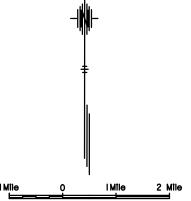
YEAR SHEET NO.

**GRADING & SURFACING (ASPHALT)** BRIDGE REPAIR **FENCING** SEEDING PAVEMENT MARKING



# PREPARED & SUBMITTED BY

David E. Jacobson, P.E. Director of Engineering

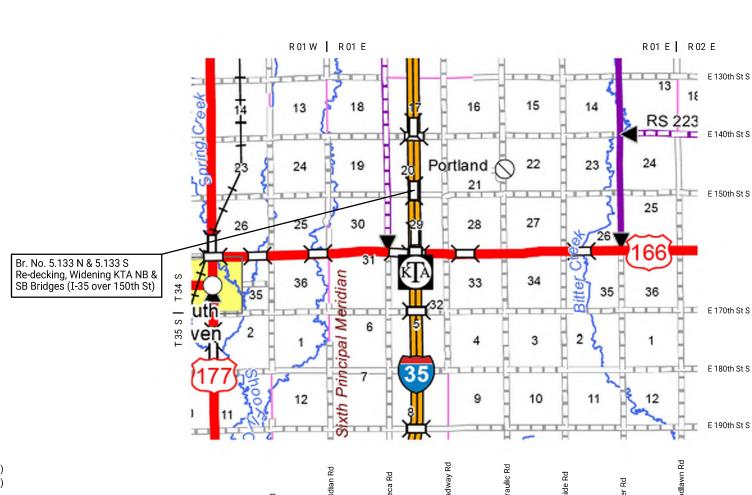
Kansas Turnpike Authority

12/8/2023

## Kansas Turnpike Authority

Contract No. 8008 Sumner County, Kansas

BR. NO. 5.133 N & 5.133 S



#### DESIGN DESIGNATION

**INDEX OF SHEETS** 

1. Title Sheet 2.-3. Typical Sections

4.-9. Plan & Profile

56.-58. Fencing

68. Seeding 69.-70. Pavement Marking

71.-83. Traffic Control

90.-103. Cross Sections

10. Pavement Details 11. Inlet Details 12.-22. Guardrail

23.-26. Concrete Safety Barrier 27.-52. Br. No. 5.133 N & 5.133 S 53. Bridge Excavation (LRFD) 54. Standard Pile Details

> 59. Summary of Quantities 60. Project Surfacing

55. Supports and Spacers for Reinforcing Steel

61.-67. Temporary Erosion & Pollution Control

84.-88. Concrete Safety Barrier (Temporary) 89. Impact Attenuator (Temporary)

ADT	=	15,900 VPD (2020)
ADT DESIGN	=	17,600 VPD (2040)
DESIGN SPEED	=	75 MPH (FINAL)
Т	=	30%
C OF A	=	FULL
CLEAR ZONE	=	38'
D	_	_

#### CONVENTIONAL SIGNS

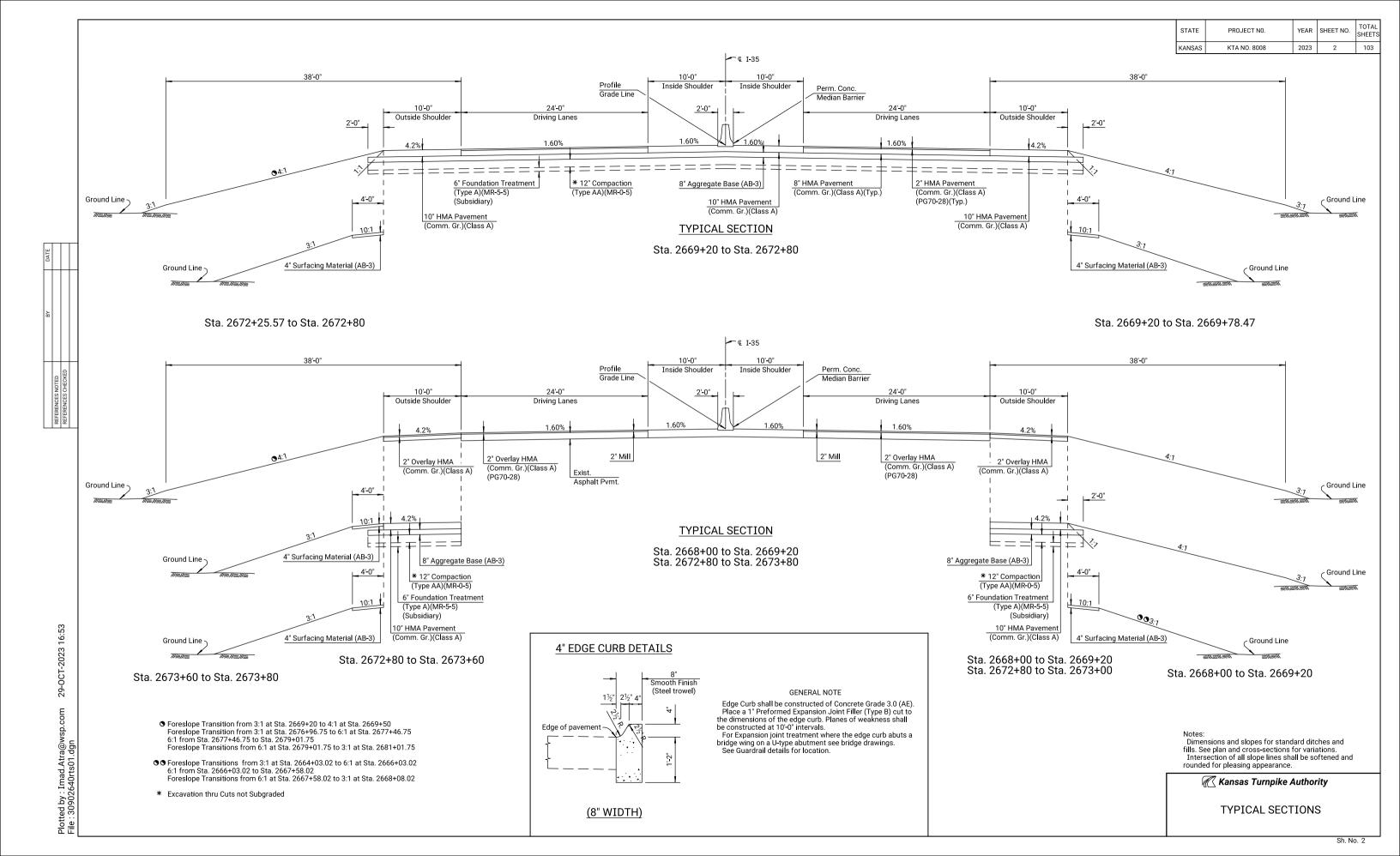
COUNTY LINE	CENTER LINE OF PROJECT	50
CITYLIMITS	TERRACE	1
STATE OR NATIONAL LINE	CULVERTS	<u> </u>
TOWNSHIP, SECTION or GRANT LINE	DROP INLET & STORM SEWER	
PROPERTY LINE	ACCESS CONTROL	سست
HIGHWAY FENCE	POWER POLE	ò
EXISTING FENCE	TELEPHONE POLE	ф
GUARDRAIL	MARSH	-822-
CONSTRUCTION LIMITS	HEDGE	
RIGHT OF WAY LINE	TREES	
TRAVELED WAY	PROFILE ELEVATION	
RAILROADS	STREAM or CREEK	

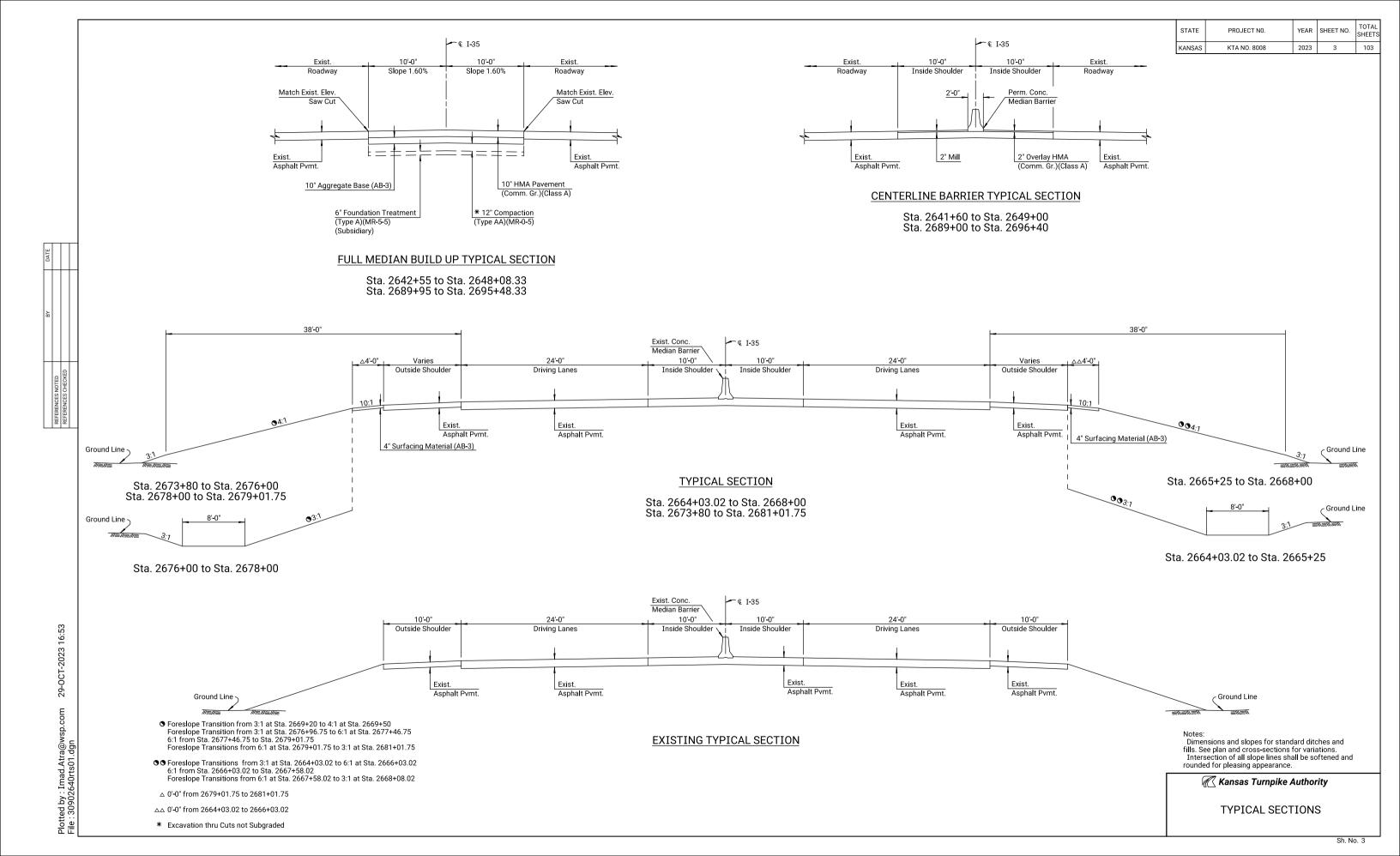
EXCEPTIONS	-	FT.			
ADDITIONS	-	FT.			
NET LENGTH OF PROJECT	1698.73	FT.	0.322	MILES	
NET LENGTH OF BRIDGES	220	FT.	0.042	MILES	
NET LENGTH OF ROAD	1478.73	FT.	0.280	MILES	

1698.73 FT.

GROSS LENGTH OF PROJECT







UTILITY SERVICE LINES, POLES, VALVE BOXES, METERS, ETC. ARE TO BE ADJUSTED AS NECESSARY BY THE OTHERS PRIOR TO CONSTRUCTION UNLESS OTHERWISE SHOWN ON THE PLANS.

EXISTING UTILITIES AND THEIR LOCATION, AS SHOWN ON THE PLANS. REPRESENT THE BEST AS COULD BE DETERMINED. THE CONTRACTOR SHALL BE REQUIRED TO WORK AROUND ANY EXISTING UTILITIES WITHIN THE RIGHT-OF-WAY WHICH DO NOT CONFLICT WITH THE PROPOSED CONSTRUCTION

THE CONTRACTOR SHALL BE AWARE THAT HE/SHE WILL BE WORKING IN CLOSE PROXIMITY OF EXISTING LITTLITIES. ANY CONFLICTS WITH SUCH UTILITIES SHALL BE REPORTED TO THE ENGINEER. THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION OF THE PROJECT WITH THE RELOCATION OF ANY EXISTING UTILITIES BY THE UTILITY COMPANY.

THE CONTRACTOR WILL BE RESPONSIBLE TO COORDINATE WITH THE UTILITY OWNER TO LOCATE AND FLAG ALL UNDERGROUND UTILITIES PRIOR TO EXCAVATION TO AVOID DAMAGING UTILITIES. THE CONTRACTOR WILL BE REQUIRED TO PROVIDE A MINIMUM ADVANCE NOTICE OF 72 HOURS TO UTILITY COMPANIES PRIOR TO EXCAVATION OR WORKING ADJACENT TO

ALL BORROW TO BE OBTAINED FROM AREAS PROVIDED BY THE CONTRACTOR SHALL BE APPROVED BY THE ENGINEER, BOTH AS TO THE SUITABILITY OF MATERIAL AND SITE LOCATION. LOCATIONS WHICH, IN THE OPINION OF THE ENGINEER, CONTAIN UNSUITABLE MATERIAL OR WILL LEAVE AN UNSIGHTLY APPEARANCE ON THE PROJECT WILL NOT BE

A GROSE VMF OF 0.87 FOR THE SOIL MANTLE HAS BEEN COMPUTED TO INCLUDE QUANTITIES FOR INITIAL CONSOLIDATION AND SETTLEMENT. EXCAVATION SHOWN TO BE WASTED SHALL BE WASTED ON SITES PROVIDED BY THE CONTRACTOR. THE SITES SHALL BE APPROVED BY THE ENGINEER AS TO SUITABILITY, APPEARANCE, AND SITE LOCATION. LOCATIONS THAT, IN THE OPINION OF THE ENGINEER, WILL LEAVE AN UNSIGHTLY APPEARANCE WILL NOT BE APPROVED.

ALL TREES, HEDGE ROWS, SHELTERBELTS, AND WOODY SHRUBS NOT SHOWN TO BE REMOVED AND LOCATED BETWEEN THE CONSTRUCITON LIMITS AND RIGHT-OF-WAY LINE SHALL BE REMOVED UNLESS DIRECTED BY THE ENGINEER TO BE SPARED. ALL TREES WITHIN THE APPROPRIATE CLEAR ZONE SHALL BE REMOVED.

UNLESS OTHERWISE NOTED TO BE MILLED, REMOVAL OF EXISTING SURFACE WILL BE PAID FOR UNDER THE BID ITEM "ROCK EXCAVATION" STANDARD SPECIFICATIONS.

MISCELLANEOUS REMOVALS, INCLUDING ROCK EXCAVATION, REQUIRED FOR THE PLACEMENT OF PAVING, TEMPORARY BARRIER AND TEMPORARY GUARDRAIL SHALL NOT BE PAID FOR DIRECTLY BUT SHALL BE SUBSIDIARY TO OTHER REMOVAL "BID ITEMS".

ALL SAWCUTS SHALL BE FULL DEPTH AND SHALL NOT BE PAID FOR DIRECTLY BUT SHALL BE SUBSIDIARY TO OTHER PAVEMENT "BID ITEMS".

ALL MATERIAL FROM MILLING IS TO BECOME THE PROPERTY OF THE CONTRACTOR.

EXISTING GUARDRAIL WHICH IS REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTOR.

EXISTING CONCRETE SAFETY BARRIER WHICH IS REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTOR.

TEMPORARY CONCRETE SAFETY BARRIER SHALL BE PROVIDED BY THE KTA. THE CONTRACTOR SHALL COORDINATE WITH THE KTA FOR THE DELIVERY AND USE OF TEMPORARY CONCRETE SAFETY BARRIER FROM THE WELLINGTON MM 19 AND K-15 MM 45 STOCKPILE SITES.

THE CONTRACTOR SHALL COORDINATE TRAFFIC CONTROL WITH OTHER CONSTRUCTION AND MAINTENANCE PROJECTS WITHIN THE AREA THE CONTRACTOR SHALL PROVIDE, ERECT, AND MAINTAIN TRAFFIC

CONTROL DEVICES IN ACCORDANCE WITH THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.) AS SHOWN ON THE TRAFFIC CONTROL PLANS AND SUBJECT TO THE ENGINEER'S APPROVAL. ACCESS TO RESIDENCES AND PROPERTIES SHALL BE MAINTAINED AT ALL TIMES.

PRIOR TO BIDDING, EACH BIDDER SHALL VISIT THE SITE AND SATISFY THEMSELVES OF SURFACE & SUBSURFACE CONDITIONS. EACH BIDDER SHALL ALSO FULLY INFORM THEMSELVES AS TO THE EXTENT OF THE SCOPE OF WORK TO BE PERFORMED.

THE EXISTING GRANULAR BASE MATERIAL SHALL BE REMOVED AND PAID FOR AS "COMMON EXCAVATION." THIS MATERIAL MAY BE USED IN THE EMBANKMENTS, BUT SHALL NOT BE WITHIN THE TOP 18" UNDER PAVEMENT OR THE TOP 6" OF GRADING SLOPES TO BE SEEDED.

THE CONTRACTOR SHALL COORDINATE WITH THE KTA FOR THE REMOVAL AND RELOCATION OF BRIDGE DECK SENSORS.

P.I. © Sta. 2595+01.85 N 152,216.273 E 18,547,189.850

UTILITIES Communication<sub>1</sub>

Chickasaw Telecommunications (Indian Nation Fiber Optics) Àttn. Keith Merrill PH: 1-405-533-2211 (0) PH: 1-405-269-1532 (C)

Communication<sub>2</sub>

Kansas Turnpike Authority 9401 F Kellog Wichita KS 67207 PH: 1-316-682-4537

Water<sub>1</sub>

Sumner Co. RWD #4 Attn. Briant Osborne Arkansas City, KS 67005 PH: 1-620-441-7683

PROJECT SURVEY CONTROL

Horizontal and Vertical project control is based on OPUS solutions. Two control points were set along the project area. Each point was observed twice on different days for a minimum of four hours.

Hor. Datum: KRCS Zone 18 Arkansas City Vert. Datum: NAVD 1988

Datum Bench Mark #10: "T" Post 0.2' Below Ground 49.12' Rt. at Sta. 2661+76.69 Flev 1166 02 NAVD 1988

STATE

KANSAS

PROJECT NO.

KTA NO. 8008

Communication 1 Indian Nation Fiber Optics Communication<sub>2</sub> Kansas Turnpike Authority

