SEC/SUR 10 TWP 49N RGE 33W

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

See Civil Package 2: Early Grading for project reference points and project coordinate points.

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B03-07 Vertical Drain at End Bents

B03-08 Details of Intermediate Bents B03-09 Details of End Bent No. 5

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B03-21 Light Anchorage Details
B03-22 Decorative Pedestrian Fence Details
B03-23 Details of Conduit System on Structure

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B03-04 Substructure Layout B03-05 Details of End Bent No.

B03-06 Details of End Bent No.

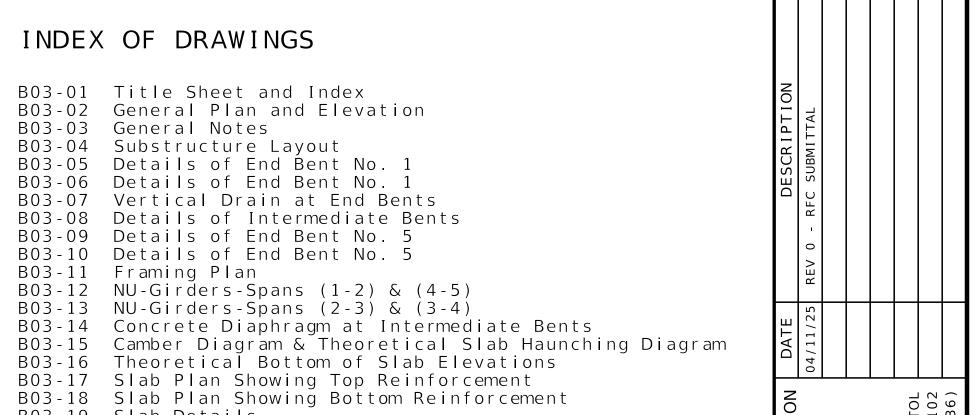
B03-10 Details of End Bent No. 5 B03-11 Framing Plan

ROBBINS PE-2017015106, 04-11-25 DATE PREPARED 04/11/2025 I - 70 SHEET NO. BR B03-01 COUNTY JACKSON JOB NO. J4I1486D CONTRACT ID.

WILLIAM **ROBBINS**

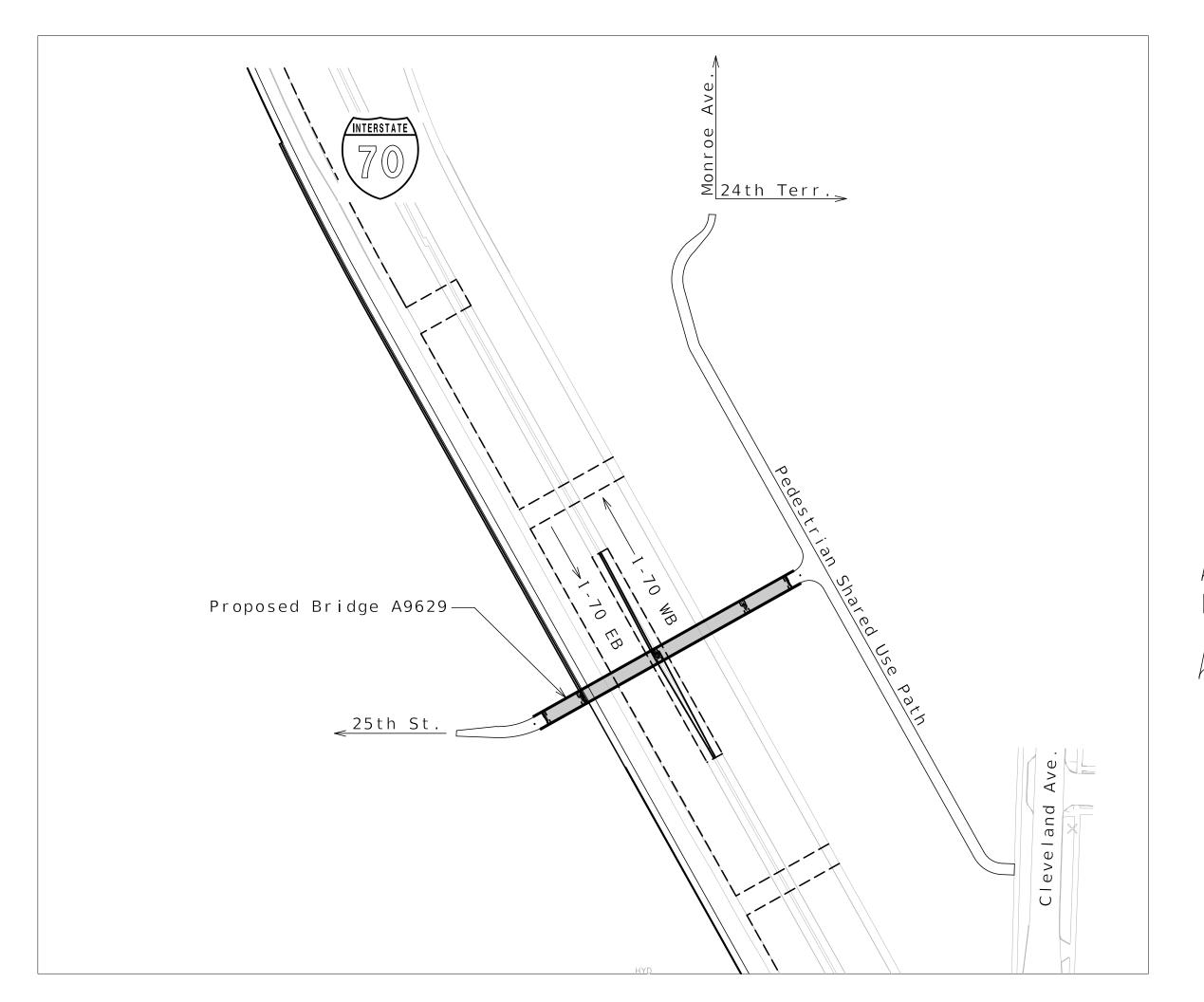
BRIDGE NO. A9629

240807-C01 PROJECT NO.









LOCATION SKETCH

Released For Construction Not to Scale

Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

BRIDGE: 25TH STREET PEDESTRIAN BRIDGE OVER ROUTE I-70

ROUTE I-70 FROM ROUTE I-670 TO ROUTE 40 ABOUT 2.4 MILES EAST OF ROUTE I-670 TIE STATION 190+65.00 (@ I-70)

Sheet No. B03-02 of B03-28

Note: This drawing is not to scale. Follow dimensions.

Checked JAN 2025

Design Specifications: 2020 AASHTO LRFD Bridge Design Specifications (9th Ed.) and 2023 AASHTO Guide Specifications for LRFD Seismic Bridge Design (3rd Edition) Seismic Design Category = A (Nonseismic) Design earthquake response spectral acceleration coefficient at 1.0 second period.S_{D1}≤0.15 Acceleration Coefficient (effective peak ground acceleration coefficient), $A_s = N/A$ 2009 AASHTO LRFD Guide Specifications for the Design of Pedestrian Bridges and 2015 Interim Revisions. Design Loading: Vehicular = H10 Truck minus lane load Pedestrian = 90 lb/sfFuture Wearing Surface = 35 lb/sf Earth - 120 lb/cf Equivalent Fluid Pressure - 45 lb/cf (Min.) Superstructure: Simply-Supported, non-composite for dead load. Continuous composite for live load. Design Unit Stresses: Člass B Concrete (End Bents below Const. Jt.) f'c = 3,000 psiClass B-1 Concrete (Intermediate Bents except Drilled Shafts) f'c = 4.000 psiClass B-2 Concrete (Drilled Shafts) f'c = 4.000 psiClass B-2 Concrete (Superstructure, except Prestressed Girders, f'c = 4,000 psiand Pedestrian Curb) Class B-1 Concrete (Pedestrian Curb) f'c = 4.000 psiReinforcing Steel (ASTM A615 Grade 60) fy = 60,000 psiStructural HP Steel Pile (ASTM A709 Grade 50) fy = 50,000 psiFor prestressed girder stresses, see Sheets No. B03-12 thru B03-13. Neoprene Pads: Neoprene Bearing Pads shall be 60 durometer and shall be in accordance with Sec 716. All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted. Reinforcing Steel:

All reinforcing in the pedestrian curb, light blisters, slab, concrete diaphragms, End Bents No. 1 and 5 and Intermediate Bents No. 2, 3 & 4 shall be epoxy coated. Reinforcing in the drilled shaft shall be uncoated.

Minimum clearance to reinforcing steel shall be 1-1/2", unless otherwise shown.

Concrete Protective Coatings: Concrete and masonry protective coating shall be applied on bridge wingwall and end face of end bent to a minimum of 18" below finished ground line in accordance with Sec 711.

Sacrificial graffiti protective coating shall be applied on bridge wingwall and end face of end bent to a minimum of 18" below finished ground line in accordance with Sec 711.

Abbreviations:

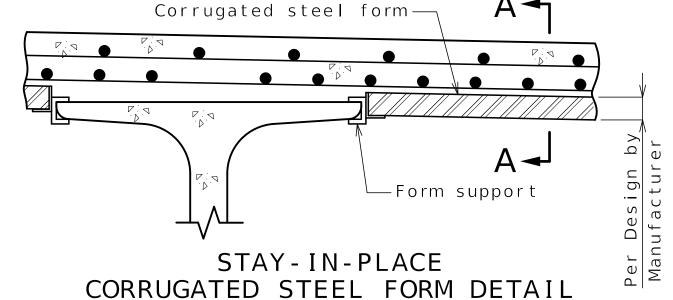
E.F. denotes Each Face N.F. denotes Near Face F.F. denotes Far face

U.N.O. denotes Unless Noted Otherwise

	F	oundation	Data			
				Bent Number		
Туре	Design Data	1	2	3	4	5
	Pile Type and Size	HP 12x53				HP 12x53
	Numb e r e	3				3
	Approximate Length Per Each f	18				18
Load	Pile Point Reinforcement e	a AII				
Bearing Pile	Min. Galvanized Penetration (Elev.) f	Full Length				Full Length
	Minimum Tip Penetration (Elev.) f	911				899
	Criteria for Min. Tip Penetration	Min. Embed.				Min. Embed.
	Pile Driving Verification Method	DT				N/A
	Resistance Factor	0.65				0.50
	Minimum Nominal Axial Compressive Resistance ki	168				218
	Numb e r e		1	1	1	
	Foundation Material		Limestone	Shale	Limestone	
	Elevation Range f	t	900-895.5**	899-882	900-899**	
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	f	19.6	4.5	19.6	
	Foundation Material		Shale	Limestone	Shale	
Drilled	Elevation Range f	t	895.5-879	882-870	899-882	
Shaft	Minimum Nominal Axial Compressive Resistance (Side Resistance) ks	f	4.9	19.6	4 . 9	
	Minimum Nominal Axial Compressive Resistance (Tip Resistance) ks	f	End bearing not required*	End bearing not required*	End bearing not required*	

Detailed DEC 2024 Checked JAN 2025

Note: This drawing is not to scale. Follow dimensions.



Date: 04/11/2025 Fill corrugation with foam (Typ.) — Bottom of slab SECTION A-A

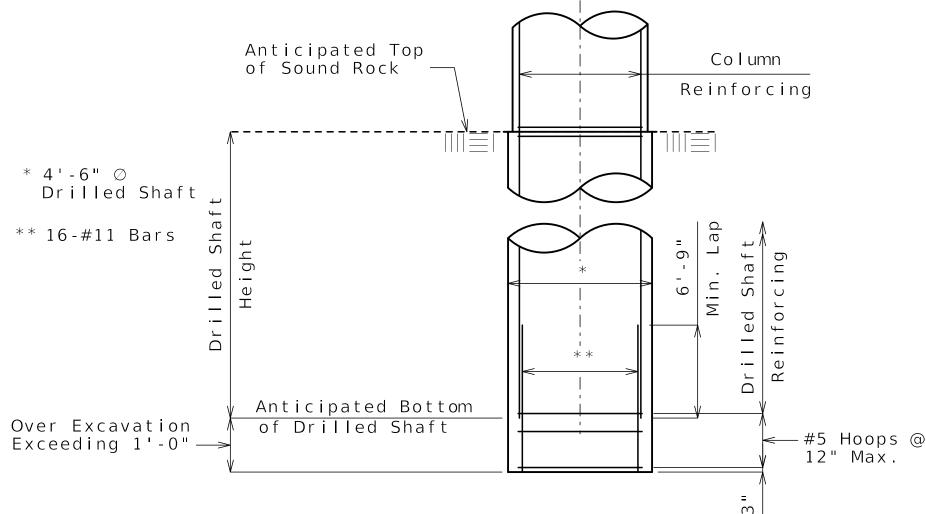
Stay-In-Place Corrugated Steel Form Notes:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.



Load Bearing Piles:

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads/Resistance Factor

Prebore for piles at End Bent No. 1 to elevation 913. End Bent No. 1 HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

Prebore for piles at End Bent No. 5 to elevation 899. At End Bent No. 5 verify the bottom of the prebore is clean and contains no loose rubble. Set pile and seat with tap of backhoe bucket or equivalent method. Prebore shall be backfilled with Class B concrete to elevation 904. Backfill remaining length of prebore per Sec 702. All piles shall be galvanized down to minimum

galvanized penetration (elevation). Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

DT = Dynamic Testing

Sheet No. B03-03 of B03-28

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Revision: 0.0 Package: BRD-03-25th_ST_PED

> 04-11-25 04/11/2025 ROUTE I - 70 DISTRICT

> > BR | B03-03 COUNTY JACKSON

DATE PREPARED

MO

SHEET NO.

VAN

WILLIAM ROBBINS

PE-2017015106 /

JOB NO. J4I1486D CONTRACT ID. 240807-C01 PROJECT NO.

