U.I.P. REHABILITATE & WIDEN EXISTING (43' - 60' - 36') PRESTRESSED CONCRETE I-GIRDER SPANS

PROSPECT HD WENTZVILLE

LOCATION MAP

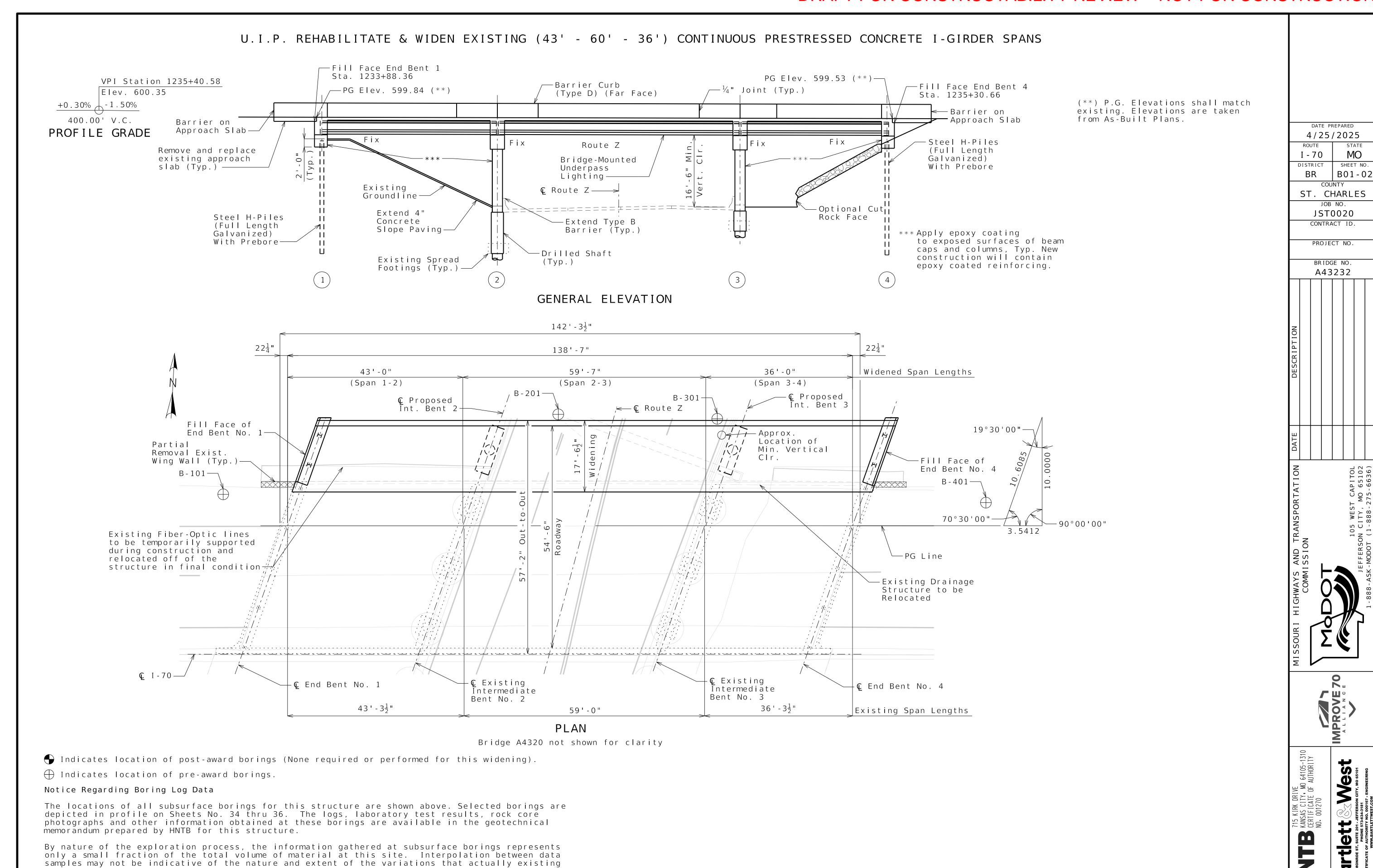
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4/25/2025 I - 70 BR B01-01 COUNTY ST. CHARLES JOB NO. JST0020 CONTRACT ID. PROJECT NO. BRIDGE NO. A43232

BRIDGE: ROUTE I-70 WB OVER ROUTE Z

ROUTE * FROM * TO *
ABOUT * MILES * OF *
___ STATION _____



GENERAL PLAN AND ELEVATION

between sampling locations.

	Founda	tion Data	a		
			Bent	Numb e r	
Туре	Design Data	1	2	3	4
	Pile Type and Size	HP12x53			HP12x53
	Number ea	2	1	-	2
	Approximate Length Per Each ft				
Load	Pile Point Reinforcement ea	2	-	-	2
Bearing Pile	Min. Galvanized Penetration (Elev.) ft	Full Length	-		Full Length
	Pile Driving Verification Method				
	Resistance Factor				
	Minimum Nominal Axial Compressive Resistance kip				
	Number ea		1	1	
	Foundation Material				
	Σ Elevation Range ft				
	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf				
	Foundation Material				
Rock Socket	Elevation Range ft				
JUCKEL	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf				
	Minimum Nominal Axial Compressive Resistance (Tip Resistance) ksf				

WEAP = Wave Equation Analysis of Piles

DT = Dynamic Testing

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

Minimum Nominal Axial Compressive Resistance (Side Resistance + Tip Resistance) = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factors}}$

Driven Piles:

All piles shall be galvanized full length.

Manufactured pile point reinforcement shall be used on all piles in this structure.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Prebore for piles at Bents 1 and 4 to elevations XXX.XX.

Prebore holes shall be backfilled with sand (or other approved material), in accordance with Sec 702, before piles are driven to bearing.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.) Seismic Performance Category A

Design Loading:

Vehicular = HS20 Modified & Military 24,000 lb Tandem Axle (1983) (New and Existing Girders)
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf
Superstructure: Simply-Supported, Non-Composite for Dead Load. Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure)
Class B-2 Concrete (Drilled Shafts & Rock Sockets)
Class B-1 Concrete (Barrier)
Class B-2 Concrete (Superstructure, except
Prestressed Girders Beams and Barrier)
Reinforcing Steel (ASTM A706 Grade 60)
Structural Steel HP Pile (ASTM A709 Grade 50)
For prestressed girder stresses, see Sheets No. X thru X.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Pile Protective Coatings:

Piles shall be galvanized in accordance with Sec 702 and Sec. 1081.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

MBS Refers to mechanical bar splices. Mechanical bar splices shall be in accordance with Sec 706 or 710

All exposed substructure elements will contain epoxy coated reinforcing.

Hooks and bends shall be in accordance with CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Traffic Handling:

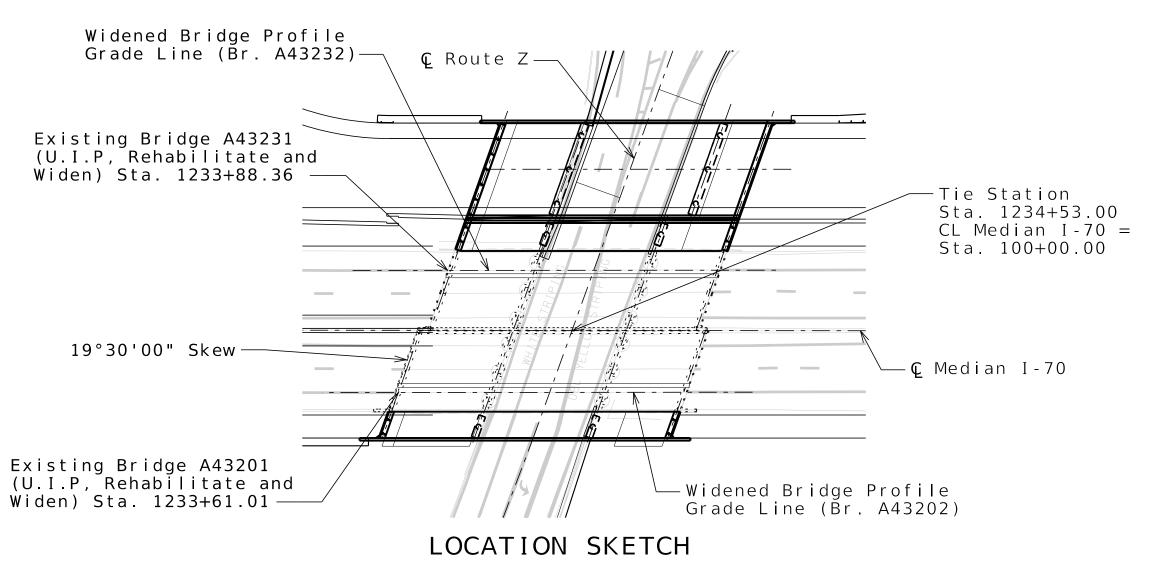
Vertical clearance for Route Z traffic during construction shall be 15 ft. minimum over a 2 - 12 foot lane horizontal opening centered on Route Z.

For traffic phasing during construction, see Maintenance of Traffic Plans.

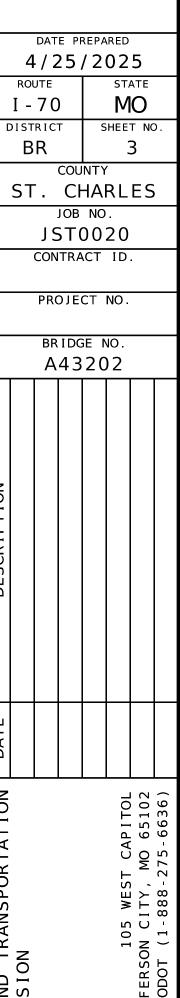
Concrete Protective Coatings:

Protective coating for concrete bents and piers (Epoxy) shall be applied to all exposed substructure surfaces and in accordance with Sec 711.

Aesthetic Stain shall be applied to the vertical faces of the exterior barrier, deck and Prestressed Concrete Girders as shown on the plans in accordance with Sec 711.



GENERAL NOTES (1 OF 2)





HNTB KANSAS CITY, MO 64105-1310

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General Notes (cont.):

Bridge Deck:

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC4 and a finish type I, II, or III.

Slab shall be cast-in-place with a stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirements and coating designation G165 of ASTM A653. Complete Shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for falsework and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.

Miscellaneous:

High strength bolts, nuts and washers will be sampled for quality assurance as specified in Sec 106.

Roadway surfacing adjacent to bridge ends shall match new bridge wearing surface (roadway item).

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

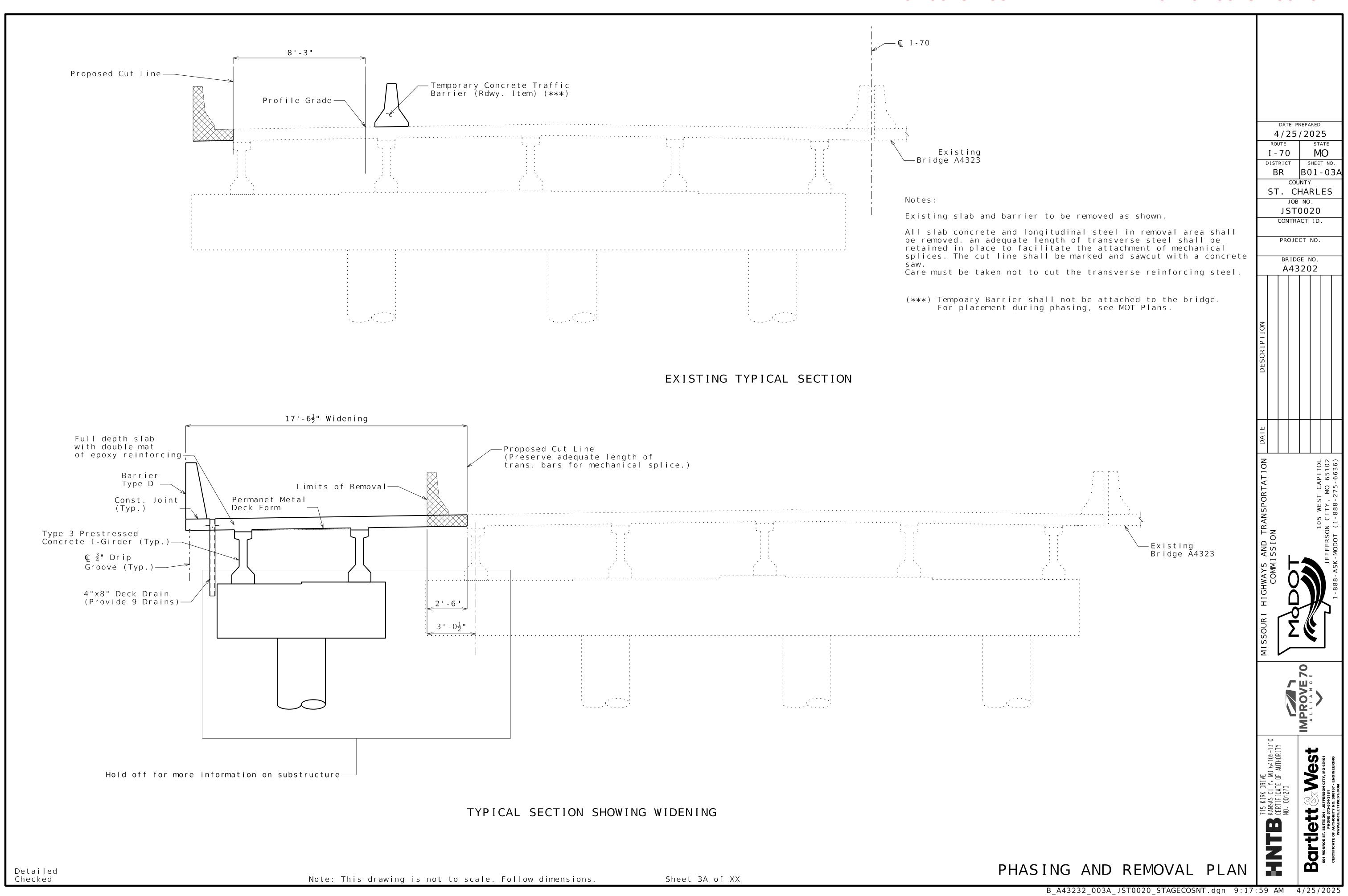
Contractor shall verify all dimensions in field before finalizing shop drawings.

