sign to the near edge of the pavement.
- 2) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- 3) The height of the secondary sign mounted below another sign may be 4' measured from the bottom of the sign to the near edge of the pavement. Signs shall not overlap each other.



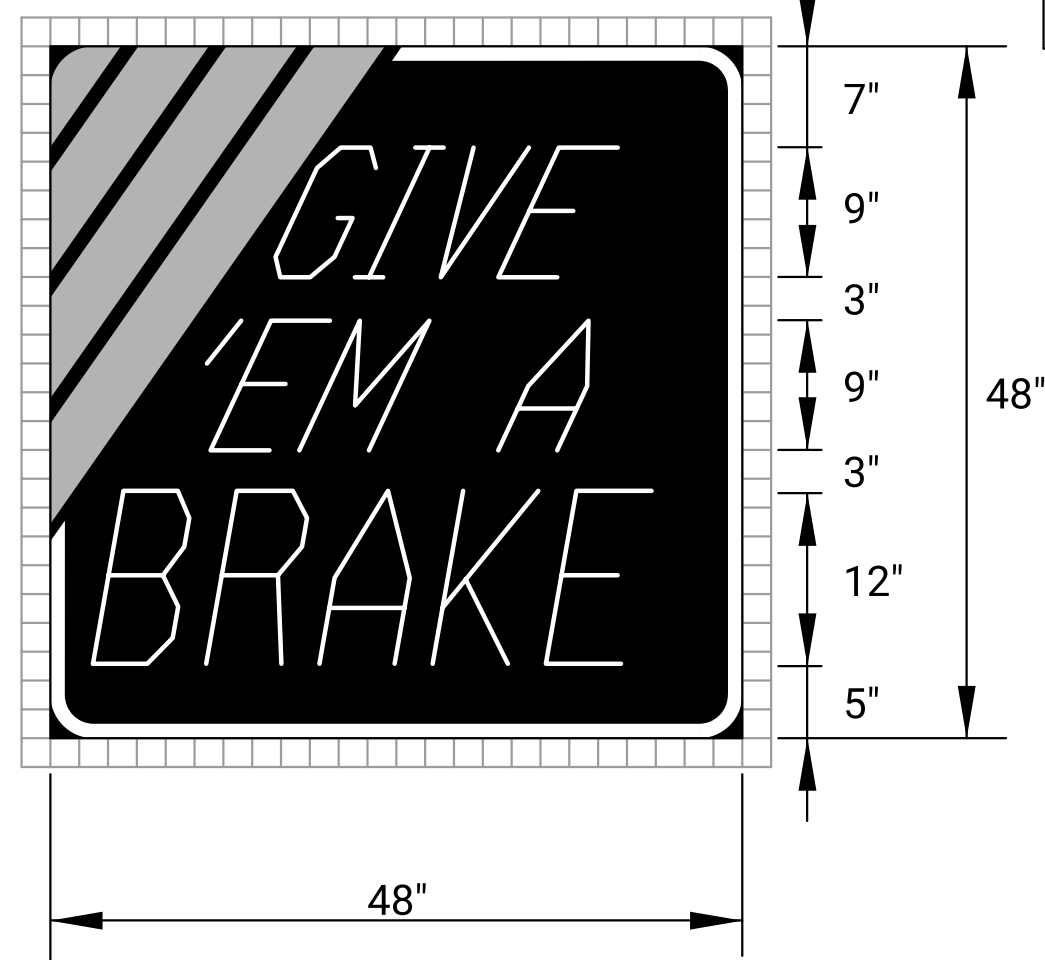
#### URBAN

- 1) Signs shall be mounted at a minimum height of 7' measured from the bottom of sign to the near edge of the pavement.
- 2) Neither portable nor permanent sign supports should be located on sidewalks or areas designated for pedestrian or bicycle traffic.
- 3) Signs mounted lower than 7' should not project more than 4" into pedestrian facilities.
- 4) The height from of the secondary sign mounted below another sign may be 6' measured from the bottom of sign to the near edge of the pavement. Signs shall not overlap each other.
- 5) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- \* 6) Pedestrian detour signing shall be a minimum of 2' measured from the top of the pedestrian pathway to the bottom of the sign and shall not protrude into the walkway nor shall it project beyond the back of curb.

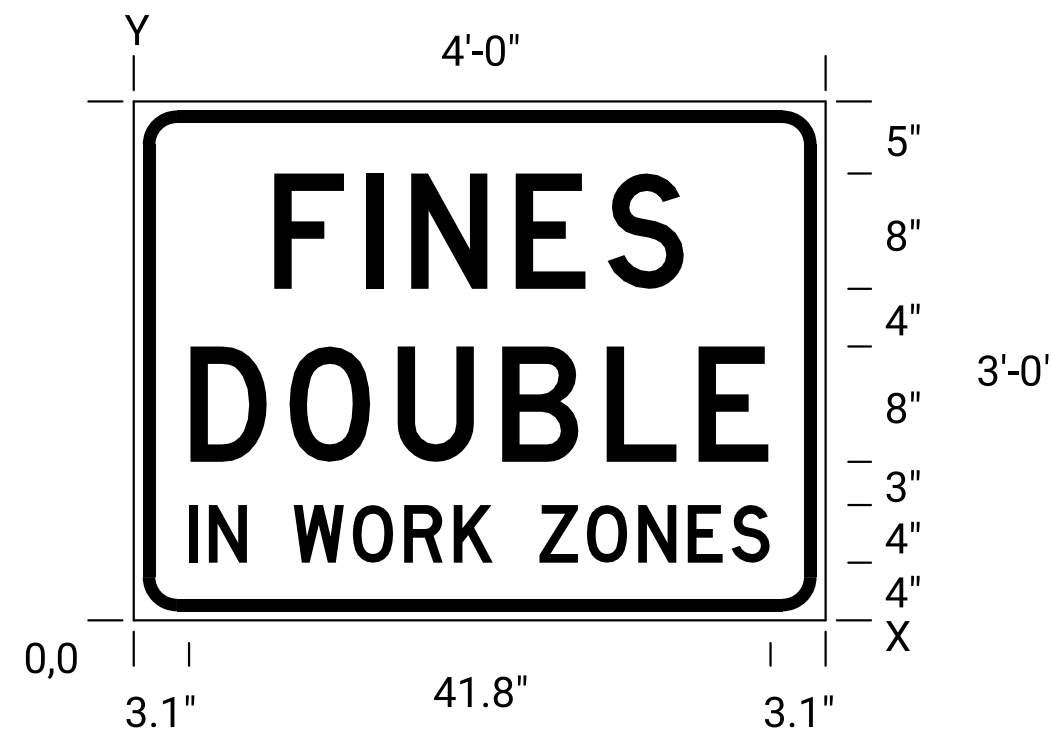


When the sign width is equal to or greater than 9', three or more wood posts may be used with a minimum of 4' between the centerline of each post. All signs less than 9' in width shall use a maximum of two wood posts.

- In the case of hitting rock when driving posts
1. Shift the sign location. Do not violate minimum sign spacing.
  2. With the engineer's approval, use acceptable alternative sign stands.



KI-104a



KI-105a

Dimensions in inches

Spacings are to start of next letter

Y FONT	LETTER SPACINGS																HT LEN
23.0 D	9.7	6.4	3.2	7.3	6.4	5.4	9.7										8.0
11.0 D	3.9	6.9	7.5	7.3	6.4	4.9	3.9										28.6
4.0 D	3.1	1.6	2.7	3.2	4.3	3.8	3.6	2.8	3.2	3.4	3.8	3.6	3.2	2.7	3.1		8.0
																	40.3
																	4.0
																	41.8

Notes:

Typically, there are two sets of informational signs installed per project: one for each direction of traffic.

Install signs a minimum of 500' in advance of the road work ahead sign. The engineer may designate a more appropriate location if conditions dictate.

The informational signs are not to interfere with the traffic control signs for the project.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	111	141

Sign Number	GIVE EM A BRAKE
Width x Height	4'-0" x 4'-0"
Border Width	1.0"
Corner Radius	4.0"
Stripe Width	3.0"
Mounting	Ground
Background	Type: Non-Reflective Color: Black
Legend/Border	Type: Reflective Color: White
Legend Font	Dutch 801 Roman SWC 25 Degree Slant
Stripes	Type: Reflective Color: Orange

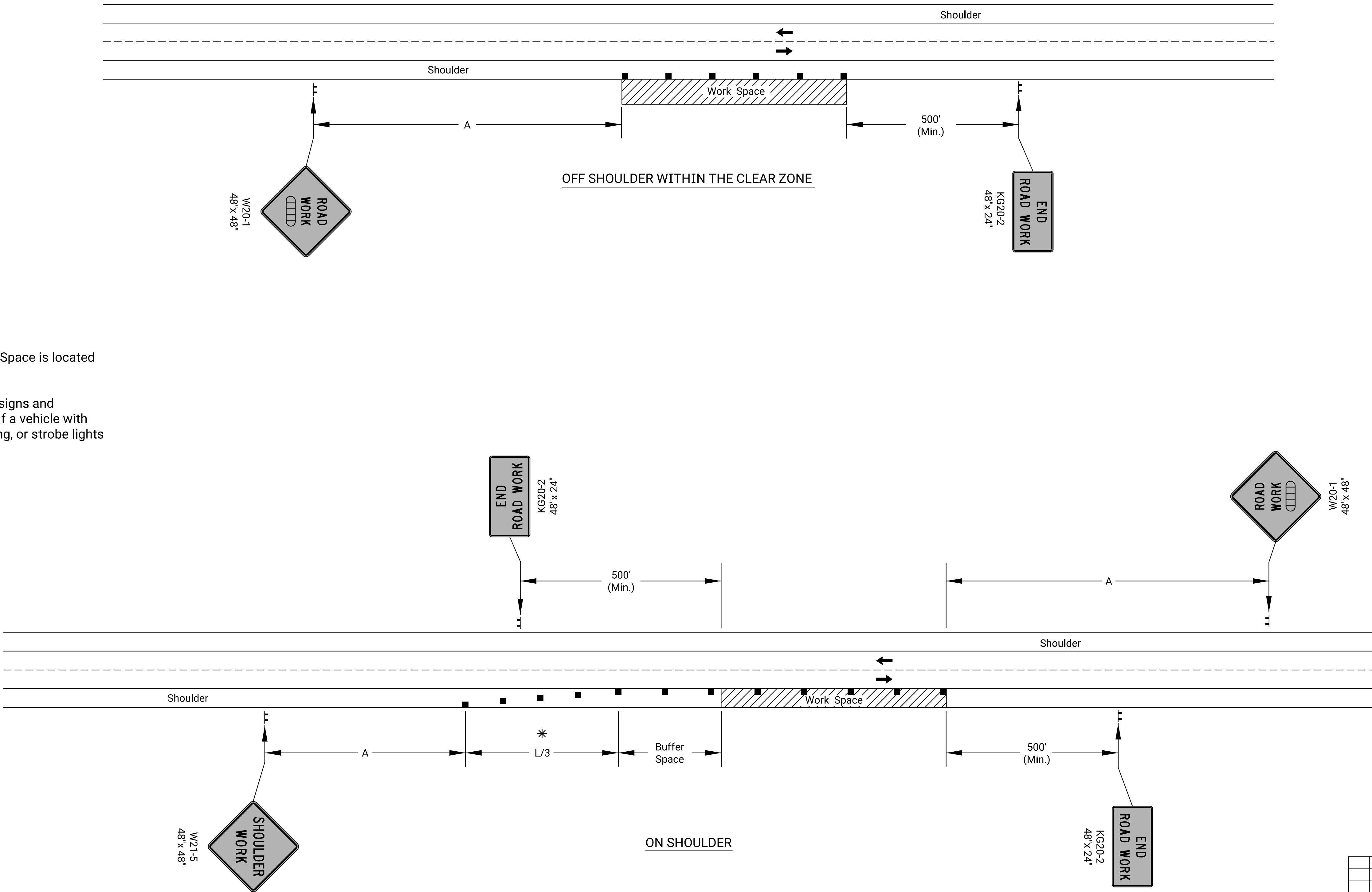
Sign Number	FINES DOUBLE
Width x Height	4'-0" x 3'-0"
Border Width	0.9"
Corner Radius	3.0"
Mounting	Ground
Background	Type: Reflective Color: White
Legend/Border	Type: Non-Reflective Color: Black

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL SIGN INFORMATION				
TE710				
FHWA APPROVAL 06-01-15   APPD. Kristina Erickson				
DESIGNED R.W.B.	DETAILED R.W.B.	QUANTITIES R.W.B.	TRACED R.W.B.	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	





STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	113	141



Notes:

No traffic control is required if the Work Space is located outside of the clear zone.

For operations of 60 minutes or less, all signs and channelizing devices may be eliminated if a vehicle with high-intensity rotating, flashing, oscillating, or strobe lights is used.

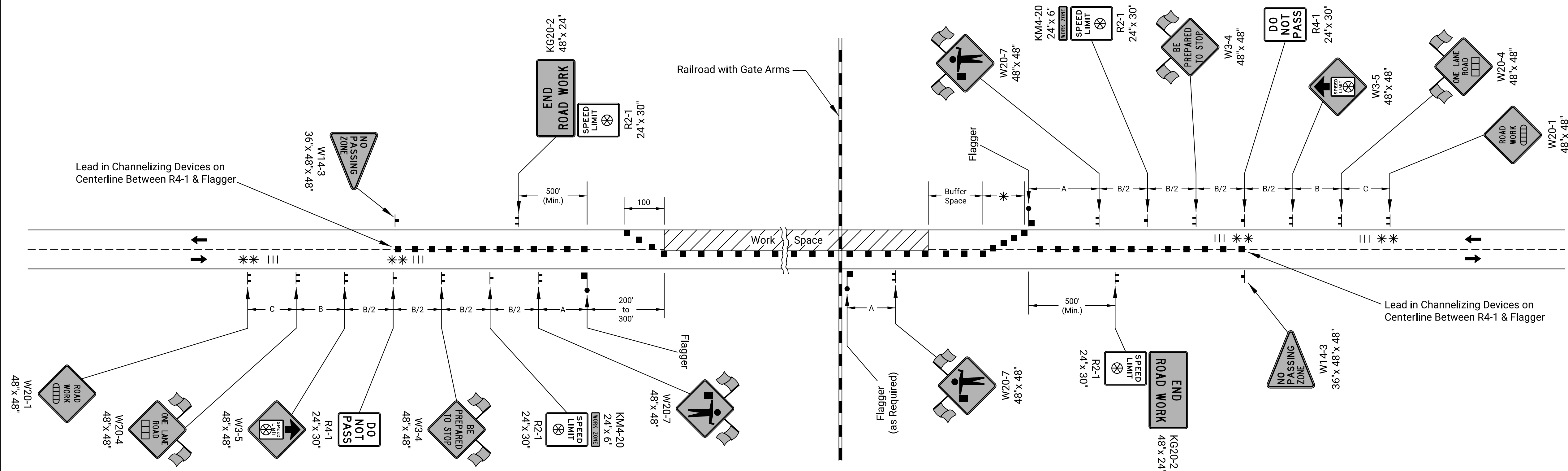
- Channelizing Device
- Ahead, 1500 ft, or 1 Mile

\* Omit taper if paved shoulder is less than 8' wide.

NO.	DATE	REVISIONS	BY	APPD	
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL SHOULDER WORK UNDIVIDED ROADWAY					
TE720					
FHWA APPROVAL		06-01-15	APPD.	Kristina Ericksen	
DESIGNED	L.E.R.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	114	141

# FLAGGER



USE TE731 FOR FLAGGER OR PILOT CAR ON ROADWAYS  
WITH CONCRETE SHOULDERS GREATER THAN 8 FT.

Notes:

Trucks hauling material to the project should STOP at the Flagger. After stopping, upon approval of the Engineer, trucks may be allowed to move around the Flagger.

Place a Flagger at all highway and major collector intersections and at-grade railroad intersections with lights and gates in the work space to control traffic crossing the tracks to the left of the gate arm. The need for a Flagger at minor side road intersections shall be determined by the Engineer. Place a W20-7 (Flagger symbol) sign on each side road that is controlled by a Flagger.

Existing signs shall not be covered or removed between Flagger stations.

Temporary rumble strips may be used in lieu of lead in channelizing devices when the roadway is less than or equal to 30' including paved shoulders. When extenuating circumstances exist, the Area Engineer may elect to eliminate both the lead in channelizers and the rumble strips.

- \* Minimum six (6) channelizers spaced at 20' intervals.

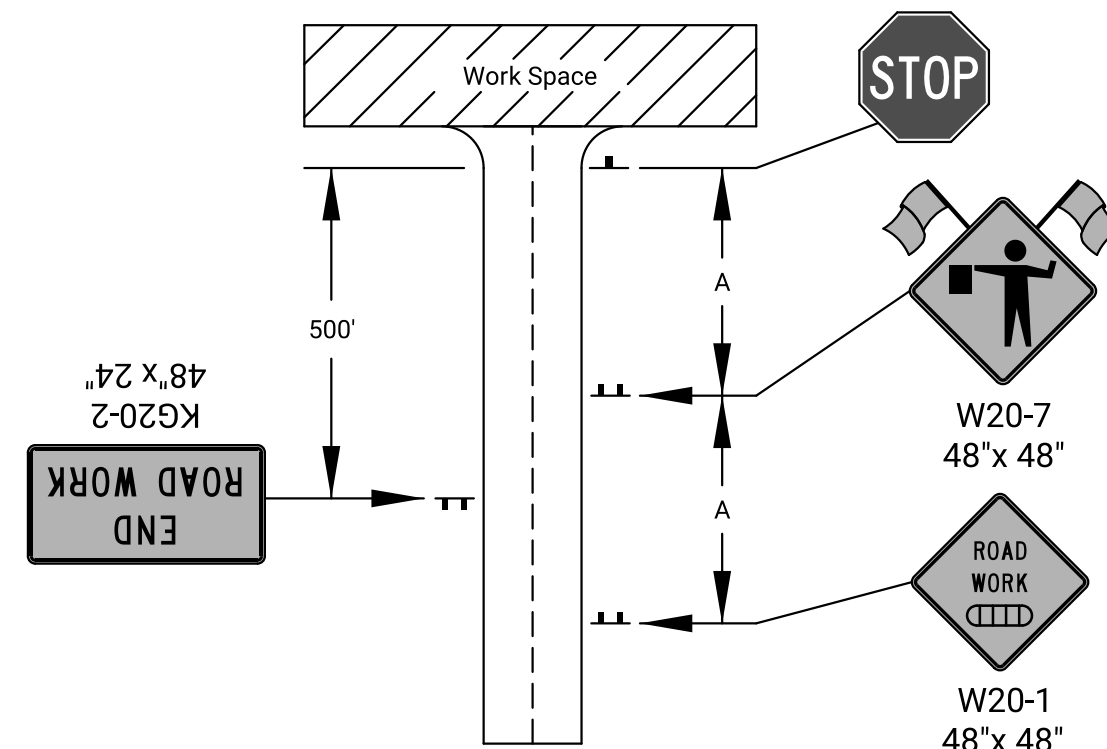
Optional rumble strips may be placed: One set between the W20-1 and W20-4, and one set between the R4-1 and W3-4, on each approach.

△ Not required on substantial maintenance projects  
(1R).

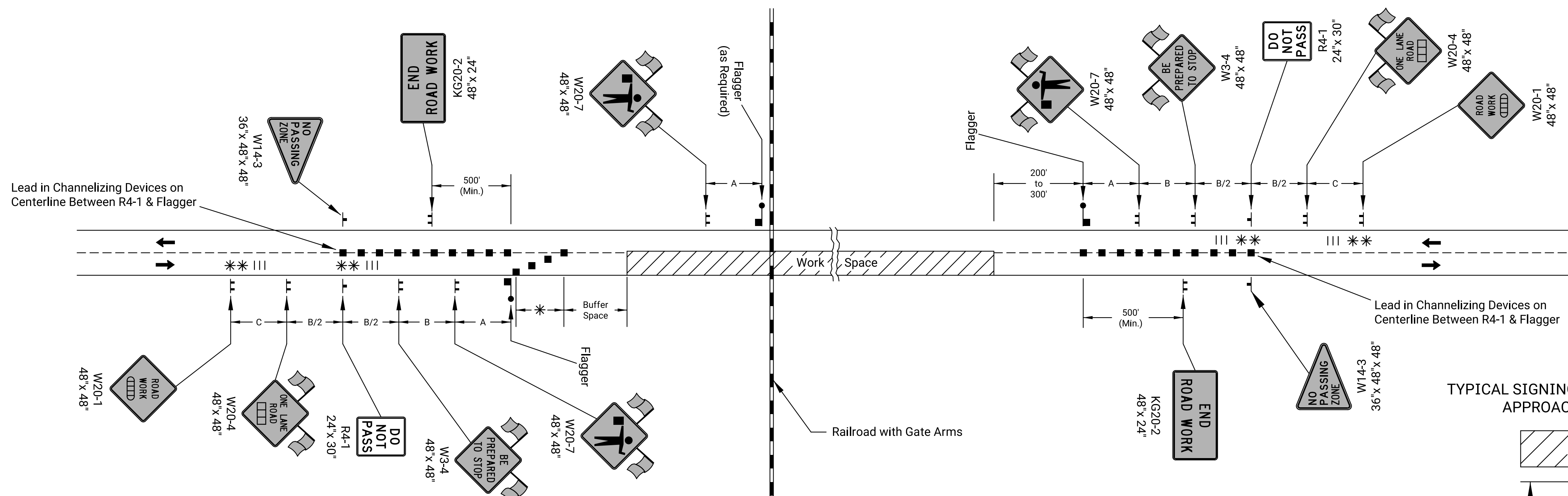
△△ The KG20-5 (WAIT FOR PILOT CAR) sign shall be mounted on an approved portable support and not attached to the existing stop sign post.

The KG20-5 sign shall be placed immediately in front of the existing stop sign, a minimum of 6" below the bottom of the stop sign. The sign should be removed or covered when there is no pilot car.

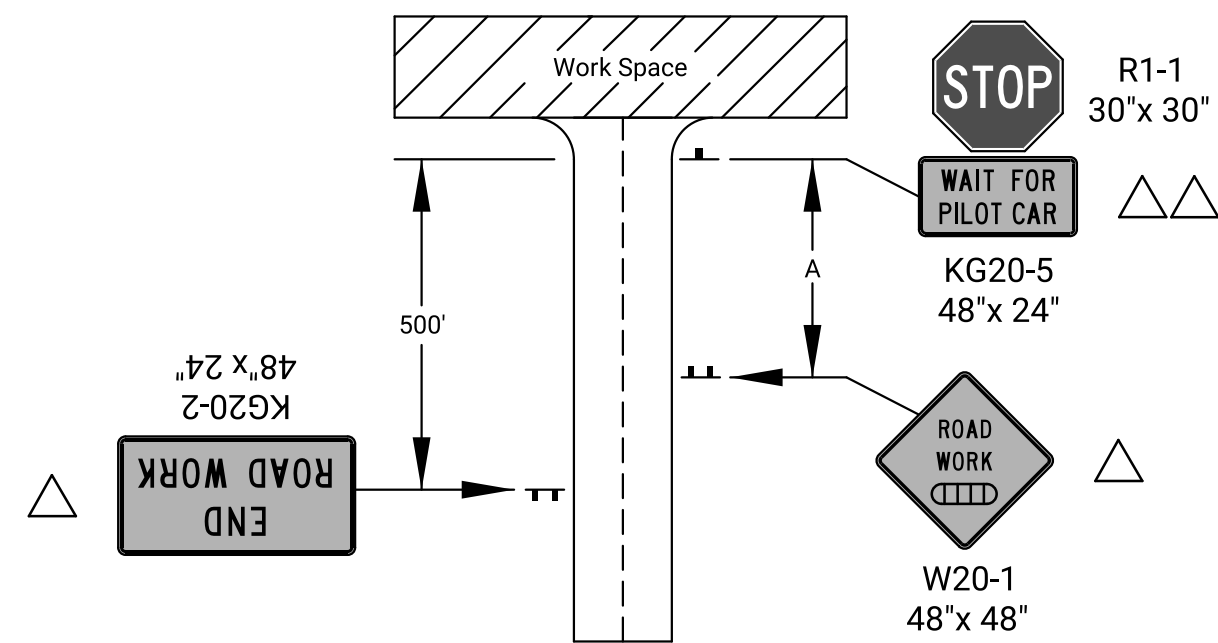
### TYPICAL SIGNING FOR HIGHWAY OR MAJOR COLLECTOR APPROACH TO WORK SPACE



## FLAGGER AND PILOT CAR



## TYPICAL SIGNING FOR A MINOR SIDE ROAD APPROACH TO WORK SPACE



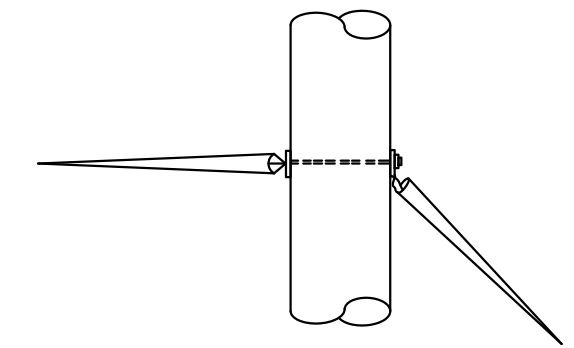
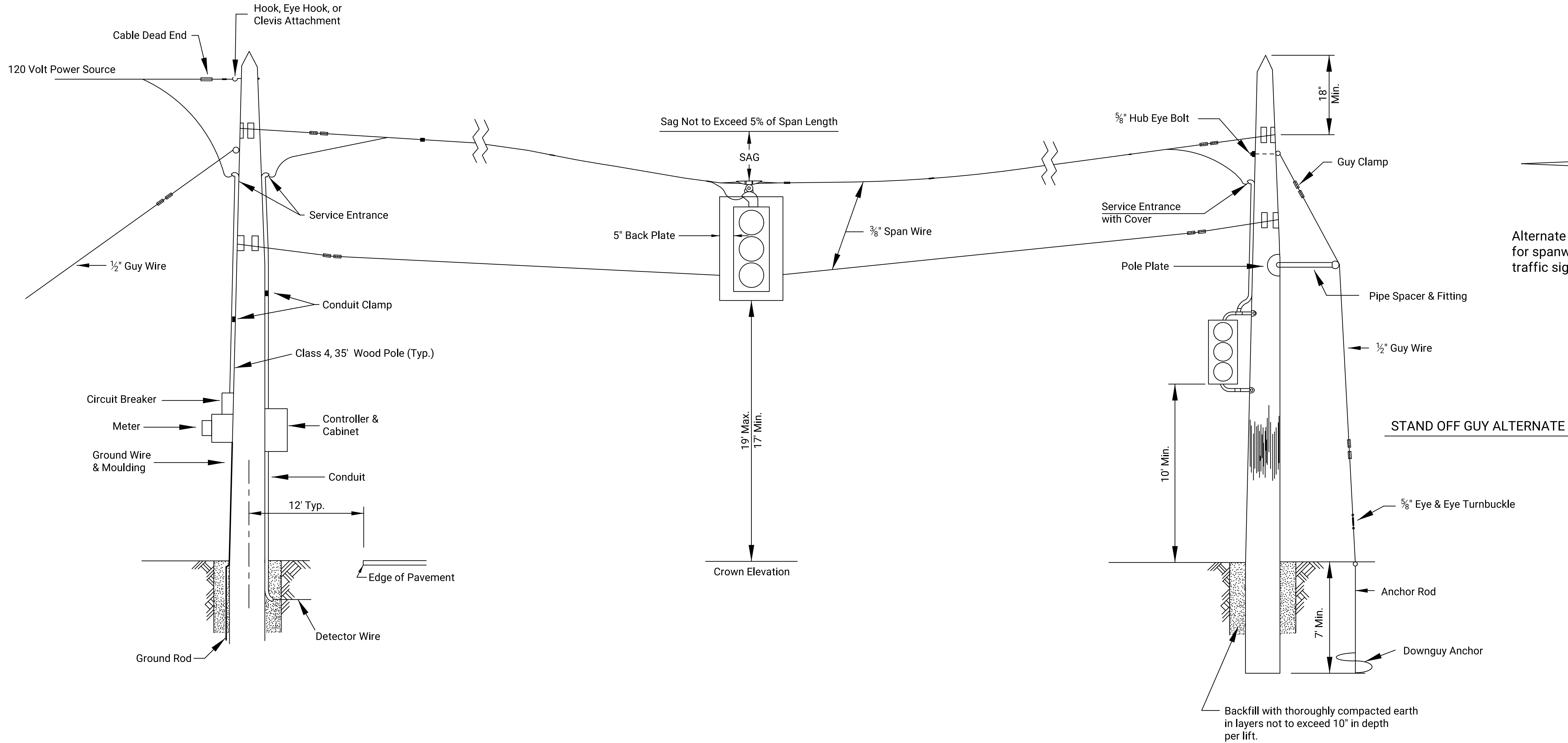
- Channelizing Device
- ▤ Ahead, 1500 ft, or 1 Mile
- ▥ Ahead, 1000 ft, 1500 ft, or ½ Mile
- ⊗ Speed to be Determined by the Engineer
- Type "A" Low Intensity Warning Light
- ▦ Temporary Portable Rumble Strips

NO.	DATE	REVISONS					BY	APP'D	
<p style="text-align: center;">KANSAS DEPARTMENT OF TRANSPORTATION</p> <p style="text-align: center;"><b>TRAFFIC CONTROL FLAGGER OR PILOT CAR</b></p> <p style="text-align: center;">TE730</p>									
FHWA APPROVAL		06-01-15		APP'D.		Kristina Ericksen			
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES		TRACED			
DESIGN CK.		DETAIL CK.		QUAN.CK.		TRACE CK.			