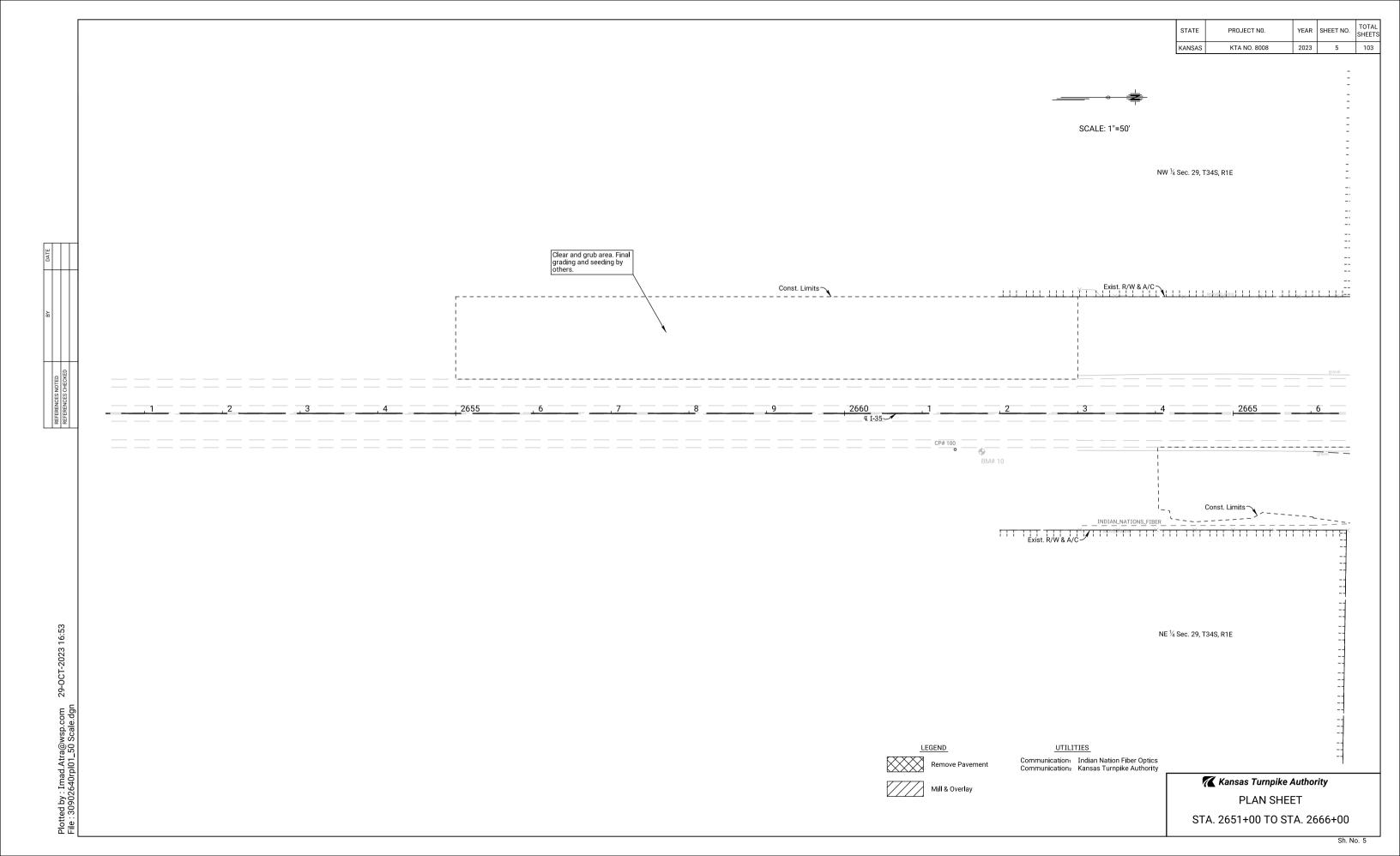
UTILITIES

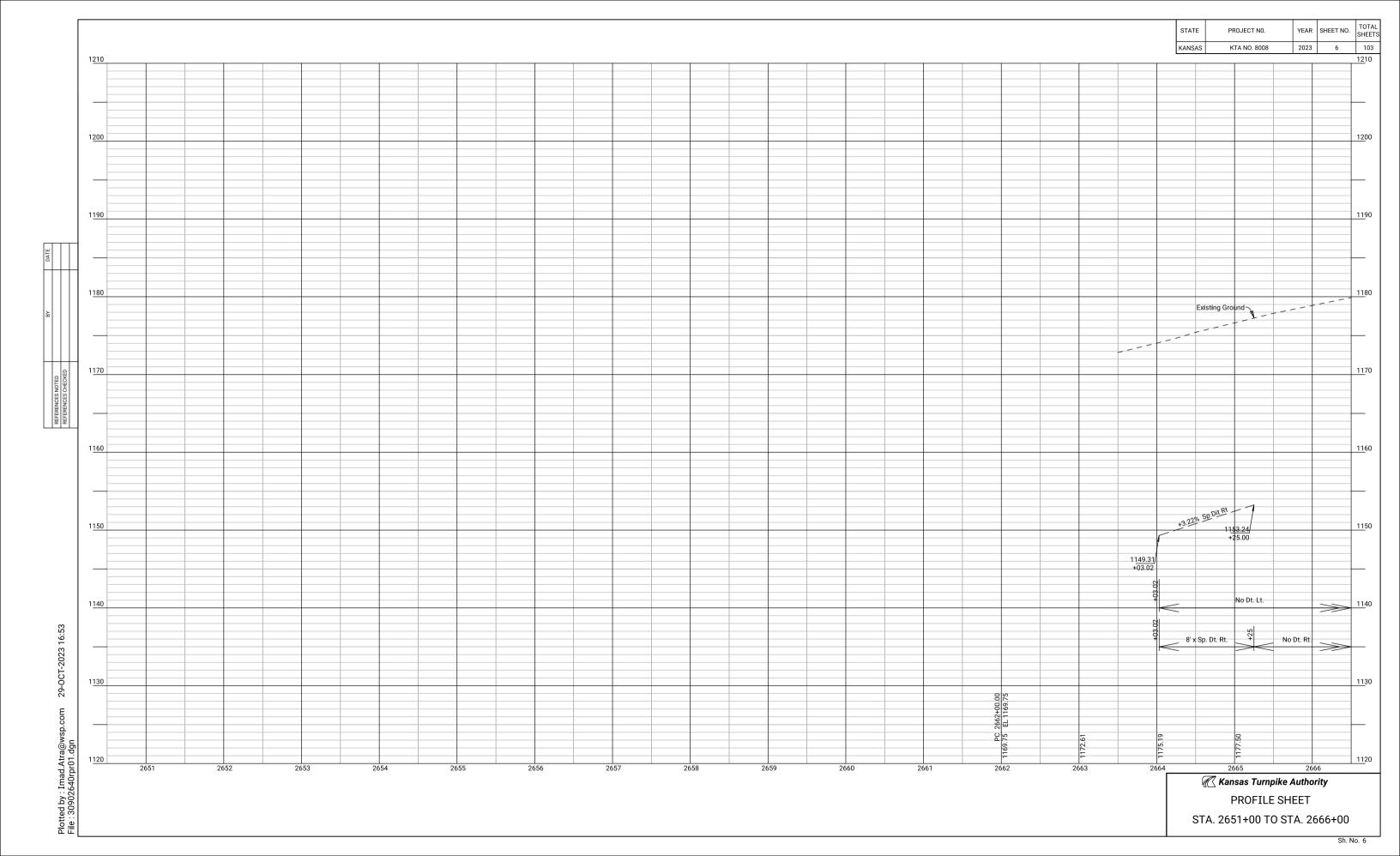
Kansas Turnpike Authority

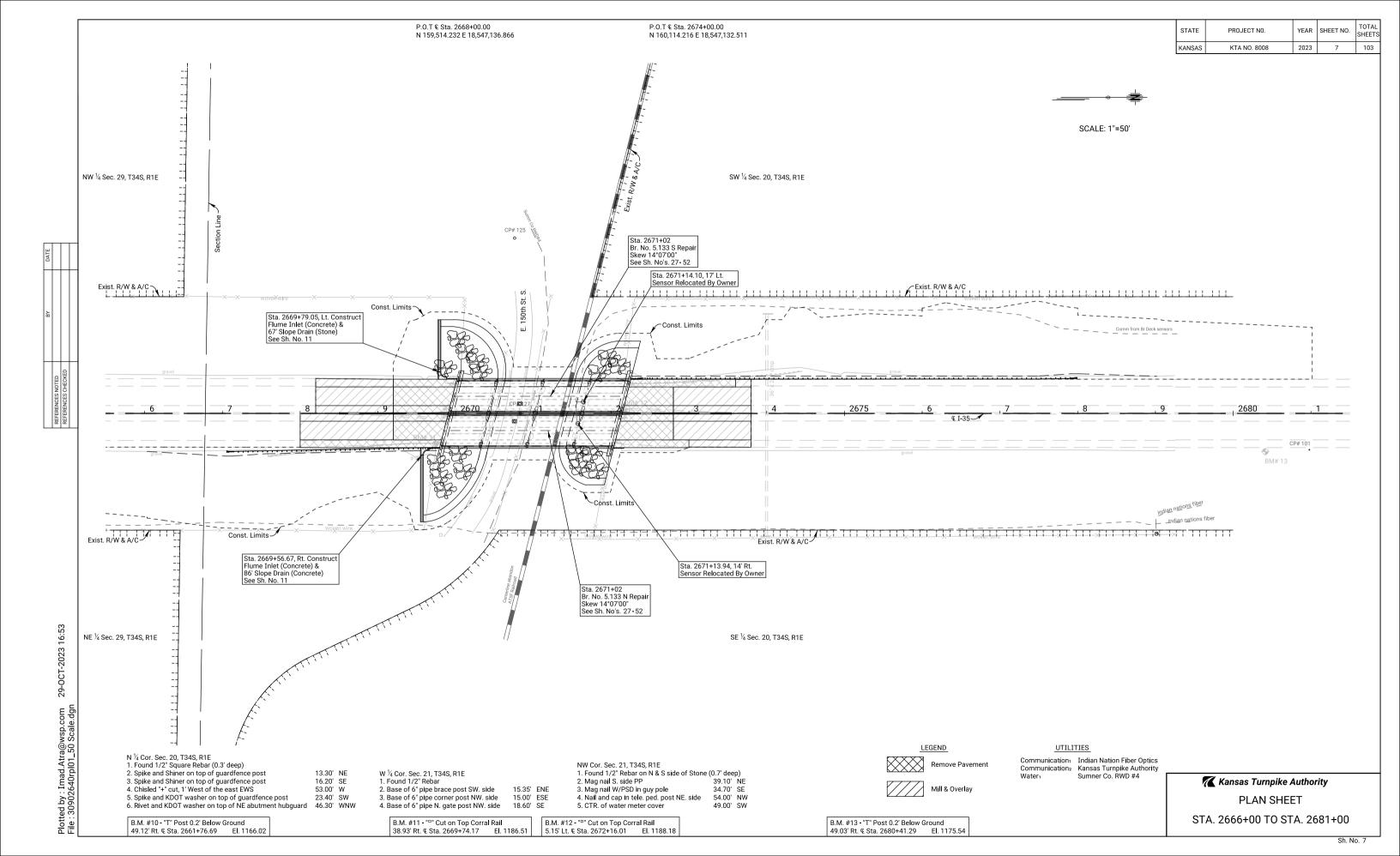
PLAN SHEET STA. 2640+00 TO STA. 2651+00

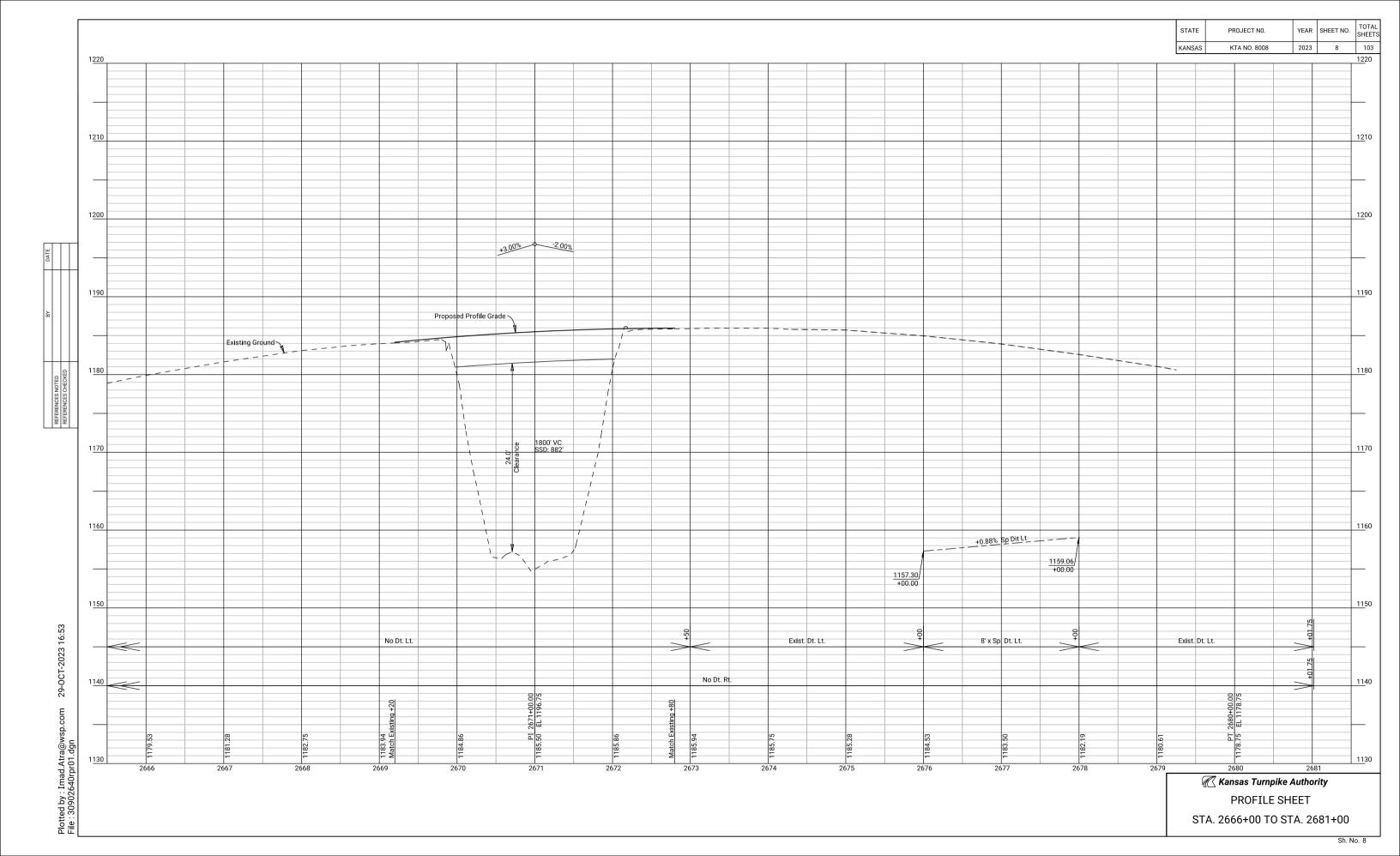
YEAR SHEET NO. SHEETS

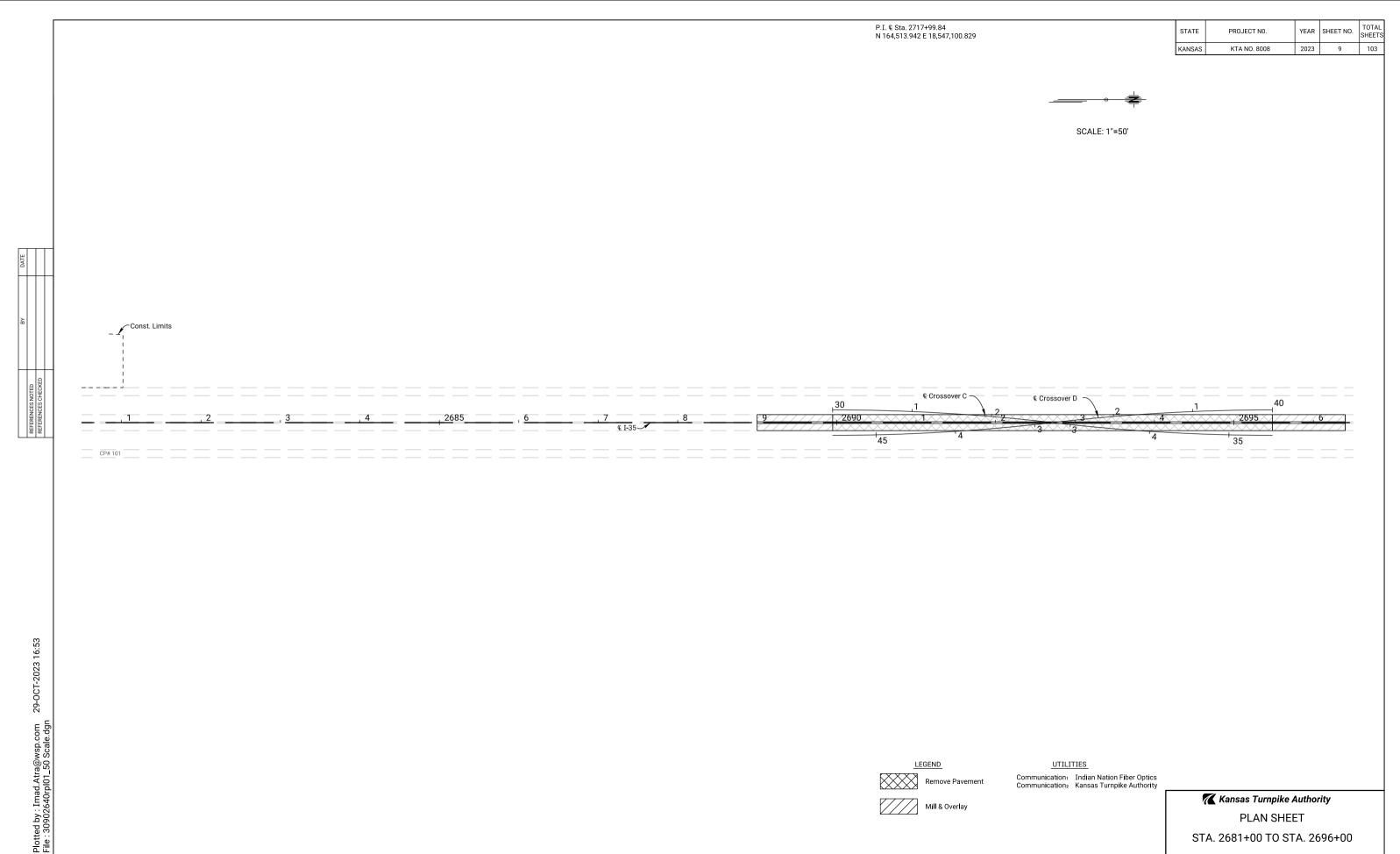
2023

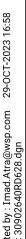


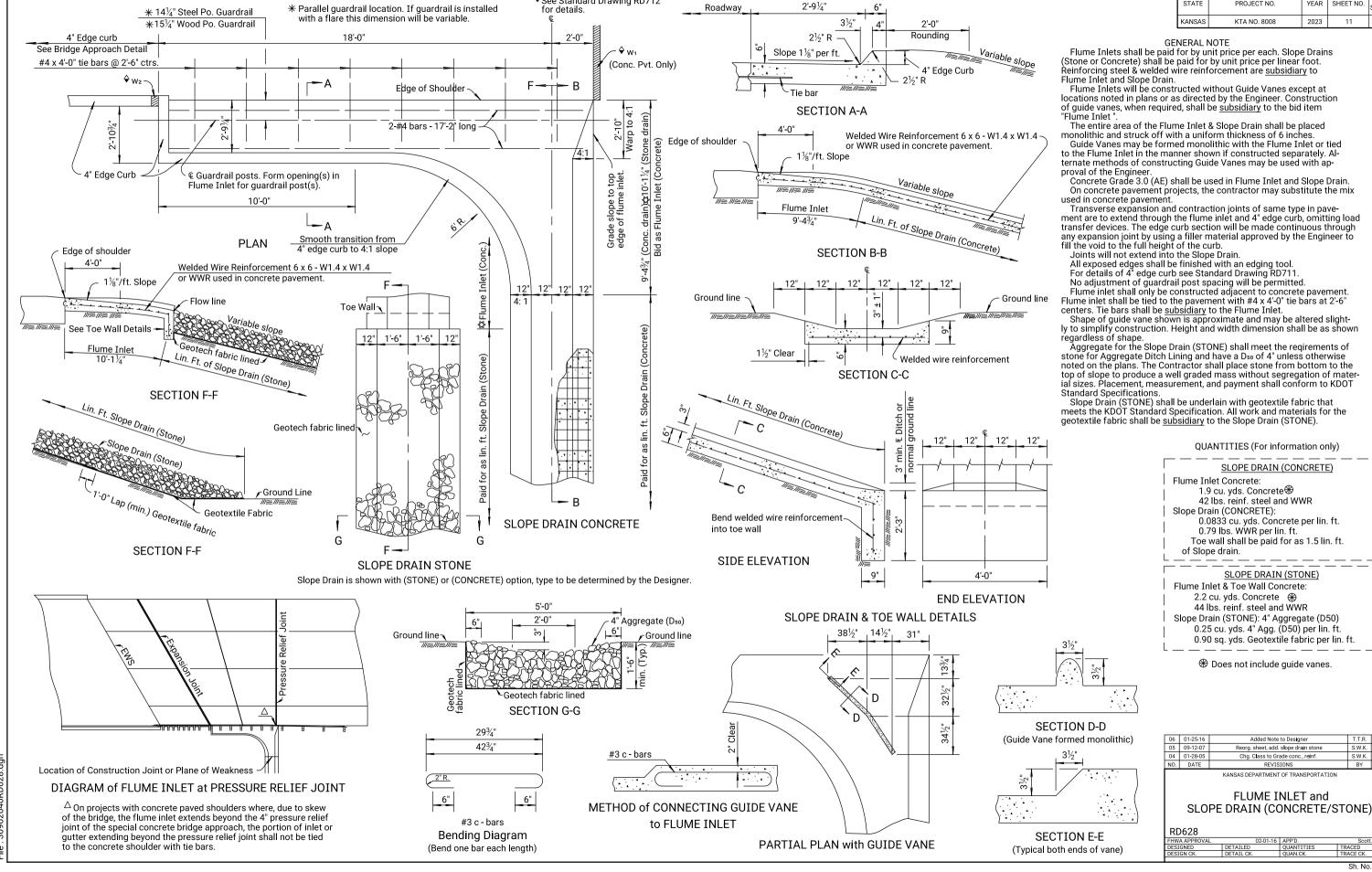












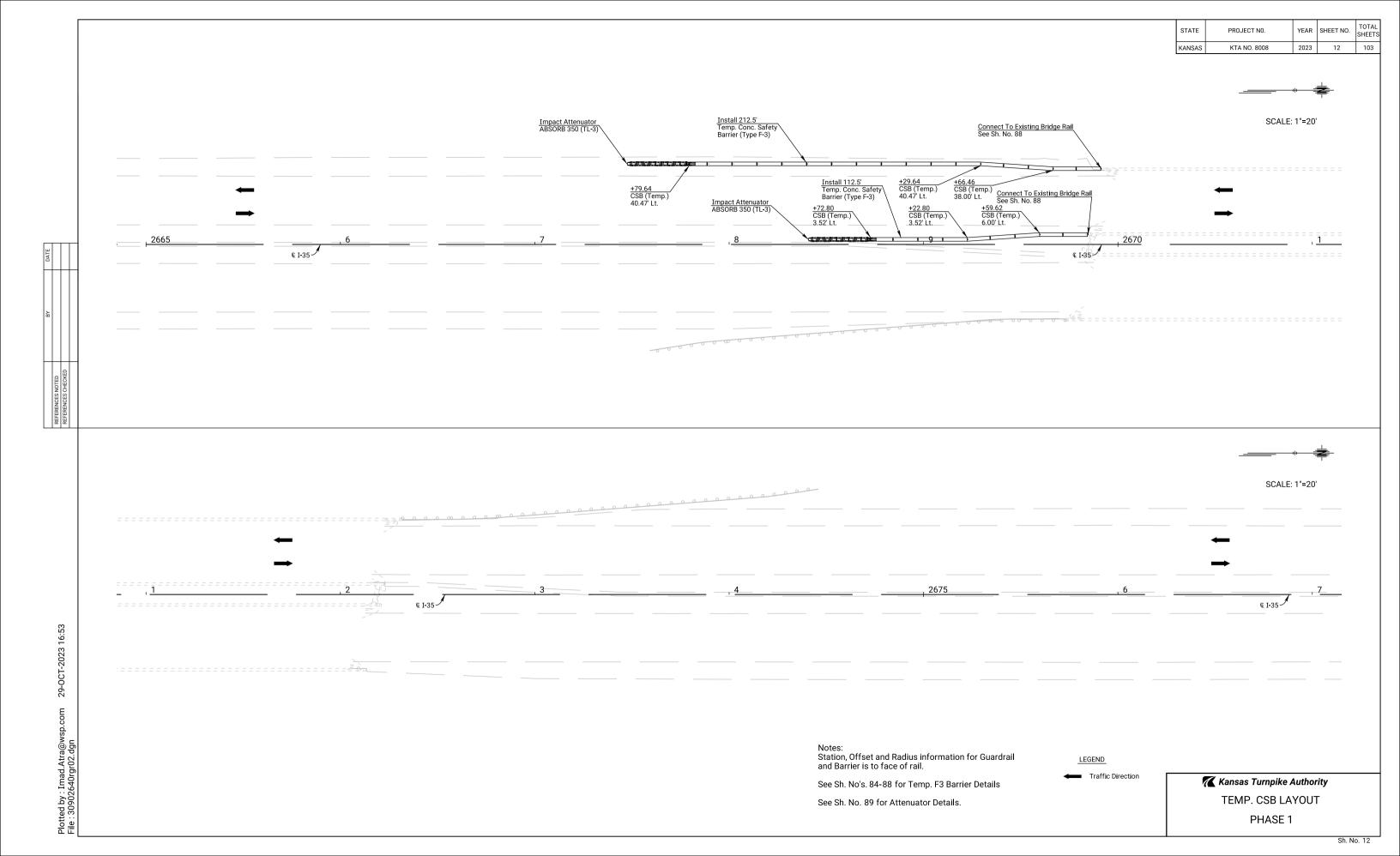
♦See Standard Drawing RD712

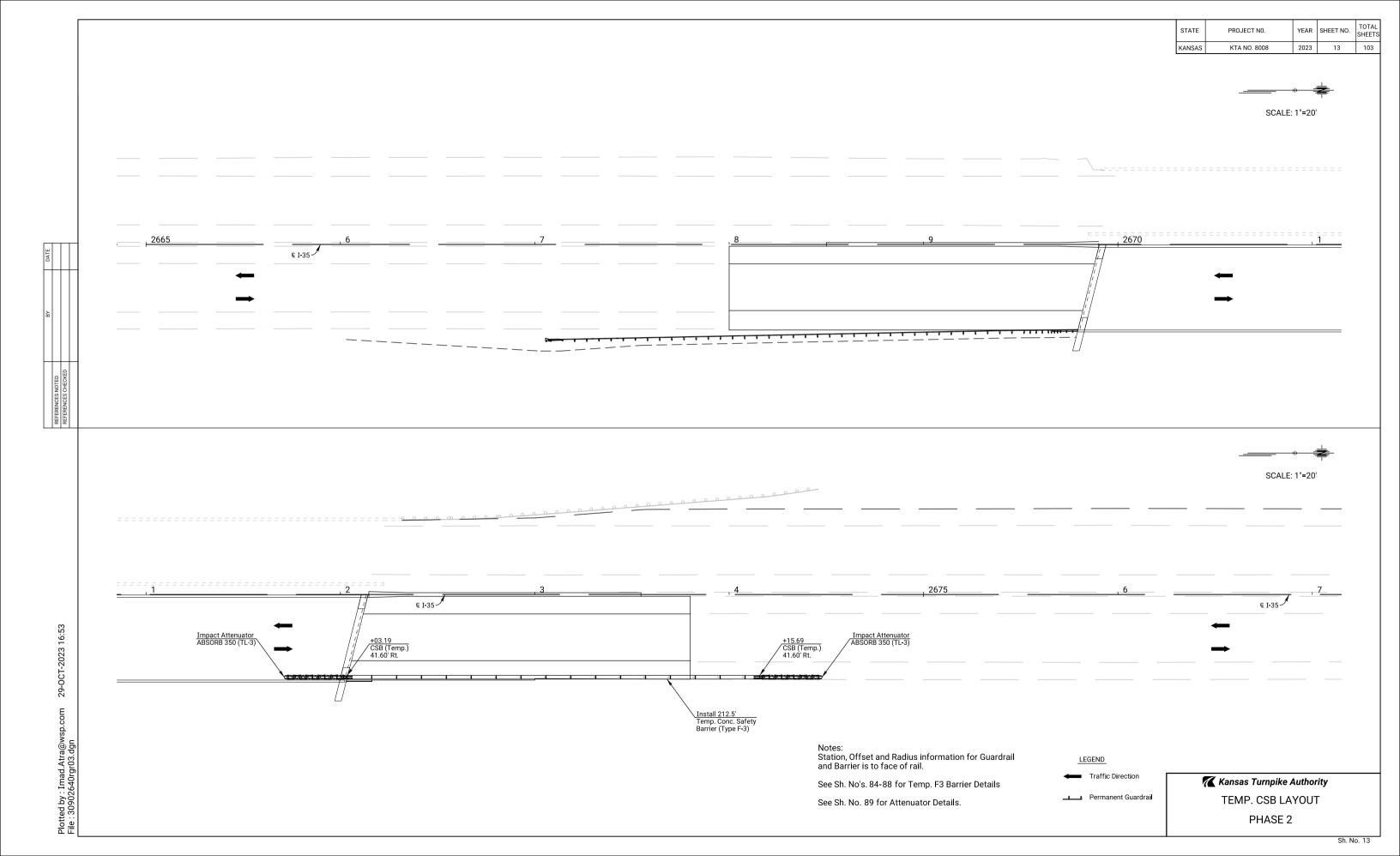
S.W.K. J.O.B.