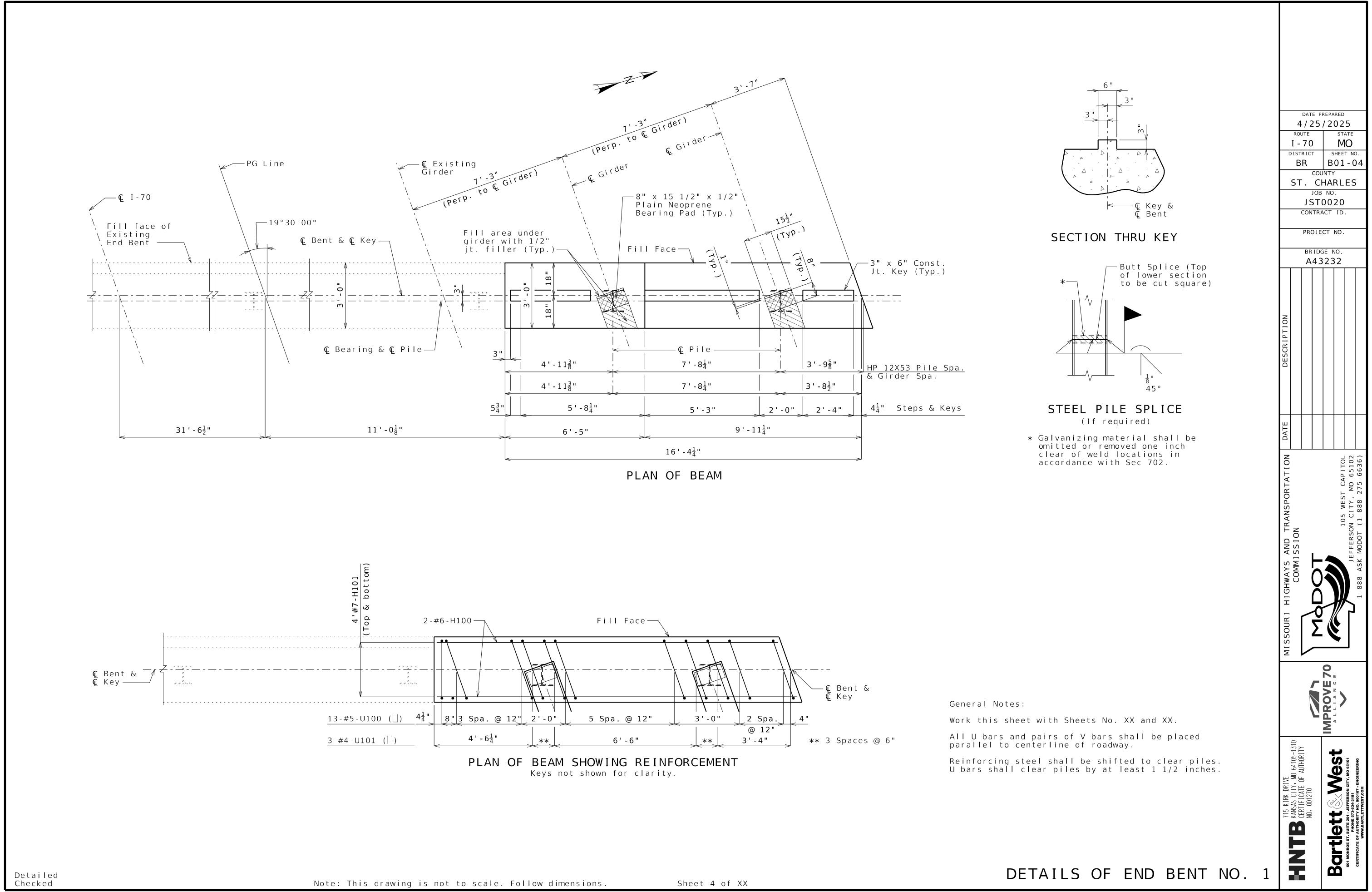
Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters, for deformed bars unless otherwise noted.

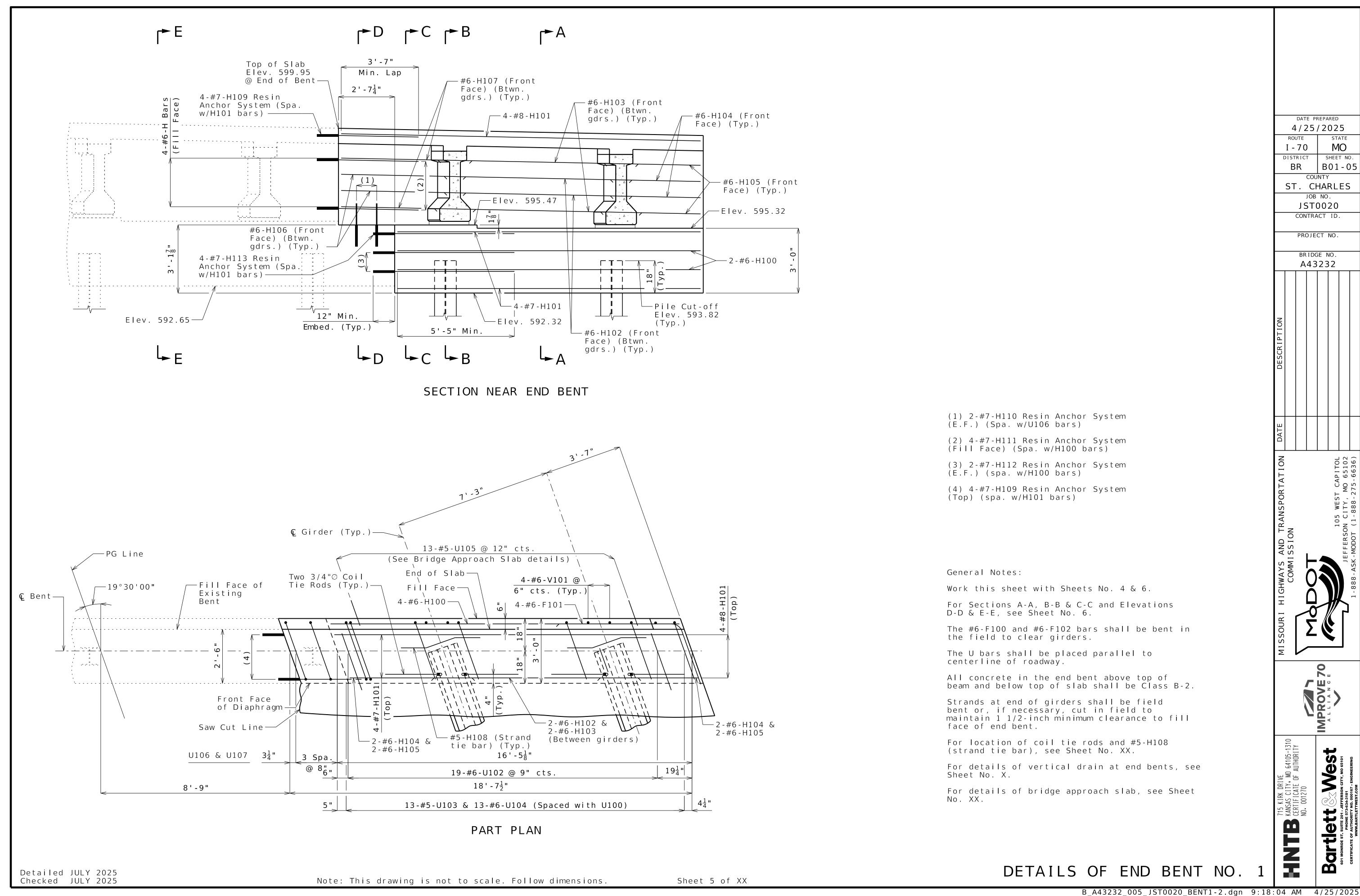
DATE PREPARED 4/25/2025 ROUTE I - 70 MO DISTRICT SHEET NO. B01-03 COUNTY ST. CHARLES JOB NO. JST0020 CONTRACT ID. PROJECT NO. BRIDGE NO. A43232