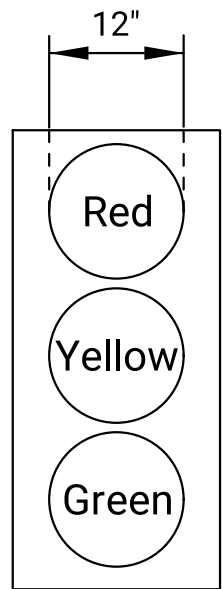




STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	116	141



Alternate method of mounting for spanwire & downguy traffic signal support pole.



SIGNAL INDICATIONS

#### GENERAL NOTES

The engineer in charge of construction will need to approve all locations for traffic signals to be installed. Final positions & aiming of signal faces to be determined in the field.

Trailer mounted portable traffic signals may be substituted for span wire signals.

The traffic signal system shall conform to and be operated according to the requirements of the M.U.T.C.D.

Contact local utility companies to advise them of installation and coordinate power hook-up if needed.

All wiring installed shall conform to the national electrical code and local ordinances & requirements.

The power supply and the operation & maintenance of the signal system shall be the responsibility of the contractor.

Note:  
See TE734 for additional information.

NO.	DATE	REVISIONS		BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL					
TEMPORARY TRAFFIC SIGNAL DETAILS					
TE733					
FHWA APPROVAL		06-01-15	APP'D.	Kristina Erickson	
DESIGNED	L.E.R.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.

The control equipment shall be designed in such a manner that the normal dwell condition shall be an all red" signal display. Upon receipt of a detector actuation from one approach, the signals facing that approach shall cycle to a green indication for a minimum period (minimum green). Subsequent detector actuations from the same direction shall result in additional green time being allocated to that movement (unit extension). In the event that an actuation exists for the direction of travel not having the right of way, a maximum green time setting shall provide a preset time limit for the direction having the right of way.

The control equipment shall provide for different clearance sequences, one for each required phase.

If the green indication has been displayed to one approach to the zone, no vehicle actuation exists on the opposite approach and another actuation occurs during the yellow display to the approach just serviced, the display shall proceed to an all red display for a period of time (red revert) to prevent the display of green - yellow - green indications to the motorist.

If the right of way is to be transferred to another approach, an all red indication shall be provided so that opposing traffic does not meet within the one way zone.

Response to a vehicle actuation from another approach shall be immediate if all timings have expired. In the event that all time settings have not expired at the point at which a vehicle actuation occurs, the system shall continue to provide the appropriate clearance interval timings before acting upon an actuation input.

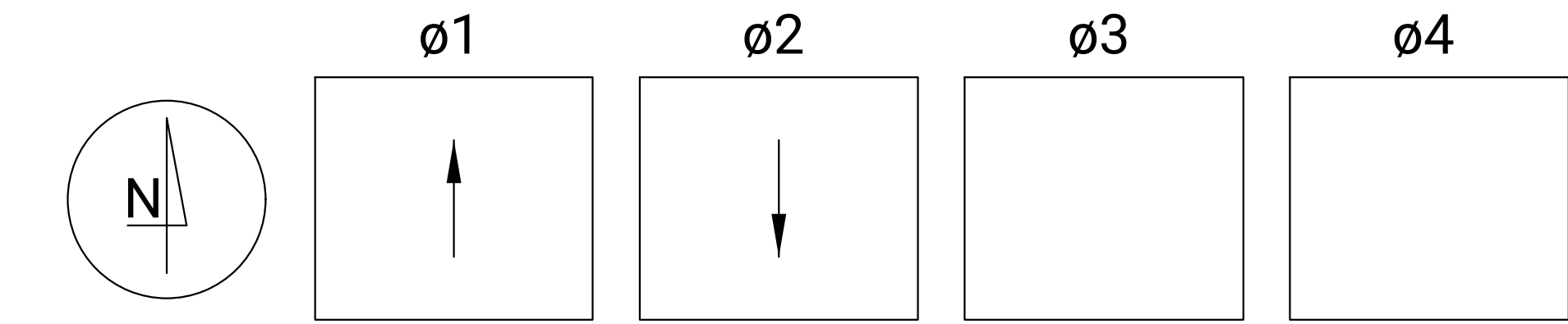
Vehicle actuations received from the detector at approaches other than that which last received a green indication shall have preference over additional actuations received from the end which last had the right of way in the event that any clearance interval timings have not expired when the actuation(s) occurs. If all timings have expired, response shall be on a first come, first served basis.

All time settings shall be user adjustable and shall be accomplished from the equipment front panel by way of a keyboard and menu screen format. All applicable portions of the KDOT standard specifications for vehicle actuation shall apply except that a standard NEMA conflict monitor shall be acceptable.

Signals shall be capable of actuation. On asphalt roadways, detection loops may be sawed into the road. Commercially made loop mats may also be used. Do not cut loops into concrete pavement. Other types of detection may be used if approved prior to installation by the Engineer. Do not use microwave detection systems in urban areas. Detector shall be set to operate in the locking mode.

If used, detection loops shall be 6' by 6' and have three turns of wire (see detail). Center loops in the lane of traffic and locate 100' behind the stop line. Cut slots in pavement for loops 5/16" wide with 1" minimum depth. Fill slots with asphalt or an approved elastic epoxy sealant (concrete pavement) to within 1/8" of pavement surface. Other than a "western union" type splice or approved connector at their junction, feeder cable and loop wire shall be of continuous run with no splices. The loop and the feeder cable connection shall be twisted 2 turns per foot.

SIGNAL PHASING AND TIMING

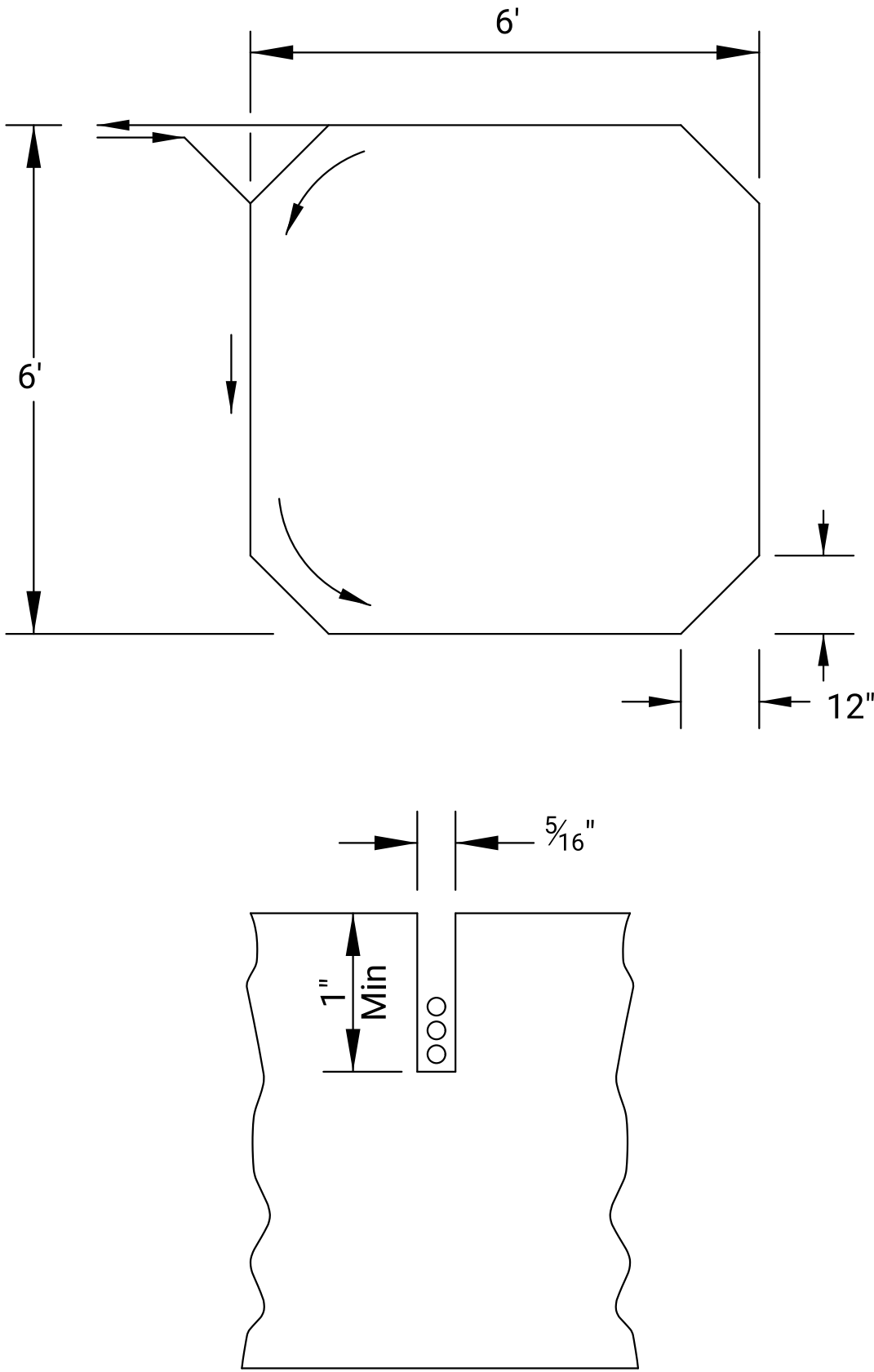


Phase	Minimum Green	Maximum Green	Yellow	All Red
2	10	60	4	92
3	10	60	4	92

Phase		Stationing
2 & 3	Stopline	43+55
2 & 3	Signal	44+35
2 & 3	Signal	88+65
2 & 3	Stopline	89+45

All times in seconds.  
Normal dwell shall be "all red".  
Unit extension shall be 3.0 seconds.  
Red revert shall be 5.0 seconds.

LOOP DETECTOR DETAIL



NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL TEMPORARY TRAFFIC SIGNAL DETAILS				
TE734				
FHWA APPROVAL		06-01-15	APPD.	Kristina Ericksen
DESIGNED	L.E.H.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.		DETAIL CK.		TRACE CK.

Note: See TE733 for additional information.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	118	141

## SUMMARY OF TRAFFIC CONTROL DEVICES (EACH)

[illegible]

## SUMMARY OF TRAFFIC CONTROL DEVICES (EACH PER DAY)

- \* Quantity most used on the project at any one time

[illegible]

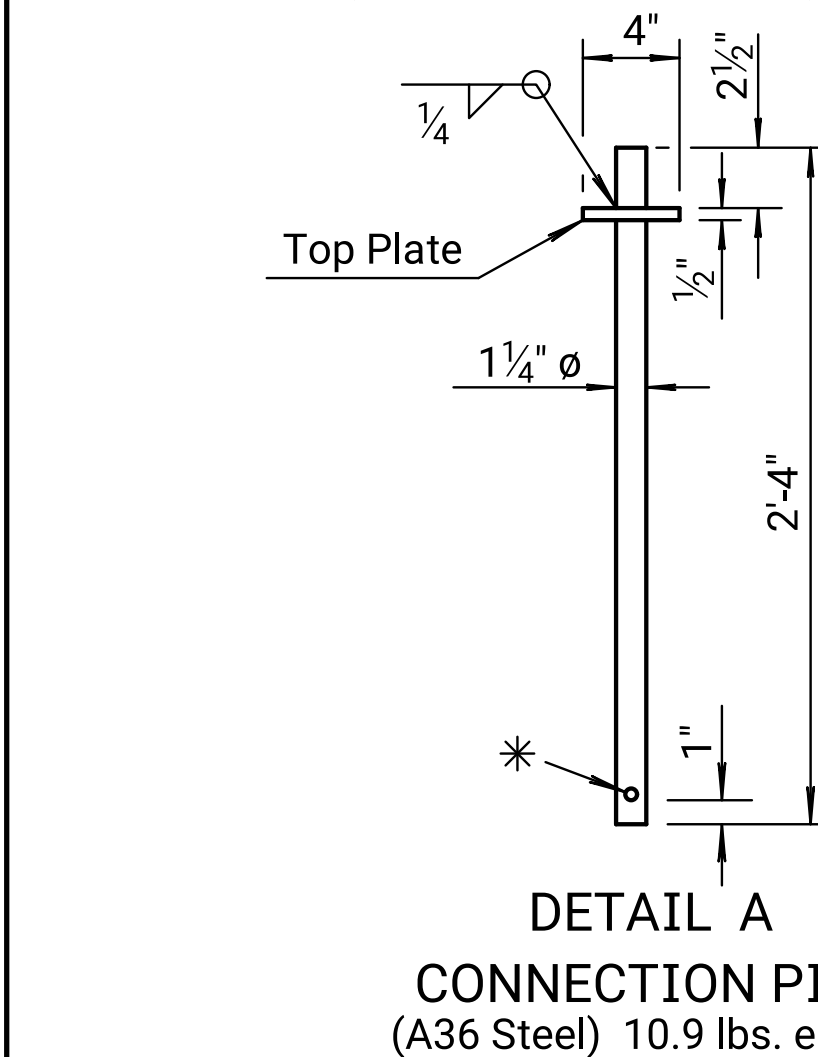
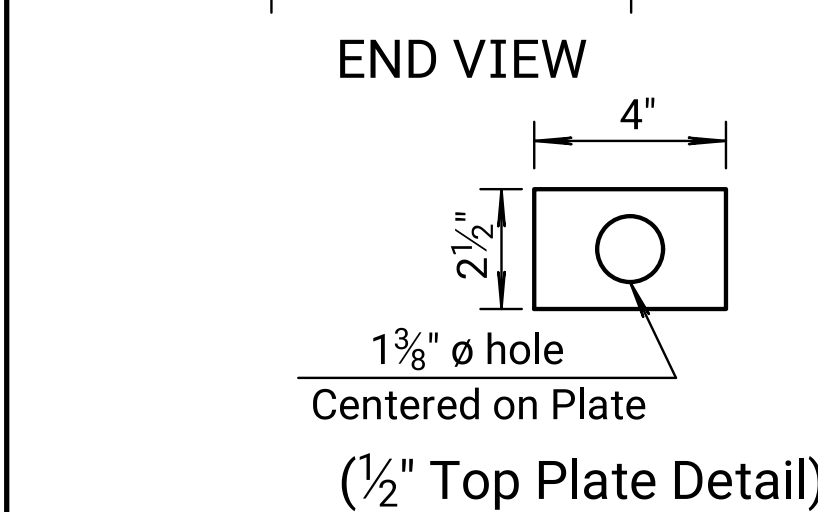
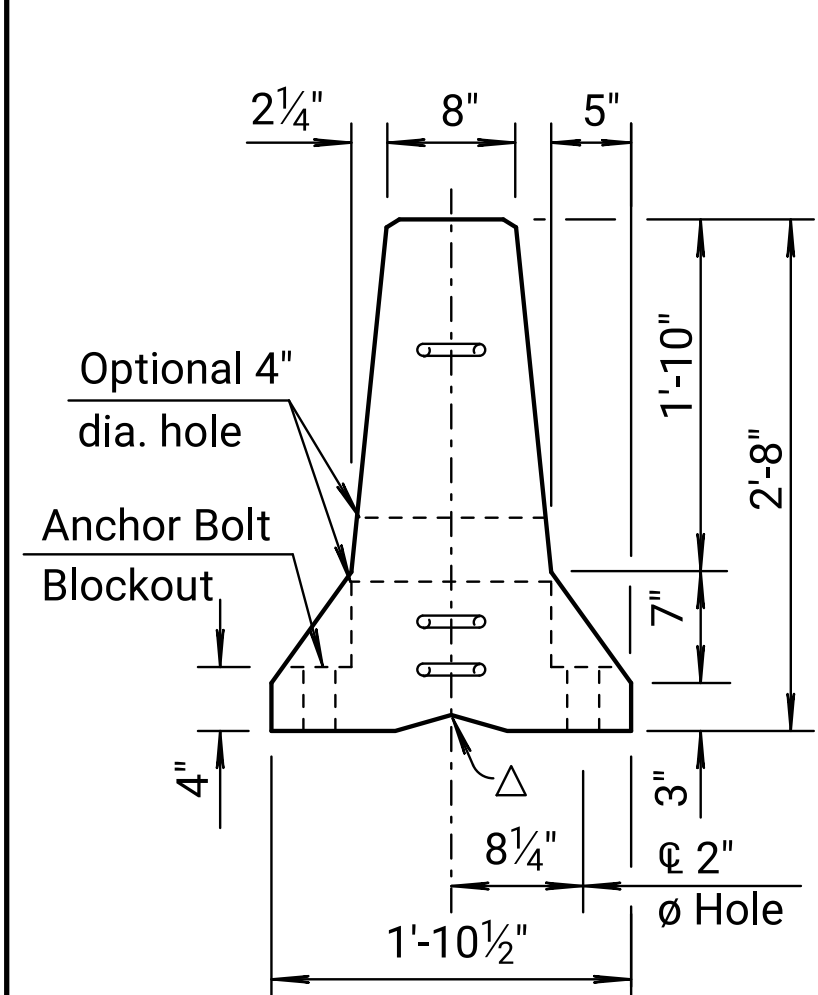
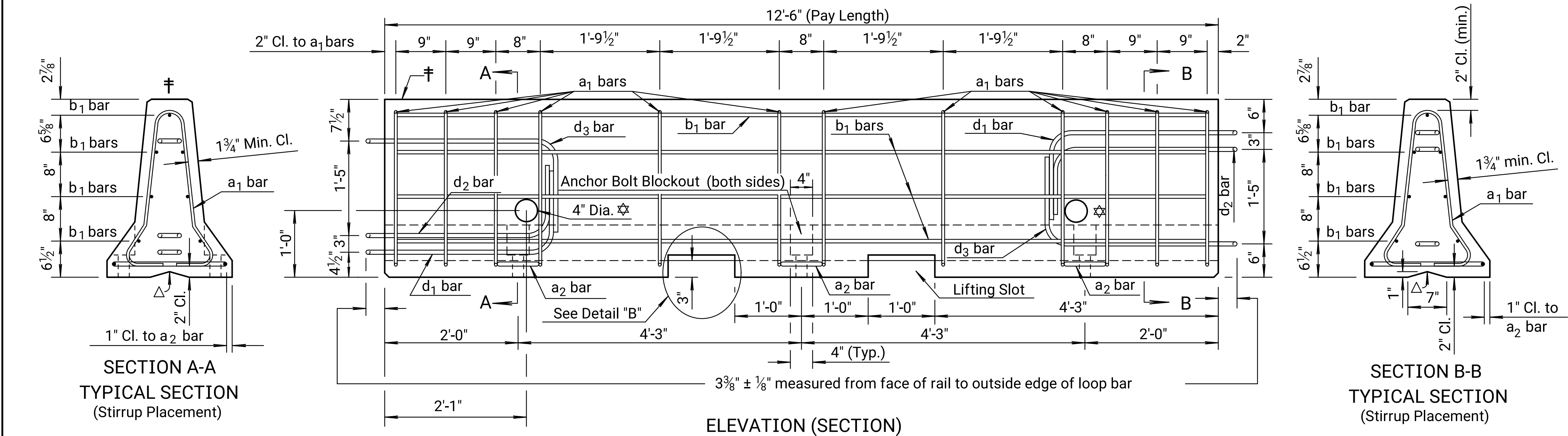
Barricades *		Channelizing Devices *		
Type 3 (4' to 12')	Pedestrian	Fixed	Portable	Pedestrian
20			85	

Lighted Devices *	
Work Zone Warning Light (Type "A" Low Intensity)	20
Work Zone Warning Light (Red Type "B" High Intensity)	
Arrow Display	
Portable Changeable Message Sign	