BRIDGE NO. A9629

DRILLED SHAFT OVER EXCAVATION DETAIL For Drilled Shaft Details

see Intermediate Bent Details

Drilled Shafts:

Minimum Nominal Axial Compressive Resistance (Side Resistance + Tip Resistance) = Maximum Factored Loads/Resistance Factors

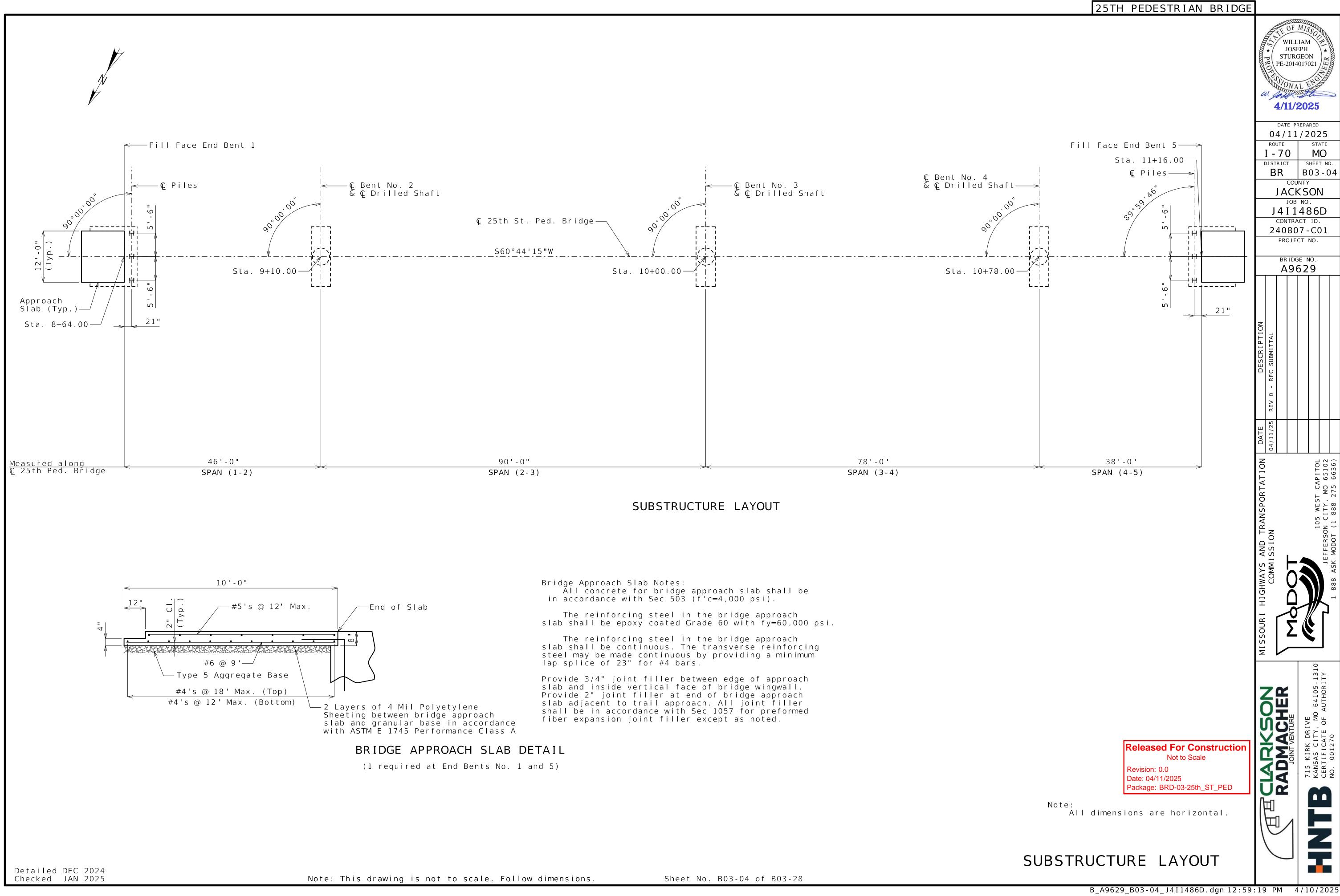
Uncased drilled shaft used for constructability. Layer 1 data shown in table is top most layer used for design and may not correspond to top of drilled shaft shown elsewhere in plans.

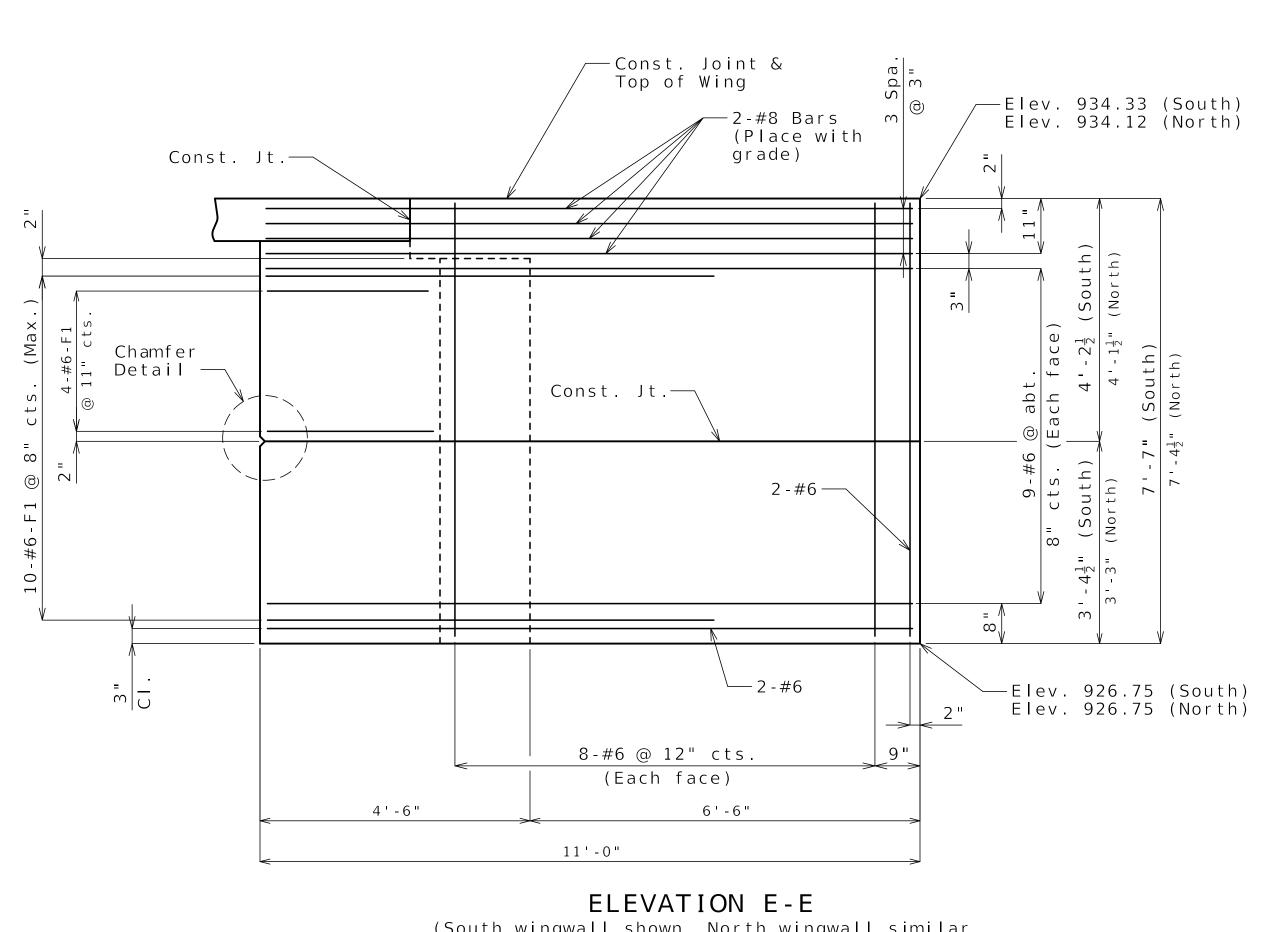
Sonic logging testing shall be performed on all drilled shafts.

Drilled shafts shall be constructed in accordance with project Drilled Shaft AAS.

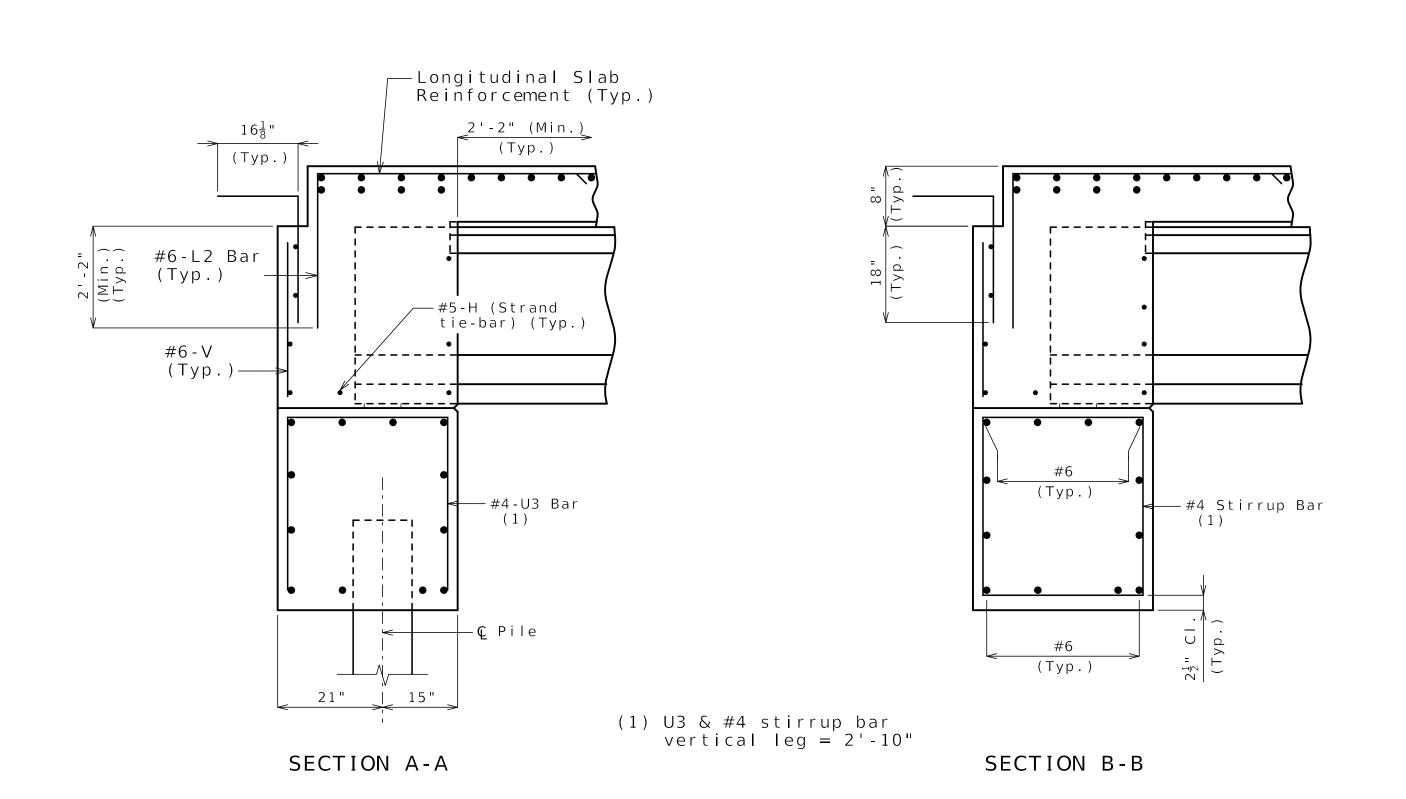
* Drilled shaft length for drilled shafts is controlled by lateral design. End bearing not required. ** Top of elevation range corresponds to 18" below future WB I-70 lane construction by others and EB I-70 lane construction with this project at Bents No. 2 and 4, respectively.

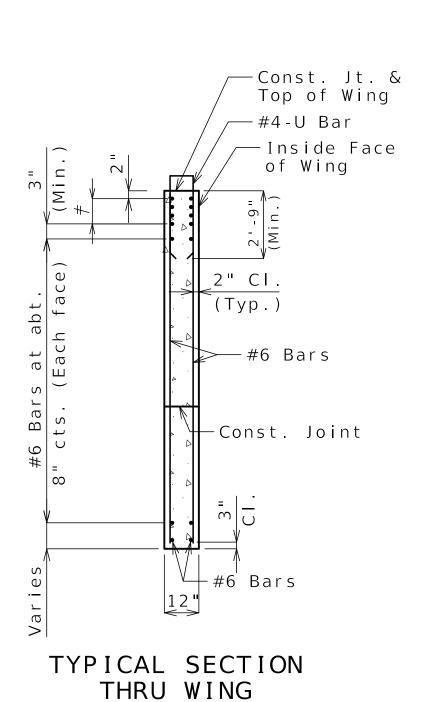
GENERAL NOTES



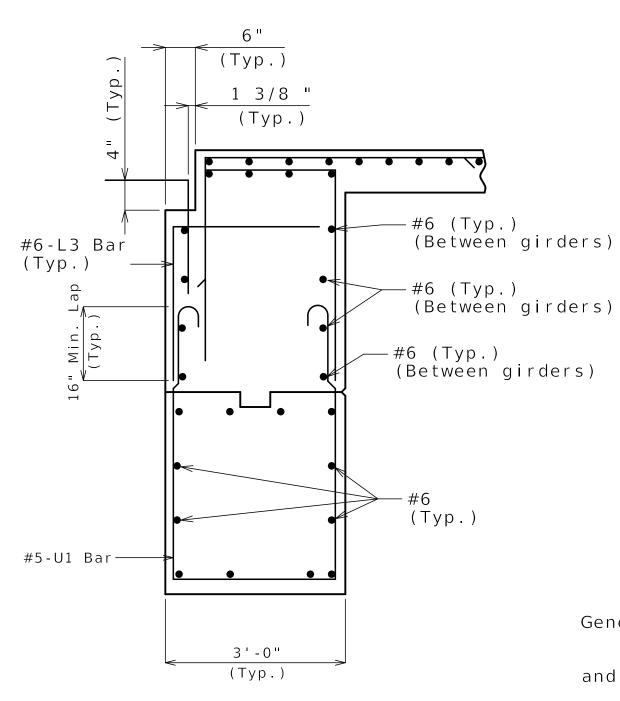


(South wingwall shown, North wingwall similar

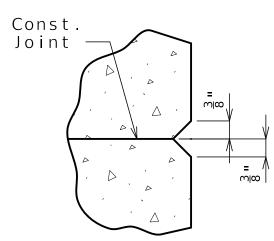


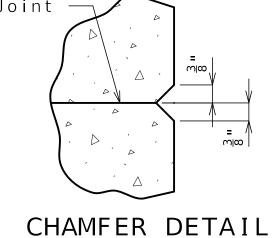


#8 Bars at 3" cts.
 (Each face)(Place with grade)
 See Elevation E-E for number of bars



SECTION C-C

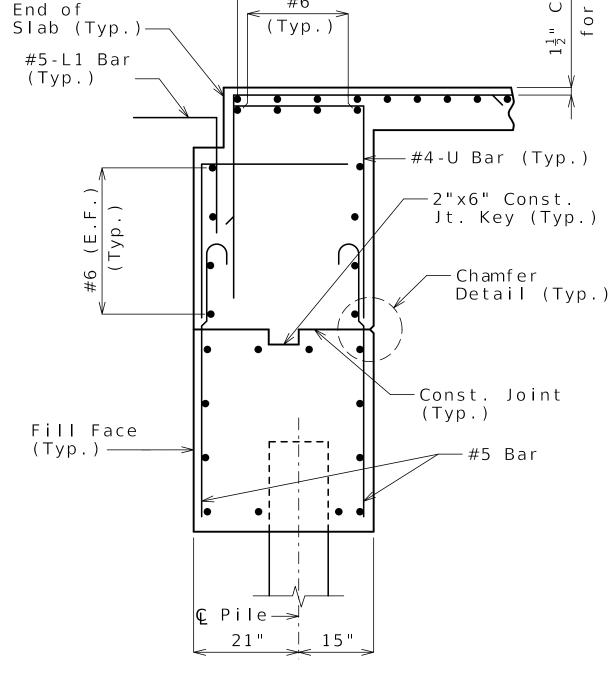




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Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

<u>Transverse Slab</u> Reinforcement (Typ.)



#6

SECTION D-D

General Notes:
Work this sheet with Sheet No. B03-05.
For location of Sections A-A, B-B, C-C and D-D and Elevation E-E, see Sheet No. B03-06.
For reinforcement of the pedestrian curb, see Sheet No. B03-20.

DETAILS OF END BENT NO. 1



DATE PREPARED 04/11/2025 I - 70 MO

SHEET NO.