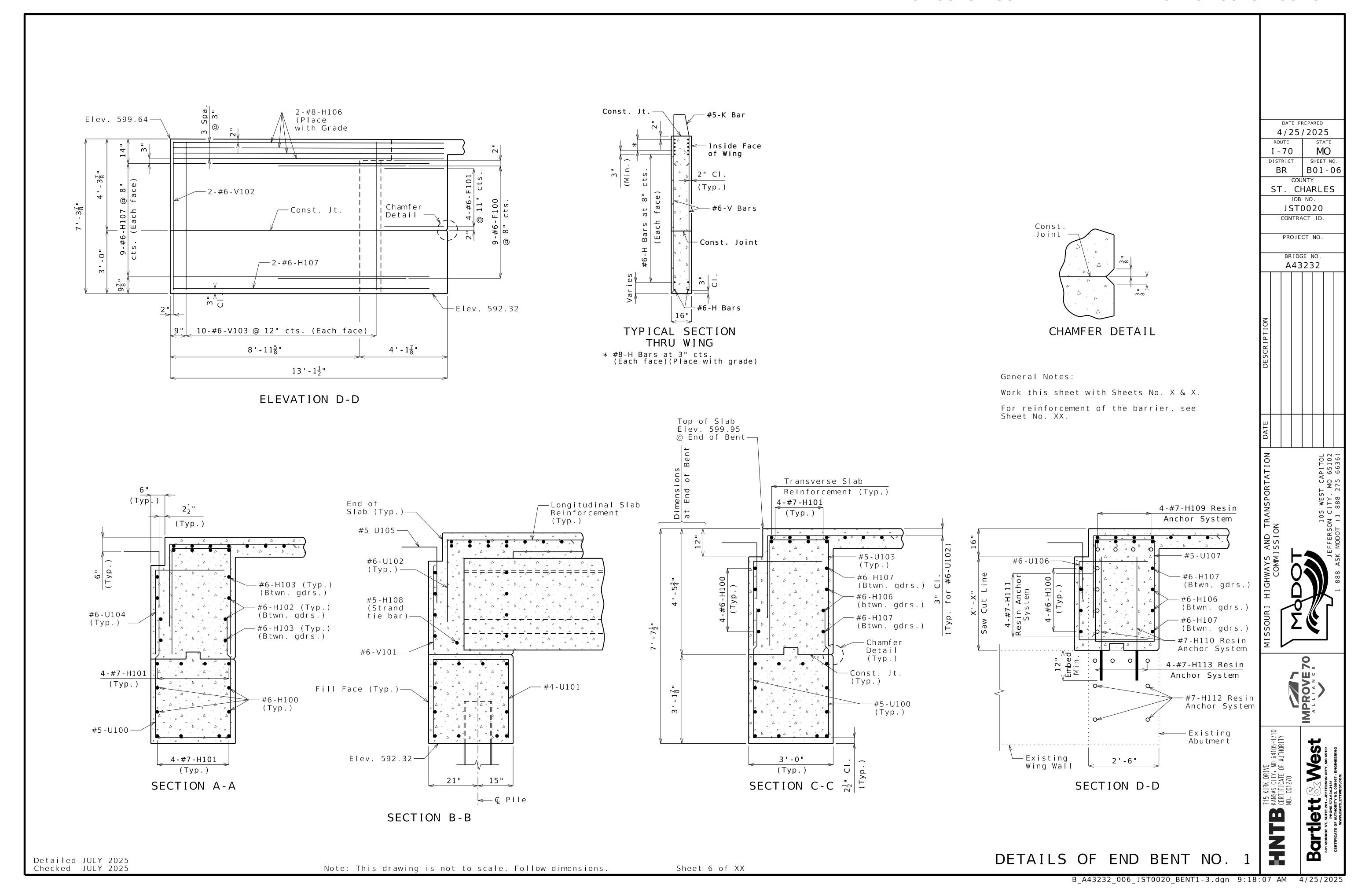
Bartlett

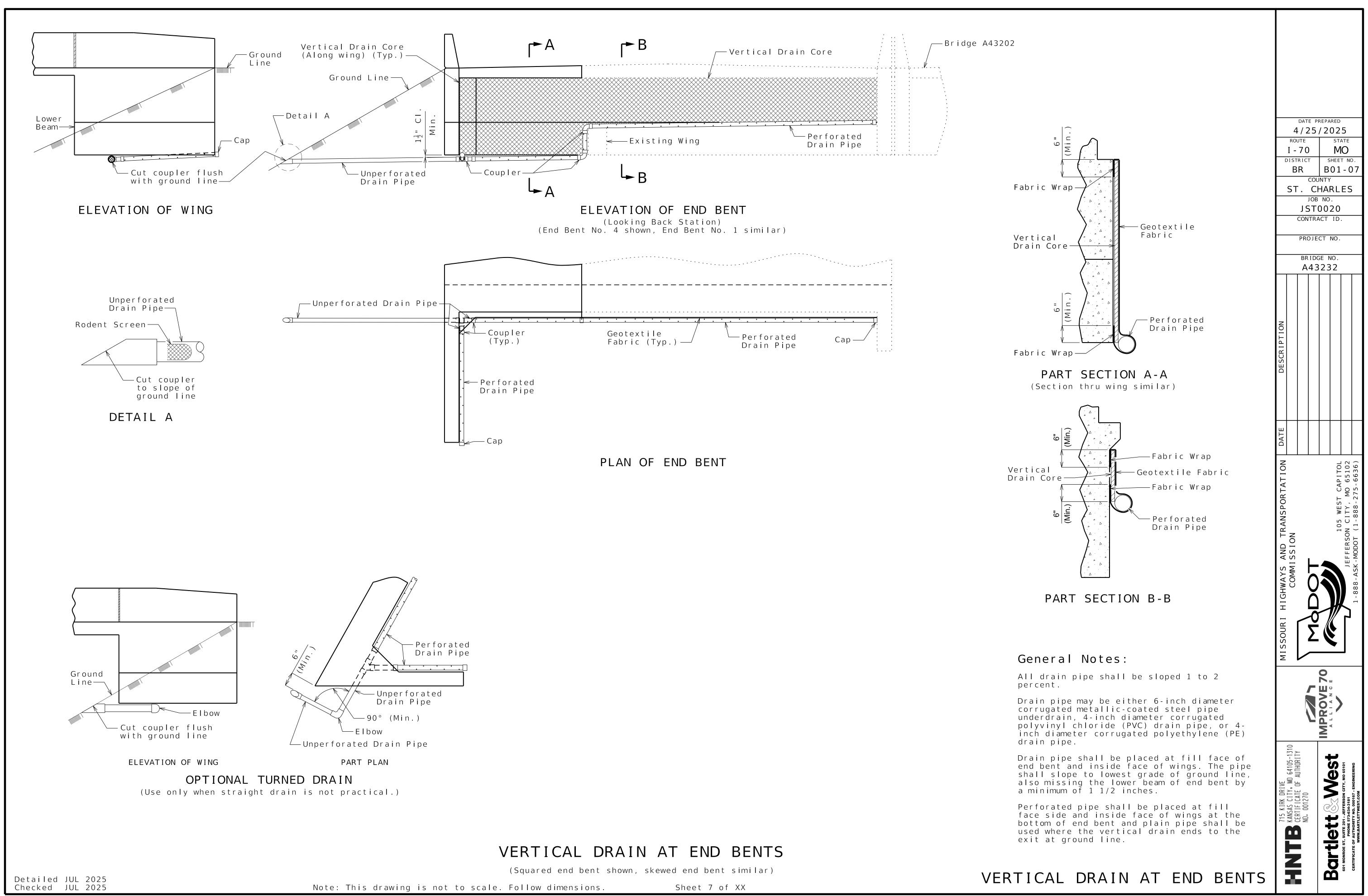
GENERAL NOTES AND ESTIMATED QUANTITIES

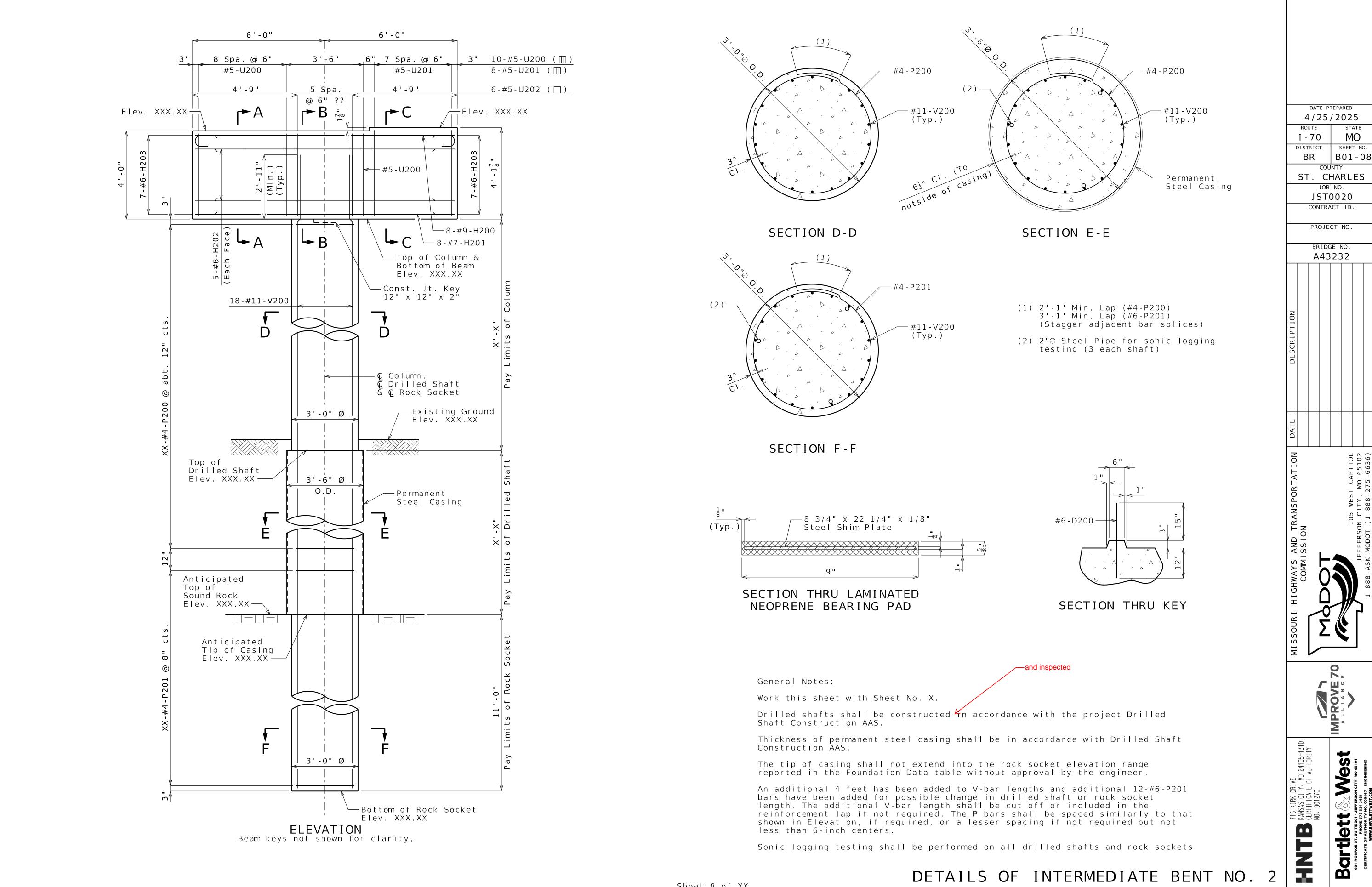


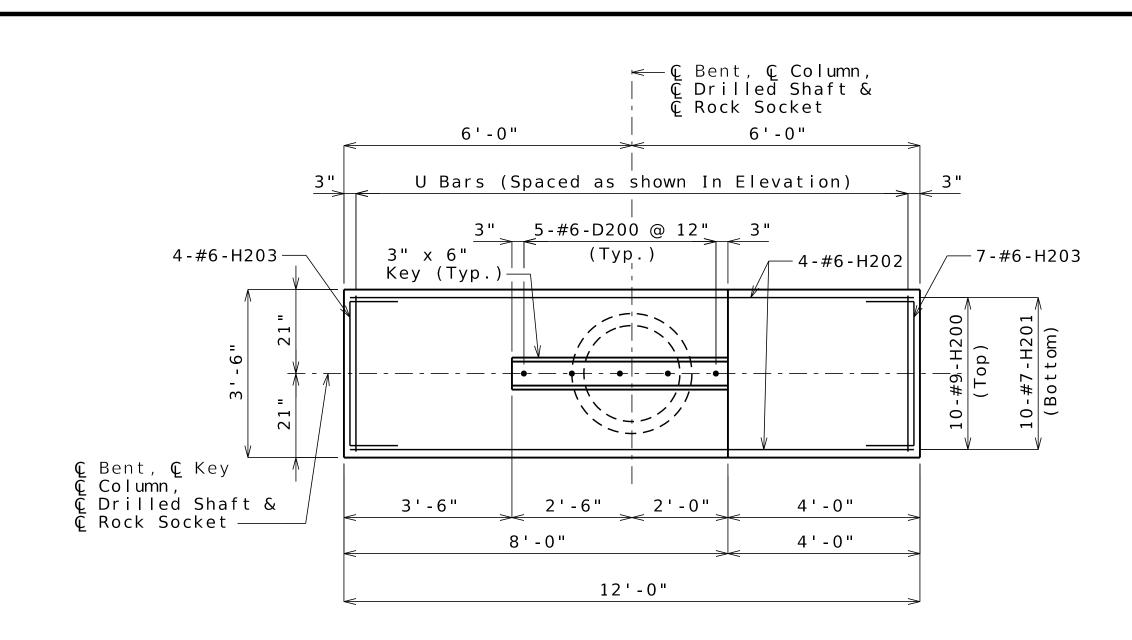




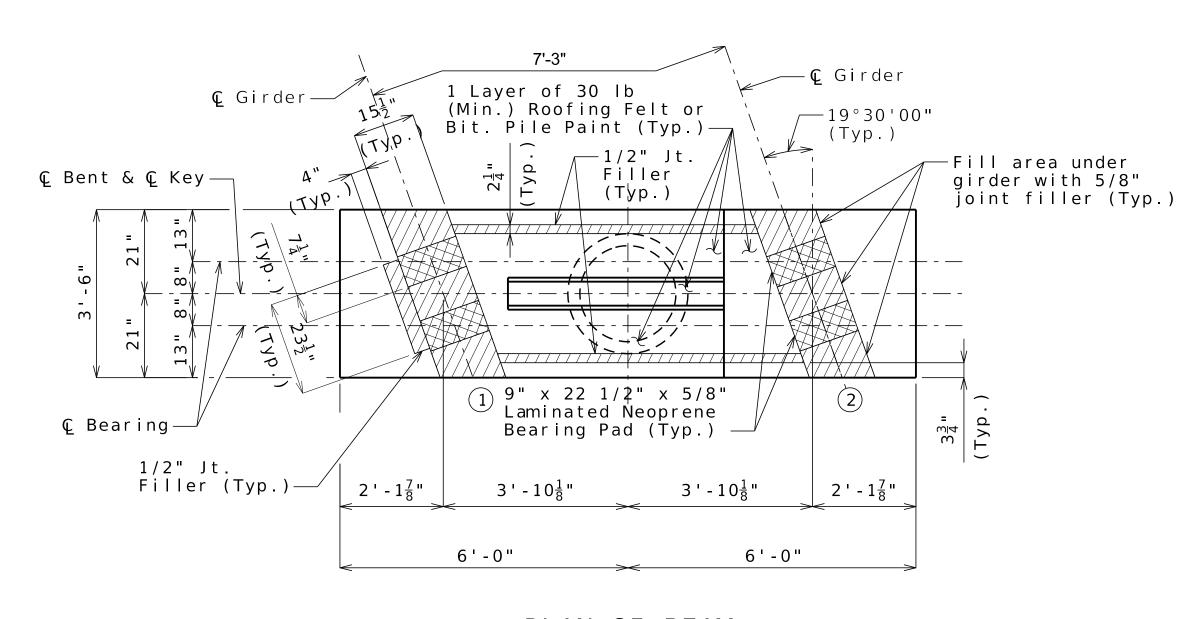




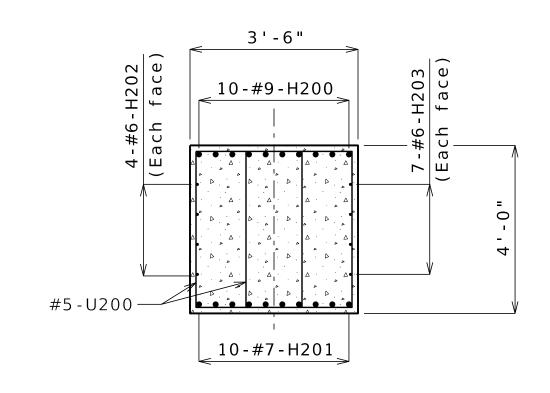




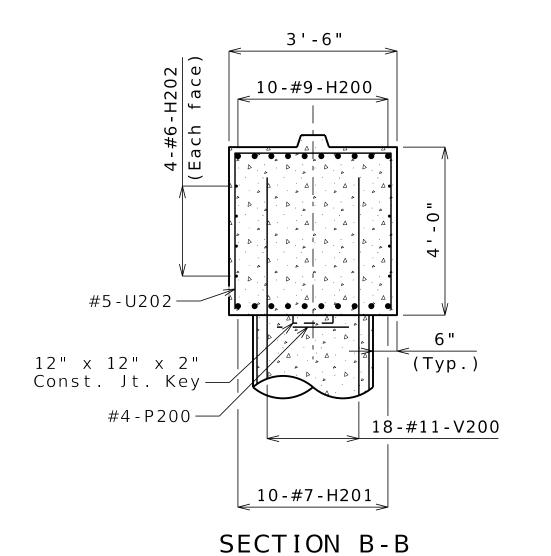
PLAN SHOWING REINFORCEMENT

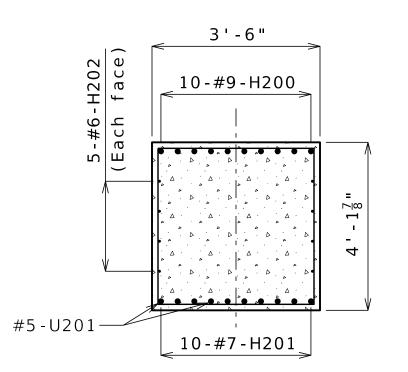


PLAN OF BEAM



SECTION A-A





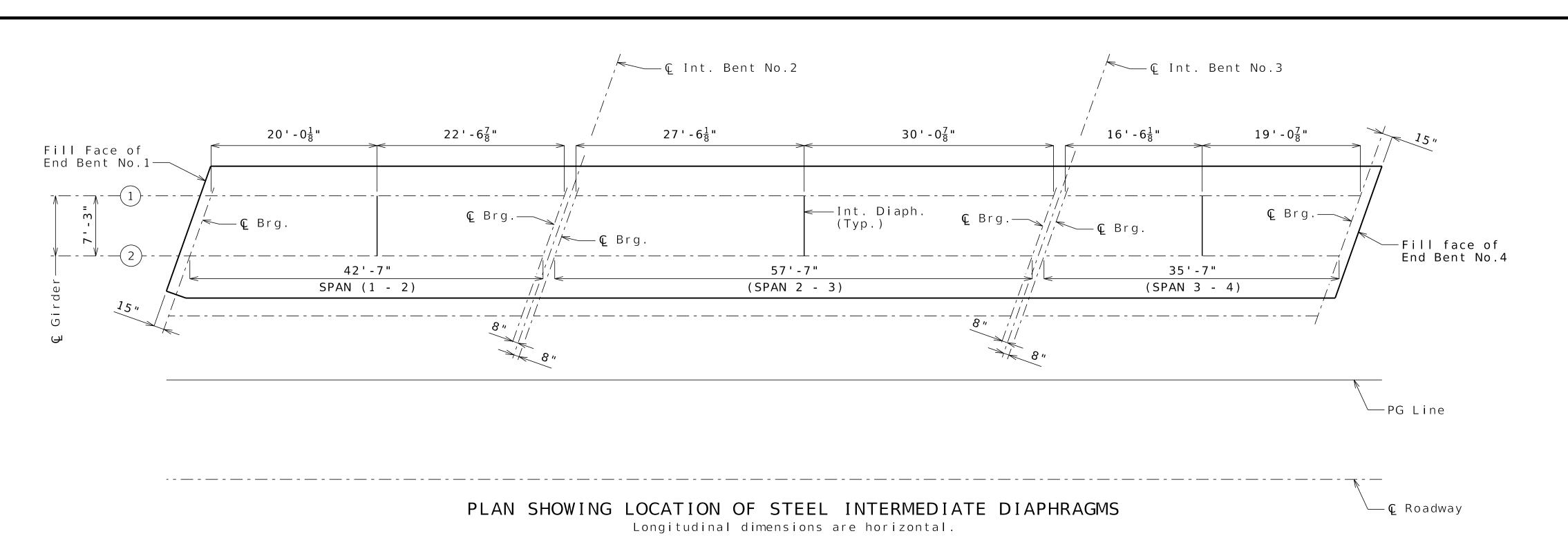
SECTION C-C

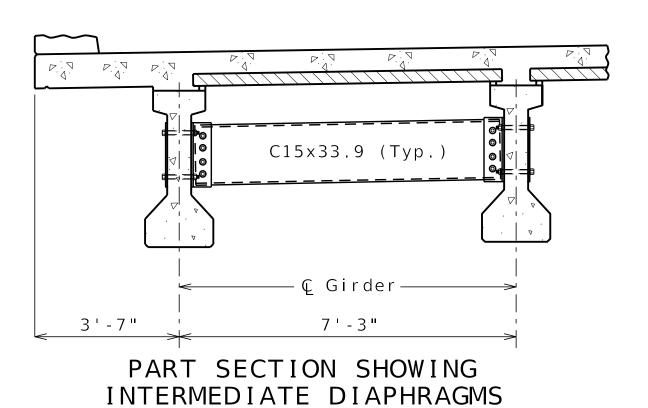
General Notes:

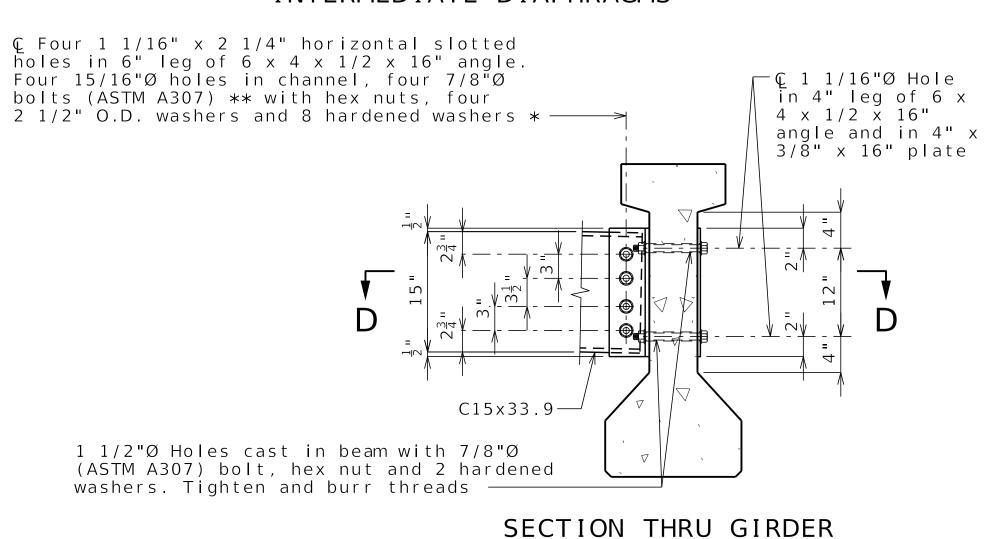
Work this sheet with Sheet No. X.

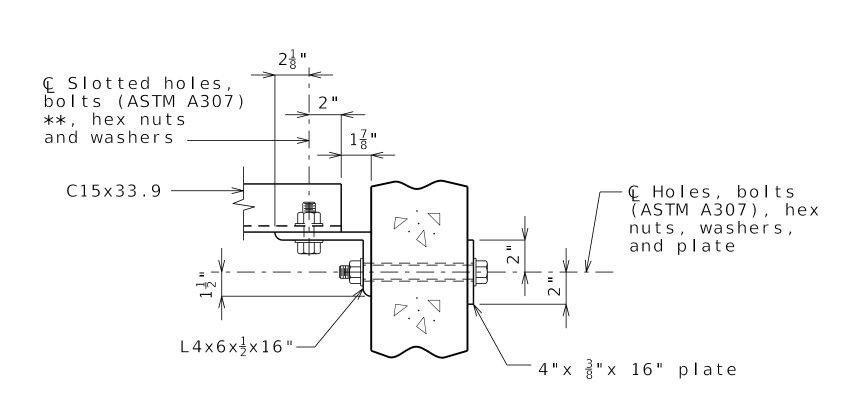
For steps 2 inches or more, use 2 1/4x 1/2-inch joint filler up vertical face.

DATE PREPARED 4/25/2025 MO I - 70 DISTRICT SHEET NO. B01-09 COUNTY ST. CHARLES JOB NO. JST0020 CONTRACT ID. PROJECT NO. BRIDGE NO. A43232 **Bartlett**









SECTION D-D

STEEL DIAPHRAGM NOTES:

 \star In lieu of 2 1/2" outside diameter washers, contractor may substitute a 3/16" (Min. thickness) plate with four 15/16" Ø holes and one hardened washer per bolt.

** Bolts shall be tightened to provide a tension of one-half that specified in Sec 712 for high strength bolt installation. ASTM F3125 Grade A325 Type 1 bolts may be substituted for and installed in accordance with the requirements for the specified ASTM A307 bolts.

All diaphragm materials including bolts, nuts, and washers shall be galvanized.

Fabricated structural steel shall be ASTM A709 Grade 36 except as noted.

Payment for furnishing and installing steel intermediate diaphragms will be considered completely covered by the contract unit price for Steel Intermediate Diaphragm for P/S Concrete Girders.

Shop drawings will not be required for steel intermediate diaphragms and angle connections.

GIRDER LAYOUT

DATE PREPARED 4/25/2025

COUNTY

ST. CHARLES

JOB NO.

JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A43232

70

Bartlett

IMPROVE

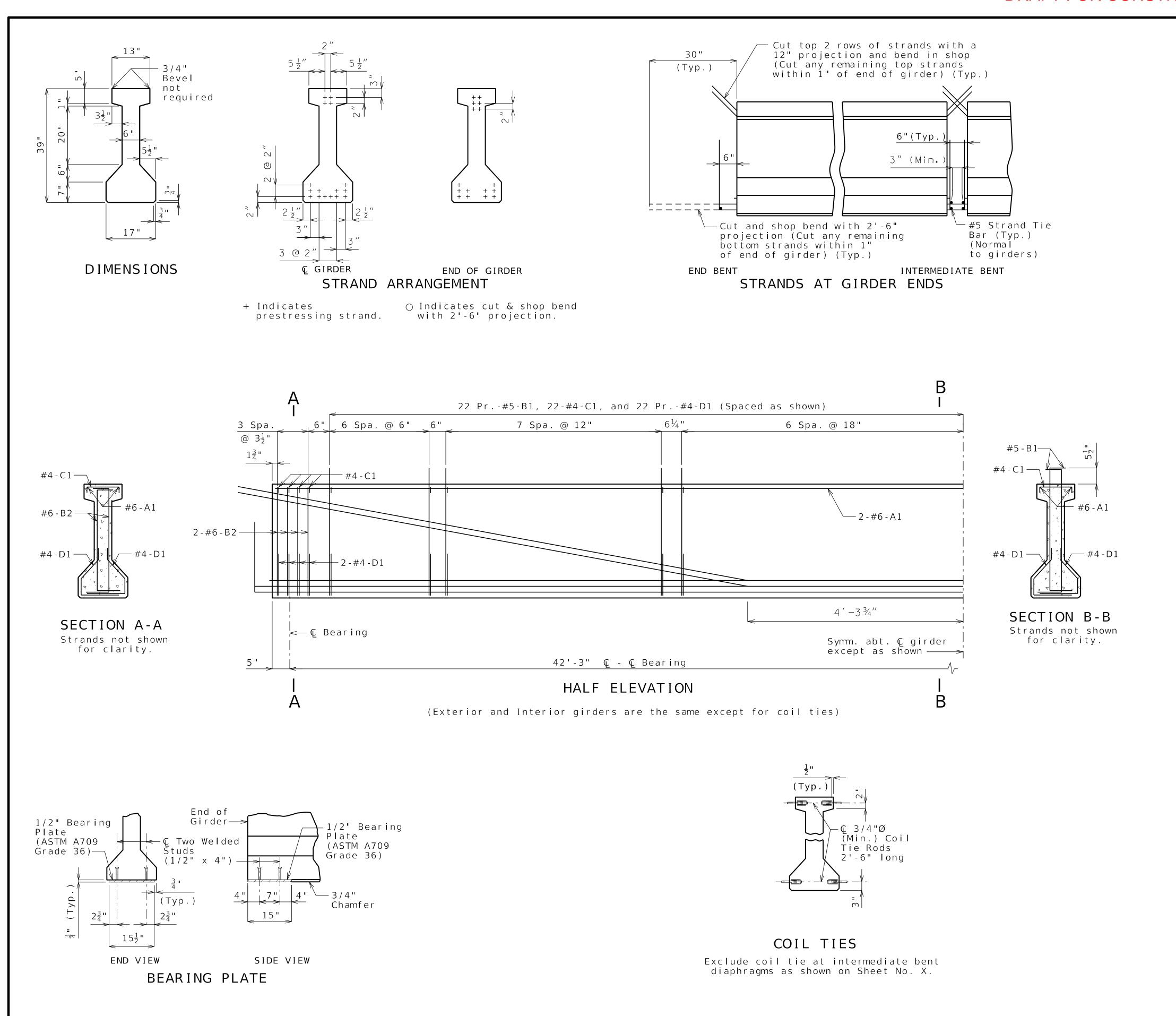
SHEET NO

B01-17

I - 70

DISTRICT

AT DIAPHRAGM



Note: This drawing is not to scale. Follow dimensions.

Detailed JUL 2025

Checked JUL 2025

I-GIRDERS - SPAN (1-2)

Sheet 18 of XX

ВІ	LL	OF	REINFO	RCIN	G STEEL -	EACH GIRDER
NO.		ZE & ARK	ACTUAL LENGTH	SHAPE	BENDI	NG DIAGRAM
2	6	A1	42'-10"	20	6"	11"
					4	= \(\frac{1}{\sqrt{2}} \)
86	5	В1	4'-7"	115	= 4	SHAPE 10S
16	6	В2	4'-0"	115	= 1	T 2 5"
					13"	1
51	4	C1	13"	105	SHAPE 9S	= 7 = 1
102	4	D1	2'-5"	95		Ψ <u>Ψ</u> <u> </u>
					SHAPE 20	SHAPE 11S

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with f'c = 6,000 psi and f'ci = 4,500 psi.

Use 16 strands, 0.6"Ø Grade 270, with an initial prestress force of 703 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

For Girder Camber Diagram, see Sheet No. 23.

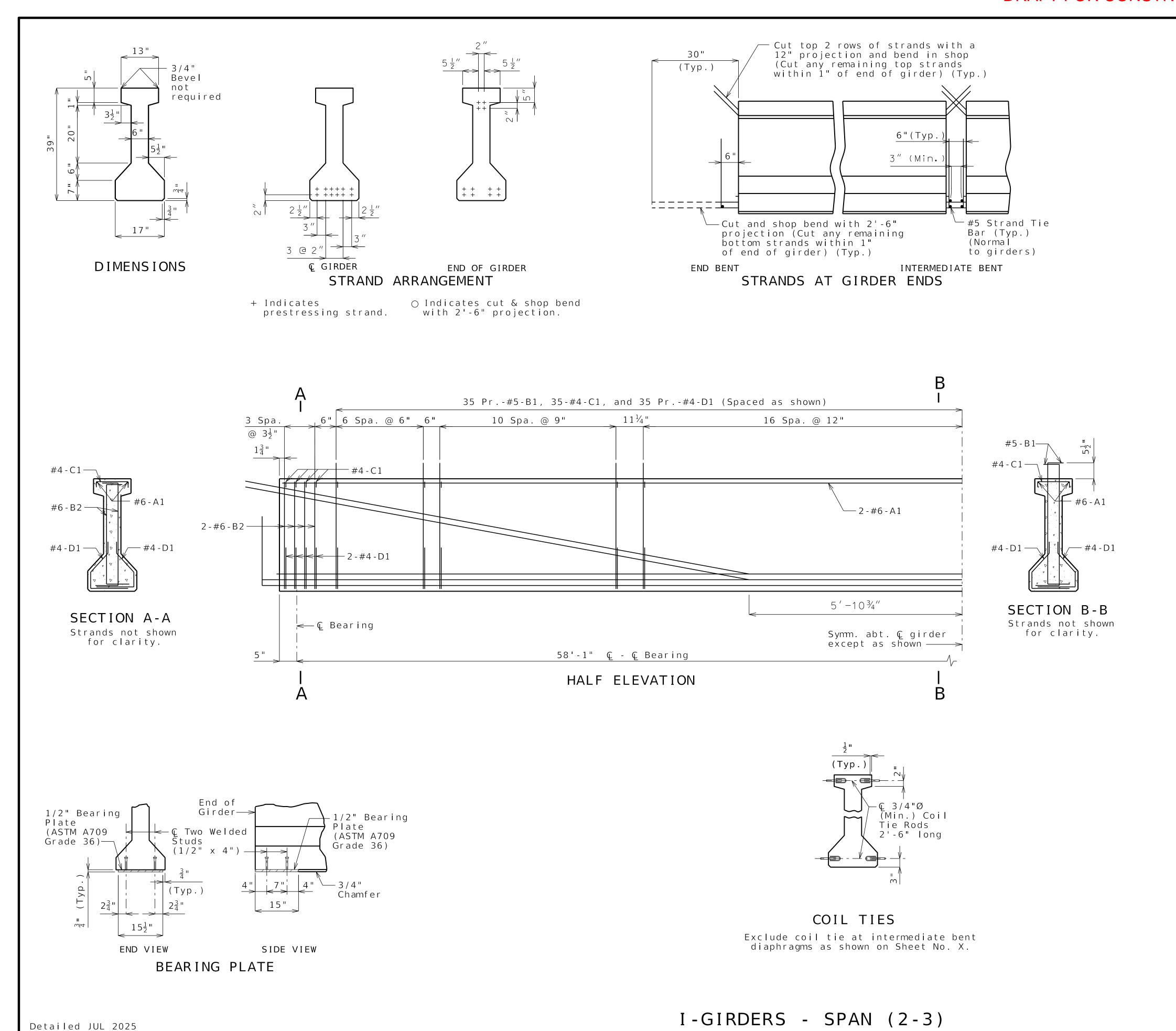
For location of coil inserts at slab drains, see Sheet No. 22.