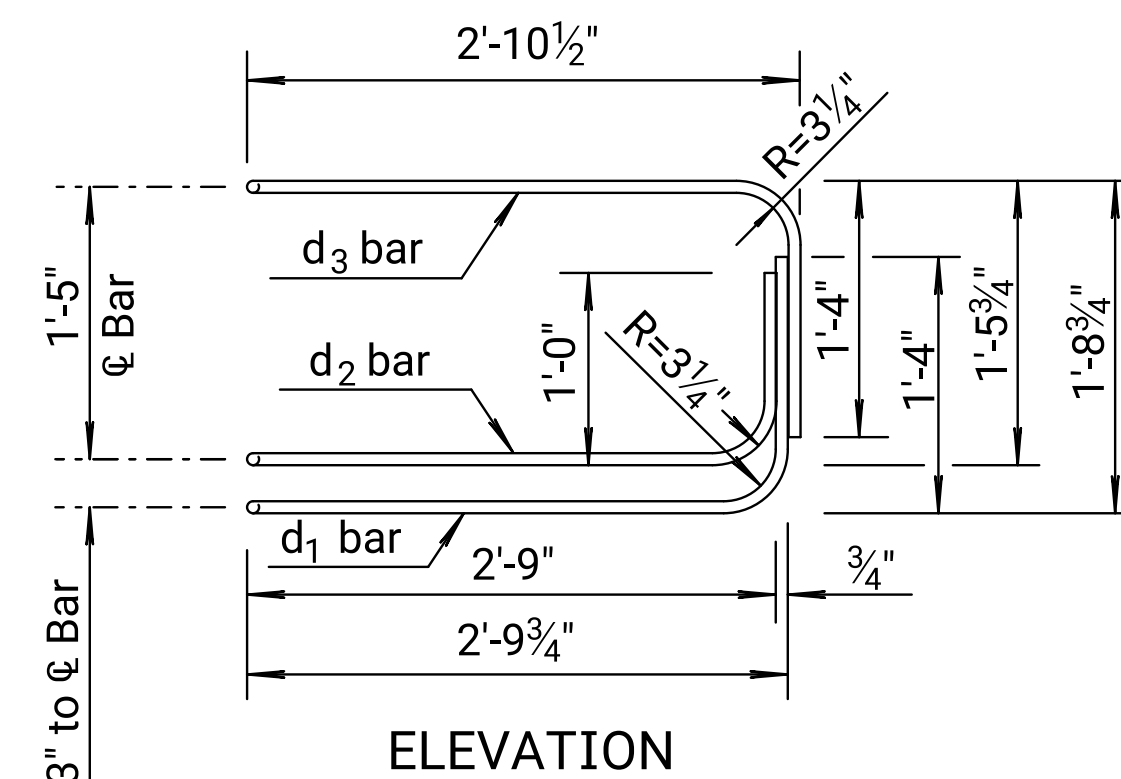
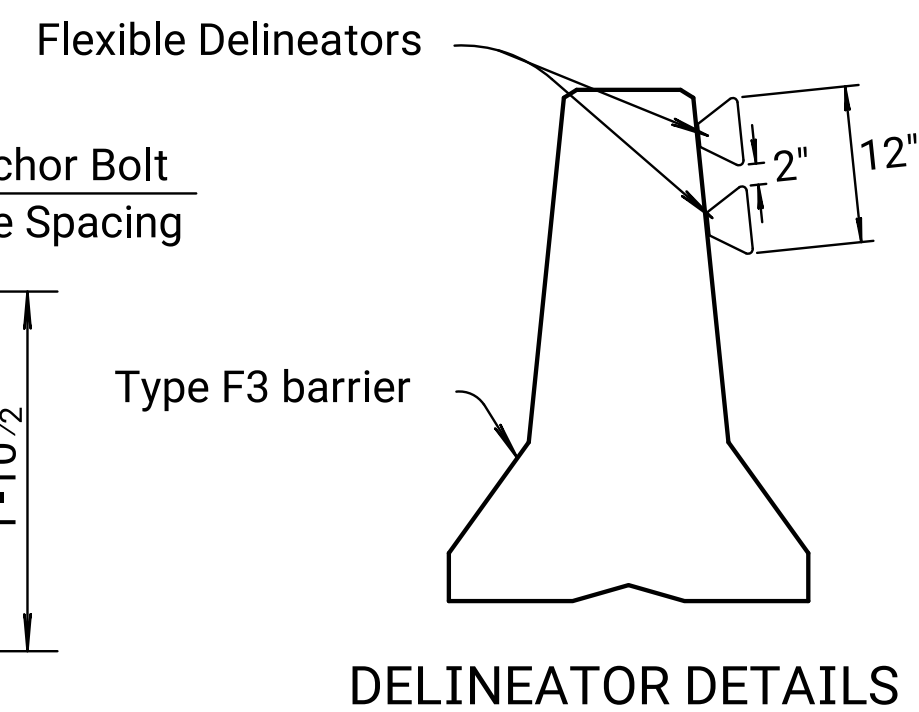
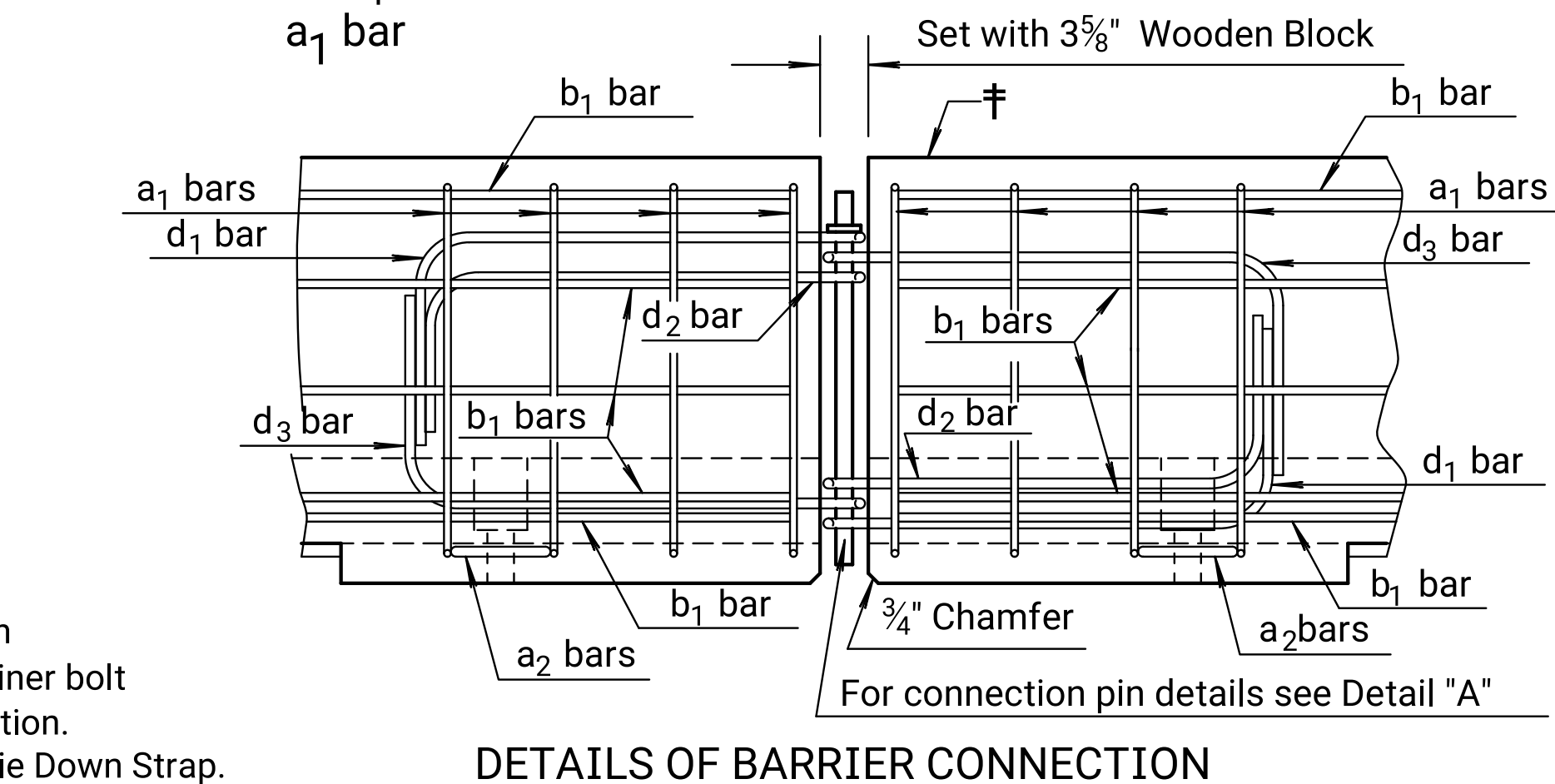
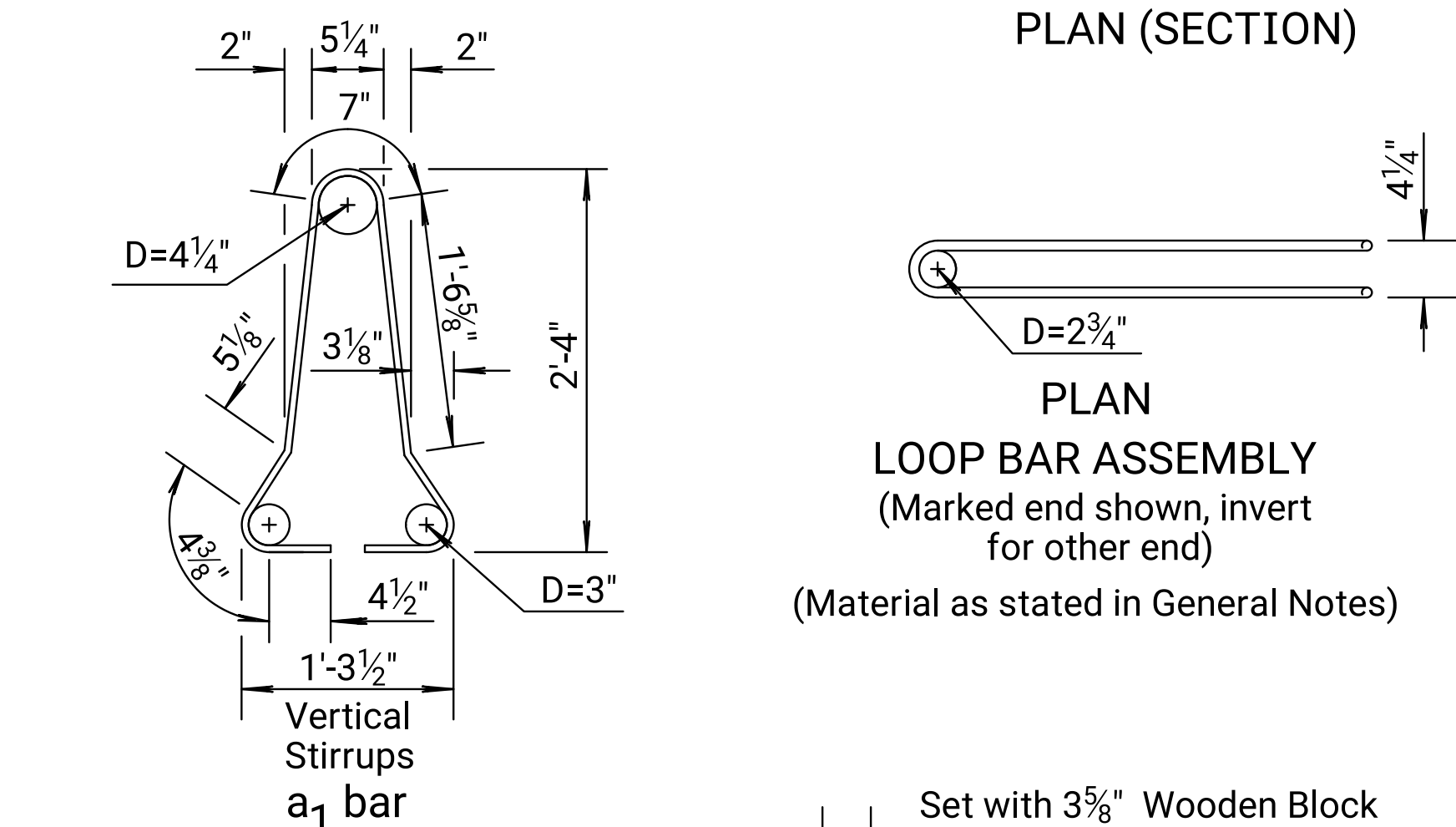
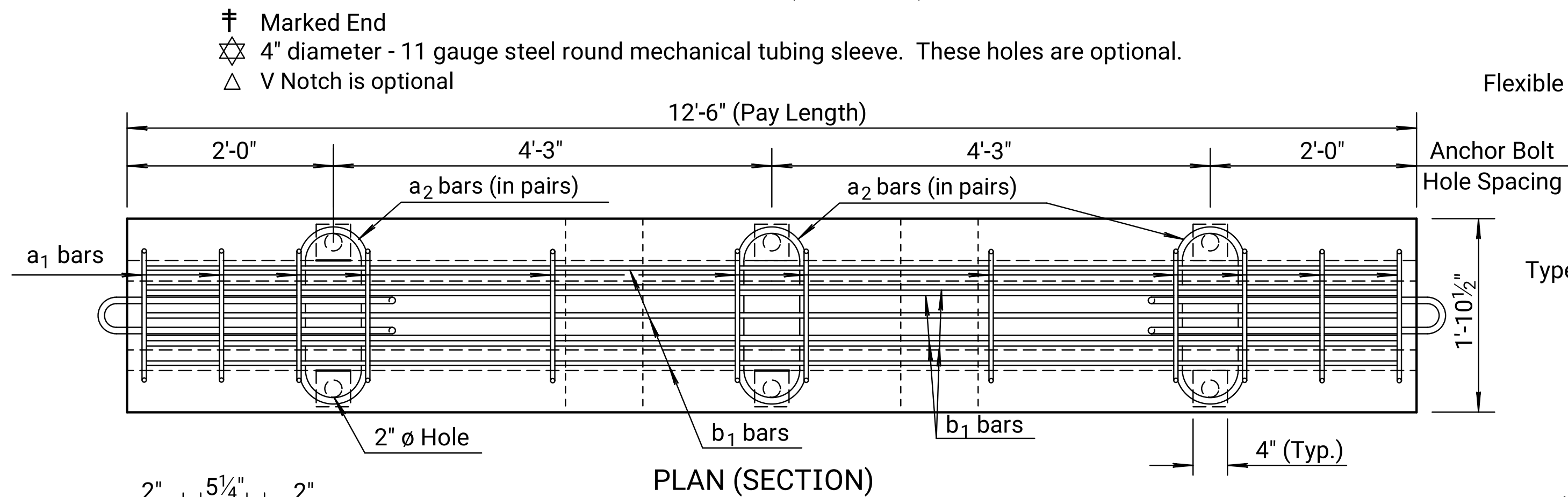
[illegible]







NO.	DATE	REVISIONS		BY	APP'D
<p style="text-align: center;">KANSAS DEPARTMENT OF TRANSPORTATION</p> <p style="text-align: center;"><b>TRAFFIC CONTROL</b></p> <p style="text-align: center;"><b>SUMMARY OF DEVICES</b></p> <p style="text-align: center;"><b>RECAPITULATION OF QUANTITIES</b></p> <p>TE795</p>					
FHWA APPROVAL		06-01-15		APP'D. Kristina Erickson	
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	119	141

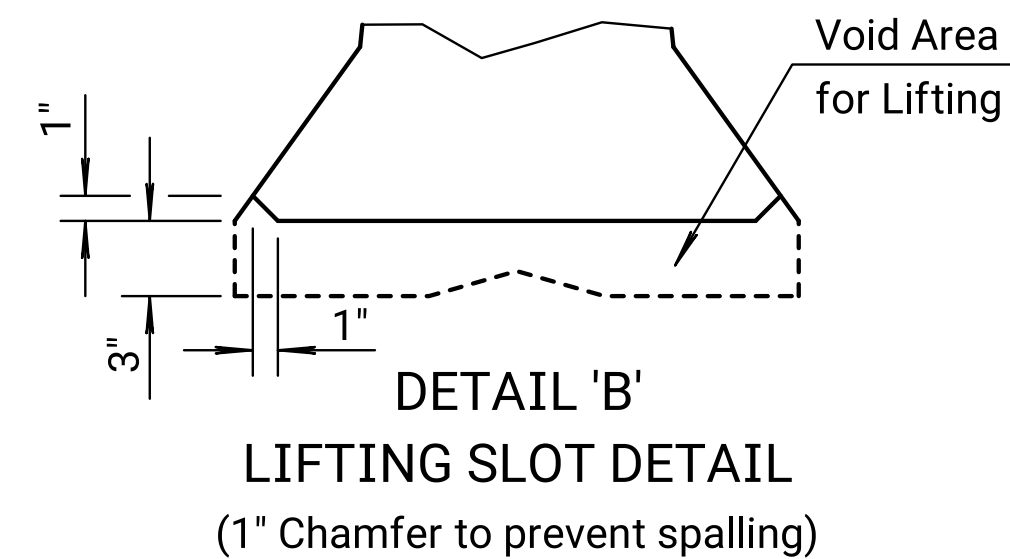


\*  $\frac{5}{8}$ "  $\varnothing$  hole for retainer bolt. The retainer bolt & nut are installed at Contractor's option.  
Note: Retainer bolt & nut required with Tie Down Strap.



Per 12'-6" Barrier Section					
REINFORCING A615 Gr. 60					
Bar	Bar Size	Shape	No. of Bars	Length Ft.	Weight Lbs.
a <sub>1</sub>	#4		12	6'-0"	48.1
a <sub>2</sub>	#6		6	2'-11"	26.3
b <sub>1</sub>	#5		7	12'-2"	88.8
LOOP ASSEMBLY					
d <sub>1</sub>	#6		2	8'-5"	25.3
d <sub>2</sub>	#6		2	7'-7"	22.8
d <sub>3</sub>	#6		2	8'-6"	25.5

Concrete Quantity = 1.3 C.Y.  
(Dimensions are out to out of bars unless otherwise noted.)



NOTE: At no time shall the barriers be lifted, moved, etc.  
by use of the loop bars:  $d_1$ ,  $d_2$  or  $d_3$ .

**GENERAL NOTES:**  
**MATERIAL:** Use ASTM A615, Grade 60 reinforcing bars, except for the loop bars ( $d_1$ ,  $d_2$  and  $d_3$ ).  
 The loop bars ( $d_1$ ,  $d_2$  and  $d_3$ ) shall be  $\frac{3}{4}$ " smooth steel bars with a minimum yield of 60 ksi, a tensile strength of not less than 1.25 times the yield strength but a minimum of 80 ksi, a minimum 14% elongation in 8 inches, and passing a 180 degree bend test using a 3.5 D pin bend diameter. The loops shall be installed within  $\frac{1}{8}$ " of the plan dimensions.

Use air-entrained concrete with  $f'_c = 5,000$  p.s.i.

SECTION: The section furnished must generally comply with dimensions shown. Requests for minor variations in section geometry and attachments may be submitted to the Engineer for approval.

**LIFTING SLOTS:** Lifting slots shall be constructed where specified on the plans to facilitate the drainage of water after installation on the roadway.

**TEMPORARY CONCRETE SAFETY BARRIER:** Furnishing and placing of all materials when required and all labor and equipment required to position the temporary barrier shall be included in the Contract unit price bid for "Concrete Safety Barrier (Type F3)(Temporary)". Any relocation of the barrier required for the project shall be paid in accordance with the Special Provisions under the bid item "Concrete Safety Barrier (Type F3) (Temporary-Relocate)". Unless otherwise noted on the Plans, the Temporary Concrete Safety Barrier shall become the property of the Contractor and shall be removed from the site upon acceptance of the completed project.

Approximate weight of one unit equals 2.7 tons.

**PLACEMENT:** Barrier shall be placed on a paved surface. All loose dirt and sand shall be removed from the roadway surface just prior to placement of the barrier.

After the barrier is placed and the connection pin is inserted, tension or pull the barrier such that the installation is taut and the connection pin cannot freely move vertically. If the connection pin or loop bar assembly are damaged during the tensioning process, it is the responsibility of the Contractor to repair the damaged area or replace the temporary barrier section.

**MARKING:** The left end (†) of each barrier shall be permanently marked by stamping or forming into the barrier the following information:

- Type F3
- Manufacturer code (as specified by KDOT Bureau of Const. & Maint.)
- Date manufactured (month and year)

**DELINERATION:** Delineators shall be spaced on 50' centers, except through curves where they shall be spaced on 25' centers. See Standard Drawing RD610 for additional details.

The delineation shall be mounted on the side of the Temporary Concrete Safety Barrier with two delineators at each location. Each delineator shall have a minimum height-to-width ratio of 1.75, and a minimum reflective surface area of 7 sq. in.. The delineators shall be affixed to the Temporary Concrete Safety Barrier as recommended by the manufacturer.

Delineators shall be attached to bridge rail or other structures in construction zones when roadway is narrowed and traffic is adjacent to the structure. The method and location of placement shall be similar to permanent barrier delineation.

When traffic flow is in one direction, the delineators shall be yellow when used on the left, white when used on the right. When traffic flow is in both directions delineators shall be placed back-to-back, and shall correspond to the color of the edge line.

The work and materials required for the installation of delineators as mentioned shall be subsidiary to the bid item "Concrete Safety Barrier (Type F3) (Temporary)".

If necessary, include Standard Drawing RD622A for Taper Section, Standard drawing RD622B for anchor and tie down details, Standard Drawing RD622C for Bridges with thermal expansion of 1½" or greater and Standard Drawing RD622D for Barrier Layouts.

The Contractor shall be responsible for maintaining a clear area, shown as dimension "A" on Standard Drawing RD622B. The clear area is located behind the Temporary Concrete Safety Barrier and shall be kept free of any equipment, material stockpiles or other obstacles. For non-anchored roadway applications, dimension "A" shall be a minimum of 2'-0".

07	09-11-17	Revised Markers	A.L.R.	S.W.K.
06	07-17-17	Revised General Note	A.L.R.	S.W.K.
05	08-27-15	Added Note, Pay Length	K.E.K.	S.W.K.
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY  
CONCRETE SAFETY BARRIER  
TYPE F3

RD622

FHWA APPROVAL		03-05-18	APP'D.		Scott W. King
DESIGNED	DETAILED		QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.		QUAN. CK.	TRACE CK.	

KDOT Graphics Certified 05-12-2022

Sh. No. 119



Note to Designer: For use on Haunched slab bridges, the Road Designer shall coordinate with the Bridge Designer for "corridor in the reinforcing steel layout to accommodate barrier anchoring". Road Designer shall coordinate barrier layout with Bridge Designer to accommodate for expansion during construction.

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:40  
File : KA572901rss622b-01.dgn

GENERAL NOTES:  
INSTALLATION: Holes into the pavement to anchor the concrete safety barrier may be drilled after positioning barrier. When anchoring with 3 bolts on traffic side, install barrier with through anchor bolt where possible, use grouted anchor bolts where through bolt can't be used. Do not drill into or otherwise damage support beams, girders, or expansion joints. All work & materials required for the installation of the anchors are subsidiary to the bid item "Concrete Safety Barrier".  
UTILITIES & STRUCTURES (Stakes): Verify buried utilities & structures within stake depth. If conflicts between stake & buried elements exist, up to 2 stakes maximum in a single barrier may be omitted if adjacent barriers have 3 stakes each.

ANCHORAGE: Use galvanized grouted anchor bolts, through anchor bolts, nuts & washers that meet standard specifications. Install 3 anchor bolts or asphalt pins per barrier on the traffic side except on transition barrier as shown.  
BARRIER REMOVAL: Completely remove all anchor systems. Remove grouted or drop-in anchor system by drilling the anchor with a core barrel 2x the diameter of the insert. Core to a depth equal to the installed depth & remove the core, prepare the hole by removing dust & debris. Fill hole with material that meets KDOT Pre-qualified "Non-shrink grouts for grouting anchor bolts & reinforcing into previously poured concrete". Follow the manufacturer's procedures for mixing, hole preparation & curing. To fill through bolt anchor or screw-in anchor system, remove & completely fill the hole using instructions for drop-in

anchors except no coring is required.  
For removed or relocated barrier on flexible pavement, fill stake holes completely with hot or cold asphalt patch material. Work & materials required to remove & patch anchor holes are subsidiary to the bid item "Concrete Safety Barrier".  
TEMPORARY BARRIERS: Temporary Barriers shown in the details of this drawing are not allowed for permanent installations.  
See KDOT Standard Drawing "Temporary Concrete Safety Barrier Type F3 Transition Layouts" for transition details between anchored & free-standing barriers. See KDOT Standard Drawing "Temporary Concrete Safety Barrier Type F3" for details & quantities not shown on this sheet.  
SIGNING: For sign spacing, traffic control device details & reference notes, see Index of Sheets.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	120	141

NO ANCHORAGE REQUIRED unless shown on plans

"A" DISTANCE	A ≥ 4'	A ≥ 2'
LOCATION	Span Bridge	Box Culverts Roadways - Flexible or Rigid Pvmnt.

Traffic Side

Edge of deck

A

Type F3 barrier

⊗ Applies on span bridges when the action creates a height differential of ≤ 2". Measured from the toe of the barrier, the "A" distance should be free of obstacles and equipment.

Traffic Side

Type F3 barrier

≤ 2"

Pavement or Span Bridge

Traffic Side

Type F3 barrier

≤ 2"

Pavement or Span Bridge

BARRIER LOCATION NEAR HEIGHT DIFFERENTIAL

ANCHORAGE	ANCHOR BARRIER with 3 BOLTS ON TRAFFIC SIDE	ANCHOR BARRIER with 3 BOLTS ON TRAFFIC SIDE	ANCHOR BARRIER with TIE-DOWN STRAP	ANCHOR BARRIER with STAKES
"A" DISTANCE	0' ≤ A < 2'	0' ≤ A < 2'	Δ 2' ≤ A < 4'	6" ≤ A < 2'
LOCATION	Span Bridge	Span Bridge Box Culverts Roadways - RIGID Pavement	Span Bridge Box Culverts Roadways - RIGID Pavement	Box Culverts (ceiling below stake depth) Roadways - FLEXIBLE Pavement

THROUGH BOLT (Preferred)

1 1/8" Ø Anchor Bolt with Heavy Hex Nut or Threaded Rod With Alternate Top (ASTM A307 or F1554 Grade 55)

3" x 3" x 1/2" Square Washer (A36)

2" Ø bolt

2" Ø hole

1/2" x 4" x 4" Square Washer (A36)

1 1/8" Ø Heavy Hex Jam Nut

WITH EXISTING ASPHALT OVERLAY

27/8" x 5/8" x 1 5/8" Cold Drawn DOM steel tube (fy = 72 ksi, min.)

Asphalt overlay

3" Ø hole through asphalt and concrete bridge deck

ALT. DRILLED AND GROUTED ANCHOR

⚡Note: Alternate Drilled and Grouted Anchor installation avoids damage to the support beams, girders or expansion joint. The State Bridge Office shall approve the use of the Alternate Drilled and Grouted Anchor installation for bridge applications.

1 1/8" Ø Anchor Bolt or Threaded Rod Alternate (ASTM A307 or F1554 Grade 55) with 5 1/2" or longer embedment per Manufacturing Recommendation (f'c= 4 ksi min. Conc.) to develop ultimate strength of anchor bolt or threaded rod.

5 1/2" min.

2" Ø hole

Manufacturer Recommended Grout or cement

TIE-DOWN STRAP

3/4" Ø x 1 3/4" ASTM 449 Bolt with Red Head 3/4" drop in anchor, Red Head large diameter Tapcon (LDT) 3/4" Ø x 4 1/2" long, or Simpson Titen HD 3/4" Ø x 5" long

Δ "A" distance may be reduced to 6" if traffic does not travel under the bridge.

With prior approval from the State Bridge Office, anchoring the barrier with 3 bolts (Through Bolt OR Drilled & Grouted Anchor options) on the traffic side may be used in lieu of tie-down straps.

STAKED DOWN

⊗ Pre-drill 1 1/2" Ø holes in flexible pavement prior to installing stakes.