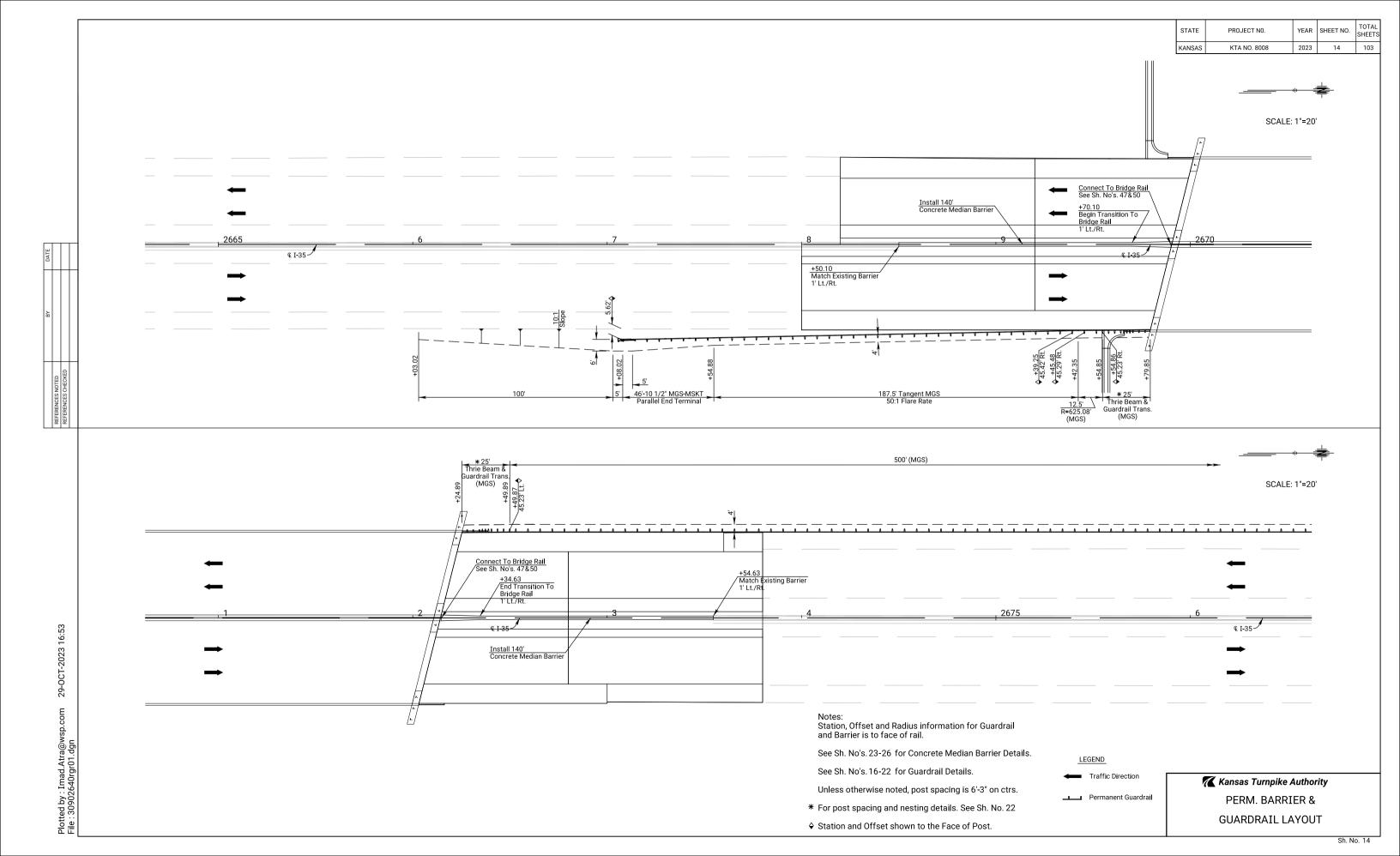
STATE

PROJECT NO.

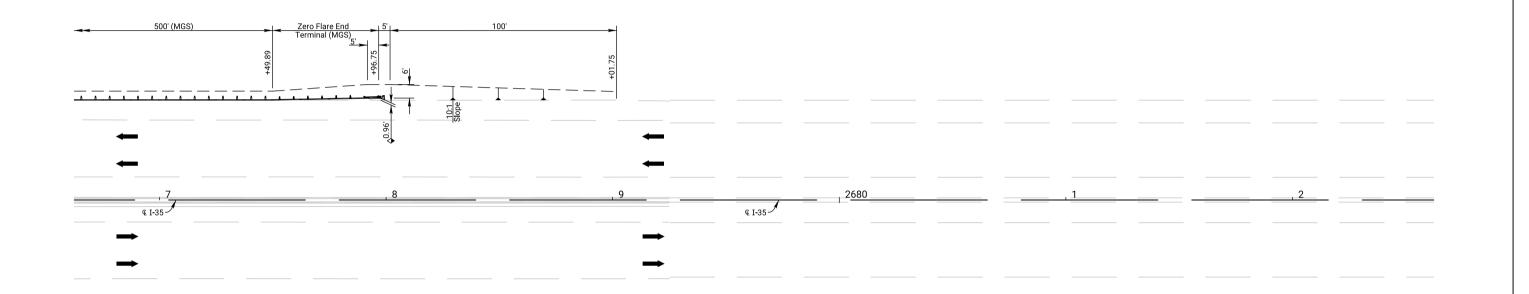
YEAR SHEET NO.







YEAR SHEET NO. TOTAL SHEETS STATE PROJECT NO. KANSAS KTA NO. 8008 2023 SCALE: 1"=20'



Notes: Station, Offset and Radius information for Guardrail and Barrier is to face of rail. See Sh. No's. 23-26 for Concrete Median Barrier Details. See Sh. No's. 16-22 for Guardrail Details.

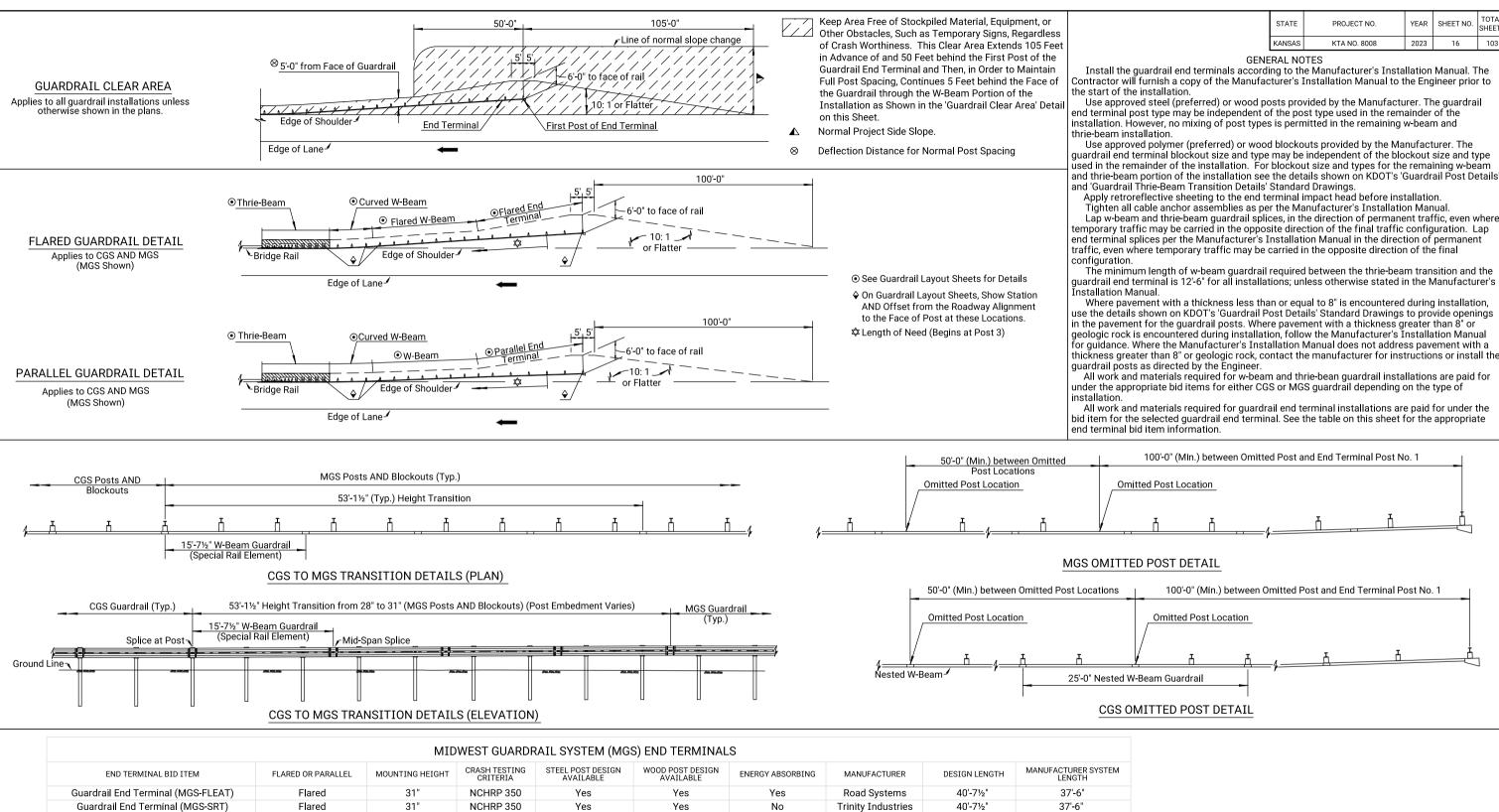
Unless otherwise noted, post spacing is 6'-3" on ctrs.

\* For post spacing and nesting details. See Sh. No. 22

♦ Station and Offset shown to the Face of Post.

Traffic Direction Kansas Turnpike Authority Permanent Guardrail PERM. BARRIER & **GUARDRAIL LAYOUT** 

LEGEND



MIDWEST GUARDRAIL SYSTEM (MGS) END TERMINALS														
END TERMINAL BID ITEM	END TERMINAL BID ITEM FLARED OR PARALLEL MOUNTING HEIGHT CRASH TESTING CRITERIA STEEL POST DESIGN AVAILABLE WOOD POST DESIGN AVAILABLE ENERGY ABSORBING MANUFACTURER SYSTEM LENGTH													
Guardrail End Terminal (MGS-FLEAT)	Flared	31"	NCHRP 350	Yes	Yes	Yes	Road Systems	40'-7½"	37'-6"					
Guardrail End Terminal (MGS-SRT)	Flared	31"	NCHRP 350	Yes	Yes	No	Trinity Industries	40'-71/2"	37'-6"					
Guardrail End Terminal (MGS-MSKT)	Parallel	31"	MASH	Yes	No	Yes	Road Systems	46'-10½"	46'-10½"					
Guardrail End Terminal (MGS-SOFTSTOP)	Parallel	31"	MASH	Yes	No	Yes	Trinity Industries	46'-10½"	50'-91/2"					

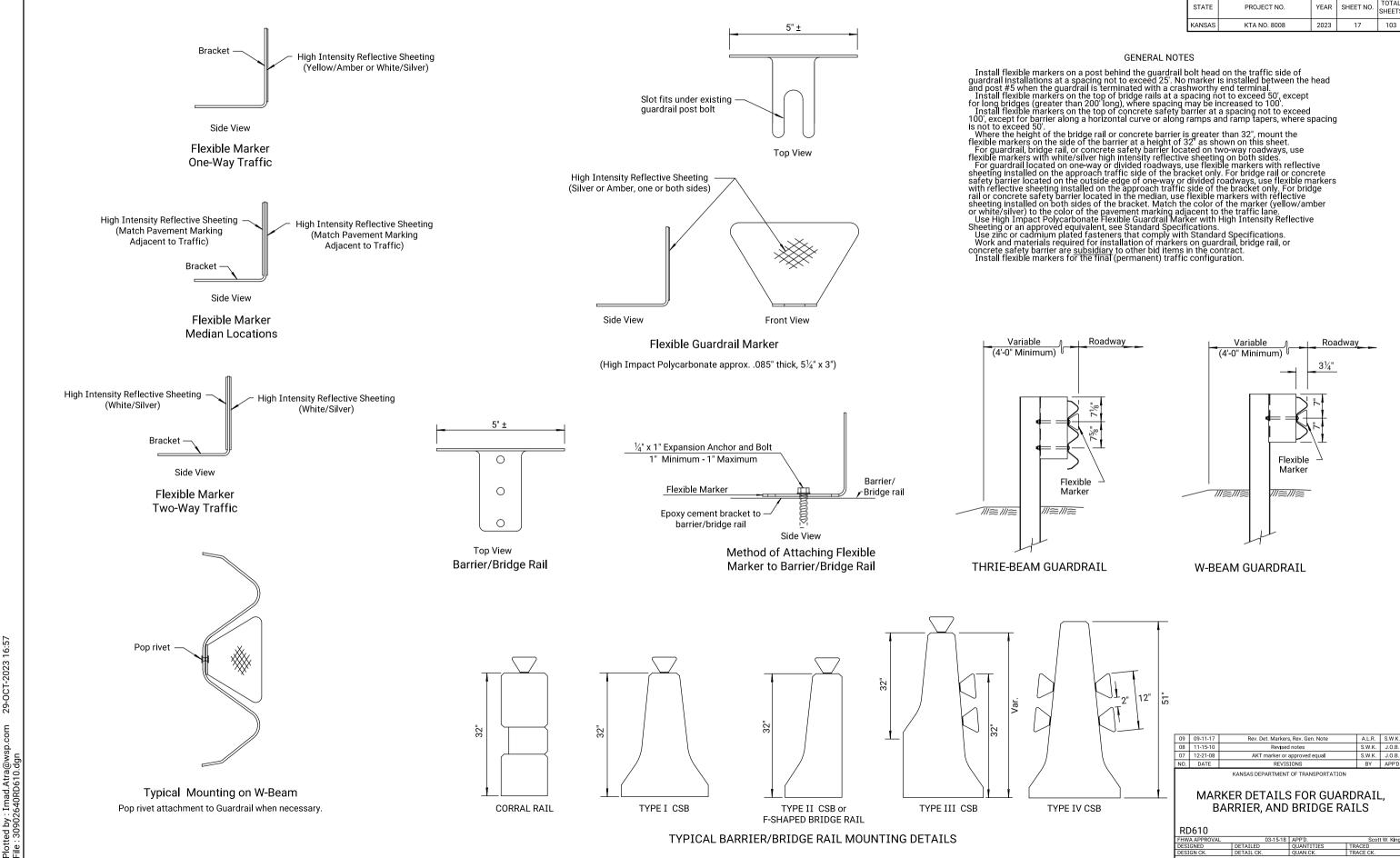
	CONVENTIONAL GUARDRAIL SYSTEM (CGS) END TERMINALS													
END TERMINAL BID ITEM	END TERMINAL BID ITEM FLARED OR PARALLEL MOUNTING HEIGHT CRASH TESTING CRITERIA STEEL POST DESIGN AVAILABLE WOOD POST DESIGN AVAILABLE ENERGY ABSORBING MANUFACTURER DESIGN LENGTH MANUFACTURER SYSTEM LENGTH													
Guardrail End Terminal (FLEAT)	Flared	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	37'-6"	37'-6"					
Guardrail End Terminal (SRT)	Flared	28"	NCHRP 350	Yes	Yes	No	Trinity Industries	37'-6"	37'-6"					
Guardrail End Terminal (SKT)	Parallel	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	50'-0"	50'-0"					

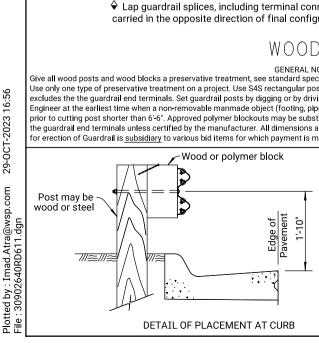
ADD, OMITTED POST AND TRANS, DETAILS 01 06-05-18 INITIAL RELEASE

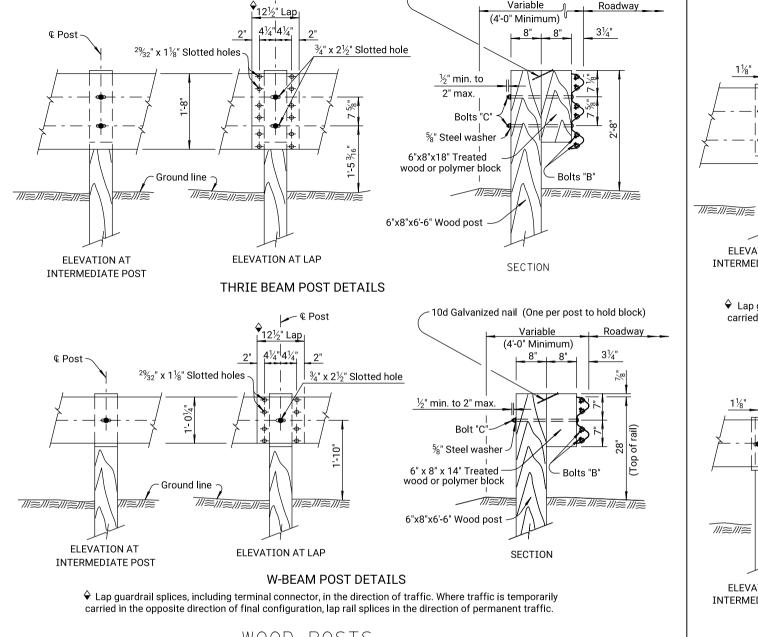
KANSAS DEPARTMENT OF TRANSPORTATION

**GUARDRAIL AUXILIARY DETAILS** 

RD606







₽ Post

### WOOD POSTS

curb.

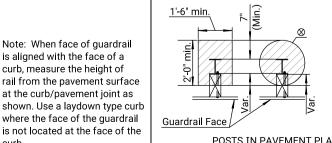
Give all wood posts and wood blocks a preservative treatment, see standard specifications. Thoroughly saturate all cuts, injuries and bolt holes on wood posts and blocks with preservative. Use only one type of preservative treatment on a project. Use S4S rectangular posts and wood blocks, see standard specifications. Use only one post/blockout type within guardrail run, this excludes the the guardrail end terminals. Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations. Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post. Contractor must obtain Engineer approval prior to cutting post shorter than 6-6". Approved polymer blockouts may be substituted for wood blockouts. Only one type of blockout is permitted on each guardrail installation. This excludes the guardrail end terminals unless certified by the manufacturer. All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made. Where guardrail posts are installed in pavement, form openings in the pavement for the guardrail posts.

Note: When face of guardrail

is aligned with the face of a

curb, measure the height of

at the curb/pavement joint as



10d Galvanized nail (One per post to hold block)

Variable

POSTS IN PAVEMENT PLAN (ALTERNATE GEOMETRIES) Applies to All Wood and All Steel Posts (Steel Posts Shown)

Slurry Grout (Low Strength). See **KDOT's Standard Specifications** 

**ELEVATION AT** 

INTERMEDIATE POST

///=///=

⊗ Diameter may vary from 1'-6" (min.) to 2'-0".

Note: Low Strength Grout must have a 28-day compressive strength of 120 psi or less. All work and materials related to posts in pavement are subsidiary to other guardrail bid items. Rectangular geometry shown in Posts in Pavement detail. Circular geometry, as shown on this sheet, may be used at the Contractor's option

Steel post

<sup>2</sup>%<sub>32</sub>"x1%'

Slotted holes

Ground line

<sup>2</sup>%<sub>32</sub>"x1%

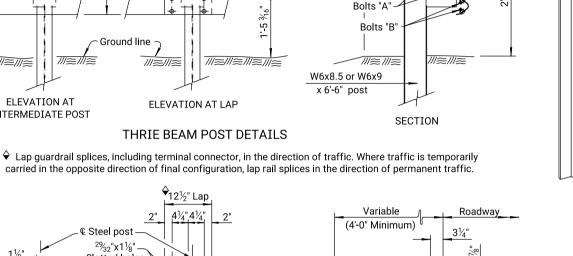
Slotted holes

3/4" x 21/2" Slot

Ground line

**ELEVATION AT** 

INTERMEDIATE POST

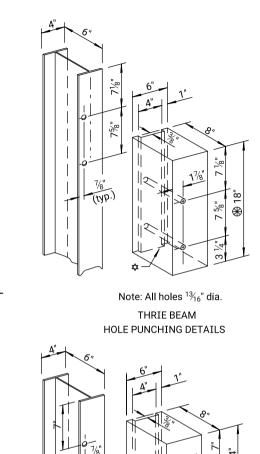


6" x 8" x 18" Treated wood or polymer block

Variable

(4'-0" Minimum)

3/4" x 21/2" Slot



STATE

KANSAS

Roadway

31/4"

PROJECT NO.

KTA NO. 8008

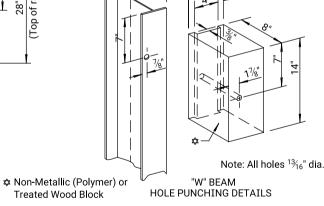
Transition Section Details.

YEAR SHEET NO.

18

2023

See Standard Drawing RD613 for Thrie-Beam



STEEL POSTS

1 1/4"

18"

С

W6x8.5 or W6x9

x 6'-6" post

///≈///≈///≈///

W-BEAM POST DETAILS

**ELEVATION AT LAP** 

6" x 8" x 14" Treated

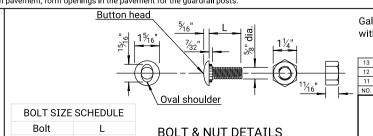
wood or polymer block

Bolts "A"

Bolts "B"

Jse grade of steel for steel posts that meets the requirements of the standard specifications. Hot dip galvanize the posts after fabrication, see standard specifications. Use only one post/blockout type within guardrail run, this excludes the guardrail end terminals. For wood/polymer blockout requriements see standard specifications. Approved polymer blockouts may be substituted for wood blockouts. Only one type of blockout is permitted on each guardrail installation. This excludes the guardrail end terminals. Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations. Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post. Contractor must obtain Engineer approval prior to cutting post shorter than 6-6" except as allowed on Standard Drawing RD617. All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made. Where guardrail posts are installed in pavement, form openings in the pavement for the guardrail posts.

**SECTION** 

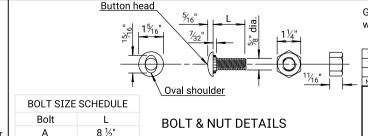


Galvanize all bolts, nuts, and washers in accordance with the KDOT's Standard Specifications.

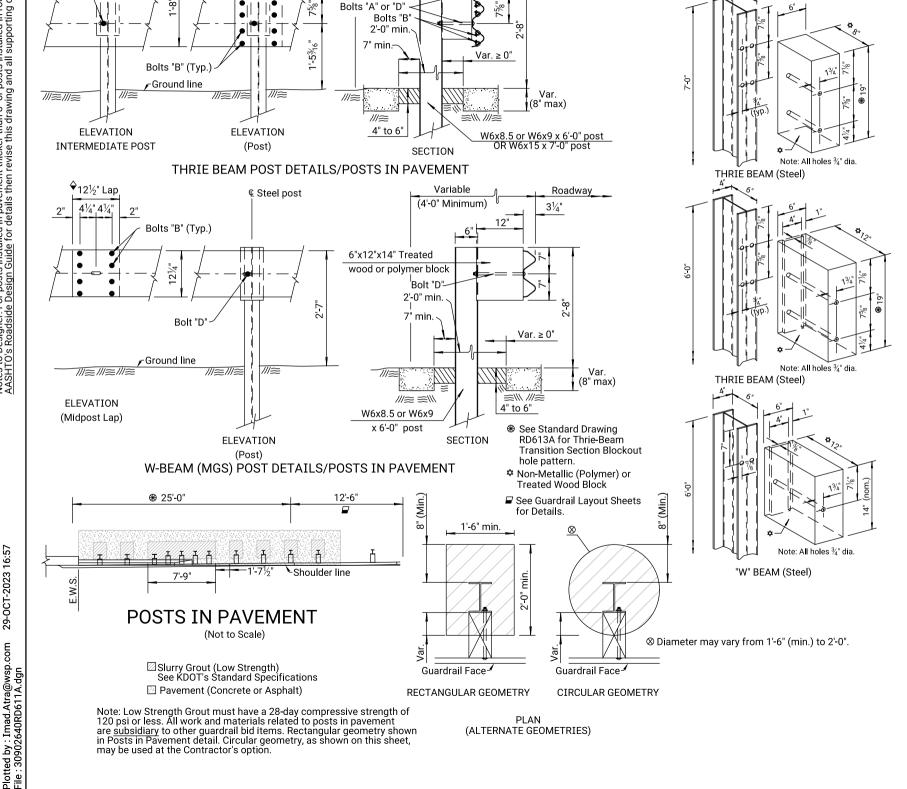
13	09-05-18	Added Det., Post In Pavement	A.L.R.	T.T.R.
12	12-14-10	Revised notes, 28" w-be	S.W.K.	J.O.B.
11	06-30-04	Remove steel blockout and notes	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APP'D
		KANSAS DEPARTMENT OF TRANSPORTATION		

**GUARDRAIL POST DETAILS** 

RD611







△ See RD613A for appropriate blockout size location.

