



DATE PREPARED

6 / 9 / 202

ROUTE
US - 61

STATE
MODISTRICT
BR

SHEET NO
9682-1

COUNTY

ST. CHARLES

JC

JST0020

CONTRACT ID

PROJECT NO

BRIDGE NO.

A9682

[illegible]

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

 105 WEST CAPITOL
JEFFERSON CITY, MO 65102



HNTB
715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270

Bartlett & West
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PHONE 573-434-3181
CERTIFICATE OF AUTHORITY NO. 000167 • ENGINEERING

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Designed	JUN	2025
Detailed	JUN	2025
Checked	JUN	2025

Note: This drawing is not to scale. Follow dimensions.

Sheet 1 of 43

BRIDGE: ROUTE 61S OVER I-70

ROUTE 61 FROM ROUTE A TO ROUTE N
ABOUT 1.3 MILES SOUTH OF ROUTE A
BEGINNING STATION 386+15.95

Foundation Data						
Type	Design Data	Bent Number				
		1	2	3	4	5
Load Bearing Pile	Pile Type and Size	HP12x53	--	--	--	HP12x53
	Number	ea 9	--	--	--	10
	Approximate Length Per Each	ft 38	--	--	--	40
	Pile Point Reinforcement	ea ALL	--	--	--	ALL
	Min. Galvanized Penetration (Elev.)	ft Full Length	--	--	--	Full Length
	Pile Driving Verification Method	WEAP	--	--	--	WEAP
	Resistance Factor	0.5	--	--	--	0.5
	Minimum Nominal Axial Compressive Resistance	kip 400	--	--	--	404
Rock Socket	Number	ea --	4	4	4	--
	Foundation Material	--	--	Shale	Shale	--
	Elevation Range	ft --	--	598.4-590.7	593.0-581.1	--
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf --	--	17.4	17.4	--
	Foundation Material	--	Limestone	Limestone	Limestone	--
	Elevation Range	ft --	592.4-588.3	590.7-583.7	581.1-560	--
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf --	31.4	31.4	31.4	--
	Foundation Material	--	Shale	--	--	--
	Elevation Range	ft --	588.3-587.6	--	--	--
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf --	17.4	*	--	--
	Foundation Material	--	Limestone	--	--	--
	Elevation Range	ft --	587.6-565.0	--	--	--
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf --	31.4	--	--	--
	Minimum Nominal Axial Compressive Resistance (Tip Resistance)	ksf --	400	400	400	--

WEAP = Wave Equation Analysis of Piles

Load Bearing Piles:

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

Rock Socket (Drilled Shafts):

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factors (Side Resistance + Tip Resistance)}}$

Driven Piles:

Prebore for piles at End Bents No. 1 and No. 5 to elevations 616.00 and 605.00, respectively.

All piles shall be galvanized full length.

Manufactured pile point reinforcement shall be used on all piles in this structure.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Prebore holes shall be backfilled with sand (or other approved material), in accordance with Sec 702, before piles are driven to bearing.

Roadway fill shall be completed to the final roadway section and up to the elevation of the bottom of the concrete beam within the limits of the structure and for not less than 25 feet in back of the fill face of the end bents before any piles are driven for any bents falling within the embankment section.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
Seismic Design Category = A (Seismic Details plus Abutment Seismic Design)

Design Loading:

Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf
Superstructure: Simply-Supported, Non-Composite for Dead Load. Continuous Composite for live loads.

Detailed JUN 2025
Checked JUN 2025

Note: This drawing is not to scale. Follow dimensions.

General Notes:

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psi
Class B-2 Concrete (Drilled Shafts & Rock Sockets) f'c = 4,000 psi
Class B-1 Concrete (Barrier) f'c = 4,000 psi
Class B-2 Concrete (Superstructure, except Prestressed Girders and Barrier) f'c = 4,000 psi
Reinforcing Steel (ASTM A706 Grade 60) fy = 60,000 psi
Structural Steel HP Pile (ASTM A709 Grade 50) fy = 50,000 psi
Structural Steel Girder Chairs (ASTM A709 Grade 36) fy= 36,000 psi
For prestressed girder stresses, see Sheets No. 22 thru 25

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Pile Protective Coatings:

Piles shall be galvanized in accordance with Sec 702 and Sec. 1081.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

All exposed substructure elements will contain epoxy coated reinforcing.

Traffic Handling:

Vertical Clearance for I-70 & Ramp 70W-Z traffic during construction shall be 15 ft. minimum over all lanes of live traffic as shown in each phase of the Maintenance of Traffic Plans.

Structure to be closed during construction. Traffic to be maintained on other structures during construction. See Maintenance of Traffic Plans for traffic control.

Structural Steel Coating:

Structural steel for the girder chairs shall be coated with not less than 2 mils of organic zinc primer. Scratched or damaged surfaces are to be touched up in the field before concrete is poured. In lieu of coating, the girder chairs may be galvanized in accordance with ASTM A123.

Concrete Protective Coatings:

Protective coating for concrete bents and piers (Epoxy) shall be applied to all exposed substructure surfaces and in accordance with Sec 711.

Miscellaneous:

High strength bolts, nuts and washers will be sampled for quality assurance as specified in Sec 106.

Stay-In-Place Forms:

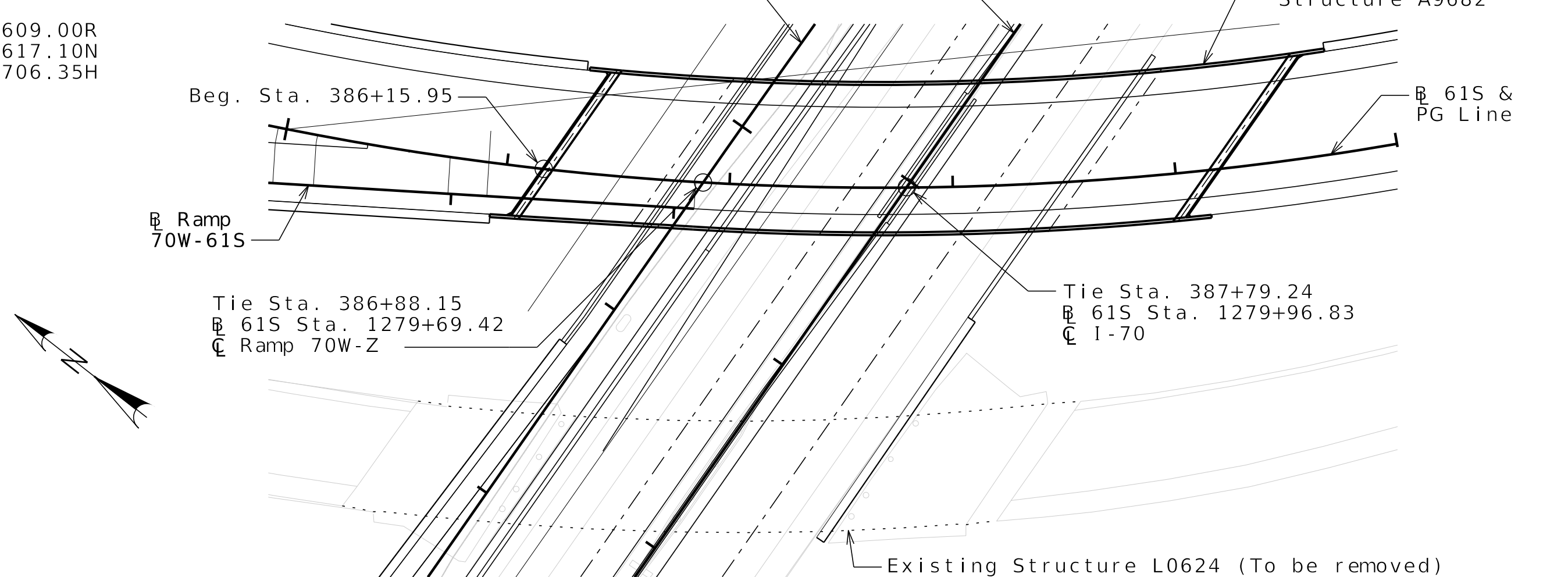
Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirements and coating designation G165 of ASTM A653. Complete Shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for falsework and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.

Standard Plans:



LOCATION SKETCH

GENERAL NOTES

STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER
PE-2021035906
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
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SHEET NO.
A9682-3

COUNTY
ST. CHARLES

JOB NO.
JST0020

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PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

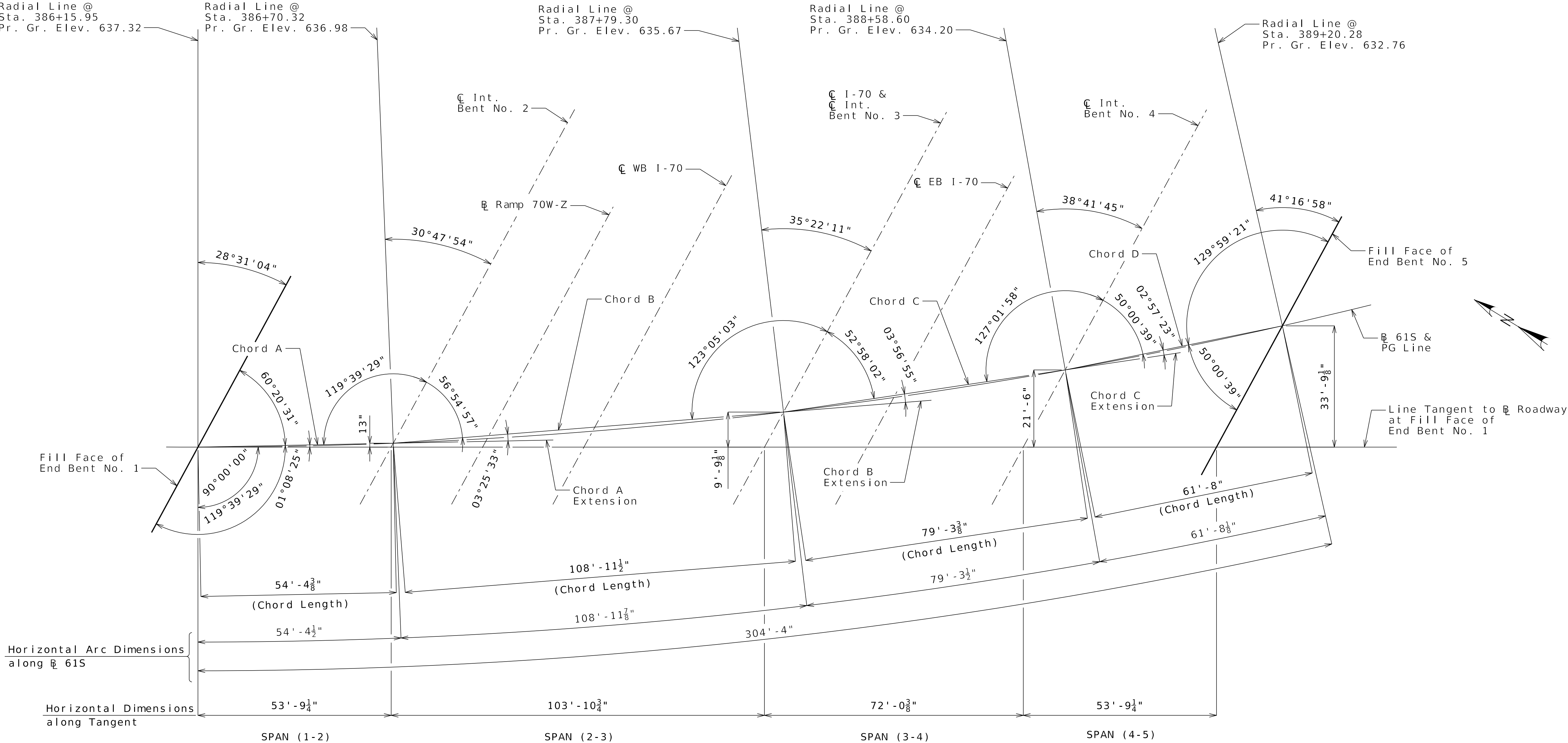
105 WEST CAPITOL
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PLAN SHOWING LAYOUT DATA FOR SUBSTRUCTURE

Notes:
All bents are parallel.
All dimensions are horizontal.

STATE OF MISSOURI
DAVID MORANDEIRA
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PE-2021035906
PROFESSIONAL ENGINEER

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B_A9682_003A_JST0020_SUBLAY.dgn 9:25:22 AM 6/9/2025



DETAILS OF HP PILE ANCHORS



- Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

Reinforcing steel shall be shifted to clear piles.
U bars shall clear piles by at least 1 1/2 inches.

DETAILS OF END BENT NO. 1



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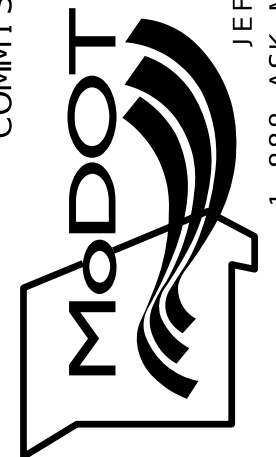
BRIDGE NO.

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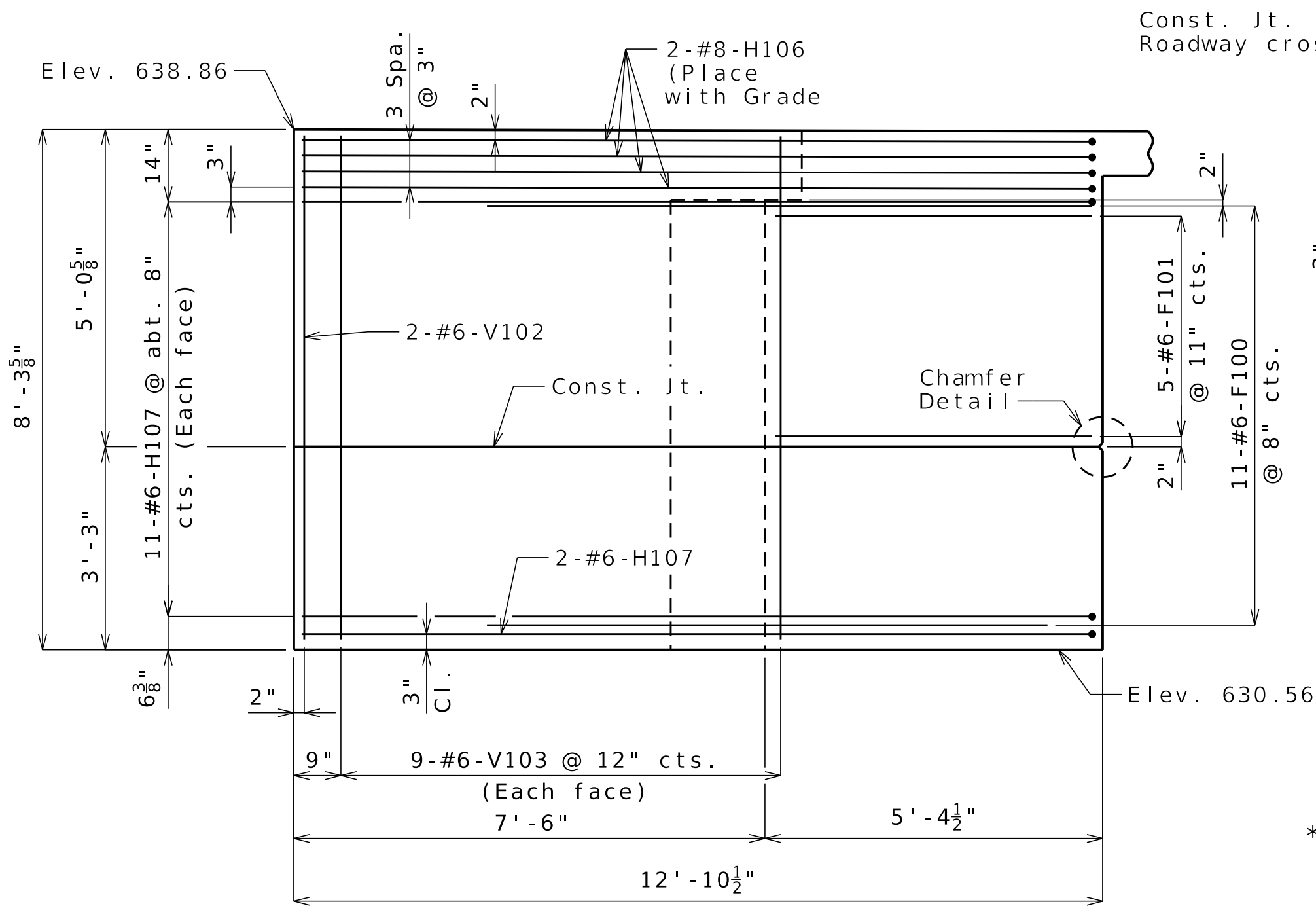
501 MONROE ST. SUITE 201 - JEFFERSON CITY, MO 65101
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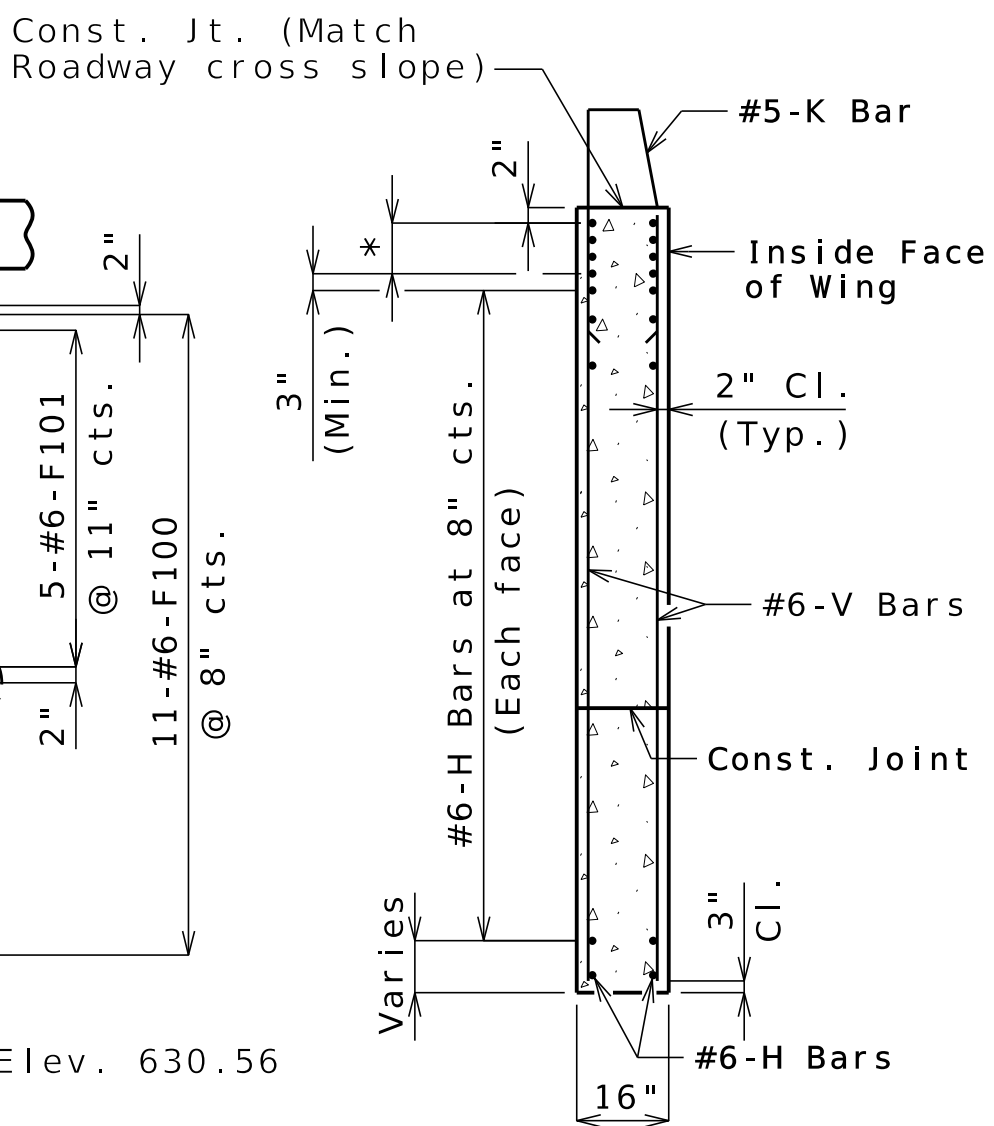
Notes:
Lap Splices for #6 - H100 bars = 4'-0" (Min.).
Lap Splices for #9 - H101 bars = 8'-6" (Min.).



For details of bridge approach slab, see Sheet No. 34.

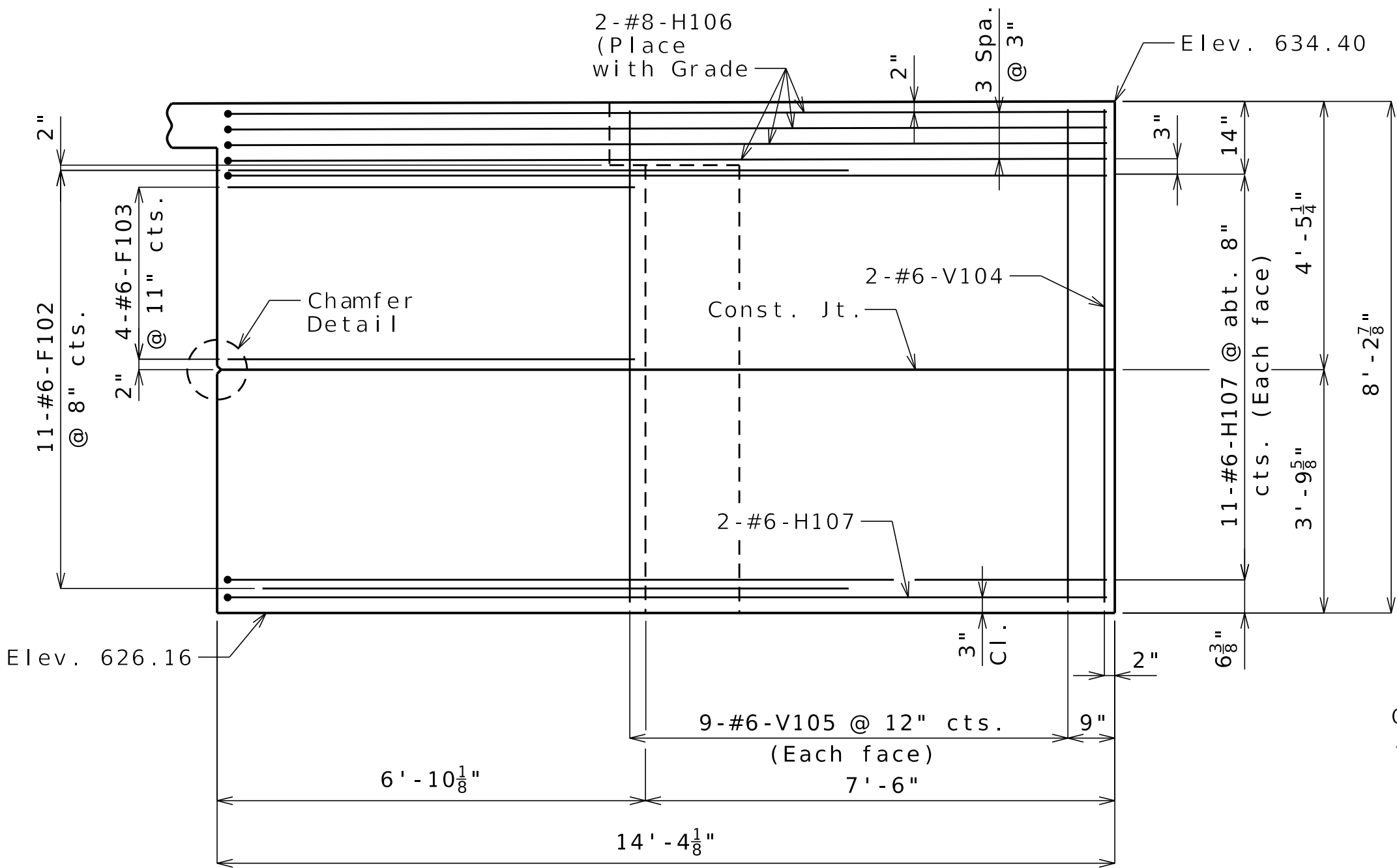


ELEVATION E-E

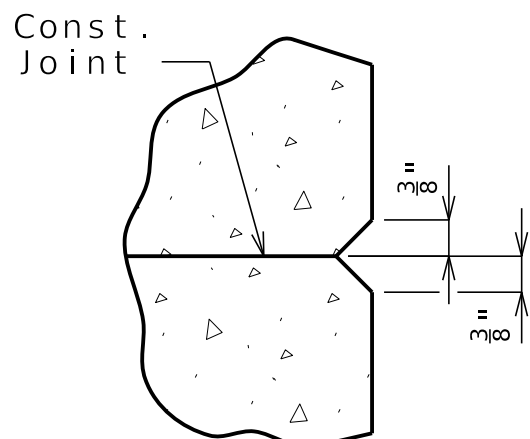


TYPICAL SECTION THRU WING

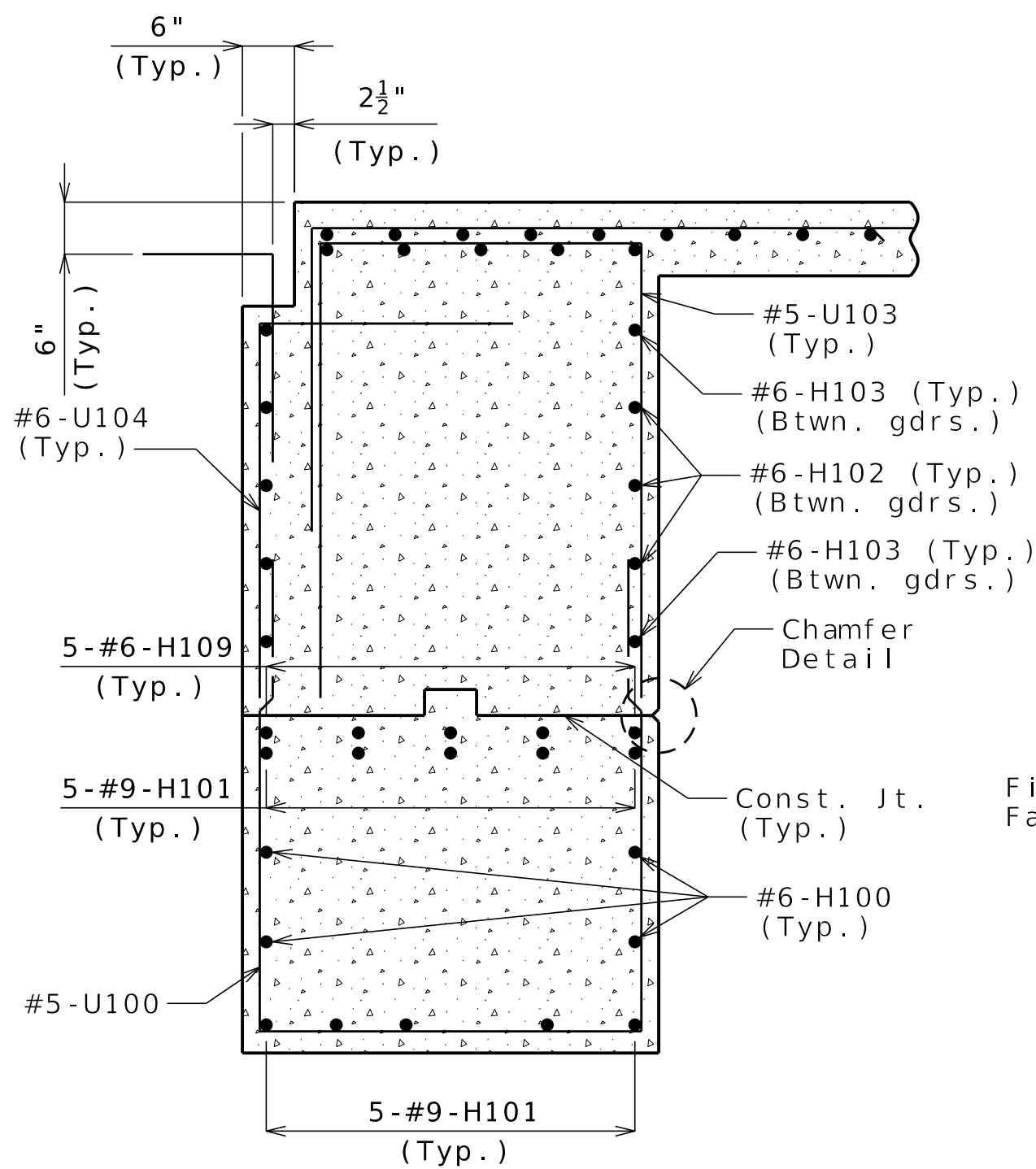
* #8-H Bars at 3" cts. (Each face) (Place with grade)



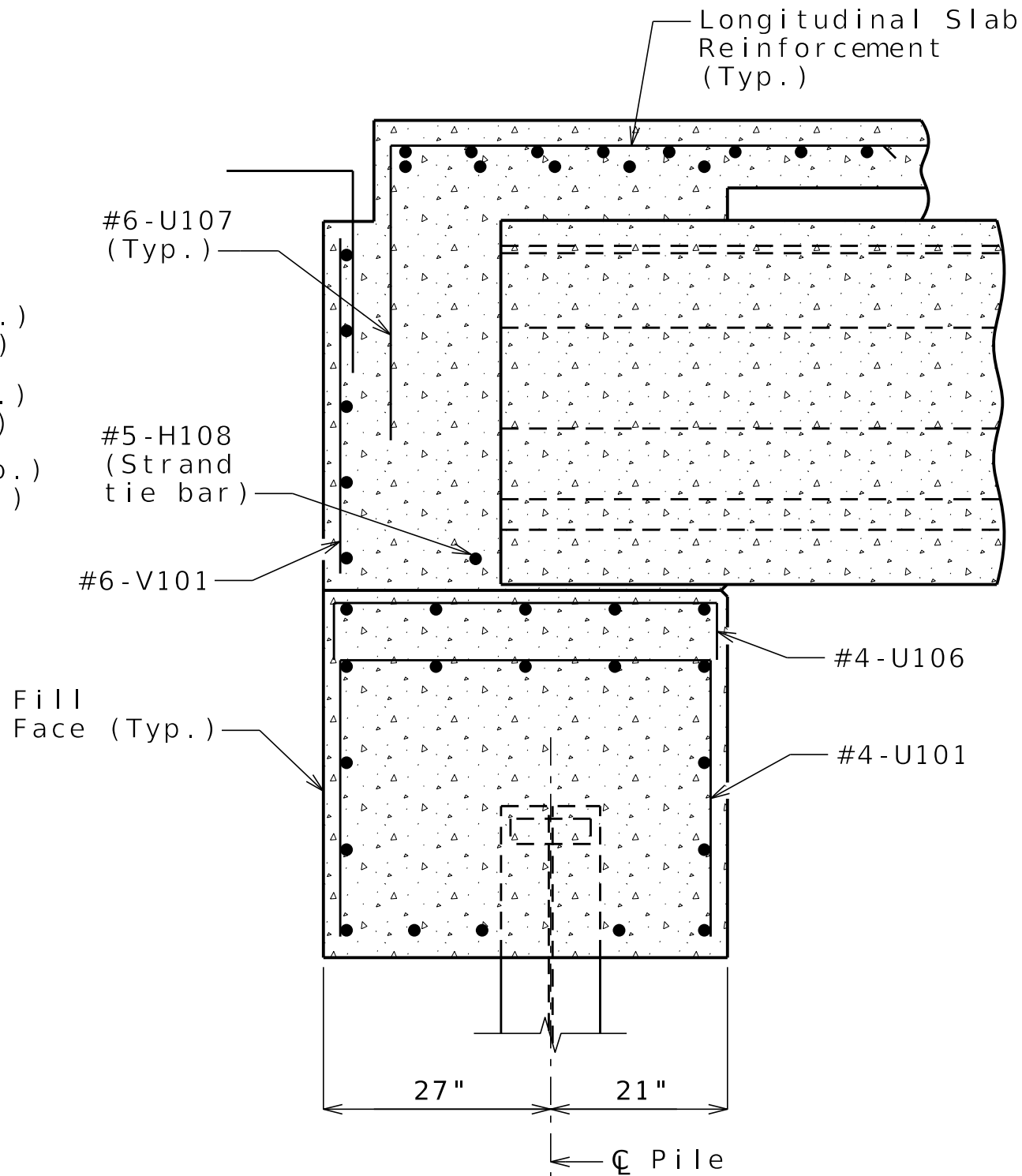
ELEVATION F-F



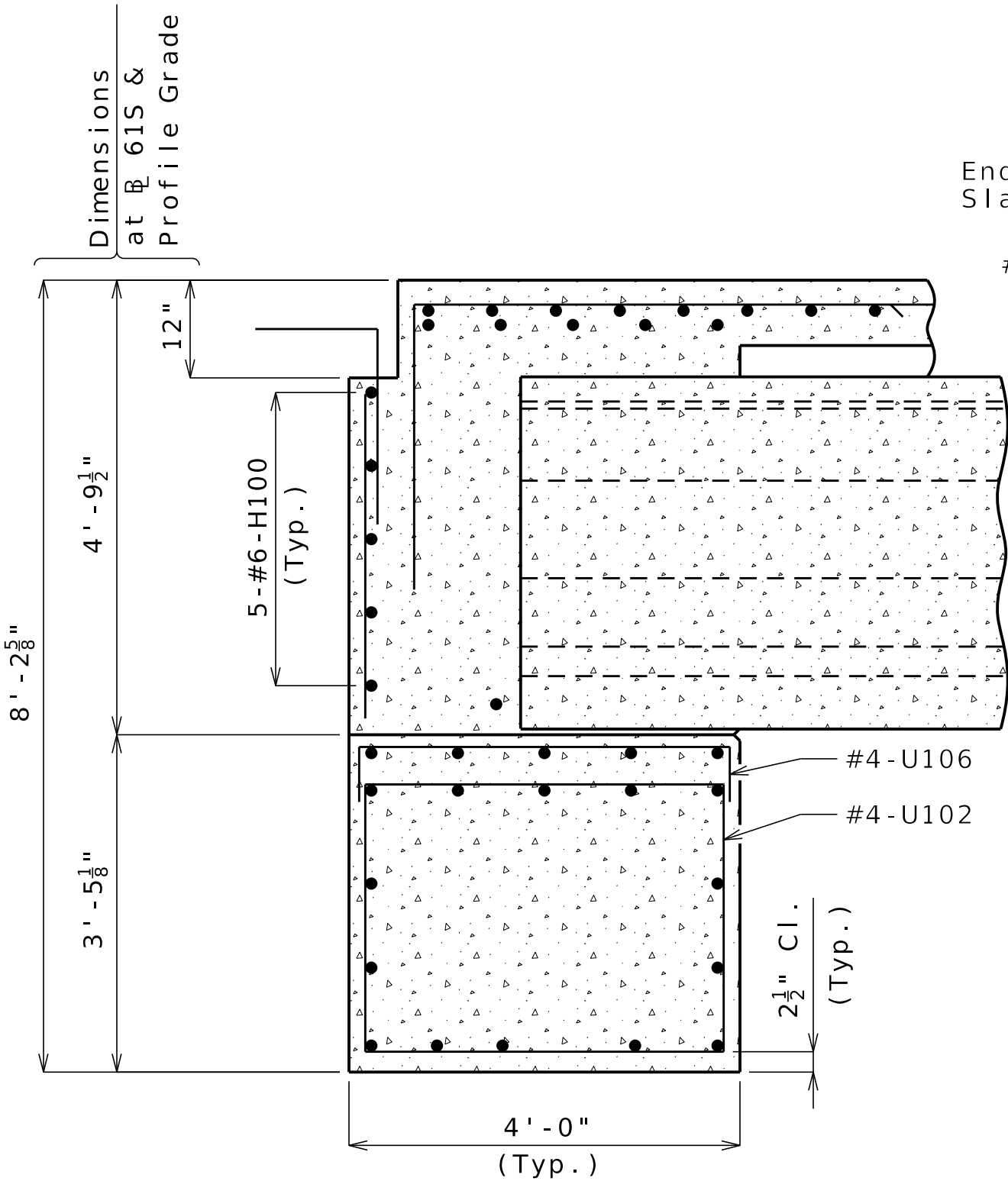
CHAMFER DETAIL



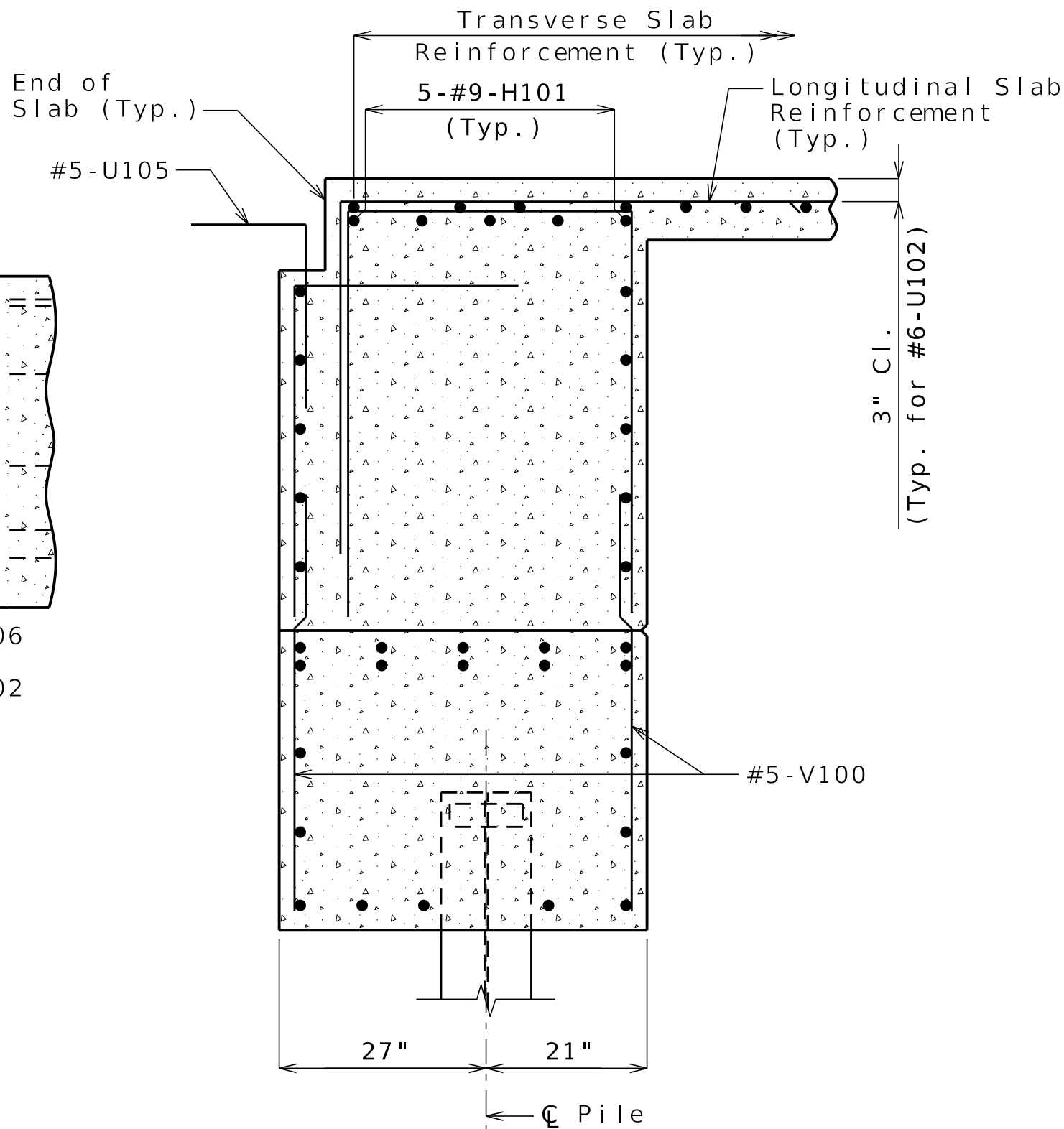
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

General Notes:

Work this sheet with Sheets No. 5 and 6.