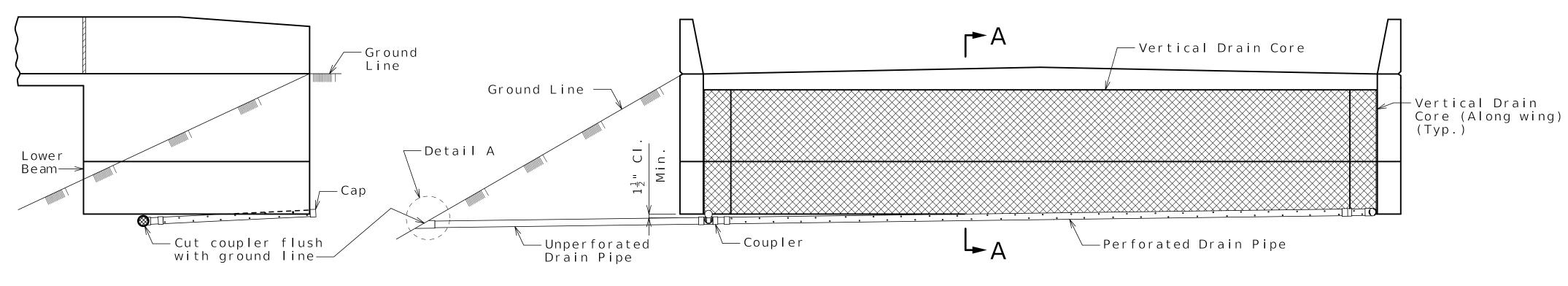
BR | B03-06 COUNTY JACKSON

DISTRICT

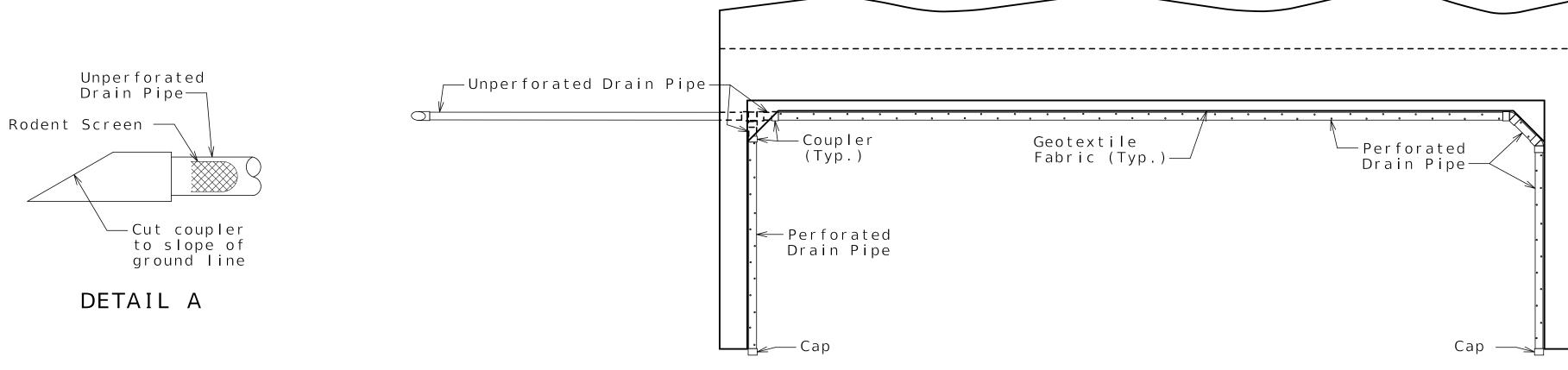
JOB NO. J4I1486D CONTRACT ID. 240807-C01 PROJECT NO.

BRIDGE NO. A9629

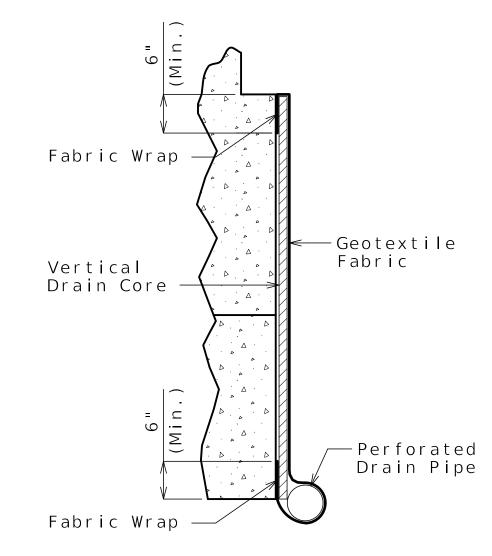
(Typ.



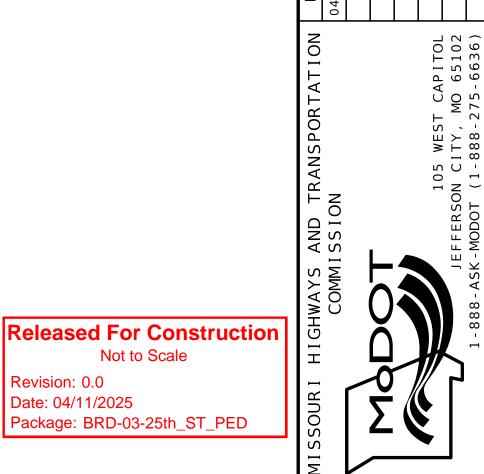
ELEVATION OF END BENT



PLAN OF END BENT



PART SECTION A-A (Section thru wing similar)



HORNER

PE-30413

DATE PREPARED

04/11/2025

BR | B03-07

COUNTY

JACKSON JOB NO.

J4I1486D CONTRACT ID.

240807-C01

PROJECT NO.

BRIDGE NO. A9629

SHEET NO.

I - 70 DISTRICT

General Notes:

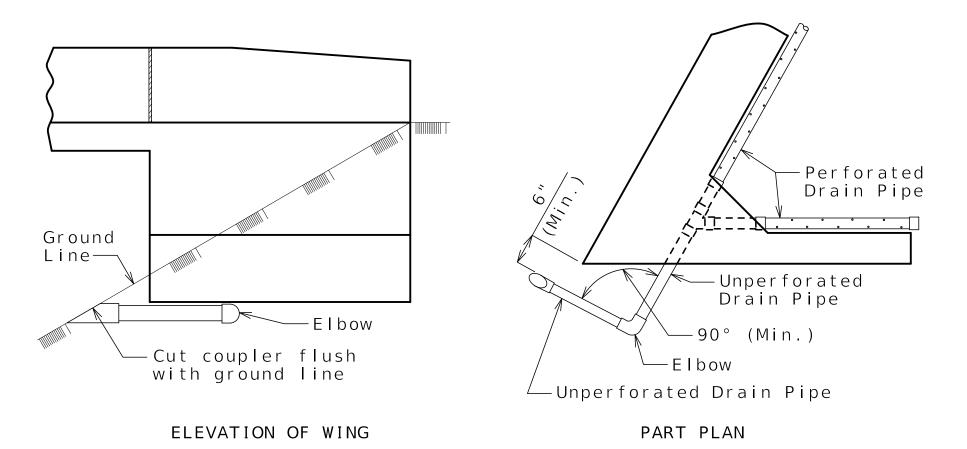
All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

VERTICAL DRAIN AT END BENTS



OPTIONAL TURNED DRAIN (Use only when straight drain is not practical.)

Detailed DEC 2024 Checked JAN 2025

ELEVATION OF WING

Not to Scale

Revision: 0.0 Date: 04/11/2025

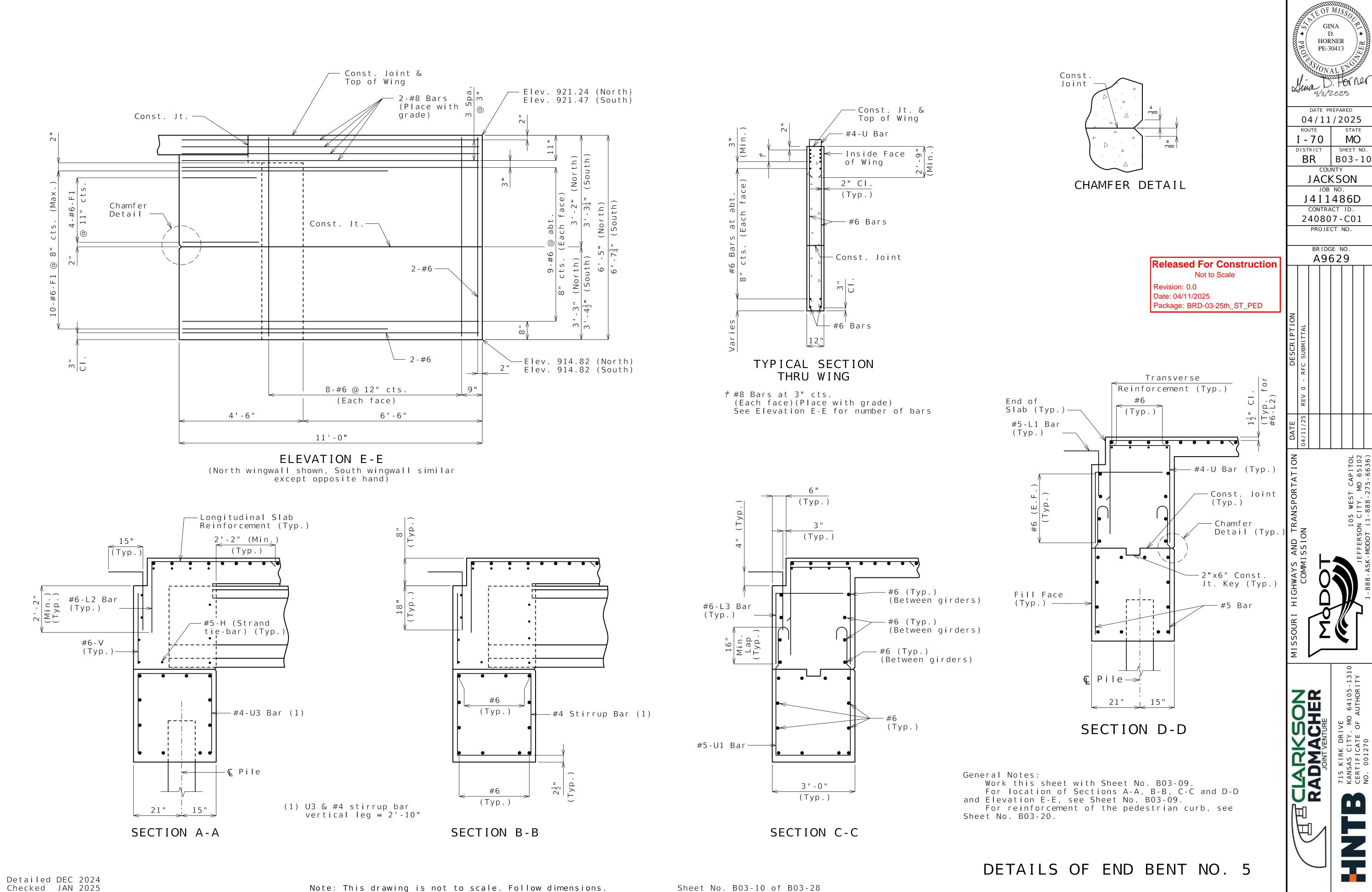
Sheet No. B03-08 of B03-28

Note: This drawing is not to scale. Follow dimensions.