Roadway

Variable

(4'-0" Minimum)

Treated wood

or polymer block

Lap quardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried

in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

♦ 12½" Lap

41/4" 41/4"

€ Steel post

€ Steel post

Bolts "A'

YEAR SHEET NO. STATE PROJECT NO. 103 KANSAS 2023 KTA NO. 8008 19

#### GENERAL NOTES (Steel Posts)

Use grade of steel for steel posts that meet the requirements of the standard specifications.

Hot dip galvanize the posts after fabrication, see standard specifications

Wood blockouts may be used through the 25'-0" thrie-beam section with wood or polymer blockouts used throughout the remainder of the w-beam installation. The blockout size and material used in the guardrail end terminal may be independent from the remainder of the installation. For wood/polymer blockout requirements see standard specifications.

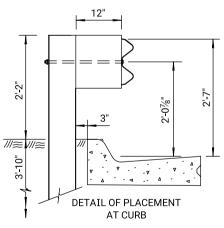
Use S4S rectangular blockouts for Thrie-Beam/W-Beam installation.

Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations.

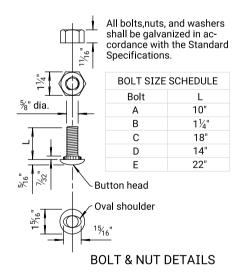
Contractor must notify Engineer at the earliest time when a

non-removable manmade object (footing, pipe, etc.) is encountered that prevents installation of a full length post.

All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is <u>subsidiary</u> to various bid items for which payment is made.



Note: Measure height of rail from the pavement surface at the curb/pavement joint as shown. A special design is needed when quardrail is not located as detailed. A Type II (laydown) curb & gutter is preferred when guardrail is adjacent



05	09-24-15	Separated Steel/Wood Post Details	S.W.K.	S.W.K.
04	11-08-12	Revised Detail, Posts in Pavement	S.W.K.	J.O.B.
03	08-01-12	Revised Note to Designer	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APP'D
		KANSAS DEPARTMENT OF TRANSPORTATION		

**GUARDRAIL POST (STEEL)** (MGS) DETAILS

RD611A

(ALTERNATE GEOMETRIES)

#### GENERAL NOTES (Wood Posts)

Give all wood posts and wood blocks a preservative treatment, see standard specifications. Thoroughly saturate all cuts, injuries and bolt holes on wood posts and blocks with preservative. Use only one type of preservative treatment

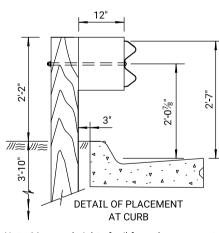
Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations.

Wood blockouts may be used through the 25'-0" thrie-beam section with wood or polymer blockouts used throughout the remainder of the w-beam installation. The blockout size and material used in the guardrail end terminal may be independent from the remainder of the installation. For wood/polymer blockout requirements see standard specifications.

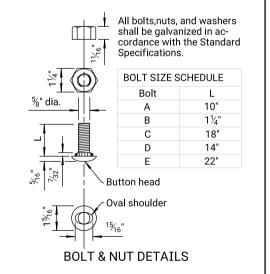
Use S4S rectangular posts/blockouts for Thrie Beam/W-Beam installation. See standard specifications for additional information.

Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post.

All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made.



Note: Measure height of rail from the pavement surface at the curb/pavement joint as shown. A special design is needed when guardrail is not located as detailed. A Type II (laydown) curb & gutter is preferred when guardrail is adjacent to curb



04	09-24-15	Initial Release	T.T.R.	S.W.K.
NO.	DATE	REVISIONS	BY	APP'D
		KANSAS DEPARTMENT OF TRANSPORTATION		

**GUARDRAIL POST (WOOD)** (MGS) DETAILS

RD611B



120 psi or less. All work and materials related to posts in pavement are subsidiary to other guardrail bid items. Rectangular geometry shown

may be used at the Contractor's option.

in Posts in Pavement detail. Circular geometry, as shown on this sheet,

★ Thrie Beam Transition. See Std. Drawing RD613A for details and general note.

4" Asphalt material placed on 4'-0" embankment widening unless flume inlet

† Terminate zero flare rate installations with a parallel guardrail end terminal. Typically parallel end terminals are flared at 50:1 over the length of the end

and slope drain is constructed. See RD611A for "Post in Pavement" details.

□ Radius= 625.08

▲ Normal project side slope. See typical sections.

terminal, but may be flared up to 26:1 or flatter.

♦ See KDOT's 'Guardrail Auxiliary Details' Standard Drawing.

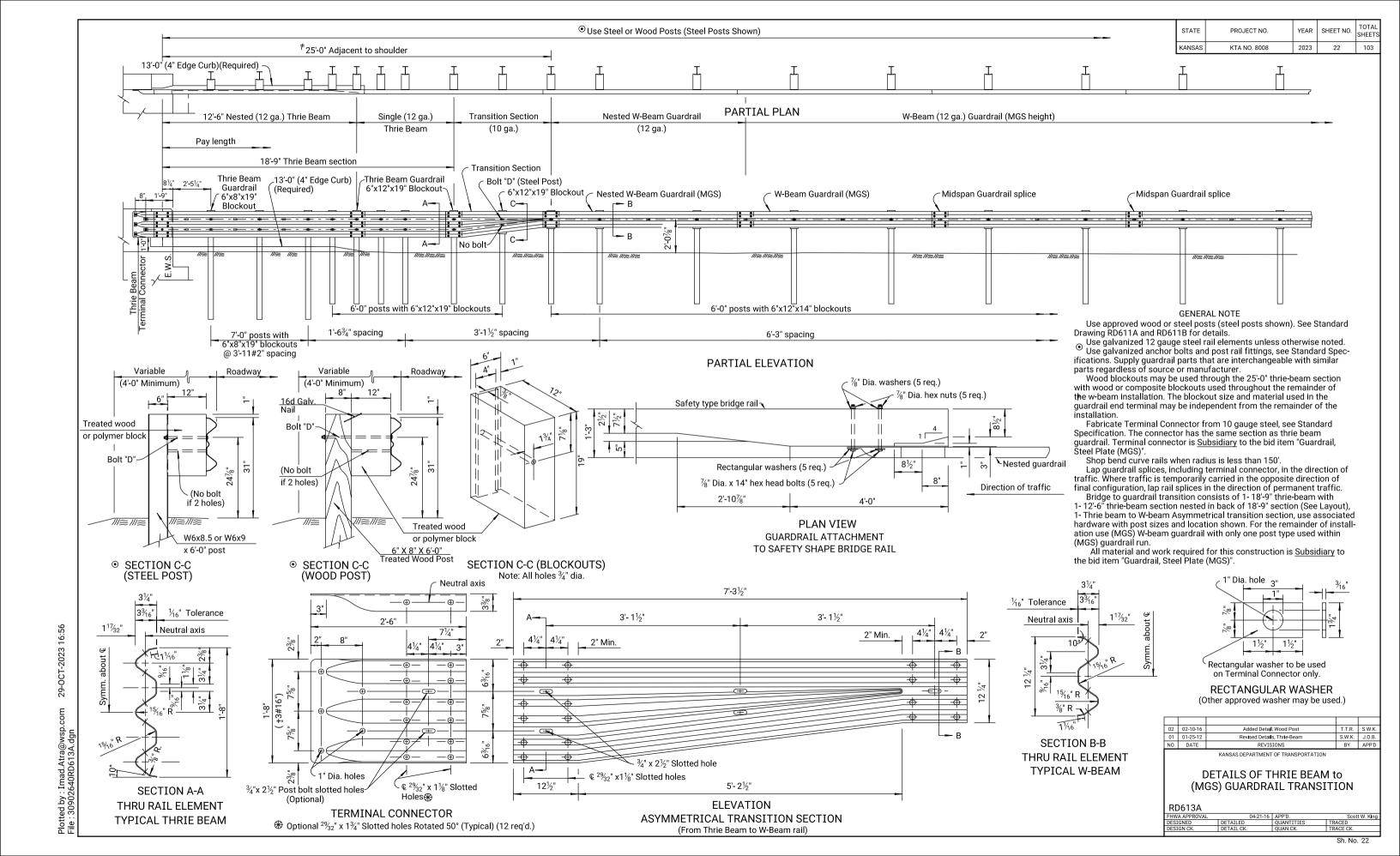
This area to be maintained free of See bridge plans for KANSAS KTA NO. 8008 2023 slope of bridge berm. fixed objects. 4"-0" from face 100'-0" of guardrail 4"-0" from face Line of normal slope change of guardrail 25'-0" <del>X</del> <sub>1</sub>12'-6" 6'-0" to face of rail Shoulder line 4 **#50** 30'-0" Edge of Traveled way **\$**(MGS-MSKT or MGS-SOFTSTOP) End Terminal Bridge ¬ Edge of Traveled way <sup>‡</sup>50 Shoulder line —10: 1 or f<u>lat</u>te<u>r</u> Varies T ♦ End ~ 6'-0" to face of rail 4"-0" from face Line of normal slope change of guardrail 100'-0" This area to be maintained free of fixed objects. THRIE BEAM TRANSITION - TWO LANES 50'-0" This area to be maintained free of fixed objects. See bridge plans for slope of bridge berm. 100'-0" Line of normal slope change-4'-0" from face of 4'-0" from face of guardrail guardrail 25'-0" + 12'-6" Varies - 6'-0" to face of rail R= **=** 10:1 or flatter 30'-0" *Y* Shoulder line 4 <sup>‡</sup>50 Edge of Traveled way-**⇔** (MGS-MSKT or MGS-SOFTSTOP) End Terminal € Entering lanes Edge of Traveled way-Shoulder line-- 10:1 or flatter Varies Slope varies Slope varies Shoulder line Edge of Traveled way Bridge
 ¬ 04 05-15-17 03 08-01-12 tem. X-LITE, Added MSKT and SOFTST S.W.K. J.O.B. Edge of Traveled way-KANSAS DEPARTMENT OF TRANSPORTATION Shoulder line-THRIE BEAM GUARDRAIL (MGS) BRIDGE APPROACH TRANSITION Guardrail on shoulder line as needed.
Use MGS-Terminal Type II TYPICAL ALIGNMENTS (PARALLEL) RD612B THRIE BEAM TRANSITION - FOUR LANES (DIVIDED)

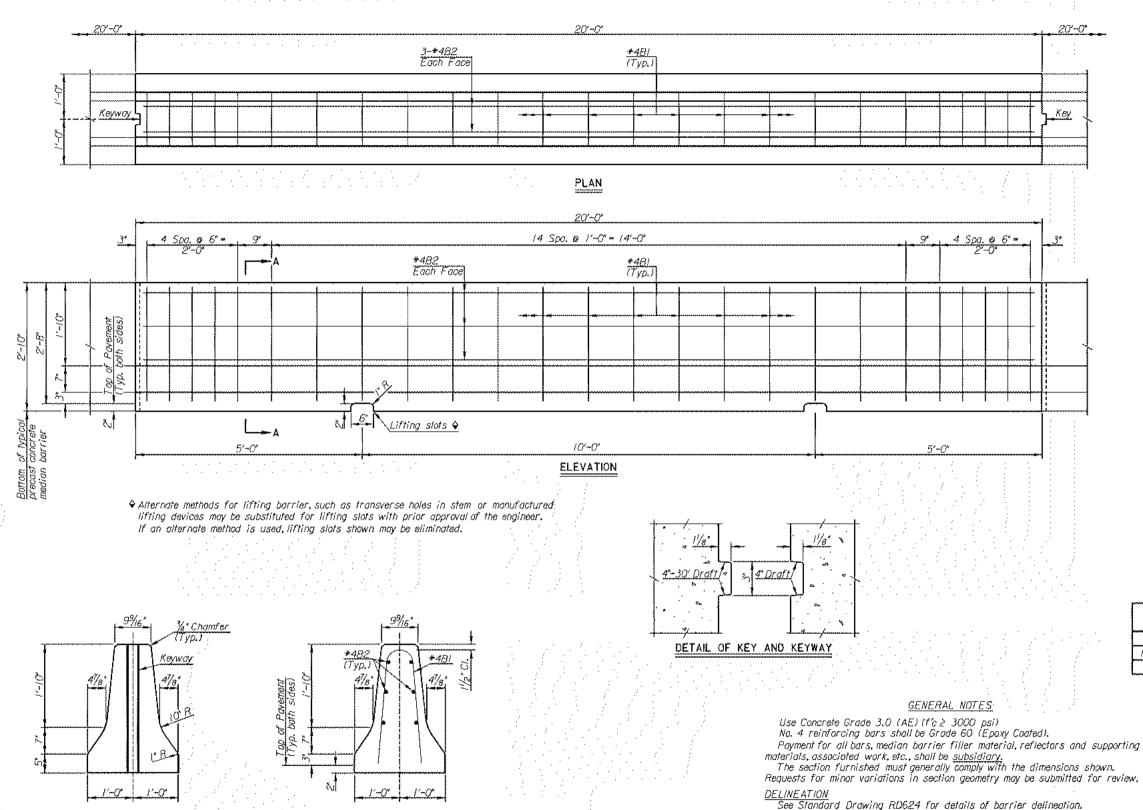
50'-0"

YEAR SHEET NO. TOTAL SHEETS

STATE

PROJECT NO.



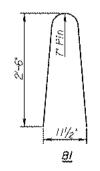


SECTION A-A

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END VIEW

STATE PROJECT NO. YEAR SHEET NO. KANSAS KTA NO. 8008 2023 23



#### BENDING DIAGRAM

(All dimensions are out to out of bars)

BILL OF REINFORCING STEEL Grade 60 (Epoxy Coated)													
Straight Bars Bent Bars													
Mark	Size	Number	Length	Mark	Size	Number	Length						
B2	4	6	19'-8"	BI	4	25	5′~6°						

Note: Double form for the concrete median barrier is available and will need to be reconditioned by the contractor prior to use. The forms are located at KTA Plaza 45 (K-15), contact Mark Roberts (316) 644-8143 to gain entry to the storage yard for viewing or pickup. The forms are to be returned to Plaza 45 in good condition after the barrier for the project has been produced. Transportation of forms and work necessary to recondition forms shall be subsidiary to bid item "Permanent Precast Concrete Median Barrier (Type IF)". Payment for all work and materials to construct and install barrier will be bid as "Permanent Precast Concrete Safety Barrier (Type IF)".

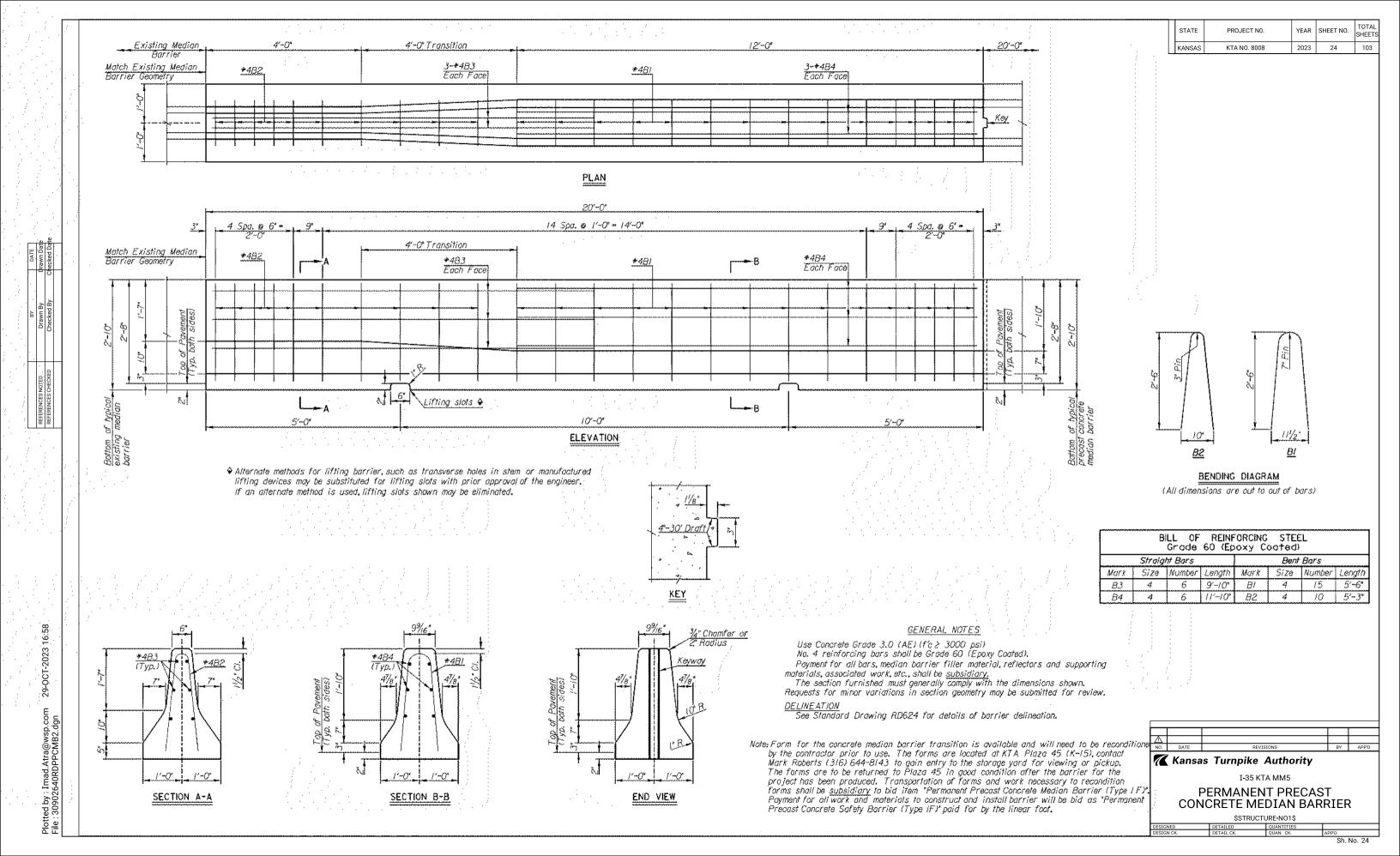


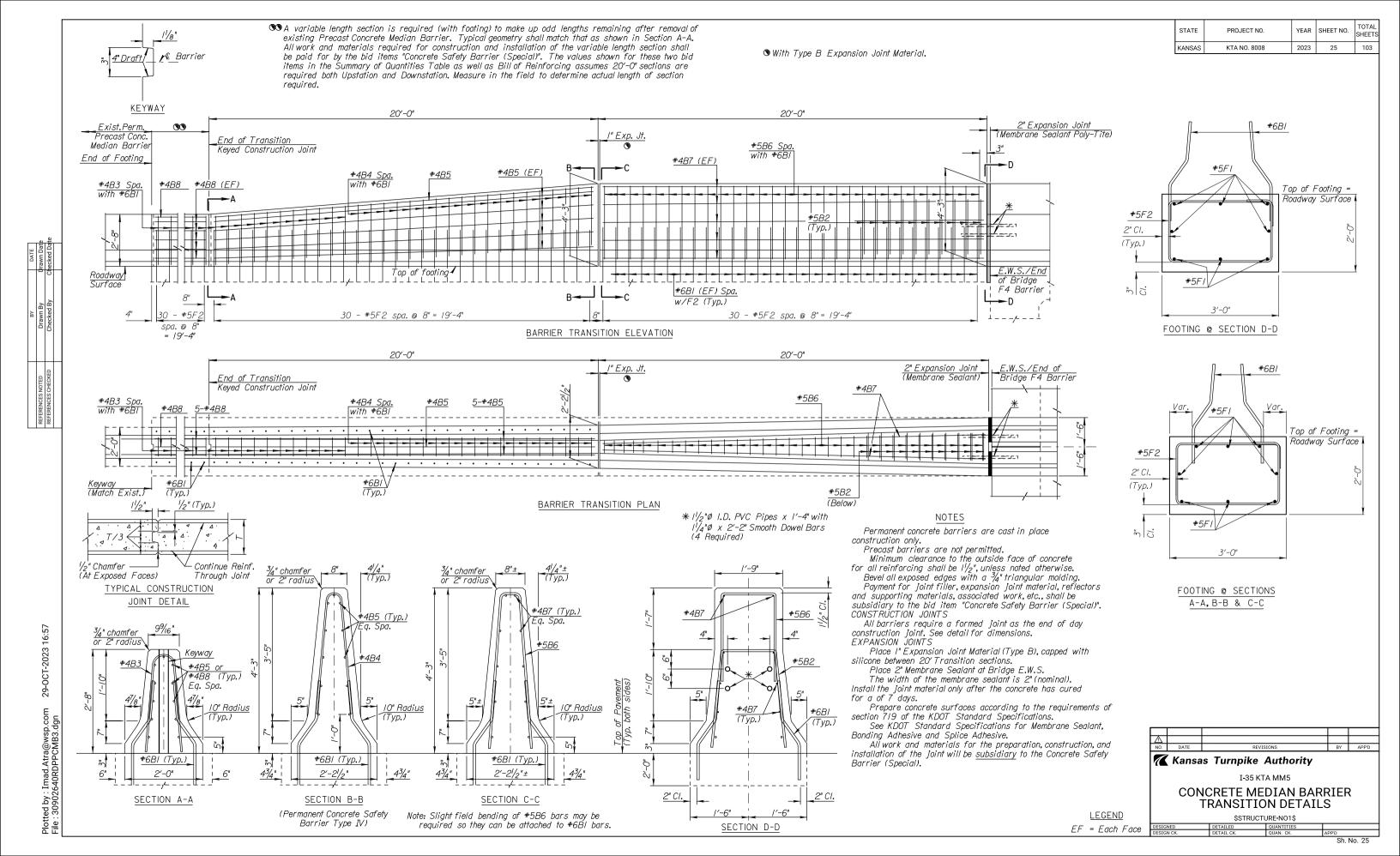
Kansas Turnpike Authority

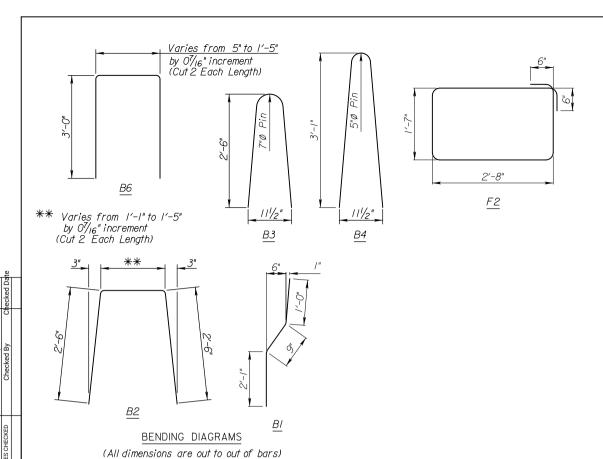
I-35 KTA MM5

PERMANENT PRECAST **CONCRETE BARRIER** 

\$STRUCTURE-NO1\$







	BILL OF REINFORCING STEEL Epoxy Coated - Grade 60														
		Straig	ht Bars			<b>₩</b> Bent	Bars								
	Mark	Size	Number	Length	Mark	Size	Number	Length							
	FΙ	#5	16	59′-8″	BI	#6	360	3'-10"							
					B2	#5	20	<i>†</i>							
	<i>B</i> 5	#4	22	19'-8"	B6 #5		60	<i>†</i>							
~	<i>B</i> 7	#4	20	19′-8"	F2	#5	180	9′-6"							
Barrier	B8	#4	22	19'-8"											
P					ВЗ	#4	82	5′-4"							
B					B4	#4	38	6′-5"							

\* See Bending Diagram † Varies Bar - See Bending Diagram

Note: All work and materials required for construction and installation of the Concrete Median Barrier Transition will be paid for by lin. ft. of Concrete Safety Barrier (Special).