For reinforcement of the barrier, see Sheet No. 33.

DETAILS OF END BENT NO. 1

Detailed JUN 2025
Checked JUN 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet 7 of 43



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10. *Journal of the American Medical Association*, 2000; 283: 2686-2692.

A9682

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** Stagger alternating V204 and V205 splice by 24". For odd number of bars, two bars in each column may splice at the same location. Cut bars as needed.

Dimension Variable Table			
Co lumn	A	B	C
1	17' - 0 ¹ / ₂ "	11' - 9"	10' - 0"
2	18' - 1 ¹ / ₈ "	11' - 9"	10' - 0"
3	19' - 3 ¹ / ₄ "	11' - 9"	10' - 0"
4	20' - 4 ¹ / ₂ "	11' - 9"	10' - 0"

ELEVATION

Beam keys not shown for clarity.

General Notes:

Drilled shafts shall be constructed and inspected in accordance with the Bridge Geotechnical Memo and the project Drilled Shaft Construction AAS.

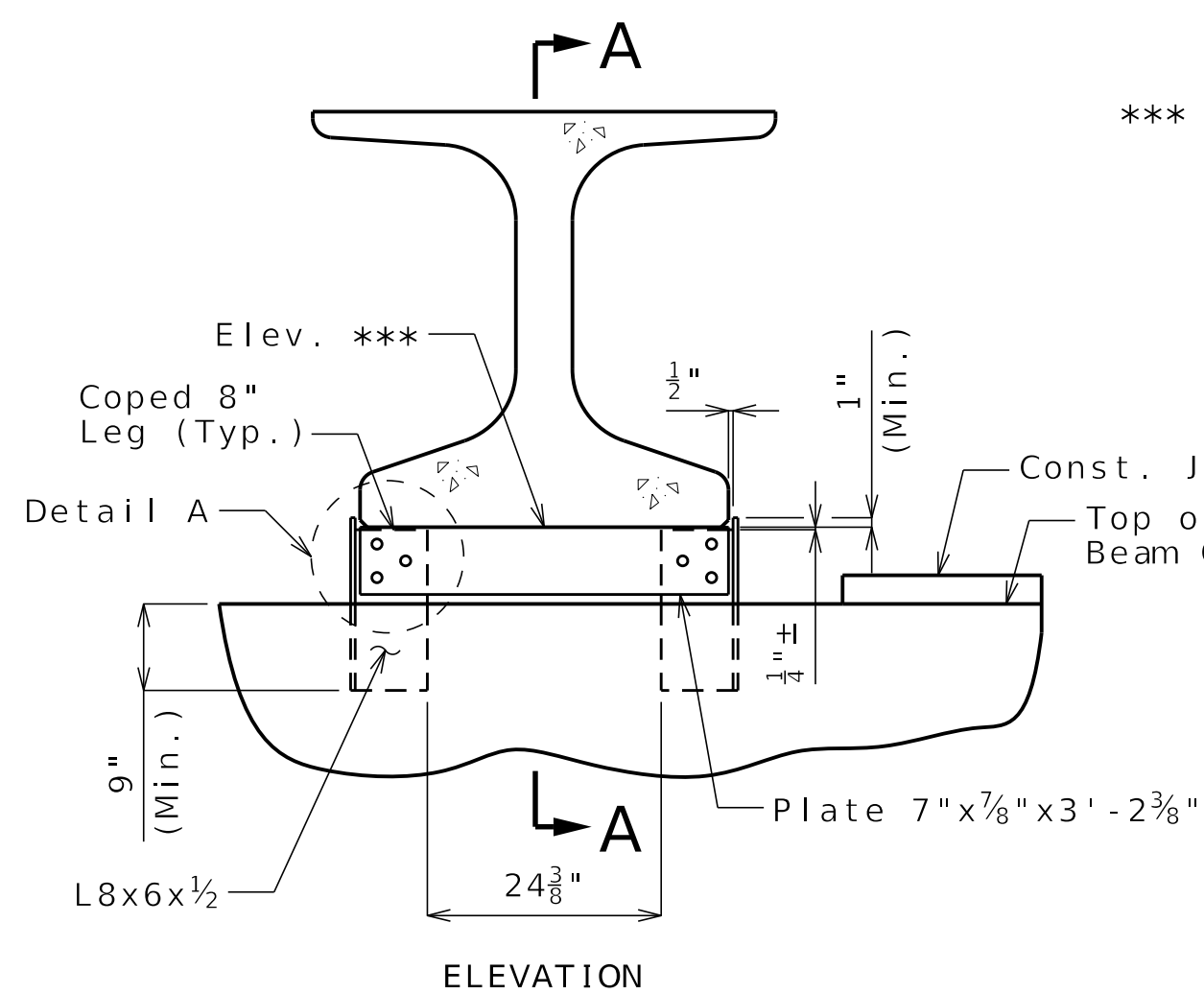
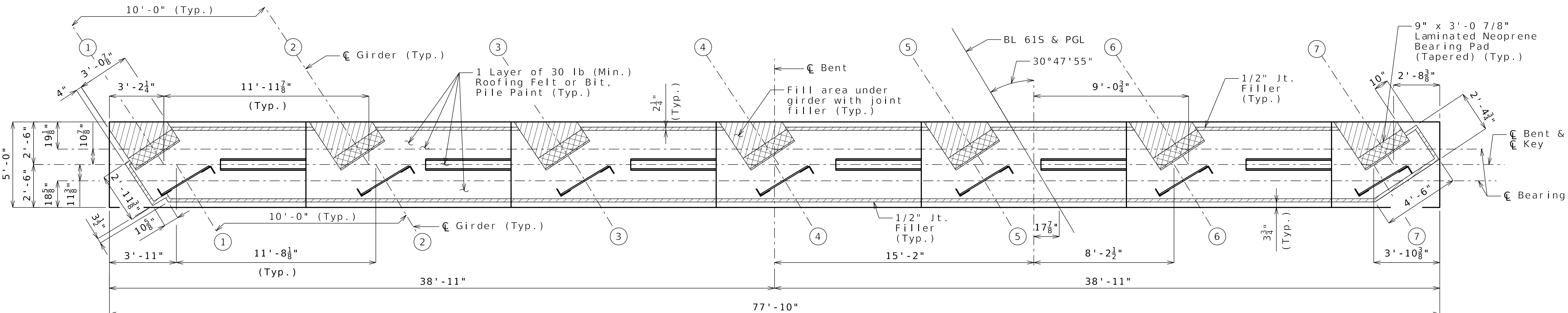
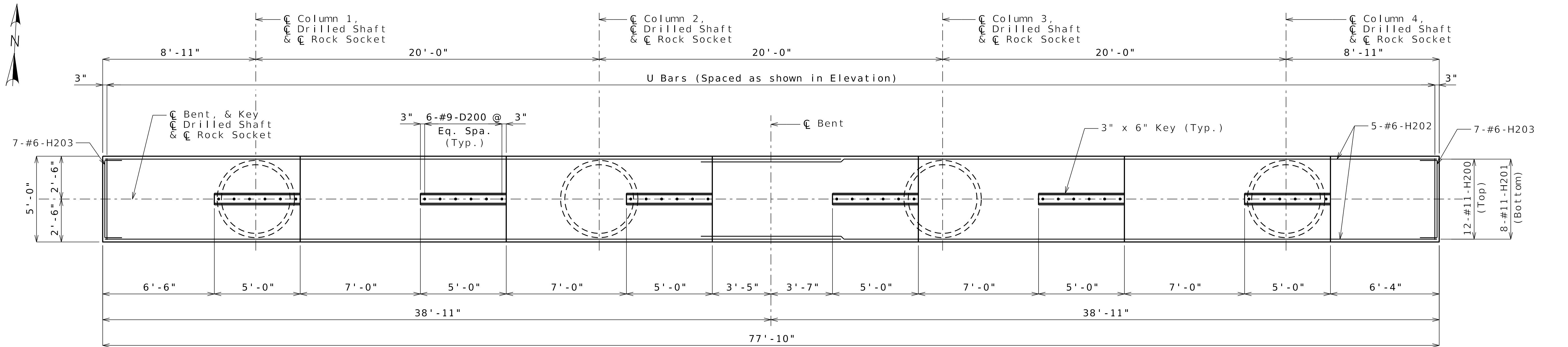
Work this sheet with Sheets No. 10 and 11.

Thickness of permanent steel casing shall be in accordance with Drilled Shaft Construction AAS.

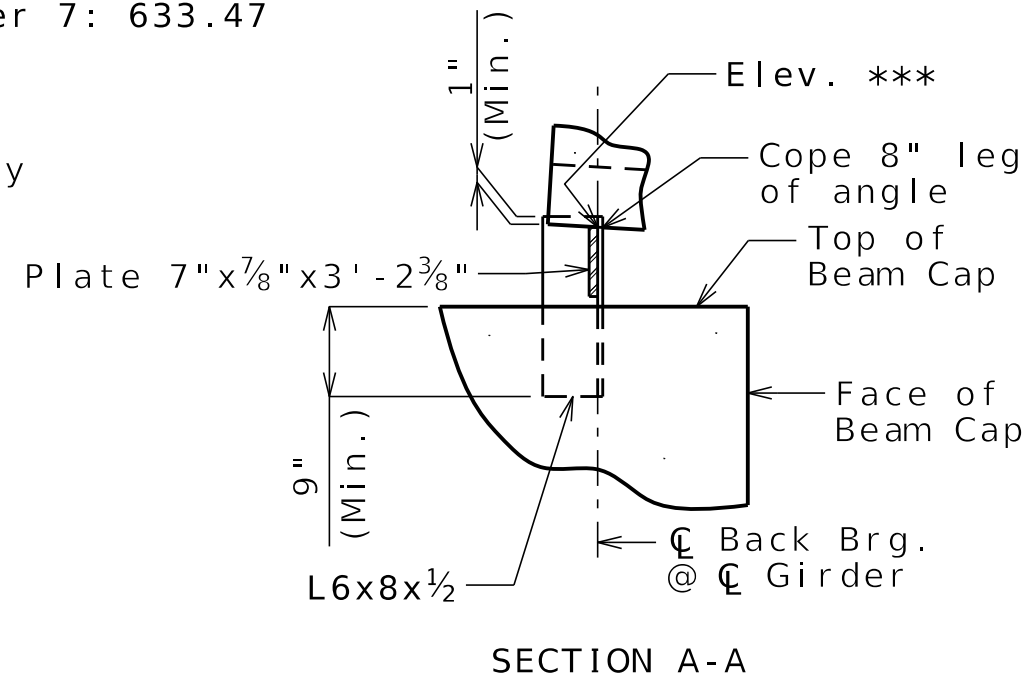
An additional 4 feet has been added to V-bar and additional 32-#6-P201 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

The tip of casing shall not extend into the rock socket elevation range reported in the Foundation Data table without approval by the engineer.

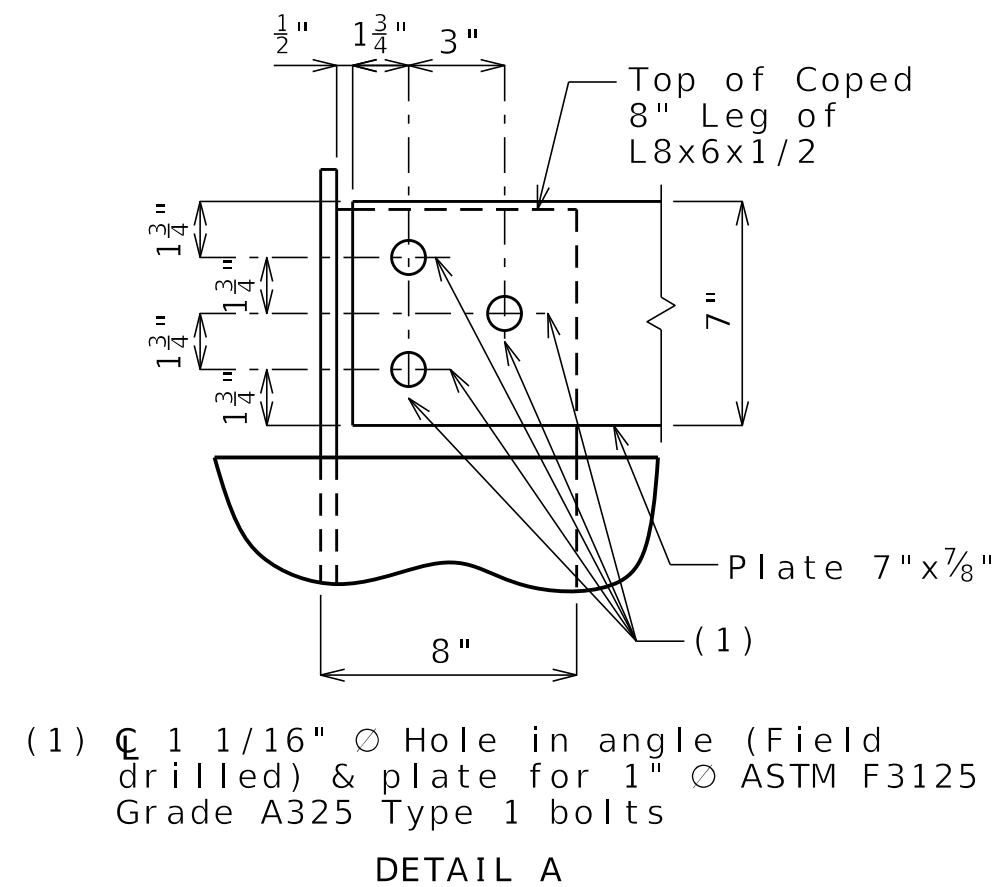


*** Girder 1: 629.55
Girder 2: 630.21
Girder 3: 630.86
Girder 4: 631.51
Girder 5: 632.17
Girder 6: 632.82
Girder 7: 633.47



GIRDER CHAIR DETAILS

PLAN OF BEAM



General Notes:

Work this sheet with Sheets No. 9 and 11.

For steps 2 inches or more, use 2 1/4x 1/2-inch joint filler up vertical face.

Structural steel for the girder beam chairs shall be coated with not less than 2 mills of inorganic zinc primer. Scratched or damaged surfaces are to be touched up in the field before concrete is poured. In lieu of coating, the girder beam chairs may be galvanized in accordance with ASTM A123.

Steel for chairs shall be ASTM A709 Grade 36.

Reinforcing steel shall be shifted to clear chairs.

DETAILS OF INTERMEDIATE BENT NO. 2



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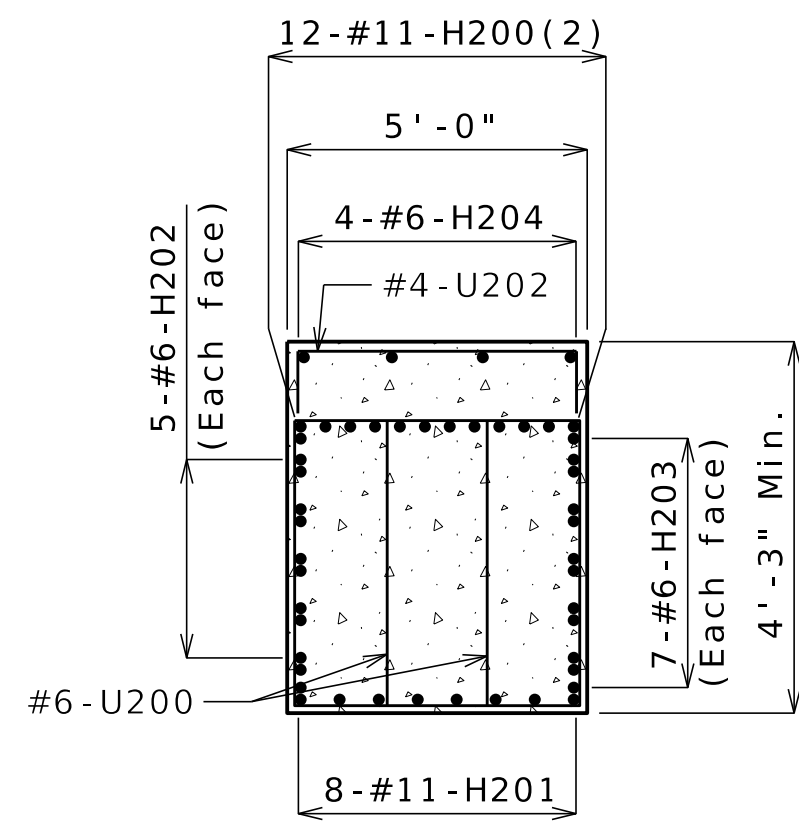
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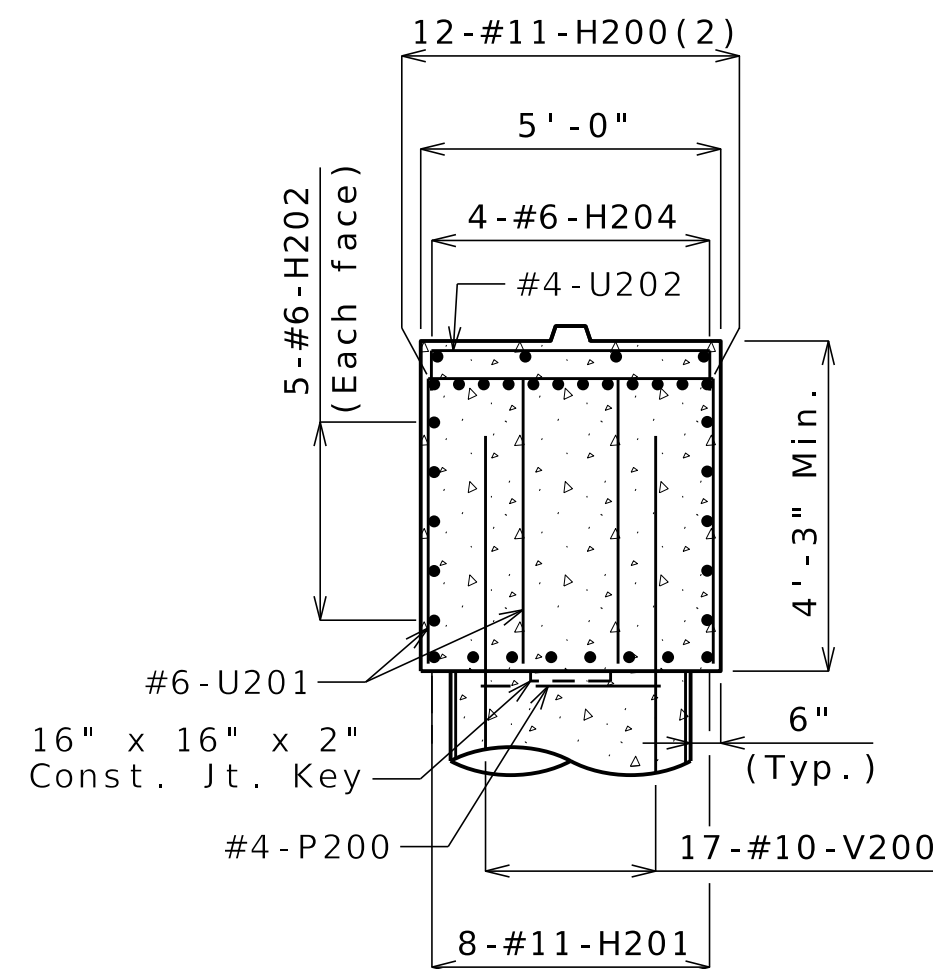
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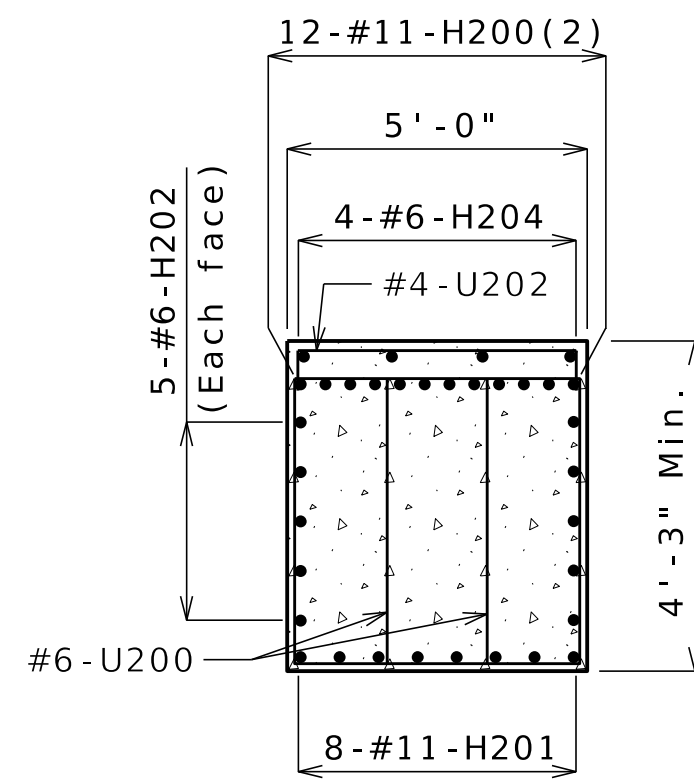
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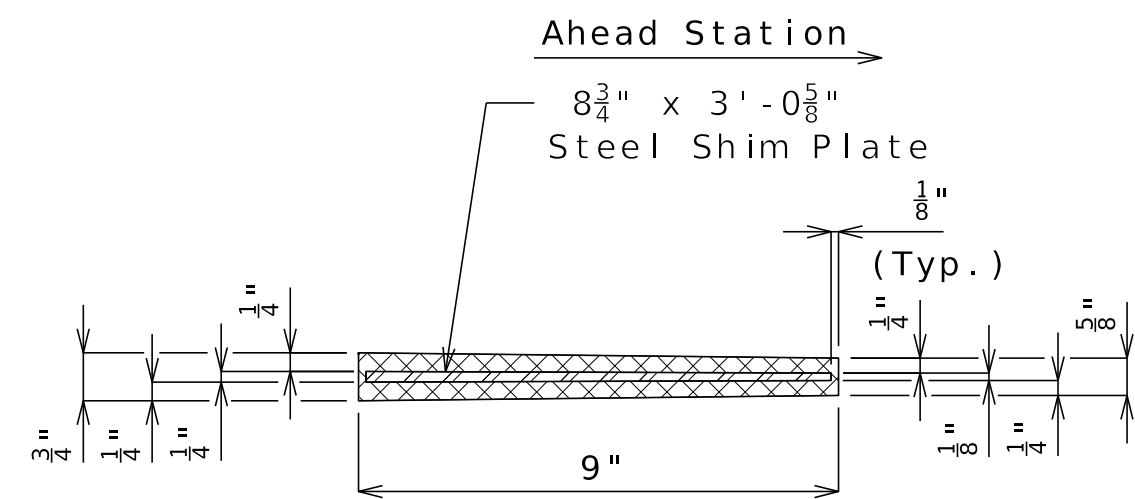
SECTION A-A



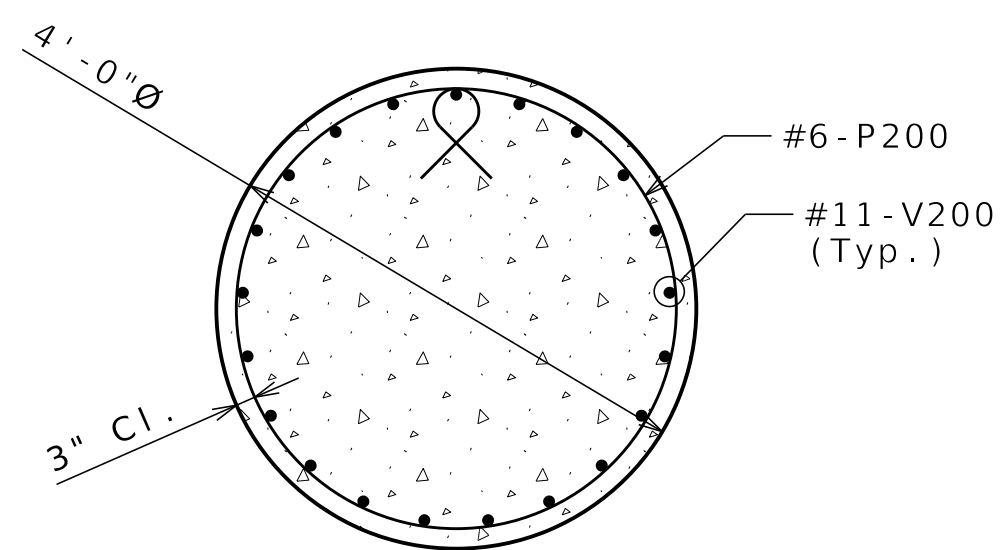
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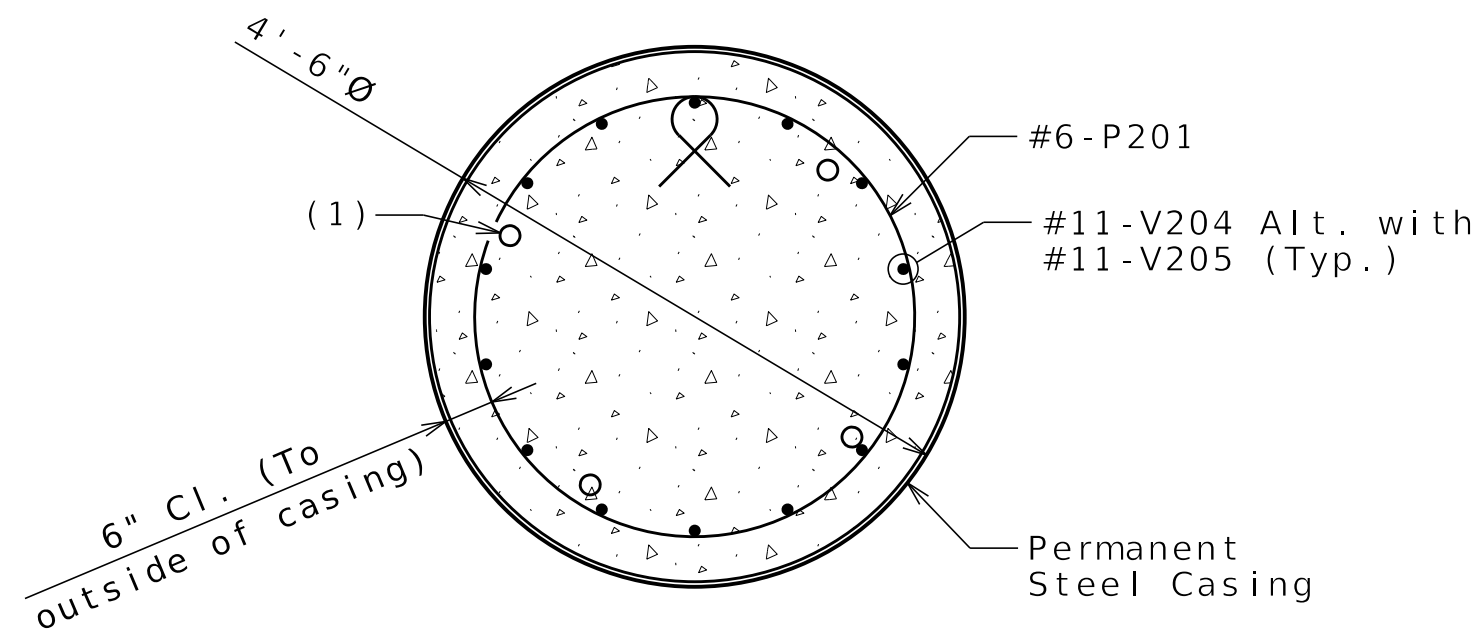
SECTION C-C



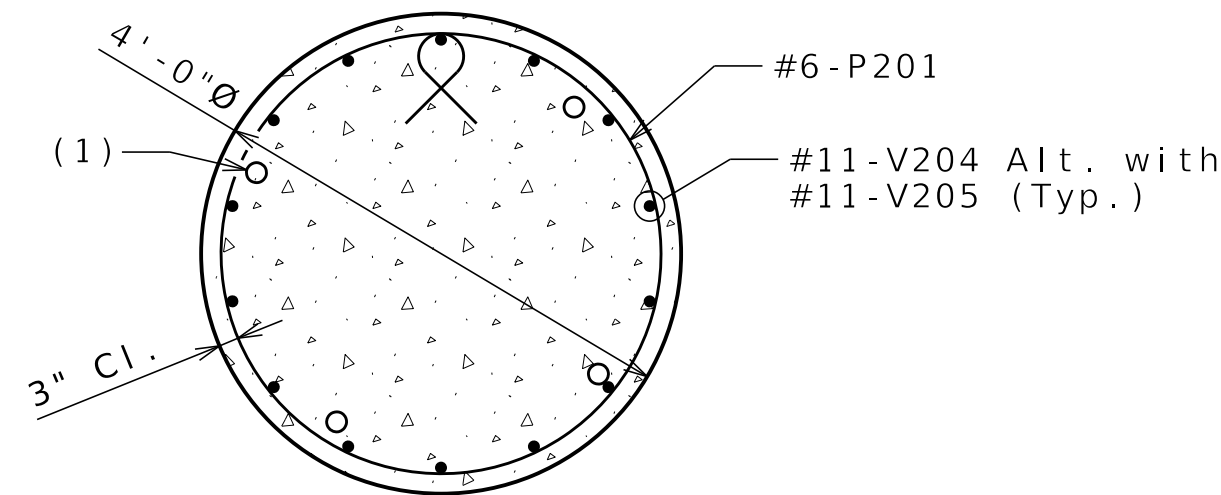
SECTION THRU LAMINATED NEOPRENE BEARING PAD (TAPERED) (BACK BRG.)
(7 Required)



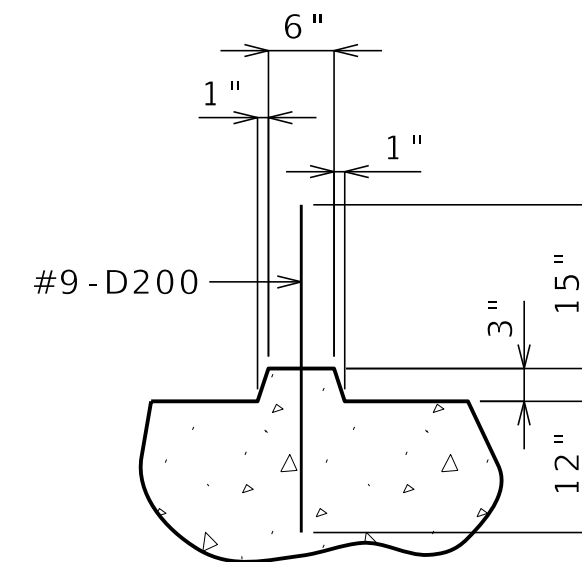
SECTION D-D



SECTION E-E

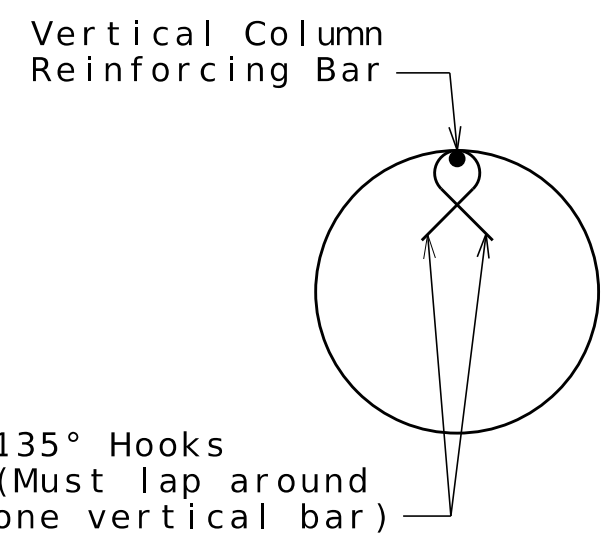


SECTION F-F



SECTION THRU KEY

- (1) 2"Ø Steel Pipe for sonic logging testing (4 each shaft)
- (2) Adjust bars as necessary to clear girder chair's embedded L-shape. Maintain 2 1/2" clear spacing between bars. Bundle bars horizontally if necessary.



SEISMIC STIRRUP BAR
(#6-P200 & P201)

Note: Seismic stirrups hooks shall be staggered around the column at about one-third of the hoop circumference.

General Notes:
Work this sheet with Sheets No. 9 and 10.