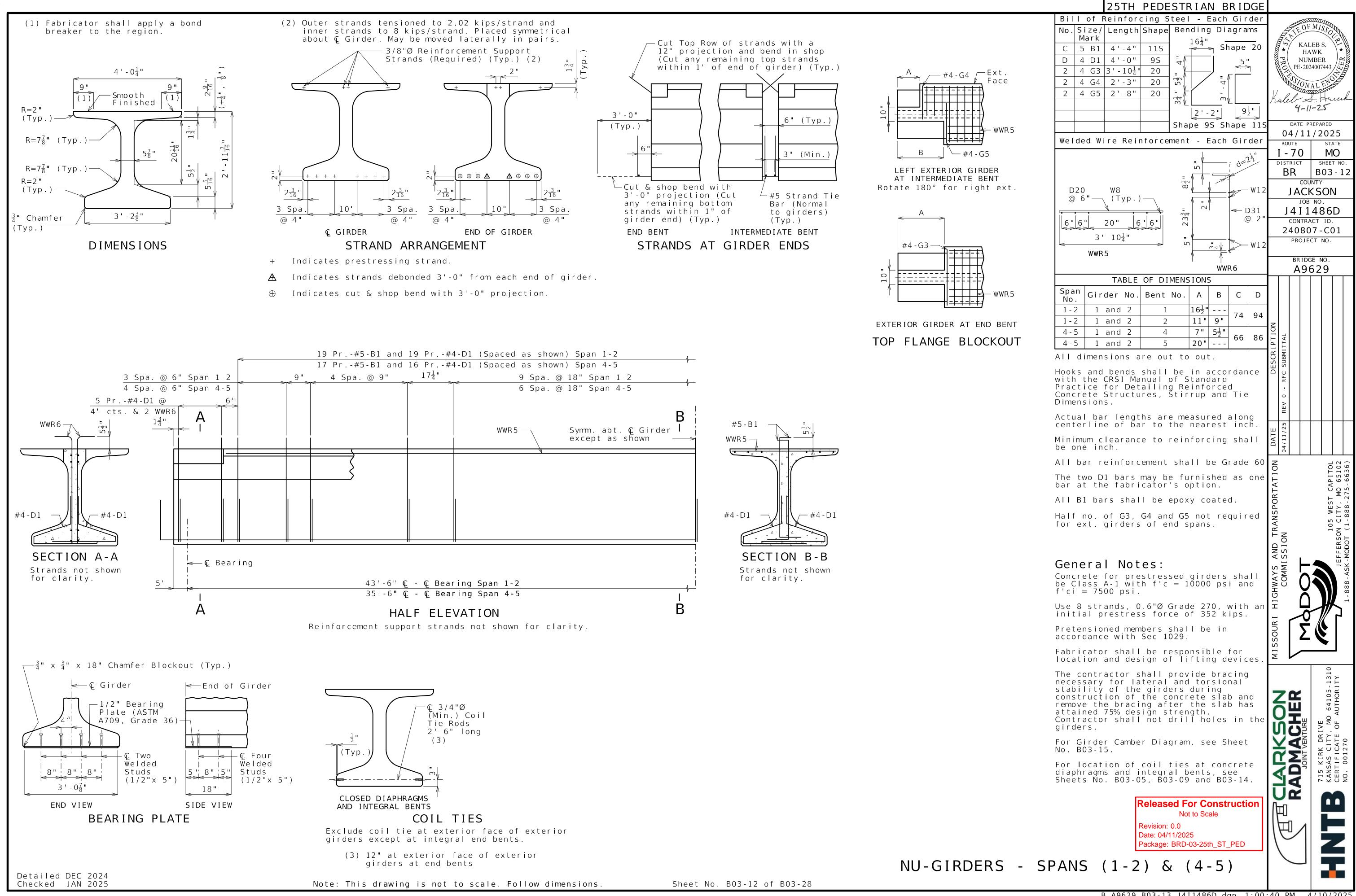
Checked JAN 2025

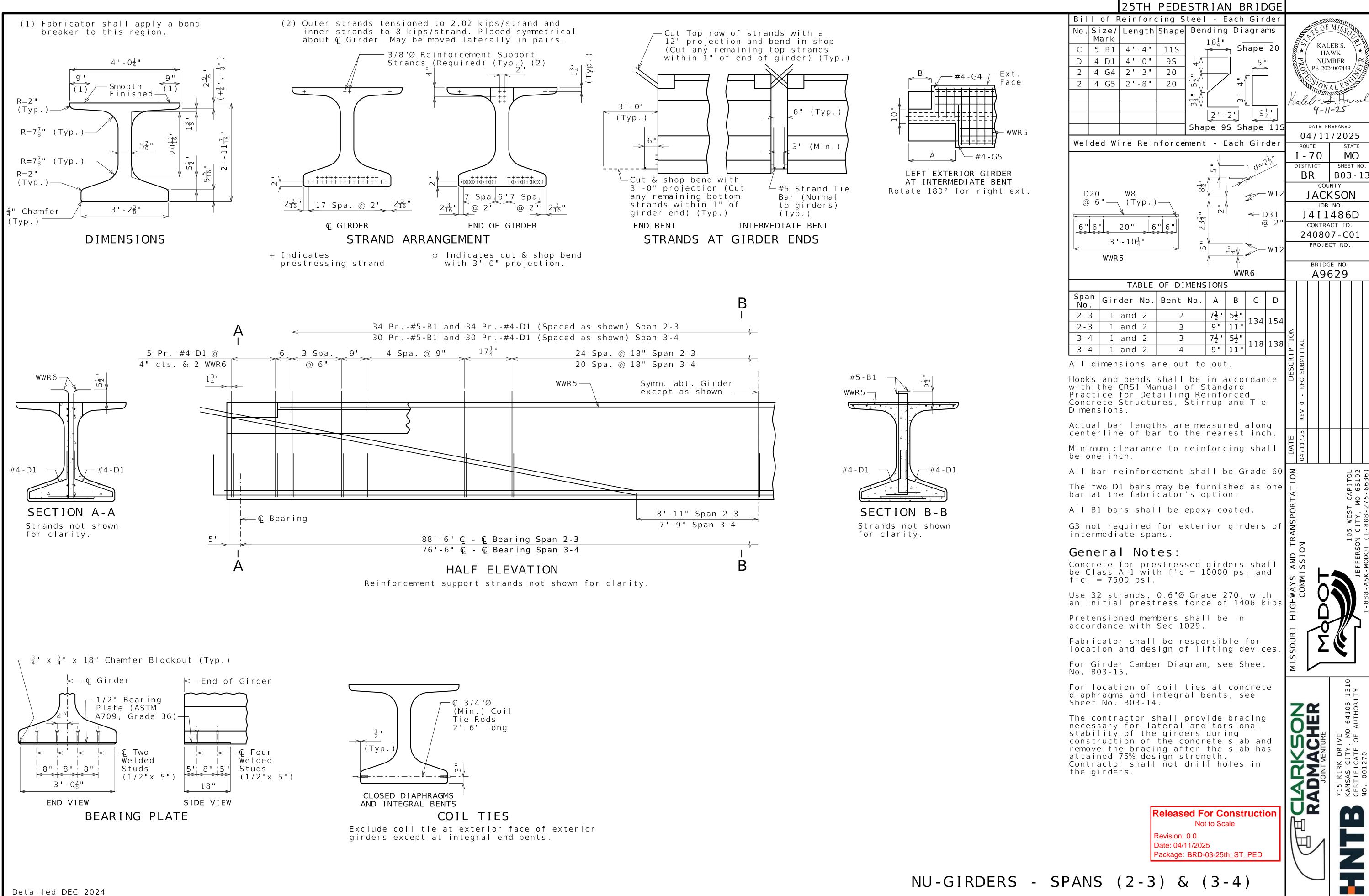
Detailed DEC 2024 Checked JAN 2025 (X) Denotes girder number

DETAILS OF END BENT NO. 5



25TH PEDESTRIAN BRIDGE KALEB S. HAWK NUMBER PE-2024007443 halel I Jank 4-11-25 DATE PREPARED 04/11/2025 ROUTE — Ç Brg. End Bent 1 ⊱ Q Int. Bent No. 3 © Int. Bent No. 4——> Ç Int. Bent No. 2 Ç Brg. End Bent 5──> I - 70 DISTRICT BR B03-11 COUNTY JACKSON JOB NO. J4I1486D _—Ç Girder No. 1 Ç 25th St. Ped. Bridge− CONTRACT ID. 240807-C01 PROJECT NO. BRIDGE NO. A9629 © Back Bearing ||| Int. Bent No. 3—>|| ← © Ahead Bearing Int. Bent No. 3 ℚ Back Bearing – Ç Ahead Bearing Int. Bent No. 2 — Ç Girder No. 2 Īnt. Bent No. 2—> \ — Fill Face of End Bent No. 1 Fill Face of End Bent No. 5— 88'-6" 76'-6" 43'-6" 35'-6" Measured along Ç SPAN (1-2) SPAN (3-4) SPAN (2-3) SPAN (4-5) FRAMING PLAN **Released For Construction** Not to Scale Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED Notes: Longitudinal dimensions shown are horizontal. All bents are parallel. FRAMING PLAN Detailed DEC 2024 Checked JAN 2025 Note: This drawing is not to scale. Follow dimensions. Sheet No. B03-11 of B03-28

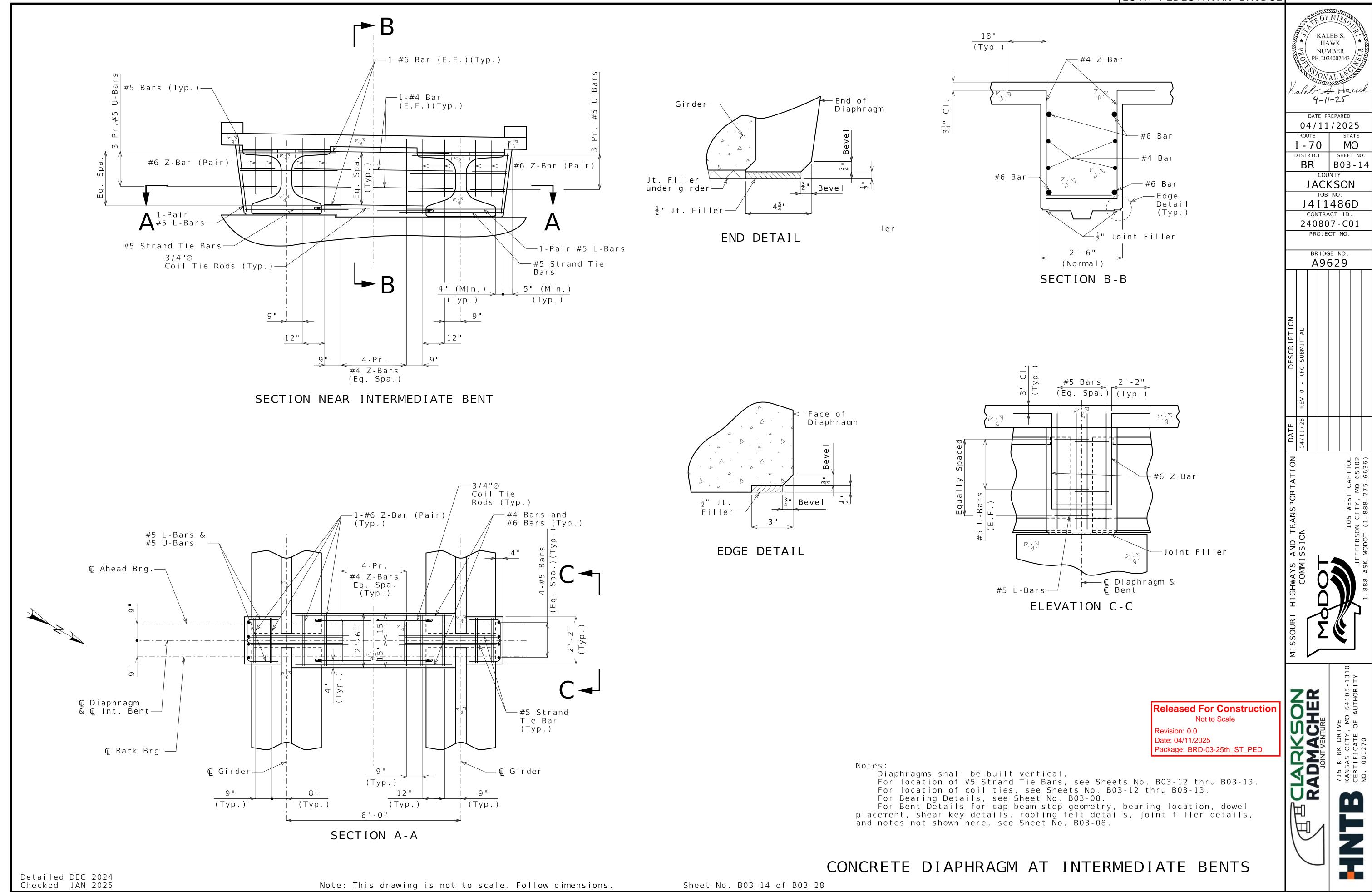


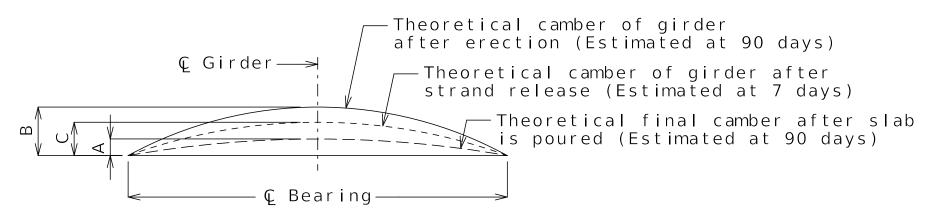


Note: This drawing is not to scale. Follow dimensions.

Checked JAN 2025

Sheet No. B03-13 of B03-28



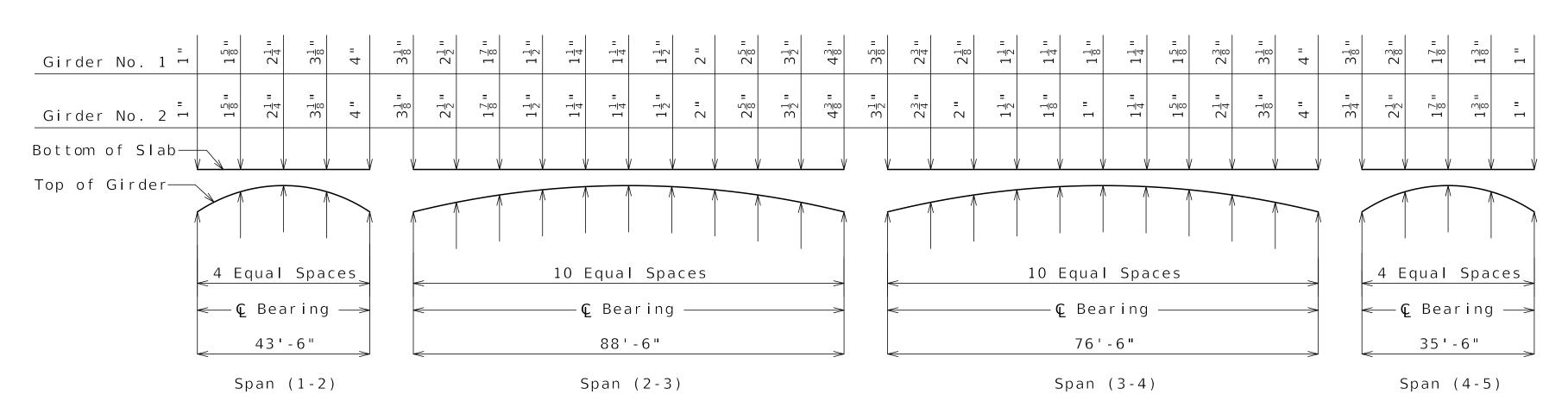


Girder	S	pan (1-2	2)	S	pan (2-3	3)	S	pan (3-	4)	Span (4-5)			
Girdei	Α	В	С	А	В	С	А	В	С	А	В	С	
1	<u>1</u> II	<u>3</u> 	<u>1</u> "	2 1 2"	4 1 "	2 7 8"	2 3 "	3 5 "	2 1 "	<u>1</u> "	<u>1</u> 11	<u>1</u> "	
2	<u>1</u> 11	<u>3</u> II	<u>1</u> "	2 1 "	4 1 "	2 7 8"	2 3 "	3 5 "	2 1 /2"	<u>1</u> "	<u>1</u> II	<u>1</u> "	

GIRDER CAMBER DIAGRAM

Conversion Factors for Girder Camber (Estimated at 90 days)

Span 2-3 & 3-4
0.1 pt. = 0.314 x 0.5 pt.
0.2 pt. = 0.593 x 0.5 pt.
0.3 pt. = 0.813 x 0.5 pt.
0.4 pt. = 0.952 x 0.5 pt.
Span 1-2 & 4-5
0.25 pt. = 0.7125 x 0.5 pt.



THEORETICAL SLAB HAUNCHING DIAGRAM

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Not to Scale

Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

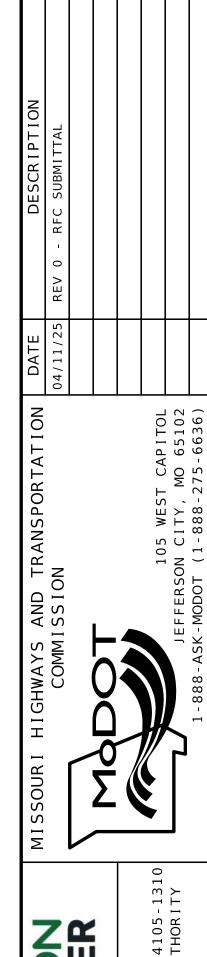
If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, adjustment of the slab haunches, or a raise in grade uniformly throughout the structure shall be necessary.

CAMBER DIAGRAM & THEORETICAL SLAB HAUNCHING DIAGRAM

Detailed DEC 2024 Checked JAN 2025

Note: This drawing is not to scale. Follow dimensions. Sheet No. B03-15 of B03-28

B_A9629_B03-16_J4I1486D.dgn 1:01:01 PM 4/10/2025



KALEB S. HAWK

NUMBER

\ PE-2024007443

4-11-25

DATE PREPARED 04/11/2025

BR B03-15
COUNTY
JACKSON

J 4 I 1486D

CONTRACT ID. 240807 - C01

PROJECT NO.

BRIDGE NO.

A9629

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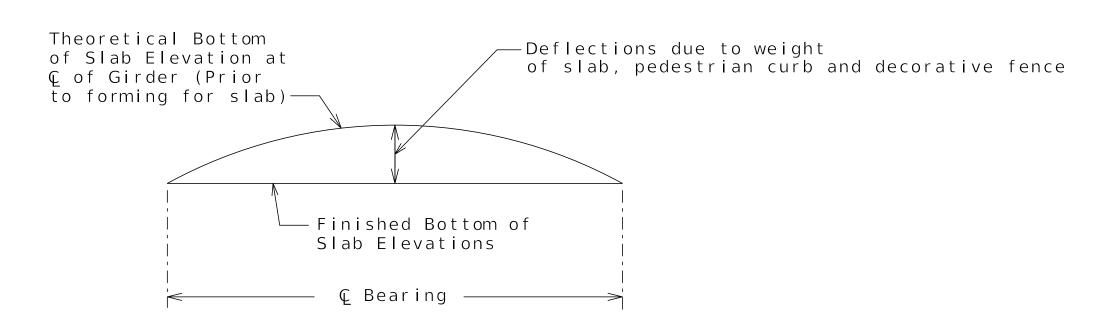
SHEET NO.

I - 70

DISTRICT

	-	Theoreti	cal Bot	tom of S	Slab Ele to forn	evations ning for	at Cen slab)*:	terline *	of Girc	ler	
Girder	S	pan (1-2) (43'-6" © Br	g QE Brg.)						
Number	€ Brg.	. 25	. 50	. 75	© Brg.						
1	933.27	932.76	932.24	931.71	931.18						
2	933.15	932.64	932.12	931.59	931.06						
Girder				S	pan (2-3) (88'-6" Ç Br	g Q Brg.)			
Number	€ Brg.	. 10	. 20	. 30	. 40	. 50	. 60	. 70	. 80	. 90	€ Brg.
1	931.11	930.73	930.35	929.95	929.55	929.13	928.70	928.25	927.80	927.33	926.86
2	930.99	930.61	930.23	929.83	929.43	929.01	928.58	928.13	927.68	927.21	926.74
Girder				S	pan (3-4) (76'-6" Ç Br	g C. Brg.)			
Number	€ Brg.	. 10	. 20	. 30	. 40	. 50	. 60	. 70	. 80	. 90	€ Brg.
1	926.79	926.45	926.10	925.75	925.40	925.03	924.66	924.29	923.90	923.51	923.12
2	926.67	926.33	925.98	925.63	925.28	924.91	924.54	924.17	923.78	923.39	923.00
Girder	S	pan (4-5) (35'-6" © Br	g C <u>E</u> Brg.)						
Number	€ Brg.	. 25	. 50	. 75	© Brg.						
1	923.05	922.62	922.20	921.77	921.34						
2	922.93	922.50	922.08	921.65	921.22						

**Elevations are based on a constant slab thickness of 6.5" and include allowance for theoretical dead load deflections due to weight of slab (including pedestrian curb and decorative pedestrian fence).