STATE

KANSAS

PROJECT NO.

KTA NO. 8008

YEAR SHEET NO.

2023 26

Kansas Turnpike Authority

I-35 KTA MM5

BILL OF REINFORCING

CONCRETE MEDIAN

BARRIER TRANSITION

\$STRUCTURE-NO1\$

DESIGNED DETAILED TOUNTITIES

Plotted by : Imad.Atra@wsp.com 29-0CT-2023 16:58 File : 30902640RDPPCMB4.dgn

#### Revised Plans

#### SUMMARY OF QUANTITIES (NORTHBOUND I-35 BRIDGE OVER E 150th St S)

							00	AVIIVIA CT	QUANTITIE	3 (1401/11110014	DI OO DINIDOL	OVERCETOOL	(1.0)					<u>/1\</u>	<u> </u>	<u> </u>
Item	Excavation Class III	Concrete (Gr. 4.0) (AE)	Concrete (Gr. 4.0) (AE)(SA)	Reinforcing Steel (Gr. 60)	Reinforcing Steel (Gr. 60) (Epoxy Coated)	Drilling and Grouting (Repair)	Piles (Steel) (HP12X53)	Abutment Strip Drain	Bridge Backwall Protection System	Structural Steel (Erection)	Falsework Inspection	Temporary Shoring	Bridge Painting (Organic Zinc with Acrylic System)	Welded Stud Shear Connectors	Bridge Deck Grooving	Slope Protection (12" Aggregate)	Drilled Shaft (60") (Cased)	Sonic Test (Drilled Shaft) (Set Price)	Core Hole (Investigative) (Lin. Ft.)	Flowable Fill (High Strength)
Location	(Cu. Yds.)	(Cu. Yds.)	(Cu. Yds.)	(Lbs.)	(Lbs.)	(Set Price)	(Lin. Ft.)	(Sq. Yds.)		(Lump Sum)	(Lump Sum)	(Lump Sum)	(Lump Sum)	(Each)	(Sq. Yds.)	(Cu. Yds.)	(Lin. Ft.)	(Each)	(=1,	(Cu. Yds.)
Abutment 1	121	13	-	1140	-	-	204	-	-	-	-	-	-	-	-	535 ⚠	-	-	-	91.6
Pier 1	* 19	* 26.1	-	* 8457	-	=	-	-	-	-	-	-	-	-	=	-	* 46.33	-	33.0	-
Pier 2	<b>*</b> 19	* 26.1	-	* 8457	-	-	-	-	-	-	-	=	-	-	=	-	* 51.00	-	33.0	-
Pier 3	* 19	* 22.1	-	* 7530	-	-	-	-	-	-	-	-	-	-	=	-	* 59.50	=	37.0	-
Abutment 2	121	13	-	1140	-	-	228	-	-	-	-	-	-	-	-	205 🛕	-	-	-	91.7
Substr. Total	-	100.3	-	-	97110	-	432	82.9	94.7	-	-	-	-	-	-	740 🗥	156.83	1	103.0	-
Superstr. Total	ı	-	374.8 🛕	26722	-	1.0	-	-	-	1.0	1.0	1.0	1.0	2544	957.5	-	=	-	-	183.3
Total	299	100.3	374.8 🛕	26722	97110	1.0	<b>†</b> 432	82.9	94.7	1.0	1.0	1.0	1.0	2544	957.5	740 🗥	157	1	103	183

#### SUMMARY OF QUANTITIES (SOUTHBOUND I-35 BRIDGE OVER E 150th St S)

							00	WIIWIA (C)	QOANTITIE	0 (00011100011	D I OO DINIDOL	OVER E 1000	11 01 0)					<u>/1\</u>	<u> </u>	<u> </u>
Item	Excavation Class III	Concrete (Gr. 4.0) (AE)	Concrete (Gr. 4.0) (AE)(SA)	Reinforcing Steel (Gr. 60)	Reinforcing Steel (Gr. 60) (Epoxy Coated)	Drilling and Grouting (Repair)	Piles (Steel) (HP12X53)	Abutment Strip Drain	Bridge Backwall Protection System	Structural Steel (Erection)	Falsework Inspection	Temporary Shoring	Bridge Painting (Organic Zinc with Acrylic System)	Welded Stud Shear Connectors	Bridge Deck Grooving	Slope Protection (12" Aggregate)	Drilled Shaft (60") (Cased)	Sonic Test (Drilled Shaft) (Set Price)	Core Hole (Investigative) (Lin. Ft.)	Flowable Fill (High Strength)
Location	(Cu. Yds.)	(Cu. Yds.)	(Cu. Yds.)	(Lbs.)	(Lbs.)	(Set Price)	(Lin. Ft.)	(Sq. Yds.)		(Lump Sum)	(Lump Sum)	(Lump Sum)	(Lump Sum)	(Each)	(Sq. Yds.)	(Cu. Yds.)	(Lin. Ft.)	(Each)	(2	(Cu. Yds.)
Abutment 1	121	13	-	1140	-	-	204	-	-	-	-	-	-	-	-	535 ⚠	-	-	-	91.6
Pier 1	9	10.8	-	3791	-	-	-	-	-	-	-	-	-	-	-	-	23.17	-	33.0	-
Pier 2	9	10.8	-	3791	1	-	-	-	-	-	-	-	-	-		-	25.50	-	33.0	-
Pier 3	9	10.8	-	3328	-	-	-	-	-	-	-	-	-	-	-	-	29.75	-	37.0	-
Abutment 2	121	13	-	1140	-	-	228	-	-	-	-	-	-	-	-	205 🗥	-	-	-	91.7
Substr. Total	-	58.4	-	-	97110	=	432	82.9	94.7	-	-	-	-	-	=	740 🛕	78.42	1	103.0	-
Superstr. Total	-	-	374.8 🛕	13189	-	1.0	-	-	-	1.0	1.0	1.0	1.0	2544	957.5	-	-	-	-	183.3
Total	269	58.4	374.8 🗥	13189	97110	1.0	†† 432	82.9	94.7	1.0	1.0	1.0	1.0	2544	957.5	740 🛕	79	1	103	183

#### **GENERAL NOTES**

EXISTING STRUCTURE: Plans of the existing structure are on file and available for inspection by qualified bidders at the Kansas Turnpike Authority, 9401 E. Kellogg, Wichita, KS 67207

REMOVAL OF EXISTING STRUCTURE: Remove the existing structures to the limits shown on the plans. Removal of existing structures is included in the bid item, "Removal of Existing Structures", Lump Sum. All materials removed from the existing structures shall become the property of the Contractor. Remove this material from the site. Clearly mark the location of the bridge that is to remain during construction. Mark the entire length of the bridge before sawing or removing any concrete. Concrete sawing shall be limited to as shown on the plans for the deck. Saw cut to neat lines all exposed surfaces of the concrete. Limit depth of cut to 1". Do not cut through reinforcement. Do not use drop-type pavement breakers on the structure that will remain. Do not use a hoe ram within 2' of a construction joint. Use a jackhammer no heavier than 15 lbs. to remove concrete within 2' of a construction joint and no heavier than 50 lbs, beyond 2' of the construction joint in a manner not to damage the deck that will remain during construction. Damage to the existing structural steel caused by procedures not conforming to the above recommendations shall be repaired as directed by the Engineer at the Contractor's expense (no cost to the KTA). Any costs incurred for testing or Engineering evaluations will be included in the Contractor's expense for repair.

DECK PROTECTIVE SYSTEM: Epoxy coat all reinforcing steel in the deck slab, rails and abutment.

ABUTMENT STRIP DRAIN: See the General Notes on the "Abutment
Strip Drain" sheet. Place the abutment strip drain, including
associated drainage pipes, behind both the newly constructed
abutment diaphragm & beam seat and the existing abutment
beam seat

BRIDGE BACKWALL PROTECTION SYSTEM: See the General Notes on the "Abutment Aggregate Drain" sheet. Place the backwall protection system onto both the newly constructed abutment diaphragm & beam seat and the existing abutment beam seat.

REINFORCING STEEL: All reinforcing steel dimensions are to the centerline of bars unless otherwise noted. All reinforcing steel shall conform to the requirements of ASTM A615, Grade 60.

Where non-coated bars come in contact with epoxy coated bars, they need not be coated.

LFD RATING FACTORS

Inventory

2.36

Operating

3.94

1.76

Leve

2002 LFD Rating, 17th Edition AASHTO

DIMENSIONS: All dimensions shown on the design plans are horizontal dimensions unless otherwise noted. Make necessary allowances for roadway grade and cross slope.

TEMPERATURE: The design temperature for all dimensions is 60° F.

BACKFILL COMPACTION: Compact backfill at the abutments.

HET (110T)

Truck HS-20 (36T)

#### **DESIGN DATA**

DESIGN SPECIFICATIONS: AASHTO Specifications, 9th Edition. Load and Resistance Factor Design.

DESIGN LOADING:

HI -93 Design Dead Load includes an allowance of 15 psf for a future wearing surface.

UNIT STRESSES:

Concrete (Grade 4.0) f'c = 4.0 ksiConcrete (Grade 4.0)(AE) f'c = 4.0 ksiReinforcing Steel (Grade 60) Fy = 60 ksiStructural Steel for H-Pile Fy = 50 ksi (A572, Grade 50) Structural Steel (Grade 50) Fy = 50 ksi

ORIGINAL DESIGN SPECIFICATIONS: AASHTO Standard Specifications, 6th Edition, 1953 Edition, Load Factor Design.

ORIGINAL DESIGN LOADING: H20-S16

NOTE: Use only steel HP12x53 piles in the new abutments. The lengths shown are for bidding purposes only. Actual lengths shall be determined by the Engineer based on Pile Driving results.

NOTE: See Sh. # 29 for quantities not listed. \* Quantities of the interior pier are included in this Pier quantity.

50

51

Bar List

52 Bending Diagrams

29	Construction Layout
30	Engeneering Geology
31	Substructure Demolition
32	Superstructure Demolition
33	Abutment Details I
34	Abutment Details II
35	Abutment Strip Drain
36	Pier Details I
37	Pier Details II
38	Framing Plan
39	Girder Elevation I
40	Girder Elevation II
41	Girder Elevation III
42	Diaphragm Details
43	Field Splice Details
44	Bearing Details
45	Deck Details I
46	Deck Details II

INDEX TO BRIDGE DRAWINGS

Summary of Quantities

General Notes

Drawing

Sheet No.

27

28

Δ	12/08/23	Add Flow Fill & D.S. qtys, revise Slope Prot & Conc qtys	SB	AH
NO.	DATE	REVISIONS	BY	APP'D

Kansas Turnpike Authority

Br. No. 5.133 S Br. No. 5.133 N

SUMMARY OF QUANTITES

Proj. KTA NO. 8008 Sumner Co.

† SUMMARY OF PILING			
S. Abutment	4 @ 57 Ft. (Vert.)		
N. Abutment	4 @ 51 Ft. (Vert.)		
†† SUMMARY OF PILING			
S. Abutment	4 @ 57 Ft. (Vert.)		
N. Abutment	4 @ 51 Ft. (Vert.)		

LRFR RATING FACTORS			
Rating Design Level Load	Inventory	Operating	
HL-93 Loading	2.10	2.73	
2018 Manual for Bridge	e Evaluation, 2	019 Interim	

Barrier Geometric Details

51" F-Shape Barrier Details II

32" Kansas Corral Barrier Details I 32" Kansas Corral Barrier Details II

**GENERAL NOTES** 

PILING: Drive all piling to penetrate or bear upon the Wellington Shale Formation. Driving shall stop when in the opinion of the Engineer additional driving may damage the piling. Drive all piling to the Pile Driving Formula Load of:

Abutments No. 1 & 2 60 Tons No. 1 & 2 & 3 220 Tons

As a minimum drive each pile to the load and penetration, but in no case shall the pile be driven to more than 110% of Pile Driving Formula Driving Load. At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Engineer may request that the Pile Driving Analyzer (PDA) equipment be used.

- PILING SPLICE LOCATION: Integral pile splice locations and weld testing criteria for Abutments will follow the "Standard Pile Details" Sheet (BR110).
- BRIDGE EXCAVATION: All excavation shall be Class III. See the "Bridge Excavation" sheet for the limits of pay excavation. Excavation quantity shown includes material behind existing abutment structure (for the construction of the abutment drainage system), including all excavation required for the trenching of the drainage pipes as shown on the "Abutment Aggregate Drain" sheet. Excavation quantity excludes the front of the abutment as well as the concrete limits of the abutment (quantity includes the work behind the existing concrete). The Excavation bid item includes backfilling and compaction after completion of other work. See KDOT Specifications.
- DEMOLITION PLANS: This is a Category B Demolition. Submit detailed Demolition Plans to the Field Engineer at least 2 weeks before the demolition meeting. Identify, on the plans, the Demolition Supervisor meeting the requirements of the KDOT Specifications. No Demolition work will begin without approved Demolition Plans. A Licensed Professional Engineer is not required.
- EXISTING DIMENSION VERIFICATION: Dimensions of the existing structure are based on old plans. Verify, by field measurement, the as-built dimensions of the existing structure and submit such verification in writing to the Engineer. The verification will include sketches, drawings, photographs and descriptions as needed to clearly define the as-built dimensions that will be incorporated in the new construction.
- DRILLING AND GROUTING (REPAIR): This item shall consist of grouting reinforcing steel, anchor bolts, tie bars, or dowel bars into the existing concrete, where required by the Engineer, with an epoxy grout. Follow KDOT Specifications 842 and any associated Special Provisions, Follow the manufacturer's directions for mixing, application and curing. Embedment length to be determined by Engineer & Manufacturer specifications, depending on location. The tools, materials, labor and incidentals necessary to complete the work shall be paid for per each by the bid item "Drilling and Grouting (Repair) (Set Price)".
- CONCRETE PLACING SEQUENCE: The sequence of placing concrete in the slab and curbs shall be as shown, or the review. Submit the alternate placing sequence to the Contractor may submit an alternate placing sequence for Engineer at the Preconstruction Conference. Include the proposed rate of concrete placement in C.Y./h, the plant capacity, placement direction, construction joint location, a description of the equipment used in placing the concrete, proposed admixtures, and the quantity of concrete in each placing segment. Any additional cost for the Contractor's alternate plan of placing concrete, including admixtures, shall be at the Contractor's expense and shall be considered subsidiary to the bid item, "Concrete (Grade 4.0)(AE)". Approval of the Contractor's alternate sequence is required prior to placement of concrete in the deck.
- CONTRACTOR CONSTRUCTION STAKING: All survey work to ensure the abutment diaphragms and approach pavement return to their original elevations and grades is included in the "Contractor Construction Staking" bid item.
- SONIC TESTING: Equip all drilled shafts with piping to allow sonic testing to be done. Install pipes at locations shown on the plans. All wet pours will be tested. Also, the Engineer has the option to require sonic, non-destructive, integrity testing at any location of concern. Sonic testing shall be paid for at the unit price set for "Sonic Test" (Drilled Shaft) (Set Price). If the sonic testing indicates defective concrete in the shaft, the Engineer will measure the first sonic test for payment, and the Contractor is responsible for subsequent sonic testing of that shaft.
- STURCTURAL STEEL: All W36x150 rolled girders, abutment, Splice plates & bent plate diaphragms, stiffeners shall meet AASHTO M270 (Grade 50W T3) requirements except as noted. All other structural steel shall meet ASTM A709 (Grade 50W), unless noted otherwise. Field Splices shall be made only where shown in these Contract Plans as a "splice". Elimination of any "splice" may be requested.

- PAINTING: The shop coats applied to Structural Steel shall conform to an inorganic zinc primer with a waterborne acrylic finish coat. The finish coat will be Kansas (Color), this color will match Federal Standard # (24097).
- Blast clean to meet SSPC-SP10 Specifications and prime coat the embedded portion of the girders, including the abutment diaphragms; the top flanges, including the shear studs in accordance with KDOT Specifications.
- Touch Up: Prepare and paint all bolts, nuts, studs, and other small areas of damaged paint (1 yd2 or less), requiring touch up, with an approved organic zinc primer.
- All labor and material for painting of the structural steel and concrete surface will be subsidiary to other bid items.
- TEMPORARY CONSTRUCTION LOADS: The Contractor will not stock pile construction materials, debris/rubble or place equipment weighing more than 20 tons or greater than bridge posted load limits on the bridge without prior written approval by the KTA Engineer. For bridges with highway traffic on or under the bridge the Contractor will provide plans showing the location, quantity and weight of the proposed materials, debris or equipment weighing more than 20 tons or greater than bridge posted load limit. These plans will bear the Seal of the Contractor's Engineer before approval is granted. The Contractor's Engineer will use AASHTO Specifications for limitations on structural capacities, as the structure is found in the field.
- BEARING (Bearing Type Steel Reinforced Elastomeric) (Method A): Bearing devices at piers 1, 2 & 3 shall be fabricated with an elastomer satisfying:
  - Shore A Durometer Hardness of 60
  - Low Temperature Grade 3 Requirements
  - Type A certification for elastomeric bearing device acceptance is required
  - Include design method and all material properties on shop details
- BOLTED CONNECTION: Girder Connections: Use \(^{8}\) diameter heavy hex structural bolts for the main member connections. Use <sup>15</sup>/<sub>16</sub>" diameter bolt holes. Do not ream during field erection. Accurately align all connections by driving \%" diameter drift pins in all corners and in  $\frac{1}{4}$  of the remaining holes in each plate. See KDOT Specifications.

Secondary Member Connections: Use  $\frac{7}{8}$ " diameter heavy hex structural bolts for the secondary member connections. Use <sup>15</sup>/<sub>16</sub>" diameter bolt holes. Oversized and/or slotted holes, as specified in the KDOT Specifications, may be used in only one of the two members connected and must be shown in the approved shop drawings. Oversized and/or slotted holes may require additional standard hardened washers or plate washers. Report to the Engineer prior to any required field reaming that will remove more than  $\frac{1}{4}$ " of material from one ply of the connected parts.

Use Direct Tension Indicators (DTIs) on all high strength bolts. Place the DTI under the bolt head and turn the nut to tighten. This method is preferred whenever possible. Face the protrusions on the DTI to the underside of the bolt head. Place a hardened flat washer under the nut. See KDOT Specifications.

DRILLED SHAFTS: Construct the drilled shafts using the cased method. A permanent casing is required. All excavation, concrete, reinforcing steel, pipes for Sonic Testing. casings, labor, and incidentals necessary to complete the shaft as shown on the details and as directed by KDOT Specifications shall be included in the bid items "Drilled Shaft (5 ft) (Cased)". In no case shall the bottom of the drilled shaft be placed higher than the elevation shown.

Drill an Investigative Core Hole at the location(s) shown on the plans. See KDOT Specifications

Backfill the annular space between the temporary casing and the permanent casing, as defined in the KDOT Specifications.

If the location of the top of the shaft is such that the casing cannot be overtopped to remove concrete impurities, provide extra casing length to over-pour the concrete in the shaft and chip back to the plan elevation of the top of the shaft.

- $\stackrel{ riangle}{ riangle}$  INVESTIGATIVE CORE HOLES: Contact KTA Personnel when the schedule for the investigative core holes have been established so that a member of the staff may be on site when the work is being performed.
- ERECTION ELEVATION CHECKS: Following removal of the deck concrete and proposed girder erection, the Contractor shall take a profile of each girder line at  $\frac{1}{10}$ th points. Adjust the depth of the concrete fillets over the girders so that the finished slab is constructed to match the top of existing pavement elevation. The Contractor shall send calculated fillet depths to the Engineer for approval.

- CONSTRUCTION LOADS: Limited traffic is permitted on the new sub-deck, one-course deck or any concrete overlay during the curing period, keep any exposed deck wet during the curing period. See KDOT Specifications Section 710 Tables 710-1 & 710-2 for additional information.
- WELDED STUD SHEAR CONNECTORS: Weld Shear Stud Connectors with automatically timed stud welding equipment connected to a suitable power source. All stud welding shall conform to KDOT Specifications. Blast clean the tops of the top flanges before the studs are applied.

Revised Plans

STATE

KANSAS

PROJECT NO.