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BRIDGE NO.
A9682

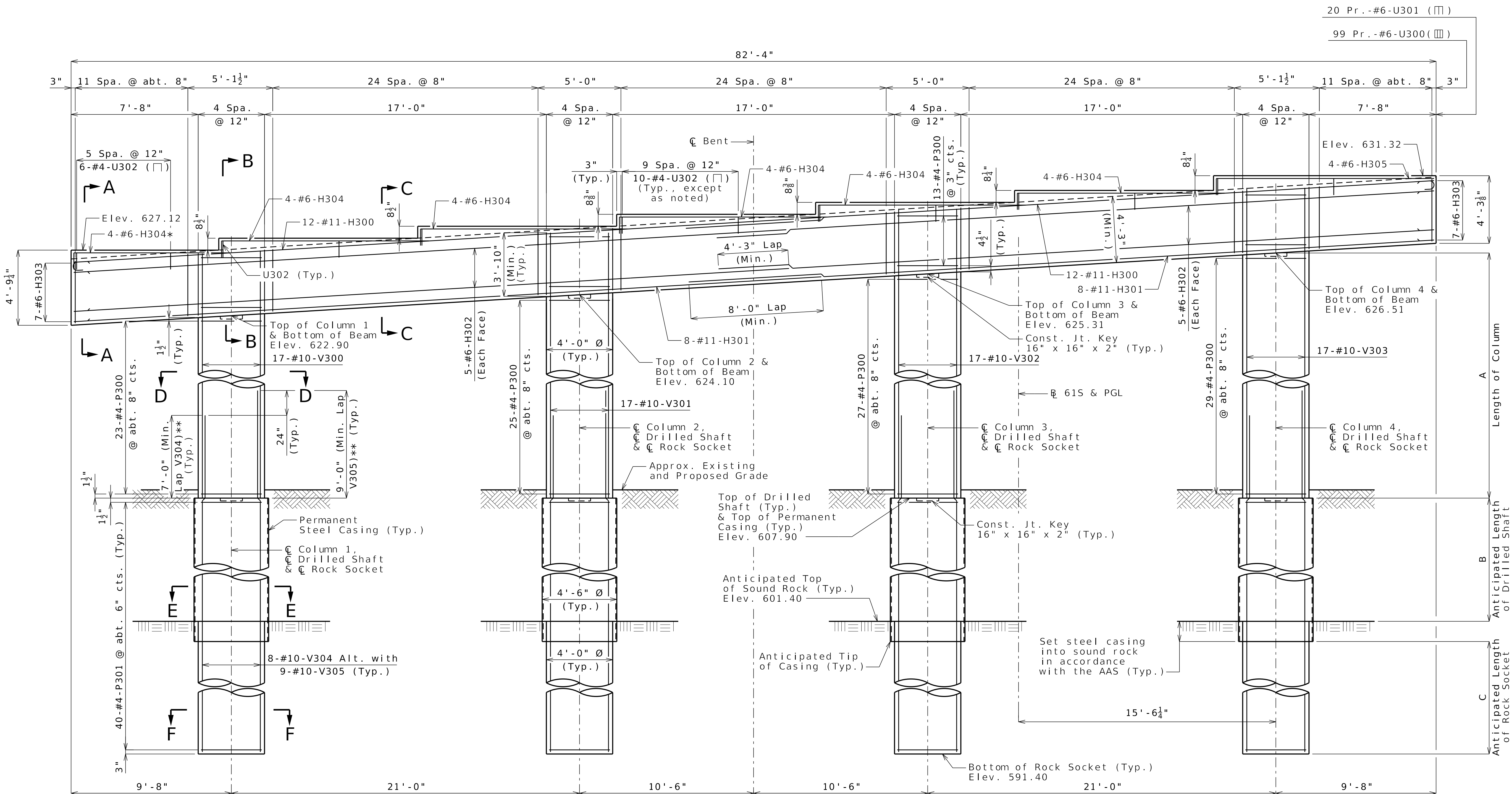
DESCRIPTION
DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

IMPROVE70
ALLIANCE

HNTB
715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
KANSAS CITY, MO
601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 8743033181
WWW.BARTLETTWEST.COM

Bartlett & West
ENGINEERING



ELEVATION
Beam keys not shown for clarity.

* Cut H304 bars as needed to fit step length.
** Stagger alternating V304 and V305 splice by 24". For odd number of bars, two bars in each column may splice at the same location. Cut bars as needed.

Dimension Variable Table			
Column	A	B	C
1	15'-0"	6'-6"	10'-0"
2	16'-2½"	6'-6"	10'-0"
3	17'-4⅞"	6'-6"	10'-0"
4	18'-7¼"	6'-6"	10'-0"

General Notes:

Drilled shafts shall be constructed and inspected in accordance with the Bridge Geotechnical Memo and the project Drilled Shaft Construction AAS.

Work this sheet with Sheets No. 13 and 14.

Thickness of permanent steel casing shall be in accordance with Drilled Shaft Construction AAS.

An additional 4 feet has been added to V-bar and additional 32-#4-P301 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

The tip of casing shall not extend into the rock socket elevation range reported in the Foundation Data table without approval by the engineer.

DETAILS OF INTERMEDIATE BENT NO. 3



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
6/9/2025

ROUTE STATE
US-61 MO

DISTRICT SHEET NO.
BR A9682-12

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

DESCRIPTION

DATE

DESCRIPTION

DATE

DESCRIPTION

DATE

DESCRIPTION

DATE

DESCRIPTION

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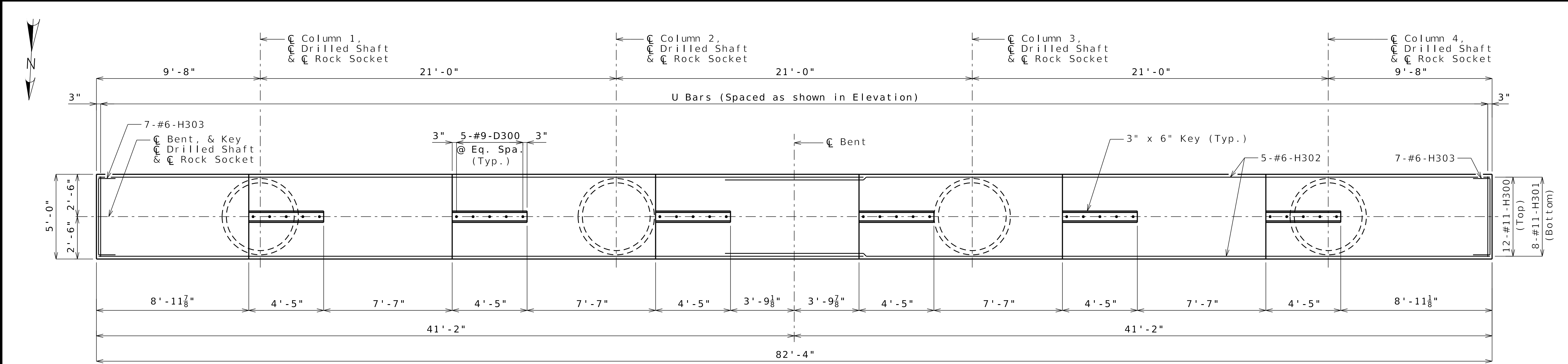
DESCRIPTION

DATE

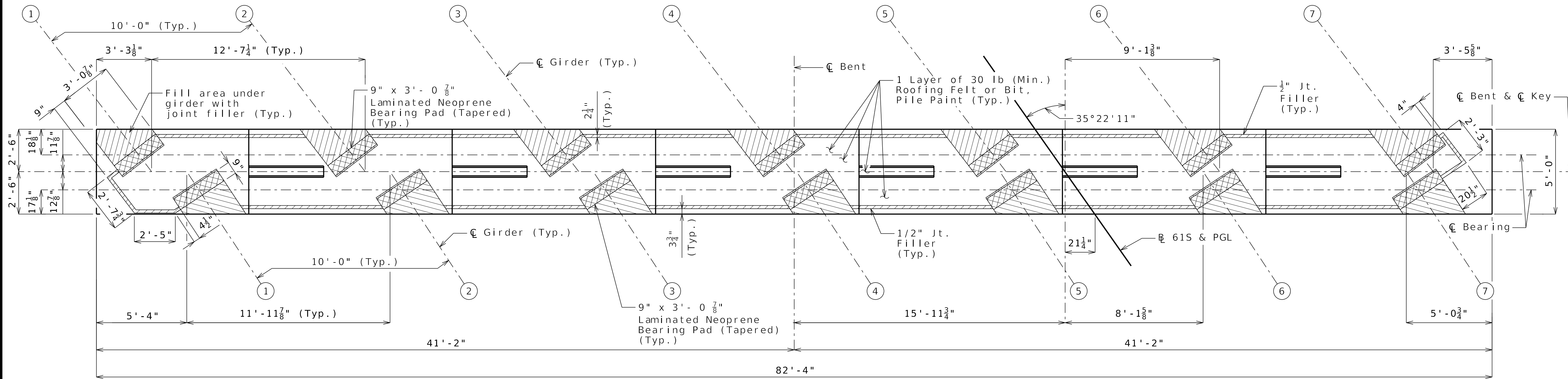
DESCRIPTION

DATE

DESCRIPTION



PLAN SHOWING REINFORCEMENT

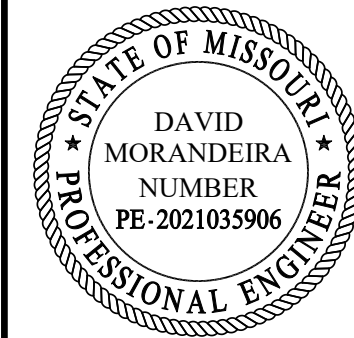


PLAN OF BEAM

General Notes:

Work this sheet with Sheets No. 12 and 14.

For steps 2 inches or more, use 2 1/4x 1/2-inch joint filler up vertical face.



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
6/9/2025

ROUTE
US-61

DISTRICT
BR

SHEET NO.
A9682-13

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

IMPROVE 70
ALLIANCE

HNTB

715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270

Bartlett & West

601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 572-630-3181
CERTIFICATE OF AUTHORITY
WWW.BARTLETTWEST.COM

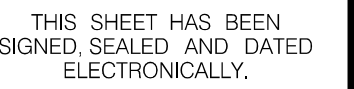


Note: Seismic stirrups hooks shall be staggered around the column at about one-third of the hoop circumference.



General Notes:

Work this sheet with Sheets No. 12 and 13.



DATE PREPARED

DATE PREPARED

6 / 9 / 2025

ROUTE	STATE
US - 61	MO

DISTRICT	SHEET NO
BR	A9682 - 1

COUNTY

ST. CHARLES

JOB NO.

JST0020

CONTRACT ID.

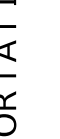
PROJECT NO.

BRIDGE NO.

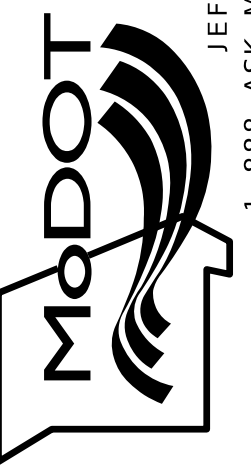
A9682

[illegible]

**MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION**

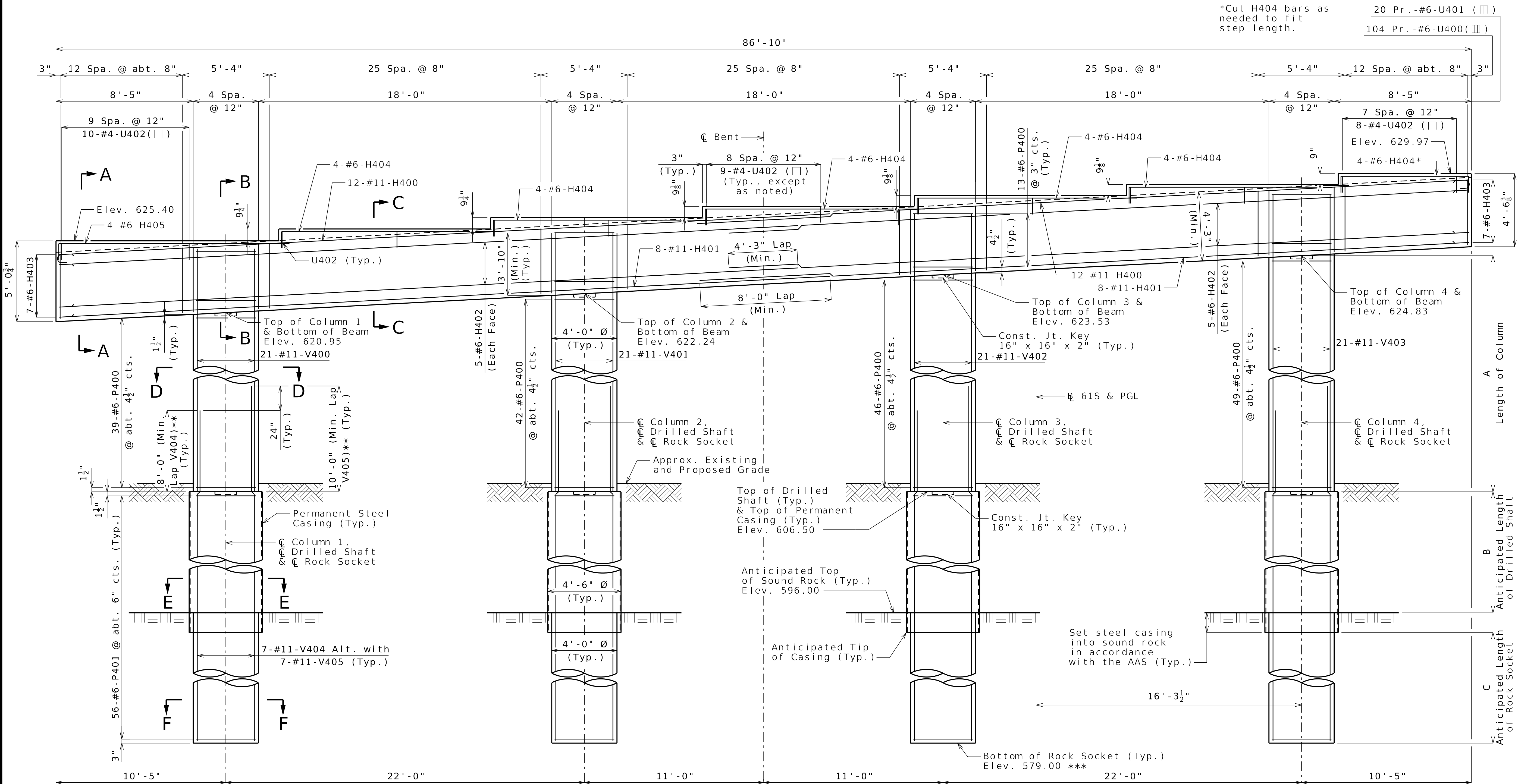
The logo for the Missouri Department of Transportation (MoDOT) features the word "MoDOT" in a bold, sans-serif font. The "M" and "O" are connected, and the "D" is a simple outline. Below the text is a stylized graphic of a road or highway curving upwards and to the right, with three curved lines above it suggesting a bridge or a road surface.

105 WEST CAPITAL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



HNTB
715 KIRK DRIVE
KANSAAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270

Bartlett & West
601 MONROE ST. SUITE 201 • JEFFERSON CITY, MO 65101
PHONE 573-624-2198
FAX 573-624-2199
CERTIFICATE OF AUTHORITY NO. 000167 • ENGINEERING
WWW.BARTLETTWEST.COM



** Stagger alternating V404 and V405 splice by 24". For odd number of bars, two bars in each column may splice at the same location. Cut bars as needed.

Dimension Variable Table			
Column	A	B	C
1	14'-5 ³ / ₈ "	10'-6"	14'-0"
2	15'-8 ⁵ / ₈ "	10'-6"	14'-0"
3	17'-0 ³ / ₈ "	10'-6"	14'-0"
4	18'-3 ⁵ / ₈ "	10'-6"	14'-0"

ELEVATION
Beam keys not shown for clarity.

General Notes:

Work this sheet with Sheets No. 16 and 17.

Drilled shafts shall be constructed and inspected in accordance with the Bridge Geotechnical Memo and the project Drilled Shaft Construction AAS.

Thickness of permanent steel casing shall be in accordance with Drilled Shaft Construction AAS.

An additional 4 feet has been added to V-bar and additional 32-#6-P401 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

The tip of casing shall not extend into the rock socket elevation range reported in the Foundation Data table without approval by the engineer.

DETAILS OF INTERMEDIATE BENT NO. 4

Detailed JUN 2025
Checked JUN 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet 15 of 43



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
6/9/2025

ROUTE US-61 STATE MO

DISTRICT BR SHEET NO. A9682-15

COUNTY ST. CHARLES

JOB NO. JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL

JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

MoDOT

IMPROVE 70 ALLIANCE

HNTB

Bartlett & West

715 KIRK DRIVE

KANSAS CITY, MO 64105-1310

CERTIFICATE OF AUTHORITY

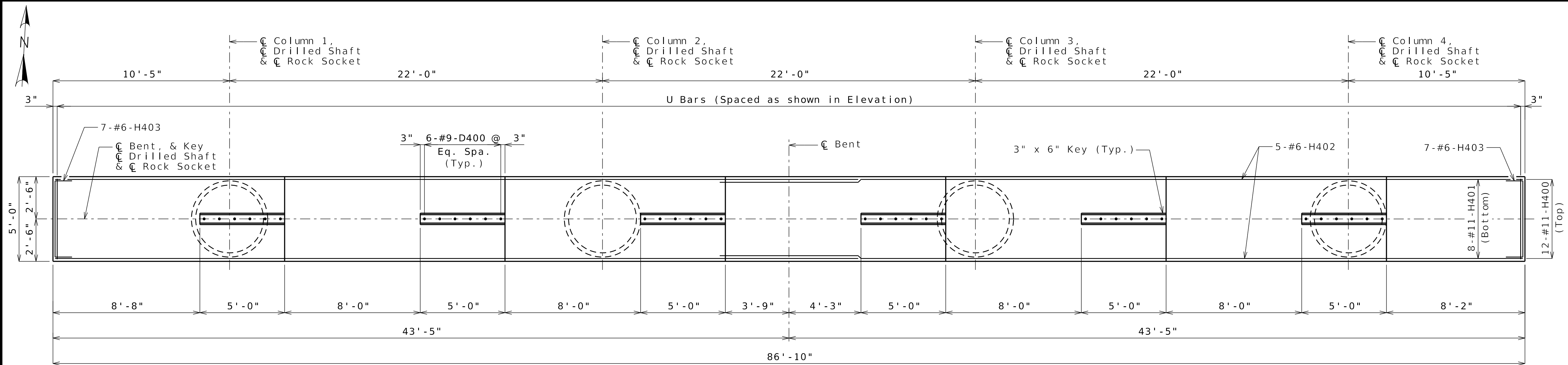
NO. 001270

601 MONROE ST., SUITE 201, JEFFERSON CITY, MO 65101

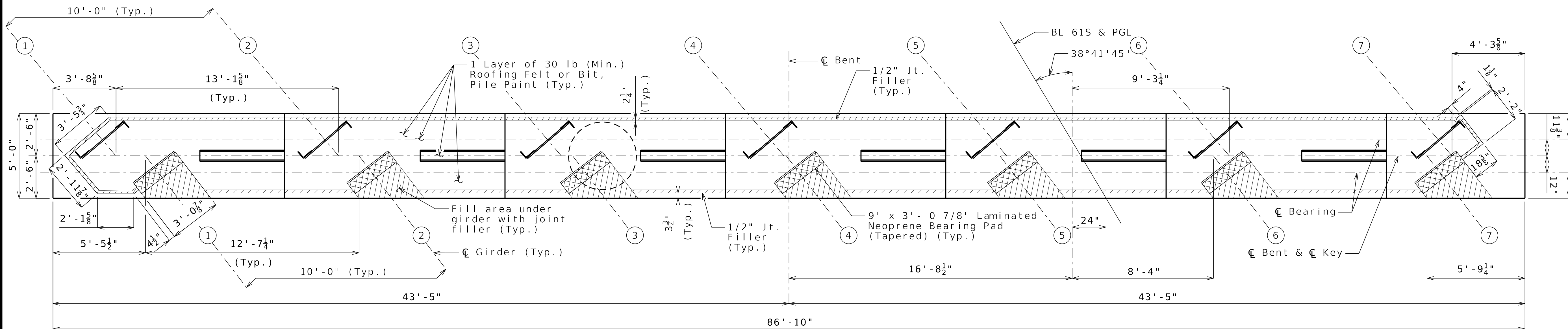
PHONE 872-633-3181

CERTIFICATE OF PROFESSIONAL ENGINEERING

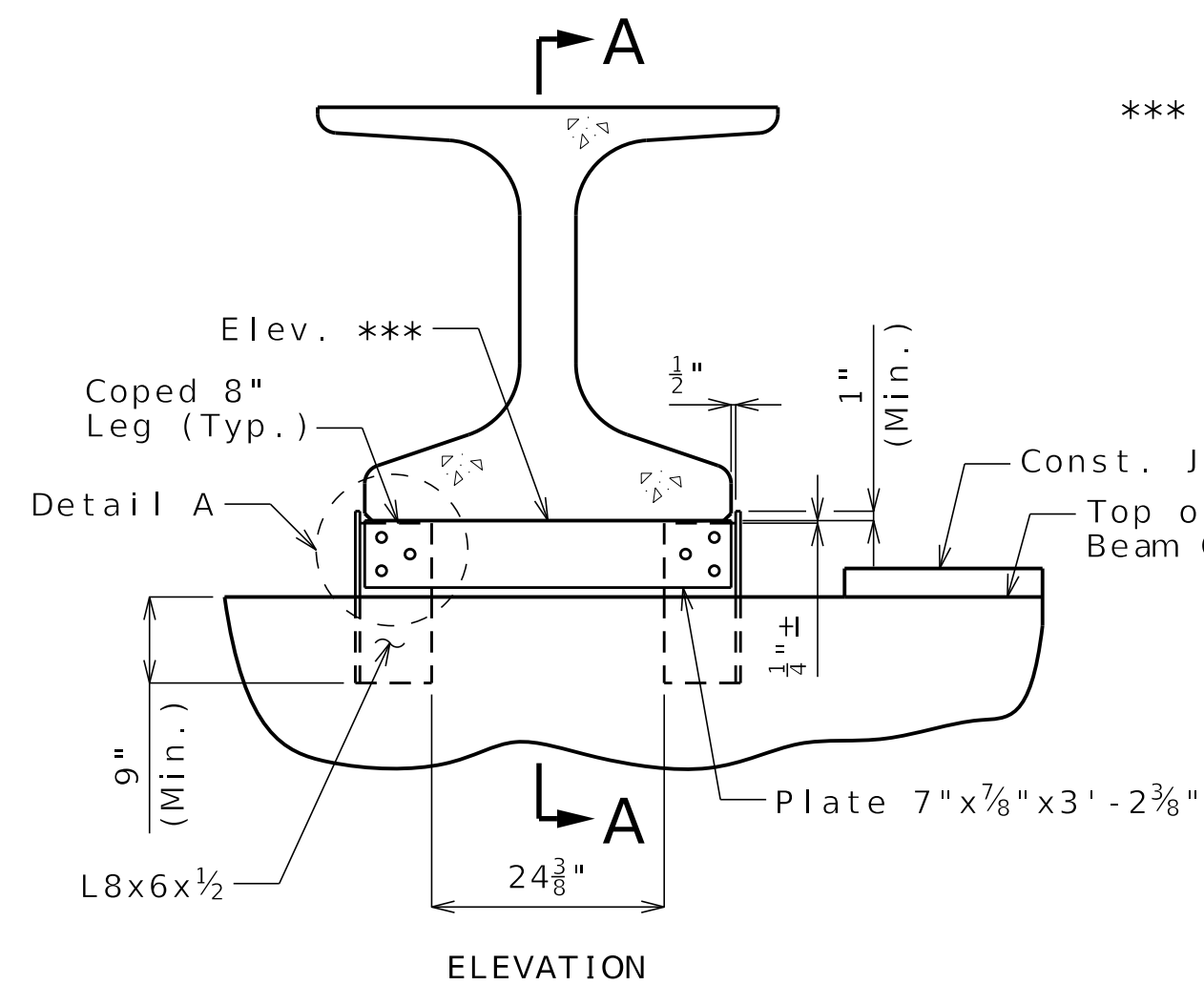
WWW.BARTLETTWEST.COM



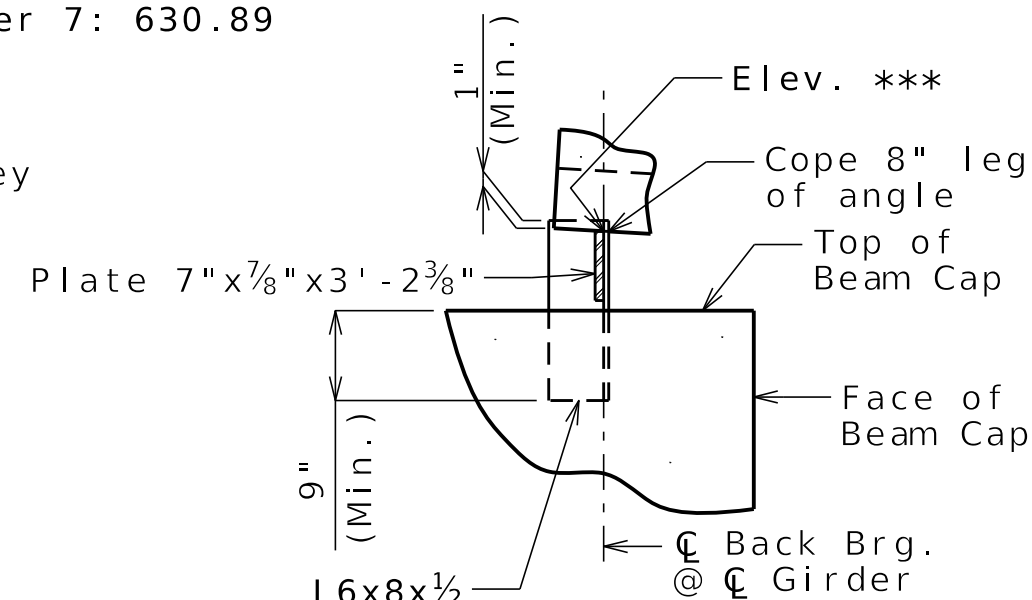
PLAN SHOWING REINFORCEMENT



PLAN OF BEAM

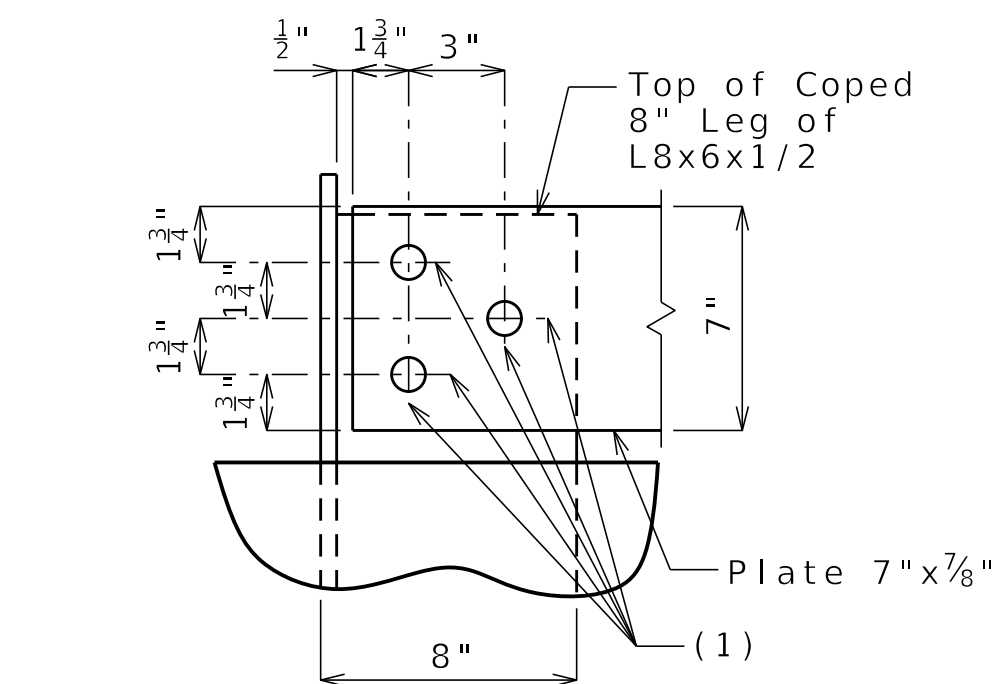


```
*** Girder 1: 626.11
    Girder 2: 626.92
    Girder 3: 627.72
    Girder 4: 628.52
    Girder 5: 629.32
    Girder 6: 630.10
    Girder 7: 630.89
```



SECTION A-A

GIRDER CHAIR DETAILS



(1) $\varnothing$ 1 1/16" $\varnothing$ Hole in angle (Field drilled) & plate for 1" $\varnothing$ ASTM F3125 Grade A325 Type 1 bolts

DETAIL A

General Notes:

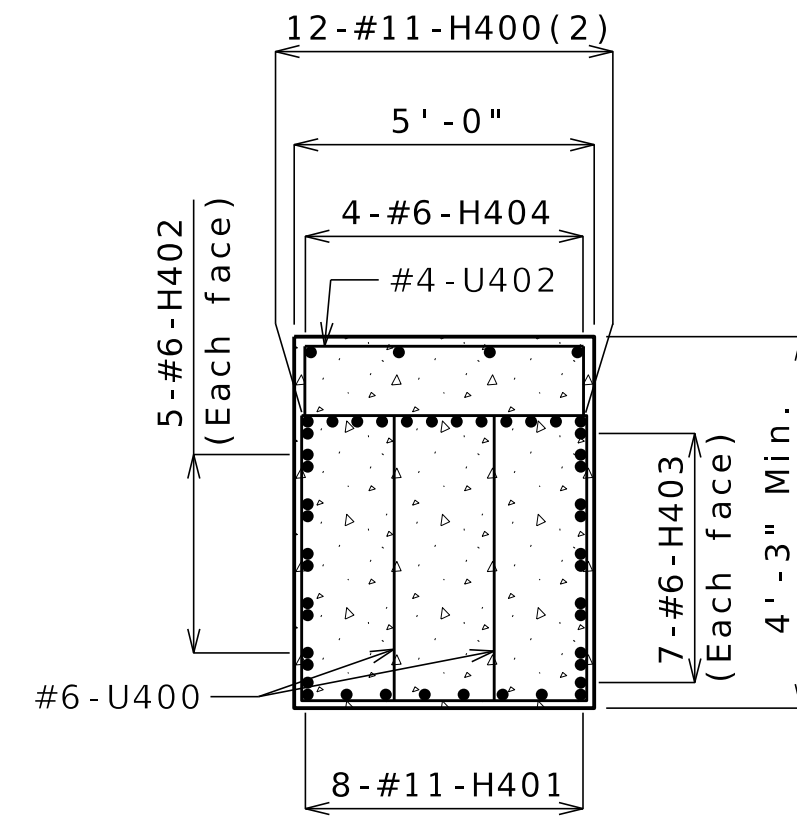
Work this sheet with Sheets No. 15 and 17.

For steps 2 inches or more, use 2 1/4x 1/2-inch joint filler up vertical face.

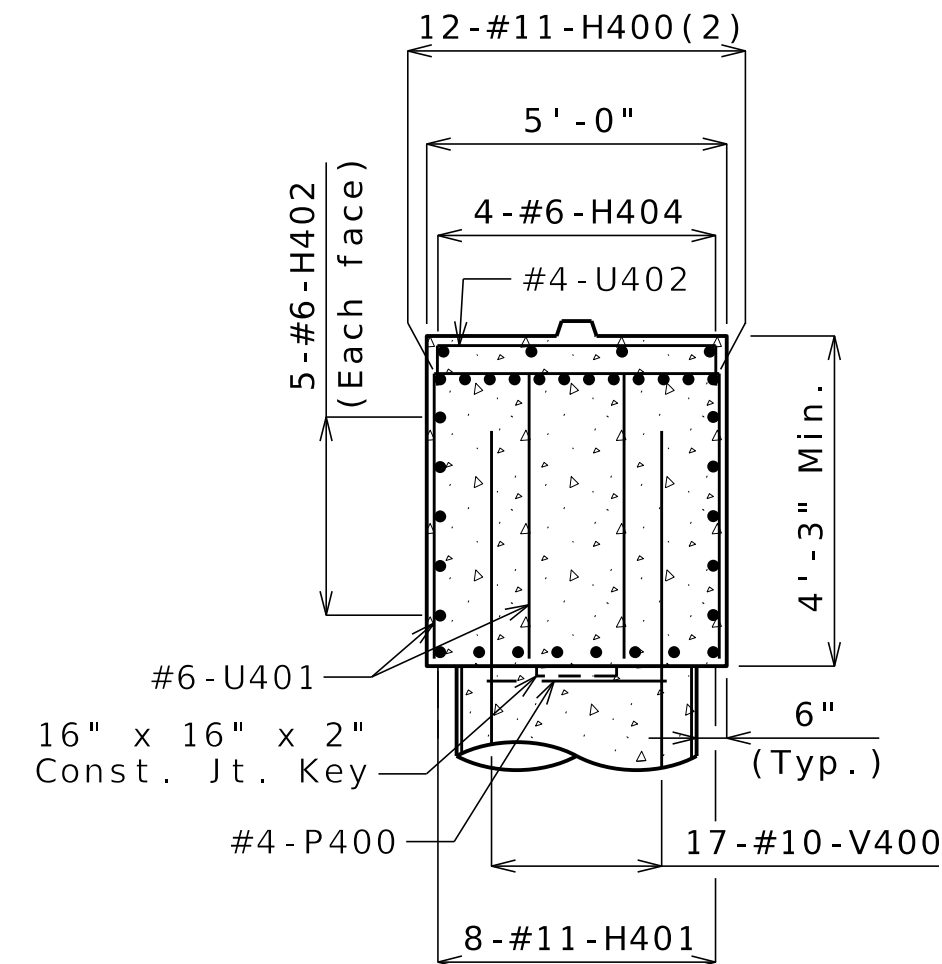
Structural steel for the girder beam chairs shall be coated with not less than 2 mills of inorganic zinc primer. Scratched or damaged surfaces are to be touched up in the field before concrete is poured. In lieu of coating, the girder beam chairs may be galvanized in accordance with ASTM A123.

Steel for chairs shall be ASTM A709 Grade 36.

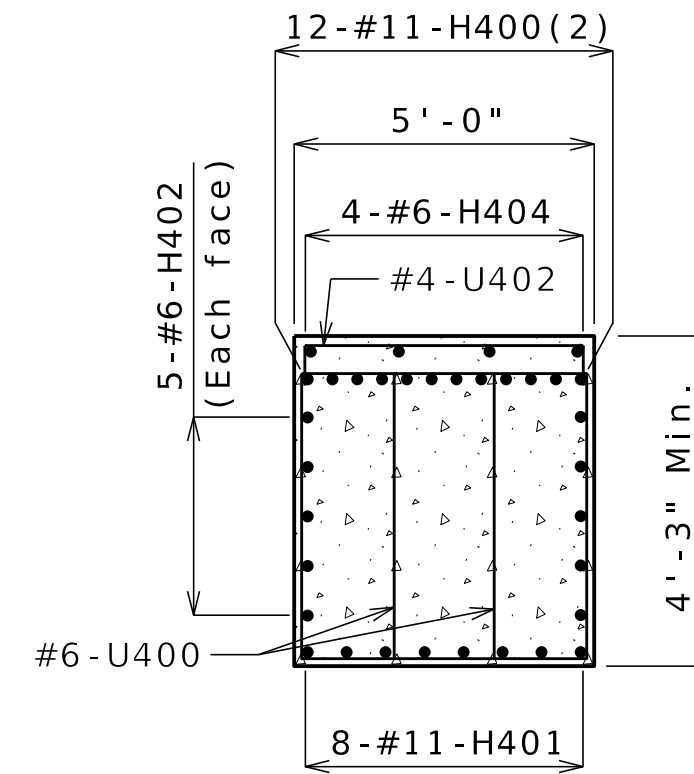
Reinforcing steel shall be shifted to clear chairs.