For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 8 and 21.

DATE PREPARED 4/25/2025 I - 70 MO DISTRICT SHEET NO. B01-18 COUNTY ST. CHARLES JOB NO. JST0020 CONTRACT ID. PROJECT NO. BRIDGE NO. A43232 ш IMPROVE A L L L A N O IRK DKIVE S CITY, MO 64105-1310 FICATE OF AUTHORITY

I - GIRDERS - SPAN (1-2)

Bartlett



Note: This drawing is not to scale. Follow dimensions.

Checked JUL 2025

Sheet 19 of XX

ВІ	LL	OF	REINFO	RCIN	G STEEL -	EACH GIRDER
NO.		ZE & ARK	ACTUAL LENGTH	SHAPE	BENDI	NG DIAGRAM
2	6	A1	58'-8"	20	6"	11"
					4	= \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
138	5	В1	4'-7"	115	= 4	SHAPE 10S
16	6	В2	4 ' - 0 "	115	= 1	T 2 5"
					 	T
77	4	C1	13"	105	SHAPE 9S	1 = 1
154	4	D1	2 ' - 5 "	95		<u>√</u>
					SHAPE 20	SHAPE 11S

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with f'c = 6,000 psi and f'ci = 4,500 psi.

Use 12 strands, 0.6"Ø Grade 270, with an initial prestress force of 527 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

For Girder Camber Diagram, see Sheet No.

The 1 1/2"Ø holes shall be cast in the web for steel intermediate diaphragms.
Drilling is not allowed. For location of holes and details of steel intermediate diaphragms, see Sheet No. 17.

For location of coil inserts at slab drains, see Sheet No. 22.

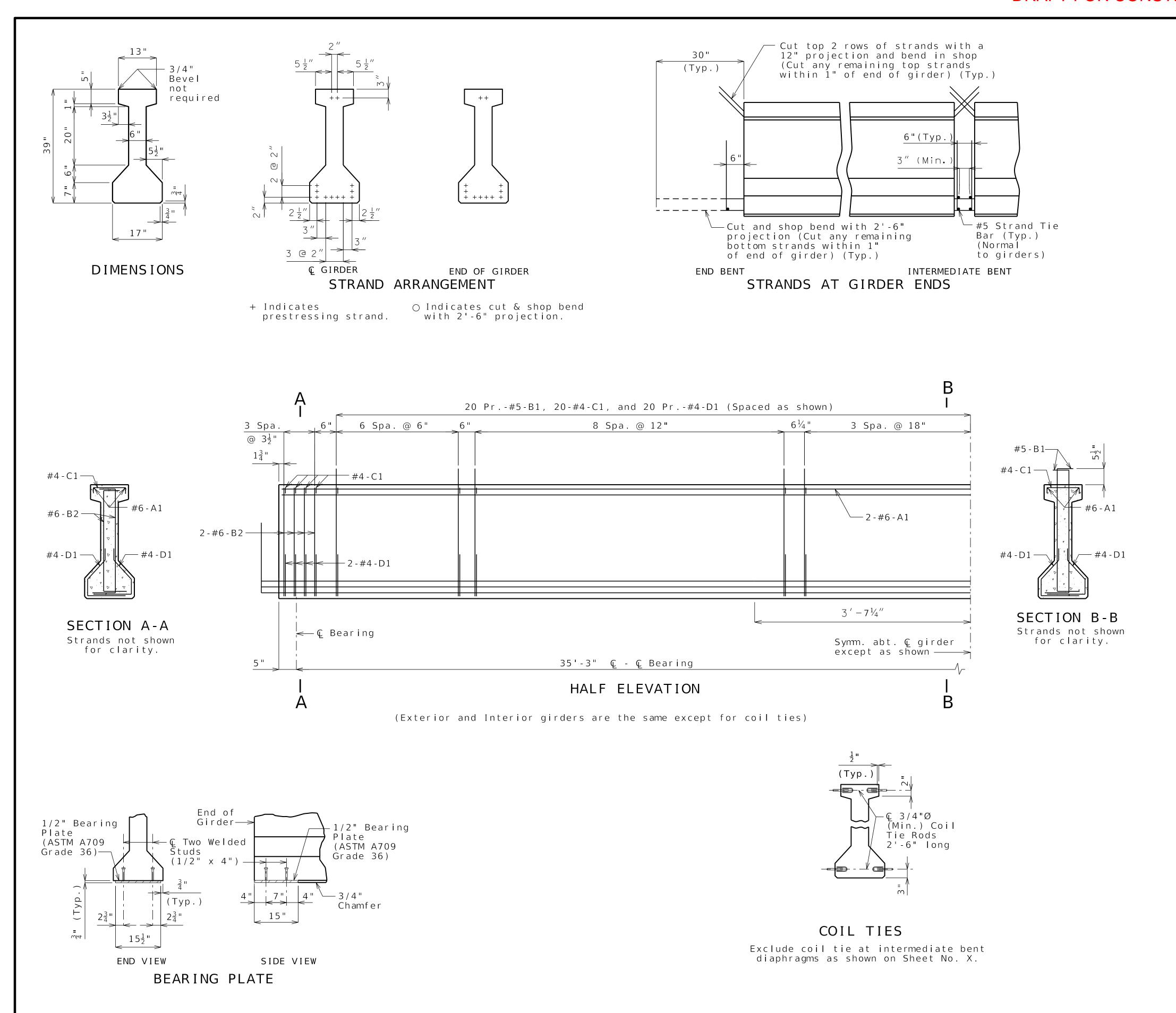
For location of coil ties at concrete diaphragms, see Sheet No. 21.

DATE PREPARED 4/25/2025 I - 70 MO DISTRICT SHEET NO. B01-19 COUNTY ST. CHARLES JOB NO. JST0020 CONTRACT ID. PROJECT NO. BRIDGE NO. A43232 70

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I- GIRDERS - SPAN (2-3)



Note: This drawing is not to scale. Follow dimensions.

Detailed JUL 2025

Checked JUL 2025

I-GIRDERS - SPAN (3-4)

Sheet 20 of XX

	SIZE &	ACTUAL	I	G SIEEL -	EACH GIRDER
NO .	MARK	LENGTH	SHAPE	BENDII	NG DIAGRAM
2	6 A1	35'-10"	20	6"	11"
				4	= \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
78	5 B1	4'-7"	115	= 4	SHAPE 10S
16	6 B2	4'-0"	115	= 1	□ ○ 5"
				13"	
47	4 C1	13"	105	SHAPE 9S	7 2 = 1
					m m
94	4 D1	2 ' - 5 "	95		Ψ <u>Ψ</u> [9½"
				SHAPE 20	SHAPE 11S

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with f'c = 6,000 psi and f'ci = 4,500 psi.

Use 14 strands, 0.6"Ø Grade 270, with an initial prestress force of 615 kips.

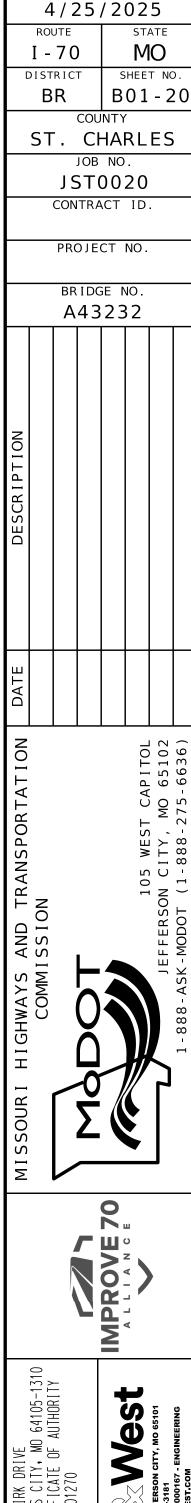
Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

For Girder Camber Diagram, see Sheet No.

For location of coil inserts at slab drains, see Sheet No. 22.

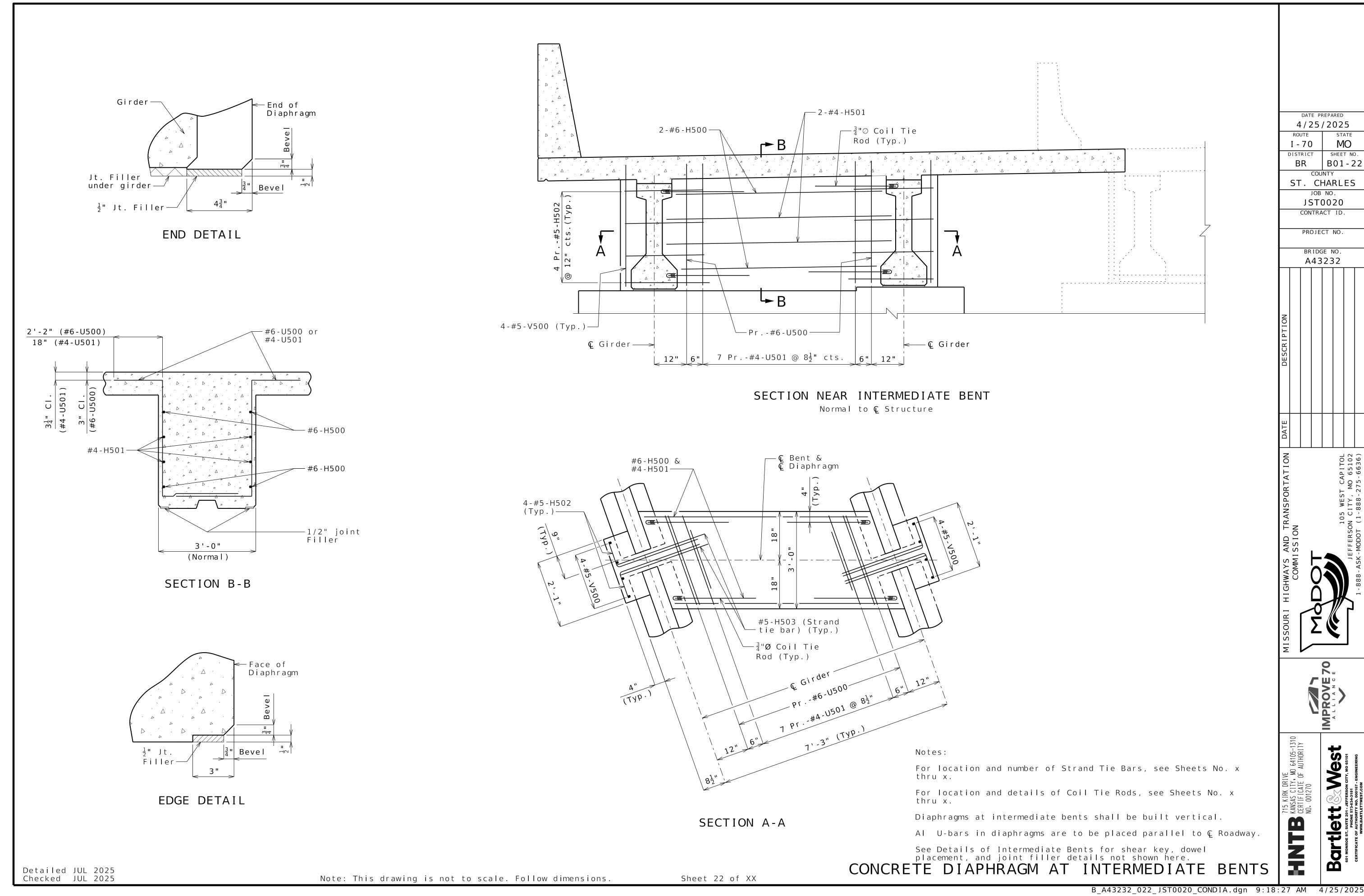
For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 15 and 21



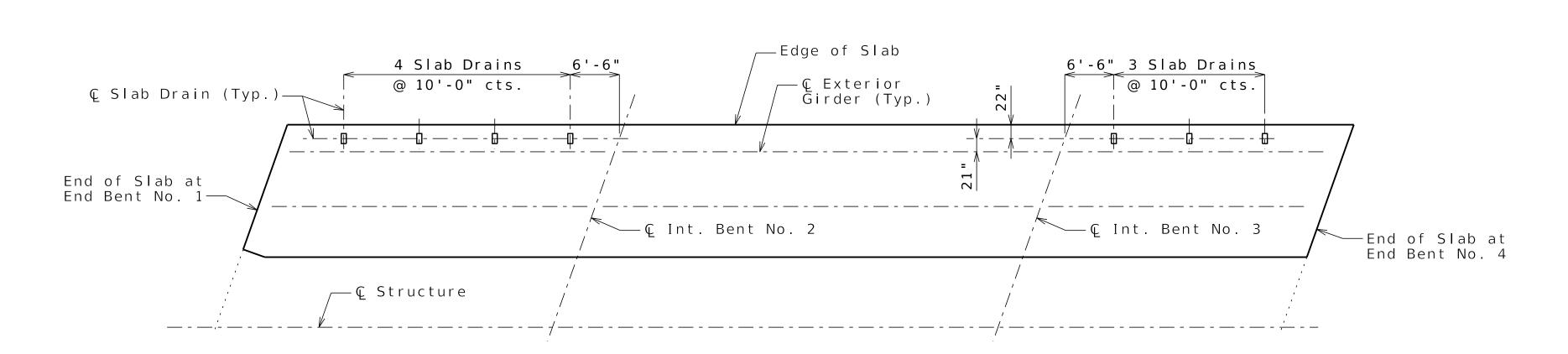
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DATE PREPARED

I - GIRDERS - SPAN (3-4)



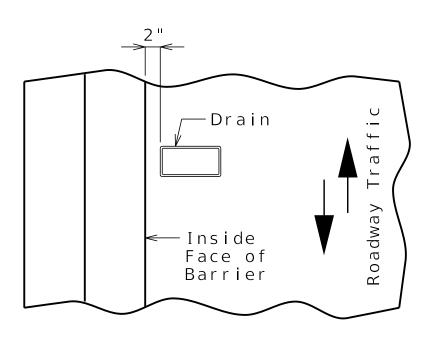
General Notes:



PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS

└← Ç 9/16"Ø Hole in angle for 1/2"Ø bolt with 2 hardened Angle (1/4" min. washers, lock washer, and nut 1/2" max. thickness) (3" min. legs) x 2" long— € 9/16"Ø Holes for 1/2"Ø bolt with lock - Prestressed washer and nut (Typ.) — Girder Web ___C Coil Insert & 9/16"Ø Hole for 1/2"Ø bolt with Drainlock washer Bent Strip 10 Gage (Min.) x 2"- $L2 \times 2 \times \frac{1}{4}$ $\frac{9}{16}$ " Slot in L2x2x $\frac{1}{4}$ —

PART SECTION SHOWING BRACKET ASSEMBLY



PART PLAN OF SLAB AT DRAIN

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shal be galvanized in accordance with ASTM A123

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil insert required for the bracket assembly attachment shall be located on the prestressed girder shop drawings.