KTA NO. 8008

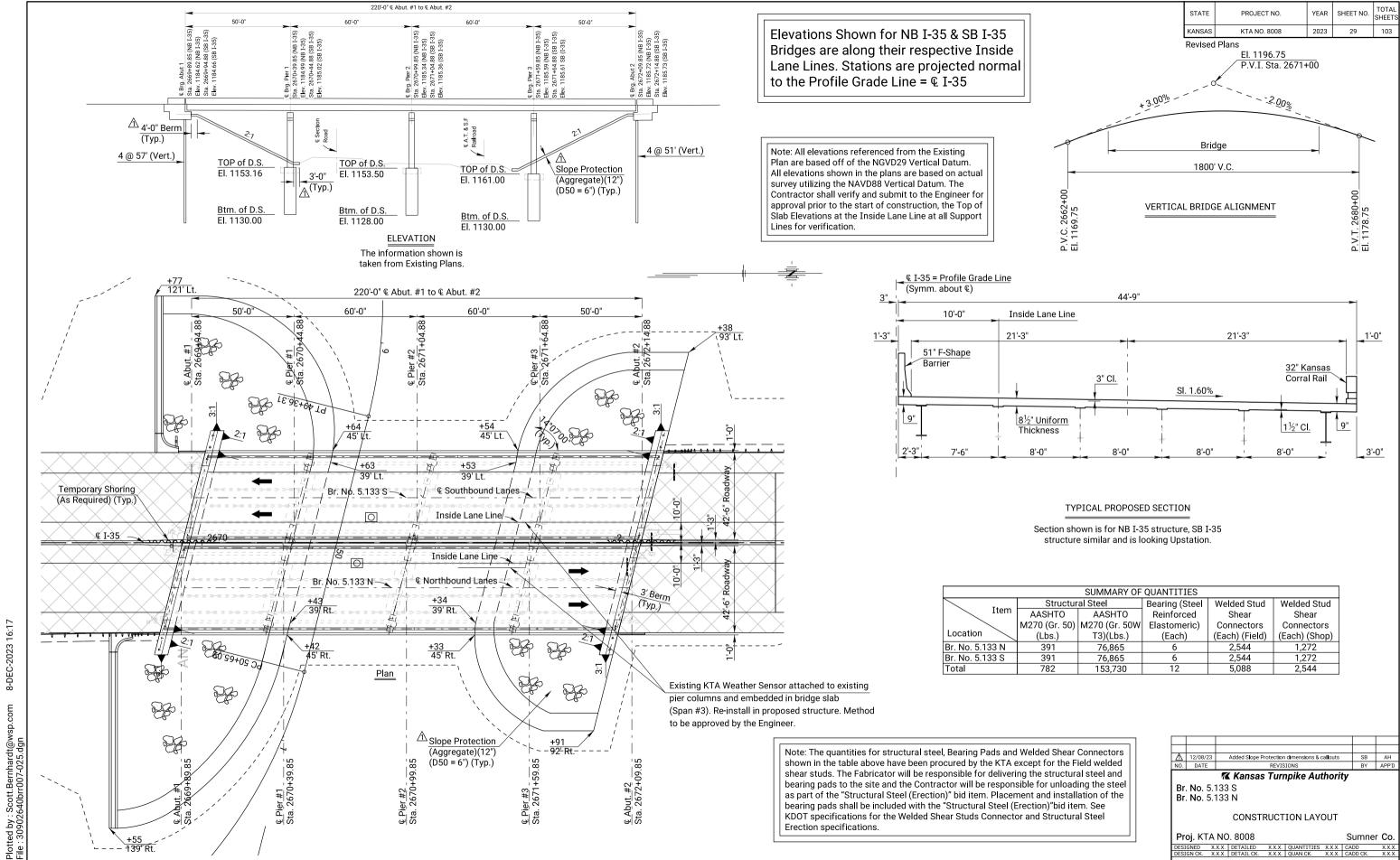
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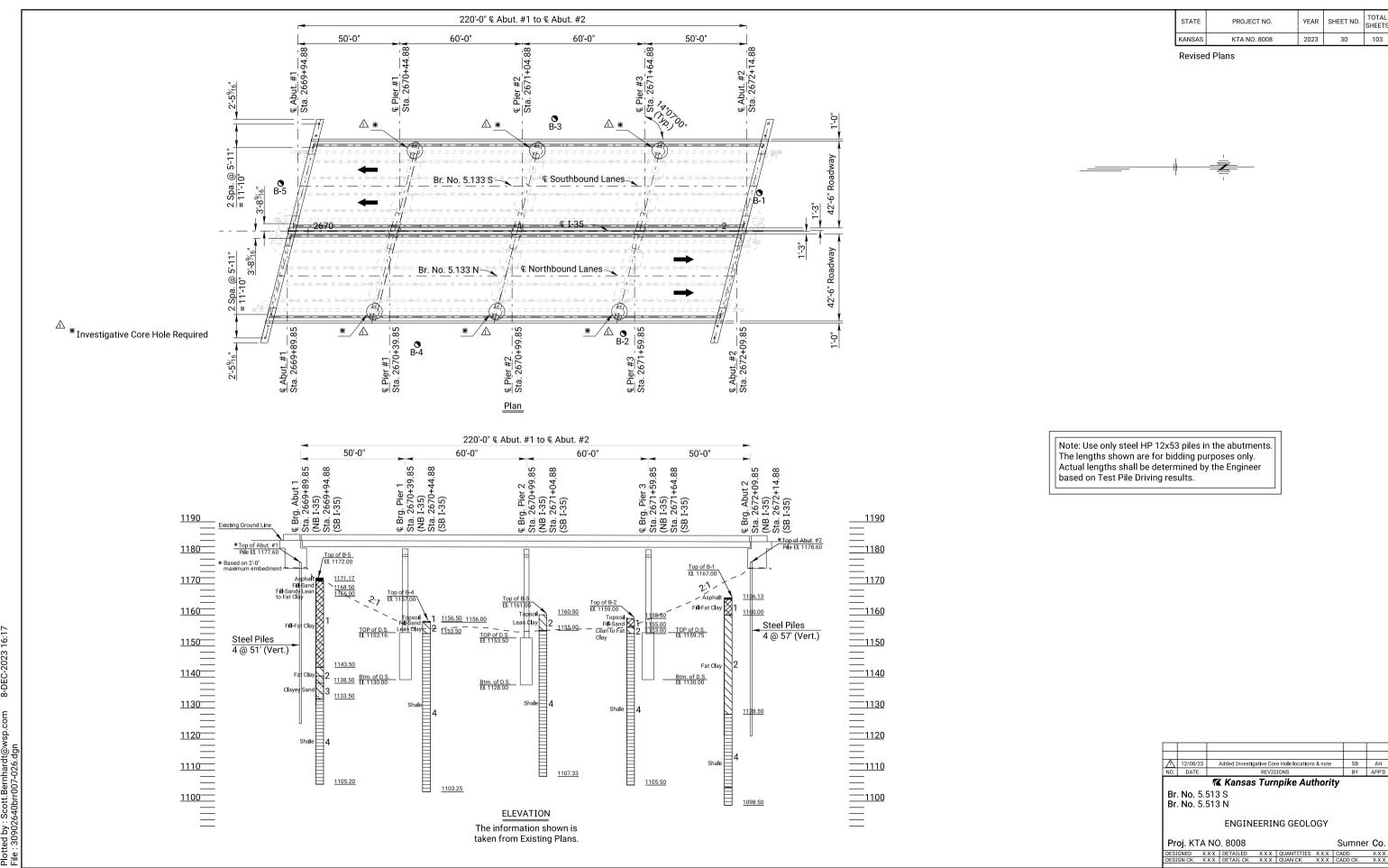
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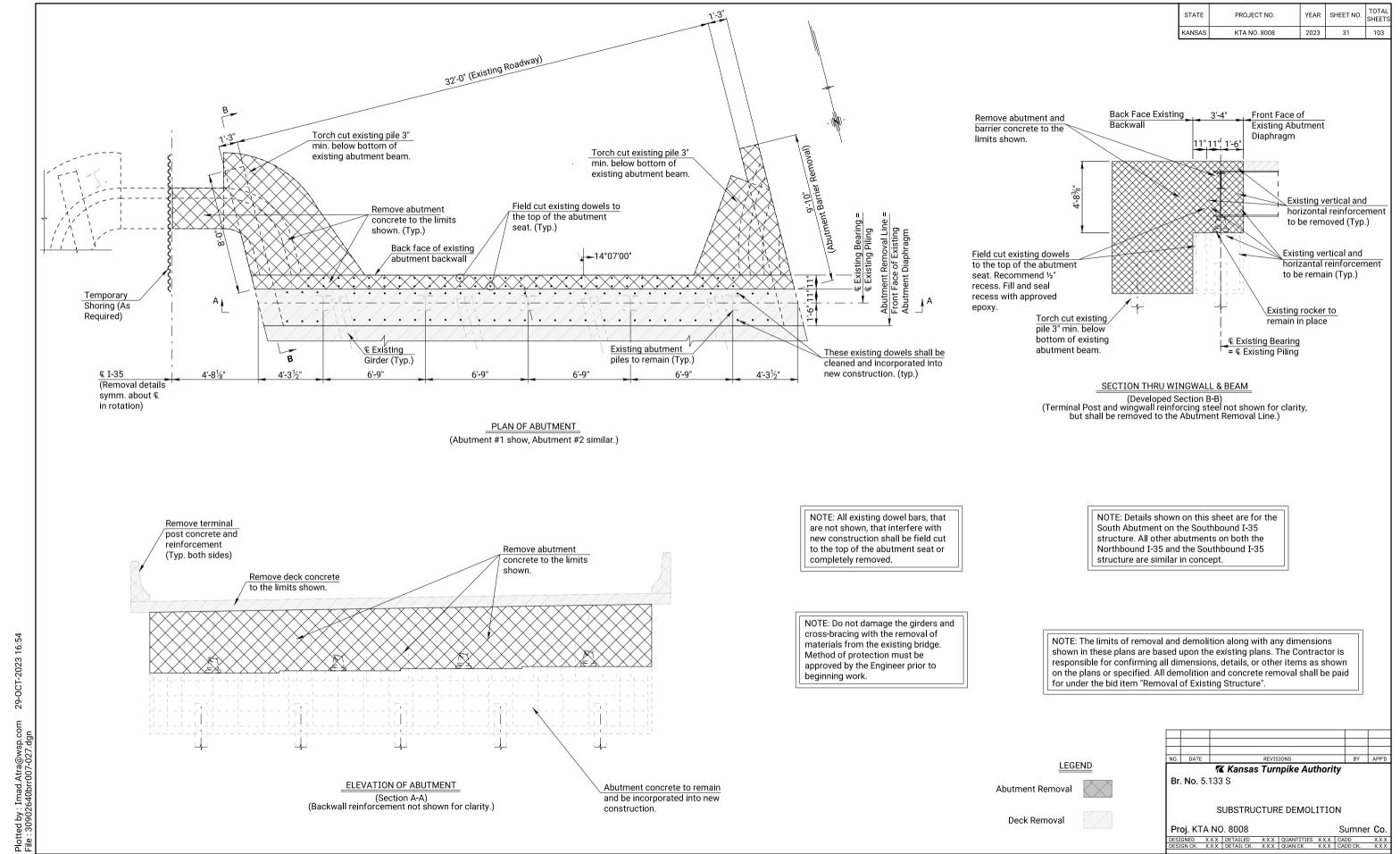
2023 28

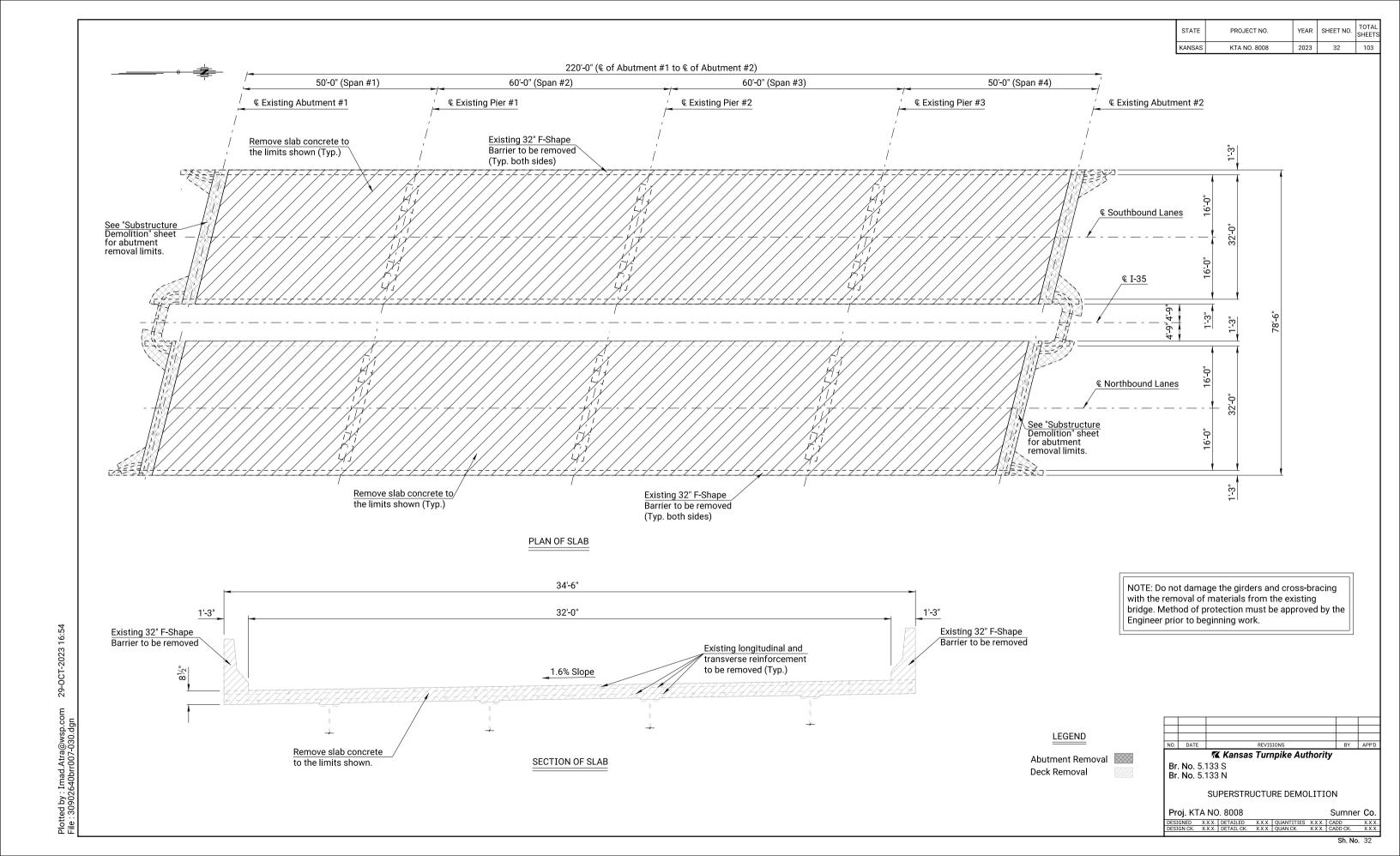
- FILLETS: Camber the steel girders for the total dead load deflection and the vertical curvature, if necessary. The ordinates shown for concrete dead load deflection represent the amount of camber that should be in the girders after they are erected and bolted but prior to placing the floor forms. After the structural steel is completely erected and the falsework bents are removed, measure the camber in the field by taking a profile of each girder. Correct any variation between the actual camber and the concrete dead load deflection shown in the plans by varying the depth of the concrete fillets over the girders so that the finished floor is constructed to the theoretical grade. The minimum depth of the slab over the girder shall be 9 inches not including the fillet.
- FALSEWORK PLANS: A licensed Professional Engineer shall design the falsework details. Details shall bear the seal of a licensed Professional Engineer. See the KDOT Bridge Design Manual, Section 16.1 "Review and Approval of Falsework Plans", for a listing of items to be included on the falsework plan. Submit electronic plans conforming to Section 105 of the Standard Specification with details in compliance with KDOT Specifications to the Field Engineer for review.
- FALSEWORK INSPECTION: This project has falsework plan requirements which are considered "Category 1" by KDOT specifications. The falsework designer of record will conduct an inspection of the asbuilt falsework. The bid item, "Falsework Inspection" is full compensation for all materials, labor and equipment. See KDOT specifications.
- TEMPORARY SHORING: The bid item "Temporary Shoring" includes all labor and material necessary to furnish shoring at the abutments for the removal of rocker bearings and support of superstructure until the replacement of bearing devices is complete. Maintain the temporary shoring until the Engineer authorizes its removal. Remove temporary shoring prior to concrete pour for abutment diaphragm. The temporary shoring plans are to be designed and sealed by a registered Professional Engineer. Submit design calculations and shoring plans to the Field Engineer for review 6 weeks before work is scheduled to begin. Work shall not begin until the Engineer grants approval. Temporary Shoring shall not hinder the long-term capacity for longitudinal movement of the superstructure. Any incidental temporary shoring required for the construction of the abutment diaphragms and wingwalls shall be included in the "Temporary Shoring" bid item.
- REPAIR OF EPOXY COATED REINFORCING STEEL: Replace any epoxy coating that is removed from the reinforcing steel during the concrete removal process. Thoroughly clean damaged areas with a stiff wire brush to remove dirt and damaged coating. Apply an approved patching material in accordance with the manufacturer's recommendations. Avoid dripping any patching material onto existing concrete that will have new concrete placed against it. See KDOT Specifications.
- MULTI-LAYER POLYMER CONCRETE OVERLAY: Apply the polymer overlay to the new bridge deck limits. See KDOT Specification Section 729 for more information.
- BOLTS: All bolts, nuts and hardened flat washers shall conform to the heavy hex structural requirements of ASTM F3125 Grade A325, Type 1, and KDOT Specifications unless otherwise noted. Direct Tension Indicators (DTIs) are to comply with the requirements of the latest edition of ASTM F959. No allowance will be made for high strength bolts used for permanent or temporary connections. This work is subsidiary to the bid item, "Structural Steel". The number of bolts is shown for the convenience of the Contractor.
- FABRICATION OF FIELD SPLICES: Prepare joints for the field splices in accordance with KDOT Specifications. Use Type "B" shop laydown.
- BEARING (STEEL REINFORCED ELASTOMERIC): Elastomeric Bearing Device shall be factory bonded to the steel sole plate by a vulcanization process. The steel sole plate is subsidiary to the bid item, "Bearing (Steel Reinforced Elastomeric)" and shall be furnished by the bearing device fabricator.
- CONCRETE: Superstructure concrete is bid as Concrete (Grade 4.0)(AE). Bevel all exposed edges of all concrete with a 34" triangular molding, except as otherwise noted on the plans. Construction joints are optional with the Contractor, but if used, place only at locations shown, or at locations approved by the Engineer.

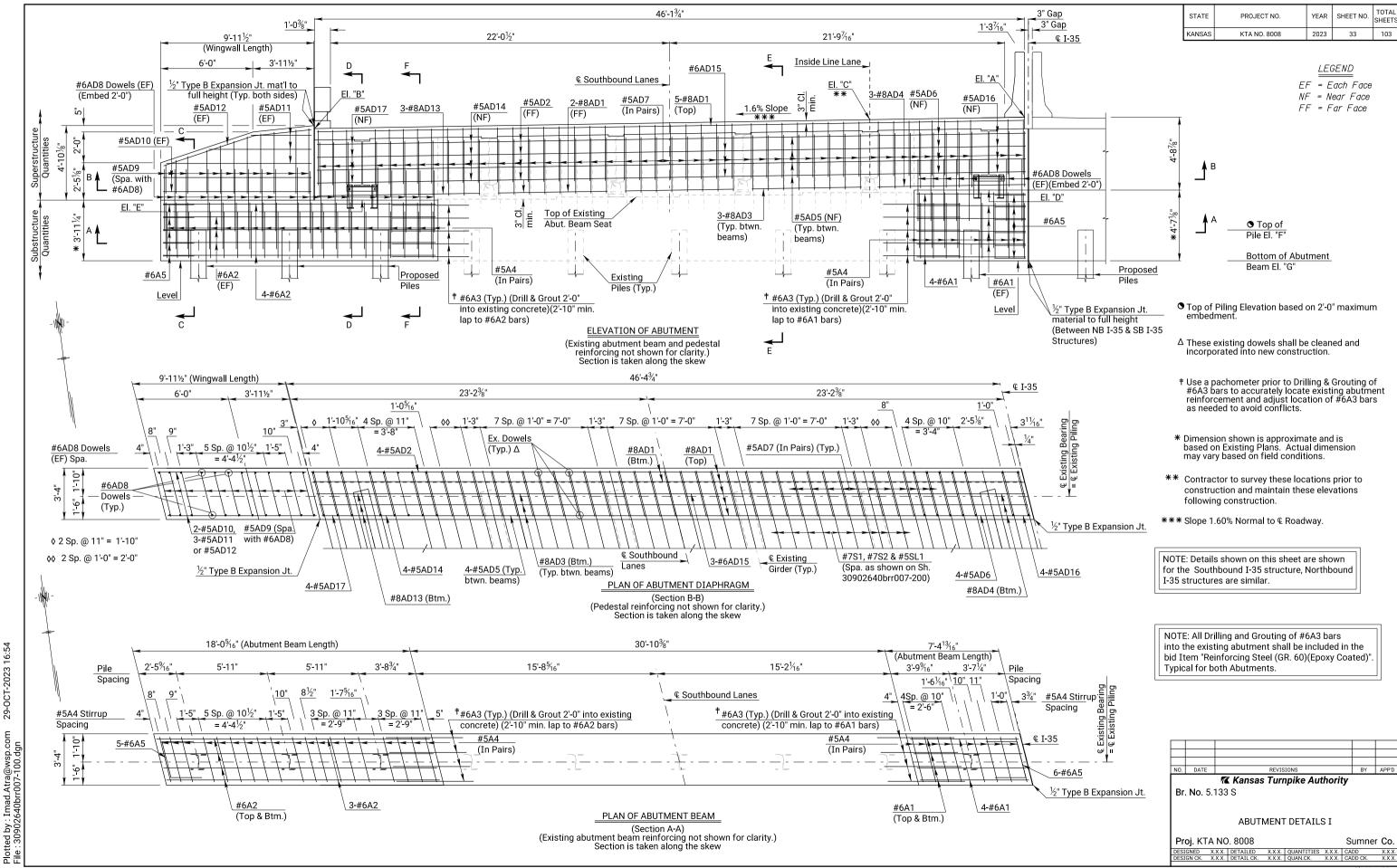
Λ	12/08/23	Added Sonic Testing & Investigative Core Holes note	s. SB	АН
NO.	DATE	REVISIONS	BY	APP'E
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		GENERAL NOTES		
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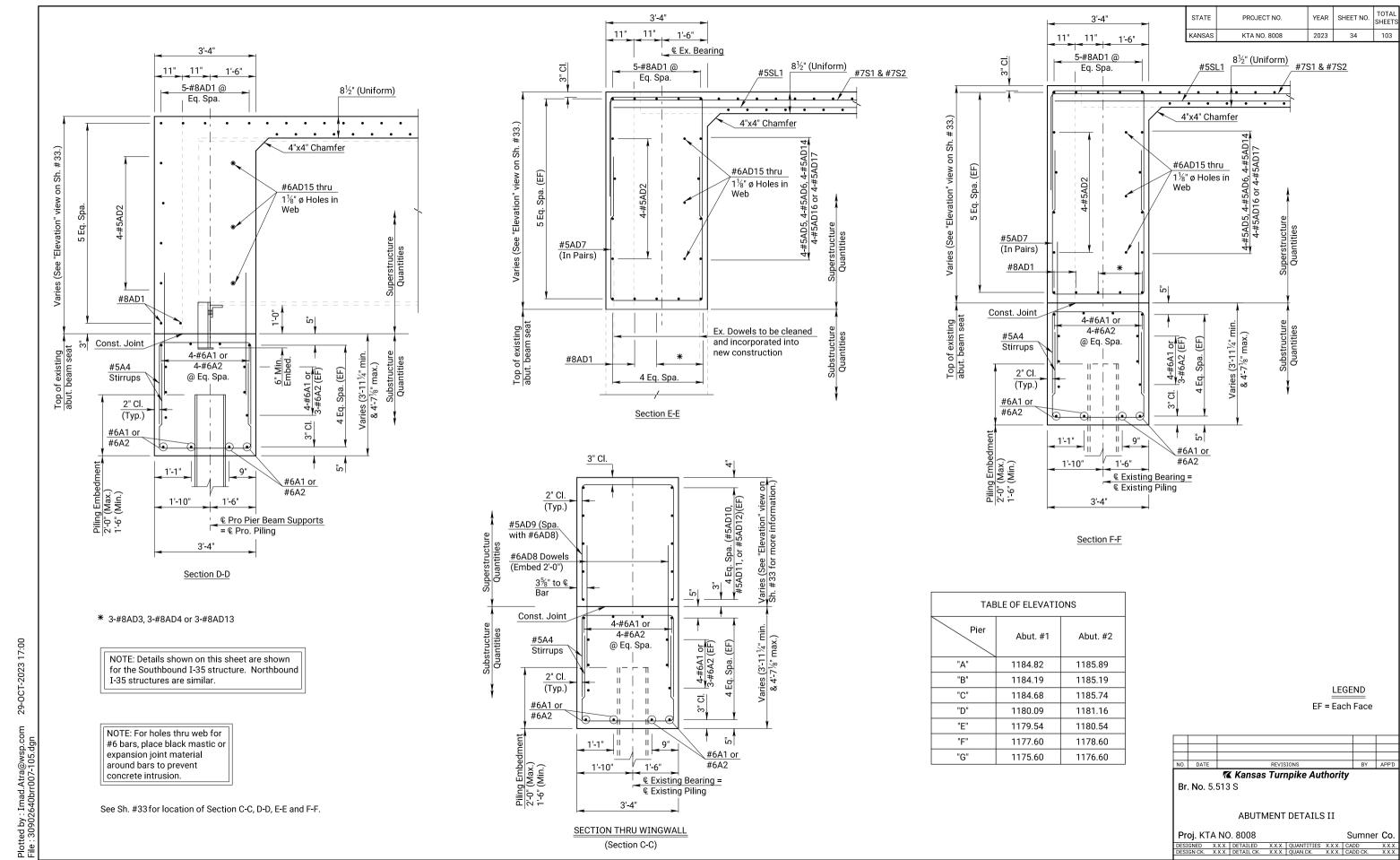


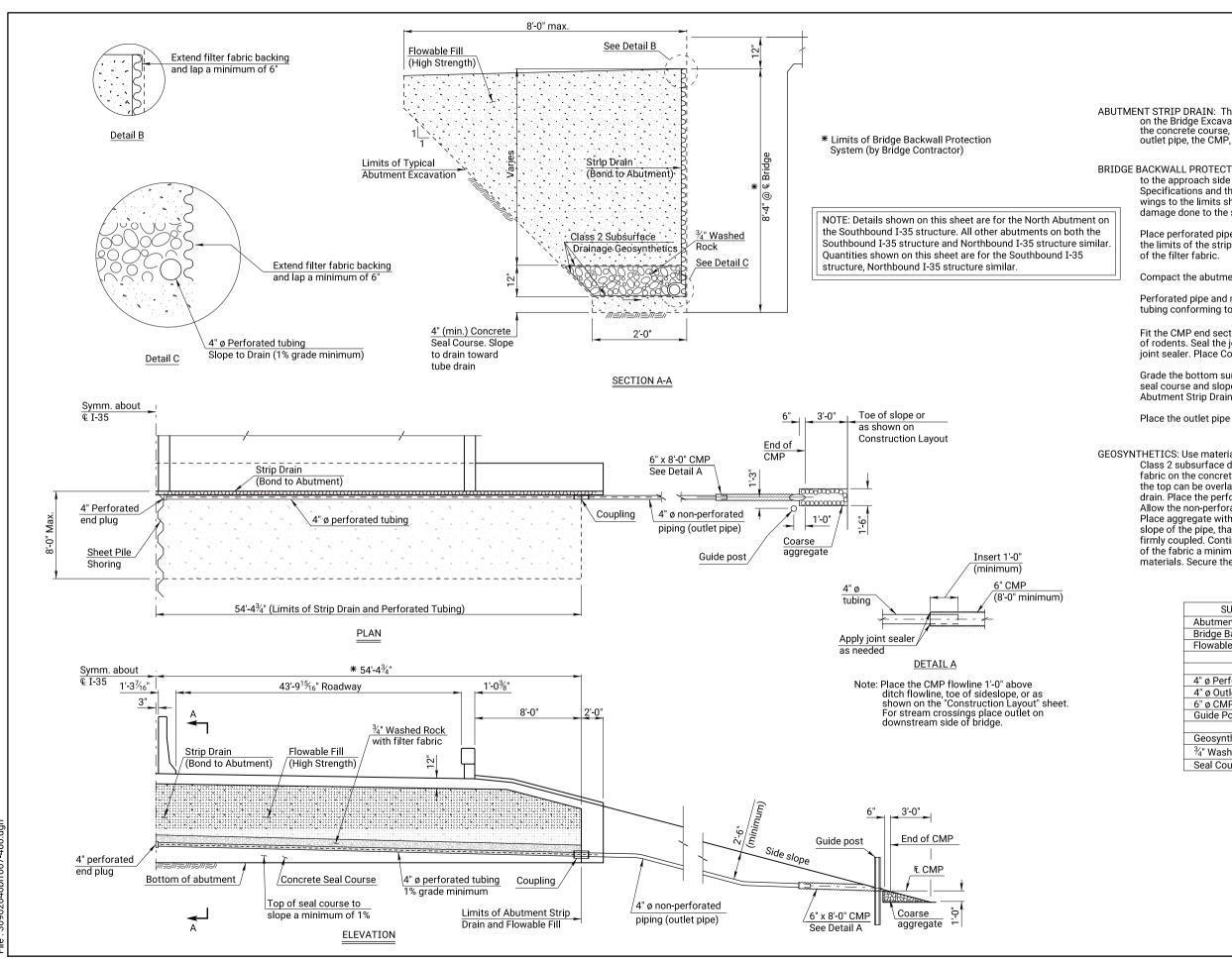












**GENERAL NOTES** 

STATE

KANSAS

PROJECT NO.