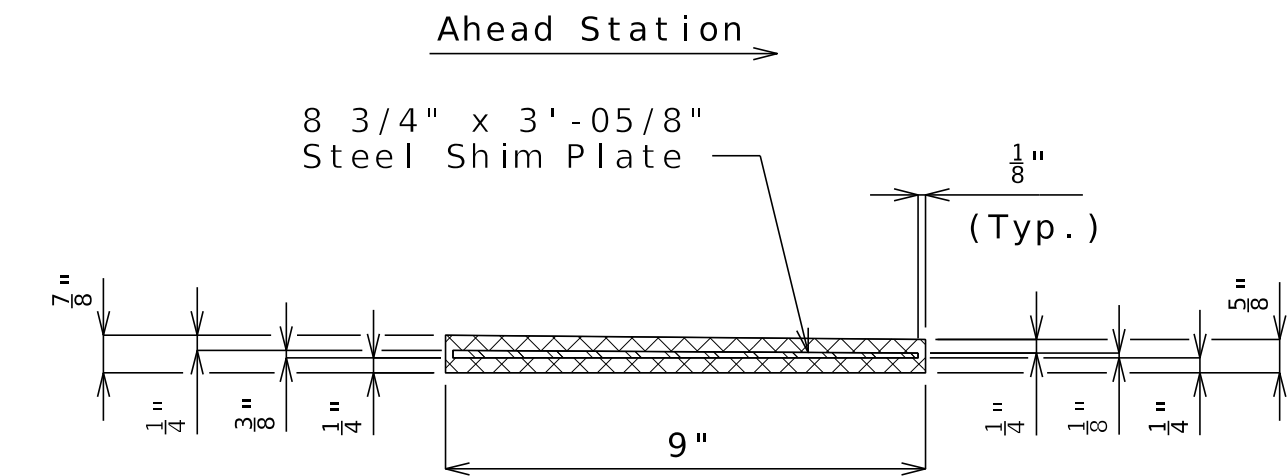
SECTION A-A



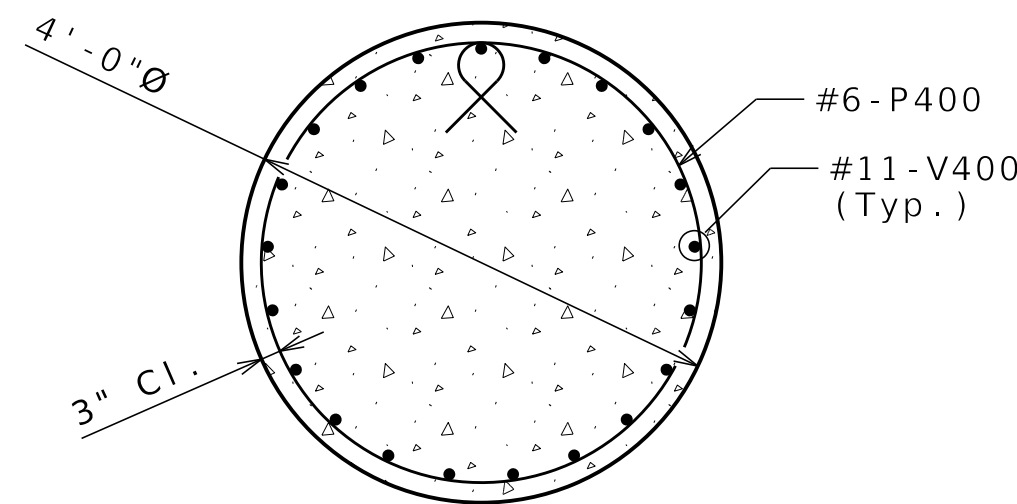
SECTION B-B



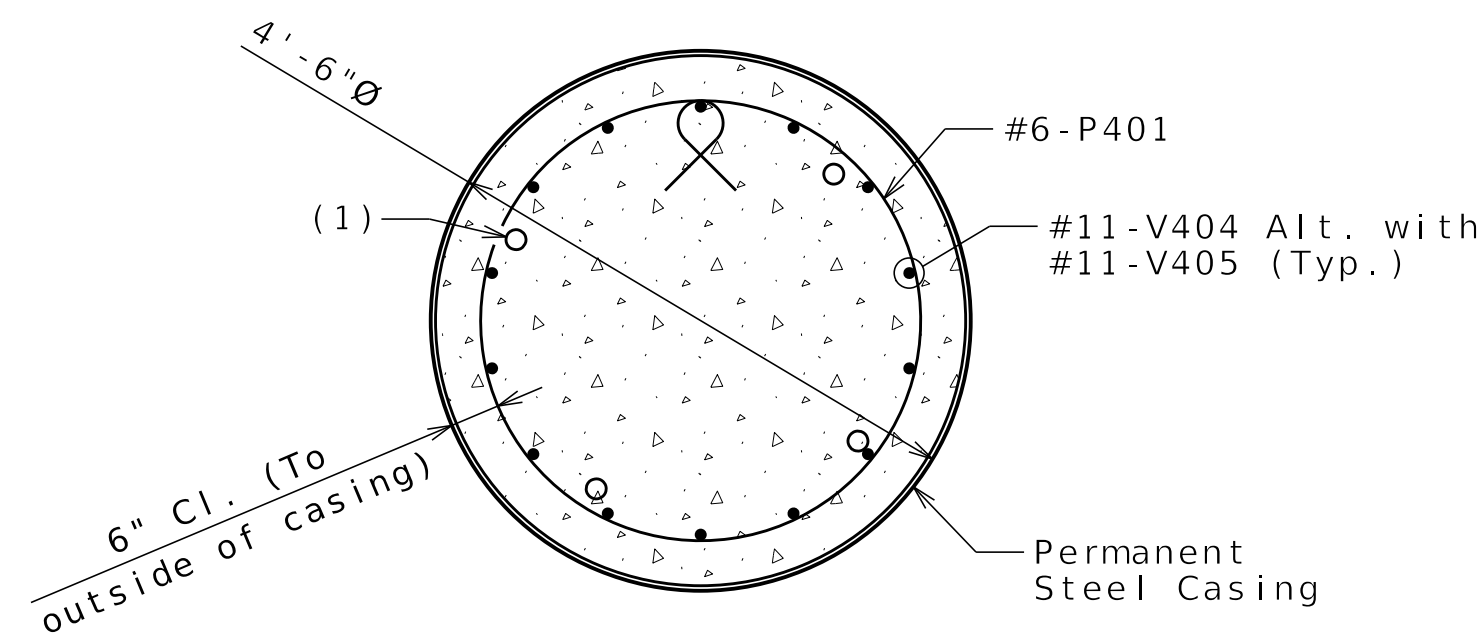
SECTION C-C



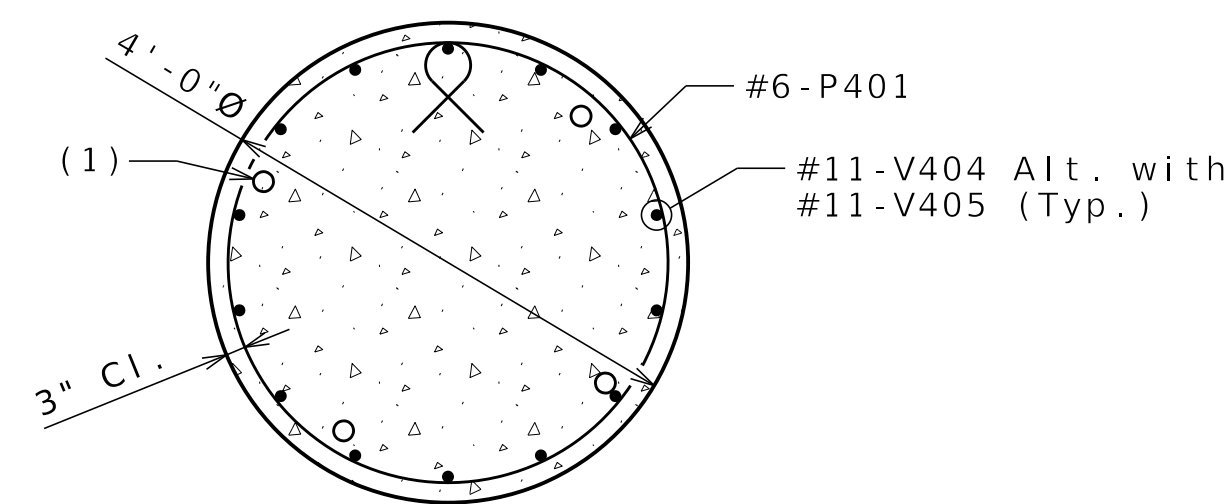
SECTION THRU LAMINATED
NEOPRENE BEARING PAD (TAPERED)
(7 Required)



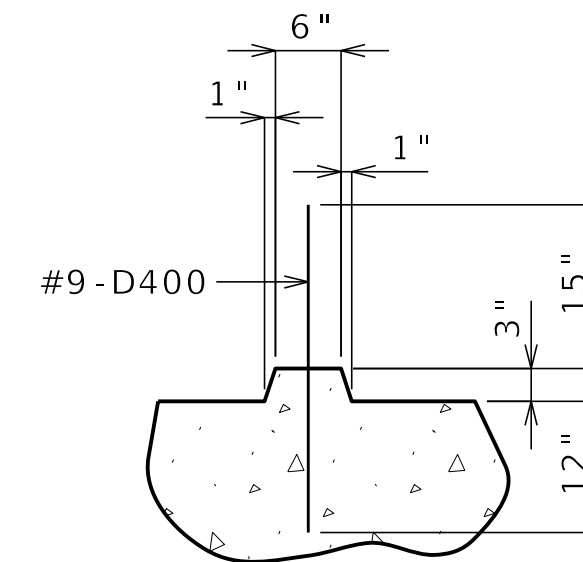
SECTION D-D



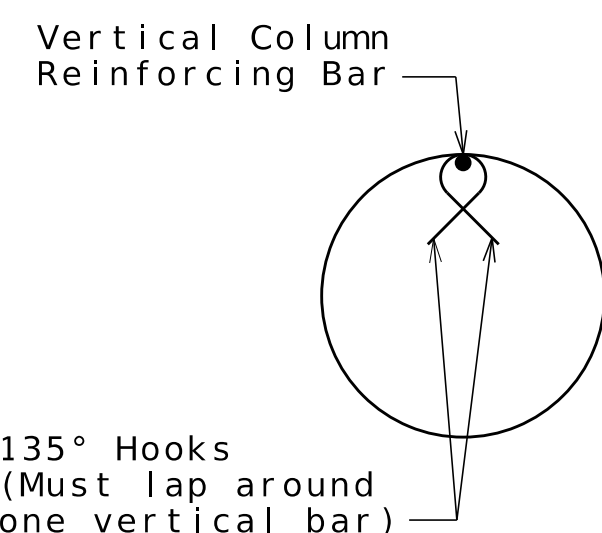
SECTION E - E



SECTION F - F



SECTION THRU KEY



SEISMIC STIRRUP BAR
(#6-P400 & P401)

Note: Seismic stirrups hooks shall be staggered around the column at about one-third of the hoop circumference.

- (1) 2"Ø Steel Pipe for sonic logging testing (4 each shaft)
- (2) Adjust bars as necessary to clear girder chair's embedded L-shape. Maintain 2 1/2" clear spacing between bars. Bundle bars horizontally if necessary.

General Notes:

Work this sheet with Sheets No. 15 and 16.



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED

/ 9 / 2025

US - 61	MO
---------	----

DISTRICT	SHEET NO.
BB	12555-17

BR	A9682-17
COUNTY	

ST. CHARLES

JOB NO.

JST0020

CONTRACT ID:

PROJECT NO.

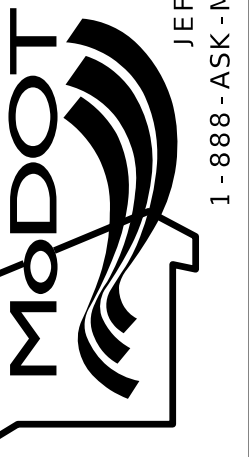
BRIDGE NO.
A9682

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MISSOURI HIGHWAY AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