STAKE DETAIL

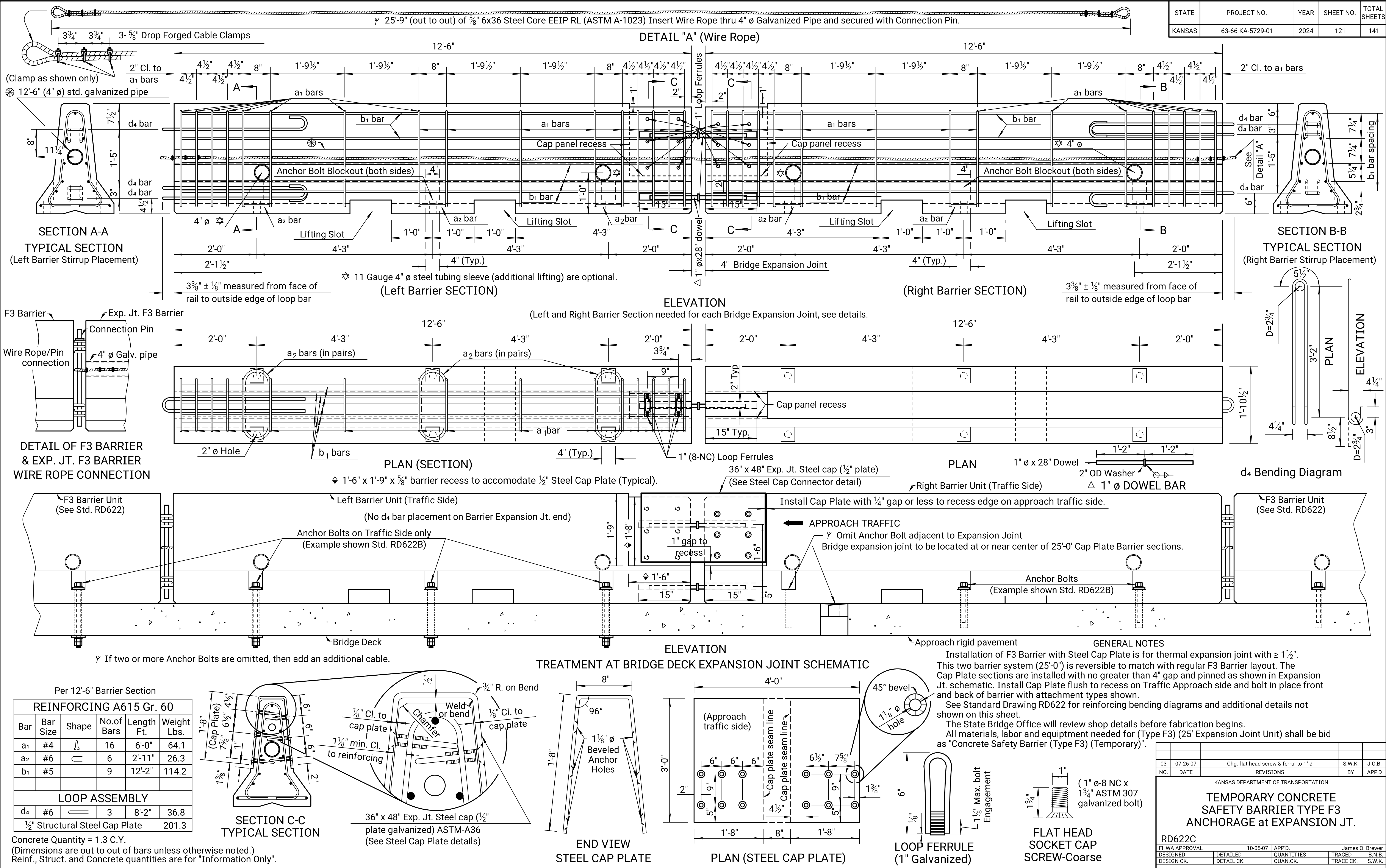
STOP PLATE DETAIL

07	04-21-21	Revised Layouts for Br. Deck & Road Pvmnt. Apps	A.L.R.	S.W.K.
06	12-31-13	Rev. Note (Alt. Drill. & Grout. Anch.)	S.W.K.	J.O.B.
05	06-27-11	Revised General Note	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
RD622B				
TEMPORARY CONCRETE SAFETY BARRIER TYPE F3 ANCHORAGE				
FHWA APPROVAL 04-21-21 APPD. Scott W. King				
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	
KDOT Graphics Certified 05-13-2022 Sh. No. 120				

Note to Designer: This F3 Barrier Anchorage at Expansion Joint is only for use on bridges with thermal expansion of 1/2" or greater at the recommendation and review of Bridge Designer. Bridges longer than 1,000 feet require a Special Design.

Plotted by : Brogan Andrews@ks.gov  
File : KA572901rss622c-01.dgn  
16-APR-2024 17:35

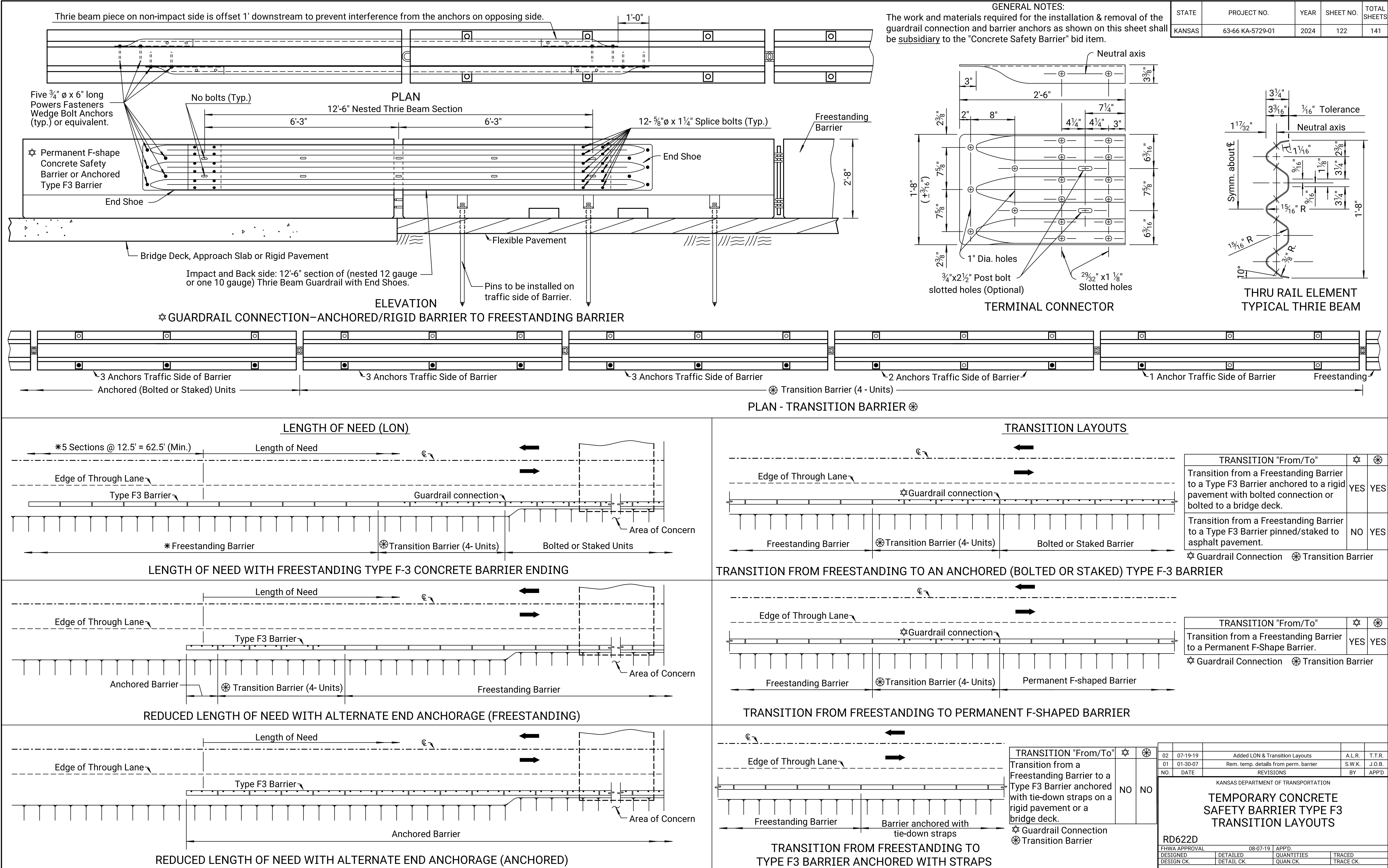
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	121	141



NO.	DATE	REVISIONS	BY	APPD.
03	07-26-07	Chg. flat head screw & ferrul to 1" ø	S.W.K.	J.O.B.
KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY CONCRETE SAFETY BARRIER TYPE F3 ANCHORAGE at EXPANSION JT.				
RD622C				
FHWA APPROVAL 10-05-07 APPD. James O. Brewer				
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	S.W.K.
DOT Graphics Certified 05-13-2022 Sh. No. 121				



\* Note to Designer: The minimum length for Freestanding Temporary Concrete Safety Barrier (TCSB) installations is 11 sections @ 12.5' = 137.5'. In a Freestanding TCSB installation, if both ends are anchored, fewer than 11 sections of Freestanding TCSB may be used. The 5 sections of Freestanding TCSB beyond the length of need may be reduced to 3 sections if the barrier is located on the exit end and traffic is traveling in a single direction.



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	123	141

GENERAL NOTES

Details shown on this sheet for ABSORB-M are for Information Only and may not be an exact detail of ABSORB-M. See the Manufacturer's Installation Manual furnished to the Engineer for details of components and installation for the ABSORB-M.

ABSORB-M is an anchorless system designed to perform on a variety of foundations including concrete, asphalt, and any other surfaces capable of bearing the weight of the system with a maximum cross-slope of 8.0%. Contact the manufacturer for slopes greater than 8.0%.

The installation area should be flattened and free from large debris. The ABSORB-M system should be approximately parallel with the barrier or ¢ of merging barriers. Maintain a clear area 20' parallel from the back of barrier for a distance of 75' behind ABSORB-M Terminal Assembly free of stockpiled material, equipment, temporary signs or obstructions regardless of crash worthiness. Do not install Absorb-M Impact Attenuator in Narrow Medians, on Elevated Structures or where Clear Area can't be achieved.

The front element of the ABSORB-M system should be left empty, while the remaining system is to be filled with water. Installation in Kansas requires an anti-freeze solution to prevent the water from freezing. See the Manufacturer's Installation Manual for acceptable anti-freeze solutions.

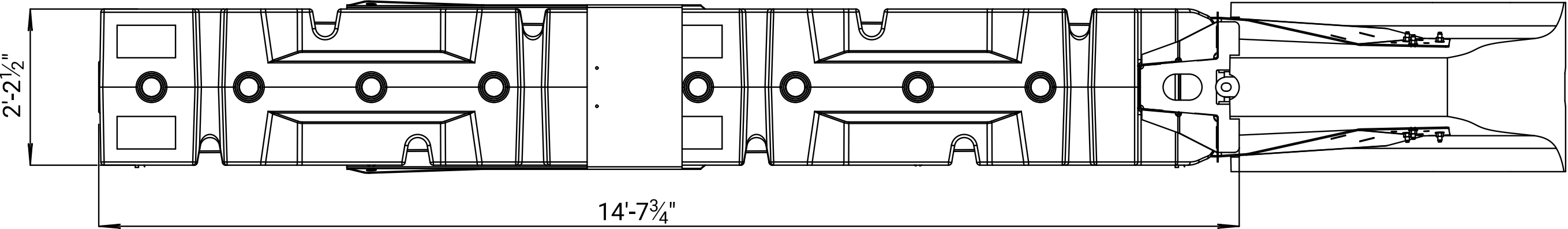
The unit shown on this sheet abuts a barrier up to 24" wide. The ABSORB-M TL2 & TL3 can be connected to permanent or temporary safety shape, constant slope, and other shapes of barriers up to 42" height. Refer to the Manufacturer's Installation Manual for these connections. Ψ Anchorage holes must be drilled to depth as per the Manufacturer's Installation Manual and cleared of debris to achieve proper anchorage.

For system relocation information, see Manufacturer's Installation Manual.

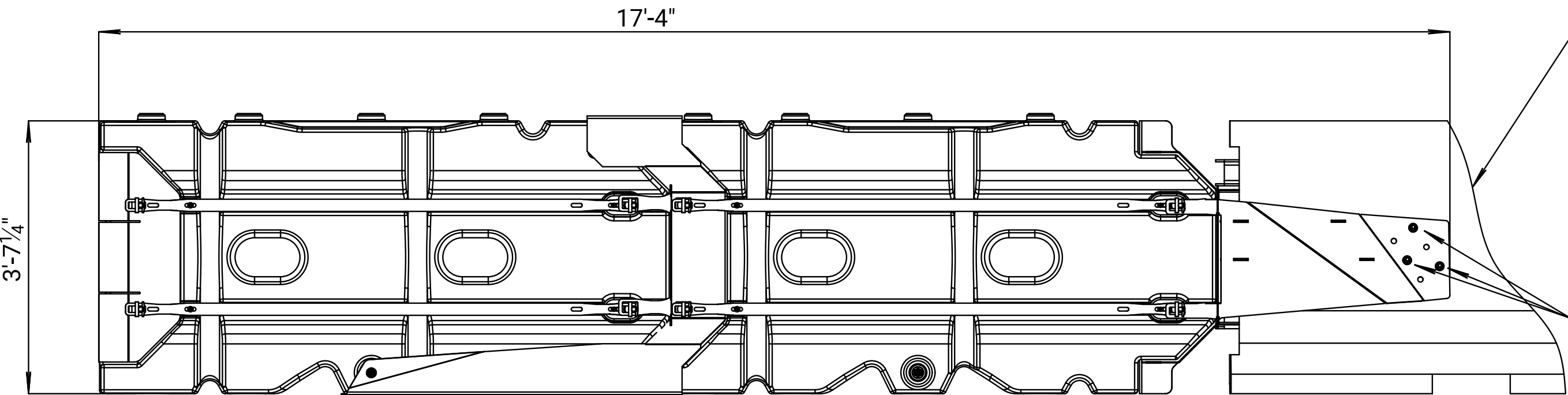
All work and material required for installation of this attenuator shall be paid under the bid item "Impact Attenuator (Temporary)(TL-2 or TL-3)".

Design Parameters	
Design Speed (mph)	Flare Rate (a:b)
70	15:1
60	14:1
55	12:1
50	11:1
45	10:1
40	8:1

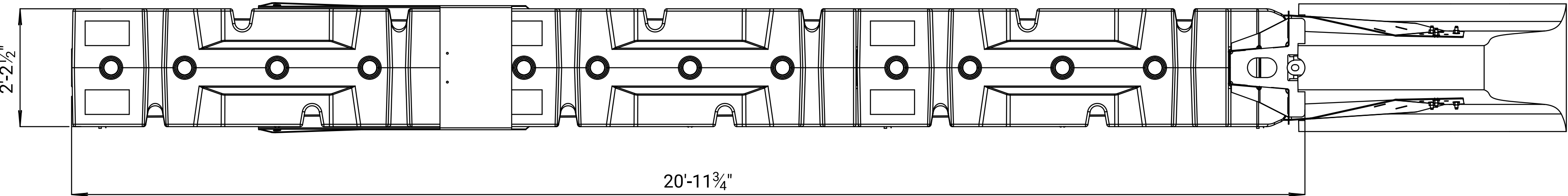
01	04-15-22	Initial Release	A.L.R.	S.W.K.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY IMPACT ATTENUATOR ABSORB-M				
RD621A				
FHWA APPROVAL		04-21-22	APPD.	Scott W. King
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	



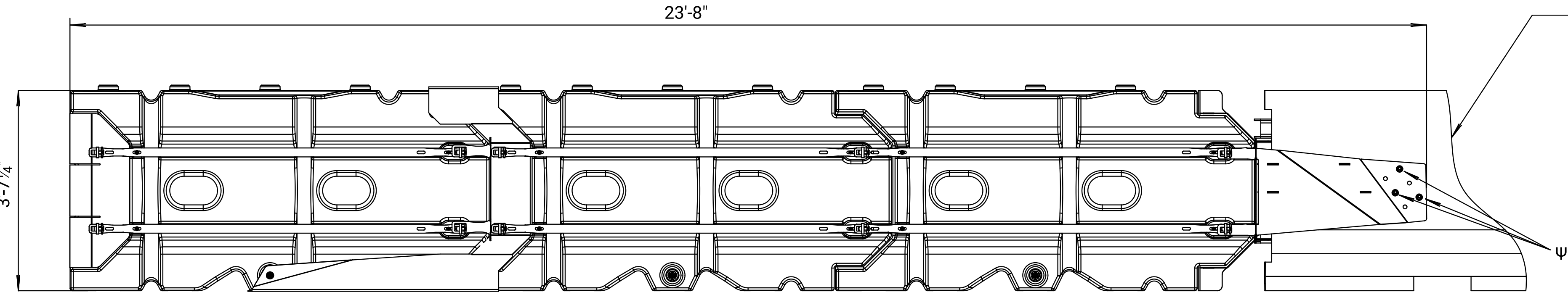
PLAN (TL2 ≤ 45 mph)



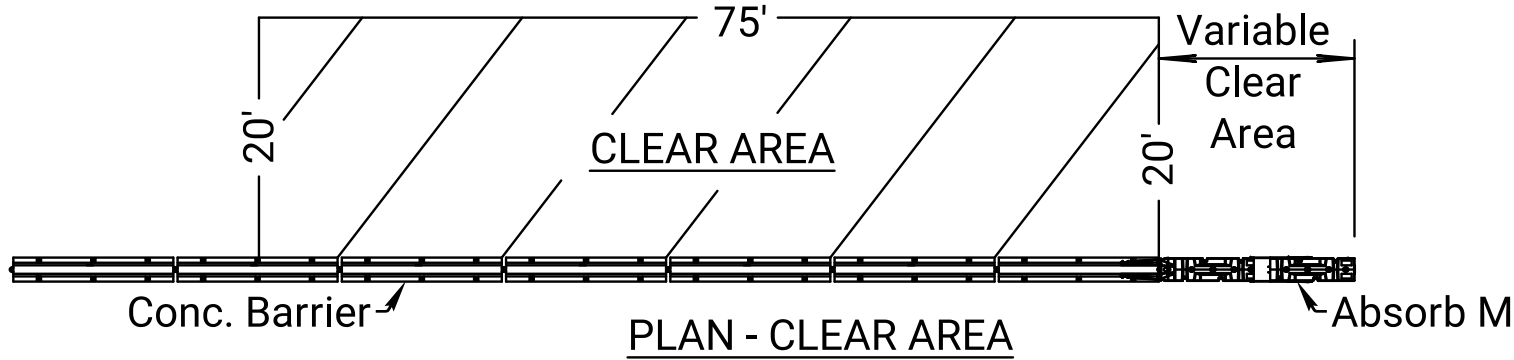
ELEVATION (TL2 ≤ 45 mph)



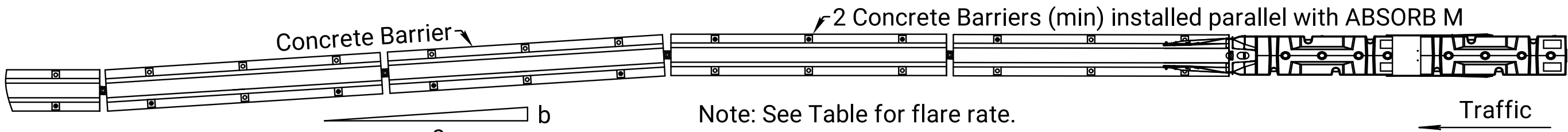
PLAN (TL3 > 45 mph)



ELEVATION (TL3 > 45 mph)



PLAN - CLEAR AREA



PLAN - FLARED LAYOUT OPTION



**Note to Designer:** Report the quantity for temporary concrete safety barrier in linear feet. The quantity is calculated by multiplying the number of barrier units by 12'-6".

Plotted by : Brogan.Andrews@ks.gov 16-APR-2024 17:30  
File : KA572901rss052-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	124	141

SUMMARY OF QUANTITIES									
			☉ CONCRETE SAFETY BARRIER (TYPE F3)					IMPACT ATTENUATOR (TL-3) (TEMPORARY)	COMMENTS
LOCATION (ROUTE)	STATION (OR STATION RANGE)	SIDE	FLARE RATE (WHERE APPLICABLE)	TEMPORARY	TEMPORARY-RELOCATE	TEMPORARY-INSTALL ONLY	UNIT		
PHASE 2									
K-63	48+00	LT						1	
K-63	48+00 to 48+25	LT		25			L.F.		
K-63	48+25 to 49+00	RT	14:1	75			L.F.		
K-63	49+00 to 85+00	RT		3,600			L.F.		
K-63	85+00 to 85+75	RT	14:1	75			L.F.		
K-63	85+75 to 86+00	LT		25			L.F.		
K-63	86+00	LT						1	
PHASE 3									
K-63	48+00	RT						☆ 1	
K-63	48+00 to 48+25	RT			25		L.F.		
K-63	48+25 to 49+00	LT	14:1		75		L.F.		
K-63	49+00 to 85+00	LT			3,600		L.F.		
K-63	85+00 to 85+75	LT	14:1		75		L.F.		
K-63	85+75 to 86+00	RT			25		L.F.		
K-63	86+00	RT						☆ 1	
</									

● The quantity reported does not include the 3<sup>5</sup>/<sub>8</sub>" gap between 12'-6" sections of barrier. The 3<sup>5</sup>/<sub>8</sub>" gap will not be included in the pay length for Concrete Safety Barrier (Type F3) (Temporary).

☆ Relocated from Phase 2.

See the Summary of Quantities on Sheet No. 67 for Recap of Temporary Concrete Safety Barrier and End Treatments.

Design Parameters	
Design Speed (mph)	Flare Rate (a:b)
70	15:1
60	14:1
55	12:1
50	11:1
45	10:1
40	8:1
30	7:1

Note: The flare rates listed here apply only to temporary concrete safety barrier installations. See temporary concrete safety barrier layouts included in the plans for variations. Typical alternate flare rates may be used as approved by the Engineer.

01	02-11-15	Initial Release				K.E.K.		S.W.K.	
NO.	DATE	REVISIONS				BY		APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION									
<p style="text-align: center;">SUMMARY OF QUANTITIES          TEMPORARY CONCRETE SAFETY          BARRIER AND END TREATMENTS</p>									
RD0052									
FHWA APPROVAL		09-16-15		APP'D.		James O. Brewer			
DESIGNED		DATED		QUANTITIES		TRACED			
DESIGN CK.		DETAIL CK.		QUAN. CK.		TRACE CK.			

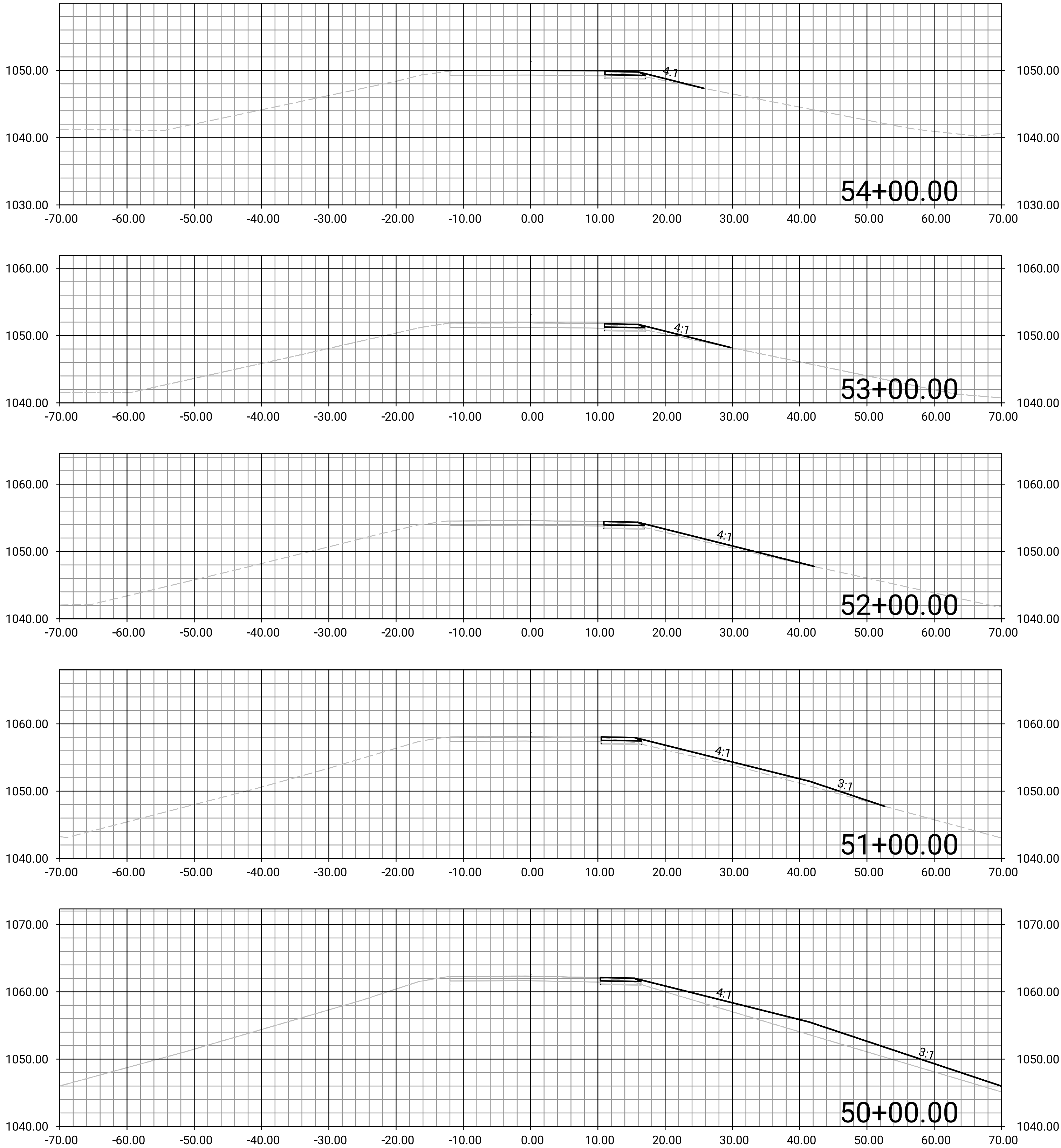
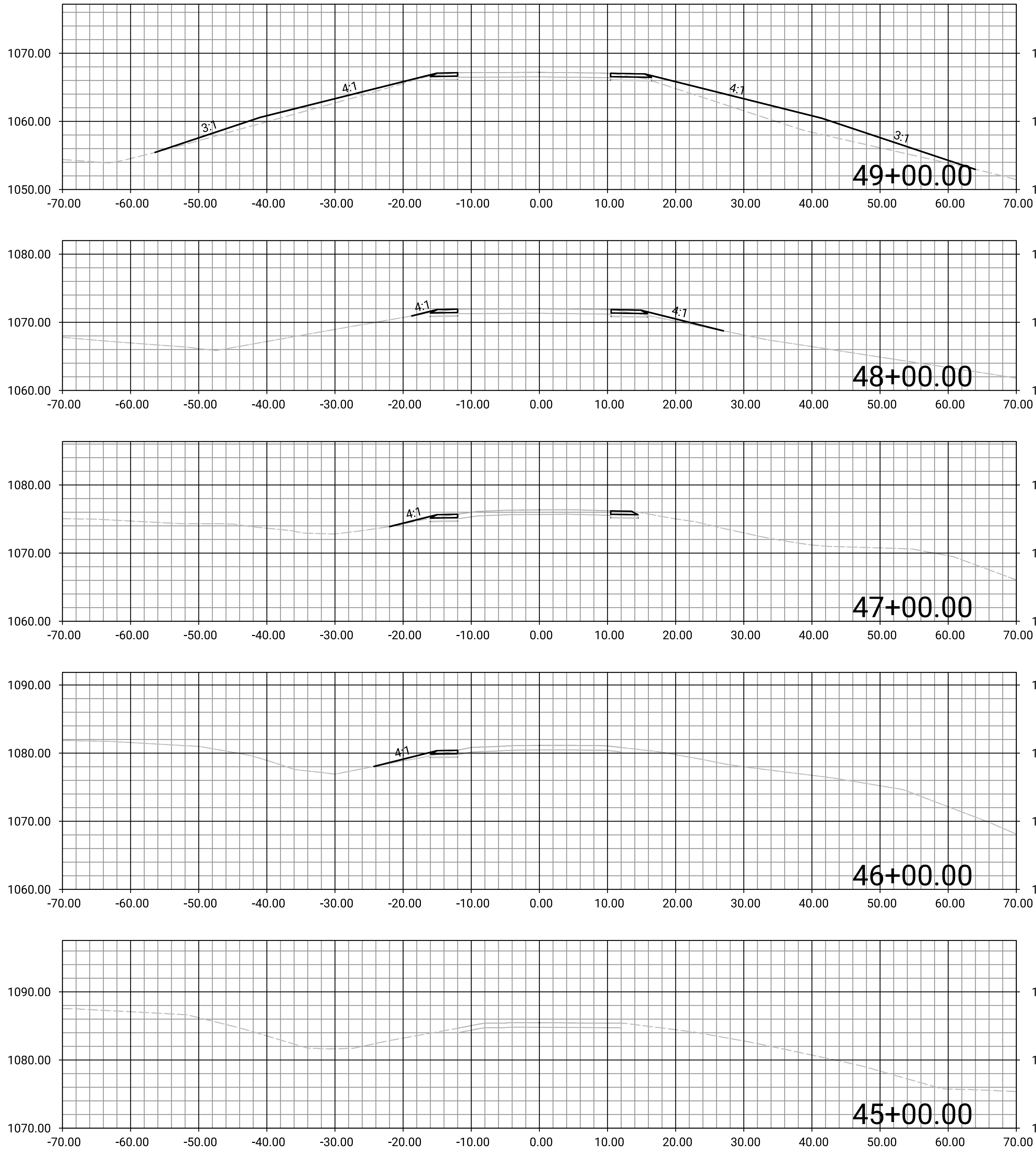
CADconform Certify This File