KTA NO. 8008

YEAR SHEET NO.

35

2023

ABUTMENT STRIP DRAIN: The Bridge Contractor shall excavate to the limits shown on the Bridge Excavation sheet, grade the bottom of the backfill area, construct the concrete course, place the strip drain, and place the perforated pipe, the outlet pipe, the CMP, and the backfill.

BRIDGE BACKWALL PROTECTION SYSTEM: Apply a Bridge Backwall Protective System to the approach side of the abutments and the wings in accordance with KDOT Specifications and the manufacturer's recommendations. Cover the abutments and wings to the limits shown on the details. Prior to backfilling, repair any damage done to the system at no charge to the KTA.

Place perforated pipe next to the strip drain. Use non-perforated pipe outside the limits of the strip drain. Enclose the perforated pipe with the extension of the filter fabric.

Compact the abutment backfill. See the KDOT Specifications.

Perforated pipe and non-perforated outlet pipe shall be corrugated polyethylene tubing conforming to KDOT Specifications.

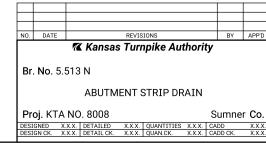
Fit the CMP end section with  $\frac{1}{4}$ " galvanized mesh screen to prevent the entrance of rodents. Seal the joint between the outlet pipe and the end section with a joint sealer. Place Coarse aggregate at the outlet end as shown.

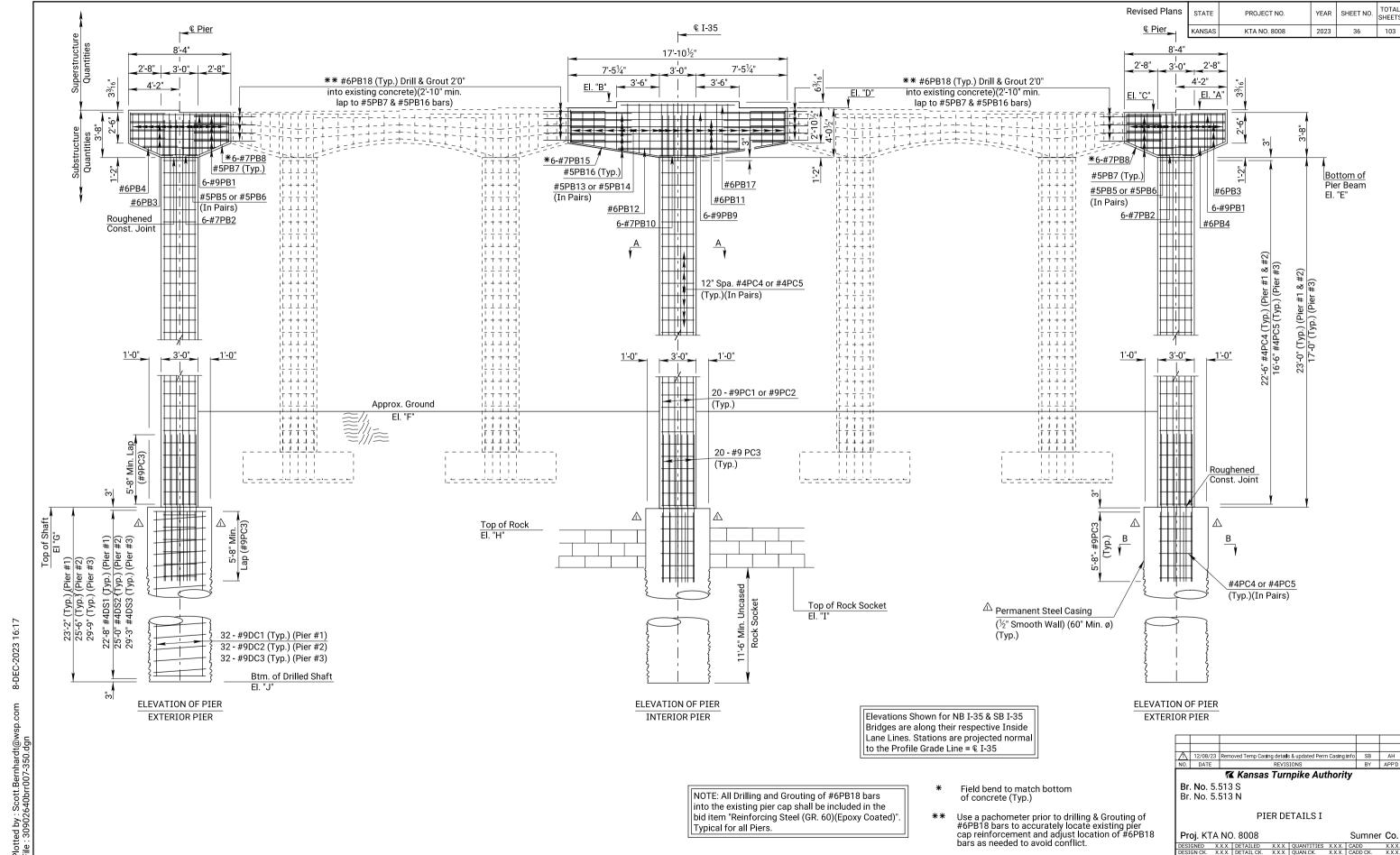
Grade the bottom surface of the excavated area to drain. Construct the concrete seal course and slope to drain. Materials and labor shall be  $\underline{\text{subsidiary}}$  to Abutment Strip Drain.

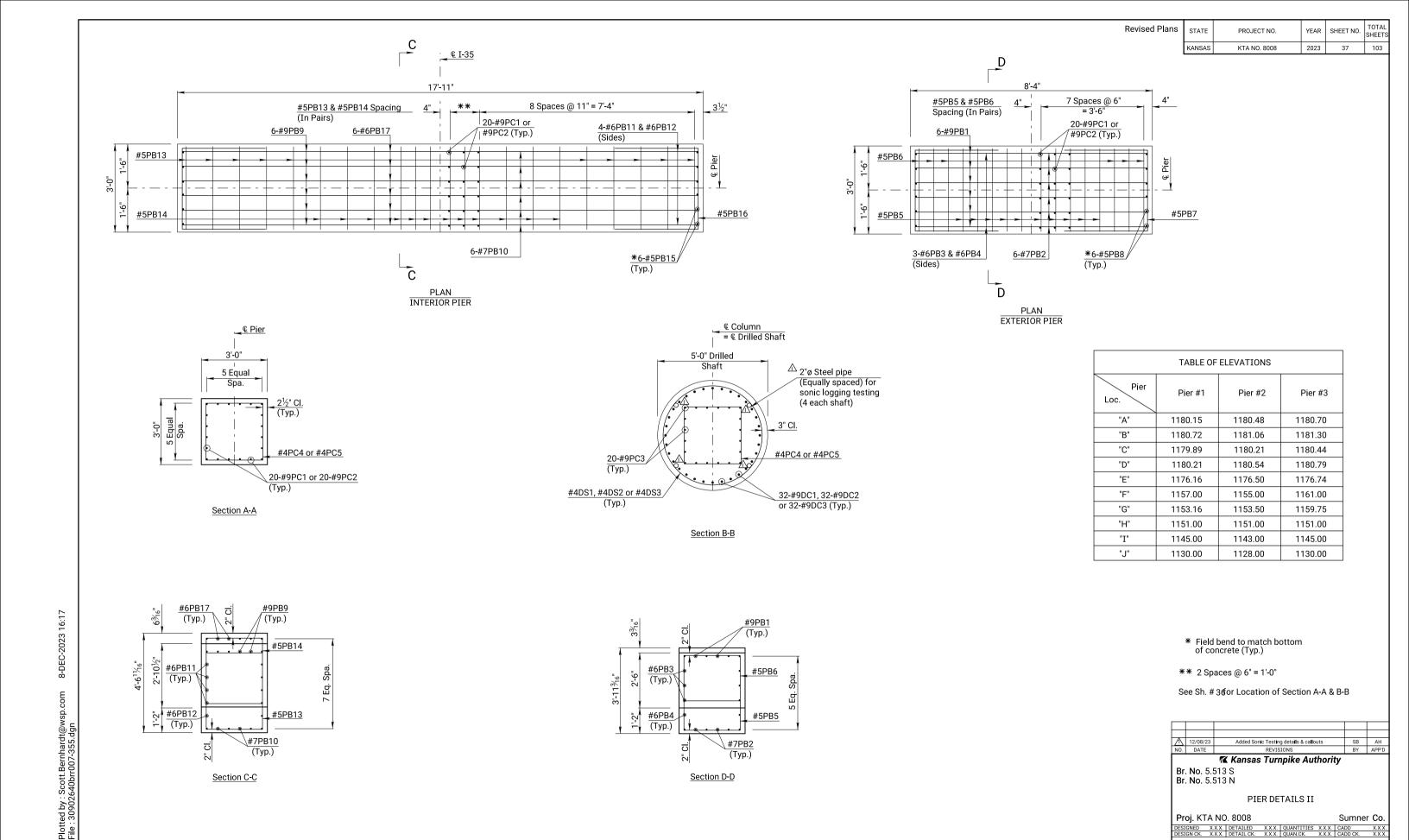
Place the outlet pipe as shown or noted (See the "Construction Layout" sheet).

GEOSYNTHETICS: Use material that complies with KDOT Specification Section 1710
Class 2 subsurface drainage fabric. Place the Class 2 subsurface drainage
fabric on the concrete seal course as shown. Allow for enough material so that
the top can be overlapped and the end folded to completely enclose the aggregate
drain. Place the perforated drain pipe and couple to non-perforated pipe as shown.
Allow the non-perforated pipe to pass through a hole carefully cut in fabric.
Place aggregate within fabric to just leave the top of the pipe visible. Verify the
slope of the pipe, that it is not damaged or displaced and that the couplers are
firmly coupled. Continue to backfill to the elevation and shape shown. Lap the top
of the fabric a minimum of 1'-6", fold and wrap the ends to enclose the drainage
materials. Secure the folds and wraps by sewing or approved methods.

(butments)		
,		
82.9 Sq. Yds.		
94.7 Sq. Yds.		
183.3 Cu. Yds.		
Items <u>subsidiary</u> to Abutment Strip Drain		
109 Lin. Ft.		
101 Lin. Ft.		
16 Lin. Ft.		
2 Each		
ligh Strength)		
96.2 Sq. Yds.		
10.1 Cu. Yds.		
7.9 Cu. Yds.		







Sumner Co.

Proj. KTA NO. 8008

# FRAMING PLAN

Details shown are shown for the Southbound I-35 structure, Northbound I-35 structure similar.

# **GENERAL NOTES**

STURCTURAL STEEL: All W36x150 rolled girders, abutment, Field Splice & bent plate diaphragms, stiffeners shall meet AASHTO M270 (Grade 50W T3) requirements except as noted. All other structural steel shall meet ASTM A709 (Grade 50W), unless noted otherwise. Field Splices shall be made only where shown in these Contract Plans as a "splice". Elimination of any "splice" may be requested.

PAINTING: The shop coats applied to Structural Steel shall conform to an inorganic zinc primer.

Blast clean to meet SSPC-SP10 Specifications and prime coat the embedded portion of the girders, including the abutment diaphragms; the top flanges, including the shear studs in accordance with KDOT Specifications.

Touch Up: Prepare and paint all bolts, nuts, studs, and other small areas of damaged paint (1 yd2 or less), requiring touch up, with an approved organic zinc primer.

All labor and material for painting of the structural steel and concrete surface will be subsidiary to other bid items.

WELDING: Material, Fabrication and Construction shall conform to KDOT Specifications. On the shop drawings, show a code or symbol in the tail of the weld symbol that refers to an approved, pre-qualified weld procedure.

FILLETS: Construct the finished deck to plan grade by varying the depth of the fillet over the beam to provide for beam profile, concrete dead load deflection and, if necessary, vertical curvature. After the beams are completely erected and the falsework bents are removed, profile each beam. Correct any variation between the actual profile and the concrete dead load deflection shown in the plans by varying the depth of the concrete fillets over the beams so that the finished floor is constructed to the theoretical grade. The minimum depth of the slab over the beam shall be 9" inches.

BOLTS: All bolts, nuts and hardened flat washers shall conform to the heavy hex structural requirements of ASTM F3125 Grade A325. Type 3, and KDOT Specifications unless otherwise noted. Direct Tension Indicators (DTIs) are to comply with the requirements of the latest edition of ASTM F959. No allowance will be made for high strength bolts used for permanent or temporary connections. This work is subsidiary to the bid item, "Structural Steel". The number of bolts is shown for the convenience of the Contractor.

FABRICATION OF FIELD SPLICES: Prepare joints for the field splices in accordance with KDOT Specifications. Use Type "B" shop laydown.

NOTE: Shop Drawings shall be submitted to the Engineer for approval prior to fabrication for the proposed rolled beam segments, all diaphragm assemblies and pier beam support assemblies.

STRUCTURAL STEEL SUMMARY									
Girder Components	M270 Gr. 50W T3, Lbs.	M270 Gr. 50W Lbs.							
W 36 x 150 Girder	66,225	-							
Abutment Diaphragm Assemblies	1,043	-							
Bent Plate Diaphragm Assemblies	5,154	-							
Stiffeners (Pier Bearing & Diaphragm)	2,049	-							
Field Splice Plates	2,394	-							
Pier Beam Support Assemblies	-	391							
Total Structural Steel	76,865	391							

Note: Quantities shown are listed for Northbound I-35 (5.133 N) structure ONLY, Southbound I-35 (5.133 S) structure the same.

\*Dimensions shown are based on Existing Plans. Actual dimension may vary based on field conditions.

#### **LEGEND**

Proposed Structural Steel Existing Structural Steel

◆ F.S. = Field Splice

NO.	DATE	REVISIONS	BY	APP'D						
Kansas Turnpike Authority										
Br. No. 5.133 S										

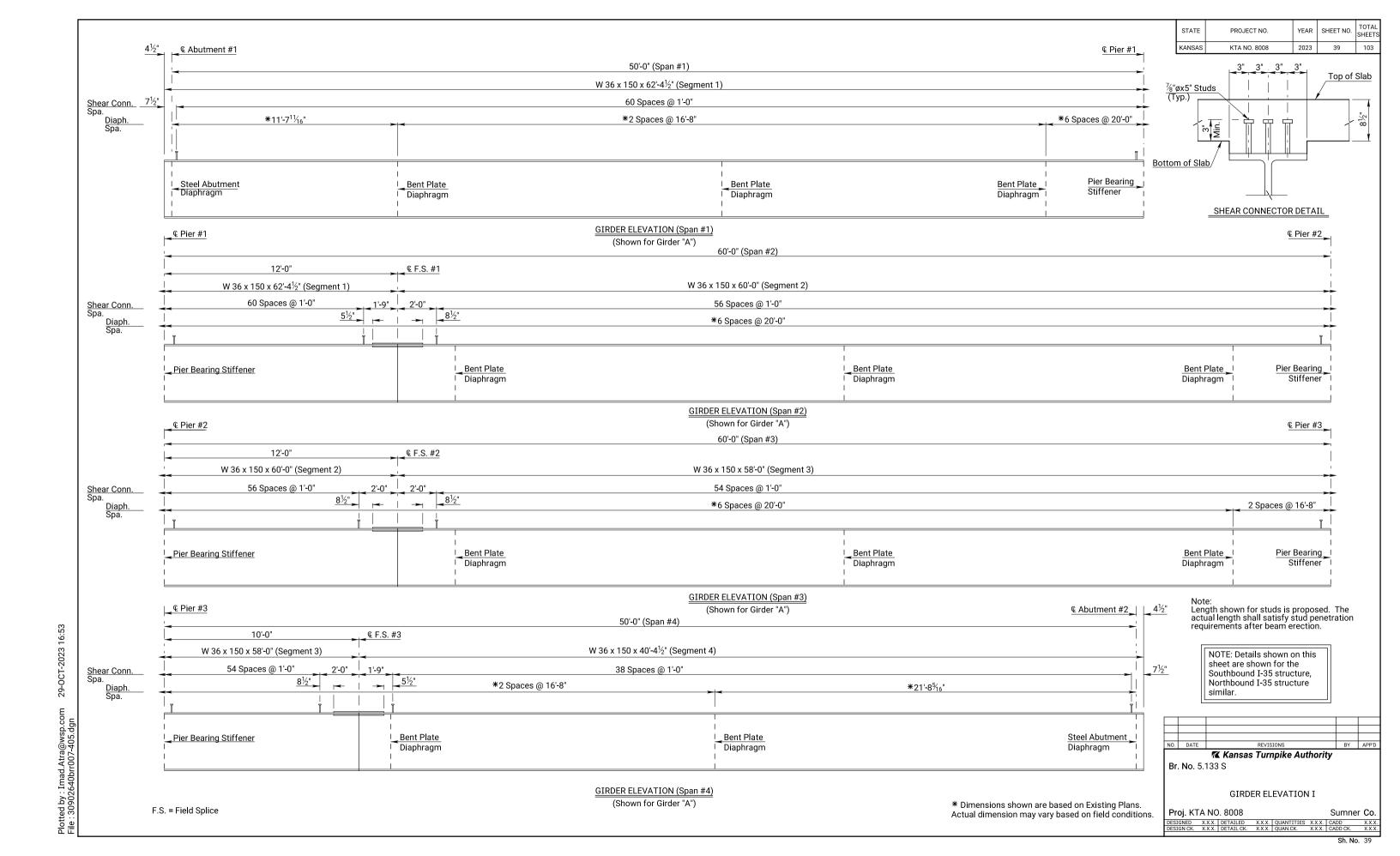
352 - 7/8" ø A325 Bolts Required

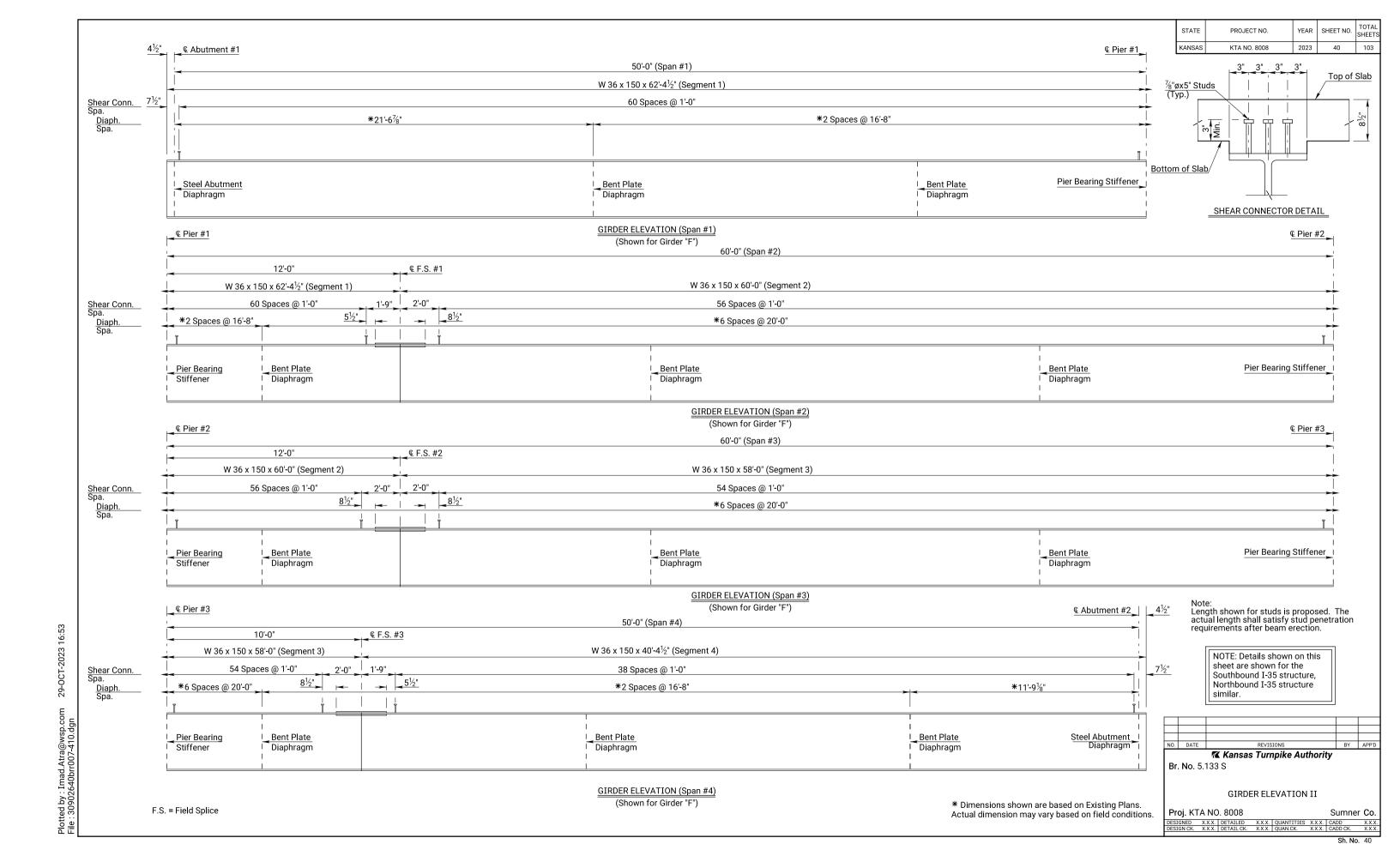
252 For Abutment and Bent Plate Diaphragms.