Coil inserts shall have a concrete pullout strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolt required to attach the slab drain bracket assembly to the prestressed girder web shall be supplied by the prestressed girder fabricator.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8" x 4".

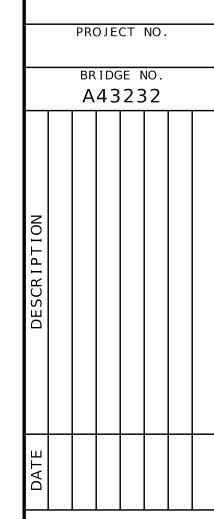
Minimum reinforced wall thickness shall be 1/4 inch.

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free cut.



DATE PREPARED

4/25/2025

COUNTY

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JOB NO. JST0020

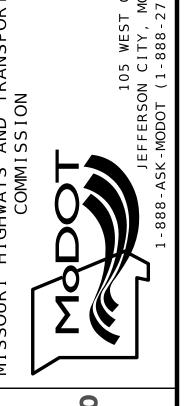
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SHEET NO. B01-23

I - 70

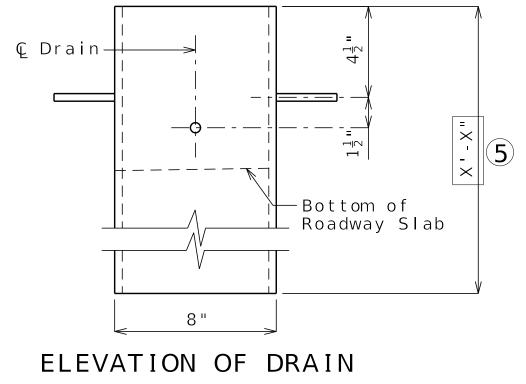
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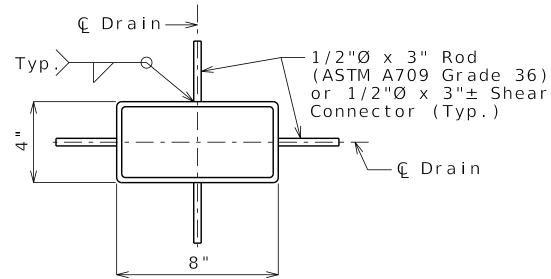




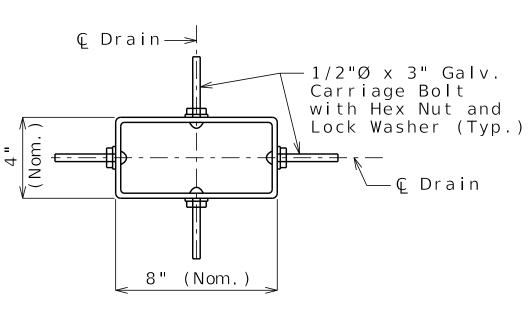
Bartlett

SLAB DRAINS





PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION

Detailed Checked

— Top of

1" (Min.)

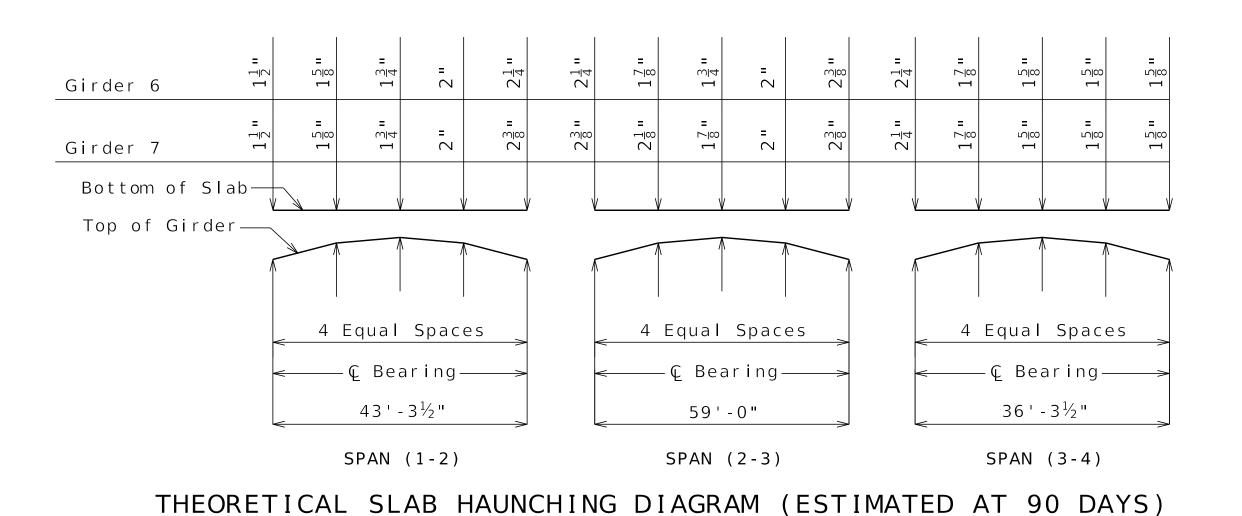
PART SECTION NEAR DRAIN

Roadway Slab

= 8 Bo as ns

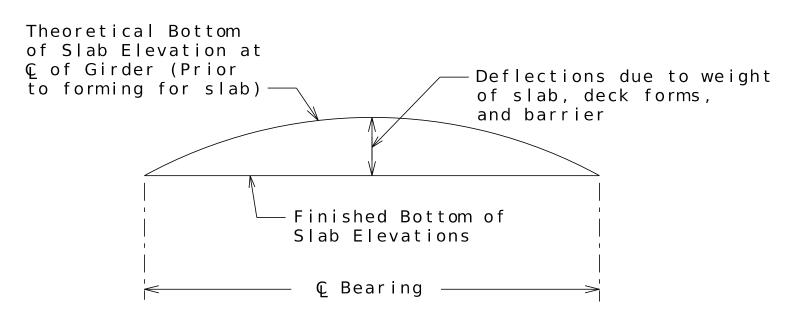
20 2 = ... 2 × ...

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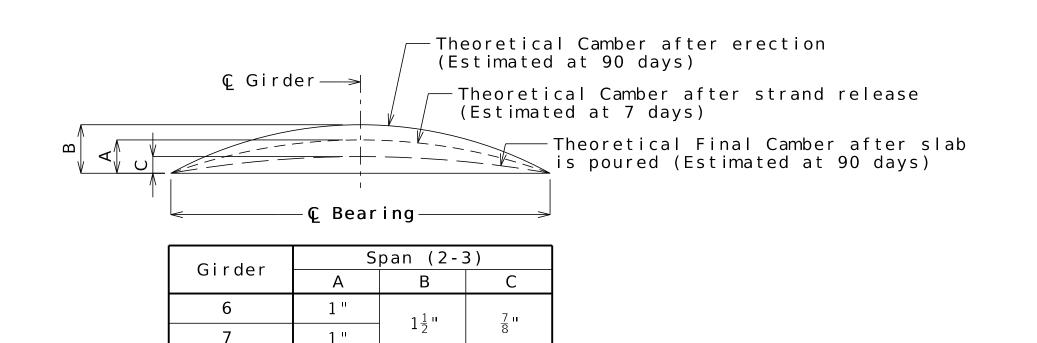


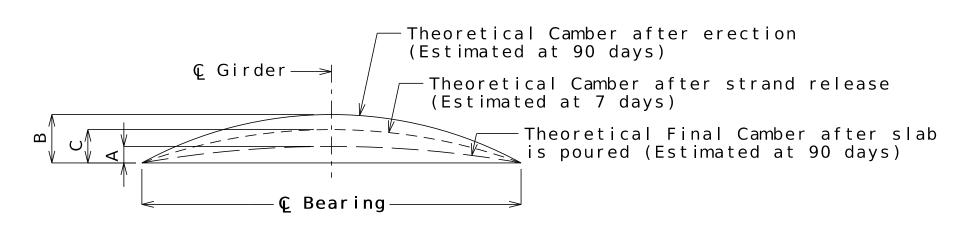
If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches shall be necessary. The haunch shall be limited to ensure the projecting girder reinforcement is embedded into the slab at least 2 inches.

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



TYPICAL SLAB ELEVATIONS DIAGRAM

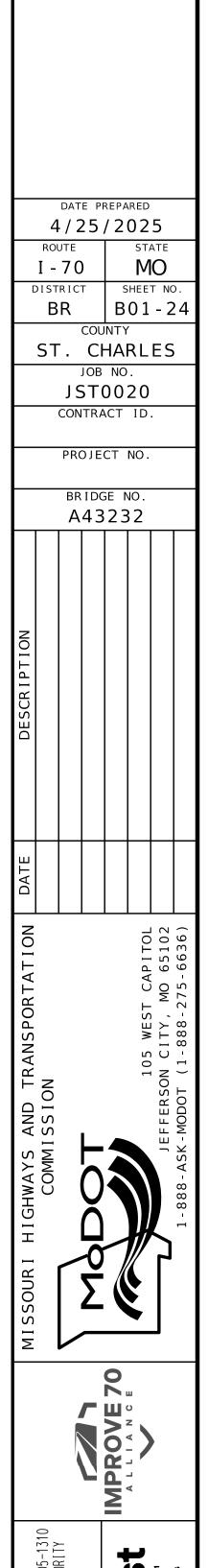




Girder	S	pan (1-2	2)	S	pan (2-3	3)	S	pan (3-4	4)
Girder	А	В	С	Α	В	С	А	В	С
6	<u>3</u> ::	<u>1</u> 11	<u>1</u> 11	1 "	15"	<u>7</u> 11	<u>3</u> "	<u>1</u> 11	<u>3</u>
7	<u>3</u>	2	2	1 "	18	8	<u>3</u> II	2	8

GIRDER CAMBER DIAGRAM

Conversion Factors for Girder Camber (Estimated at 90 days): $0.25 \text{ pt.} = 0.7125 \times 0.5 \text{ pt.}$



Detailed JUL 2025 Checked JUL 2025

DATE PREPARED 4/25/2025

ST. CHARLES

JOB NO.

JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A43232

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IMPROVE

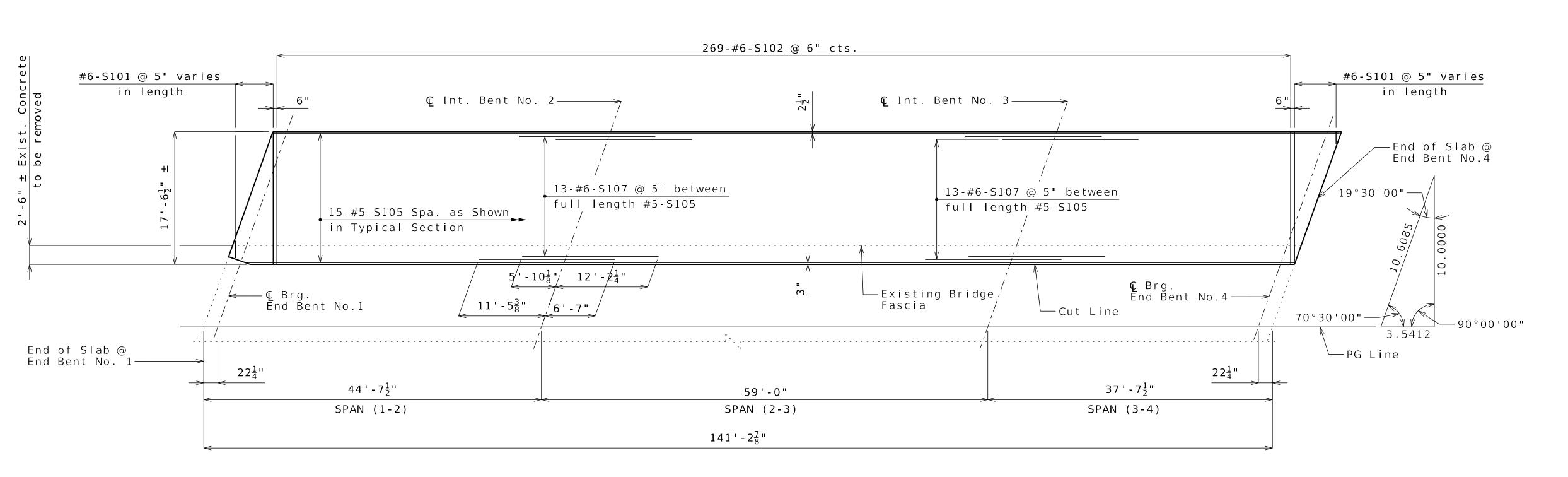
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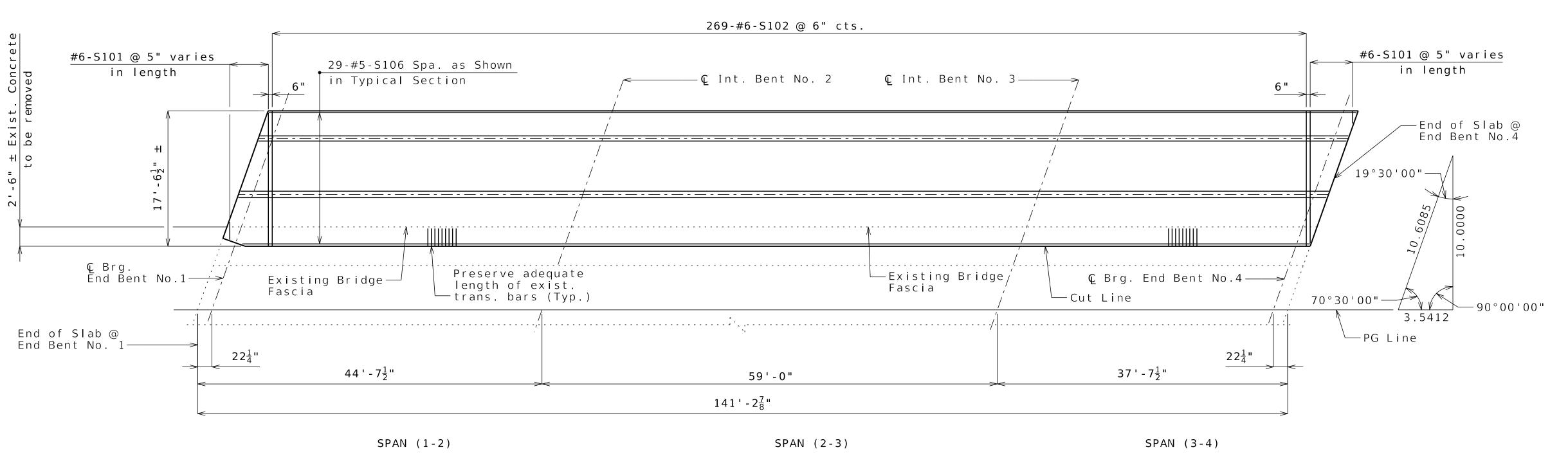
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SHEET NO. B01-25