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	125	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan Andrews@ks.gov  
File : KA572901rco-00.dgn

16-APR-2024 17:27



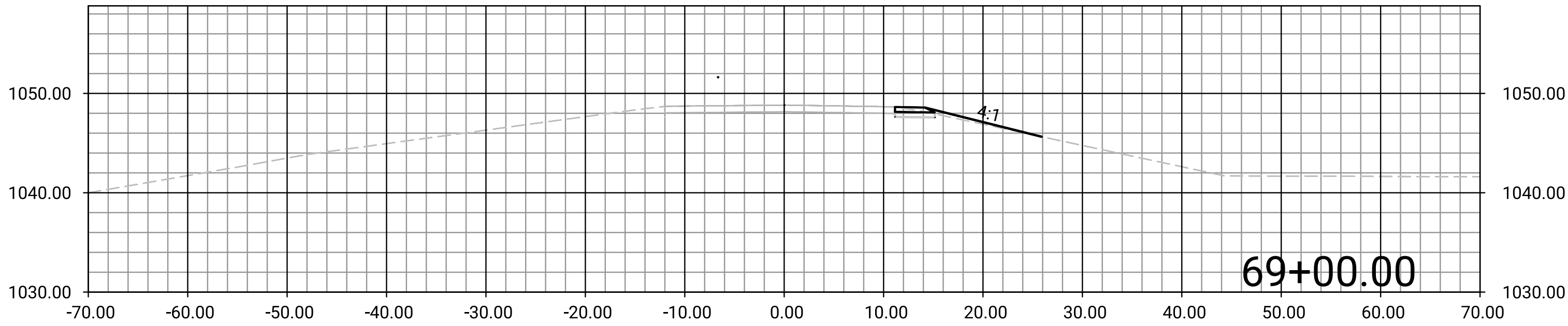
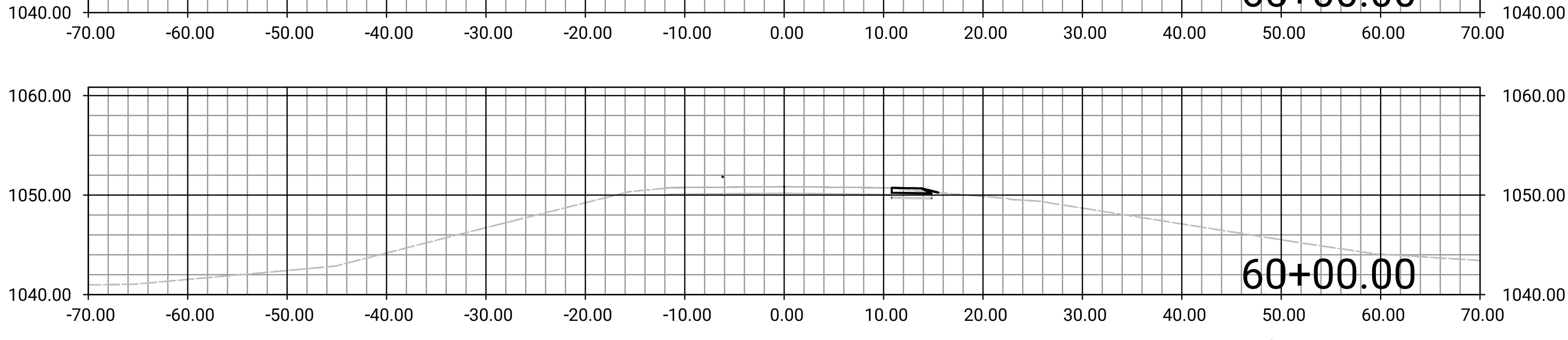
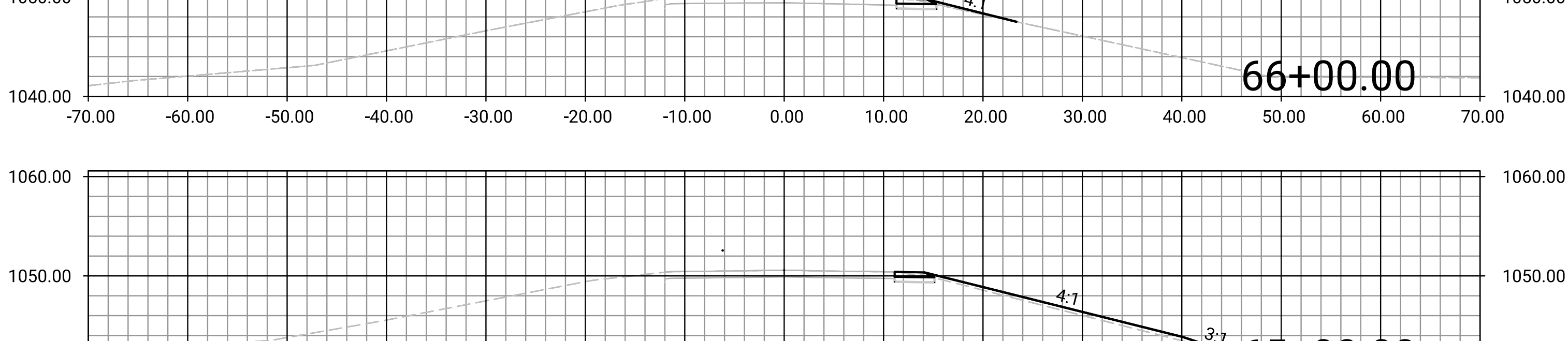
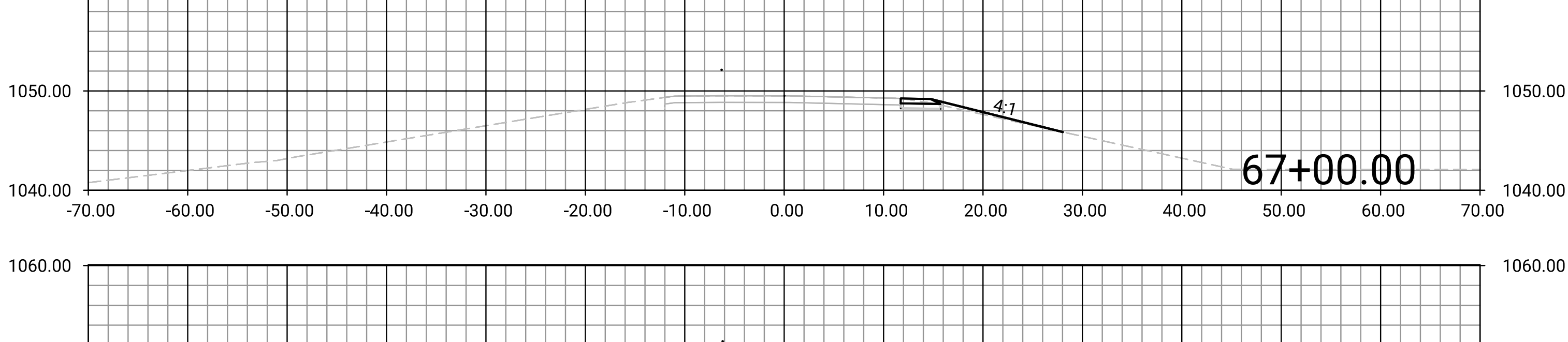
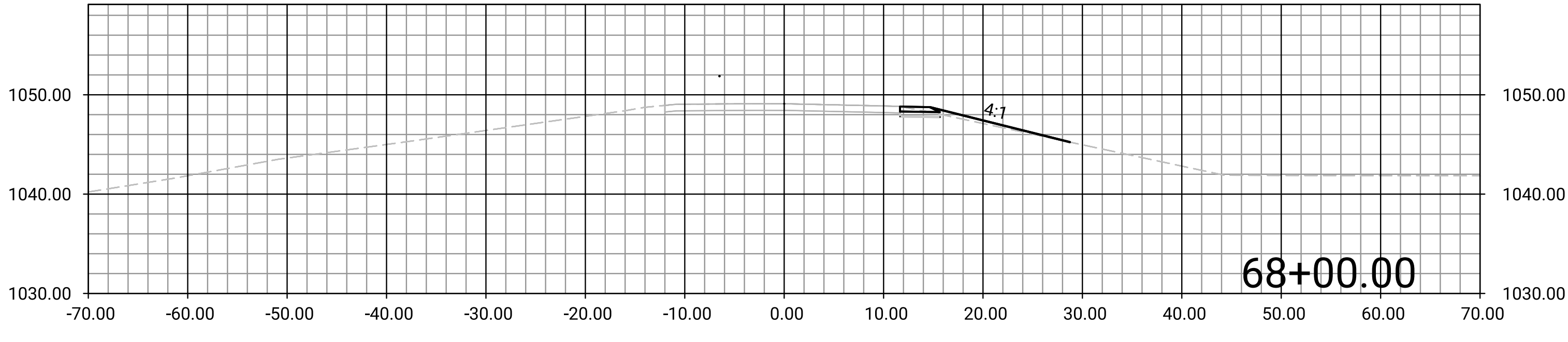
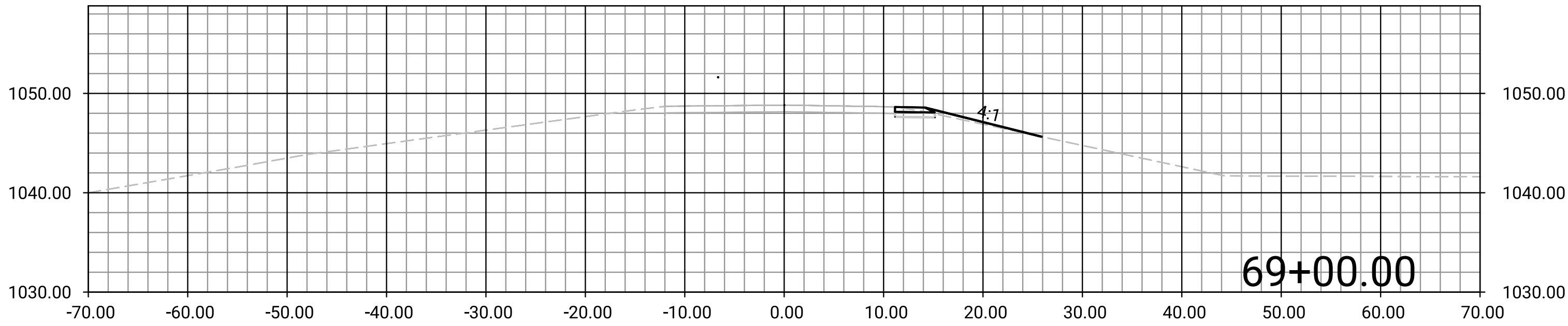
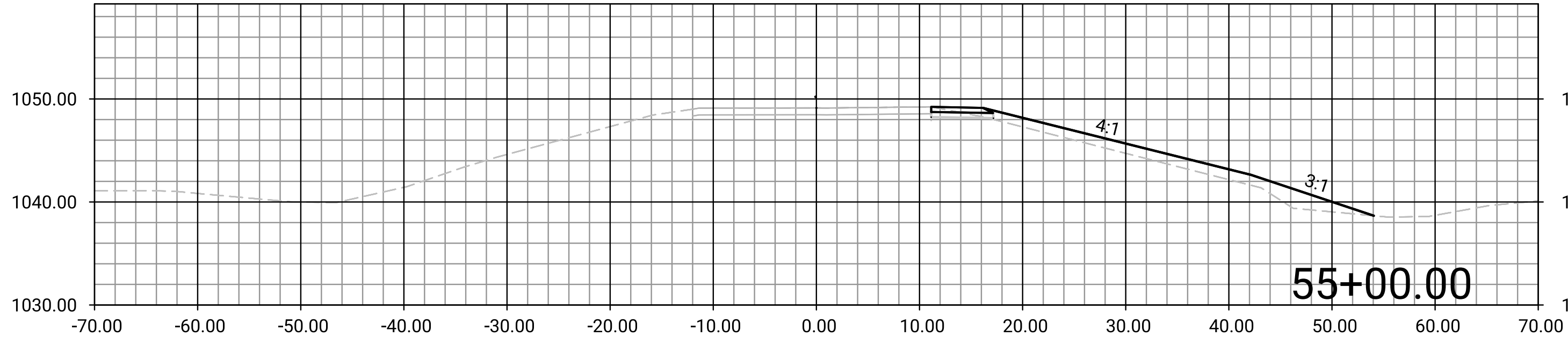
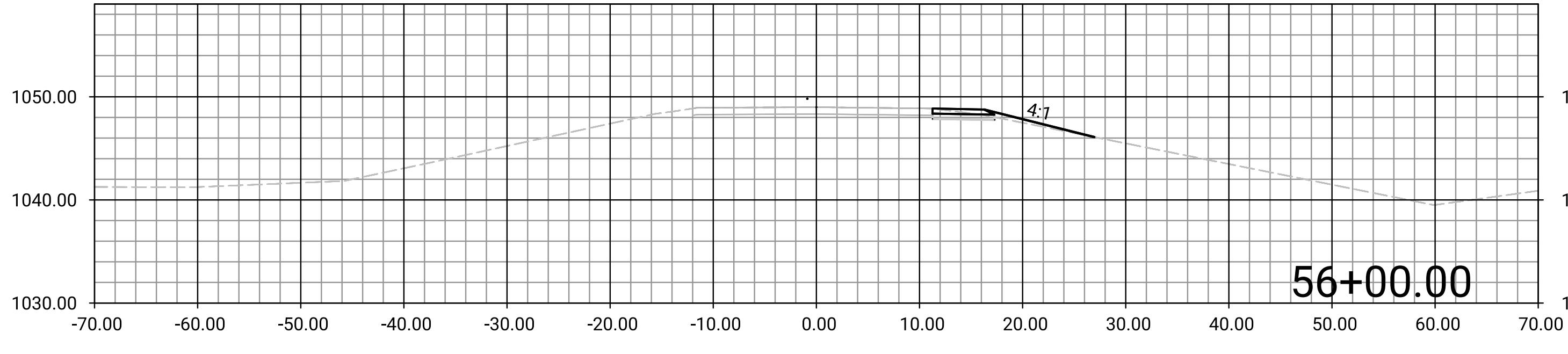
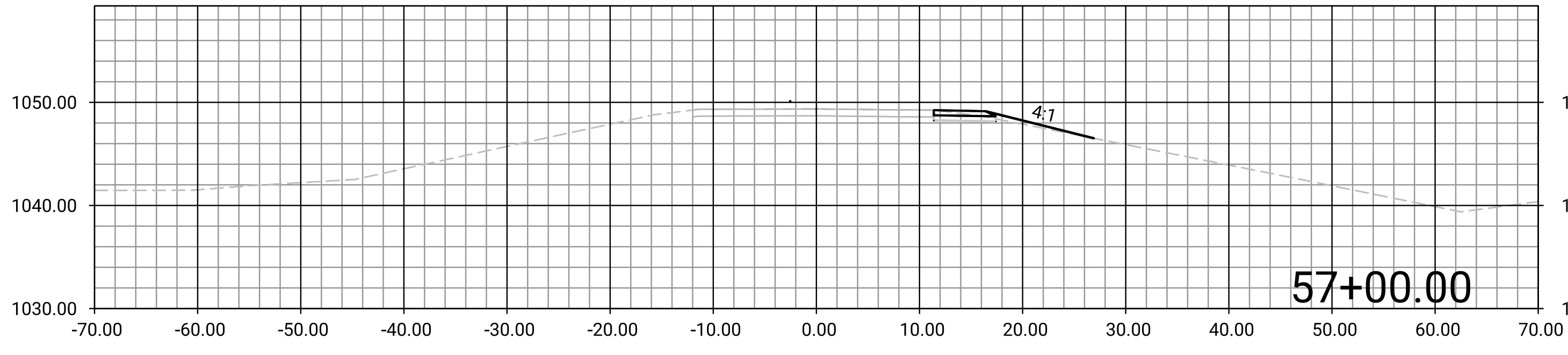
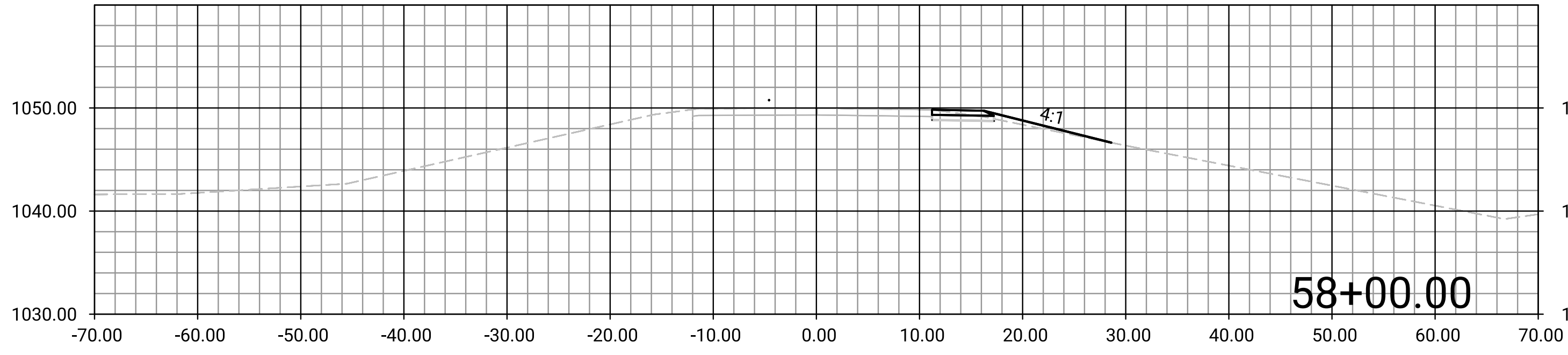
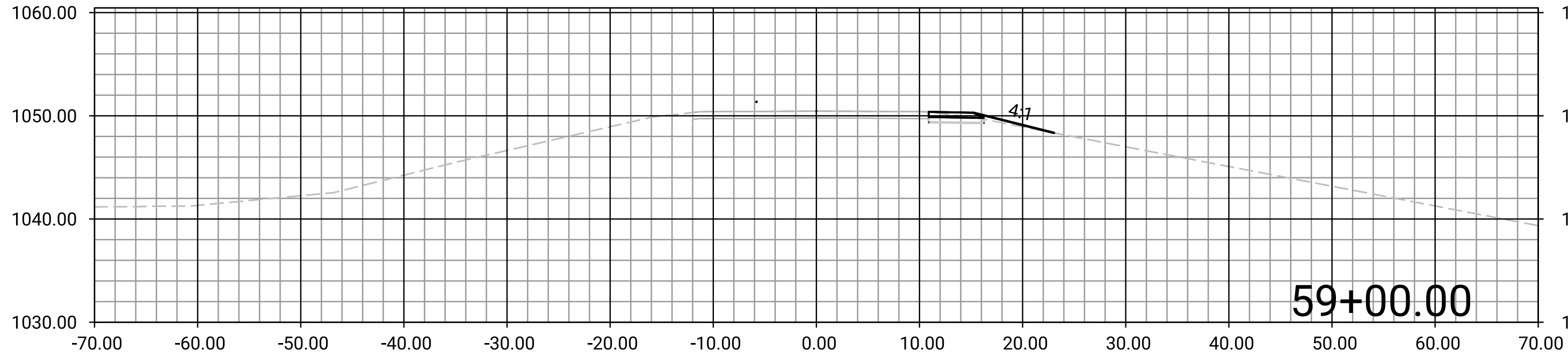
TEMPORARY WIDENING  
STA. 45+00 TO STA. 54+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	126	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan Andrews@ks.gov  
File : KA572901rco-00.dgn