TYPICAL SLAB ELEVATIONS DIAGRAM

THEORETICAL BOTTOM OF SLAB ELEVATIONS

KALEB S.
HAWK
NUMBER
PE-2024007443

White Street Control of the Co

DATE PREPARED

04/11/2025

ROUTE STATE

I - 70 MO

DISTRICT SHEET NO.

BR B03-16

JACKSON JOB NO.

J4I1486D

CONTRACT ID.
240807 - C01

PROJECT NO.

DATE DESCRIPTION
04/11/25 REV 0 - RFC SUBMITTAL
O4/11/25 REV 0 - RFC SUBMITTAL



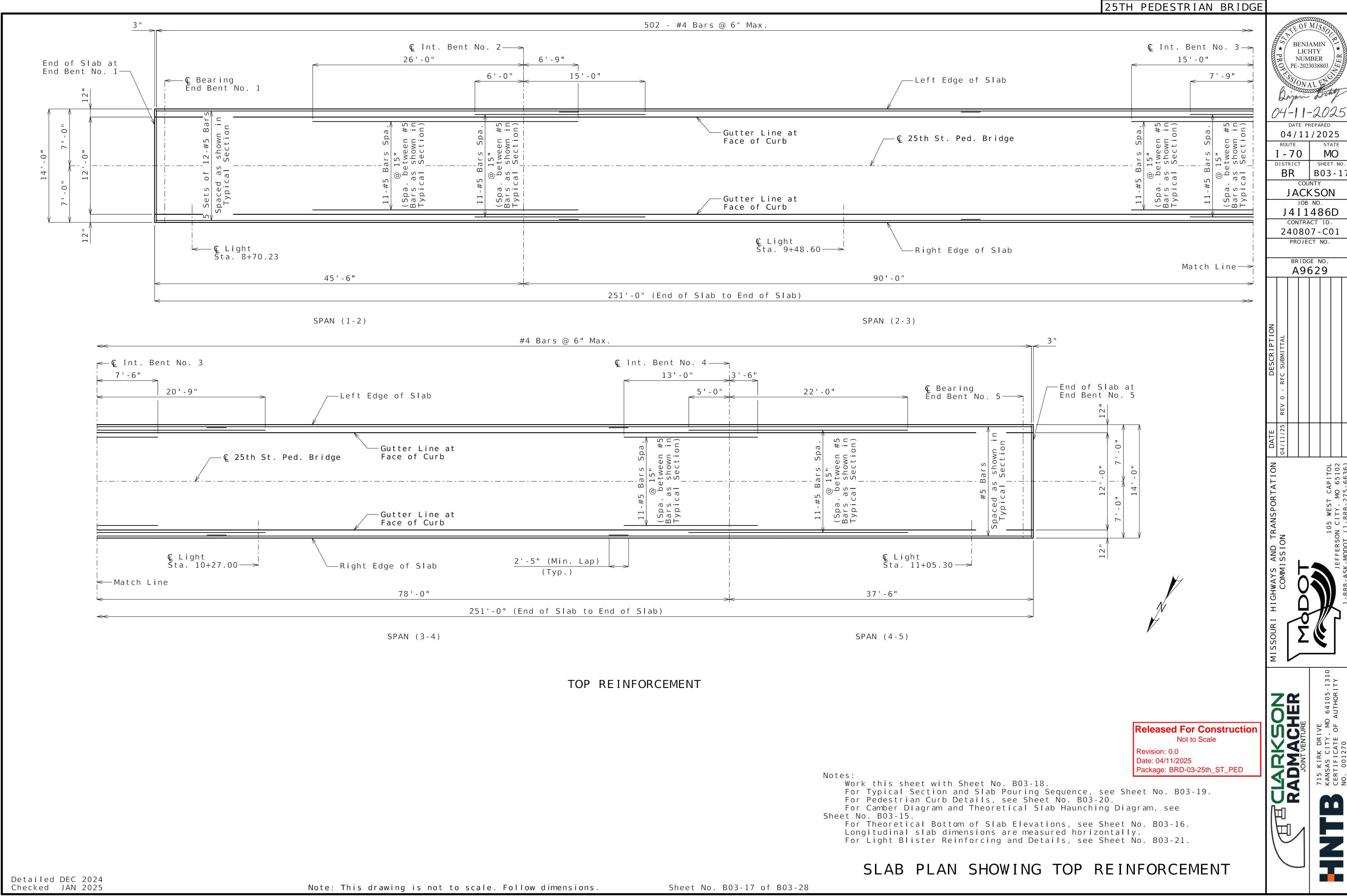
RADMACHER
JOINT VENTURE
715 KIRK DRIVE
KANSAS CITY, MO 64105-13
CERTIFICATE OF AUTHORITY

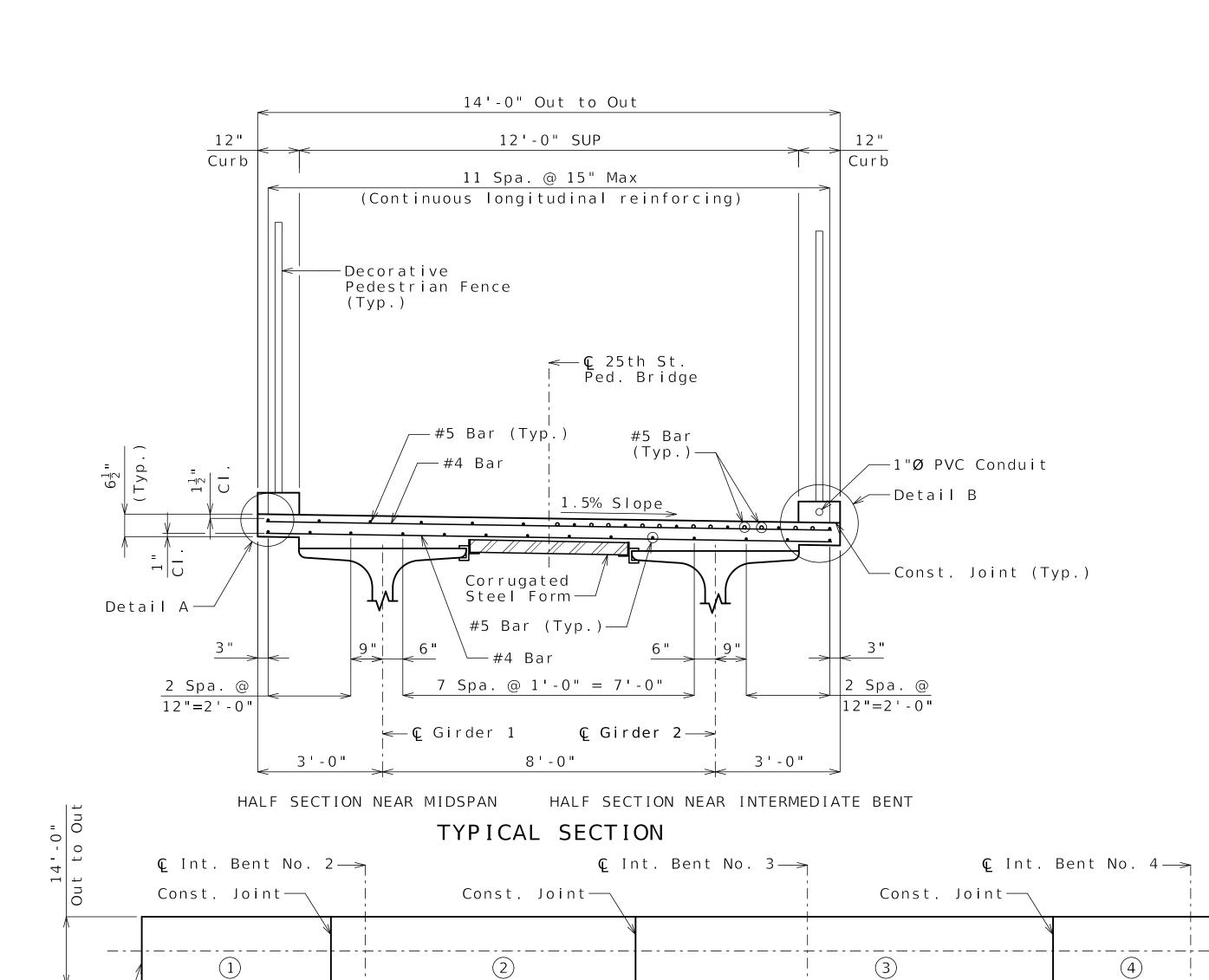
Released For Construction

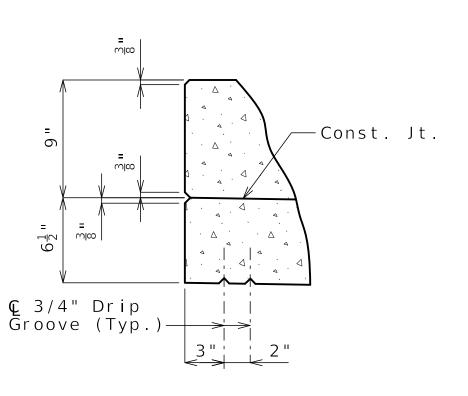
Not to Scale

Package: BRD-03-25th_ST_PED

Revision: 0.0 Date: 04/11/2025



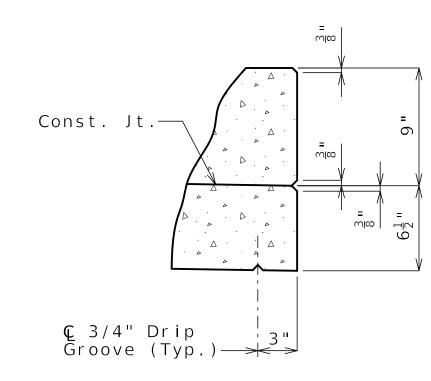




DETAIL A (High side of slab)

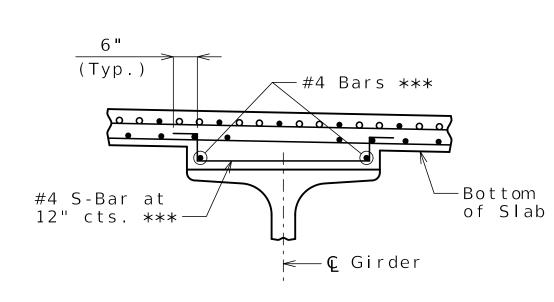
-End of Slab at End Bent No. 5

-**Ç** 25th St. Ped. Bridge



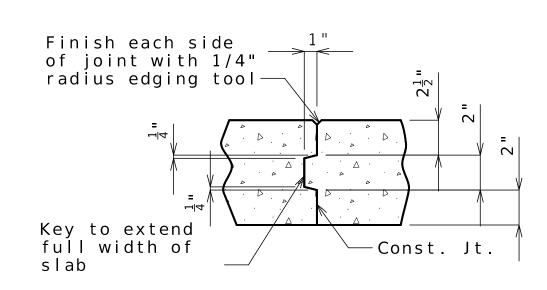
DETAIL B

(Low side of slab)



HAUNCH REINFORCING DETAIL (Prestressed Girders)

*** Contractor shall provide #4 Bars and #4 S-Bars as necessary where the haunch exceeds 4 inches measured at centerline of girder. See Theoretical Slab Haunching Diagram on Sheet No. B03-15 for haunch thickness.



SLAB CONSTRUCTION JOINT

S AND TRAN

LICHTY NUMBER
PE 2023038803

DATE PREPARED

04/11/2025

BR B03-19

COUNTY JACKSON JOB NO.

J4I1486D

CONTRACT ID.

240807-C01 PROJECT NO.

BRIDGE NO.

A9629

MO

SHEET NO.

I - 70

DISTRICT

Released For Construction Not to Scale Revision: 0.0 Date: 04/11/2025

Package: BRD-03-25th_ST_PED

For Plan of Slab Showing Top and Bottom Reinforcement, see Sheets No. B03-17 and B03-18.

For reinforcement of Pedestrian Curb not shown, see Sheet No. B03-20.

For Details of Conduit System of Structure, see Sheet No. B03-23. For Decorative Pedestrian Fence Details, see Sheet No. B03-22.

SLAB DETAILS

		Sequen	ce of Pours		Min. Rate of Pour Cu. Yds./Hr.	
		Б	Direction		With Retarder	
Basic	4	3	2	1	25	
Sequence	End to 3	4 to 2	3 to 1	2 to End	25	
Alternate A	with Sec 703. 4+3		2	1	25	
Pours	End to 2	3	to 1	2 to End	25	
Alternate B	4 +	3	2	+ 1	25	
Pours	End t	:o 2	3 t	o End	25	
Alternate C		4 + 3	+ 2 + 1		25	
Pours		Fnd	to End		25	

35'-0**"**

The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours, and shall pour and satisfactorily finish the slab pours at the rate given.

The concrete diaphragm at the intermediate bent and integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

SLAB POURING SEQUENCE

Detailed DEC 2024 Checked JAN 2025

End of Slab at End Bent No. 1 \longrightarrow 7 ' - 0 "

55'-0"

SPAN (2-3)

90'-0"

38 ' - 6 **"**

SPAN (1-2)

45'-6"

50'-0"

SPAN (3-4)