TOP REINFORCING PLAN

Longitudinal dimensions are horizontal.



PLAN SHOWING LOCATION OF SLAB SHOWING BOTTOM REINFORCEMENT Longitudinal dimensions are horizontal.

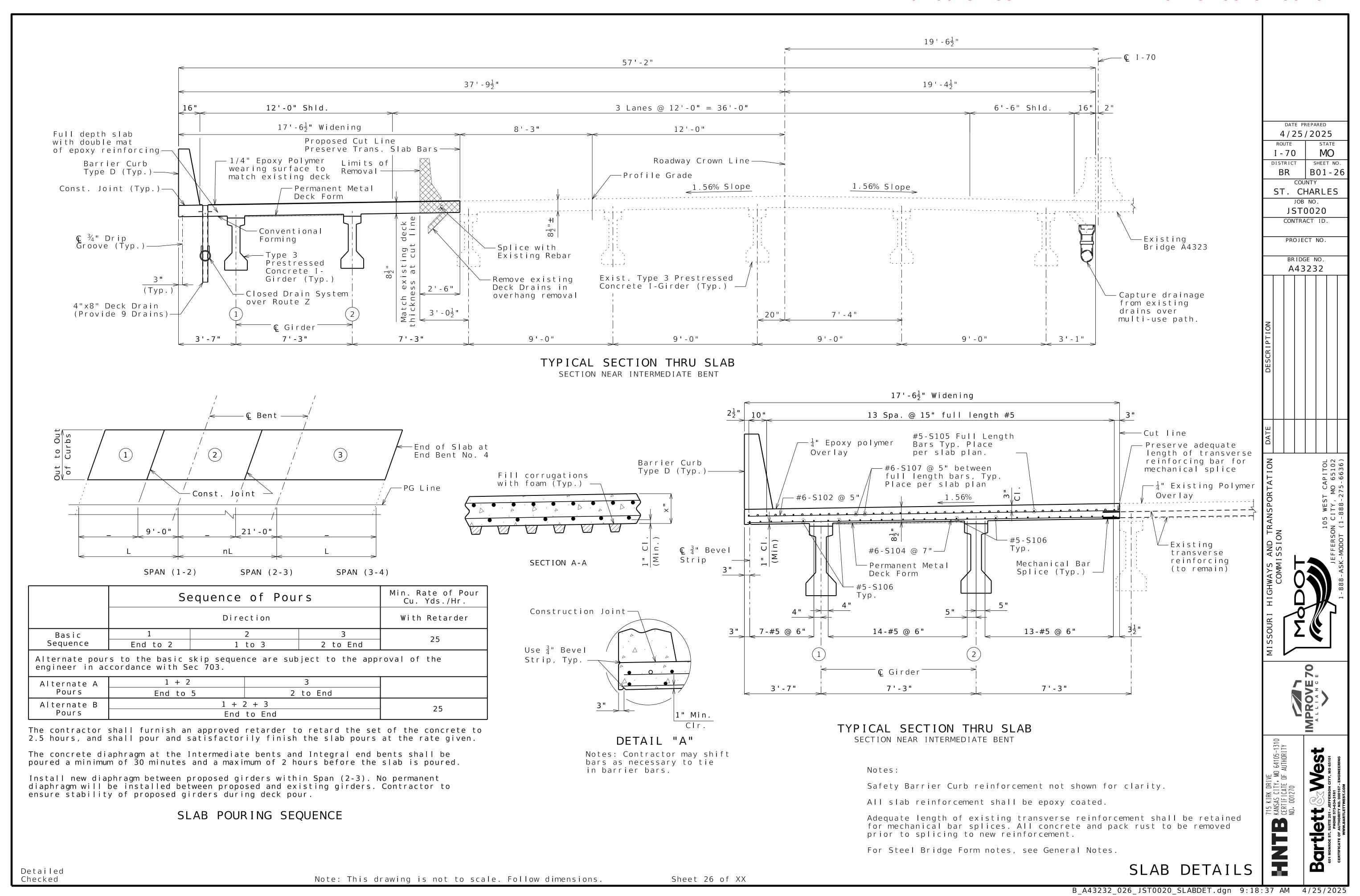
Notes:

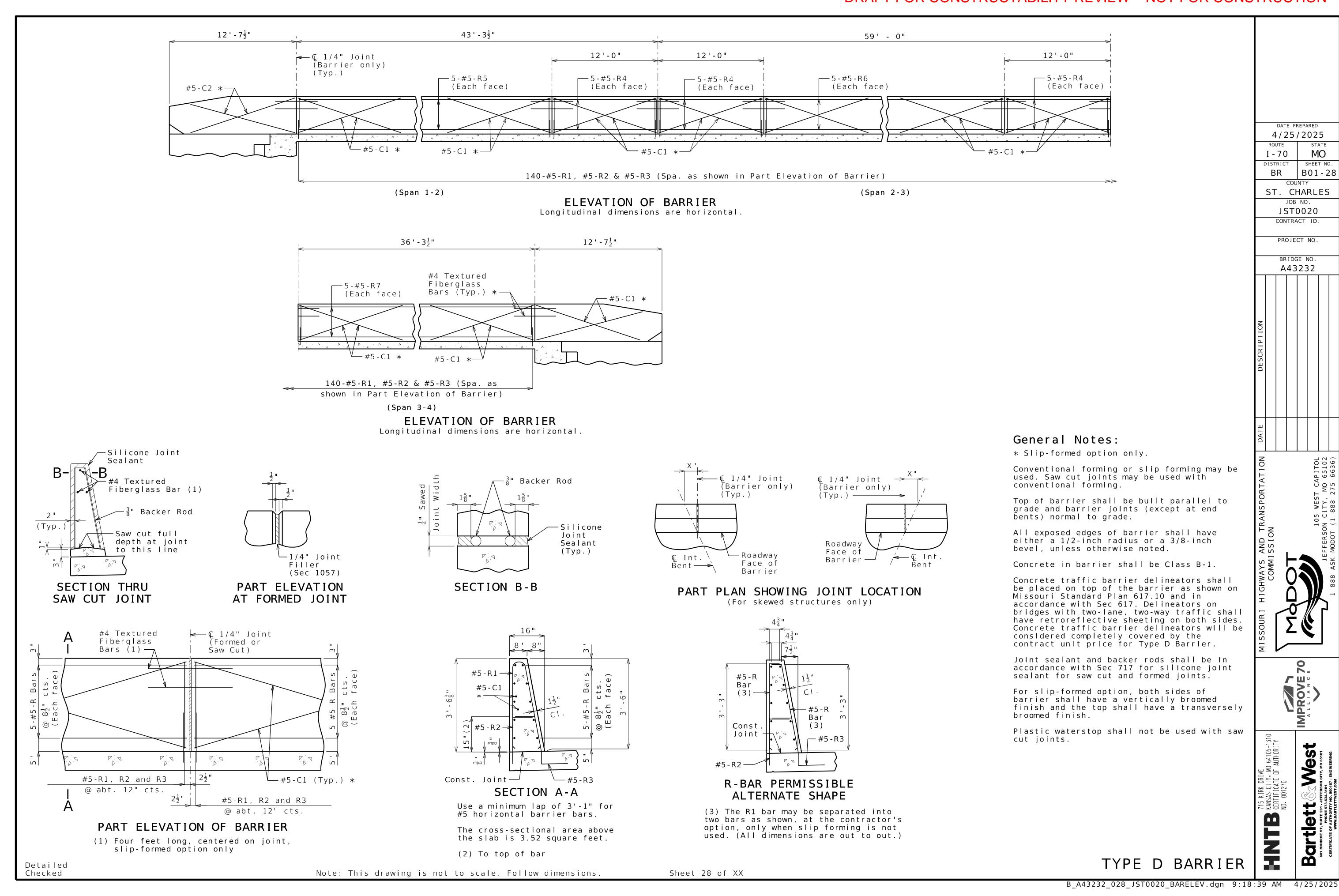
Longitudinal slab dimensions are measured horizontally. For Section Thru Slab and Slab Pouring Sequence, see Sheet No. XX.

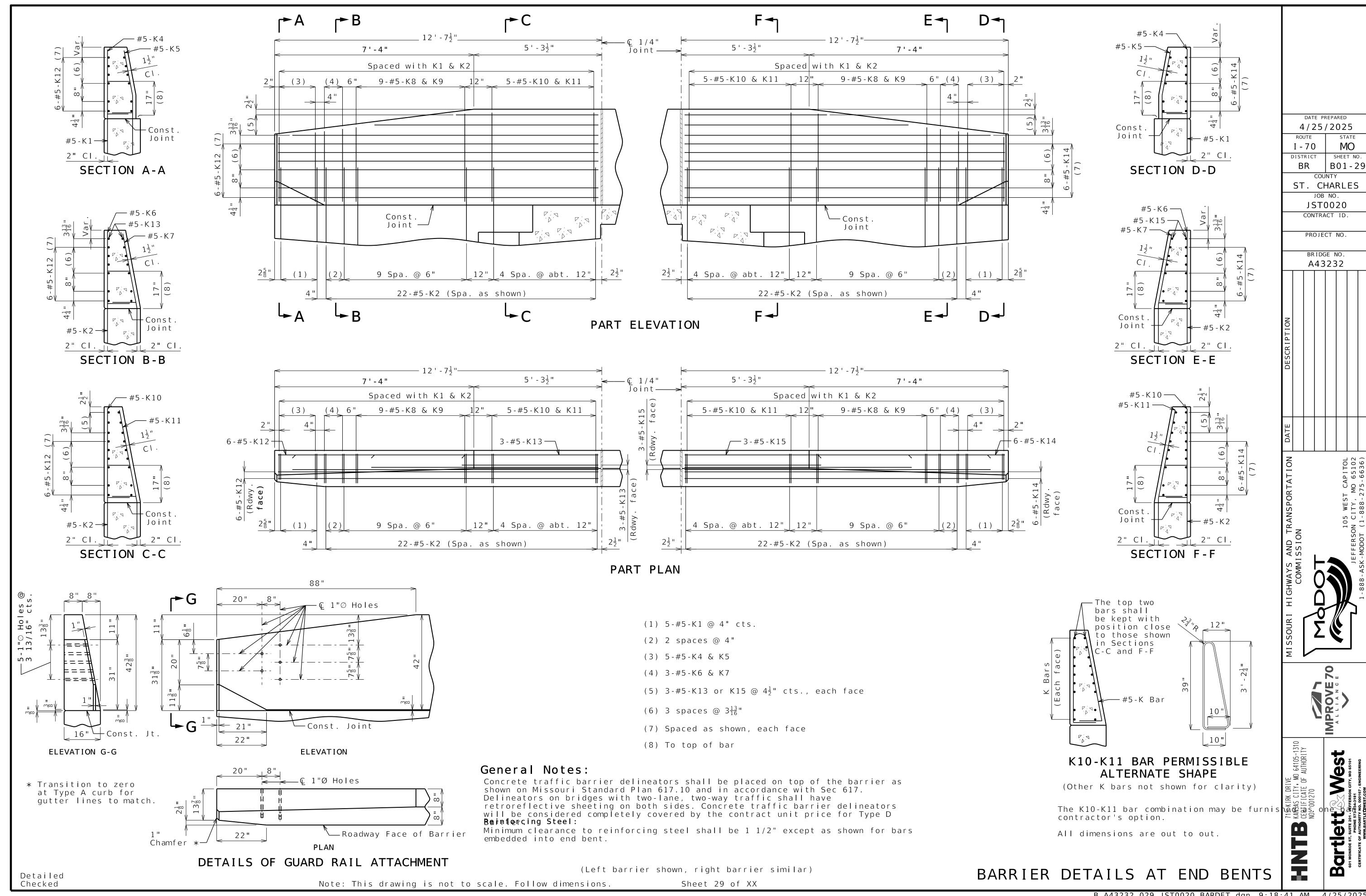
For Details and Reinforcement of Safety Barrier Curb not shown, see Sheets No. XX, XX & XX.