16-APR-2024 17:28

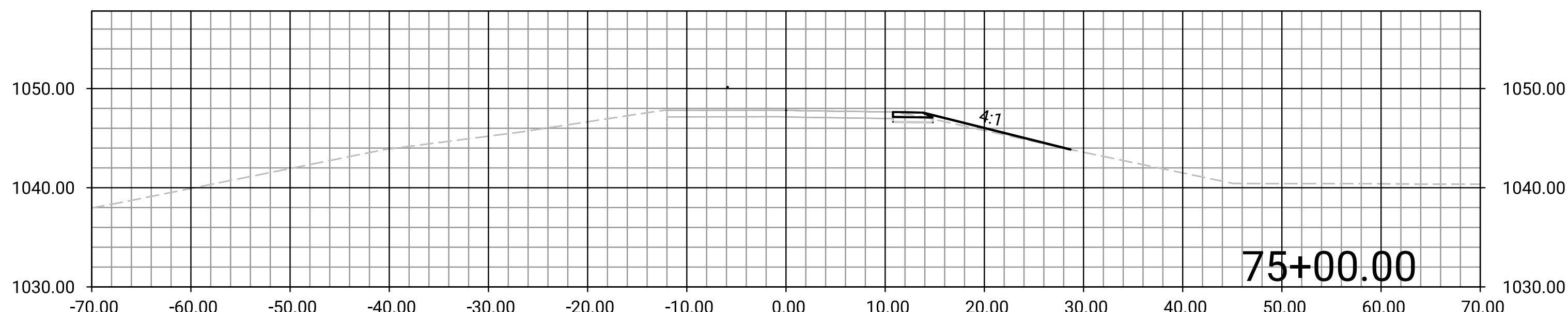
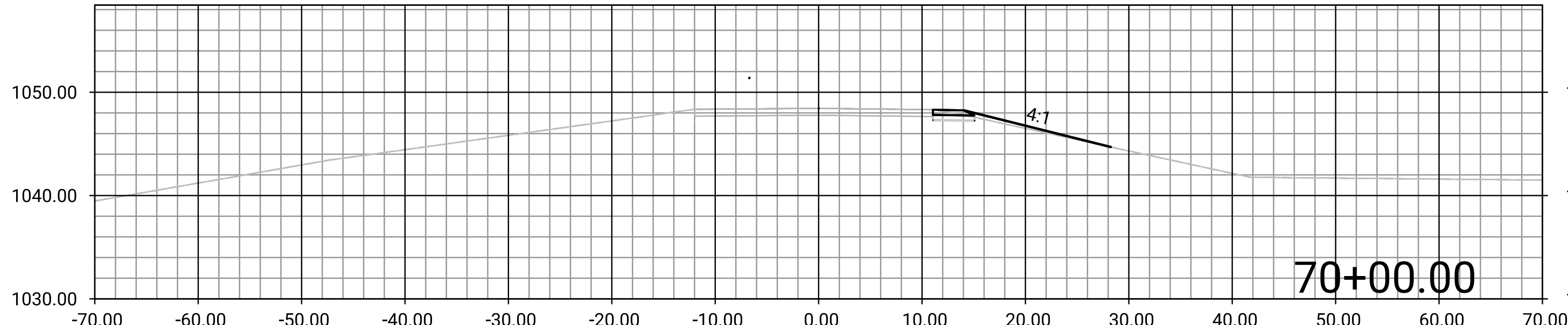
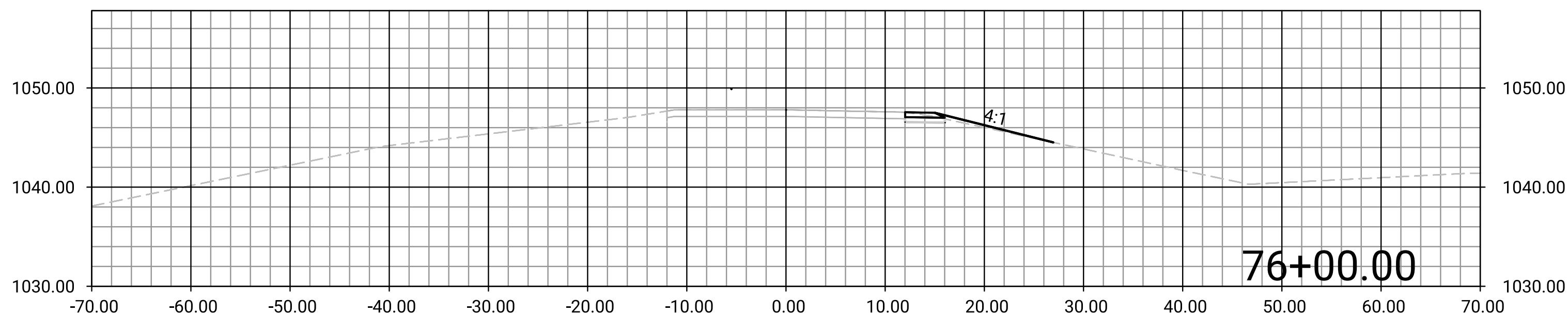
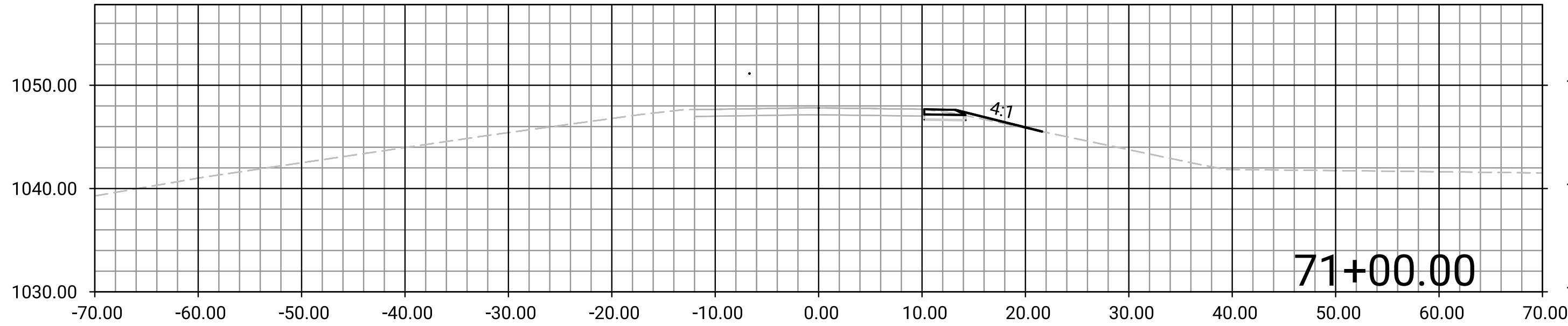
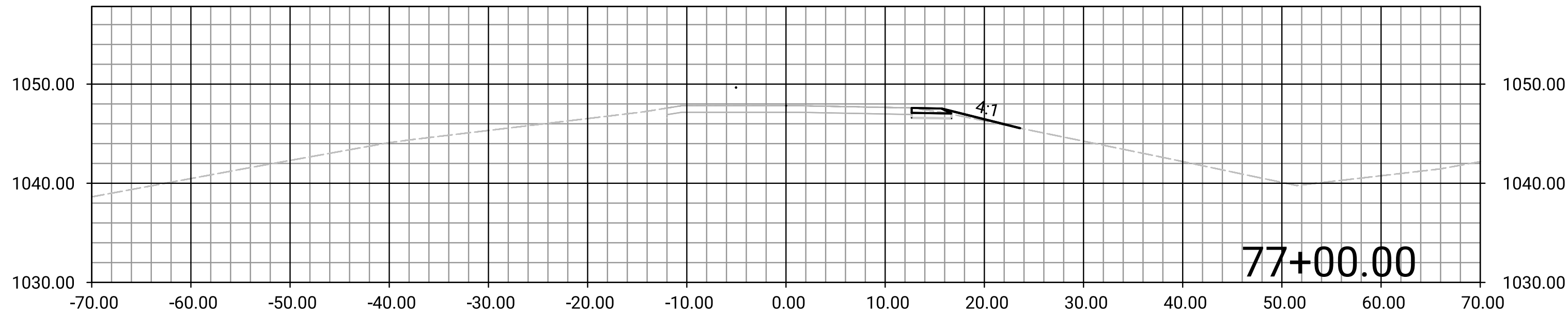
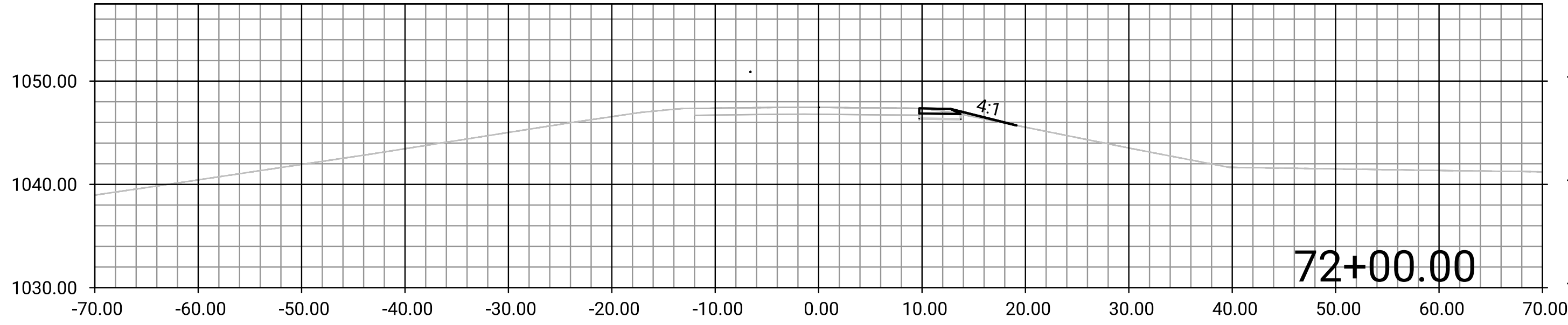
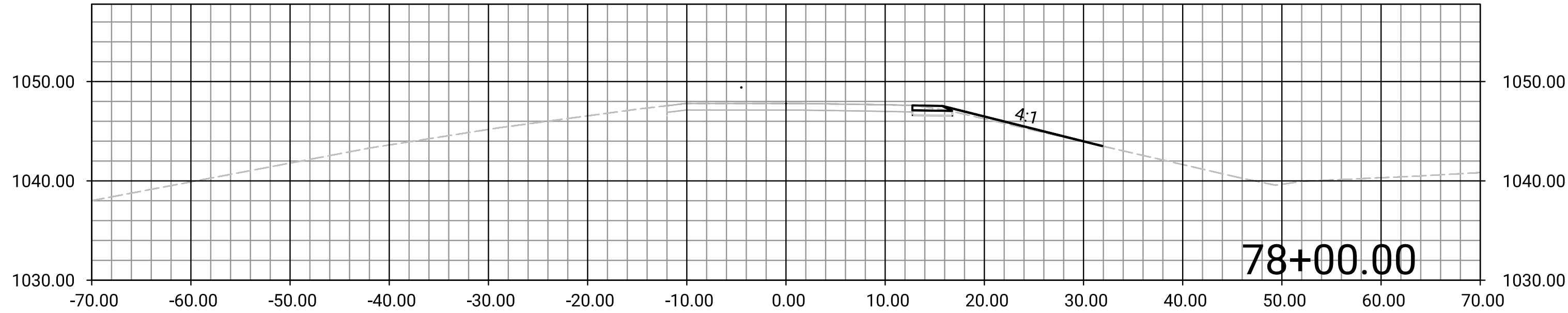
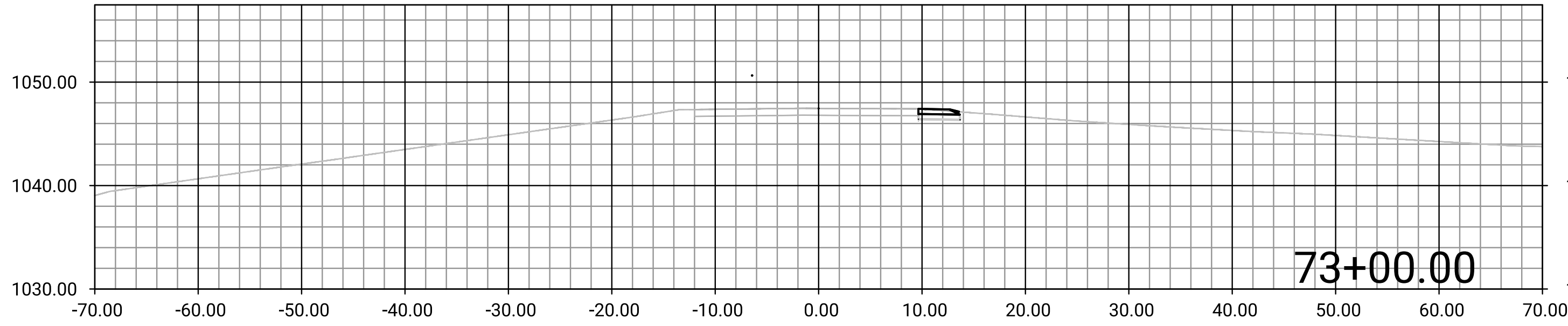
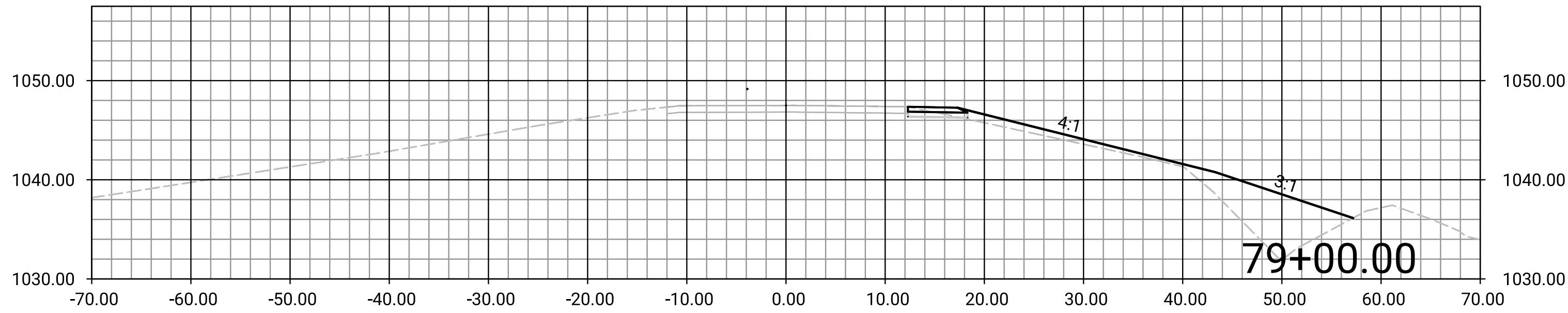
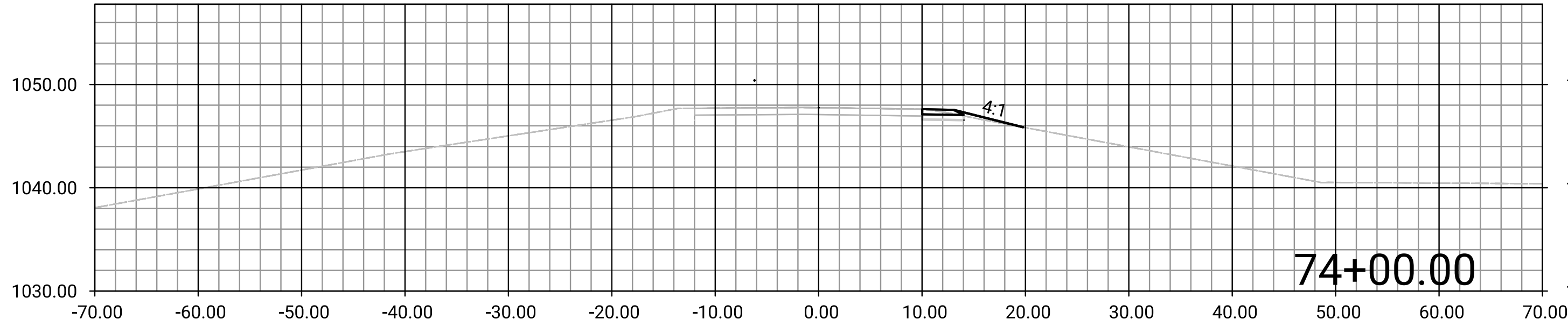


TEMPORARY WIDENING  
STA. 55+00 TO STA. 69+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	127	141

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:29  
File : KA572901rco-00.dgn



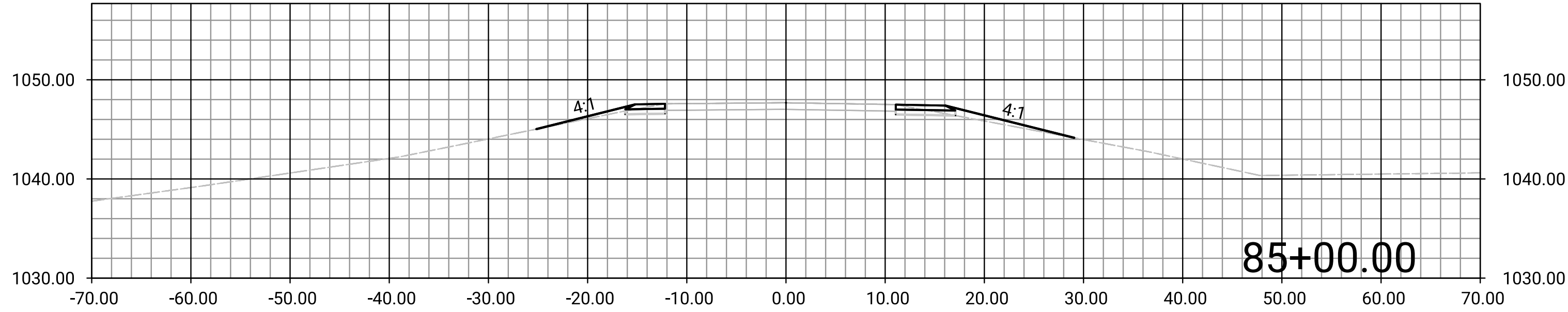
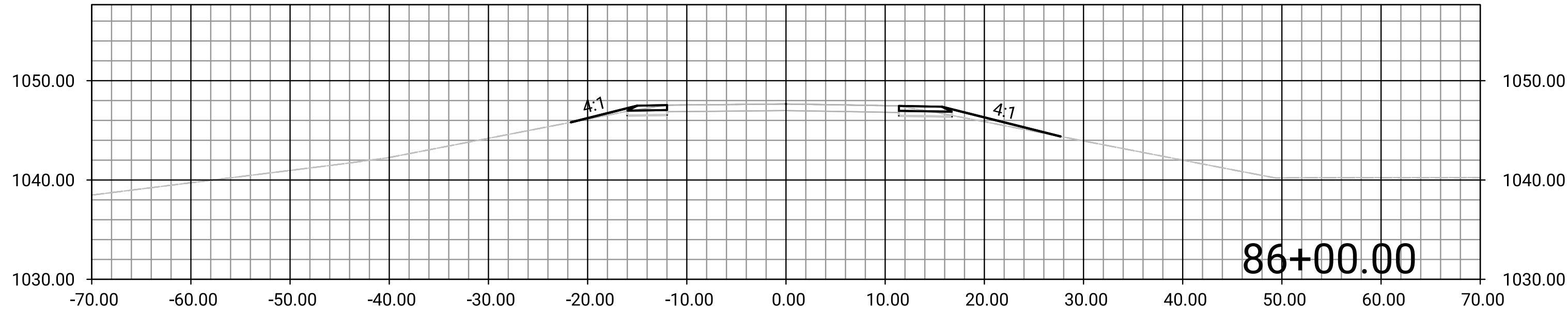
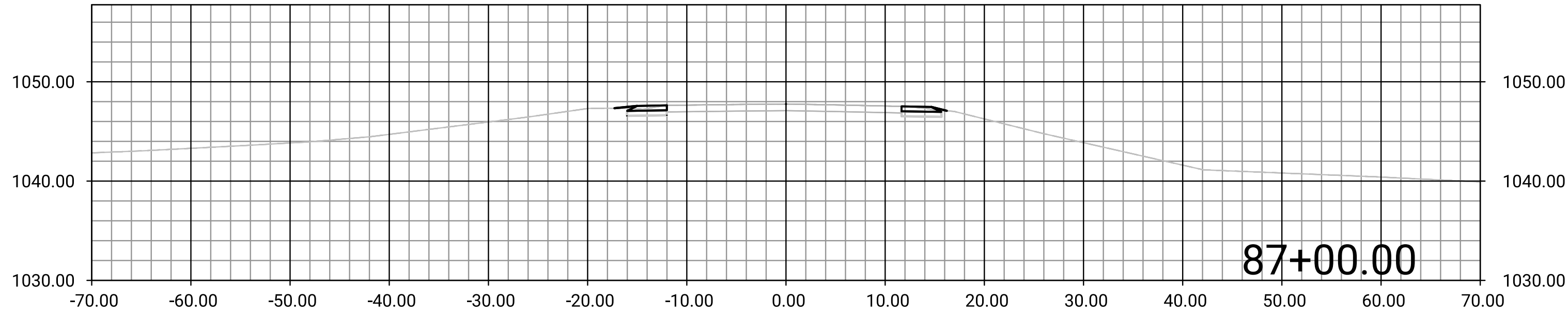
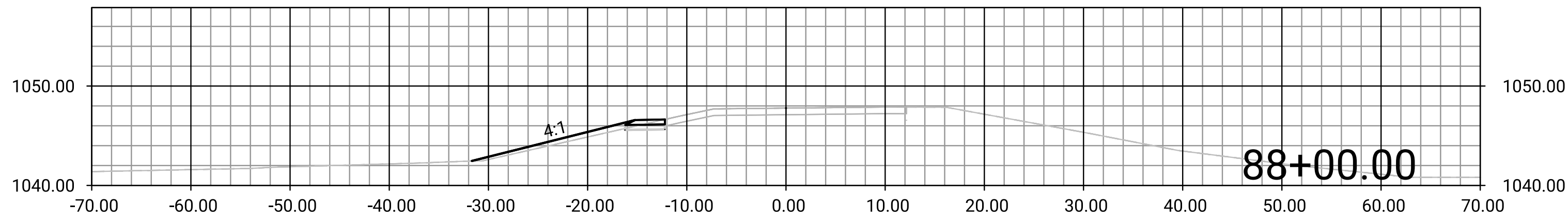
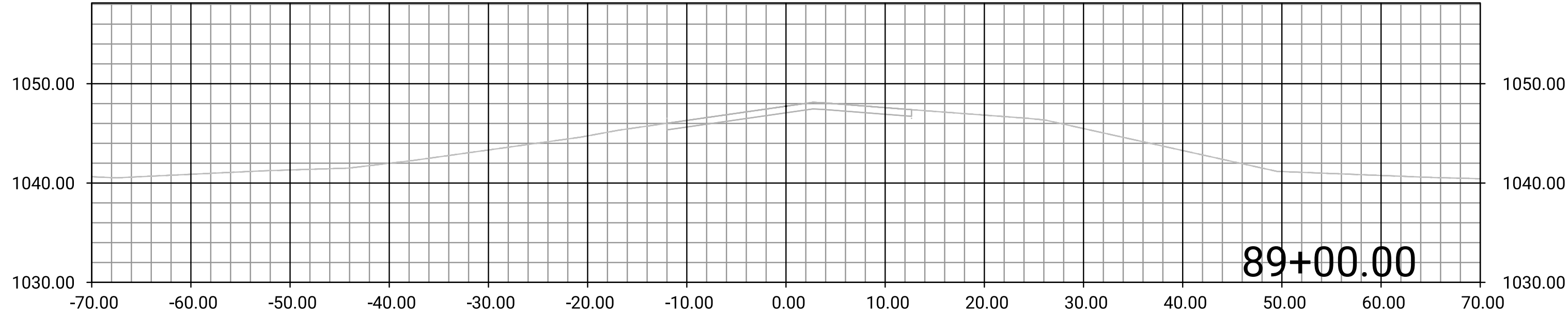
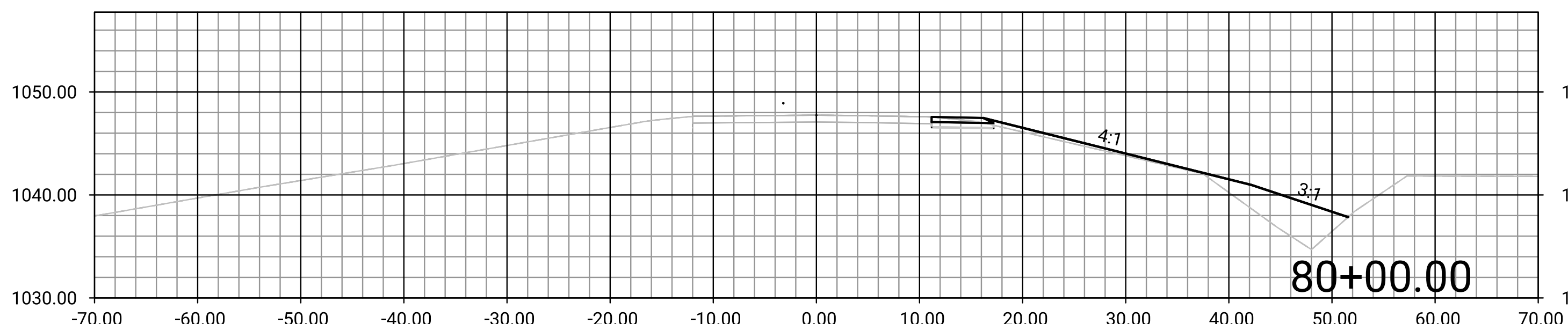
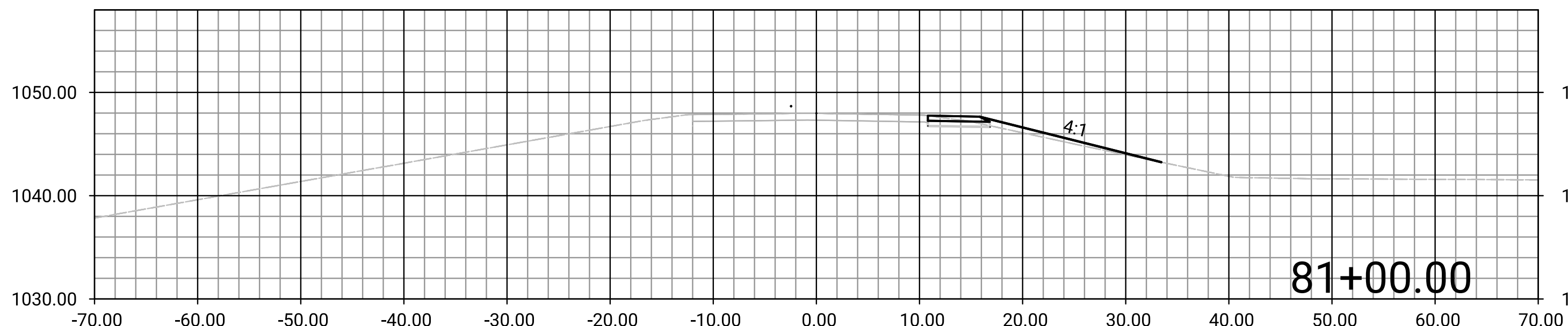
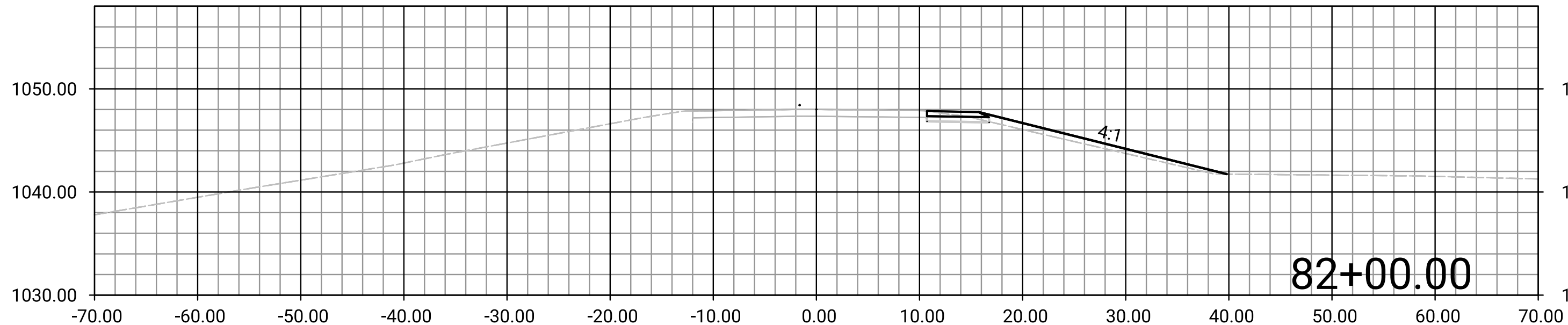
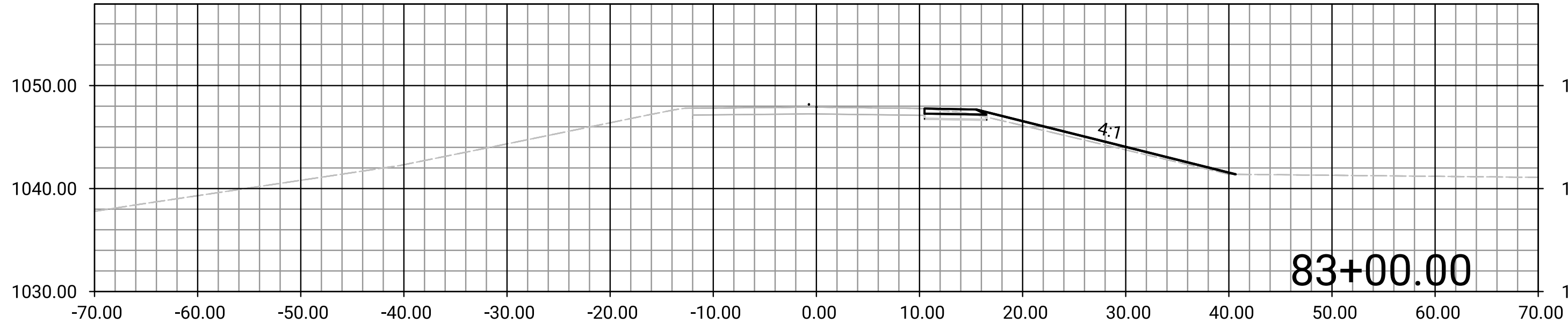
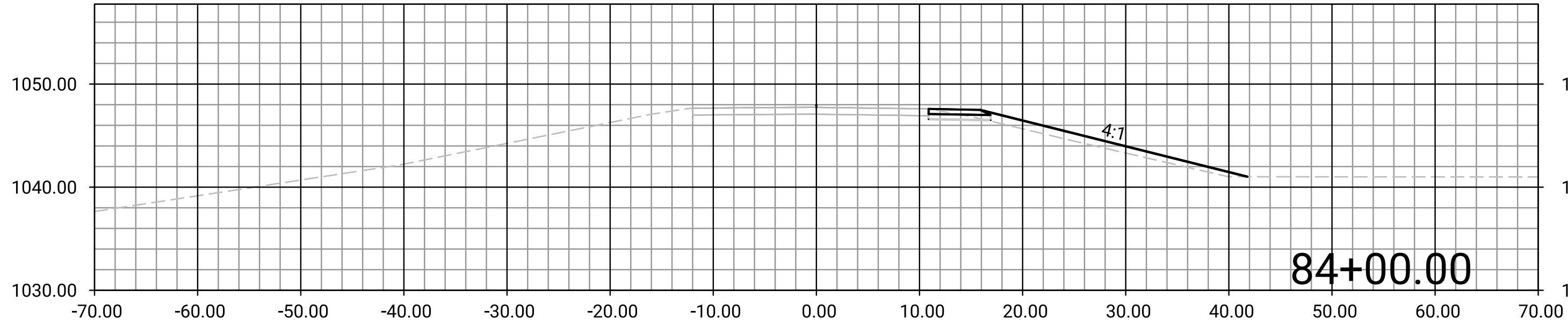
TEMPORARY WIDENING  
STA. 70+00 TO STA. 79+00  
SCALE 1:10



DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:29  
File : KA572901rco-00.dgn

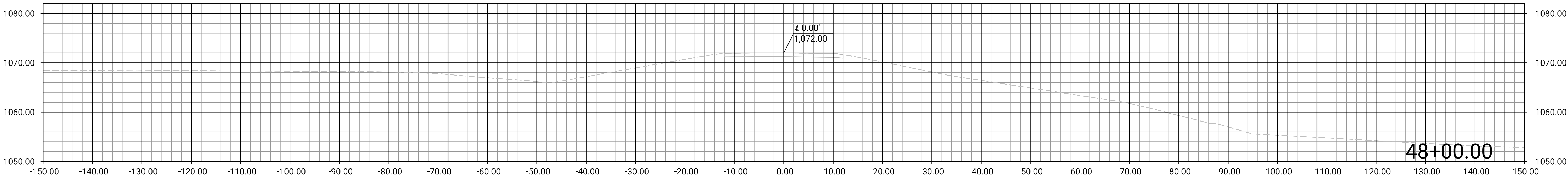
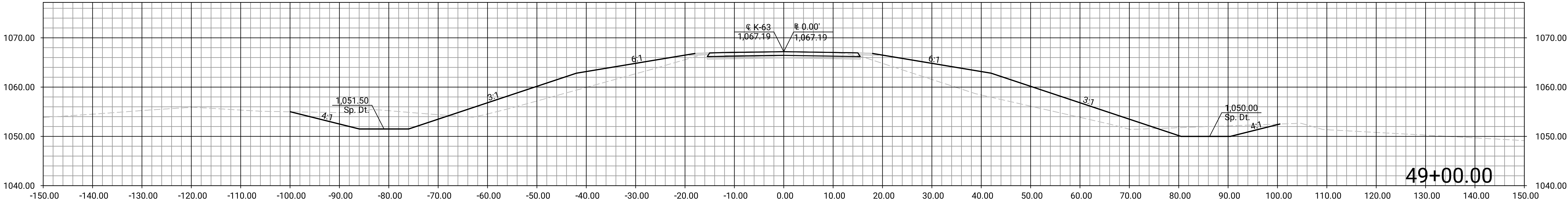
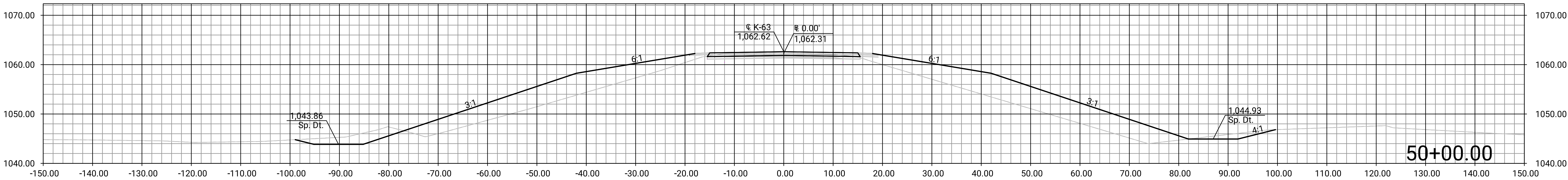
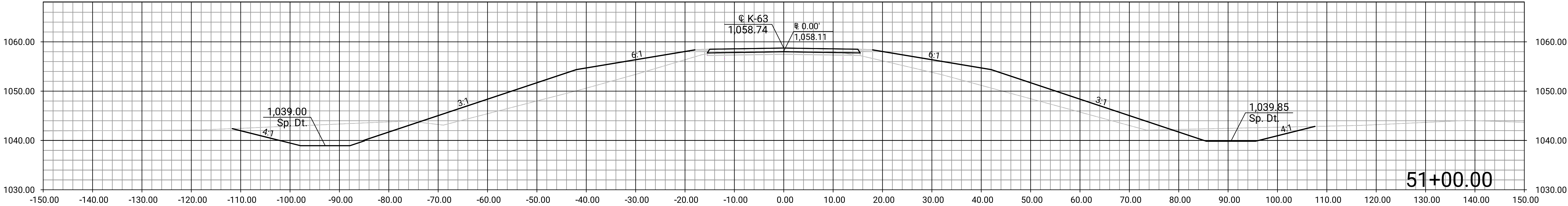
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	128	141



TEMPORARY WIDENING  
STA. 80+00 TO STA. 89+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	129	141

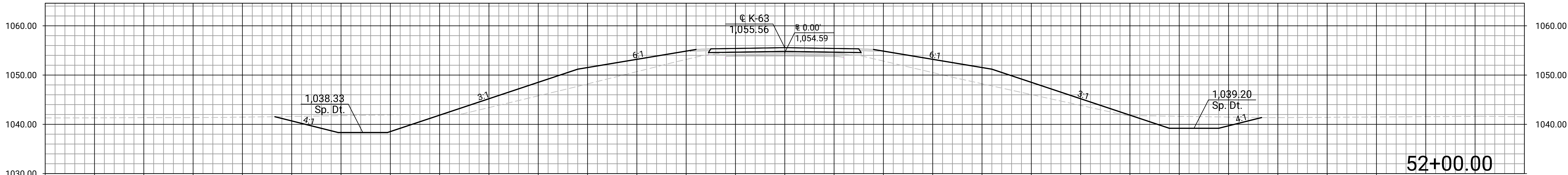
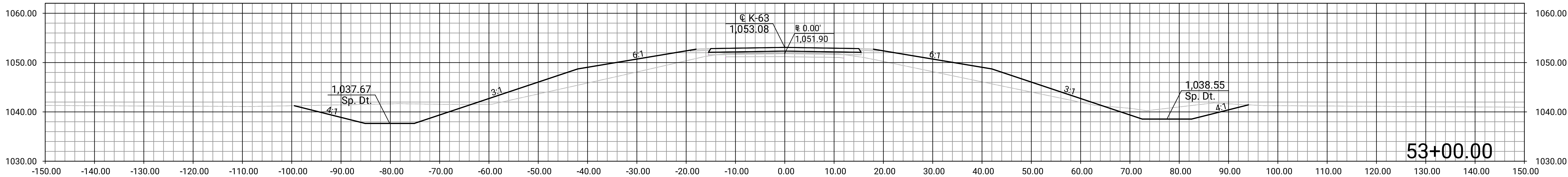
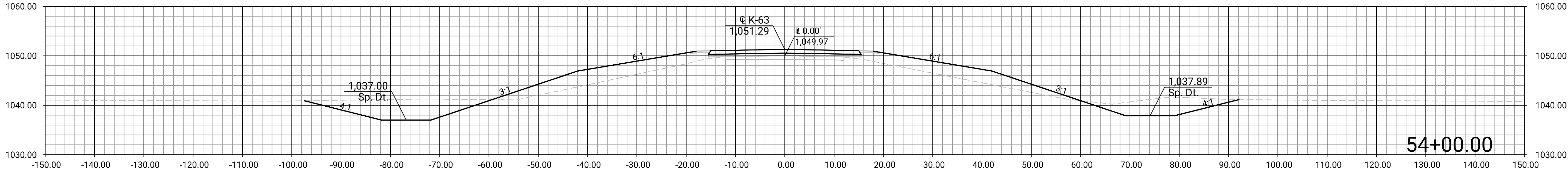
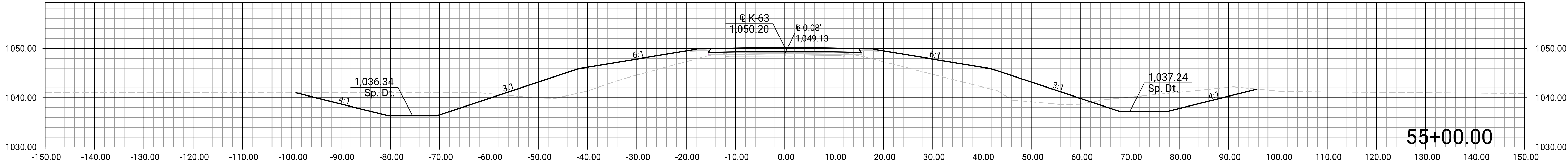
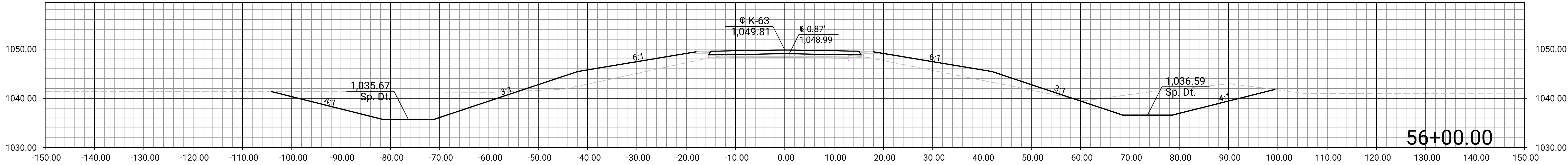
DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	



☺ K-63  
STA. 48+00 TO 51+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	130	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	



STA. 52+00 TO 56+00  
SCALE 1:10

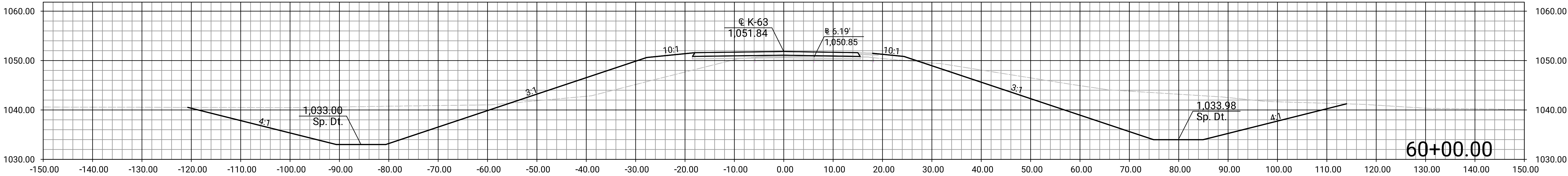
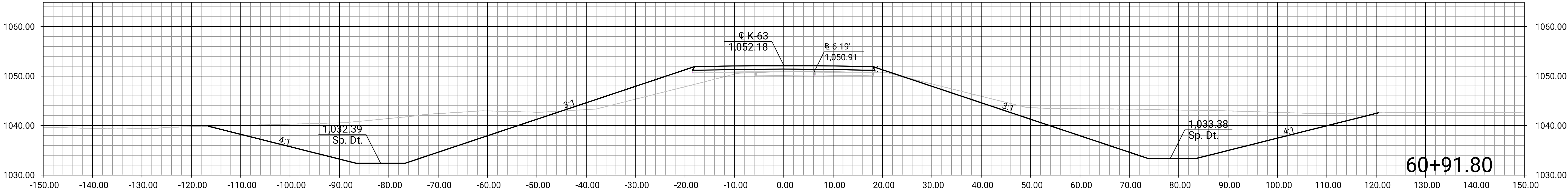
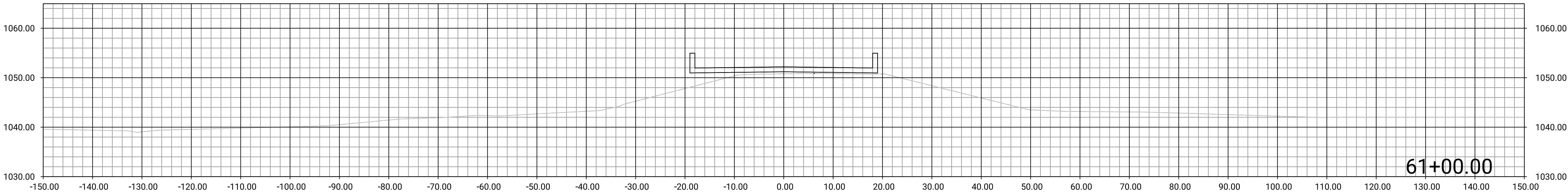
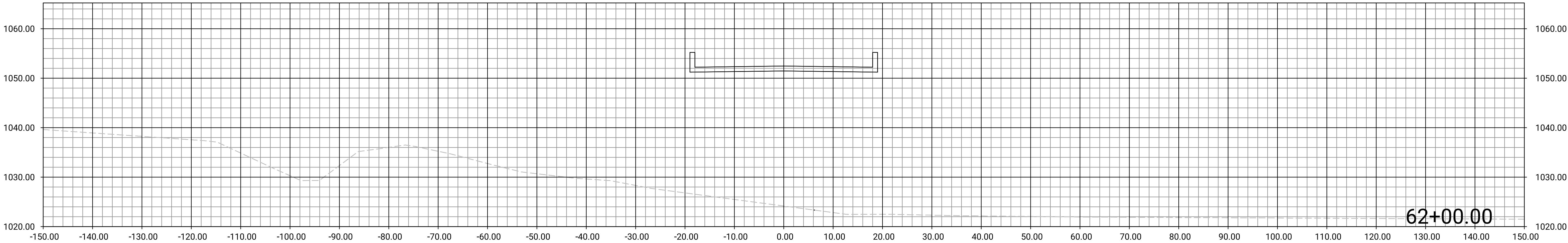






STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	132	141

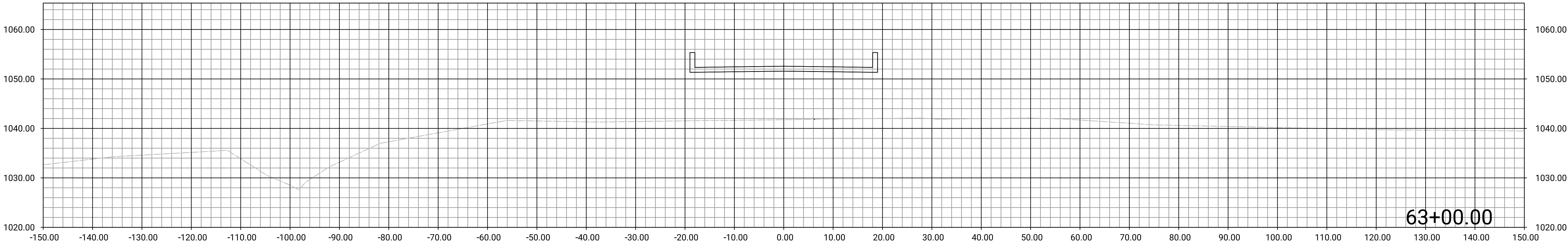
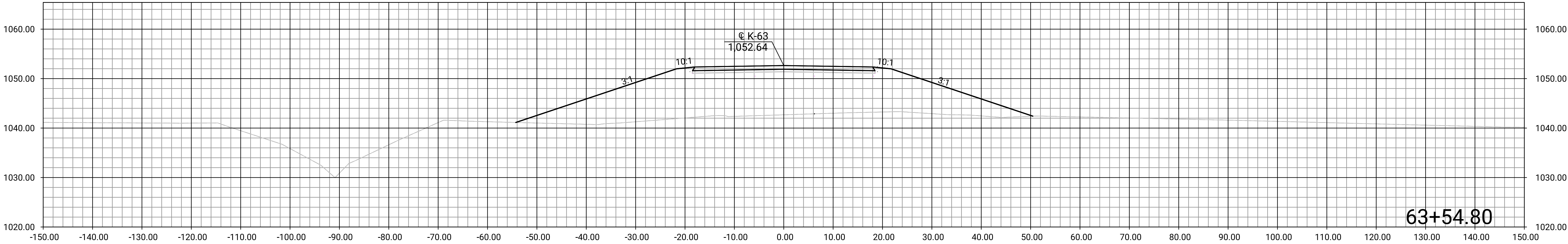
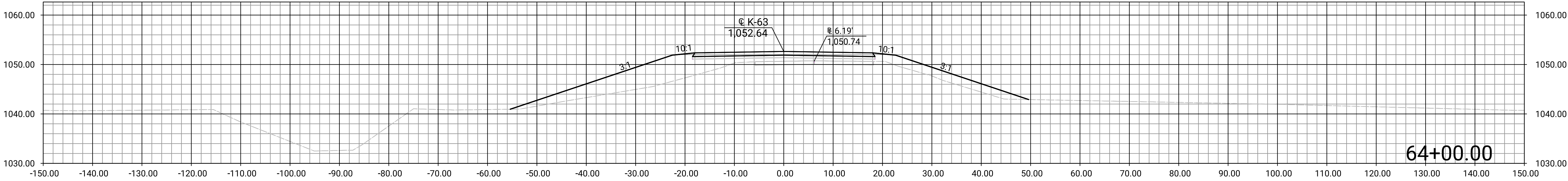
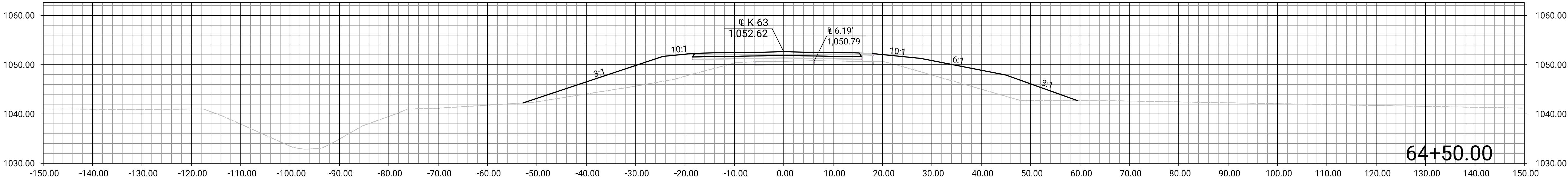
DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	



☺ K-63  
STA. 60+00 TO 62+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	133	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

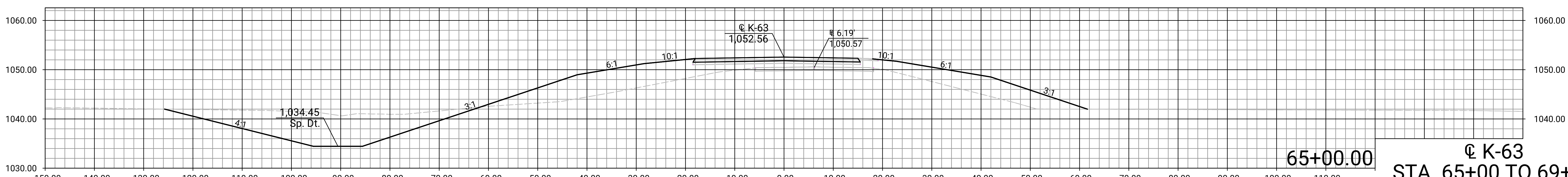
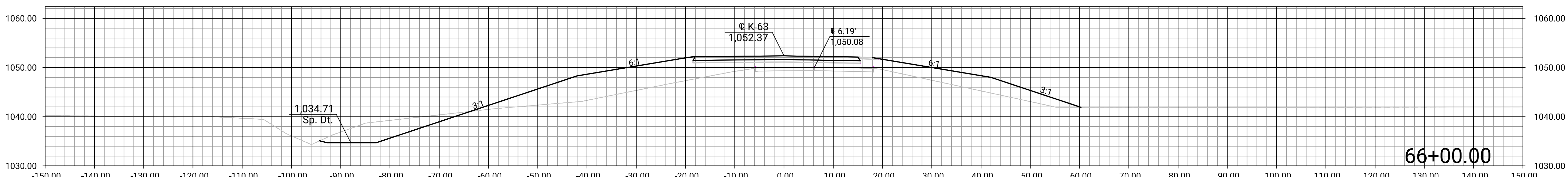
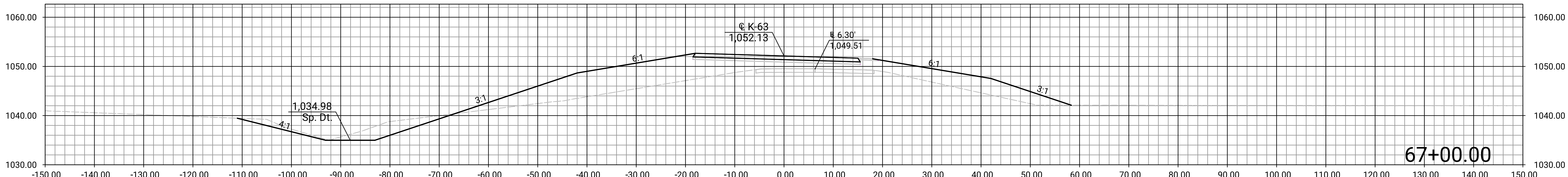
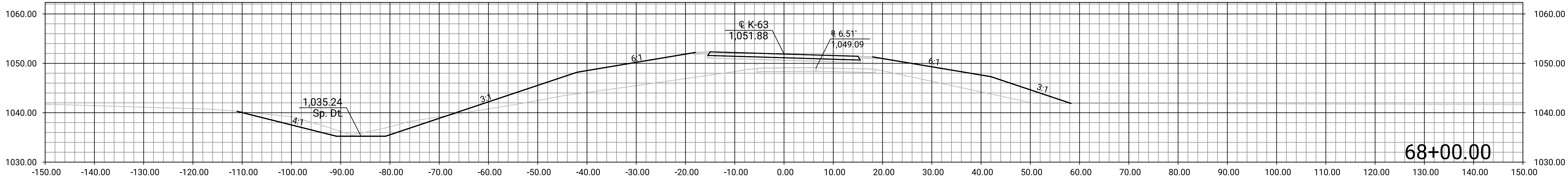
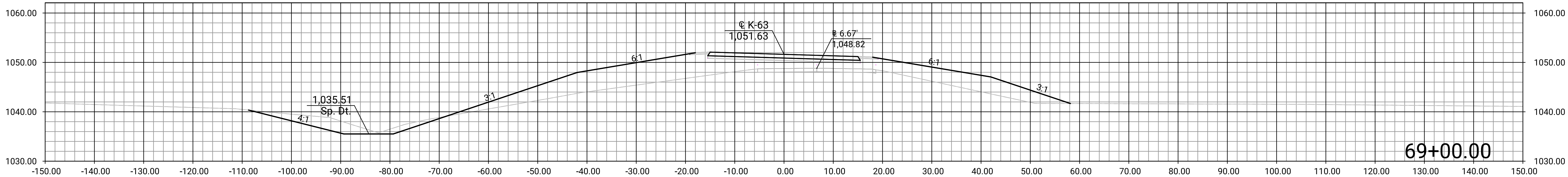


☉ K-63  
STA. 63+00 TO 64+50  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	134	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan Andrews@ks.gov  
File : KA572901rxs-01.dgn  
16-APR-2024 17:35

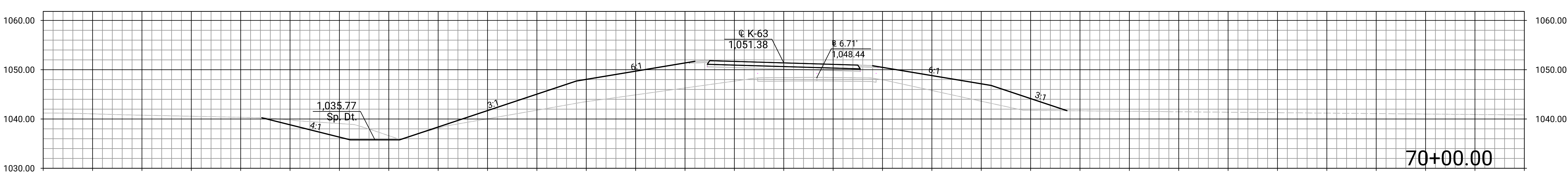
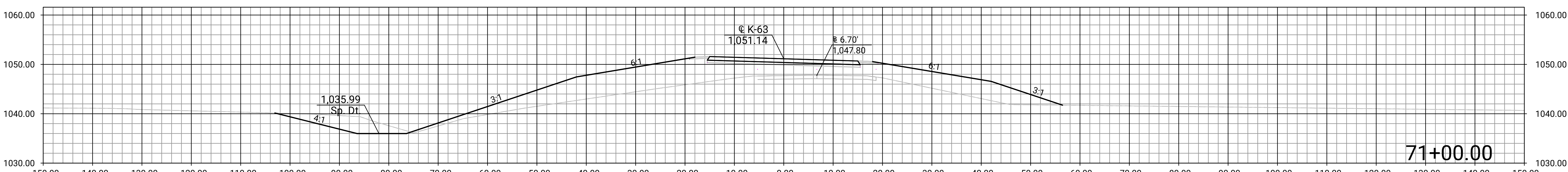
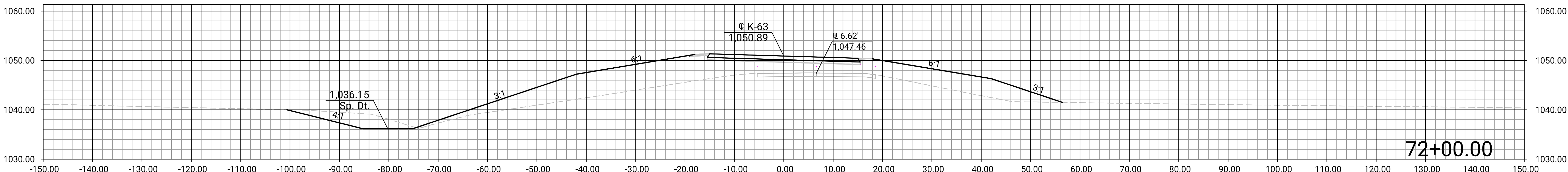
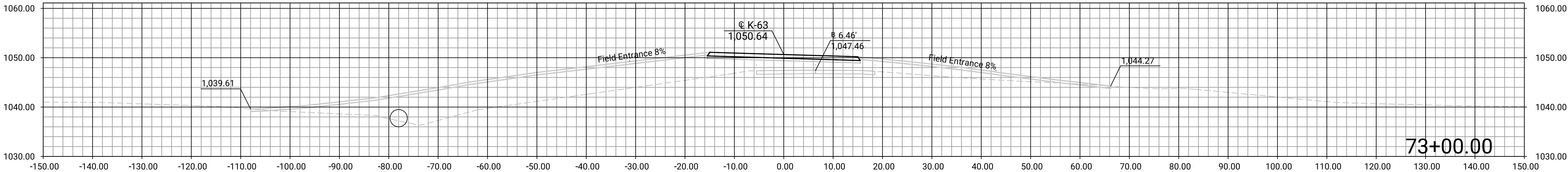
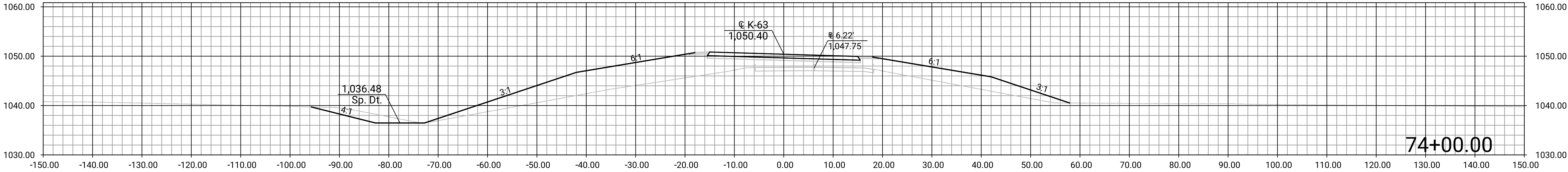