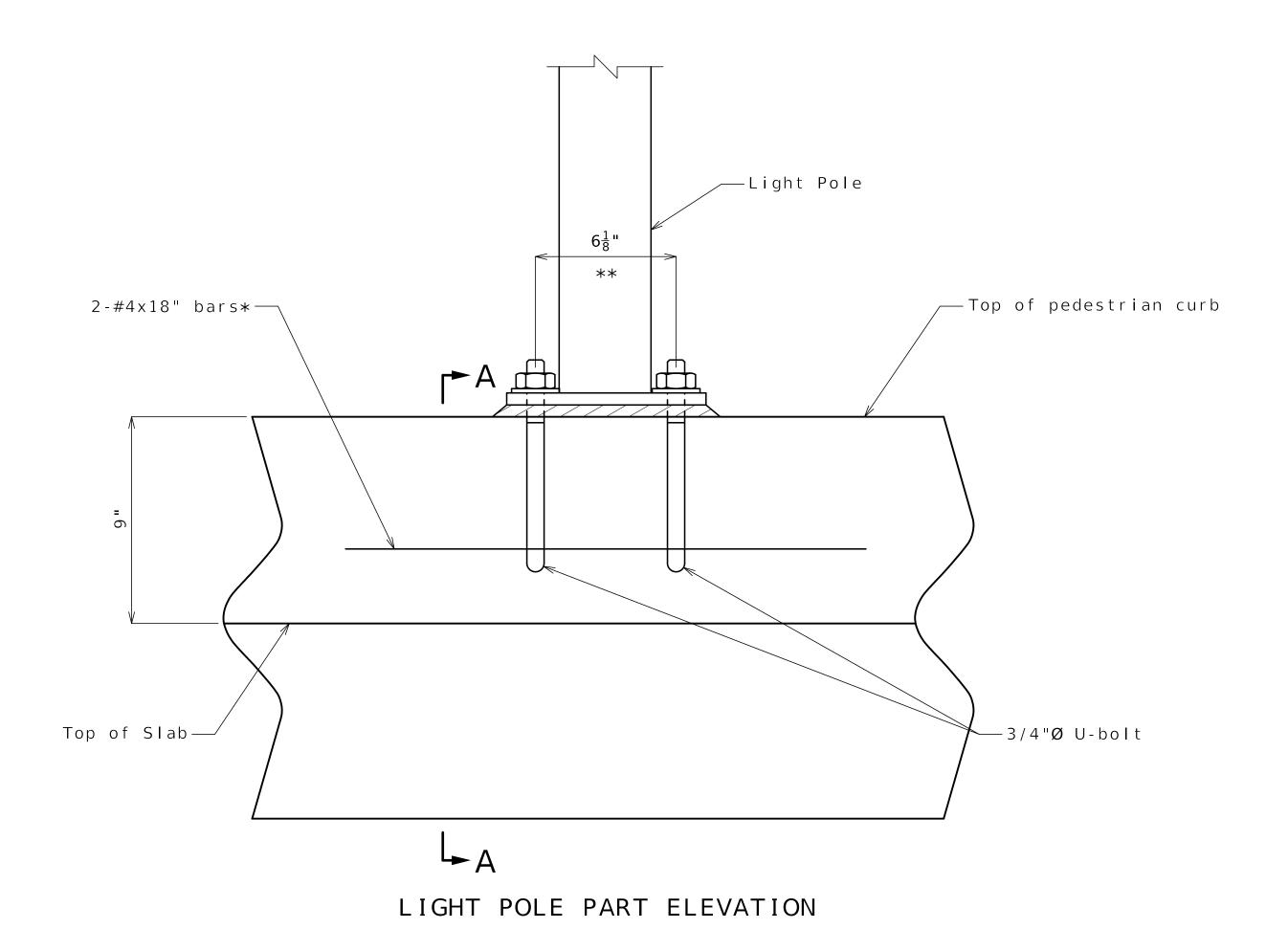
78 ' - 0 **"**

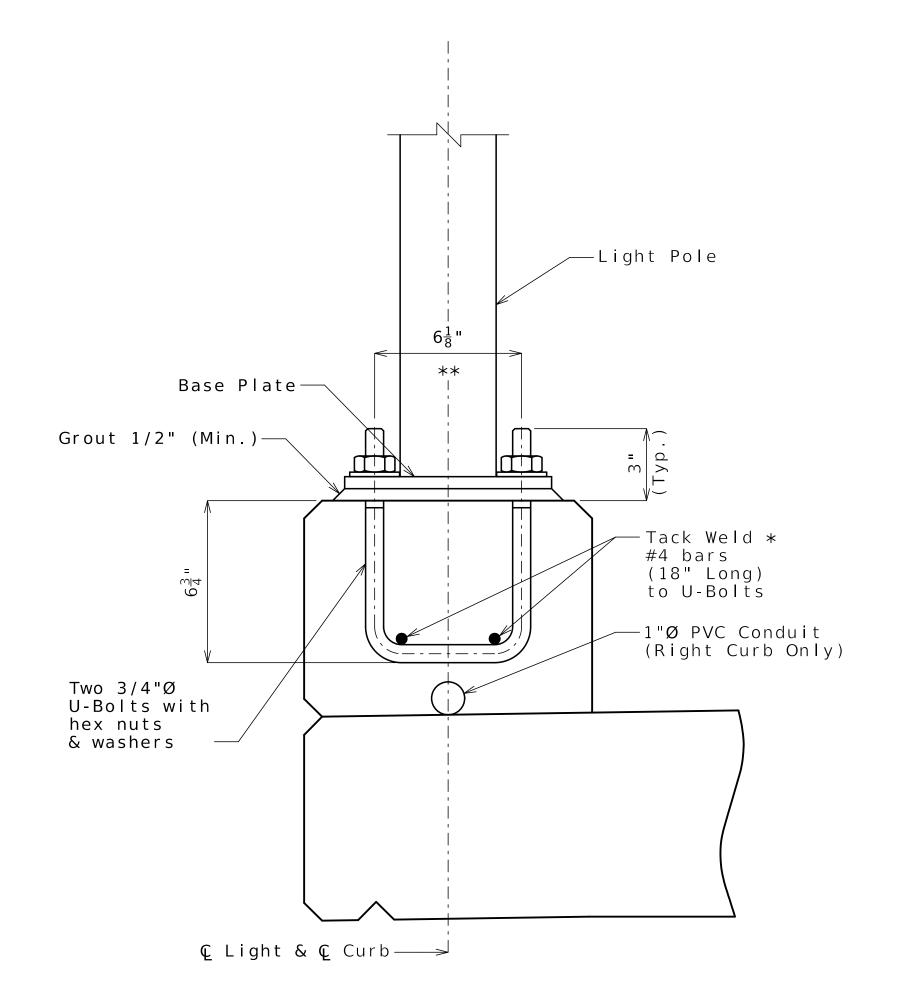
(4)

28 ' - 0 "

37**'** - 6"

SPAN (4-5)





SECTION A-A

Released For Construction Not to Scale

Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

Notes:

Base plate provided by light pole manufacturer. All base plates, U-bolts, hex nuts and washers shall be galvanized in accordance with ASTM A123 and Sec 1081.

All light poles shall be vertical. Grout shall be placed under the post base plates in

accordance with Sec 1066.

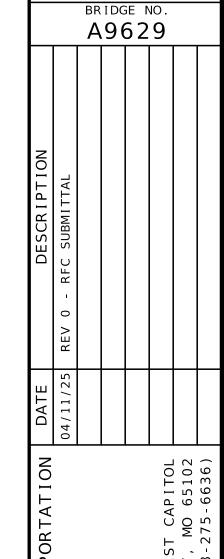
U-bolts shall be ASTM F1554 Grade 36. For locations of lights, see Sheet No. B03-17. For details of pedestrian curb, see Sheet No. B03-20. For details of decorative pedestrian fence at light locations, see Sheet No. B03-22.

Locating and pre-setting u-bolts with tack welded #4 bars prior to slab pour is recommended.

*Bars can be field cut to place around slab #4-U-bars. In this case, galvanization at cut locations shall be repaired per Sec 1081. Additional #4x18" galvanized bar shall be lapped with each cut bar.

**Dimensions based on assumed bolt circle of 8 5/8".
Contractor shall confirm dimensions with light pole supplier.

LIGHT ANCHORAGE DETAILS



LICHTY
NUMBER
PE-2023038803

DATE PREPARED 04/11/2025

> COUNTY JACKSON

JOB NO. J4I1486D

CONTRACT ID.

240807-C01 PROJECT NO.

B03-21

I - 70

Notes:

These details are a general representation of a Decorative Pedestrian Fence. The actual fence components and component positions may be different than what is shown.

Fence shall have a gloss black finish (Federal Standard #17038). See special provisions.

Base plate shall be ASTM A709, Grade 50.

All base plates, U bolts, hex nuts and washers shall be galvanized in accordance with ASTM A123 and Sec 1081.

All fence posts shall be vertical.

Grout shall be placed under the post base plates in accordance with Sec 1066.

Decorative pedestrian fencing shall be in accordance with 2020-AASHTO LRFD Bridge Design Specifications, 9th Ed.

Shop drawings and structural calculations will not be required for the decorative pedestrian fences on the Bridge Pre-qualified Products List.

All materials used in fabrication and construction of the decorative pedestrian fencing shall be in accordance with the manufacturer's specifications, except as modified in the contract documents.

Decorative pedestrian fencing system shall be supplied by only one manufacturer. Decorative pedestrian fencing system shall include all components except the anchor bolts and hardware, and #4 bars welded to the anchor bolts. The assembly of the pickets to the rails and the rails to the posts shall be the same as the style mentioned for the manufacturer.

See Bridge Pre-qualified Products List (BPPL) for a list of approved manufacturers.

Substitution for the U-bolt cages will not be permitted.

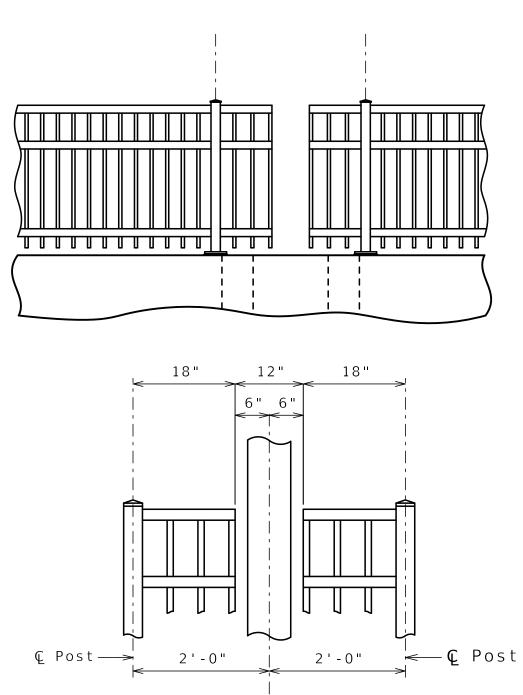
U-bolts shall be ASTM F1554 Grade 36.

For details of pedestrian curb, see Sheet No. B03-20.

Longitudinal dimensions of fence are horizontal.

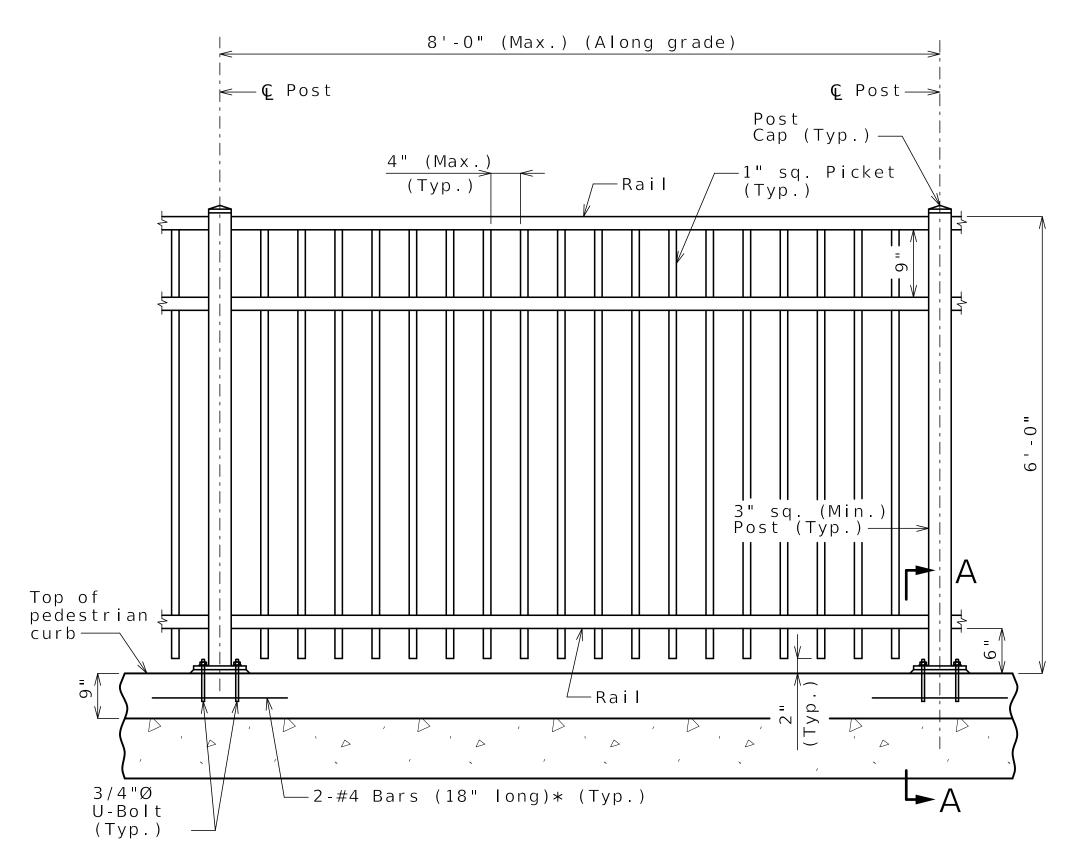
Locating and pre-setting fence post u-bolts with tack welded #4 bars prior to slab pour is recommended.

* Bars can be field cut to place around slab #4-U-bars. In this case, galvanization at cut locations shall be repaired per Sec 1081. Additional #4x18" galvanized bar shall be lapped with each cut bar.

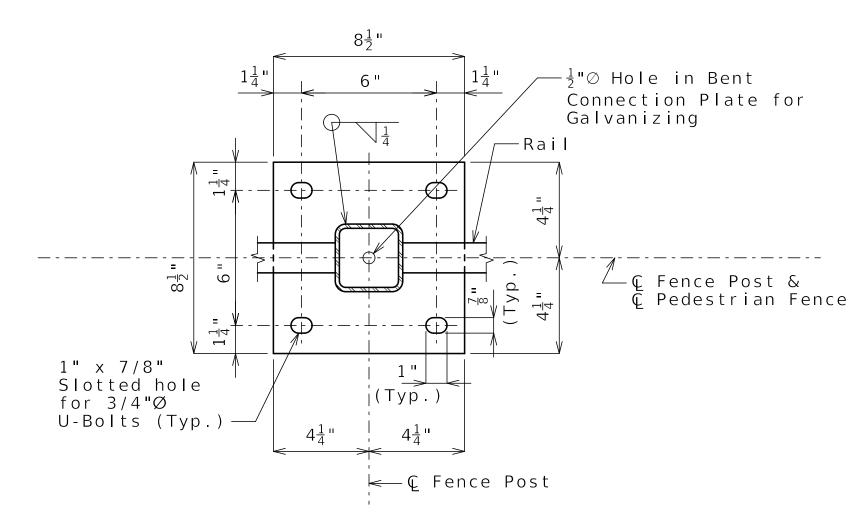


© Light Pole & Standard @ Sta. 08+70.23, 9+48.60, 10+27 00 and 11+05 30

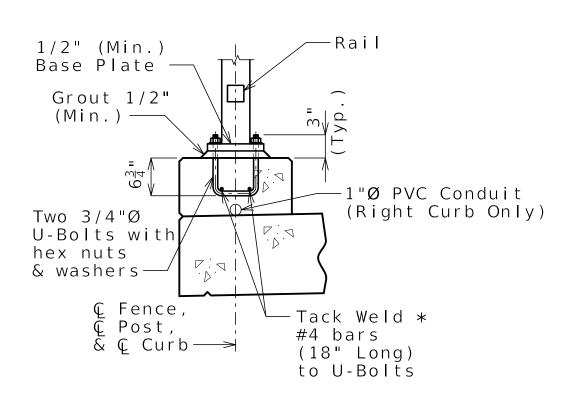
TYPICAL DETAIL OF FENCE AT LIGHT STANDARDS



TYPICAL PART ELEVATION



PART PLAN SHOWING BASE PLATE



SECTION A-A

 $\frac{1}{2}$ -inch diameter hole in base plate not shown for clarity.