HNTB
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270

Bartlett & West
601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
CERTIFICATE OF AUTHORITY NO. 000167 - ENGINEERING
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```
*      1 Space @ 12"
**     2 Spaces @ 6"
***    3 Spaces @ 6"
```

General Notes:

Work this sheet with Sheets No. 19 and 20.

All U bars and pairs of V bars shall be placed parallel to skewed steps.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.



DATE PREPARED
6/9/2025

ROUTE US - 61	STATE MO
DISTRICT BR	SHEET NO A9682 - 1

COUNTY
ST. CHARLES

JOB NO.
JST0020

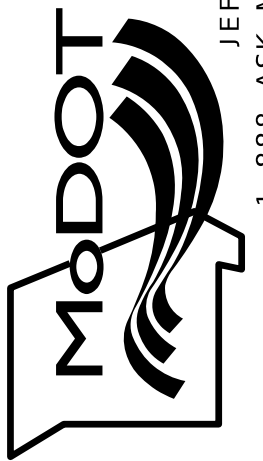
CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

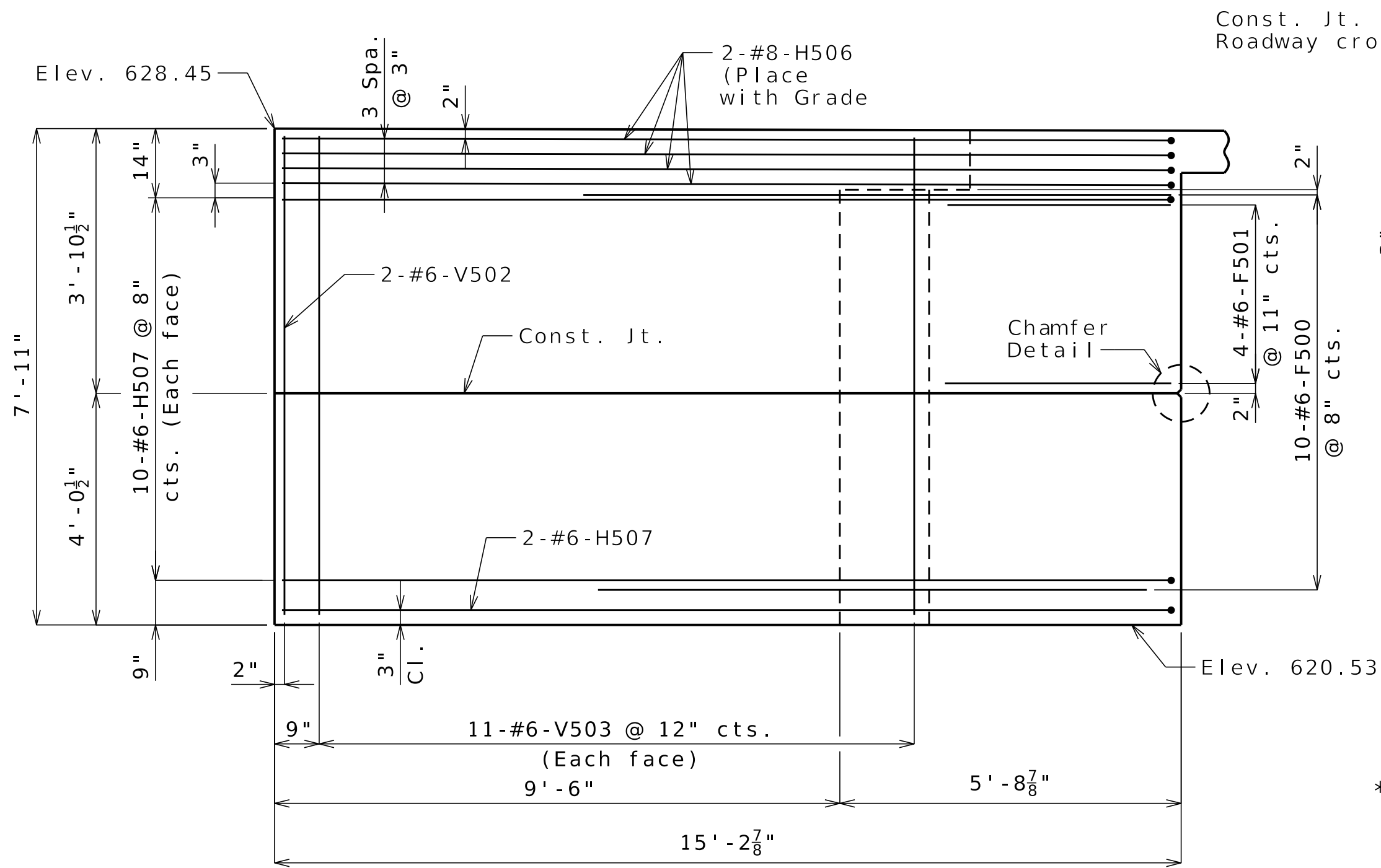
MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

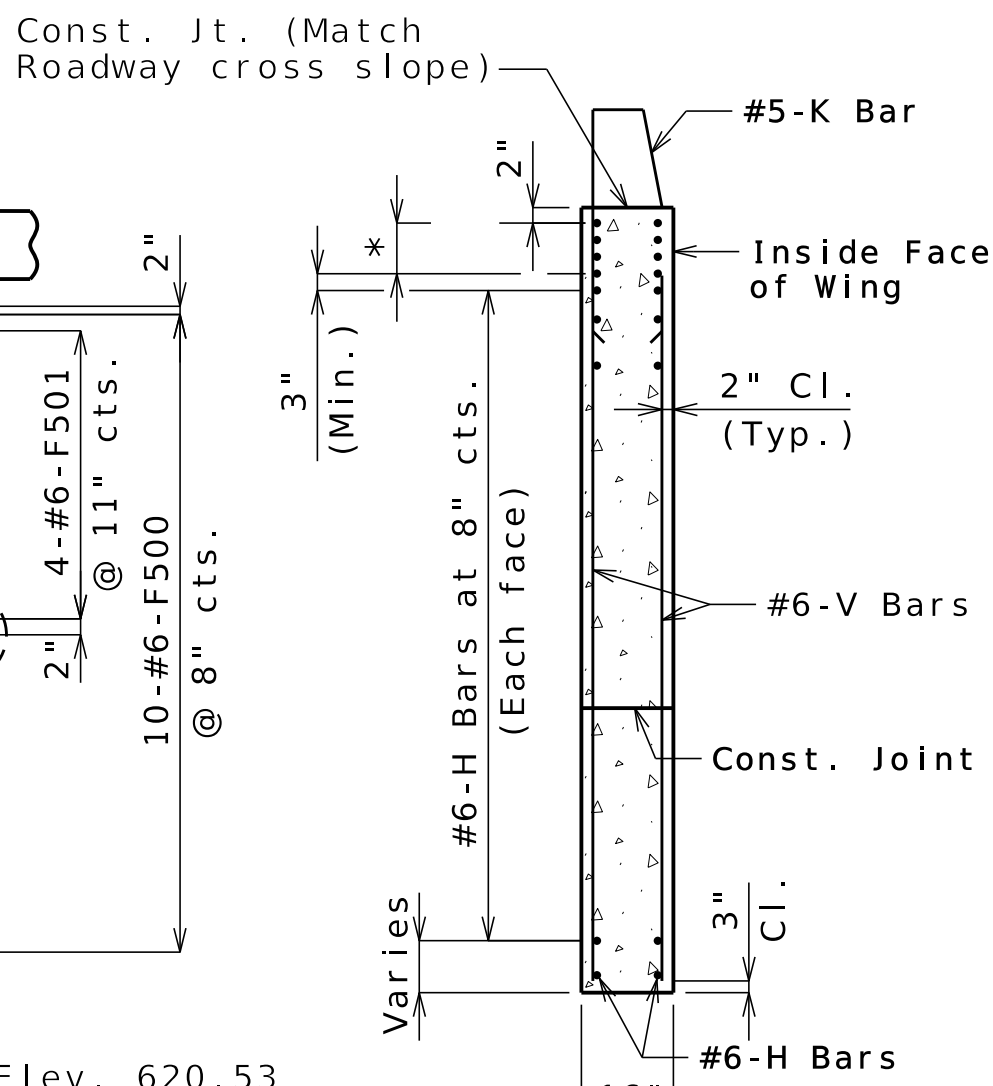


HNTB
715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270

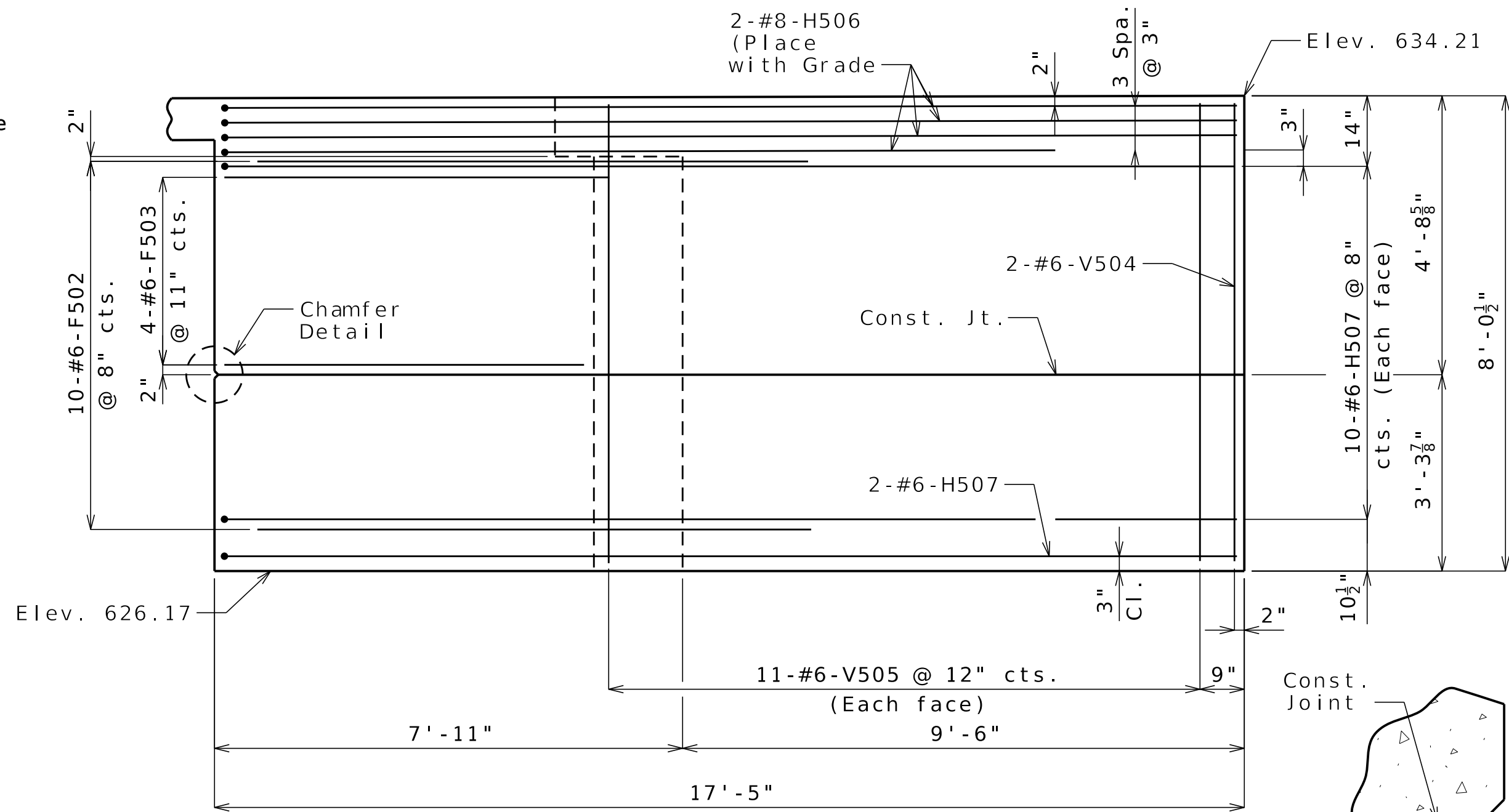
Bartlett & West
601 MONROE ST., SUITE 200 • SEPPERSON CITY, MO 65101
TEL: 636-722-7272 • FAX: 636-722-7273
CERTIFICATE OF AUTHORITY NO. 000167 • ENGINEERING
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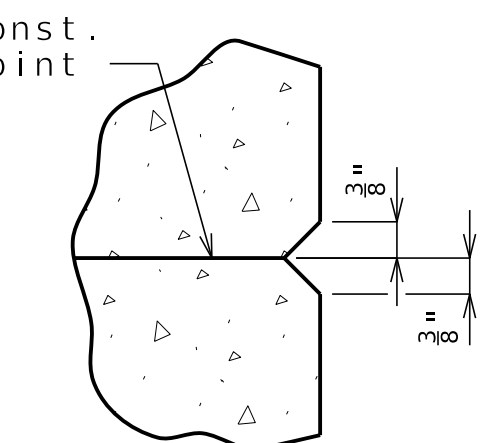
ELEVATION E-E



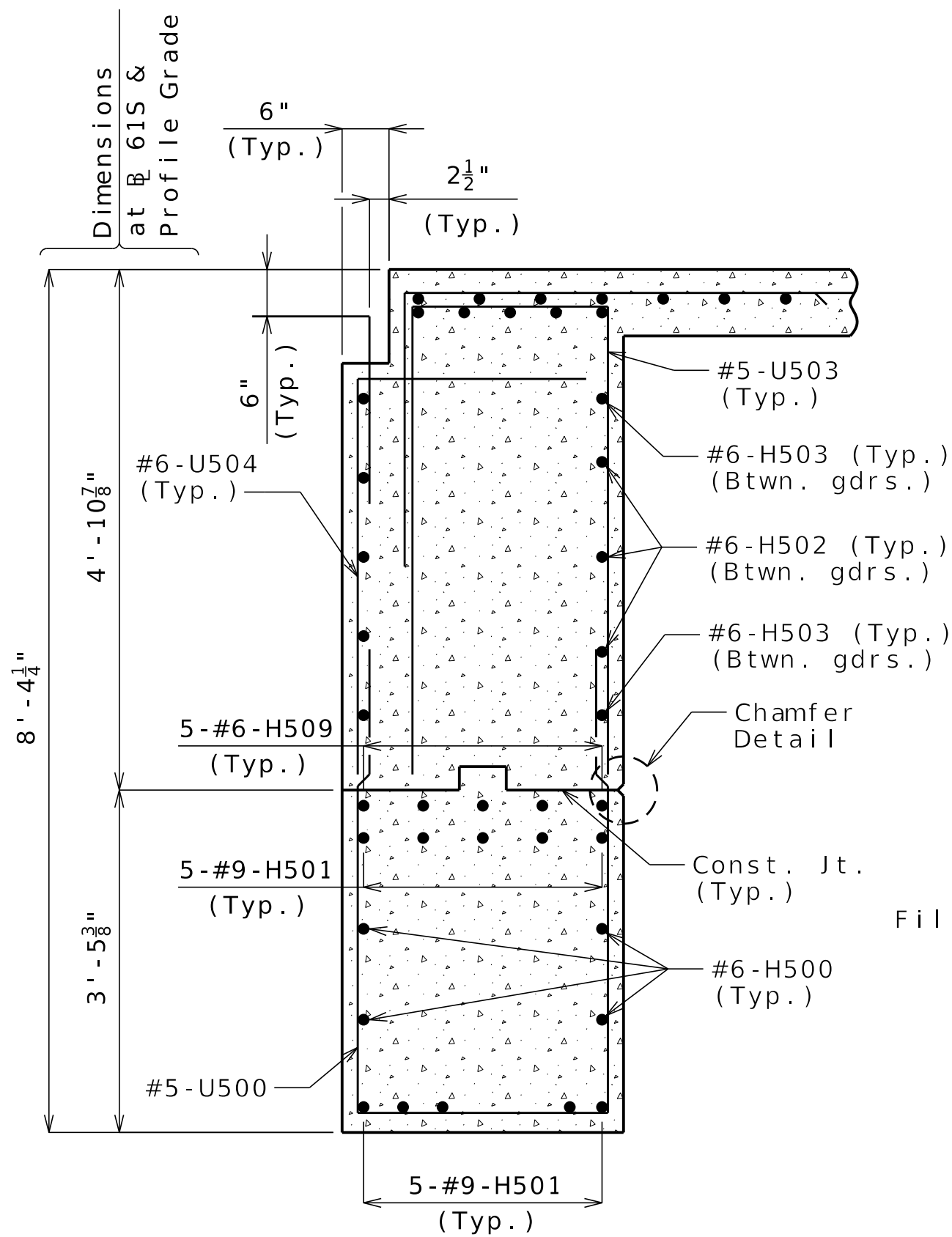
TYPICAL SECTION
THRU WING
* #8-H Bars at 3" cts.
(Each face)(Place with grade)



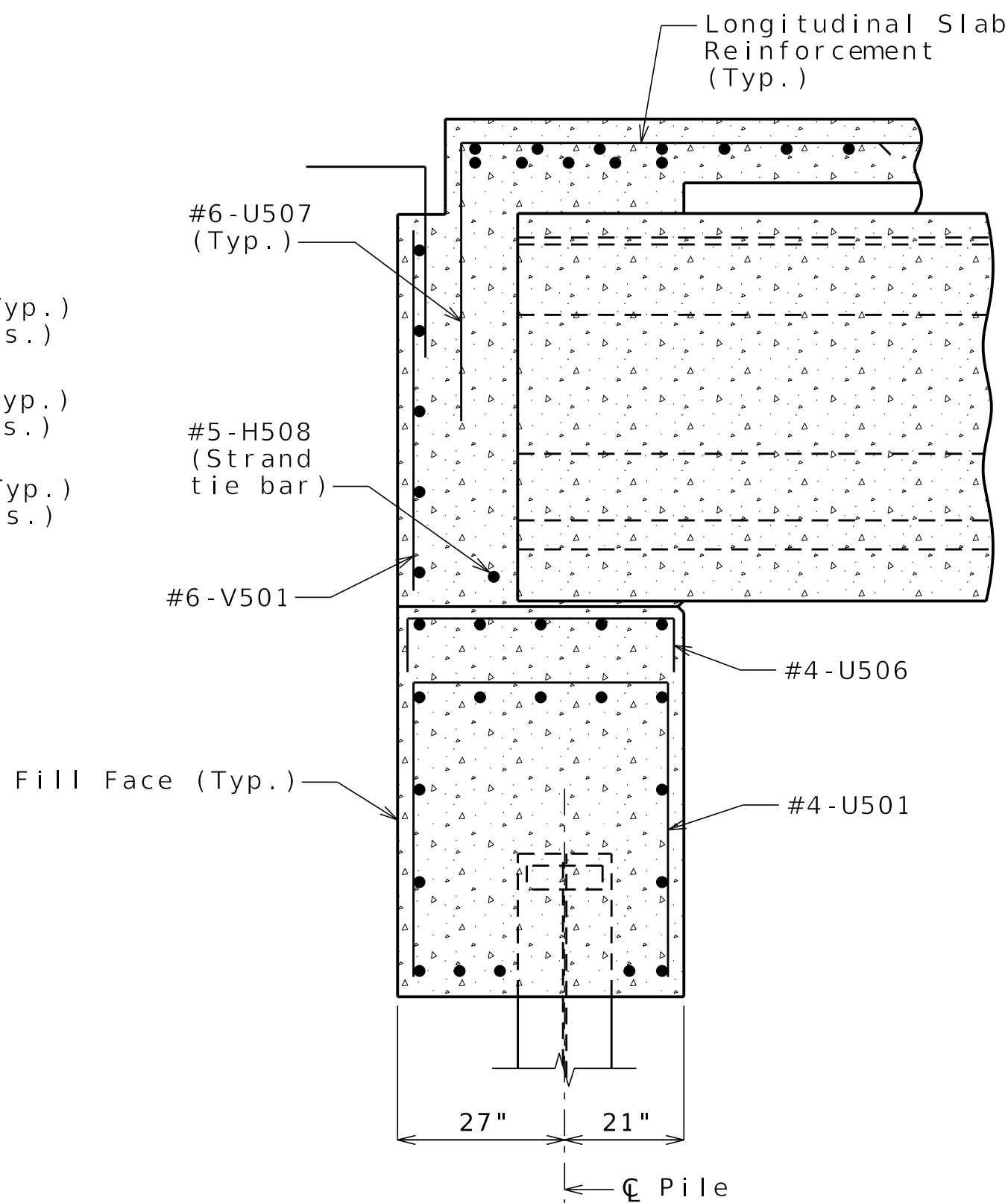
ELEVATION F-F



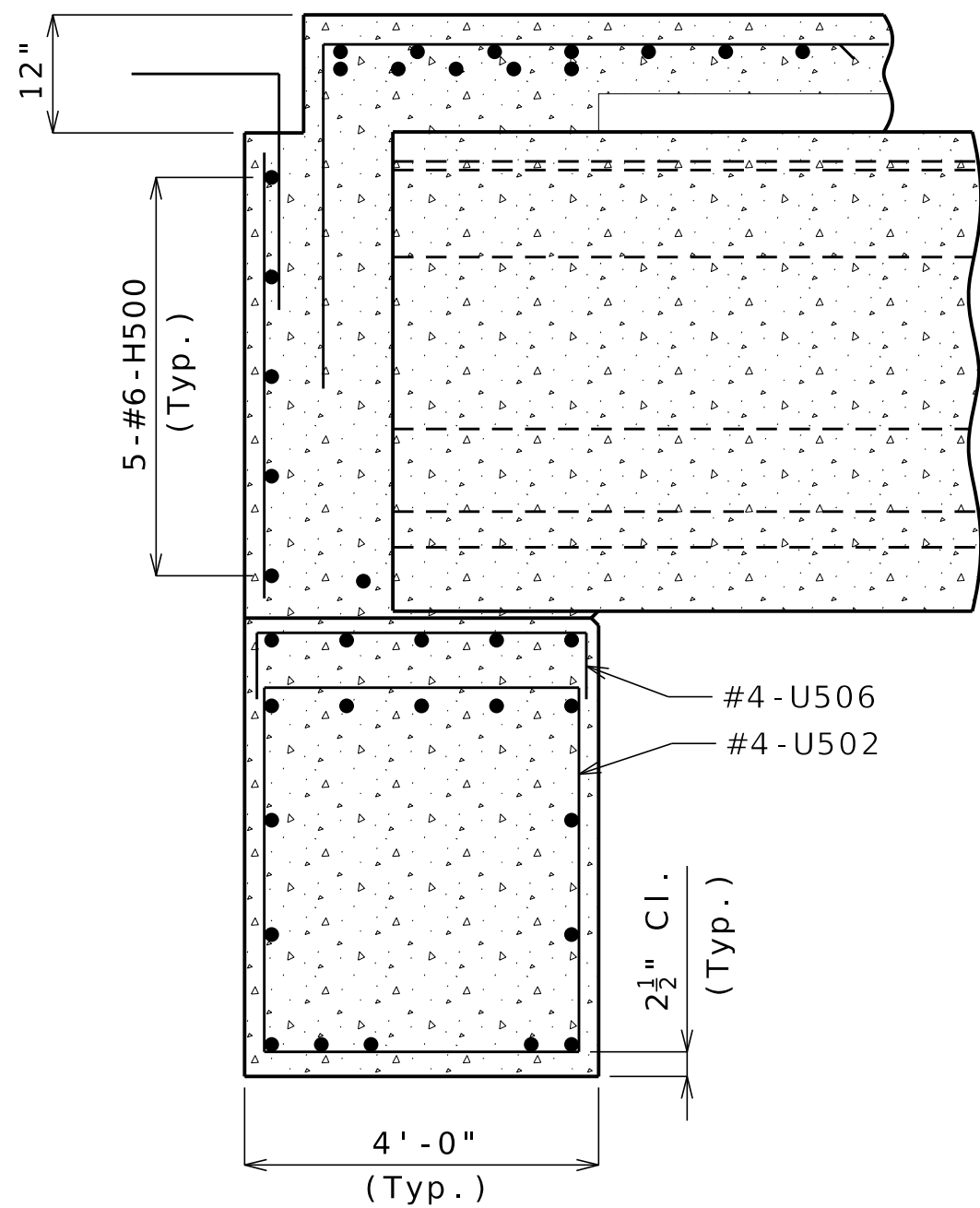
CHAMFER DETAIL



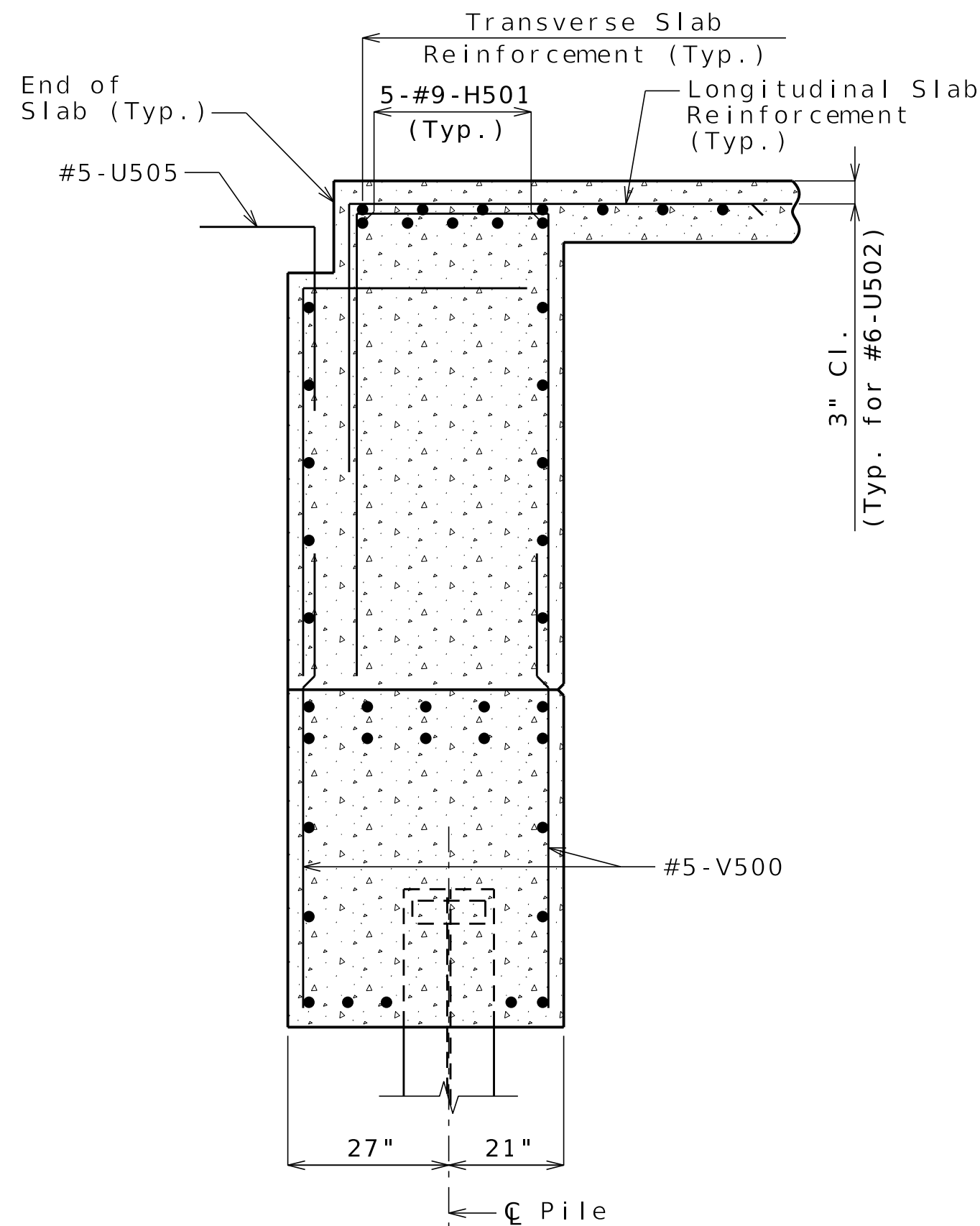
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

General Notes:
Work this sheet with Sheets No. 18 and 19.
For reinforcement of the barrier, see
Sheet No. 33.

DETAILS OF END BENT NO. 5



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
6/9/2025

ROUTE
US-61

STATE
MO

DISTRICT
BR

SHEET NO.
A9682-20

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

MoDOT

IMPROVE70
ALLIANCE

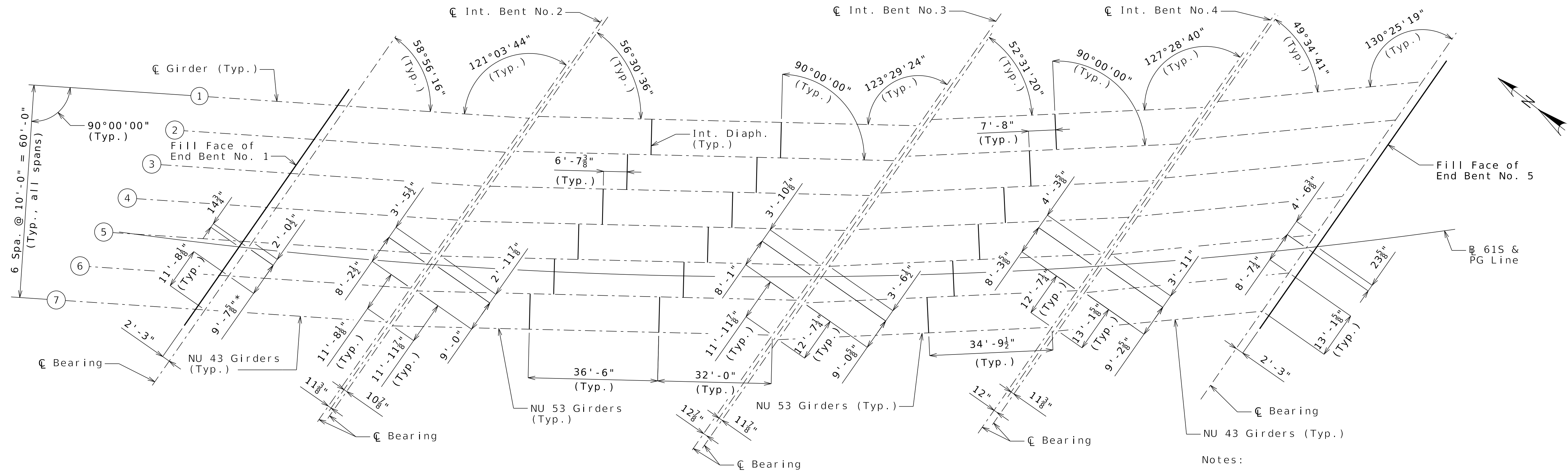
HNTB

Bartlett & West

715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
PHONE 874-33181
FAX 874-33181
WWW.BARTLETTWEST.COM

CERTIFICATE OF AUTHORITY
NO. 001270

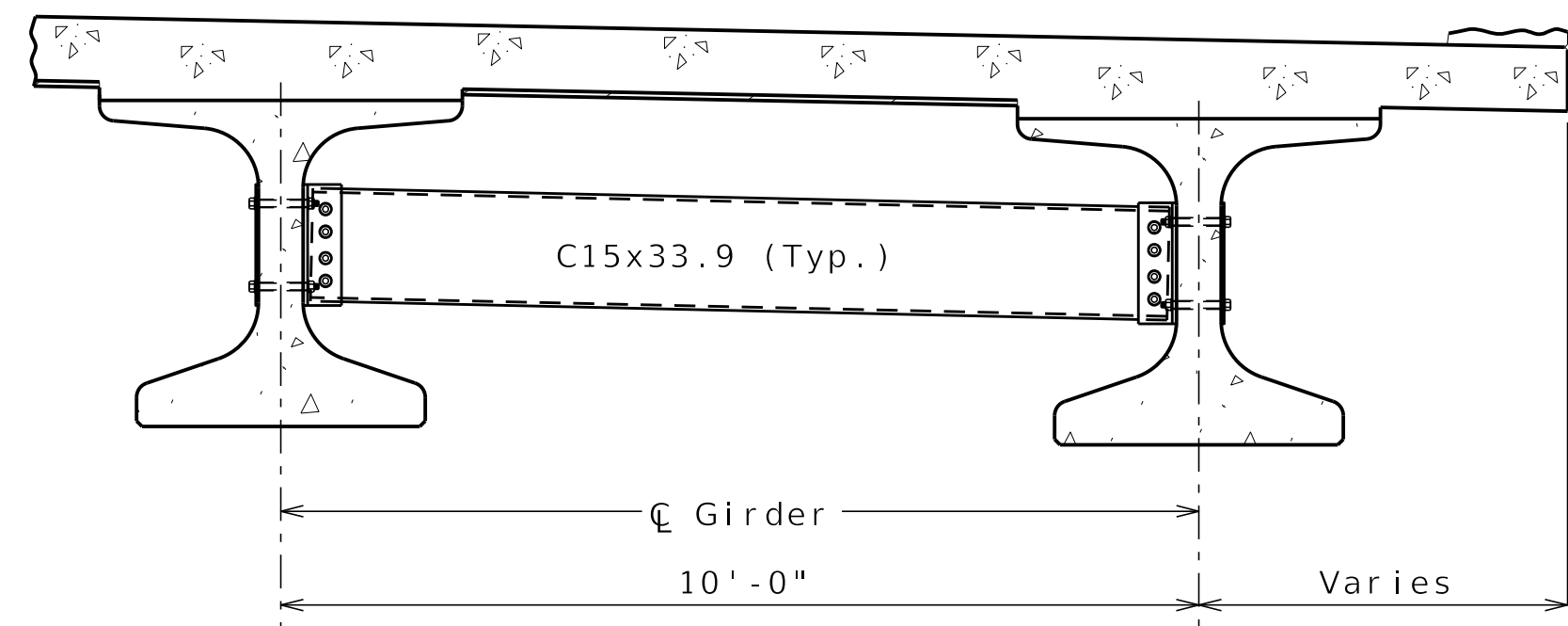
ENGINEERING



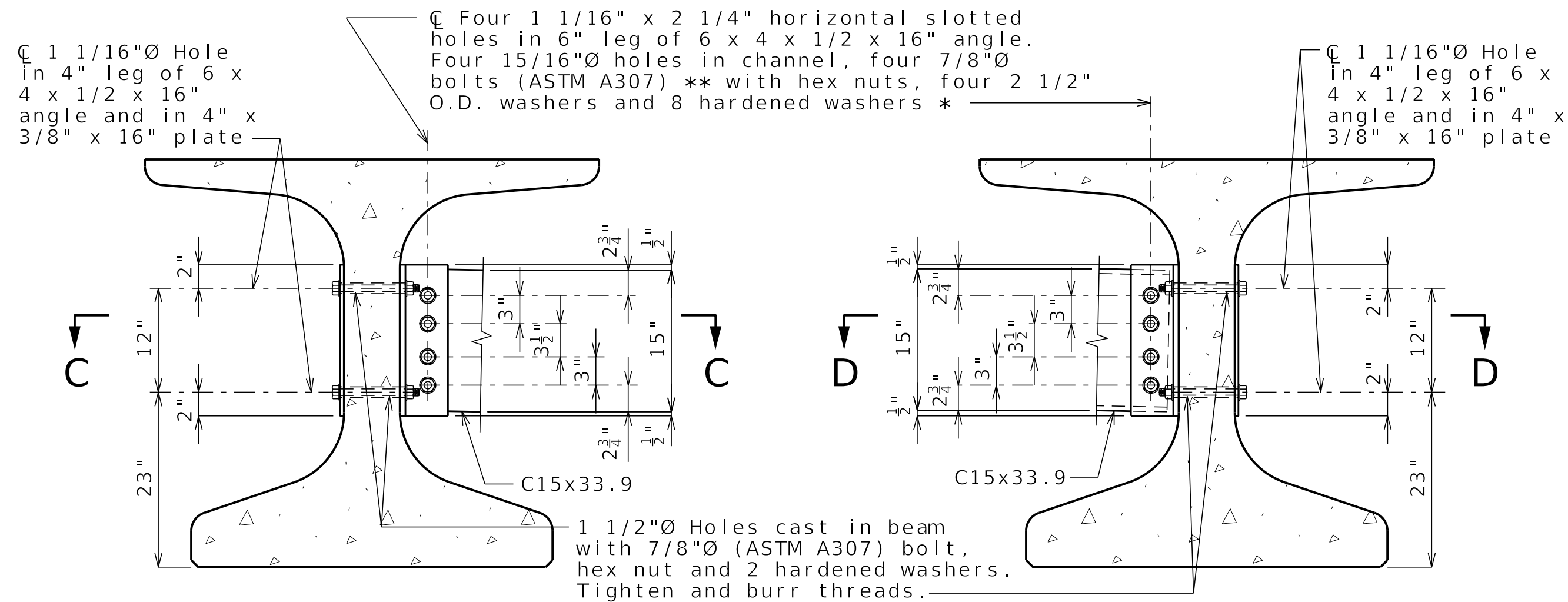
PLAN SHOWING LOCATION OF STEEL INTERMEDIATE DIAPHRAGMS
Longitudinal dimensions are horizontal.

Notes:

- All dimensions are horizontal.
- All bents are parallel.

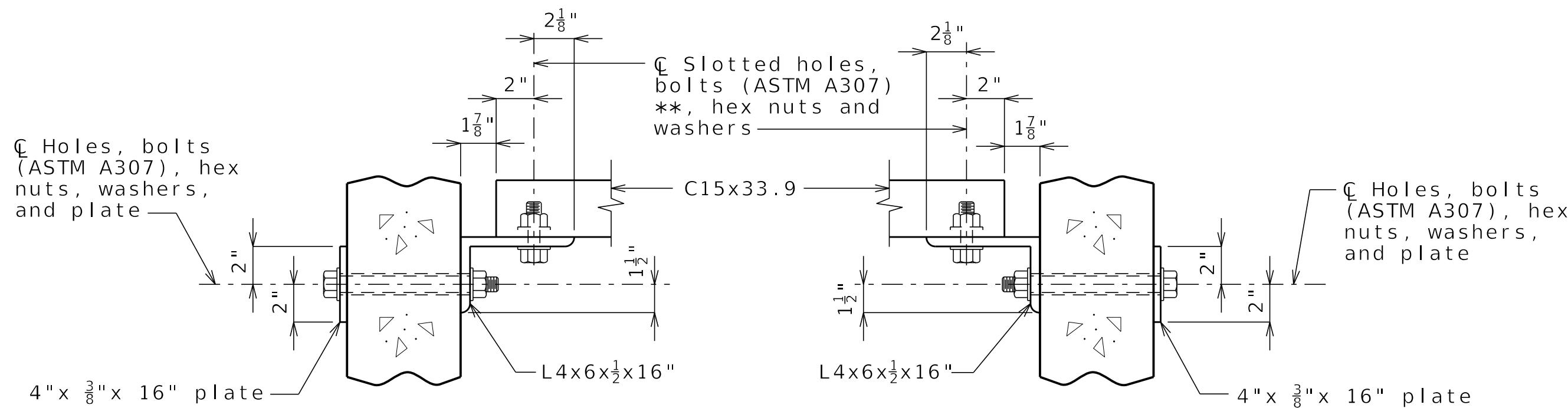


PART SECTION SHOWING
INTERMEDIATE DIAPHRAGMS



SECTION THRU INT. GIRDER
AT DIAPHRAGM

SECTION THRU EXT. GIRDER
AT DIAPHRAGM



SECTION C-C

SECTION D-D

STEEL DIAPHRAGM NOTES:

* In lieu of 2 1/2" outside diameter washers, contractor may substitute a 3/16" (Min. thickness) plate with four 15/16" Ø holes and one hardened washer per bolt.

** Bolts shall be tightened to provide a tension of one-half that specified in Sec 712 for high strength bolt installation. ASTM F3125 Grade A325 Type 1 bolts may be substituted for and installed in accordance with the requirements for the specified ASTM A307 bolts.

All diaphragm materials including bolts, nuts, and washers shall be galvanized.
Fabricated structural steel shall be ASTM A709 Grade 36 except as noted.

Shop drawings will not be required for steel intermediate diaphragms and angle connections.

(1) Fabricator shall apply a bond breaker to this region.

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about C Girder. May be moved laterally in pairs.



- + Indicates prestressing strand.
- o Indicates cut & shop bend with 3'-0" projection.



The image shows two elevation drawings of window frames, labeled WWR5 and WWR6.

WWR5 (Left): A horizontal window frame. The top section is labeled "D20 @ 6\"". The bottom section is labeled "W8 (Typ.)". The frame has a total width of $3' - 10\frac{1}{4}''$. The width is divided into segments: 6\" from the left edge, 6\" to the first mullion, 20\" between the two mullions, 6\" to the second mullion, and 6\" from the second mullion to the right edge.

WWR6 (Right): A vertical window frame. The left side is labeled "D31 @ 2\"". The right side is labeled "W12". The frame has a total height of $2' - 7\frac{1}{2}''$. The height is divided into segments: 5\" from the top edge, 8\frac{1}{2}\" to the first mullion, 2\" between the two mullions, and 5\" from the second mullion to the bottom edge. The mullions are labeled "d=2\frac{1}{2}\"". The bottom section is labeled "WWR6".



Strands not shown
for clarity.



Strands not shown
for clarity.



BEARING PLATE



Exclude coil tie at exterior face of exterior girders except at integral end bents.

[illegible][illegible]

Cut top 2 rows of strands with a 12" projection and bend in shop (Cut any remaining top strands within 1" of end of girder) (Typ.)

3'-0" (Typ.)

6"

6" (Typ.)

3" (Min.)

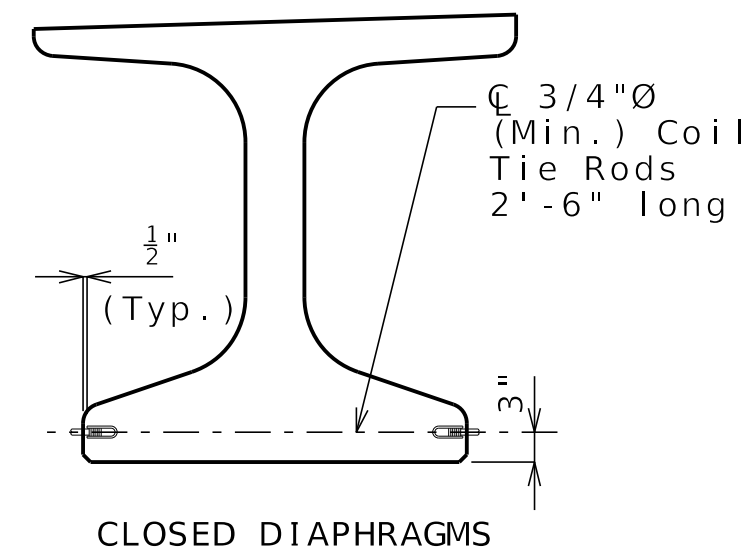
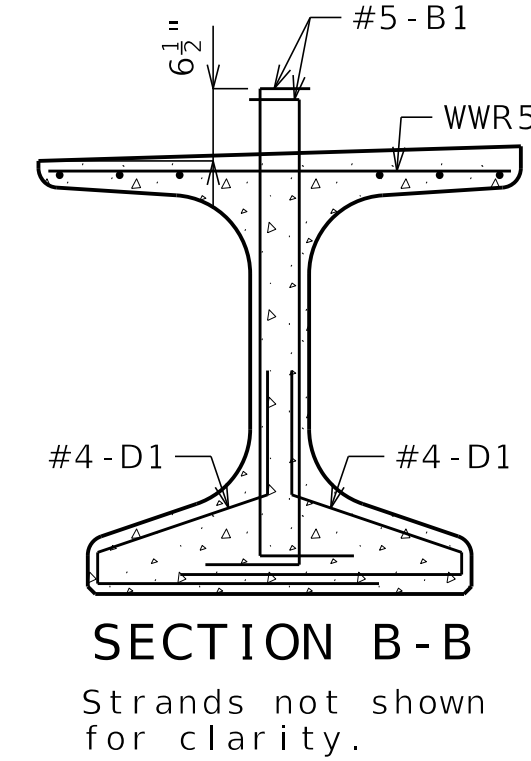
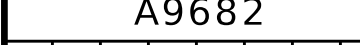
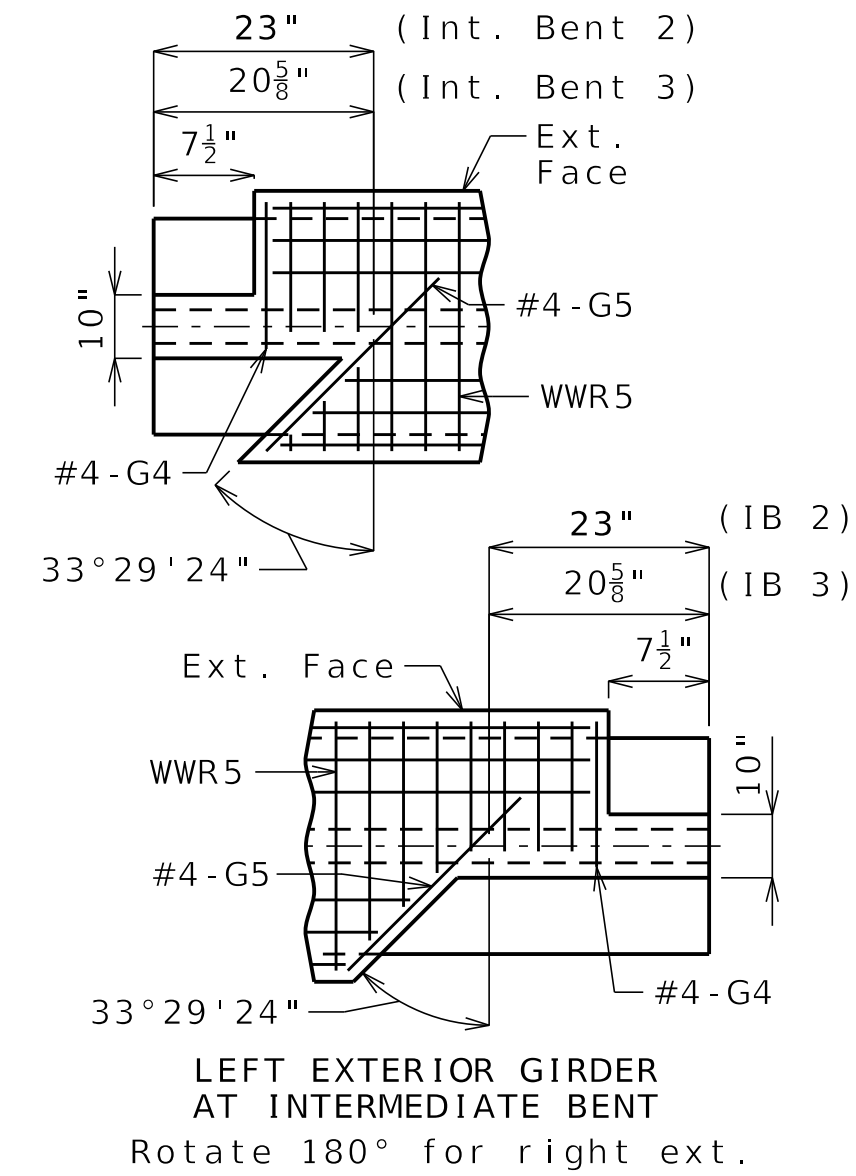
#5 Strand Tie Bar (Normal to girders) (Typ.)

Cut & shop bend with 3'-0" projection (Cut any remaining bottom strands within 1" of girder end) (Typ.)

INTERMEDIATE BENT

INTERMEDIATE BENT

STRANDS AT GIRDER ENDS



Technical drawing of a bearing plate showing end and side views with dimensions and material specifications.

END VIEW

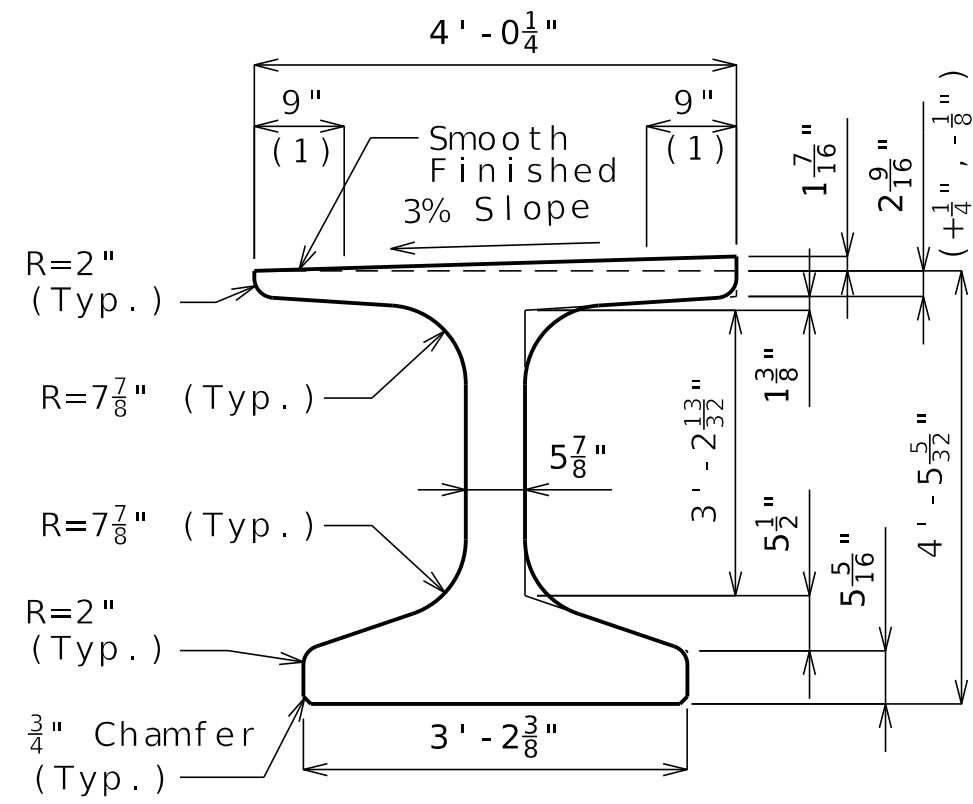
- Top dimensions: $\frac{3}{4}" \times \frac{3}{4}" \times 18"$ Chamfer Blockout (Typ.)
- Centerline: C Girder
- Material: $1/2"$ Bearing Plate (ASTM A709, Grade 36)
- Blockout width: $4"$
- Stud spacing: $8"$ (three segments)
- Total width: $3' - 0\frac{7}{8}"$
- Bottom centerline: $\text{C Two Welded Studs } (1/2" \times 5")$

SIDE VIEW

- Top dimension: End of Girder
- Blockout width: $5"$
- Stud spacing: $8"$
- Blockout width: $5"$
- Total width: $18"$
- Bottom centerline: $\text{C Four Welded Studs } (1/2" \times 5")$

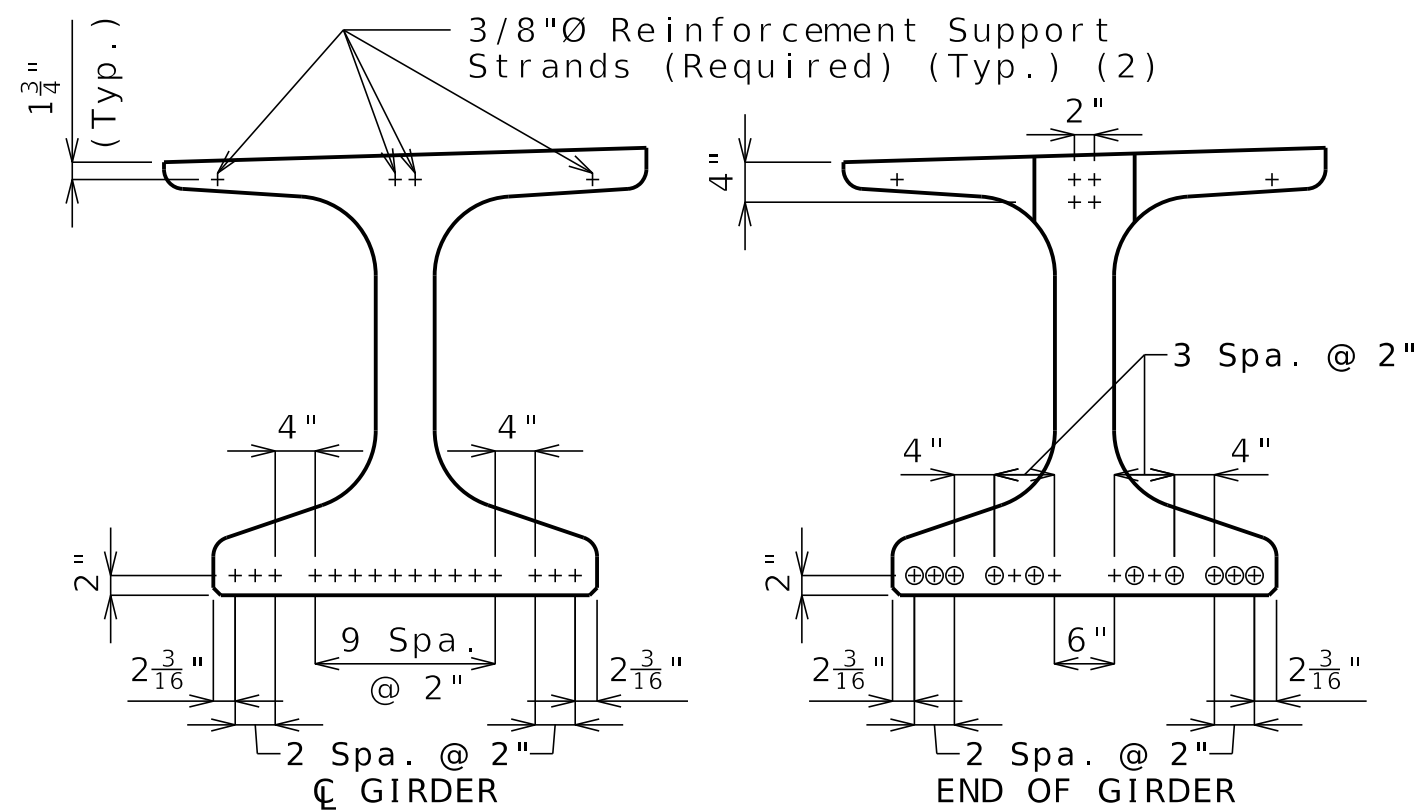
BEARING PLATE

(1) Fabricator shall apply a bond breaker to this region.



DIMENSIONS

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about $\varnothing$ Girder. May be moved laterally in pairs.



STRAND ARRANGEMENT

+ Indicates prestressing strand.

o Indicates cut & shop bend with 3'-0" projection.

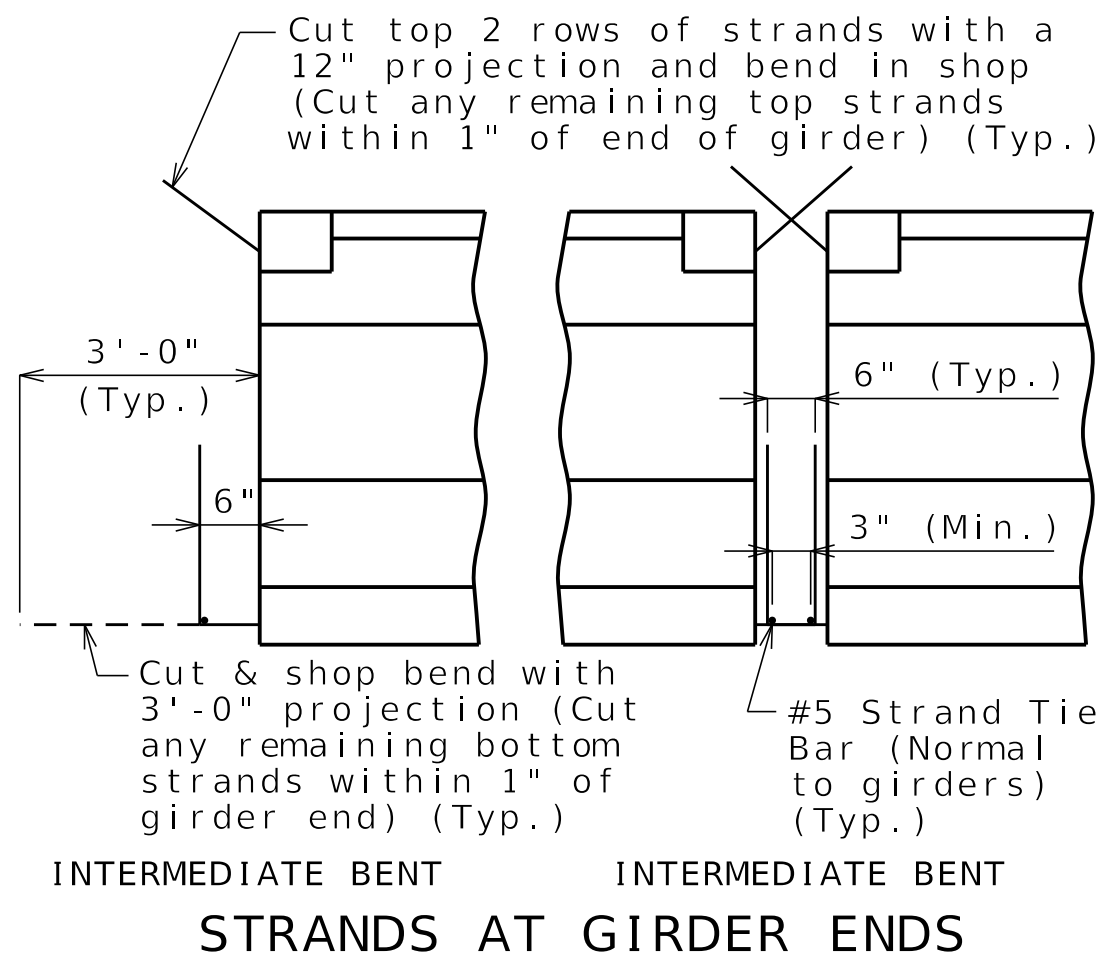
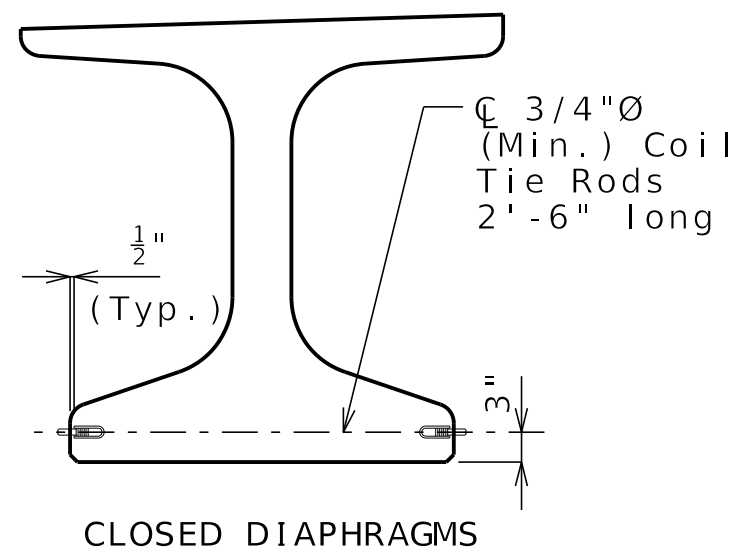
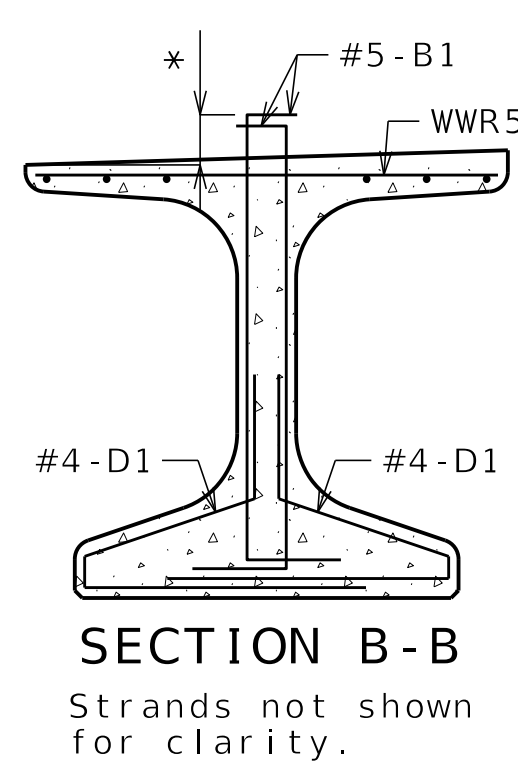
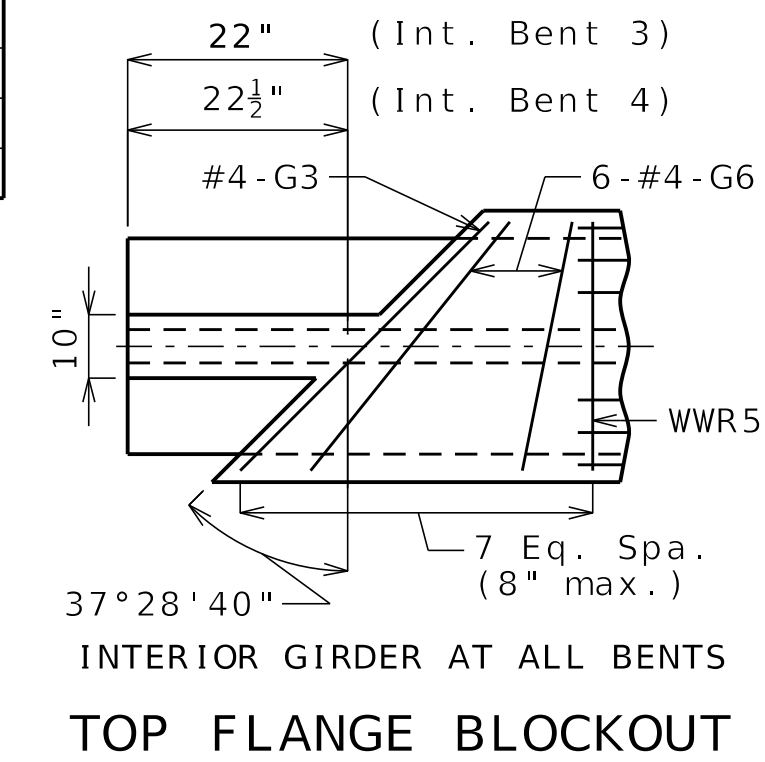
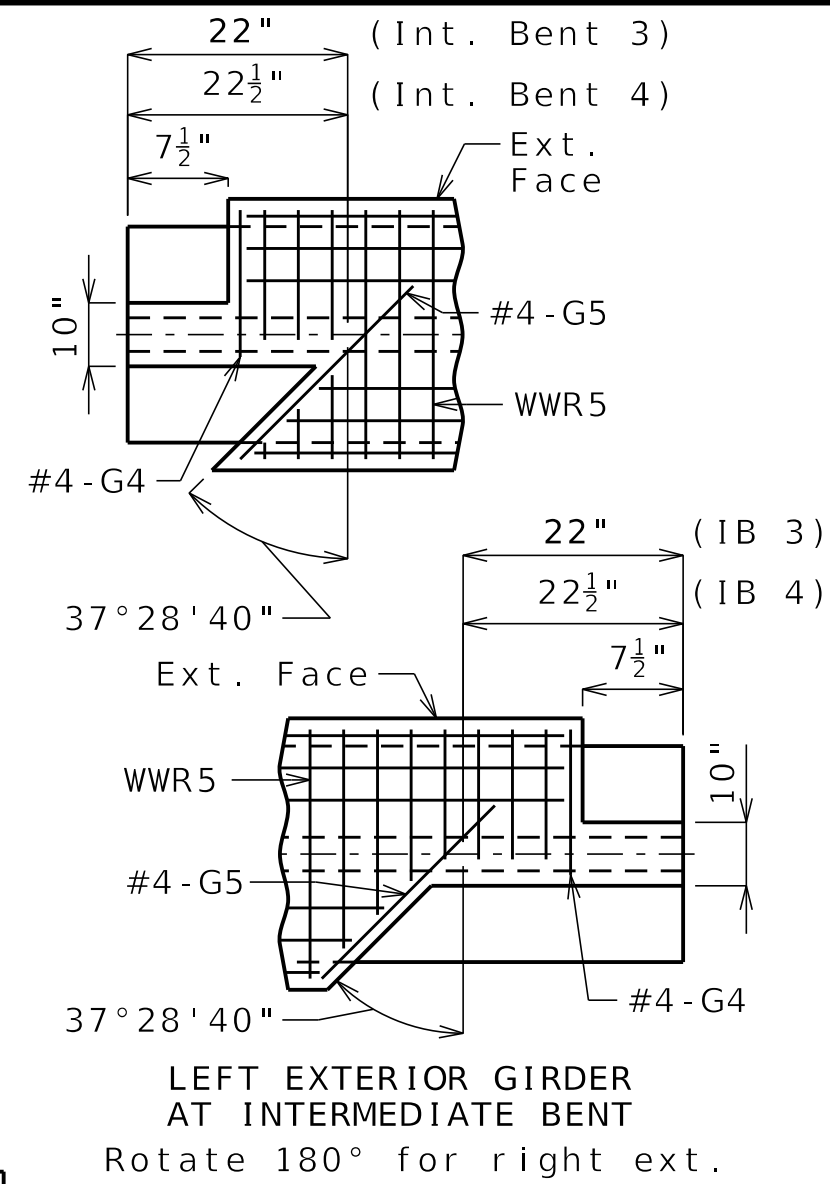


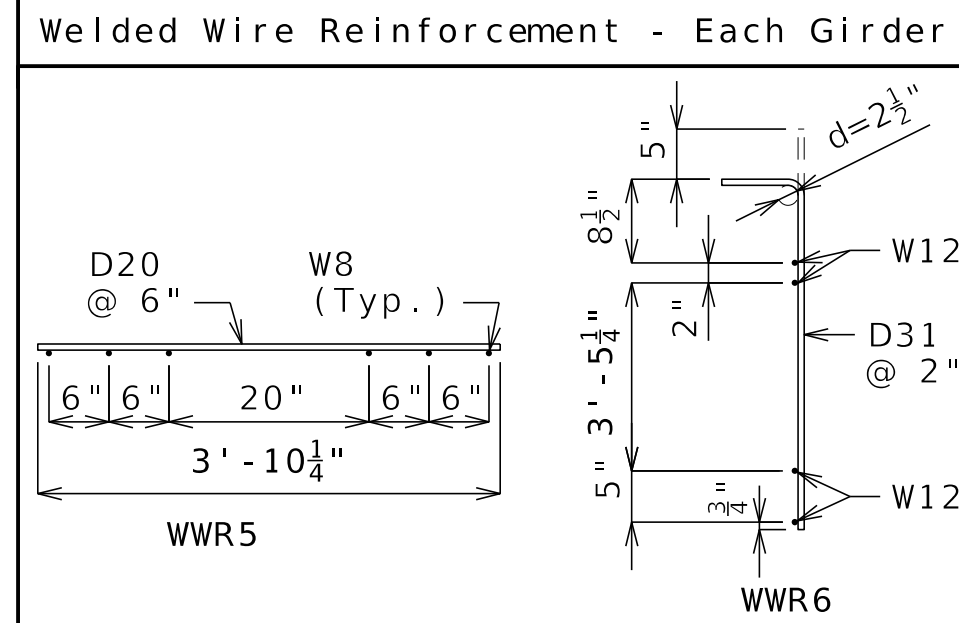
TABLE OF DIMENSIONS			
Girder No.	1-5	6 & 7	
*	6 $\frac{1}{4}$ "	7 $\frac{1}{2}$ "	
**	5'-11"	6'-0"	
***	4'-10 $\frac{3}{4}$ "	4'-11 $\frac{3}{4}$ "	



COIL TIES

Exclude coil tie at exterior face of exterior girders.

Bill of Reinforcing Steel - Each Girder					Bending Diagrams	
No.	Size/Mark	Length	Shape			
218	5 B1	**	11S		Shape 20	
238	4 D1	4'-0"	9S		Shape 95	
2	4 G3	4'-10"	20			
2	4 G4	2'-3"	20			
2	4 G5	3'-4"	20			
12	4 G6	Varies	20			



All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All bar reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

G4 and G5 not required for interior girders. G3 and G6 not required for exterior girders of intermediate spans.

General Notes:

Concrete for prestressed girders shall be Class A-1 with f'c = 8500 psi and f'ci = 6500 psi.

Use 16 strands, 0.6" $\varnothing$ Grade 270, with an initial prestress force of 703 kips.

Pretensioned members shall be in accordance with Sec 1029.

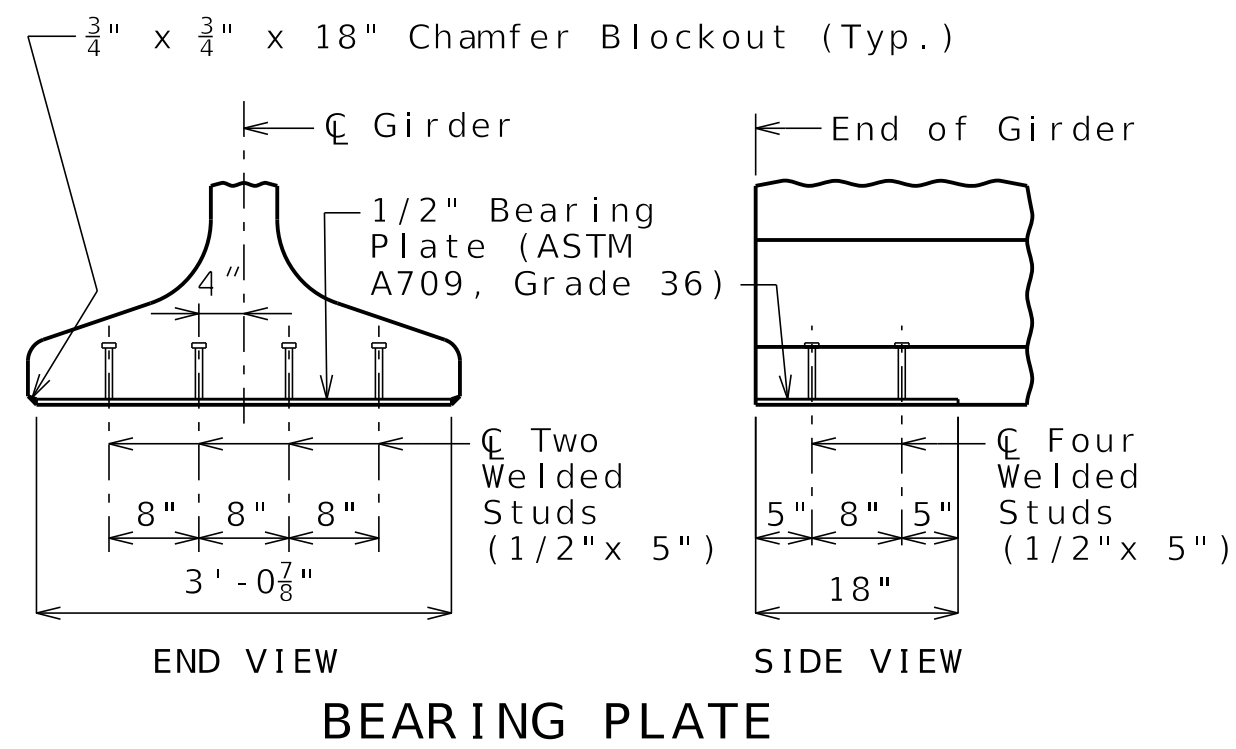
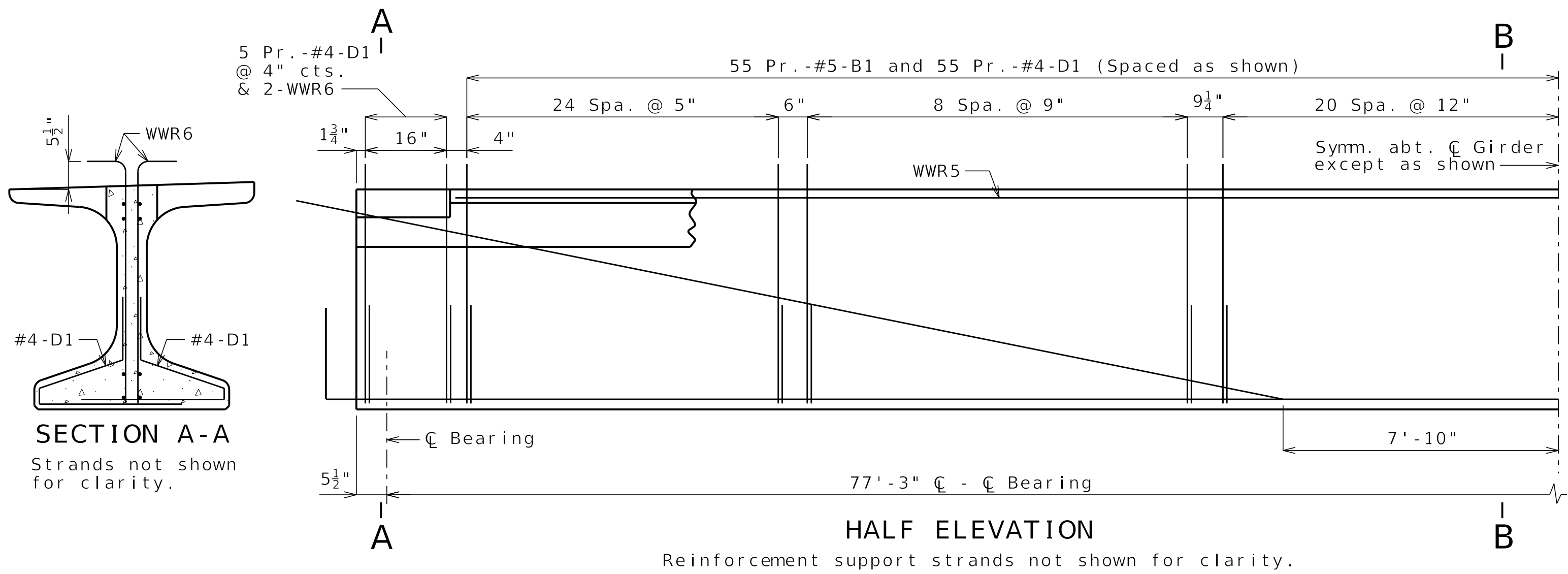
Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: coil ties, top flange blockout, application of bond breaker, and holes for steel intermediate diaphragms.

For Girder Camber Diagram, see Sheet No. 28.

The 1 1/2" $\varnothing$ holes shall be cast in the web for steel intermediate diaphragms. Drilling is not allowed. For location of holes and details of steel intermediate diaphragms, see Sheet No. 21.

For location of coil ties at concrete diaphragms, see Sheets No. 26 and 27.



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6/9/2025

ROUTE STATE

US-61 MO

DISTRICT SHEET NO.

BR A9682-24