65+00.00  
STA. 65+00 TO 69+00  
SCALE 1:10  
K-63



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	135	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

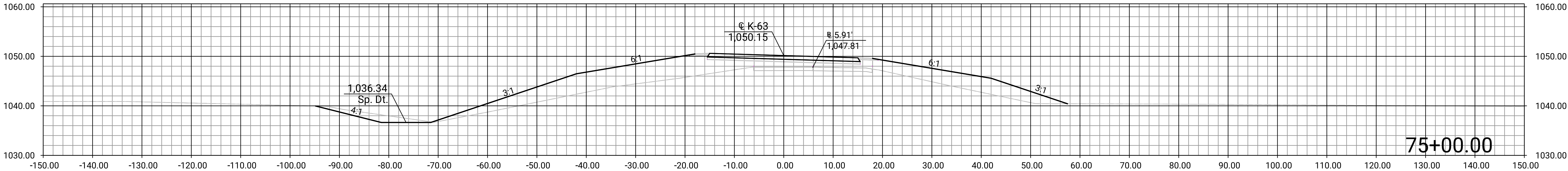
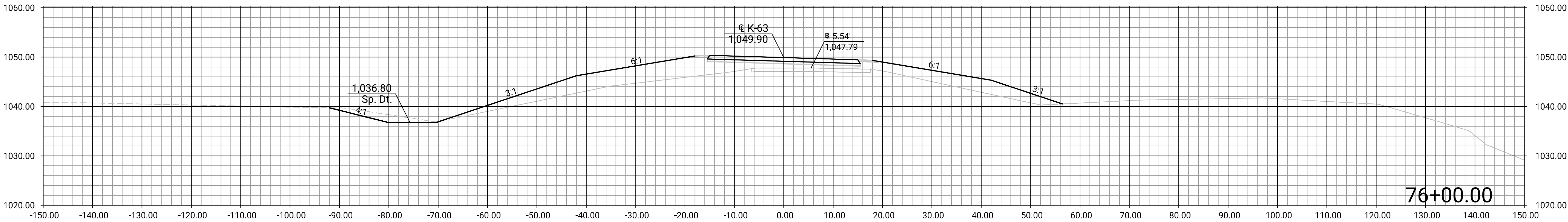
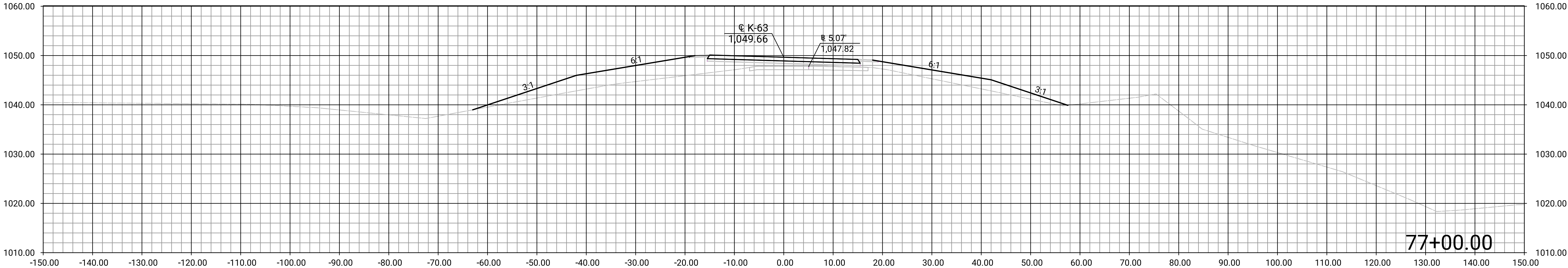


STA. 70+00 TO 74+00  
SCALE 1:10



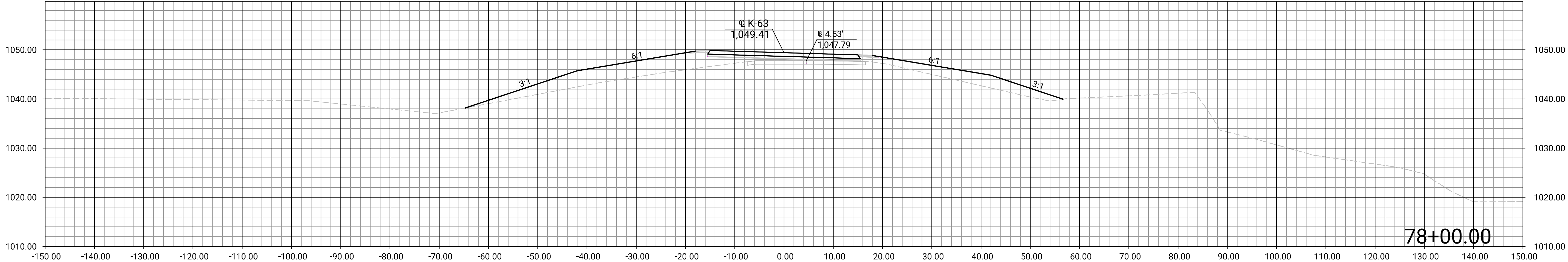
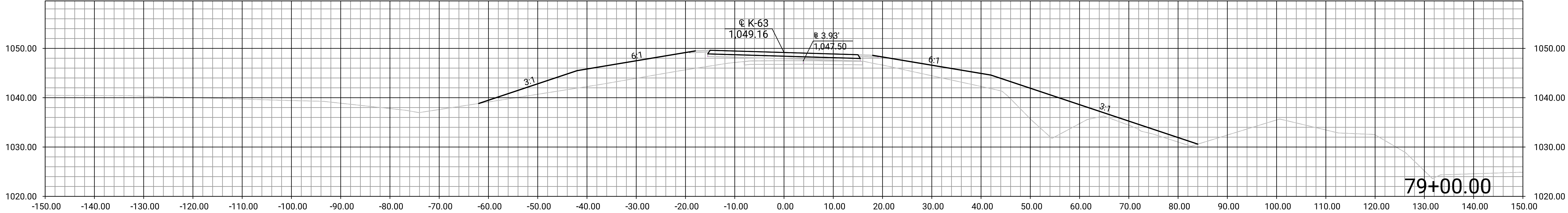
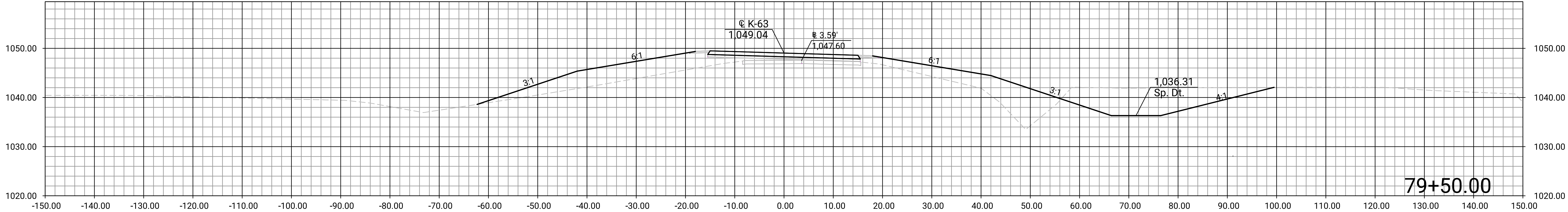
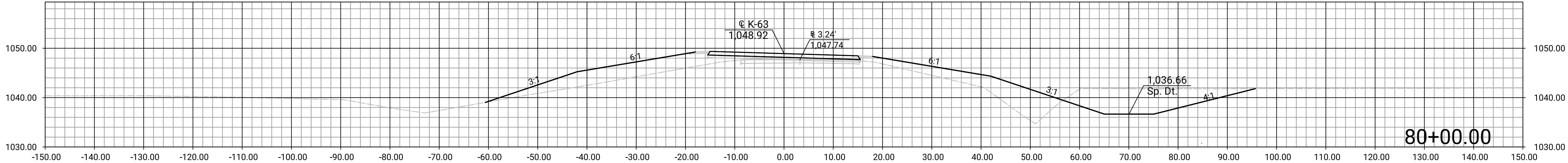
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	136	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	



☉ K-63  
STA. 75+00 TO 77+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	137	141



☺ K-63  
STA. 78+00 TO 80+00  
SCALE 1:10

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

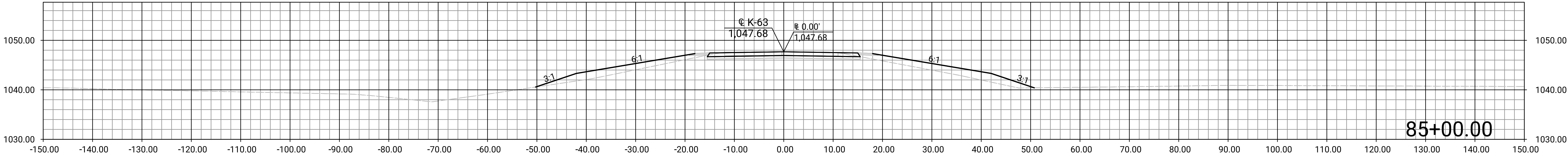
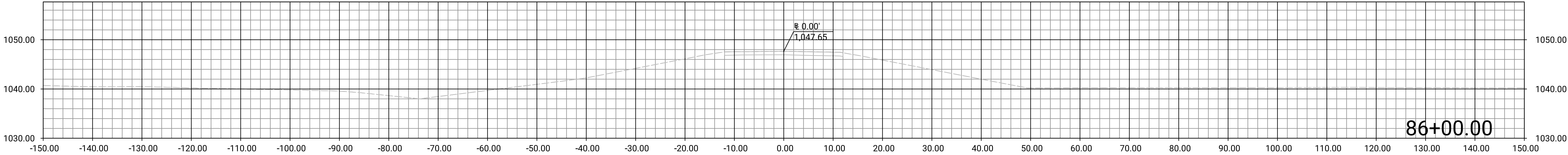
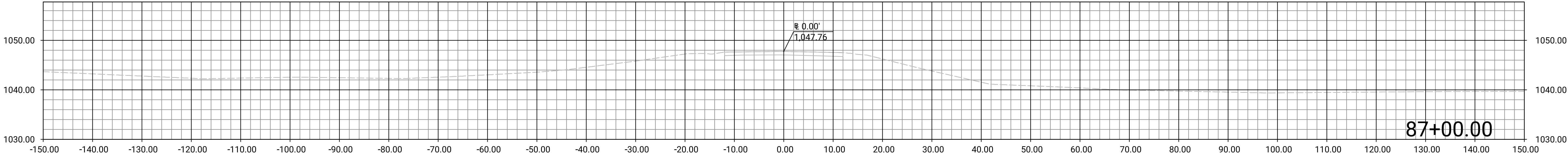
Plotted by : Brogan Andrews@ks.gov  
File : KA572901rxs-01.dgn



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	139	141

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:37  
File : KA572901rxs-01.dgn



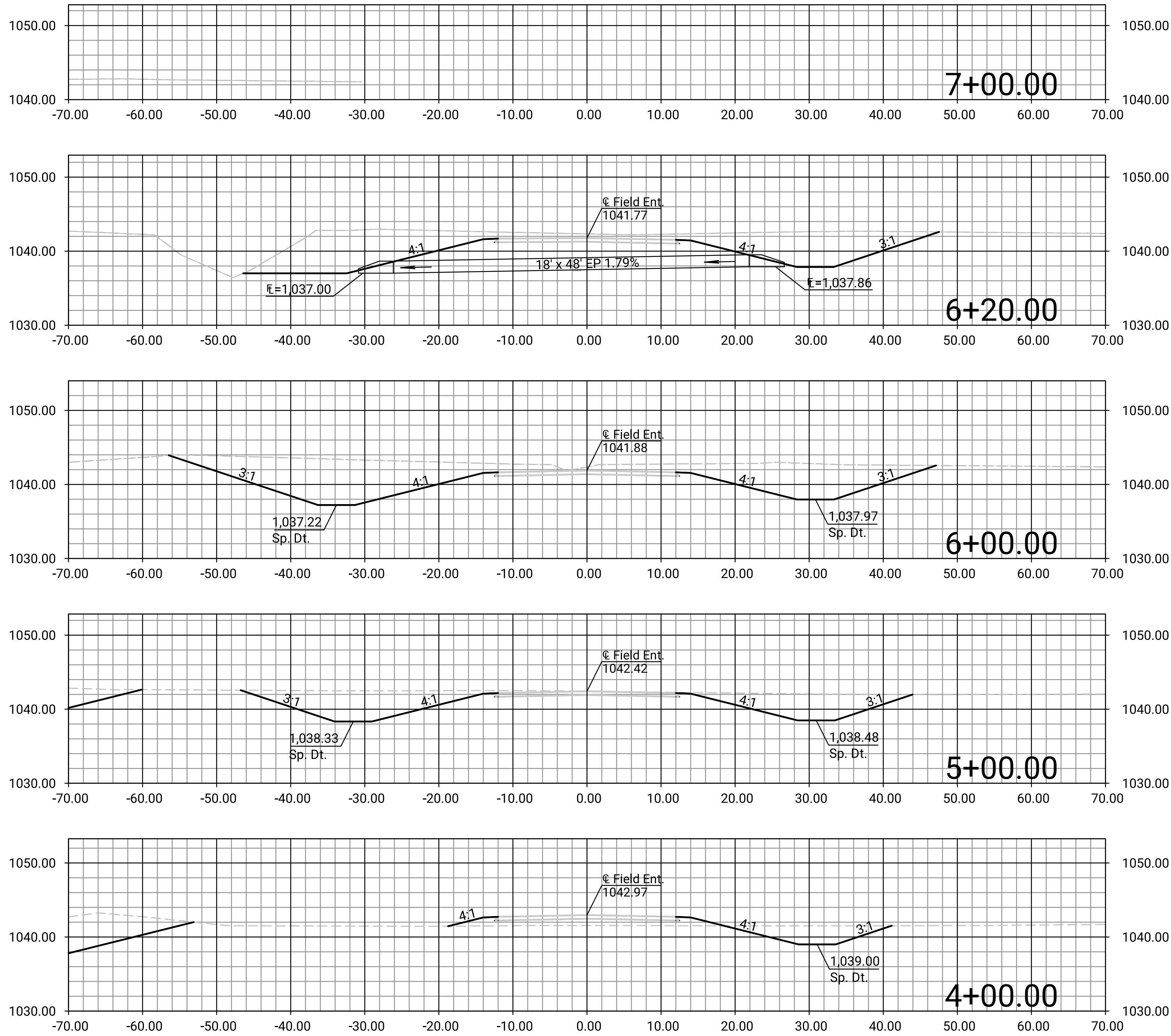
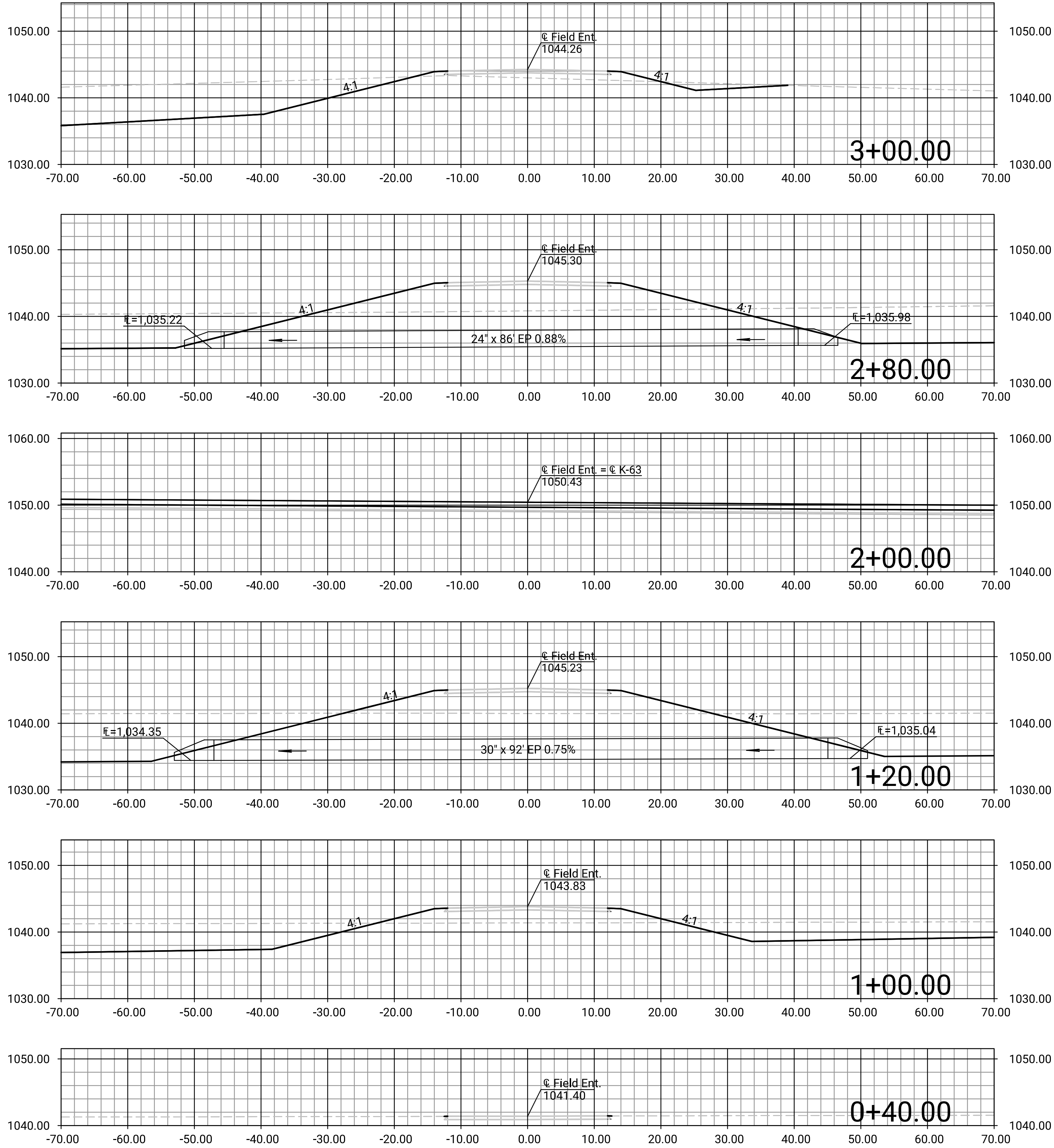
☉ K-63  
STA. 85+00 TO 87+00  
SCALE 1:10



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	140	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan Andrews@ks.gov  
File : KA572901rco-04.dgn  
16-APR-2024 17:27

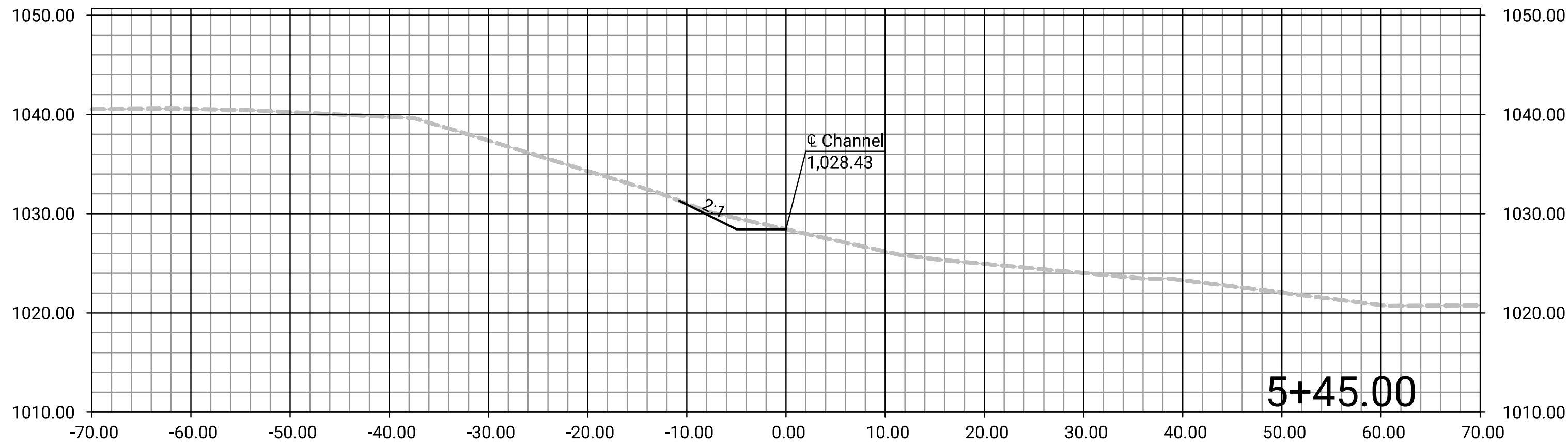
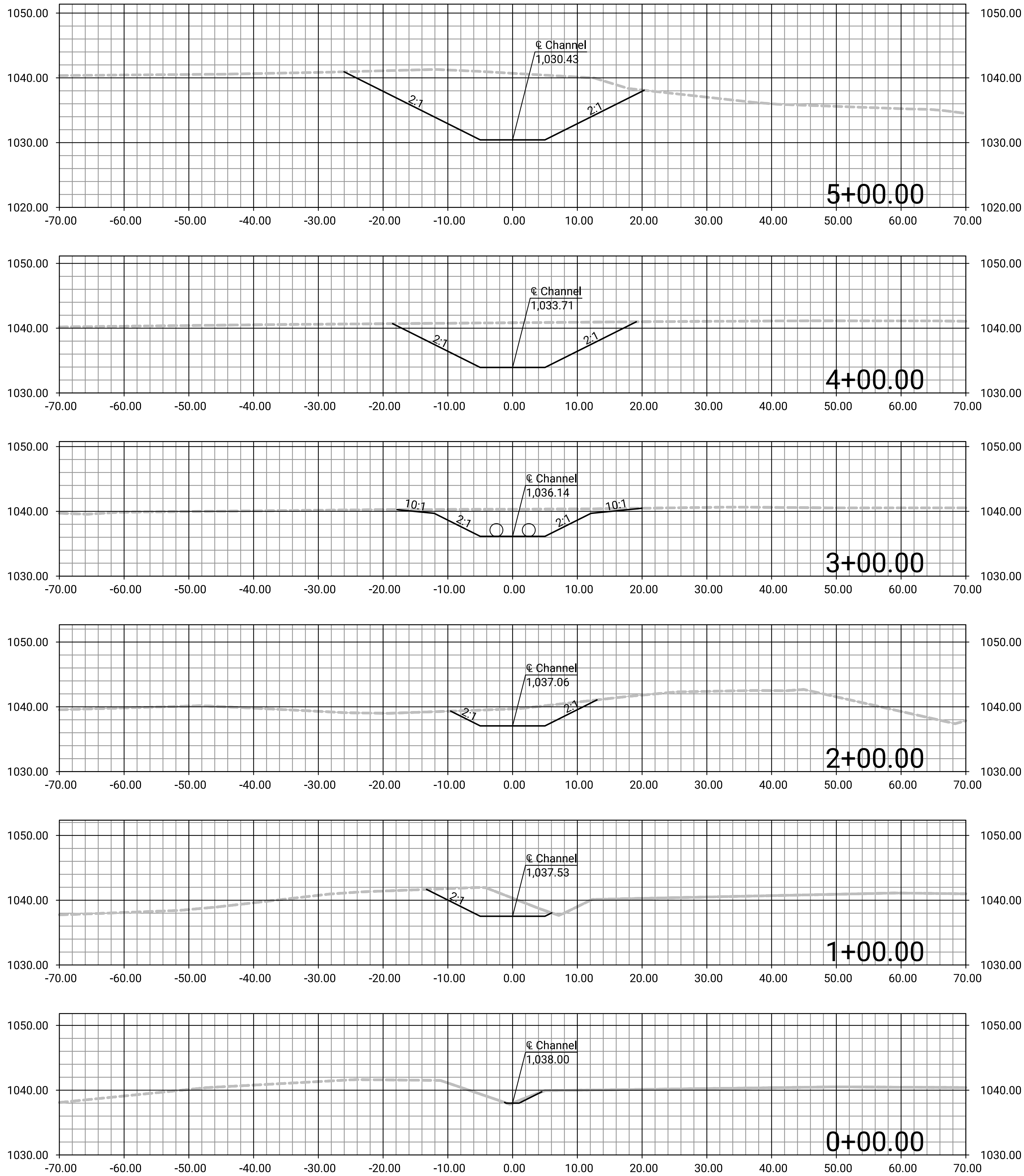


FIELD ENTRANCE  
STA. 0+40 TO STA. 7+00  
SCALE 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	63-66 KA-5729-01	2024	141	141

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	

Plotted by : Brogan Andrews@ks.gov 16-APR-2024 17:29  
File : KA572901rco-05.dgn



CHANNEL REALIGNMENT  
STA. 0+00 TO STA. 5+45  
SCALE 1:10