> Released For Construction Not to Scale Package: BRD-03-25th_ST_PED

DATE PREPARED 04/11/2025 ROUTE I - 70 DISTRICT SHEET NO. BR B03-22 COUNTY JACKSON JOB NO. J4I1486D CONTRACT ID. 240807-C01 PROJECT NO. BRIDGE NO. A9629

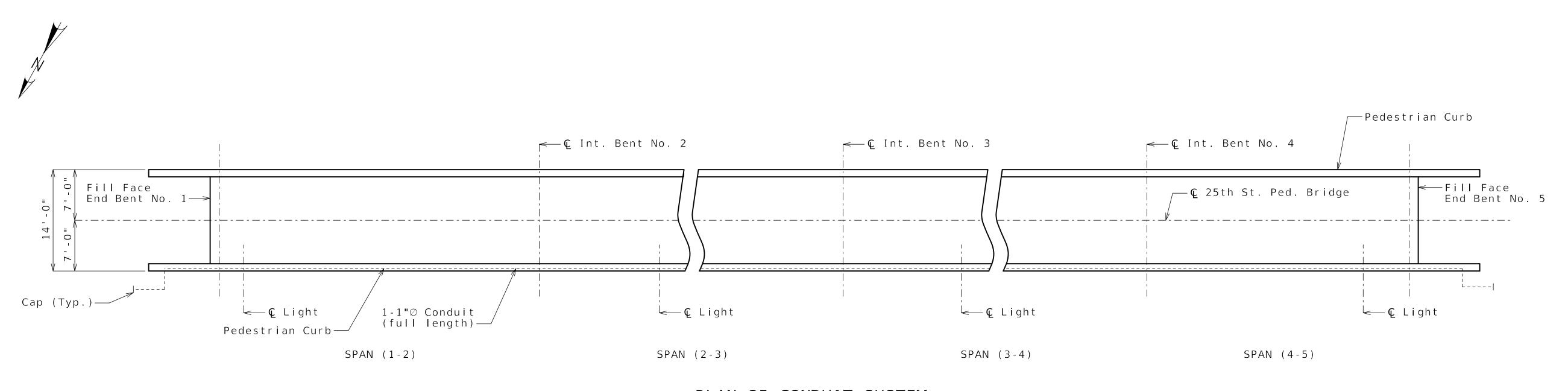
BENJAMIN

LICHTY NUMBER PE-2023038803

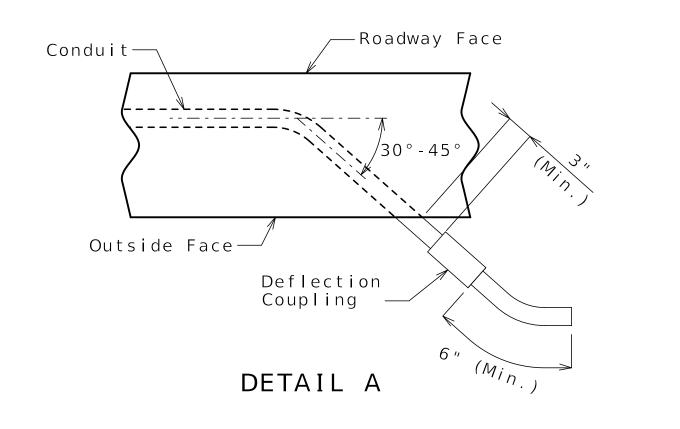
DECORATIVE PEDESTRIAN FENCE DETAILS

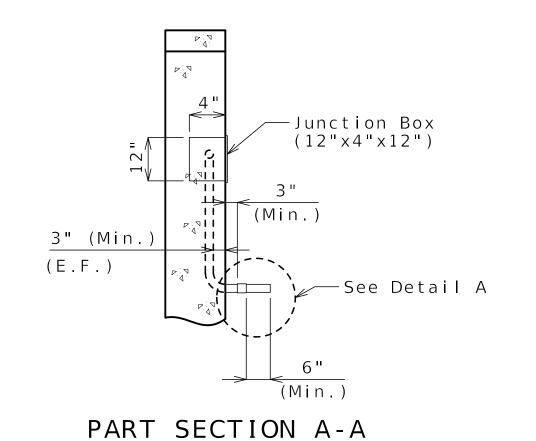
Revision: 0.0

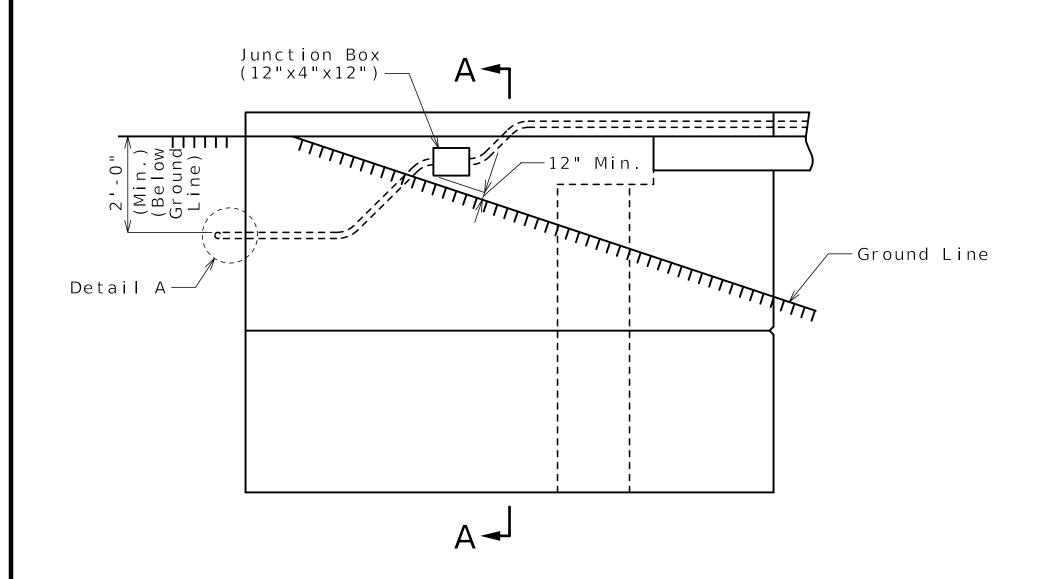
Date: 04/11/2025











PART WINGWALL ELEVATION

Not to Scale Revision: 0.0 Date: 04/11/2025

All conduit shall be rigid non-metallic schedule 40 heavy wall PVC (polyvinyl chloride plastic) with 3" minimum cover in concrete. Each section of conduit shall bear the Underwriters laboratories (UL) label. Shift reinforcing steel in field where necessary to clear conduit and junction

Expansion fittings shall be placed as shown and set in accordance with the

manufacturer's requirements and based on the air temperature at the time of setting given an estimated total movement of 1 inch at filled joints using a maximum temperature range of 120°F and a maximum temperature of 110°F.

All end bent junction boxes shall be PVC molded in accordance with Sec. 1062 and designed for flush mounting. The conduit terminations shall be permanent or separable. The terminations and covers shall be of watertight construction and shall meet requirements for NEMA 4X enclosure.

Drainage shall be provided at low points or other critical locations of all conduits and all junction boxes in accordance with Sec 707. All conduits shall be sloped to drain where possible.

DETAILS OF CONDUIT SYSTEM ON STRUCTURE

PROJECT NO.

LICHTY NUMBER PE-2023038803

		1DG 9 (9 9	
DESCRIPTION	REV 0 - RFC SUBMITTAL			
DATE	4/11/25			



Released For Construction

Package: BRD-03-25th_ST_PED

HORNER

DATE PREPARED 04/11/2025

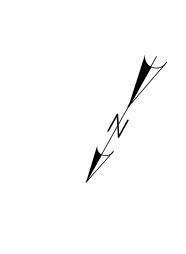
BR B03-24

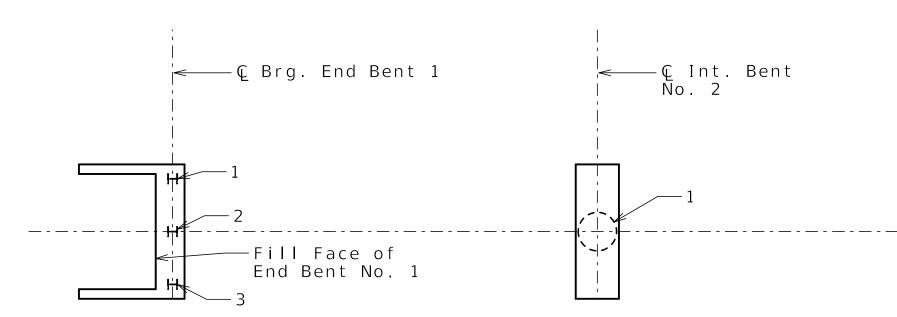
JACKSON JOB NO. J4I1486D

CONTRACT ID. 240807-C01 PROJECT NO.

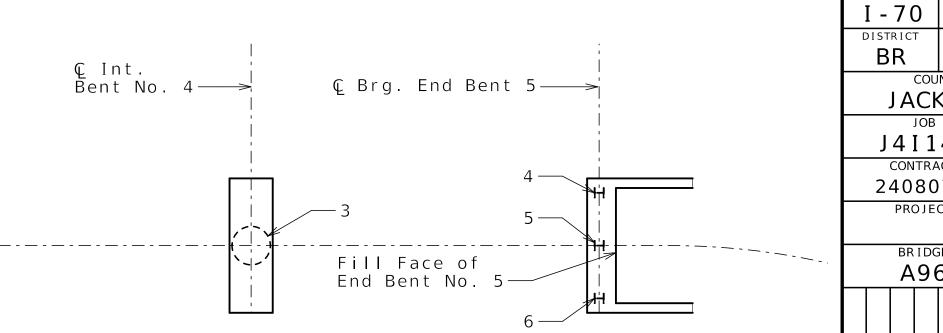
BRIDGE NO.

A9629





Ç 25th St. Ped. Bridge−



PART PLAN SHOWING PILE AND DRILLED SHAFT NUMBERING FOR RECORDING AS-BUILT PILE DATA AND AS-BUILT DRILLED SHAFT DATA

			As	s-Built P	ile Data
Pile No.	Length in Place (ft)	PDA Nom. Axial Compressive Resistance (kips)	PDA End of Drive Blow Count (blows/in.)	Actual End of Drive Blow Count (blows/in.)	R ema r k s
					End Bent No. 1
1					
2					
3					
					End Bent No. 5
4					
5					
6					

Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Drilled Shaft (Elev.)	R ema r k s
				Intermediate Bent No. 2
1				
				Intermediate Bent No. 3
2				
				Intermediate Bent No. 4
3				

As-Built Drilled Shaft Data

Note:

Indicate in remarks column: A. Pile type and grade. B. Batterí C. Driven to practical refusal
D. PDA test pile
E. Minimum tip elevation controlled
(Use when actual blow count is less than PDA blow count due to minimum tip elevation requirement. A plus sign (+) shall be placed after the PDA nominal axial compressive resistance value indicating actual value is higher than PDA value.)

Released For Construction Not to Scale Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

This sheet to be completed by design-builder.

AS-BUILT PILE AND DRILLED SHAFT DATA

				ľ	T	3					SOIL BORING	NUMBEI	R: 25		I_B ′ age 1	
PRO)JE(СТ	Impr	ove I	70 KC De	sign B	Build			NO	ORTHING/EASTING 106	61040.6 / 2777	7556.6			
DRI	LLIN	IG F	IRM	PPI		_ DRIL	LER	Josh	Stark	ey DA	TE STARTED 12/16/20	24				<u>0</u>
LOC	GGE	DΒ	Y _Za	achar	y Boyd					D/	ATE COMPLETED 12/16	/2024				
SUF	RFAC	CE E	LEV	ATIO	N 920.0'					RI	G TYPE CME-55LC					
ME	тно	D _	Wate	r Rot	ary					то	OOLING 4-1/2" Continue	ous Flight Aug	er			<u></u>
				(-							Groundwater Data			Lak)	I
	Φ			th (in)						Durin	g Drilling (ft):	N/A	ts	nt	(H)	
	Sample)e		Recovery Length	S			Pocket Pen (tsf)		2000 K 2000	Drilling (ft):	N/A	Atterberg Limits (LL-PL-PI)	Moisture Content (%)	Dry Density (PCF)	
(ft)	of St	Sample Type		ery L	Blow Counts (N-Value)	Recovery	(%	Pen	c Log		Hours (ft):	N/A	berg PL-	e O	nsity	(tsf)
Depth (ft)	Depth	mple	Sample ID	соле	ow C -Valu	Reco	RQD (%)	cket	Graphic I	Arter	110013 (11).	IN/A	tterk (LI	oistu)	у De	S (t
De	De	Sa	Sa	Re	BIG	%	RG	Po	Gre	Visi	ıal Classification and Rema	rks	⋖	W(%)	Dri	ncs
_											n, very stiff, moist, LEAN C	CLAY,				
-										organics						
<u> 1810</u>	3.5 ft			40						3.8		916.2				
_ 5		X	J-1	18	9-14-15 (29)	100		>4.5	, , , ,		t gray, medium dense, fine		49-18-31	14.9		
_										5.3 coarse gra FILL, light	brown, very stiff, moist, SII	914.7 LTY				
	1										Y, some organics					
-	8.5 ft			- 10		ļ							14			
0		X	J-2	18	6-11-16 (27)	100		>4.5					58-21-37	16.3		
-	-															
<u>-112</u>	13:5 ft					<u> </u>				13.5		906.5				
- 15		ı	J-3 C-1	22	50/1"	92	75	>4.5			, weathered, fine grained, erately hard	light				
-	15.6 ft	Н	C-2	60		100	77			3,,	,					
-		Ш	72.5.3.0.0.0													
_		Ш												0.3	165.6	1173
0		Ш								CII alavia						
_		Ц		8						20.6 - 6" clay s	Boring at 20.6'	899.4	-			
1244											ckfilled with cuttings 12/16/	/2024				
																
- 5																
-																
<u> 1978</u>																
80																
_																
Fran C																
_																
- 35																
-																
-																

			N		T	3					SOIL BORING NUM	ИВЕF	R: 25I		_ B2	
PRO	ΟJE	СТ	Imp	rove l	70 KC De	sign E	Build				NORTHING/EASTING 1061069.	6 / 2777	7612.7			<u></u>
DRI	LLII	NG	FIRM	PPI		_ DRIL	LER	Josh	Stark	еу	DATE STARTED 12/16/2024					
LO	GGE	D B	Y Z	achar	y Boyd						DATE COMPLETED 12/17/2024					
SUI	RFA	CE	ELEV	ΆΤΙΟ	N 912.2'						RIG TYPE CME-55LC					
ME	THO	OD	Wate	ery Ro	otary						TOOLING 4-1/2" Continuous Fli	ght Aug	er			
		<u> </u>			-					1	, , , , , , , , , , , , , , , , , , ,	1		Lak)	
				(in)							Groundwater Data	4				
	ble	6		gth				sf)			During Drilling (ft): N/A		mits (tent	PCF)	
6 <u>2</u> 8	Sample	Type		Len	nts	<u>></u>		en (t	Log		After Drilling (ft): N/A		Atterberg Limits (LL-PL-PI)	Con	ity (F	
(£	οţ			very	Cou	Recovery	(%)	et Pe	Jic L		After Hours (ft): N/A		rber LL-P	nre	ensi	(tsf)
Depth (ft)	Depth	Sample	Sample ID	Recovery Length	Blow Counts (N-Value)	% Rec	RQD (%)	Pocket Pen (tsf)	Graphic		Visual Classification and Remarks		Atte ()	Moisture Content (%)	Dry Density (PCF)	ncs
											FILL, very stiff, brown, moist, SILTY LEAN CLAY, organics				_	
-	3.3 ft	t		7 10/05/24	1700 LE 1704-1705-1705	an-hadded at		Vid. Introduct								
- 5	4.5 ft		J-1	12	5-8-50/2" (58)	86		>4.5		4.4		907.8	47-19-28	15.1		
3	5 ft	Н	C-1 C-2	6 60		100	100 72	1			Limestone, weathered, gray, moderately har some fossilization	d,				
1.		Ш									301116 10331112411011					
		Ш														
12		Ш												0.3	165.2	1634
10	10 ft				2											
15		Ш	C-3	60		100	80				- 6" clay seam at 10.3'					
9	l	Ш													31	
-		Ш								13.5		898.7		0.2	169.9	1736
- 15	15 64	8									Shale, highly weathered, dark gray, soft,					
10	15 ft	П	C-4	60		100	35				vuggy					
-	1	н														
		Ш									- becomes weathered at 17.4'			9.1	137.7	32
		ш									becomes weathered at 17.4					
20	20 ft	Ш														
8 -		ш	C-5	60		100	35									
-	ł	ш														
-		Ш														
- 25	l															
2.0	25 ft		C-6	60		100	47	1								
	1	Ш												11.0	133.5	20
\$1 1	1	Ш														
63		Ш														
30	30 ft	L								30.0	- becomes green-gray at 29'	882.2				
13 1 -			C-7	60		100	83				Limestone, weathered, gray, vuggy					
Ja lan										32.0		880.2				
P											Limestone, slightly weathered to fresh, gray hard, moist, slight green color 32-33'	ı				
_														0.2	166.7	1051
35 _	35 ft		C-8	60		100	77				- green shale seams from 35.2-36.2'					,,,,,,,
19			(v	

Released For Construction Not to Scale Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

Notes: For locations of borings, see Sheet No. B03-02 and Geotechnical Report.