COUNTY

ST. CHARLES

JOB NO.

JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL

JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

IMPROVE70 ALLIANCE

HNTB

715 KIRK DRIVE

KANSAS CITY, MO 64105-1310

CERTIFICATE OF AUTHORITY

NO. 001270

Bartlett & West

601 INDORE ST, SUITE 201, JEFFERSON CITY, MO 65101

PHONE 572-633-3181

CERTIFICATE OF AUTHORITY - ENGINEERING

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(1) Fabricator shall apply a bond breaker to this region.

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about C Girder. May be moved laterally in pairs.



- + Indicates prestressing strand.
- Indicates cut & shop bend with 3'-0" projection.



Bending Diagrams

Shape 20

16 $\frac{1}{4}$ "

4"

3 $\frac{1}{4}$ "

5 $\frac{1}{2}$ "

2' - 2"

4' - 0"

5"

9 $\frac{1}{2}$ "

Shape 9S

Shape 11S

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All bar reinforcement shall be Grade 60

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

G4 and G5 not required for interior girders. Half no. of G3, G4, G5 and G6 not required for ext. girders of end spans.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'_c = 8500$ psi and $f'_{ci} = 6500$ psi.

Use 14 strands, 0.6"Ø Grade 270, with an initial prestress force of 615 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: coil ties, top flange blockout, and application of bond breaker.

The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet
No. 28.

For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 19 and 26.



Strands not shown
for clarity.



Strands not shown
for clarity.



BEARING PLATE



Exclude coil tie at exterior face of exterior girders except at integral end bents.

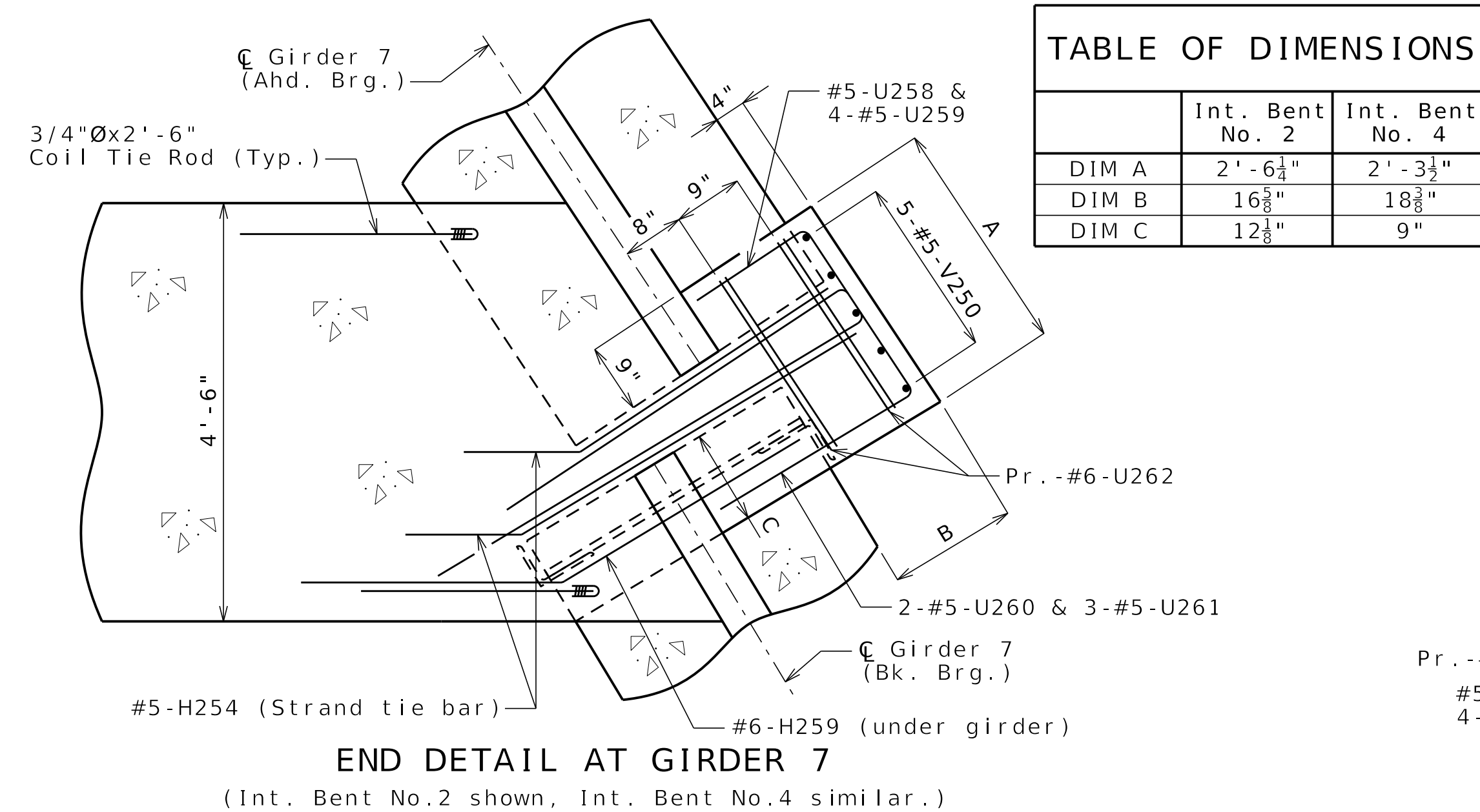
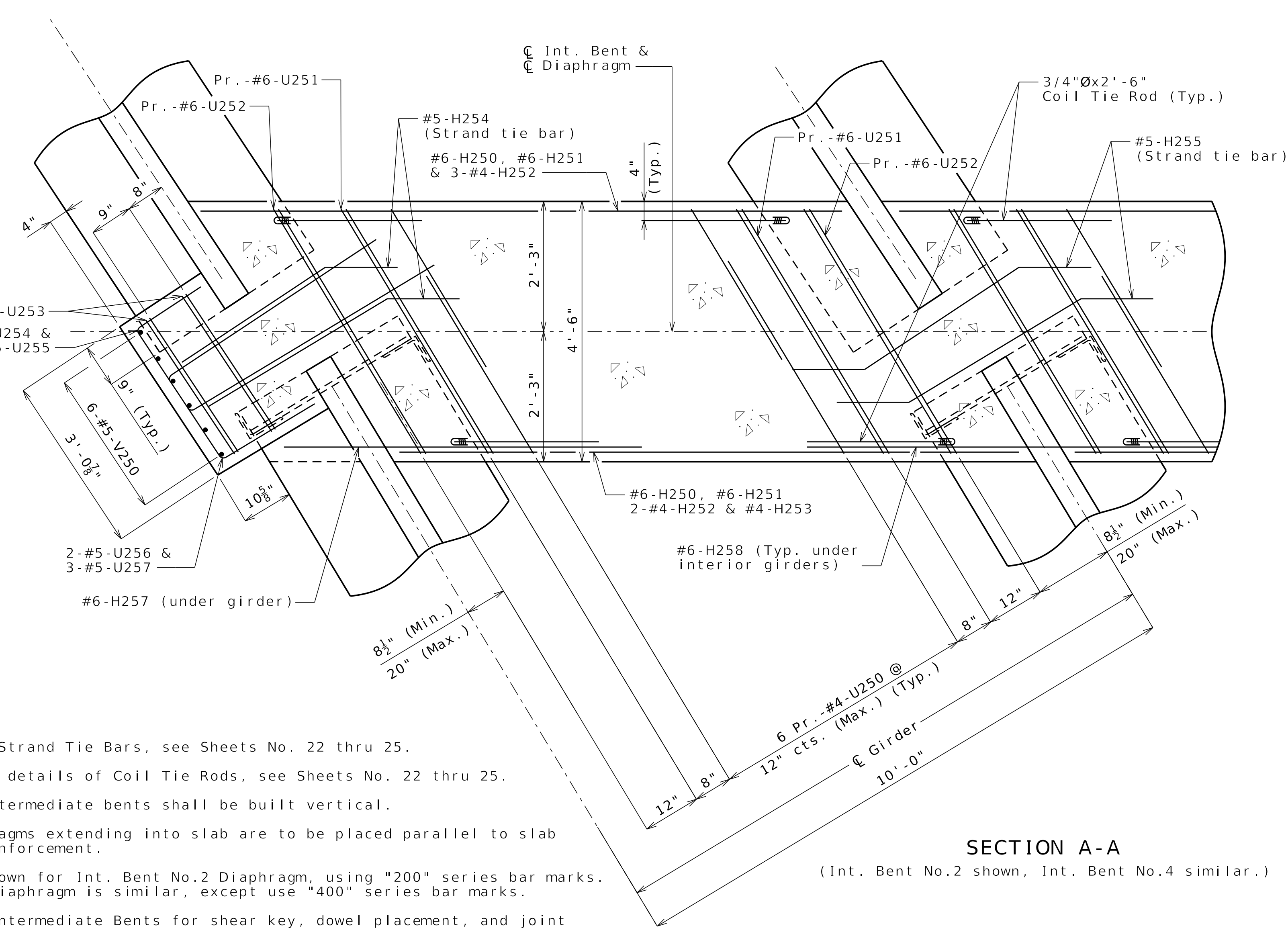
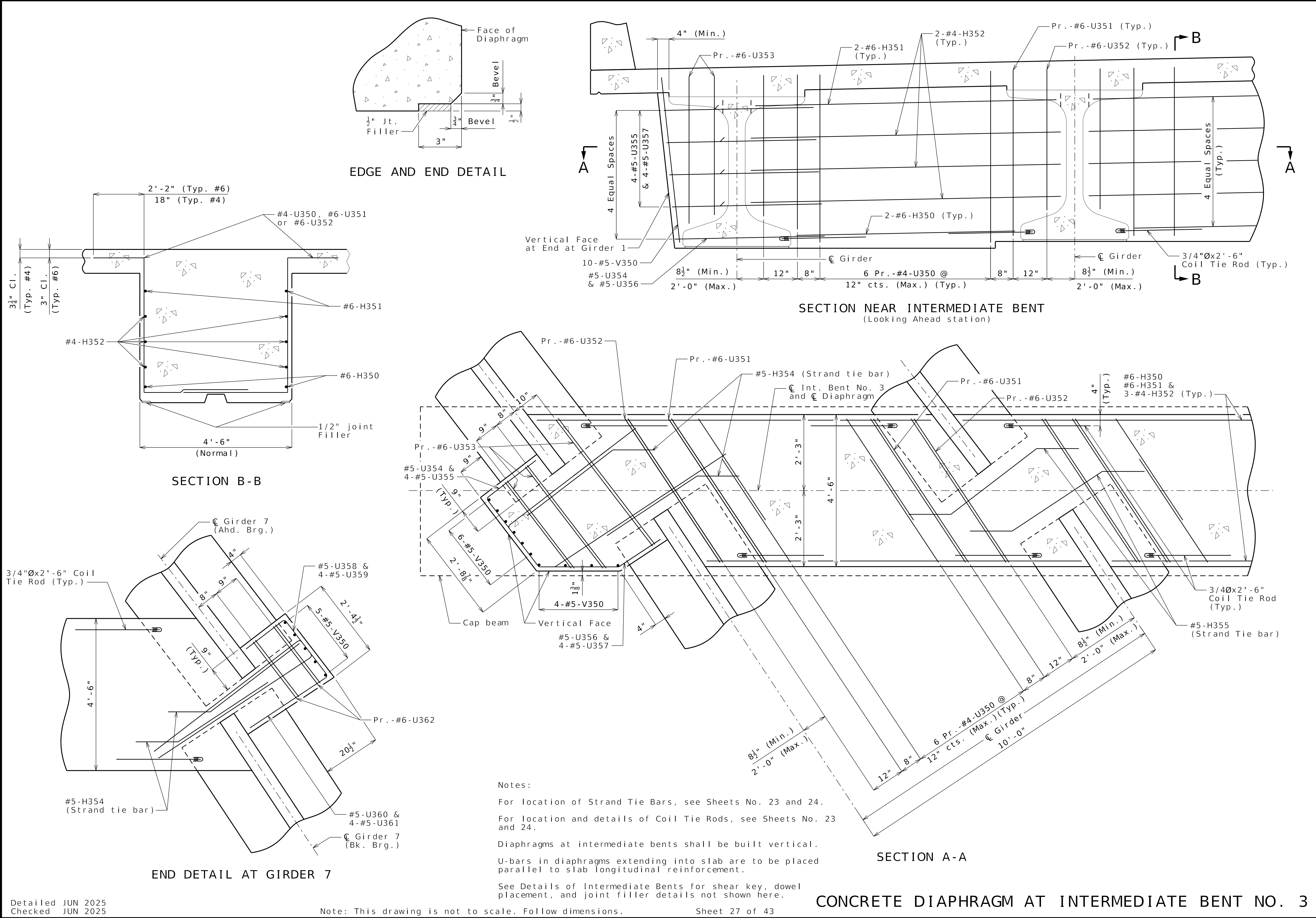


TABLE OF DIMENSIONS		
	Int. Bent No. 2	Int. Bent No. 4
DIM A	2' - 6 $\frac{1}{2}$ "	2' - 3 $\frac{1}{2}$ "
DIM B	16 $\frac{3}{8}$ "	18 $\frac{3}{8}$ "
DIM C	12 $\frac{3}{4}$ "	9"



CONCRETE DIAPHRAGM AT INTERMEDIATE BENTS NO. 2 & 4





STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER
PE-2021035906
PROFESSIONAL ENGINEER

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6/9/2025

ROUTE
US-61

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BR

SHEET NO.
A9682-27

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

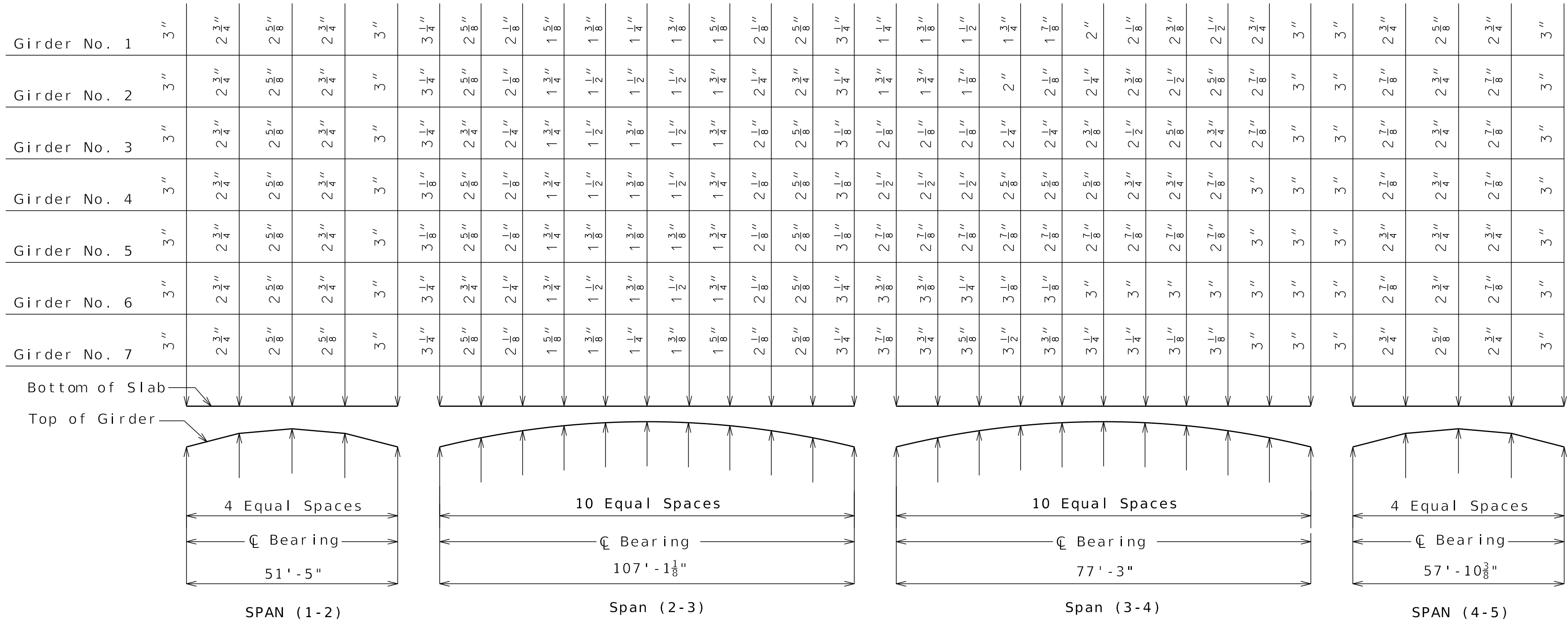
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ALLIANCE

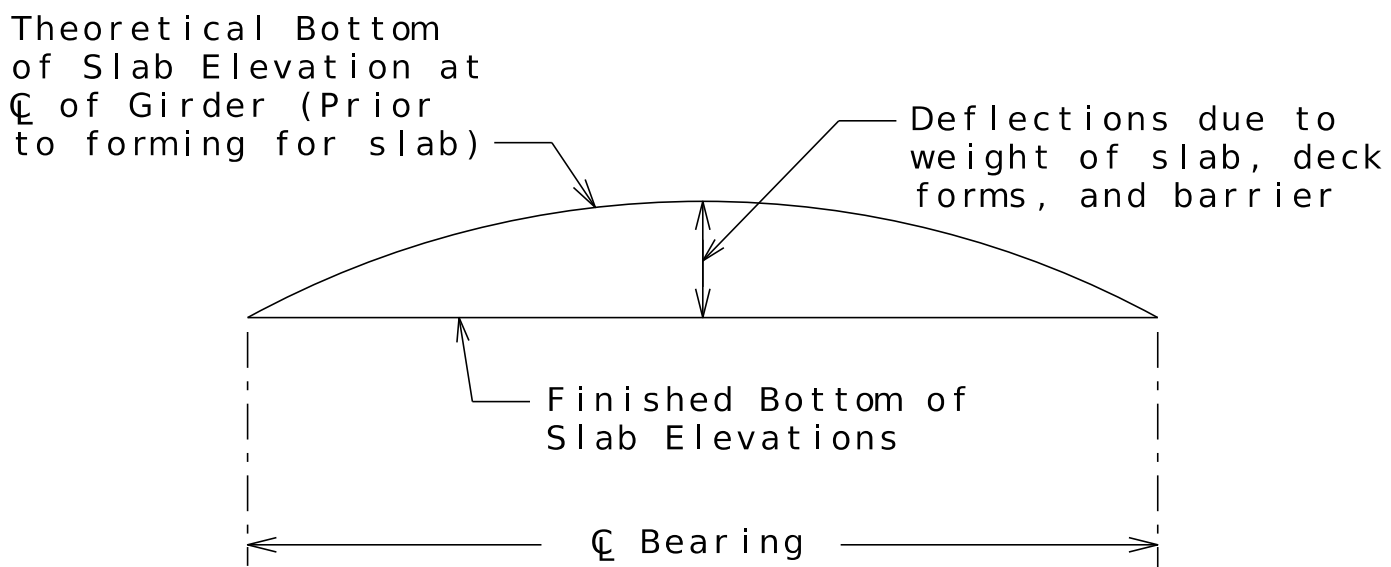
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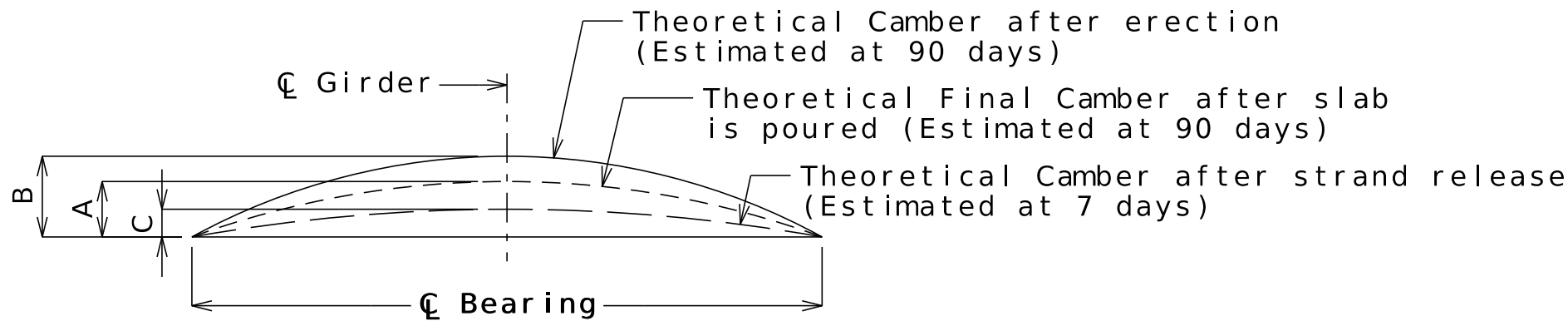
THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)



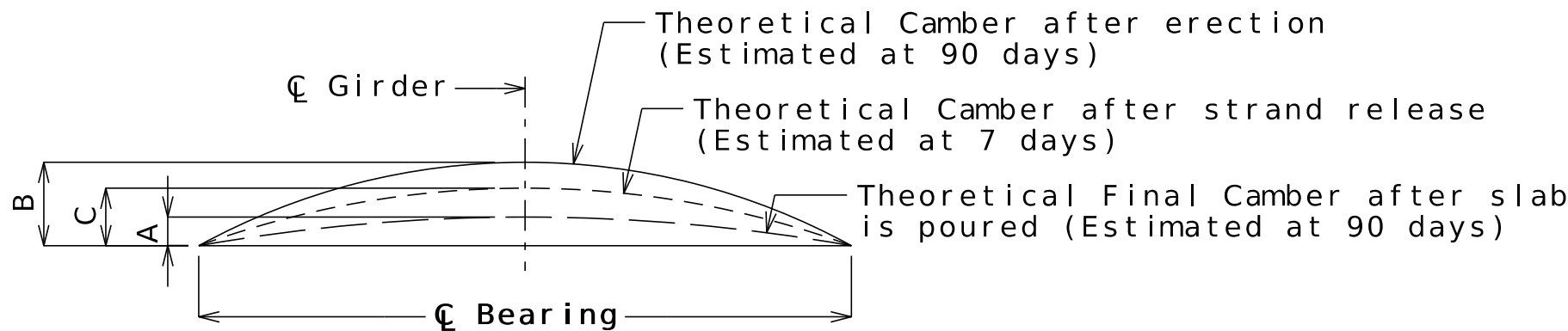
TYPICAL SLAB ELEVATIONS DIAGRAM

Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)											
Girder Number	Span (1-2) (51'-5" C Brg. - C Brg.)										
	C Brg.	.25		.50		.75		C Brg.			
1	633.92	633.82		633.71		633.59		633.47			
2	634.56	634.47		634.36		634.25		634.12			
3	635.21	635.11		635.01		634.90		634.78			
4	635.85	635.76		635.66		635.55		635.43			
5	636.49	636.40		636.31		636.20		636.09			
6	637.13	637.05		636.95		636.85		636.74			
7	637.77	637.69		637.60		637.50		637.39			
Girder Number	Span (2-3) (107'-1½" C Brg. - C Brg.)										
	C Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	C Brg.
1	633.40	633.32	633.24	633.13	633.02	632.88	632.73	632.55	632.36	632.16	631.95
2	634.08	634.01	633.93	633.84	633.72	633.59	633.44	633.27	633.07	632.87	632.65
3	634.75	634.69	634.61	634.52	634.41	634.28	634.13	633.96	633.77	633.57	633.36
4	635.42	635.36	635.29	635.20	635.09	634.97	634.82	634.65	634.47	634.27	634.06
5	636.09	636.03	635.96	635.88	635.77	635.65	635.50	635.34	635.16	634.96	634.76
6	636.76	636.70	636.63	636.55	636.45	636.33	636.19	636.03	635.85	635.65	635.45
7	637.43	637.37	637.30	637.22	637.12	637.00	636.86	636.70	636.53	636.34	636.14
Girder Number	Span (3-4) (77'-3" C Brg. - C Brg.)										
	C Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	C Brg.
1	631.78	631.65	631.51	631.36	631.22	631.06	630.90	630.73	630.56	630.38	630.20
2	632.52	632.39	632.26	632.12	631.97	631.82	631.66	631.50	631.33	631.15	630.97
3	633.27	633.14	633.00	632.87	632.72	632.57	632.42	632.26	632.09	631.92	631.74
4	634.00	633.88	633.75	633.61	633.47	633.33	633.17	633.01	632.85	632.68	632.50
5	634.74	634.61	634.49	634.35	634.22	634.07	633.92	633.76	633.60	633.44	633.26
6	635.47	635.35	635.22	635.09	634.96	634.82	634.67	634.51	634.35	634.19	634.02
7	636.20	636.08	635.95	635.83	635.69	635.56	635.41	635.26	635.10	634.94	634.77
Girder Number	Span (4-5) (57'-10½" C Brg. - C Brg.)										
	C Brg.	.25		.50		.75		C Brg.			
1	630.03	629.69		629.33		628.95		628.55			
2	630.84	630.51		630.16		629.78		629.39			
3	631.64	631.32		630.97		630.60		630.21			
4	632.44	632.12		631.78		631.42		631.04			
5	633.23	632.92		632.58		632.23		631.85			
6	634.02	633.71		633.38		633.03		632.66			
7	634.81	634.50		634.17		633.83		633.47			

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including deck forms and barrier).



Girder	Span (1-2)		
	A	B	C
Exterior	1/2"	5/8"	3/4"
Interior			



Girder	Span (2-3)			Span (3-4)			Span (4-5)		
	A	B	C	A	B	C	A	B	C
Girder No. 1	2½"	4½"	2⅞"	0⅝"	1"	0⅝"	½"	0⅝"	0⅝"
Interior	2¼"						0⅝"		
Girder No. 7	2⅜"						½"		

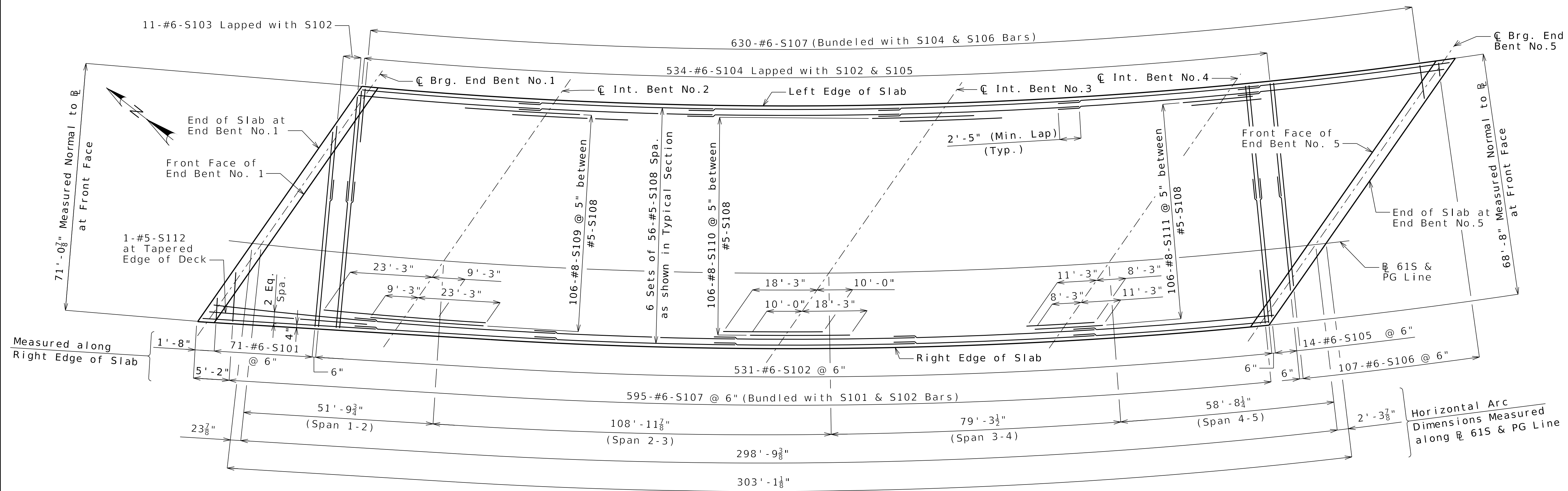
GIRDER CAMBER DIAGRAM

Conversion Factors for Girder Camber (Estimated at 90 days):

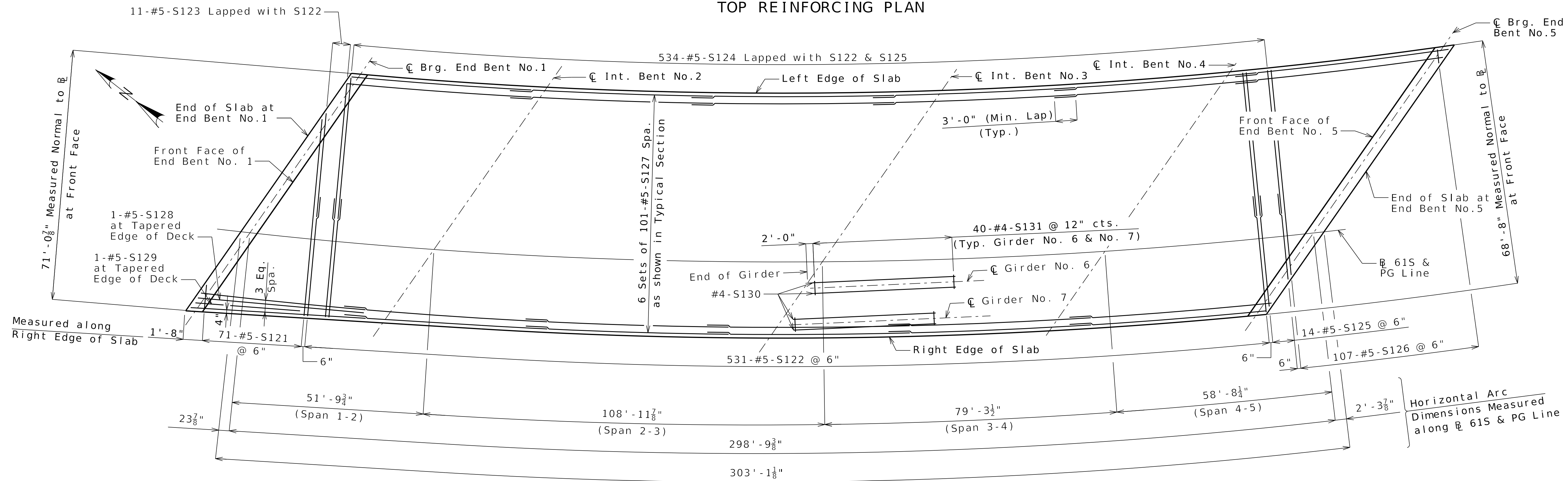
- 0.1 pt. = 0.314 x 0.5 pt.
0.2 pt. = 0.593 x 0.5 pt.
0.3 pt. = 0.813 x 0.5 pt.
0.4 pt. = 0.952 x 0.5 pt.

$$0.25 \text{ pt.} = 0.7125 \times 0.5 \text{ pt.}$$

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. The haunch shall be limited to ensure the projecting girder reinforcement or hairpin bar is embedded into the slab at least 2 inches.



TOP REINFORCING PLAN



BOTTOM REINFORCING PLAN

Notes:

Longitudinal slab dimensions are measured horizontally.

For Section Thru Slab and Slab Pouring Sequence, see Sheet No. 30.

For Details and Reinforcement of Barrier Curb not shown,
see Sheet No. 32.

For Theoretical Slab Haunching Diagram, see Sheet No. 28.

For Theoretical Bottom of Slab Elevations, see Sheet No. 28.

For Haunch Reinforcing Detail, see Sheet No. 30.

PLAN OF SLAB SHOWING REINFORCEMENT



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5/9/2025

ROUTE	STATE
US 61	MO

DISTRICT	SHEET NO.
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BR	A9682-
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COUNTY

ST. CHARLES

JOB NO.
16T0020

5310020

CONTRACT ID

CONCLUSIONS

PROJECT NO.

BRIDGE NO.
12222

A9682

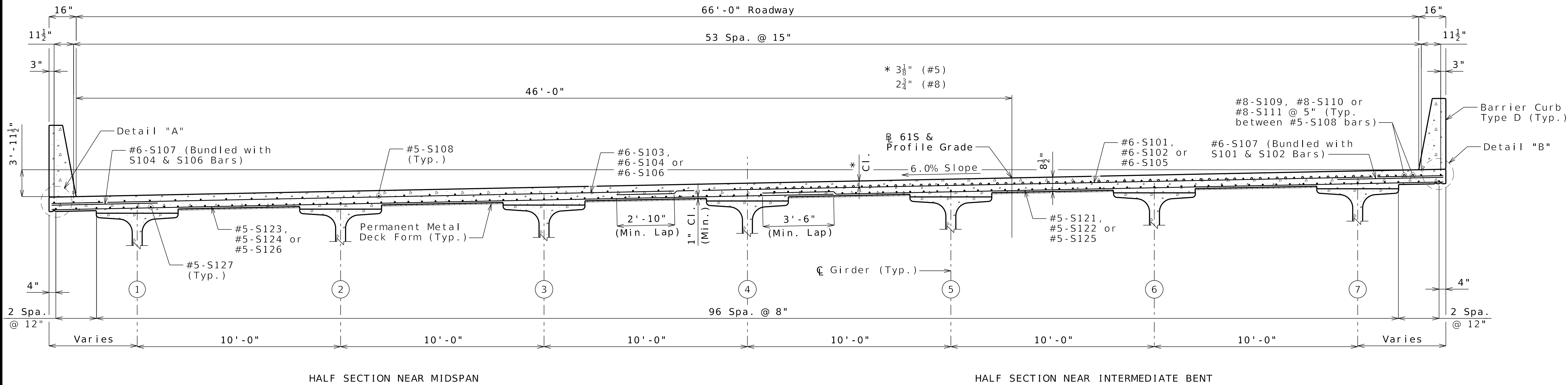
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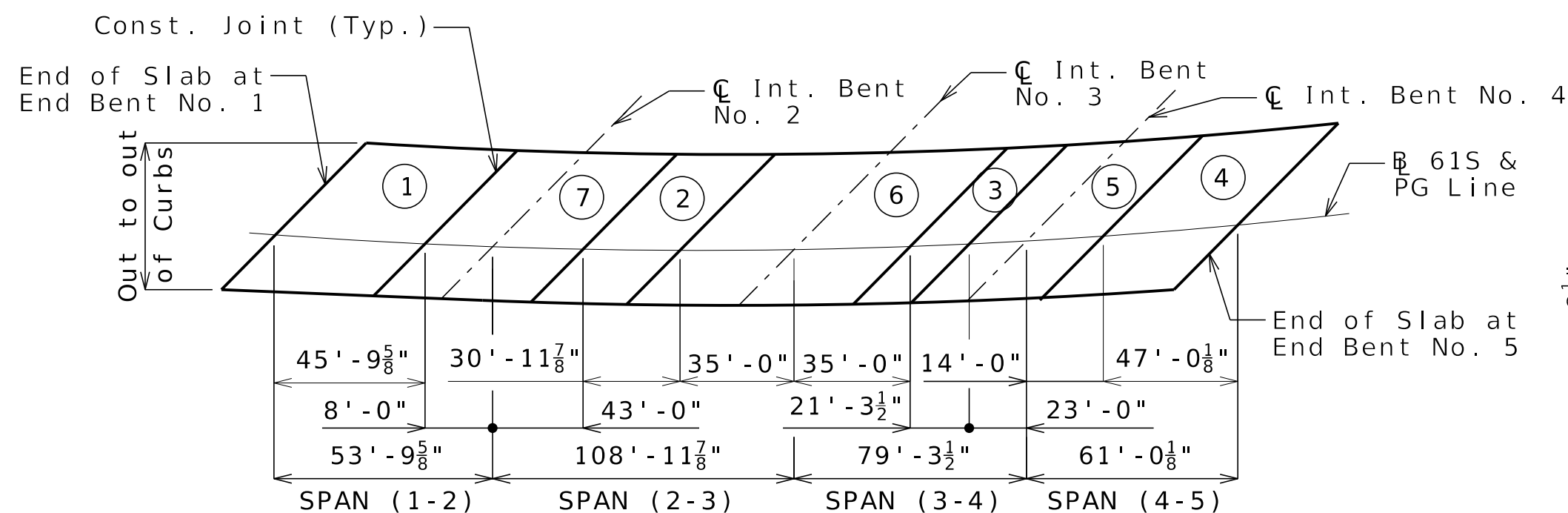


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TYPICAL SECTION THRU SLAB
(Shown Normal to B Ramp)
Slab Width varies in Span (1-2) and (2-3). Span (3-4) and (4-5) shown.



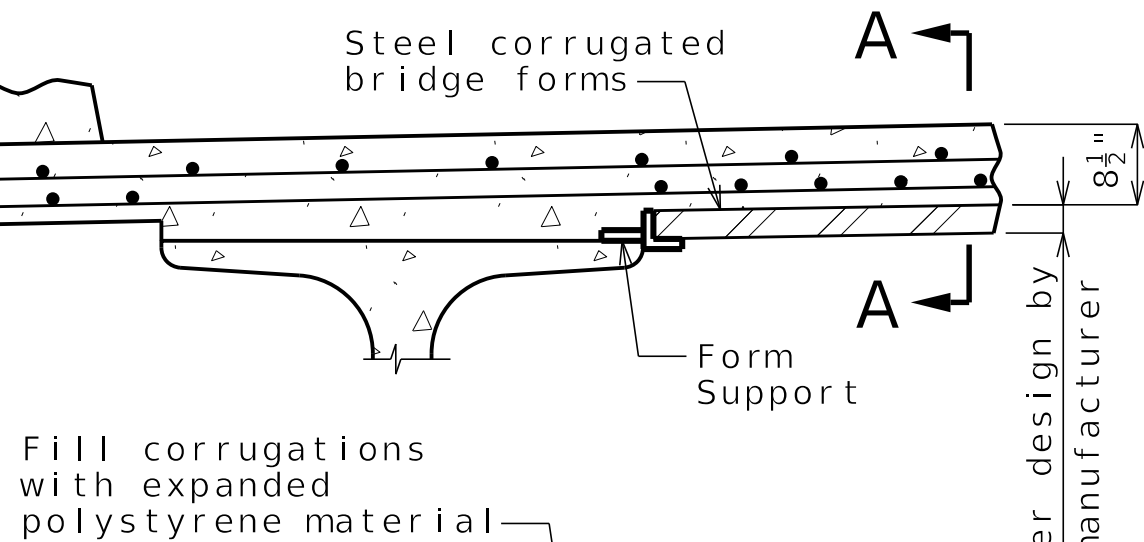
	Sequence of Pours							Min. Rate of Pour Cu. Yds./Hr.	
	Direction							With Retarder	
Basic Sequence	1	2	3	4	5	6	7	25	
	Either Direction								
Alternate pours to the basic skip sequence are subject to the approval of the engineer in accordance with Sec 703.									
Alternate A Pours	1		7 + 2		6 + 3		5 + 4		58
	7 to End		6 to 1		5 to 2		End to 3		
Alternate B Pours	1 + 7 + 2			6 + 3			5 + 4		58
	6 to End			5 to 2			End to 3		
Alternate C Pours	1 + 7 + 2				6 + 3 + 5 + 4				58
	6 to End				End to 2				
Alternate D Pours	4 + 5 + 3 + 6 + 2 + 7 + 1								58
	End to End (Upgrade)								

The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours, and shall pour and satisfactorily finish the slab pours at the rate given.

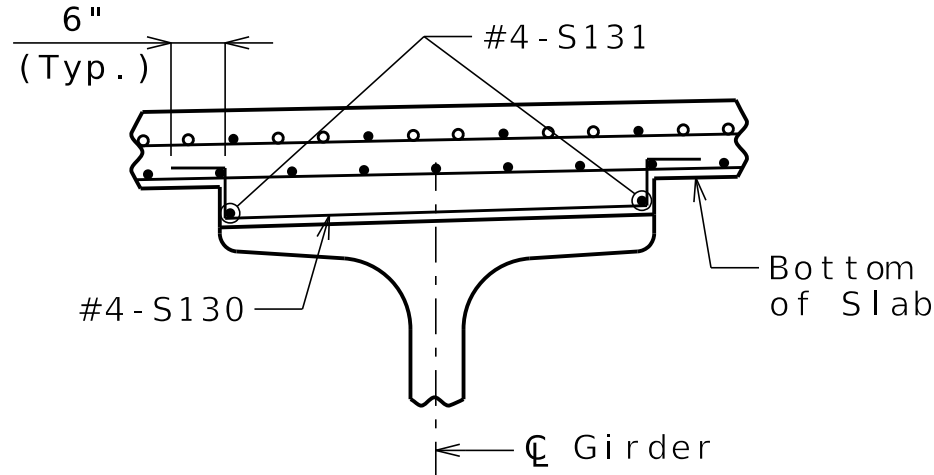
Bridge deck must be poured upgrade.

The concrete diaphragm at the intermediate bents and integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

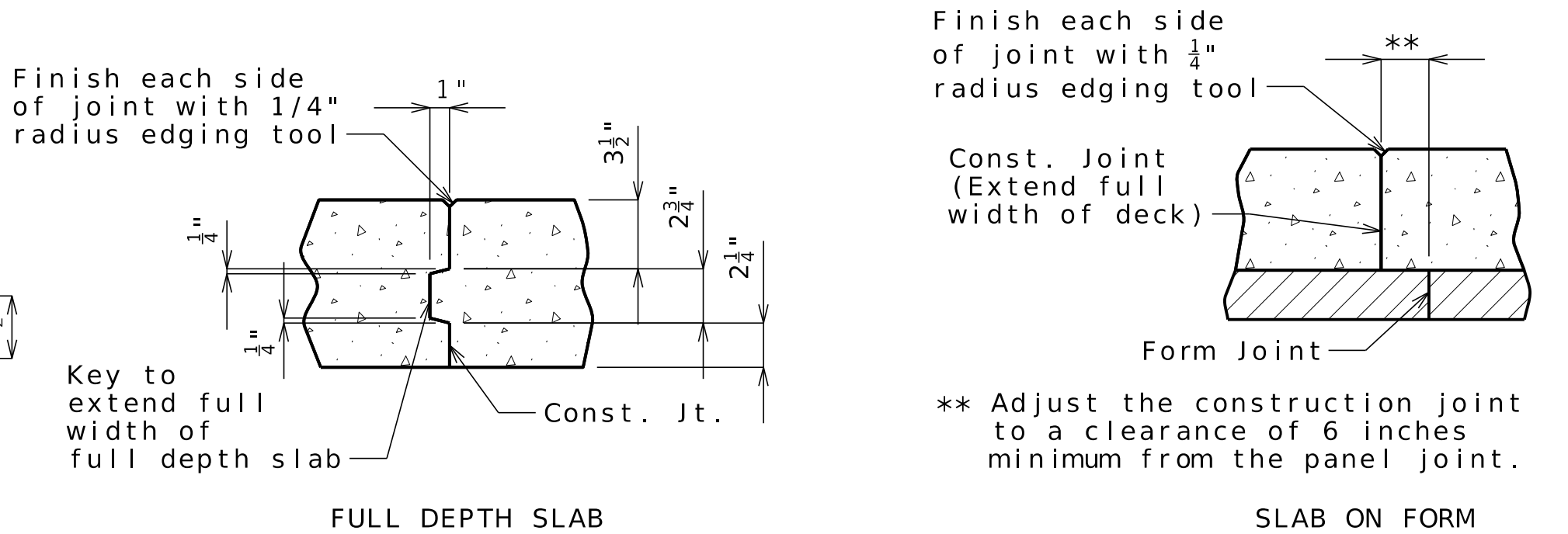
SLAB POURING SEQUENCE



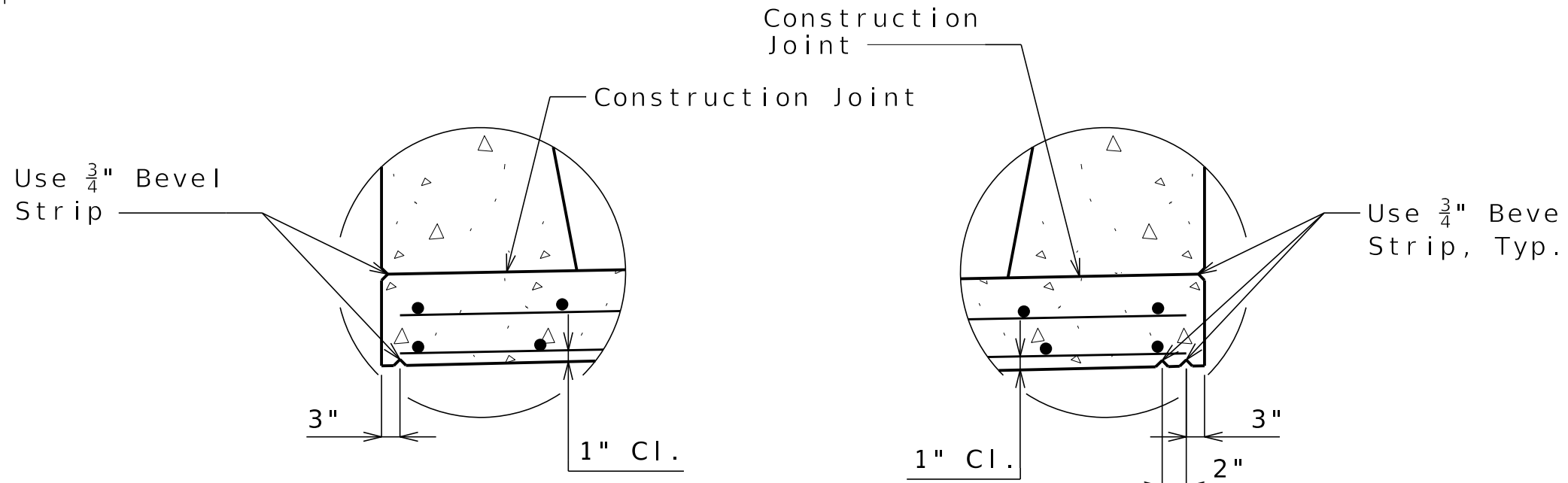
STAY-IN-PLACE FORM DETAILS



HAUNCH REINFORCING DETAIL



SLAB CONSTRUCTION JOINT



Notes:

Barrier Curb reinforcement not shown for clarity.

For Plan of Slab Showing Reinforcement and additional notes, see Sheet No. 29.

For metal deck form notes, see General Notes.

Contractor may shift bars as necessary to tie in barrier bars.

SLAB DETAILS

STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER
PE-2021035906
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
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DATE PREPARED
6/9/2025

ROUTE
US-61

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SHEET NO.
A9682-30

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

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MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

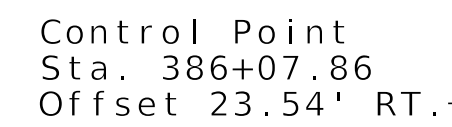
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Kink Point
Sta. 386+84.83
Offset 21.33' RT.

SPAN (1 - 2)

SPAN (2-3)

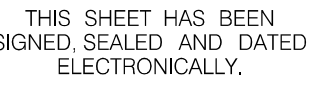
SPAN (3-4)

SPAN (4-5)

All dimensions are horizontal.
All bents are parallel

Sheet 31 of 43

B A9682 027 JST0020 SLABCURV.dgn 9:26:41 AM 6/9/2025



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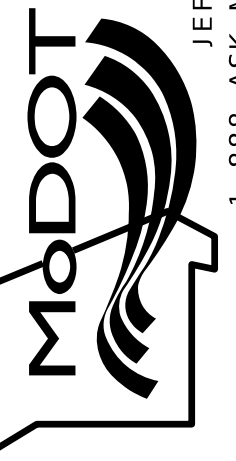
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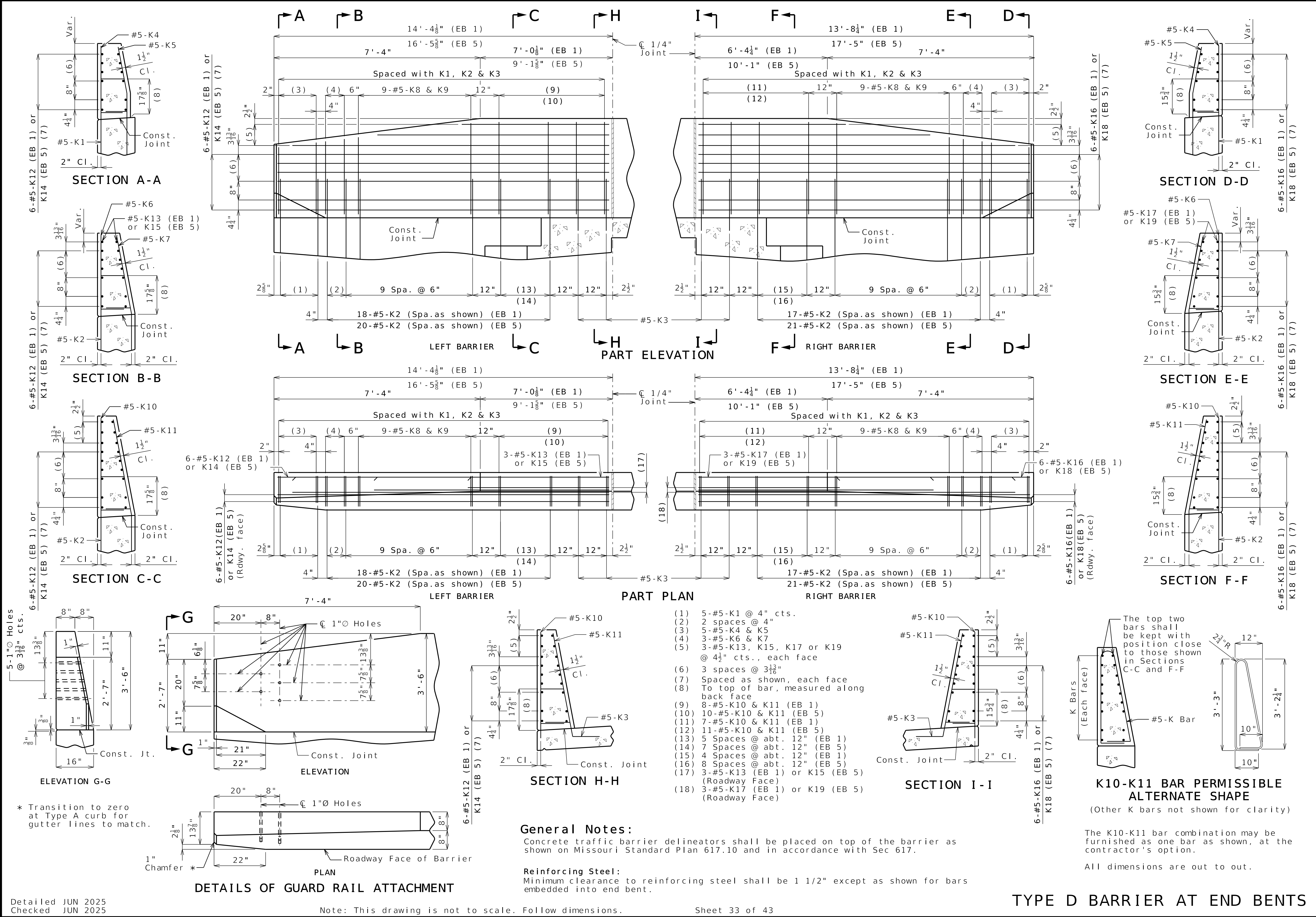


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STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER PE-2021035906
PROFESSIONAL ENGINEER

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DATE PREPARED
6/9/2025
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CONTRACT ID.
PROJECT NO.
BRIDGE NO.
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DESCRIPTION

DATE

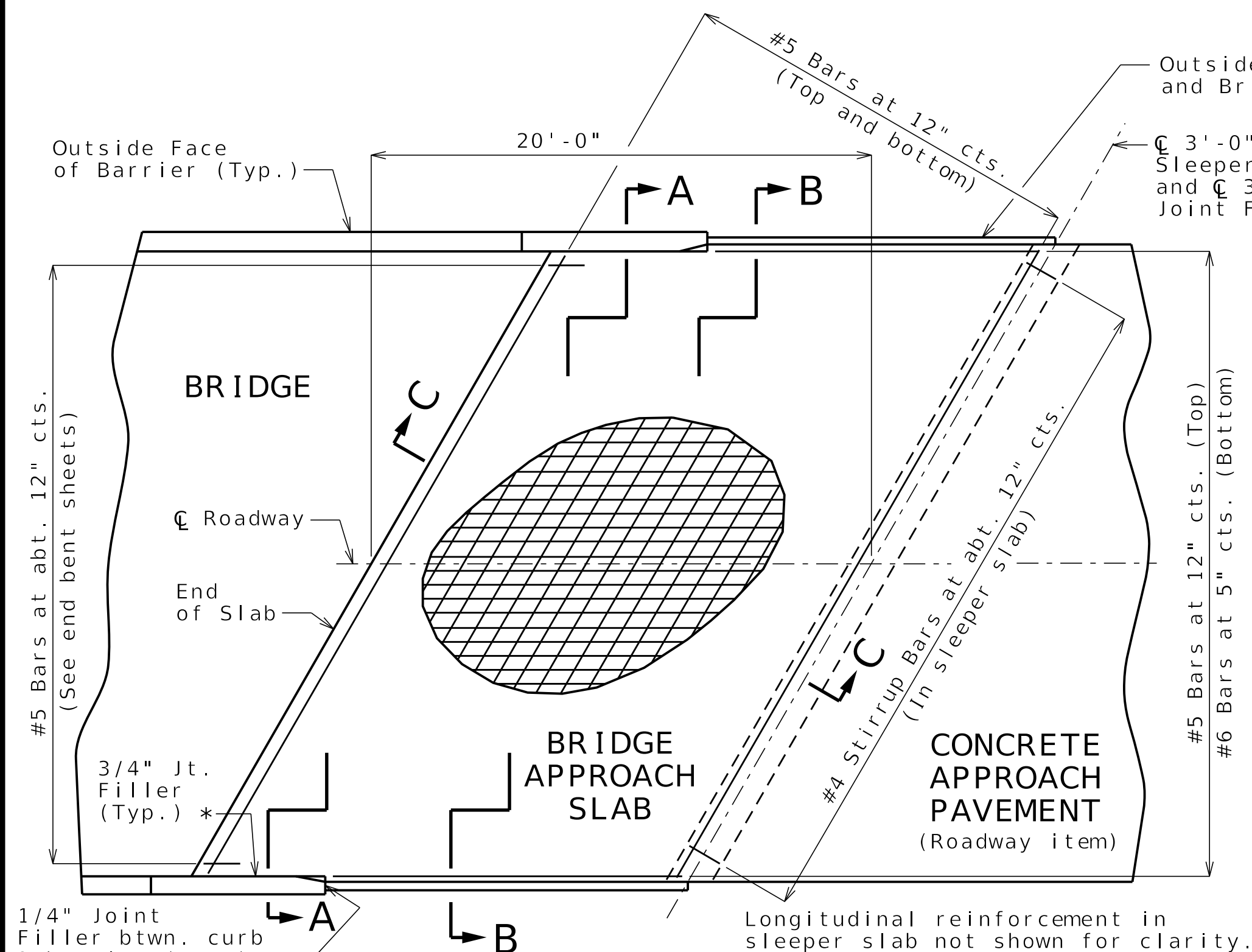
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

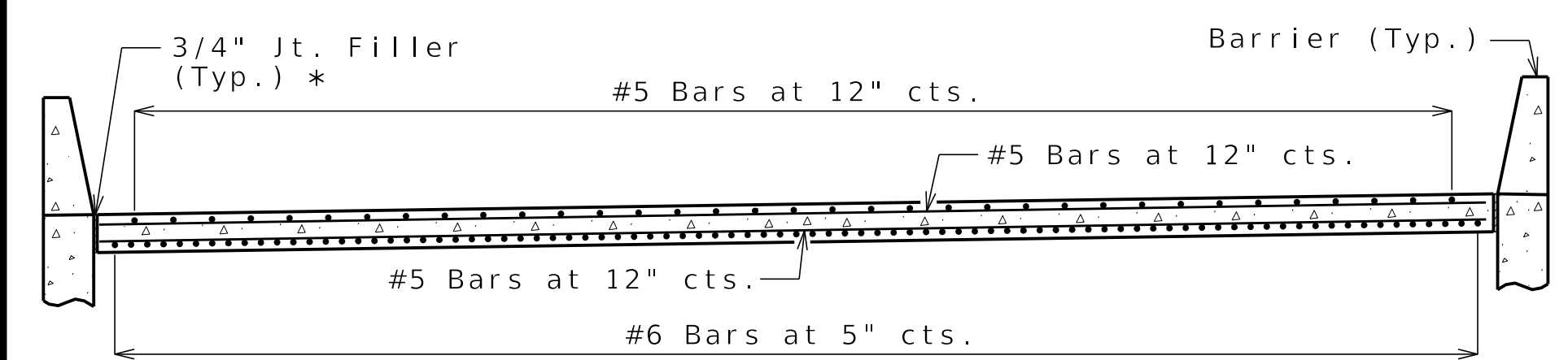
IMPROVE70
ALLIANCE

715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270
HNTB

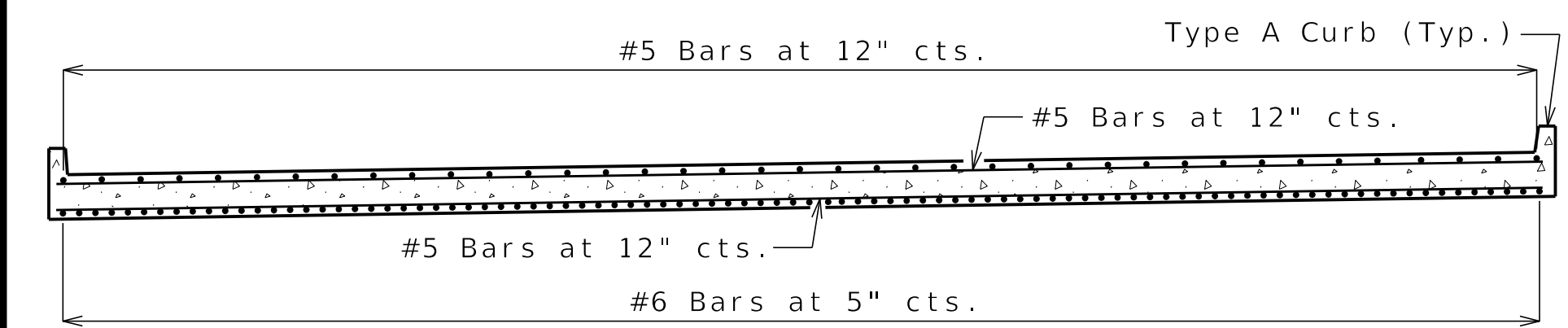
601 INDORE ST., SUITE 201 - JEFFERSON CITY, MO 65101
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FAX 872-630-3182
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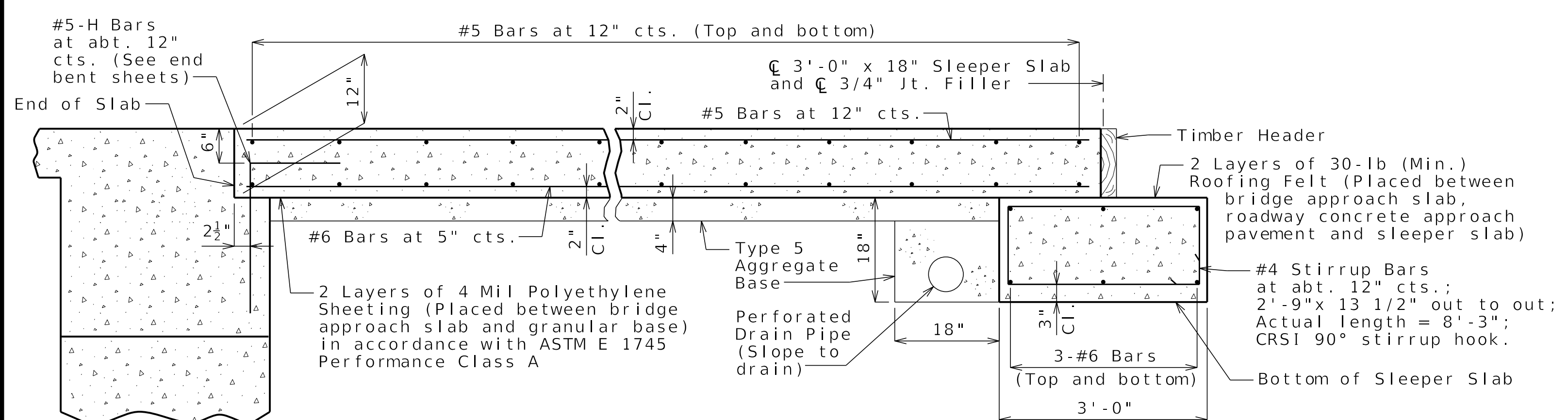
PART PLAN SHOWING REINFORCEMENT



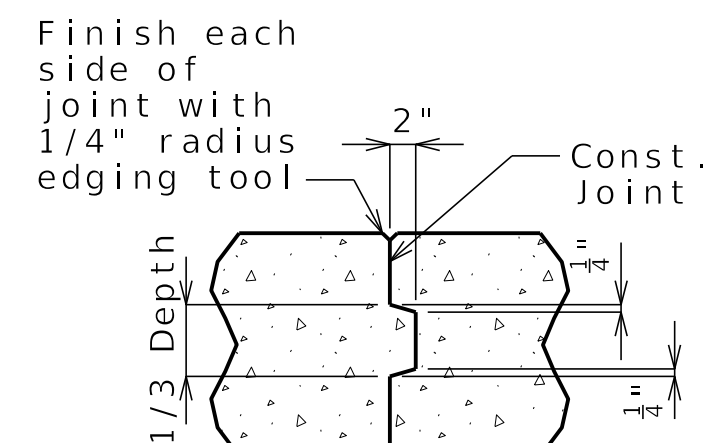
SECTION A-A



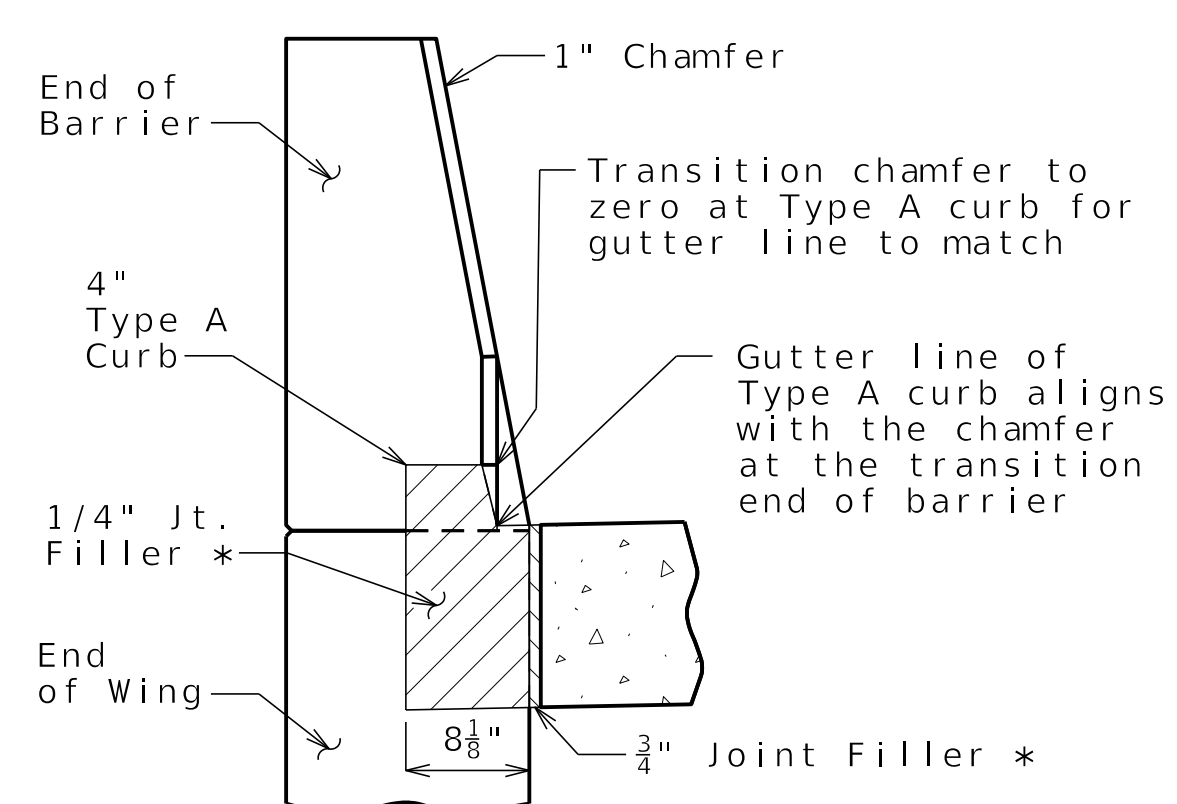
SECTION B-B



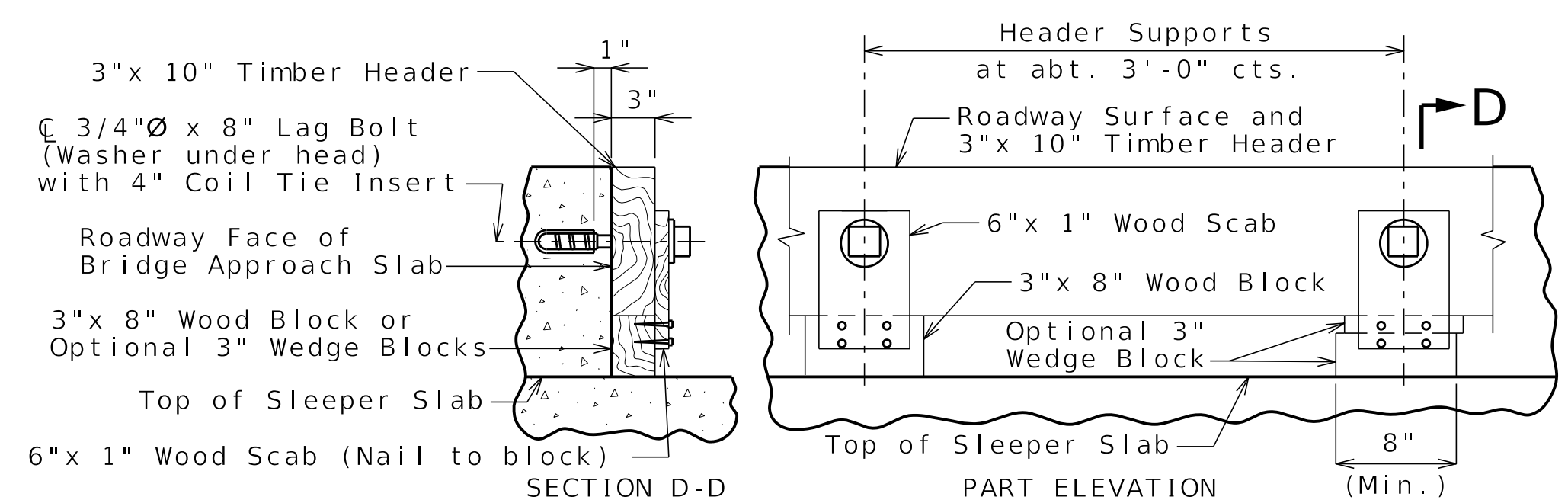
SECTION C-C



CONSTRUCTION JOINT DETAIL



SECTION BETWEEN CURB AND BARRIER



DETAILS OF TIMBER HEADER

BRIDGE APPROACH SLAB (MAJOR)

General Notes:

All concrete for the bridge approach slab and sleeper slab shall be in accordance with Sec 503 ($f'c = 4,000$ psi).

The reinforcing steel in the bridge approach slab and the sleeper slab shall be epoxy coated Grade 60 with $f_y = 60,000$ psi.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab and the sleeper slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 29 inches for #5 bars and 44 inches for #6 bars, or by mechanical bar splice.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

For concrete approach pavement details, see roadway plans.

See Missouri Standard Plan 609.00 for details of Type A curb.

* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER PE-2021035906
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.
DATE PREPARED
6/9/2025
ROUTE
US-61
DISTRICT
BR
COUNTY
ST. CHARLES
JOB NO.
JST0020
CONTRACT ID.
PROJECT NO.
BRIDGE NO.
A9682

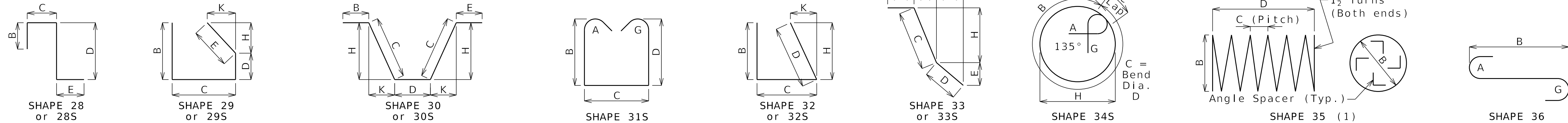
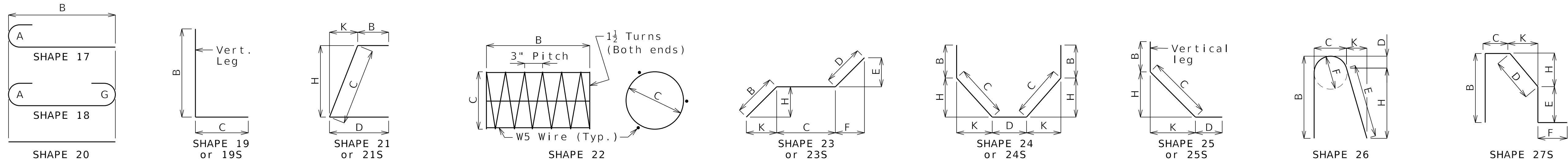
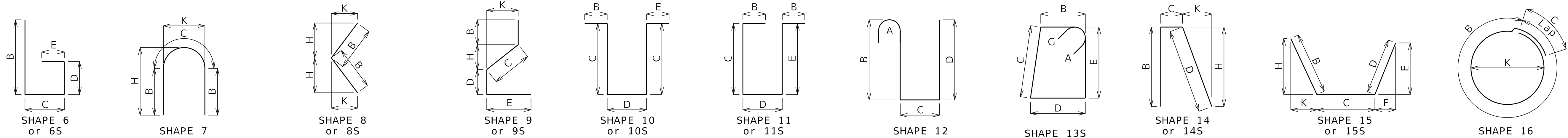
DESCRIPTION
DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

IMPROVE70
ALLIANCE

HNTB
715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
PHONE 873-633-3181
FAX 873-633-3181
WWW.HNTB.COM

Bartlett & West
601 INDEPENDENT ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 873-633-3181
FAX 873-633-3181
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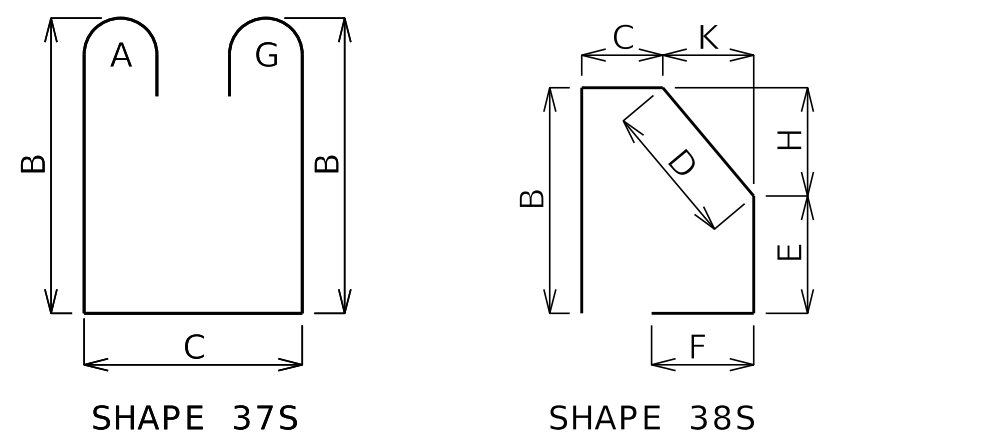


Finished Bend Dimensions D and Hook Dimensions						
Standard Pin Bend Shapes						
Size	Case	D	A or G		J	
			90°	180°	180°	
#4	1	3"	8"	6"	4"	
#5	1	3 3/4"	10"	7"	5"	
#6	1	4 1/2"	12"	8 1/4"	6"	
#7	2	5 1/4"	14"	9 3/4"	7"	
	3	7"	15"	11 1/2"	8 3/4"	
#8	2	6"	16"	11"	8"	
	3	8"	17"	13 1/4"	10"	
#9	1	9 1/2"	19 1/2"	15 1/2"	11 3/4"	
#10	1	10 3/4"	22"	17 1/2"	13 1/4"	
#11	1	12"	24 1/2"	19 1/2"	14 3/8"	
#14	1	18 1/4"	31 1/4"	27 1/2"	21 5/8"	
#18	1	24"	41 1/2"	36 1/4"	28 1/2"	

Stirrup Pin Bend Shapes (S)						
Size	Case	D	A or G		H	
			90°	135°	180°	180°
#4	2	2"	4 1/2"	4 1/2"	5"	2 7/8"
	3	3"	5"	5 1/4"	6"	3"
#5	2	2 1/2"	5 3/4"	5 3/4"	5 3/4"	3 3/4"
	3	3 3/4"	6 1/4"	6 1/4"	7"	3 5/8"
#6	1	4 1/2"	12"	7 3/4"	8 1/4"	4 5/8"

Applicable for all grades of steel.

Case 1 applies to all reinforcement. Case 2 applies to all reinforcement except for galvanized bars. Case 3 applies to galvanized bars only.



BENDING DIAGRAMS

All dimensions are out to out.

Shapes ending with an S shall be bent in accordance with stirrup pin bend shapes.

Unless otherwise noted, finished bending diameter D is the same for all bends of a shape.

(1) Shall be a deformed or plain spiral bar or wire.

Four angle or channel spacers are required for each column spiral. Spacers are to be placed on inside of spirals. Length and weight of column spirals do not include splices or spacers.

STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER
PE-2021035906
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED, AND DATED
ELECTRONICALLY.

DATE PREPARED
6/9/2025

ROUTE
US-61

STATE
MO

DISTRICT
BR

SHEET NO.
A9682-35

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

IMPROVE 70
ALLIANCE

715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270
HNTB

601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 872-630-3181
WWW.BARTLETTWEST.COM
Bartlett & West
ENGINEERING

All bars shall be Grade 60.

54 AM 6/9/202

Bill of Reinforcing Steel															
No. Req.	Size/ Mark	Location	Codes			Dimensions						Nom. Length	Actual Length	Weight	
						B	C	D	E	F	H				K
			C	SH	V	ft in.	ft in.	ft in.	ft in.	ft in.	ft in.	ft in.	ft in.	ft in.	lb
		Int. Bent No. 4													
36	9 D400	Key	E	20		2 6.00							2 6	2 6	306
24	11 H400	Beam	E	17		47 3.88							48 11	48 11	6237
16	11 H401	Beam	E	20		47 3.88							47 4	47 4	4024
20	6 H402	Beam	E	20		45 5.38							45 5	45 5	1364
14	6 H403	Beam	E	10			12.00	4 7.50					6 8	6 4	133
28	6 H404	Beam	E	19		16.13	12 9.00						14 1	13 11	585
4	6 H405	Beam	E	19		16.13	13 5.00						14 9	14 7	88
228	6 P400	Beam & Column	E	34S		10 12.00	4 5.0				3 6.00		12 8	12 7	4309
256	6 P401	Drilled Shaft		34S		10 12.00	4 5.0				3 6.00		12 8	12 7	4838
208	6 U400	Beam	E	13S		3 2.00	4 0.00	3 2.00	4 0.00				15 8	15 2	4738
40	6 U401	Beam	E	10S			4 0.00	3 2.00					11 2	10 10	651
63	4 U402	Beam	E	10S			16.13	4 9.00					7 5	7 3	305
21	11 V400	Column	E	20		18 4.38							18 4	18 4	2046
21	11 V401	Column	E	20		19 7.88							19 8	19 8	2194
21	11 V402	Column	E	20		20 11.38							20 11	20 11	2334
21	11 V403	Column	E	20		22 2.88							22 3	22 3	2482
28	11 V404	Drilled Shaft		20		39 4.50							39 5	39 5	5864
28	11 V405	Drilled Shaft		20		41 4.50							41 5	41 5	6161
		Int. Bent No. 2													
12	6 H250	Diaphragm	E	20		7 6.00							7 6	7 6	135
12	6 H251	Diaphragm	E	20		10 3.00							10 3	10 3	185
30	4 H252	Diaphragm	E	20		10 8.00							10 8	10 8	214
6	4 H253	Diaphragm	E	20		7 6.00							7 6	7 6	30
4	5 H254	Strand Tie	E	23		15.50	3 3.00				8.50	13.00	4 7	4 5	18
10	5 H255	Strand Tie	E	23		15.50	3 3.00	15.50	8.50	13.00	8.50	13.00	5 10	5 10	61
1	6 H257	Diaphragm	E	20		5 6.00							5 6	5 6	8
5	6 H258	Diaphragm	E	20		8 9.00							8 9	8 9	66
1	6 H259	Diaphragm	E	23		2 0.00	3 1.00				12.50	20.50	5 1	4 11	7
72	4 U250	Diaphragm	E	28S			18.00	4 10.00	3 6.50				9 11	9 8	465
24	6 U251	Diaphragm	E	28S			2 2.00	4 10.00	4 10.00				11 10	11 6	415
24	6 U252	Diaphragm	E	28S			2 2.00	3 6.00	4 10.00				10 6	10 2	366
4	6 U253	Diaphragm	E	28S			2 2.00	3 6.00	2 10.00				8 6	8 2	49
1	5 U254	Diaphragm	E	14S			21.00	5 0.00			5 0.00	2.50	6 9	6 8	7
4	5 U255	Diaphragm	E	14S		16.00	21.00	5 0.00			5 0.00	2.50	8 1	7 11	33
2	5 U256	Diaphragm	E	19S		4 5.00	21.00						6 2	6 1	13
3	5 U257	Diaphragm	E	14S		4 5.00	21.00	23.00			23.00	1.00	8 1	7 11	25