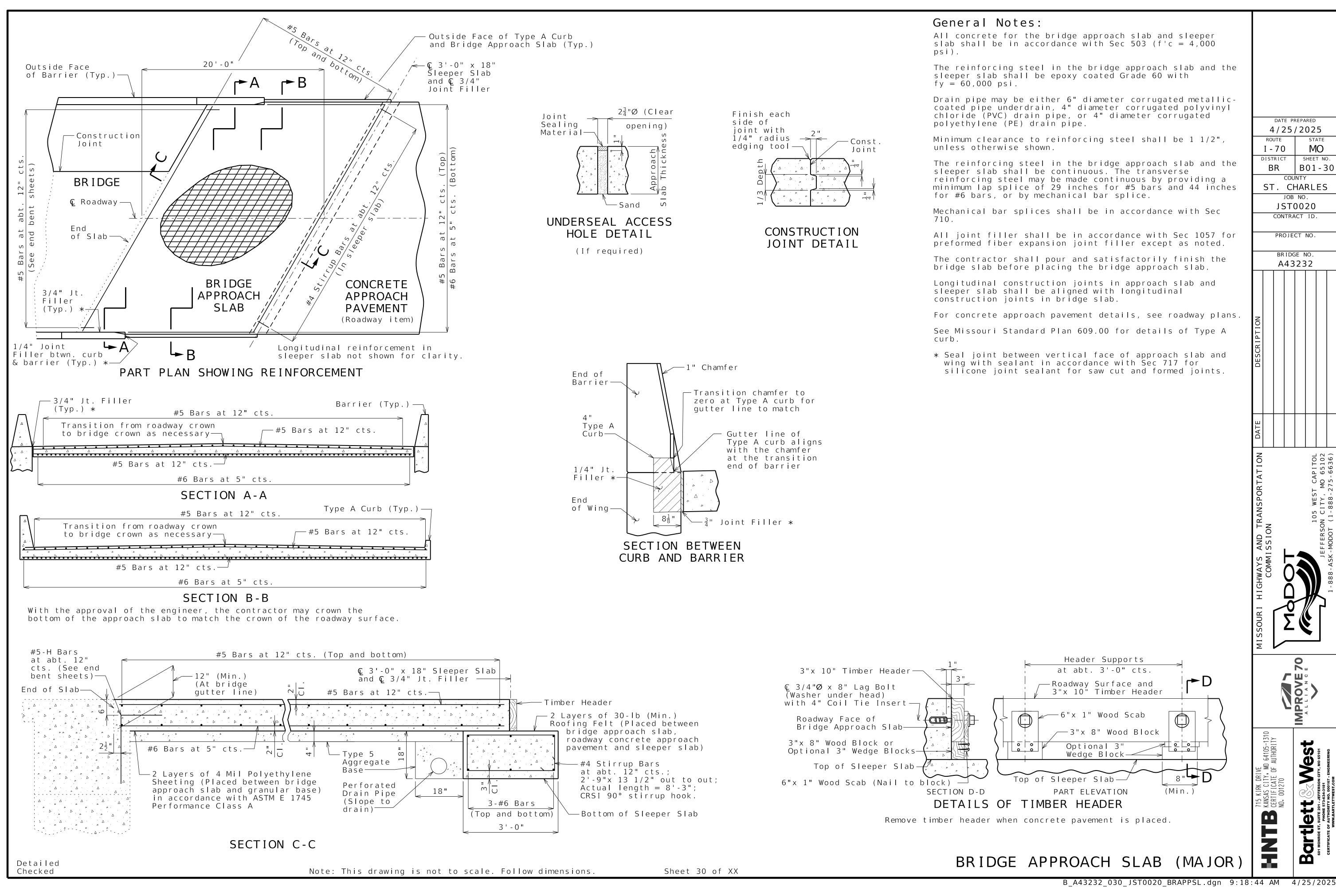
- For Theoretical Slab Haunching Diagram, see Sheet No. XX.
- For Details of Precast Prestressed Panels, see Sheet No. XX.
- For Theoretical Bottom of Slab Elevations, see Sheet No. XX.
- For details and locations of Slab Drains, see Sheet No. XX.

PLAN OF SLAB SHOWING REINFORCEMENT









DATE PREPARED

4/25/2025

COUNTY

JOB NO.

JST0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A43232

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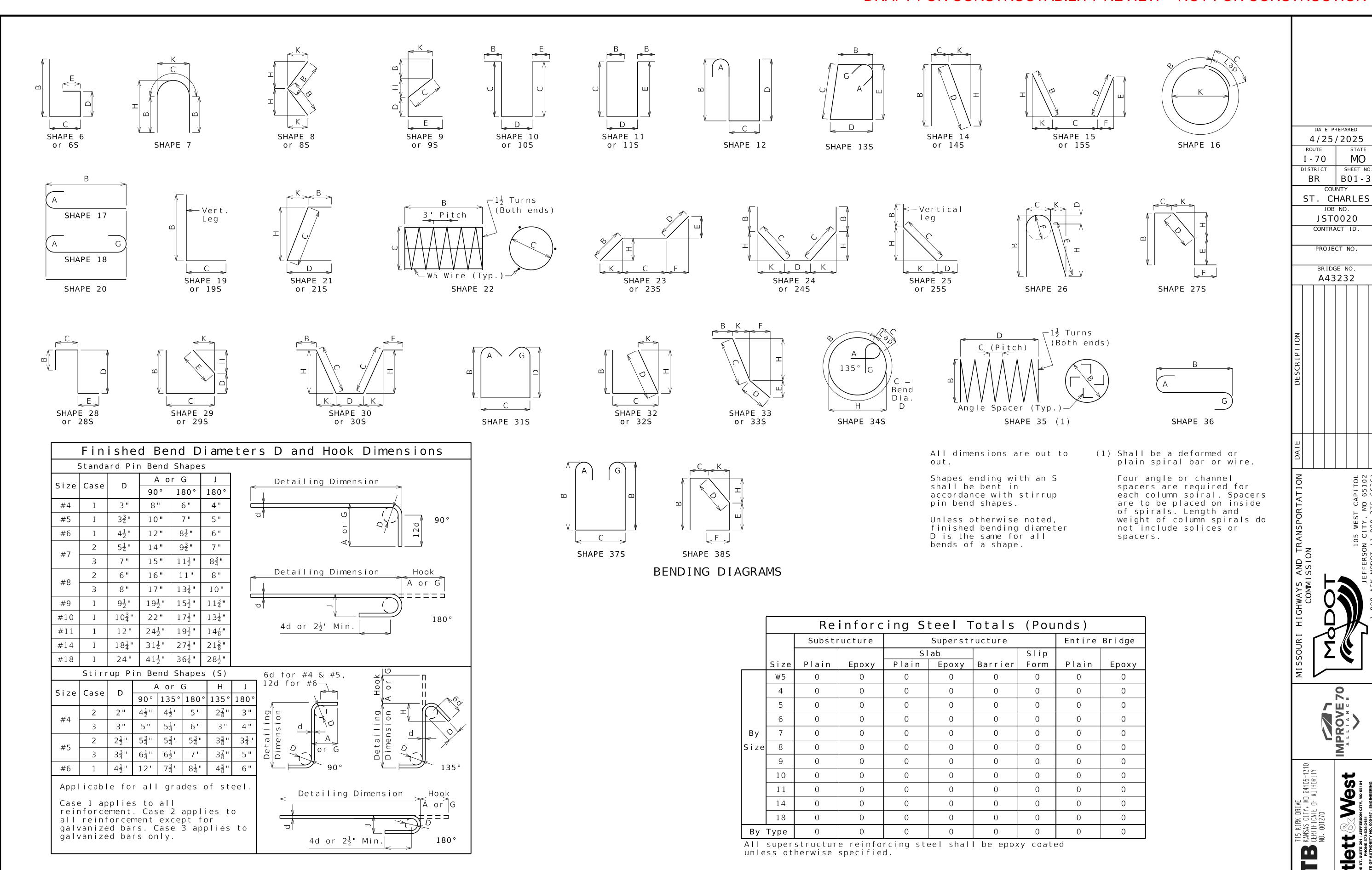
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SHEET NO. B01-31



Sheet 31 of XX

Note: This drawing is not to scale. Follow dimensions.

BAR BENDING DIAGRAMS

Bill of Reinforcing Steel Dimensions Nom. Actual								Bill of Reinforcing Steel Dimensions Nom. Actual										
No. Req.	l ———	Codes SH V	B C D E F ft in. ft in. ft in. ft in.	H K ft in. ft in.	Length	Length			Size/ Mark	Location	Codes C SH V	B C ft in. ft in.	D E	F H ft in. ft in.		Actual Length ft in.		
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	bending diagrams and steel	rein	forcing totals, see Sheet No		urmensio line and this lin	the fole and the	n equal lowing le follow	incr ine a ing l	ements ind the ine var	grams. f bars of each I between dimensio actual length di y by the specifi	ns snown mension ed incre	shown on ment.	זוו אר פי	EINFORCIN	C CTFF!	/ 1	E VI	B Cot MONE

Detailed Checked

Note: This drawing is not to scale. Follow dimensions. Sheet 32 of XX BILL OF REINFORCING STEEL (1 OF X)

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For I			forcing totals, see Sheet No		dimensio line and this lin	ns vary the fol e and th	ın equal lowing l e follov	linea Ving	rements and the line var	grams. f bars of each I between dimensio actual length din y by the specifi	ns shown mension ed increi	on this shown on ment.		EINFORCIN	C CTCC	()	F \/\	Bar Monk

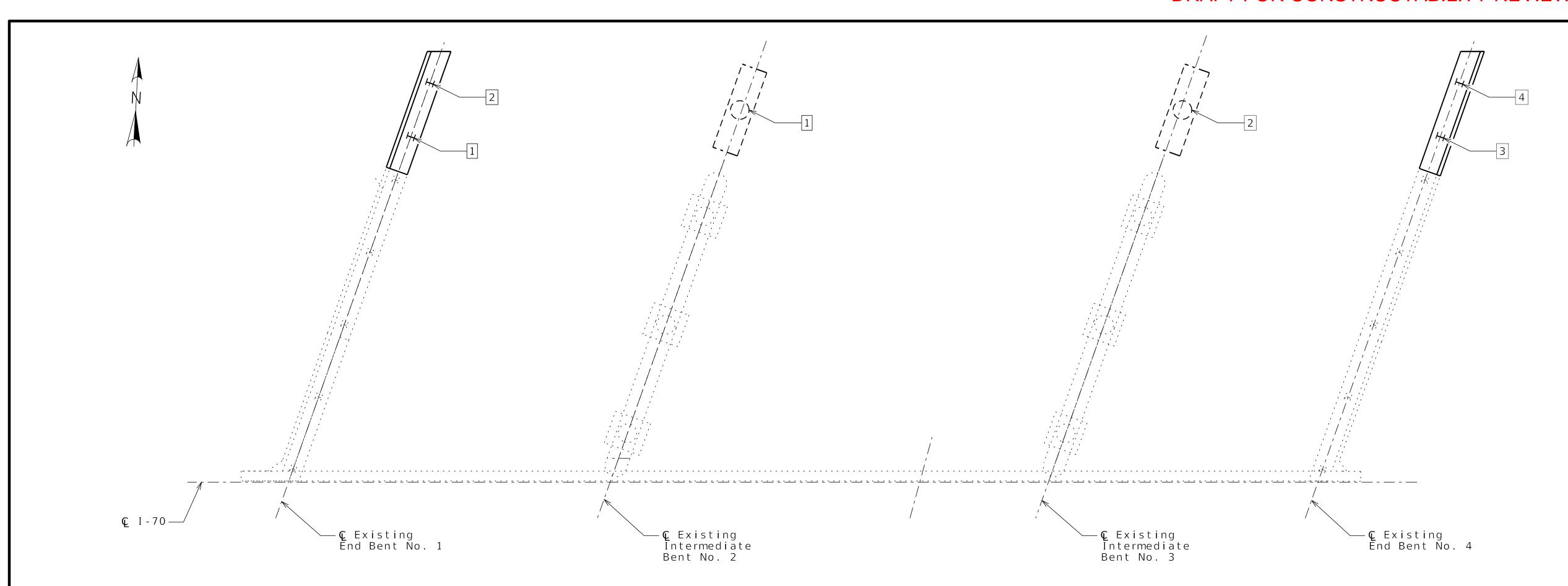
Detailed Checked

Note: This drawing is not to scale. Follow dimensions. Sheet 33 of XX BILL OF REINFORCING STEEL (2 OF X)

Bill of Reinforcing Steel Dimensions Nom. Actual							Ι			Billo	of Reinforci		Nom	Actual				
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Note: This drawing is not to scale. Follow dimensions. Sheet 34 of XX BILL OF REINFORCING STEEL (3 OF X)



DABT	PLAN SHOWING	: PII F	AND DETLIED	SHAFT	MILIMBERING	$F \cap R$	RECORDING	$\Delta S - RIIII T$	PIIF	$D \Delta S$	- RIIII T	SHAFT	DATA
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			As-Built Pile Data
Pile No.	Length in Place (ft)	Compressive	R ema r k s
			End Bent No.1
1			
2			
			End Bent No.4
3			
4			

			As-Bui	ilt Drilled Shaft Data
Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	R ema r k s
				Intermediate Bent No.2
1				
				Intermediate Bent No.3
2				

Note: Indicate in remarks column: A. Pile type and grade B. Batter C. Driven to practical refusal

This sheet to be completed by the design-builder.

AS-BUILT PILE AND DRILLED SHAFT DATA

DATE PREPARED 4/25/2025

BR B01-35 COUNTY ST. CHARLES JOB NO. JST0020 CONTRACT ID.

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Standard Drawing Guidance:	
Standard Drawing Guidance: (Do not show on plans) See Instructions & Tips. MicroStation &	
See Instructions & Tips, MicroStation & Projectwise, AttachBoringPDFsToBridgePlans on Development Section Sharepoint page for instructions for attaching PDFs as rasters.	DATE PREPARED
	4/25/2025 ROUTE STATE I - 70 MO DISTRICT SHEET NO
For one 11x17 Geotechnical Data sheet, snap to top left corner of left guidance box and snap anywhere for other corner, filling as much of the available space as possible. Delete boxes or turn off Bridge-Guidance level.	BR B01-3 COUNTY ST. CHARLES
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Detailed Checked

Note: For locations of borings, see Sheet No. 2.