BORING LOGS

WILLIAM
JOSEPH
STURGEON
PE-2014017021 4/11/2025 DATE PREPARED

04/11/2025 I - 70 MO BR | B03-25 COUNTY JACKSON JOB NO. J4I1486D CONTRACT ID. 240807-C01

PROJECT NO.

			5E N		
DESCRIPTION	:V 0 - RFC SUBMITTAL				



WILLIAM
JOSEPH
STURGEON
PE-2014017021

4/11/2025

DATE PREPARED 04/11/2025

I - 70 MO

COUNTY JACKSON JOB NO. J4I1486D CONTRACT ID. 240807-C01 PROJECT NO.

> BRIDGE NO. A9629

BR B03-26

	DJE(Impi		70 KC De	3 sign B	Build			SOIL BORING NUMBER: 25Ped_B2_1 Page 2 of 2 NORTHING/EASTING 1061069.6 / 2777612.7
								Josh	Stark	
LOC	GE	DΒ	Y <u>Z</u> a	achar	y Boyd					DATE COMPLETED 12/17/2024
SUF	RFAC	CE	ELEV	ATIO	N 912.2'					RIG TYPE CME-55LC
ME	THO	D _	Wate	ry Ro	otary		1	1		TOOLING 4-1/2" Continuous Flight Auger
	ple			gth (in)				(tsf)		Groundwater Data During Drilling (ft): After Drilling (ft): N/A Lab Lab Lab Lab Lab
(Sample	ype	۵	Recovery Length	unts)	ery		en (t	Log	During Drilling (ft): After Drilling (ft): N/A After Limits N/A N/A Visual Classification and Remarks N/A Visual Classification and Remarks
Depth (ft)	of	ample Type	Sample ID	over)	Blow Counts (N-Value)	Recovery	RQD (%)	Pocket Pen	Graphic I	After Drilling (tt): N/A After — Hours (tt): N/A Obassity Oba
Dep	Depth	Sam	Sam	Rec	Blov (N-	% R	RQD	Pocl	Grap	Visual Classification and Remarks Visual Classification and Remarks
- 40			C-8	60		100				Limestone, slightly weathered to fresh, gray, hard, moist, slight green color 32-33'
40 - -										Bottom of Boring at 40' Boring backfilled with cuttings 12/17/2024
- 45 -										
- - 50 -										
- 55 -										
60										
- 65 -										
- - 70 -										
-										

										Page 1 of 2
ORI .OC	LLIN GGE RFA	OB DB	FIRM Y Pr ELEV	PPI adip	Adhikari 902.5'*	_ DRII		David	d Aller	DATE COMPLETED 12/10/2024 RIG TYPE CME-55
ΛE	THC) П	wate	er Rot	tary					TOOLING 4-1/2" Continuous Flight Auger Lab
Depth (rt)	Depth of Sample	Sample Type	Sample ID	Recovery Length (in)	Blow Counts (N-Value)	% Recovery	RQD (%)	Pocket Pen (tsf)	Graphic Log	Groundwater Data During Drilling (ft): After Drilling (ft): After _ Hours (ft): Visual Classification and Remarks During Drilling (ft): V/A Visual Classification and Remarks
	3.5 ft									0.4 ASPHALT 902.1 1.1 Base 901.4 FILL, FILL, gray and brown, moist, LEAN CLAY 3.5 899
5	3.510	X	J-1	12	13-27-50/5" (77/11")	67		>4.5		Shale, medium bedded, weathered, gray, moderately hard
)	8.5 ft									
0 -			J-2	5.5	50/5.5"	100		>4.5		
_ 5 _	13.5 ft		J 3 C-1		50/0" ()	100	60			11.60 124.70 16
_ _ _ _	<u>18.5 ft</u>		C-2			100	86			Limey shale, thinly bedded, weathered, gray, 20.5 moderately hard 884 Limestone modium hadded slightly weathered
_ _ _ 5	23.5 ft		C-3			100	88			Limestone, medium bedded, slightly weathered, very fine grained, light gray, moderately hard
2	28.5 ft		C-4			100	91			
<u> </u>										32.5 870 Shale modium hodded weathered gray
- 5	33.5 ft		C-5			100	51			Shale, medium bedded, weathered, gray, moderately hard

Released For Construction Not to Scale Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

Notes: For locations of borings, see Sheet No. B03-02 and Geotechnical Report.

BORING LOGS

Detailed DEC 2024 Checked JAN 2025

					70 KC De			David	d Aller	NORTHING/EASTING 1061103.2 / 2777688.1 DATE STARTED 12/10/2024
LOGGED BY Pradip Adhikari SURFACE ELEVATION 902.5'*										DATE COMPLETED 12/10/2024
SUI	RFAC	CE E	ELEV	ATIOI	N 902.5'	•				RIG TYPE CME-55
ME	THO	D _	Wate	r Rot	ary	T		-		TOOLING 4-1/2" Continuous Flight Auger
				(in)						Groundwater Data
	ple			gth (i				sf)		During Drilling (ft): N/A ighthat is a few of the control of the
_	Sample	ype		Recovery Length	nts	l y		Pocket Pen (tsf)	Log	During Drilling (ft): After Drilling (ft): N/A After Lours (ft): N/A Visual Classification and Remarks N/A Visual Classification and Remarks
Depth (ft)	of	Sample Type	Sample ID	very	, Cou alue)	% Recovery	(%)	et Pe	hic L	After _ Hours (ft): N/A
Dept	Depth	Sam	Sam	Reco	Blow Counts (N-Value)	% Re	RQD (%)	Pock	Graphic	During Drilling (ft): After Drilling (ft): N/A After _ Hours (ft): N/A Visual Classification and Remarks N/A Visual Classification and Remarks
_	37.5 ft 38.5 ft		C-5			100				Shale, medium bedded, weathered, gray,
- 40		Ш	C-6			100	96			38.5 moderately hard 864 Light gray
9-		Ш								
-		Ш								
_		Н								Bottom of Boring at 43.5'
45_										Boring backfilled with cuttings and patched with asphalt 12/10/2024
-										With asphalt 12/10/2024
_										
- 50										
_										
-										
55										
-										
-										
-										
60										
£										
_										
- 65										
65 -										
-										
(° <u></u>										
- 70										
-										
-	1	1								

Note: This drawing is not to scale. Follow dimensions.

					Γŧ	3				SOIL BORING NUMBER: 25Ped_B4_1 Page 1 of 2
PRO)JE(СТ	Impr	ove I	70 KC Des	sign E	Build			NORTHING/EASTING 1061150.4 / 2777772.1
DRI	LLIN	IG I	FIRM	PPI		DRIL	LER	Josh	Stark	xey DATE STARTED 12/18/2024
LOC	GE I	D B	Y <u>C</u>	amero	on Dupont					DATE COMPLETED 12/18/2024
SUF	RFAC	CE	ELEV	ATIO	N 915.3'					RIG TYPE CME-55LC
ME.	тно	D _	Wate	r Rot	ary					TOOLING 4-1/2" Continuous Flight Auger
)						Groundwater Data Lab
	Ф			h (in)						18 (2012) - 10 (201
	Depth of Sample	e		Recovery Length	S			Pocket Pen (tsf)		
(tt)	of Sa	Sample Type		ry L	Blow Counts (N-Value)	Recovery	<u></u>	Pen	Graphic Log	After Moisture Co
Depth (pth (mple	Sample ID	cove	w C -Valu	Seco	RQD (%)	cket	aphic	After _ Hours (ft): N/A
De	De	Saı	Sai	Rec	8 Z	%	RQ	Po	Gre	Visual Classification and Remarks Visual Classification and Remarks
-										FILL, dark brown, high plasticity, moist, FAT
-										CLAY 912.8
13	3.5 ft									Shale, highly weathered, dark gray, stiff to very
_ 5		X	J-1	14	5-10-16 (26)	78		>4.5		stiff, moist 60-23-37 13.6
	5.3 ft		C-1	60		100	87			
_										6.7 908.6 Uimestone slightly weathered to fresh
16										Limestone, slightly weathered to fresh, gray-blue, hard, thin shale seams throughout
_ 0										
0	10.3 ft	Н	C-2	60		100	88	1		
		П								0.3 165.9 104
_		П								12.6 902.7 Shale, highly weathered, dark gray, soft to
_		П								14.0 stiff, interbedded with limestone 901.3
5	15.3 ft	Н	C-3	50		83	8			Limestone, slightly weathered, dark gray, hard, calcareous
			0.3							
19-										
										10.0
0.	20.3 ft	Н		59		98	52			19.8 895.5 Shale, highly weathered, dark gray, stiff to very
8 - 8			C-4	59		98	52			stiff, moist
25	25.3 ft	Ц				ii.				9.1 141.5 3
			C-5	60		100	97			interhedded with limestone from 26.11 to
(s)		П								- interbedded with limestone from 26.1' to 26.8'
15		П								- becomes green-gray, limey at 28.3'
30	30.3 ft									becomes green gray, inney at 20.0
-		П	C-6	60		100	54			
u . 										32.3 883
81										Limestone with interbedded shale, weathered, gray-green, very stiff to hard, moist
- 35	25.05									34.7 880.6
12-	35.3 ft		C-7	60		100	90			Limestone, slightly weathered to fresh, gray, hard, moist
		1 1								

Released For Construction Not to Scale Revision: 0.0 Date: 04/11/2025

Notes: For locations of borings, see Sheet No. B03-02 and Geotechnical Report.

BORING LOGS

Package: BRD-03-25th_ST_PED

WILLIAM
JOSEPH
STURGEON
PE-2014017021

4/11/2025

DATE PREPARED 04/11/2025

I - 70 MO

BR B03-27

	PROJECT Improve I 70 KC Design Build PRILLING FIRM PPI DRILLER Josh Starkey									······································	NORTHING/EASTING 1061150.4 / 2777772.1 DATE STARTED 12/18/2024						
OGGED BY Cameron Dupont										Су	DATE STARTED 12/10/20						
SURFACE ELEVATION 915.3' METHOD Water Rotary 1											RIG TYPE CME-55LC						
ME	тно	D _	Wate	r Rot	ary						TOOLING 4-1/2" Continu	ous Flight Auge	er				
	Orgundwater Data												Lab				
	le			th (in)				6		Di	uring Drilling (ft):	N/A	its	int	CF)		
	Sample	Type		Leng	ıts	>		n (tsf)	Log	Af	fter Drilling (ft):	N/A	J Lim PI)	Sonte	ty (P(
n (ft)	of	ole Ty	ole ID	very	Cour alue)	Recovery	(%)	et Pe	nic Lo	Af	fter Hours (ft):	N/A	Atterberg Limits (LL-PL-PI)	nre (ensit	(tsf)	
Depth (ft)	Depth	Sample	Sample ID	Recovery Length	Blow Counts (N-Value)	% Re	RQD (%)	Pocket Pen	Graphic		Visual Classification and Rema	arks	Atte (Moisture Content (%)	Dry Density (PCF)	UCS (tsf)	
_			C-7	60		100					tone, slightly weathered to fre	/					
- 40		П								38.3 hard, r Shale,	noist highly weathered, green-gray	877 , stiff,					
·										_38.9 moist \\\40.3 Limest	tone, slightly weathered, gray,	876.4 hard 875					
-										Botton	n of Boring at 40.3'	-					
_										Boring	backfilled with cuttings 12/18	/2024					
45																	
-																	
_																	
_																	
50																	
7 <u>-</u>																	
8. 77																	
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55																	
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87 <u></u> -																	
- 70																	
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			I														
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				-	TE	3				SOIL BORING NUMBER: 25Ped_B5_1 Page 1 of 1
PRC)JE(СТ	Impr	ove I	70 KC Des	sign B	Build			NORTHING/EASTING 1061172.1 / 2777809.8
DRI	LLIN	IG F	IRM	PPI		DRIL	LER	Josh	Stark	key DATE STARTED 12/18/2024
LOGGED BY Cameron Dupont										DATE COMPLETED 12/18/2024
SUF	RFAC	CE E	LEV	ATIO	N 929.6'					RIG TYPE CME-55LC
ME.	тно	D_	Auge	er	2					TOOLING 4-1/2" Continuous Flight Auger
		\Box								Lab
				(in)						Groundwater Data
	aldı			ıgth				(st)		During Drilling (ft): After Drilling (ft): N/A After Louisity (PCF) N/A After Mours (ft): N/A N/A N/A N/A N/A N/A N/A N/
(Sample	уре	0	Ler	ınts	ery		en (t	Log	After Drilling (ft): N/A 등 다 다 다 나는 사람이 되었다.
h (ft	of	Je T	ole II	very	Cou alue)	Recovery	(%)	et P(hic L	After Drilling (tt): After — Hours (ft): N/A Observe Temperal Learning (ft): N/A Observe Temperal Learning (ft): N/A Observe Temperal Learning (ft): Observe
Depth (ft)	Depth	Sample Type	Sample ID	Recovery Length	Blow Counts (N-Value)	% Re	RQD (%)	Pocket Pen (tsf)	Graphic	During Drilling (ft): After Drilling (ft): N/A After Phours (ft): N/A Visual Classification and Remarks N/A Visual Classification and Remarks
10	55569		903000	74 - 74	dente over	an Personals		****		FILL, dark brown, stiff, high plasticity, moist, FAT CLAY
										3.0 926.6
_	4 ft									Limestone, highly weathered to weathered,
5	5.5 ft	П	C-1	18		100	28			tan-gray, hard, vuggy, orange sandstone seams throughout
=	0.0	Ħ	C-2	60		100	63			0.2 160.0 489
-		П								
-		П								- sandstone seams every 4 inches from 8-10'
0		П								
	10.5 ft	H	C-3	60		100	78			
S 		П								- 3" shale seam, soft, moist at 11.6` 0.4 165.7 934
_		П								13.3 916.4
- 15		П								Shale, highly weathered, dark gray, soft, clayey, moist
15	15.5 ft	Ц	222 2				820123			Clayey, moist
-		П	C-4	60		100	78			
-		П								13.3 126.0 17
		П								
20	20 5 4									
-	20.5 ft	H	C-5	58		97	80			21.5
:		П								Limestone, slightly weathered to fresh, gray,
=		П								hard, calcareous 0.4 165.1 1214
_ 25		Ш								
_	25.5 ft	H	C-6	60		100	73			
_			U-0			,00	""			27.1 902.5
8-										27.3 Shale seam (3"), soft, highly weathered 902.3
-										Limestone, slightly weathered, gray, hard, moist 0.6 166.1 423
30										30.5 899.1
; -										Bottom of Boring at 30.5'
-		$ \ $								Boring backfilled with cuttings 12/18/2024
:		$ \ $								
- 35		$\ \ $								
		$ \ $								
		$ \ $								

Released For Construction Not to Scale Revision: 0.0 Date: 04/11/2025 Package: BRD-03-25th_ST_PED

Notes: For locations of borings, see Sheet No. B03-02 and Geotechnical Report.

BORING LOGS

WILLIAM
JOSEPH
STURGEON
PE-2014017021 4/11/2025

DATE PREPARED 04/11/2025 I - 70 MO BR B03-28 COUNTY

JACKSON JOB NO. J4I1486D CONTRACT ID. 240807-C01 PROJECT NO.

		96	5 2	9 9	
DESCRIPTION	04/11/25 REV 0 - RFC SUBMITTAL				
DATE	04/11/25				