1	5 U258	Diaphragm	E	21S			5 4.50	14.50			5 4.50	2.75	6 7	6 6	7
4	5 U259	Diaphragm	E	32S		16.00	14.50	5 4.50			5 4.50	2.75	7 11	7 8	32
2	5 U260	Diaphragm	E	19S		5 5.00	17.00						6 10	6 9	14
3	5 U261	Diaphragm	E	32S		5 5.00	17.00	2 4.50			2 4.50	1.25	9 3	9 0	28
4	6 U262	Diaphragm	E	28S			2 2.00	3 6.00	2 2.50				7 11	7 7	46
11	5 V250	Diaphragm	E	20		5 1.00							5 1	5 1	58

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

For bending diagrams, see Sheet No. 35.

Detailed JUN 2025
Checked JUN 2025

Codes: C = Required coatings, where E = Epoxy Coated and G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

Note: This drawing is not to scale. Follow dimensions.

Bill of Reinforcing Steel																												
No. Req.	Size/ Mark	Location	Codes			Dimensions										Nom. Length		Actual Length		Weight								
						B	C	D	E	F	H	K																
			C	SH	V	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	lb
		Int. Bent No. 3																										
12	6	H350	Diaphragm	E	20	7	9.00																					
12	6	H351	Diaphragm	E	20	10	6.00																					
36	4	H352	Diaphragm	E	20	10	11.00																					
4	5	H354	Strand Tie	E	23	16.13	3	3.00								9.50	13.00											
10	5	H355	Strand Tie	E	23	16.13	3	3.00	16.13			9.50	13.00	9.50	13.00													
72	4	U350	Diaphragm	E	28S			18.00	4	10.00	3	6.50																
24	6	U351	Diaphragm	E	28S			2	2.00	4	10.00	5	2.00															
24	6	U352	Diaphragm	E	28S			2	2.00	3	6.00	5	2.00															
6	6	U353	Diaphragm	E	28S	2		2	2.00	3	6.00	3	2.00															
		Increment = 7"						2	2.00	3	6.00	4	4.00															
1	5	U354	Diaphragm	E	27S			5	10.00	13.13	2	6.00				8.00	10.38											
4	5	U355	Diaphragm	E	38S			5	10.00	13.13	2	6.00		20.00		8.00	10.38											
1	5	U356	Diaphragm	E	38S				2	4.00	18.00	4	10.00		17.00		22.25											
4	5	U357	Diaphragm	E	38S			18.00	2	4.00	18.00	4	10.00		17.00		22.25											
1	5	U358	Diaphragm	E	21S			5	8.13	16.00					5	8.00	4.75											
4	5	U359	Diaphragm	E	32S			16.00	16.00	5	8.13				5	8.00	4.75											
1	5	U360	Diaphragm	E	19S	5		8.00	16.00																			
4	5	U361	Diaphragm	E	32S	5		8.00	16.00	2	8.63				2	8.50	2.25											
4	6	U362	Diaphragm	E	28S			2	2.00	3	6.00	2	0.50															
15	5	V350	Diaphragm	E	20	5	1.00																					
		Int. Bent No. 4																										
12	6	H450	Diaphragm	E	20	8	0.00																					
12	6	H451	Diaphragm	E	20	11	1.00																					
30	4	H452	Diaphragm	E	20	11	6.00																					
6	4	H453	Diaphragm	E	20	8	0.00																					
4	5	H454	Strand Tie	E	23	15.63	3	3.00								10.00	12.00											
10	5	H455	Strand Tie	E	23	15.63	3	3.00	15.63		10.00	12.00	10.00	12.00	10.00	12.00												
1	6	H457	Diaphragm	E	23	2	0.00	3	1.00						15.50	18.25												
5	6	H458	Diaphragm	E	20	8	3.00																					
1	6	H459	Diaphragm	E	20	7	5.00																					
72	4	U450	Diaphragm	E	28S			18.00	4	10.00	3	6.50																
24	6	U451	Diaphragm	E	28S			2	2.00	4	10.00	5	3.00															
24	6	U452	Diaphragm	E	28S			2	2.00	3	6.00	5	3.00															
6	6	U453	Diaphragm	E	28S	2		2	2.00	3	6.00	3	7.00															
		Increment = 7"						2	2.00	3	6.00	4	9.00															
1	5	U454	Diaphragm	E	38S				2	0.25	17.50	4	9.50		15.75	18.50												
4	5	U455	Diaphragm	E	38S			17.50	2	0.25	17.50	4	9.50		15.75	18.50												
2	5	U456	Diaphragm	E	27S			5	6.00	9.88	2	10.00			6.38	7.50												
3	5	U457	Diaphragm	E	38S			5	6.00	9.88	2	10.00		21.00	6.38	7.50												
1	5	U458	Diaphragm	E	21S			5	6.38	15.00					5	6.25	3.38											
4	5	U459	Diaphragm	E	32S			16.00	15.00	5	6.38				5	6.25	3.38											
2	5	U460	Diaphragm	E	19S	5		6.00	15.00																			
3	5	U461	Diaphragm	E	32S	5		6.00	15.00	2	6.50				2	6.50	1.63											
4	6	U462	Diaphragm	E	28S			2	2.00	3	5.50	2	0.00															
15	5	V450	Diaphragm	E	20	5	1.00																					

All bars shall be Grade 60.

BILL OF REINFORCING STEEL (2 OF 3)



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6 / 9 / 202

ROUTE	S
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US - 61 | M

DISTRICT	SHE
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BR | A96

COUNTY

ST. CHARL

LOB NO.

IST0020

CONTRACT ID:

PROJECT NO.

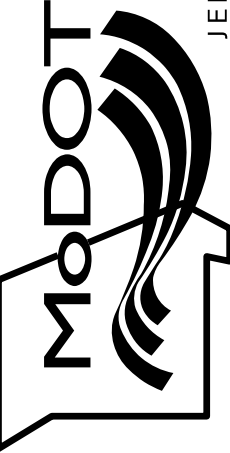
BRIDGE NO.

A9682

[illegible]

JEFFERSON COUNTY
OFFICE OF TRANSPORTATION

MISSOURI HIGH



105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY

Bartlett & West

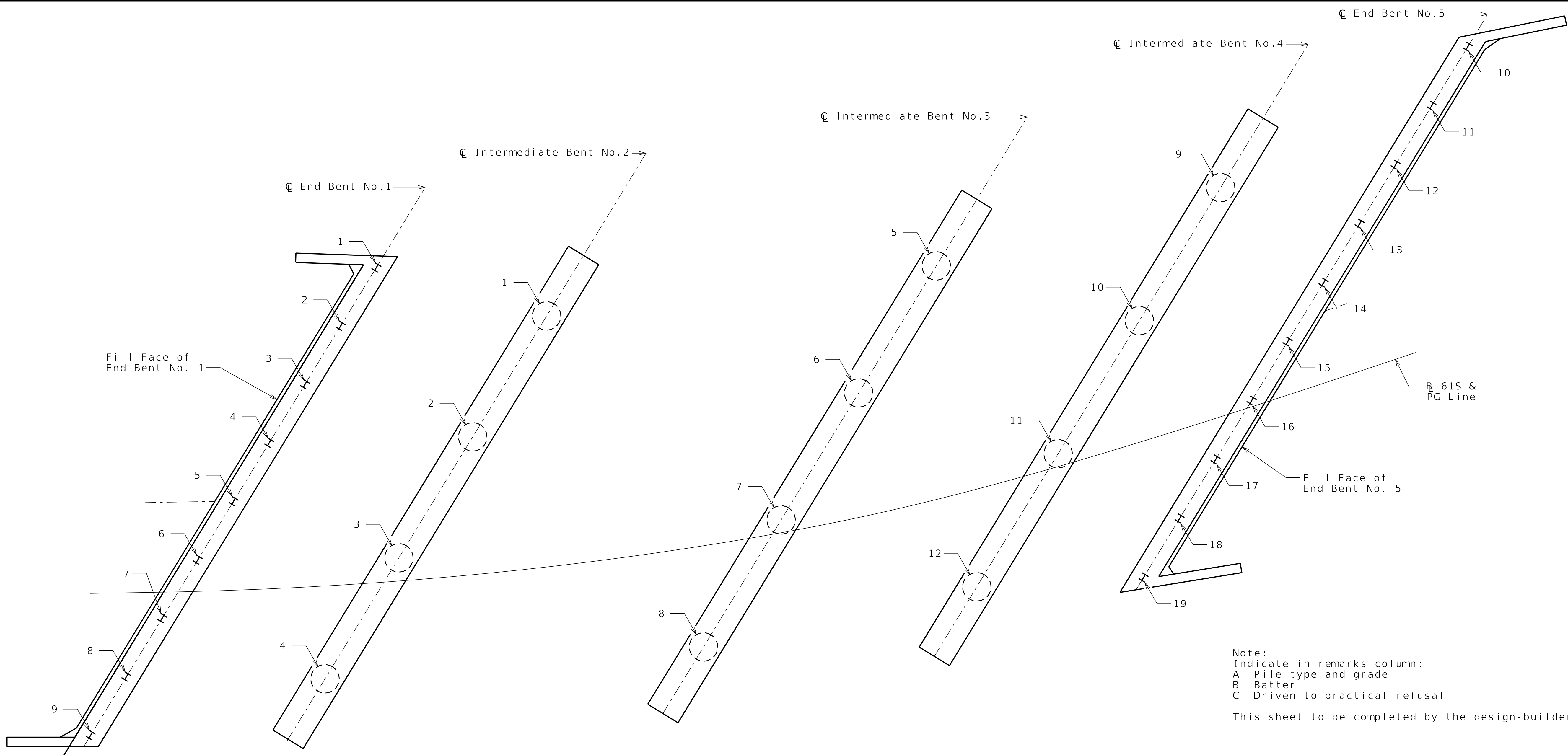
601 MONROE ST, SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 573-634-3181
CERTIFICATE OF AUTHORITY NO. 000167 - ENGINEERING
WWW.PADTIENTWEST.COM

Note: This drawing is not to scale. Follow dimensions.

Sheet 38 of 43

HNTB
775 KIRK DRIVE
KANSAS CITY, MO 64105-1743
CERTIFICATE OF AUTHORITY
NO. 001270

Bartlett & West
601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 573-554-3191
CERTIFICATE OF AUTHORITY NO. 000167 - ENGINEERING



PART PLAN SHOWING PILE & DRILLED SHAFT NUMBERING FOR RECORDING AS-BUILT PILE DATA & AS-BUILT DRILLED SHAFT DATA

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			End Bent No. 1
1			
2			
3			
4			
5			
6			
7			
8			
9			

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			End Bent No. 5
10			
11			
12			
13			
14			
15			
16			
17			
18			
19			

As-Built Drilled Shaft Data				
Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	Remarks
				Intermediate Bent No. 2
1				
2				
3				
4				
				Intermediate Bent No. 3
5				
6				
7				
8				
				Intermediate Bent No. 4
9				
10				
11				
12				

AS-BUILT PILE AND DRILLED SHAFT DATA

Detailed JUN 2025
Checked JUN 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet 39 of 43

STATE OF MISSOURI
DAVID MORANDEIRA
NUMBER
PE-2021035906
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
6/9/2025

ROUTE
US-61

STATE
MO

DISTRICT
BR

SHEET NO.
A9682-39

COUNTY
ST. CHARLES

JOB NO.
JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9682

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

IMPROVE70
ALLIANCE

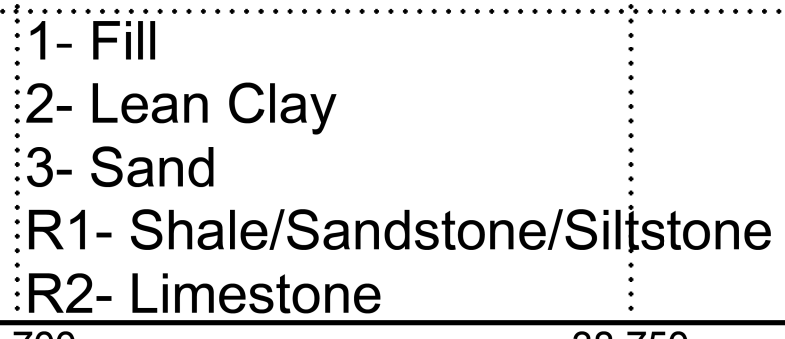
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715 KIRK DRIVE
KANSAS CITY, MO 64105-1310
PHONE 872-630-3181
FAX 872-630-3181
WWW.BARTLETTWEST.COM

Bartlett & West

601 MONROE ST., SUITE 201 - JEFFERSON CITY, MO 65101
PHONE 872-630-3181
FAX 872-630-3181
WWW.BARTLETTWEST.COM

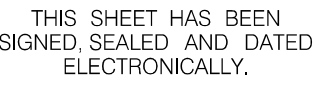
B_A9682_035_JST0020_ASBUILT.dgn 9:27:02 AM 6/9/2025



Note: This drawing is not to scale. Follow dimensions.

BORING DATA (1 OF 2)

B A9682 036 JST0020 BORDATA-1.dgn 9:27:27 AM 6/9/2025



DATE PREPARED

6/9/2025

ROUTE	STATE
101	MO

US - 61	MO
---------	----

DISTRICT	SHEET NO
DD	AGC03 4

BR	A9682-4
COUNTY	

COUNTY
ST CHARLES

ST. CHARLES
JOB NO.

IST0020

CONTRACT ID.

[illegible]

PROJECT NO.

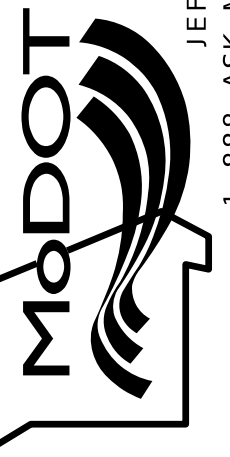
BRIDGE NO.

A9682



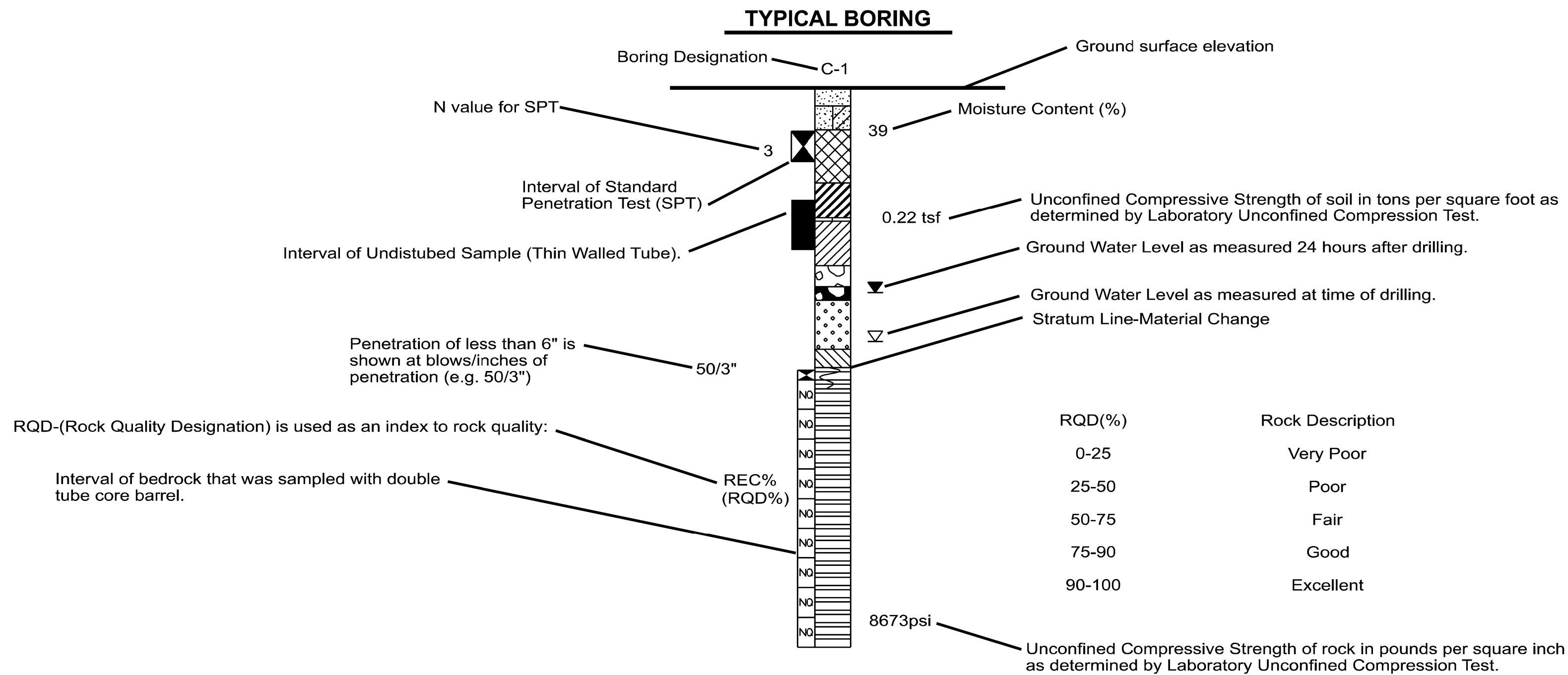
MISSOURI DEPARTMENT OF TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102



HNTB
KANSAS CITY, MO 64105-1310
CERTIFICATE OF AUTHORITY
NO. 001270

Bartlett & West
601 MONROE ST., SUITE 201 • JEFFERSON CITY, MO 65101
CERTIFICATE OF AUTHORITY NO. 00047 • ENGINEERING



KEY TO BORING LOG SYMBOLS			
LITHOLOGIC SYMBOLS <i>(Unified Soil Classification System)</i>		LITHOLOGIC SYMBOLS <i>(Rock)</i>	
	Lean Clay (CL)		Coal
	Fat Clay (CH)		Dolomite
	Lean to Fat Clay		Limestone
	Low Plastic Silt (ML)		Sandstone
	Poorly Graded Sand (SP)		Shale
	Well Graded Sand (SW)		Gypsum
	Clayey Sand (SC)		
	Poorly Graded Gravel (GP)		
	Boulders		
	Clayey Gravel (GC)		
	Gravel with Fat Clay (GP-GC)		
	Asphalt		
	Fill		
	Topsoil		
	Concrete		
	Peat		
	Poorly Graded Sand with Clayey Sand (SP-SC)		
DRILLING METHODS		SAMPLER SYMBOLS	
	Core Barrel		Auger Cuttings
	Hollow Stem Auger		Grab
	Rotary		Modified California Sampler
	Solid Stem Auger		NQ Rock Core
			NX Core Barrel
			Standard Penetration Test
			Shelby Tube
ABBREVIATIONS			
		HSA	- Hollow Stem Auger
		SSA	- Solid Stem Auger
		ST	- Shelby Tube
		SPT	- Standard Penetration Test
		NATD	- None At Time of Drilling
		N/A	- Not Available