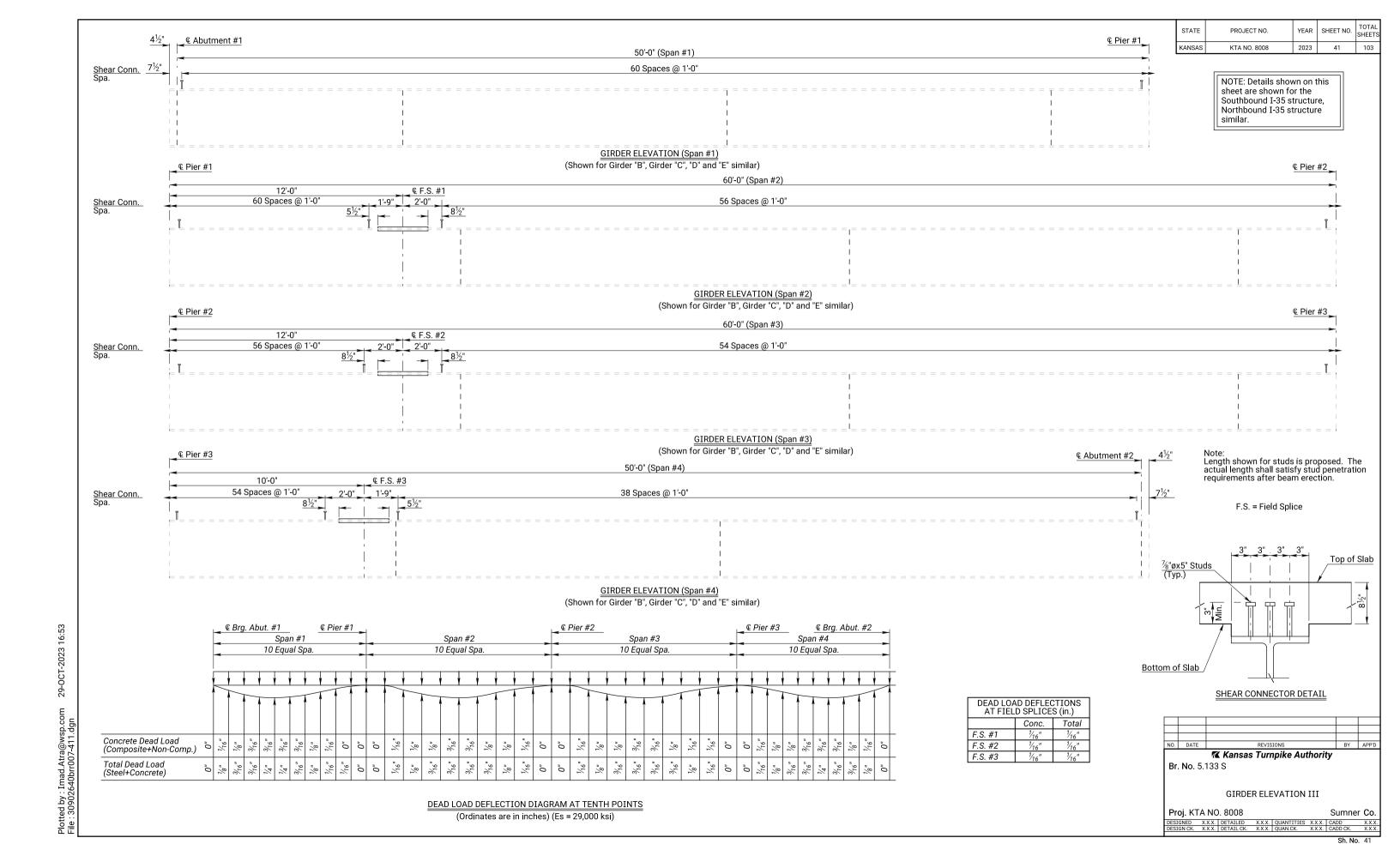
100 For Field Splices.

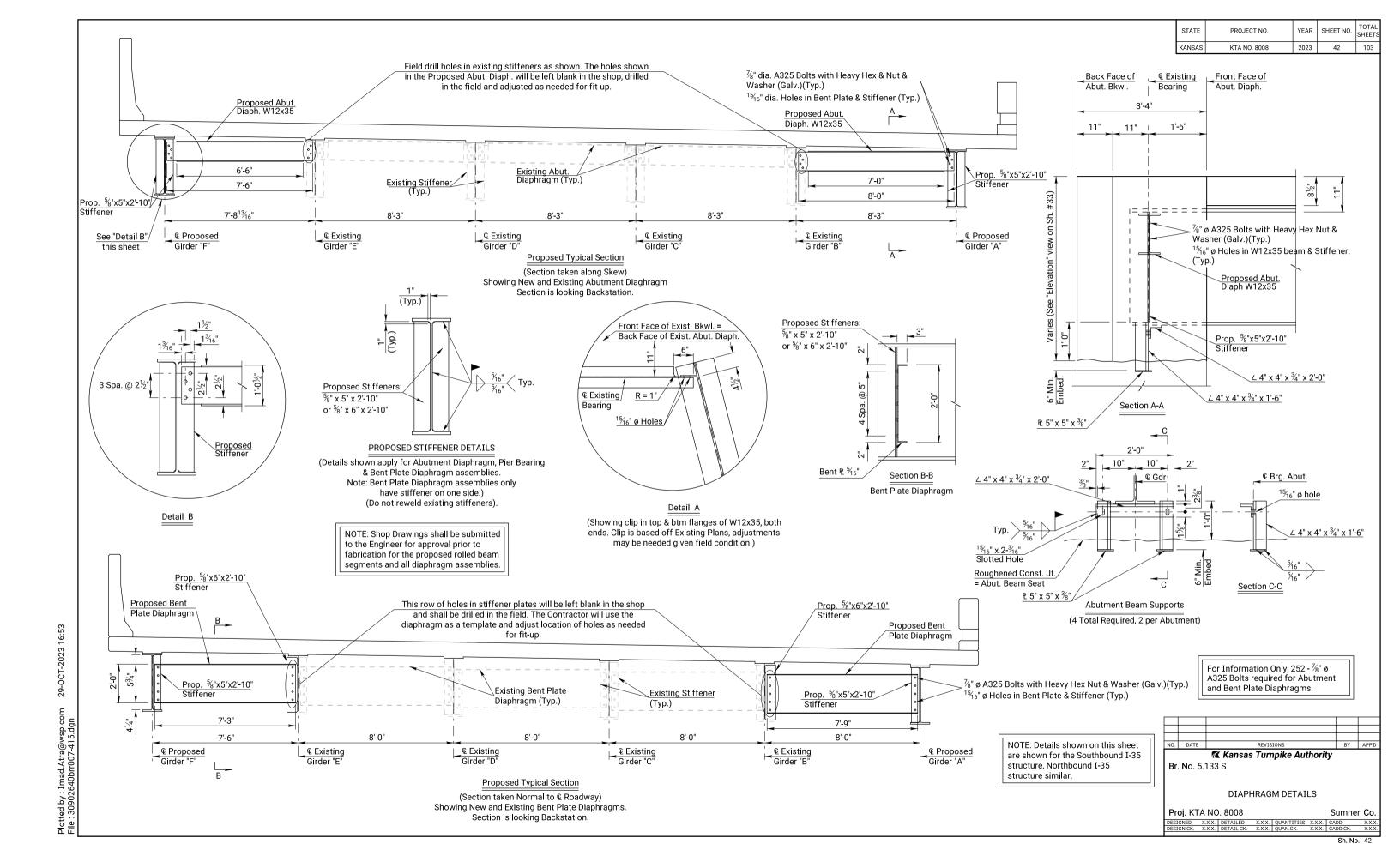
FRAMING PLAN

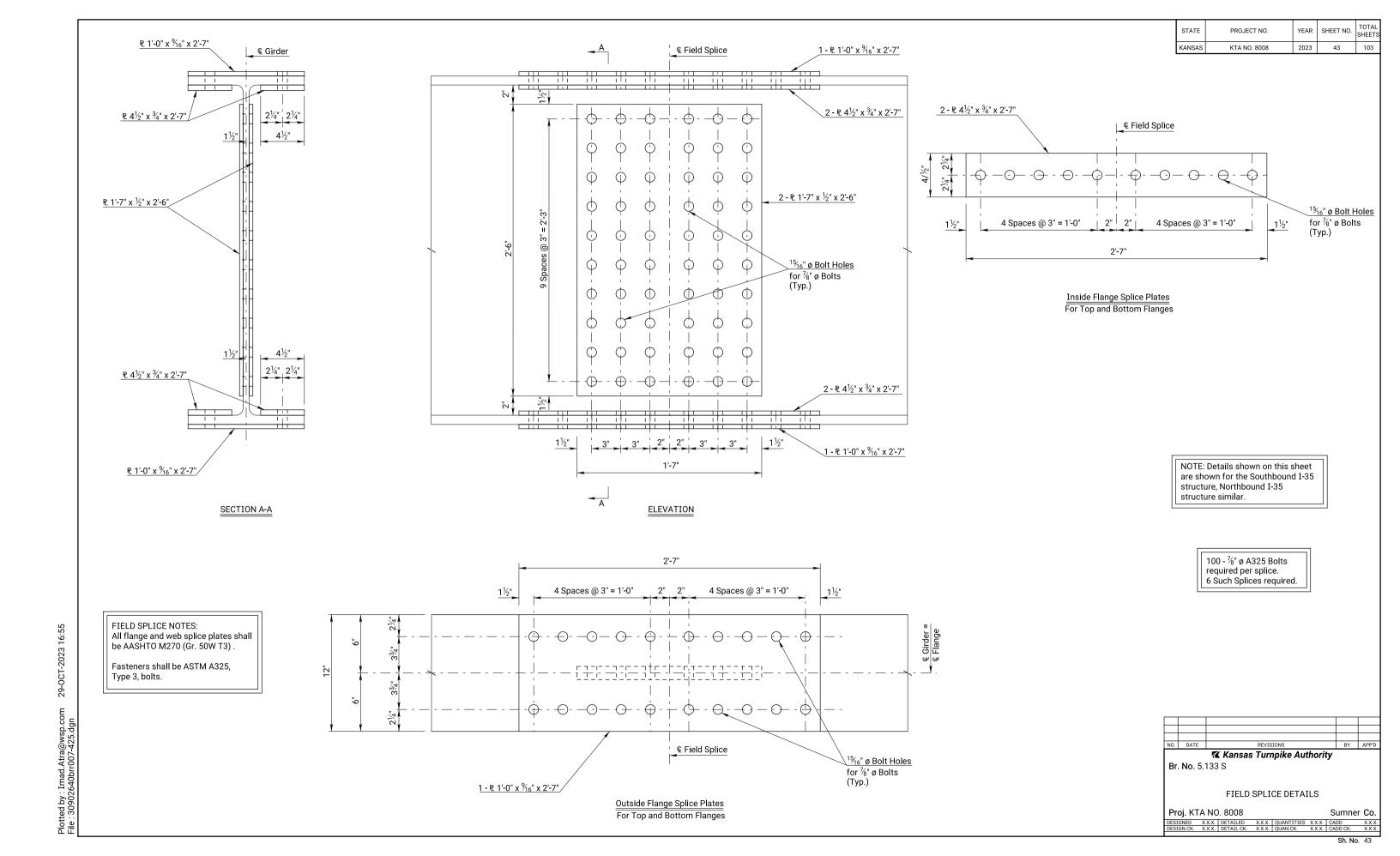
Proj. KTA NO. 8008 Sumner Co.

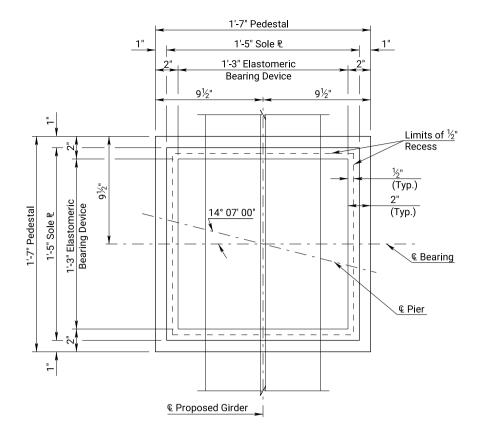




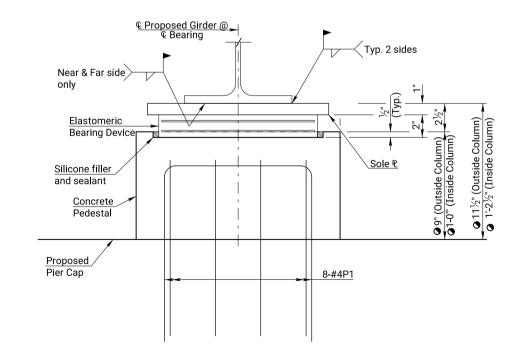




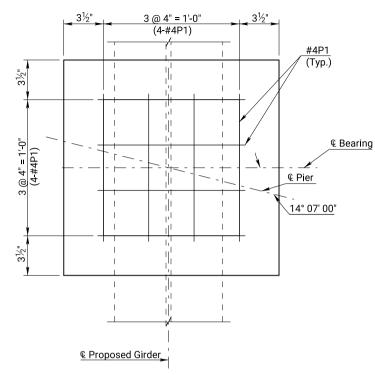


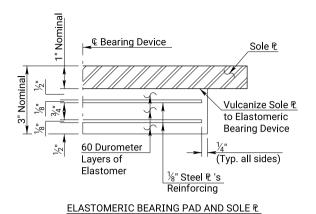


### PLAN OF BEARING DEVICE



**ELEVATION OF BEARING DEVICE** 





(2 Bearings required at each Pier;

6 Bearings required total.)

All welds shall be  $\frac{5}{16}$ " unless otherwise shown.

NOTES:

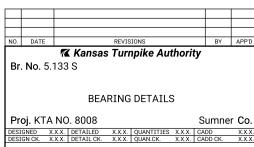
Submit Shop Drawings of Bearing Device assemblies for approval prior to fabrication.

The bid item "Bearing (Steel Reinforced Elastomeric)" includes the Sole ₱ and reinforced elastomeric bearing pad. Sandblast and paint all steel as per KDOT Specifications and latest special provisions for primer and polyurethane finish coat. All steel shall be ASTM A36.

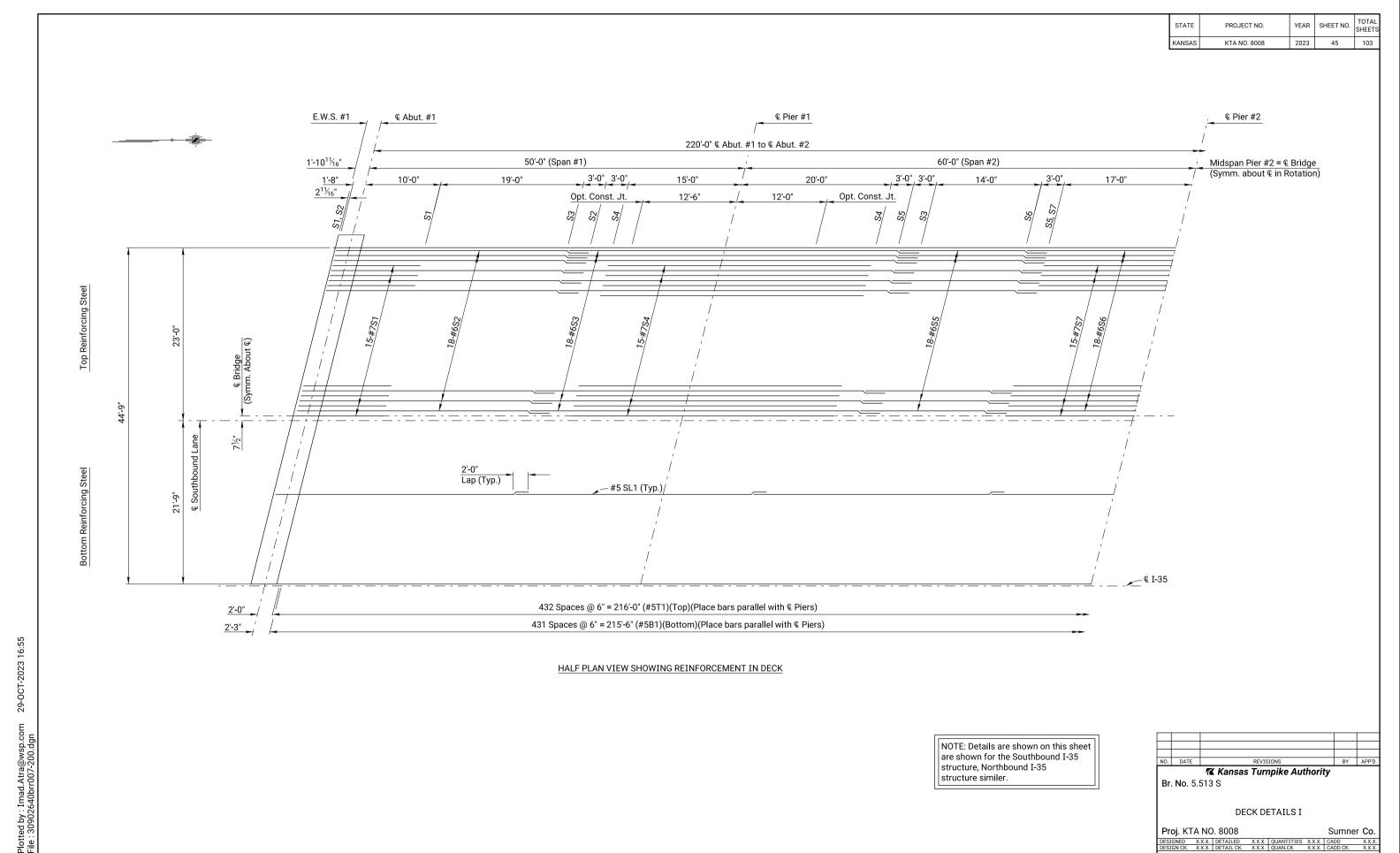
# PLAN OF PEDESTAL REINFORCING

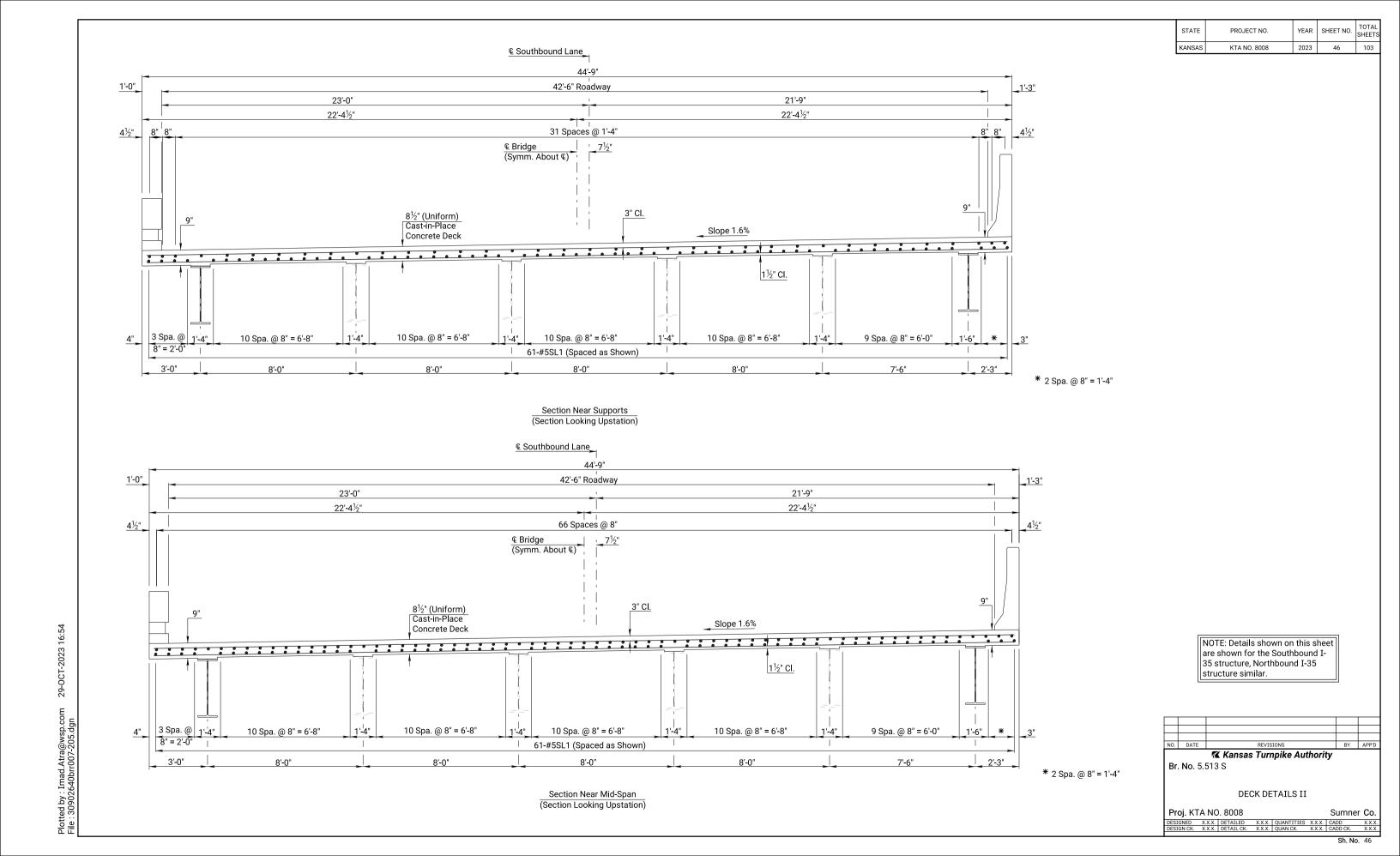
NOTE: Details shown on this sheet are shown for the Southbound I-35 structure, Northbound I-35 structure similar.

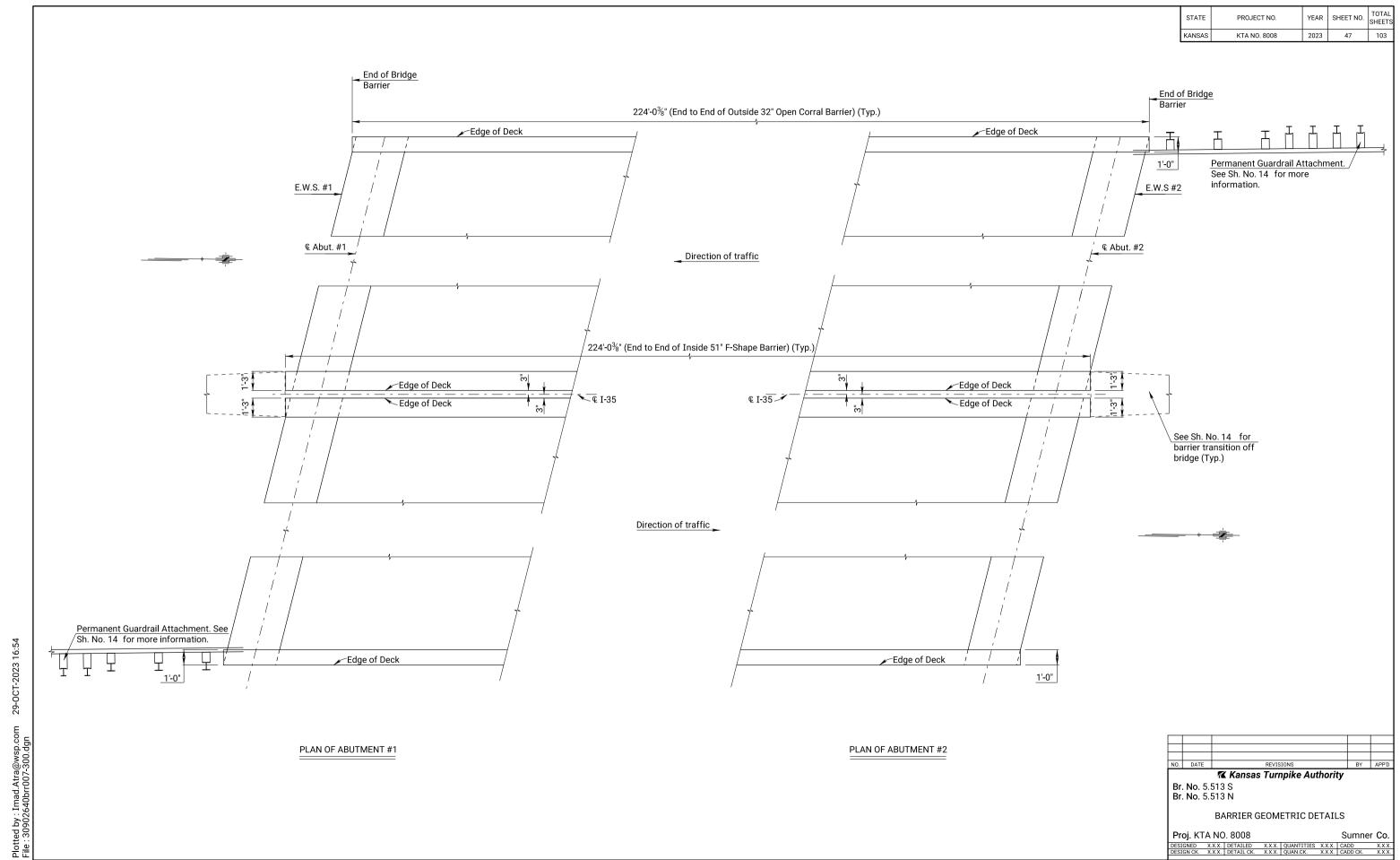
Dimension shown is approximate and is based on Existing Plans. Actual dimension may vary based on field conditions.

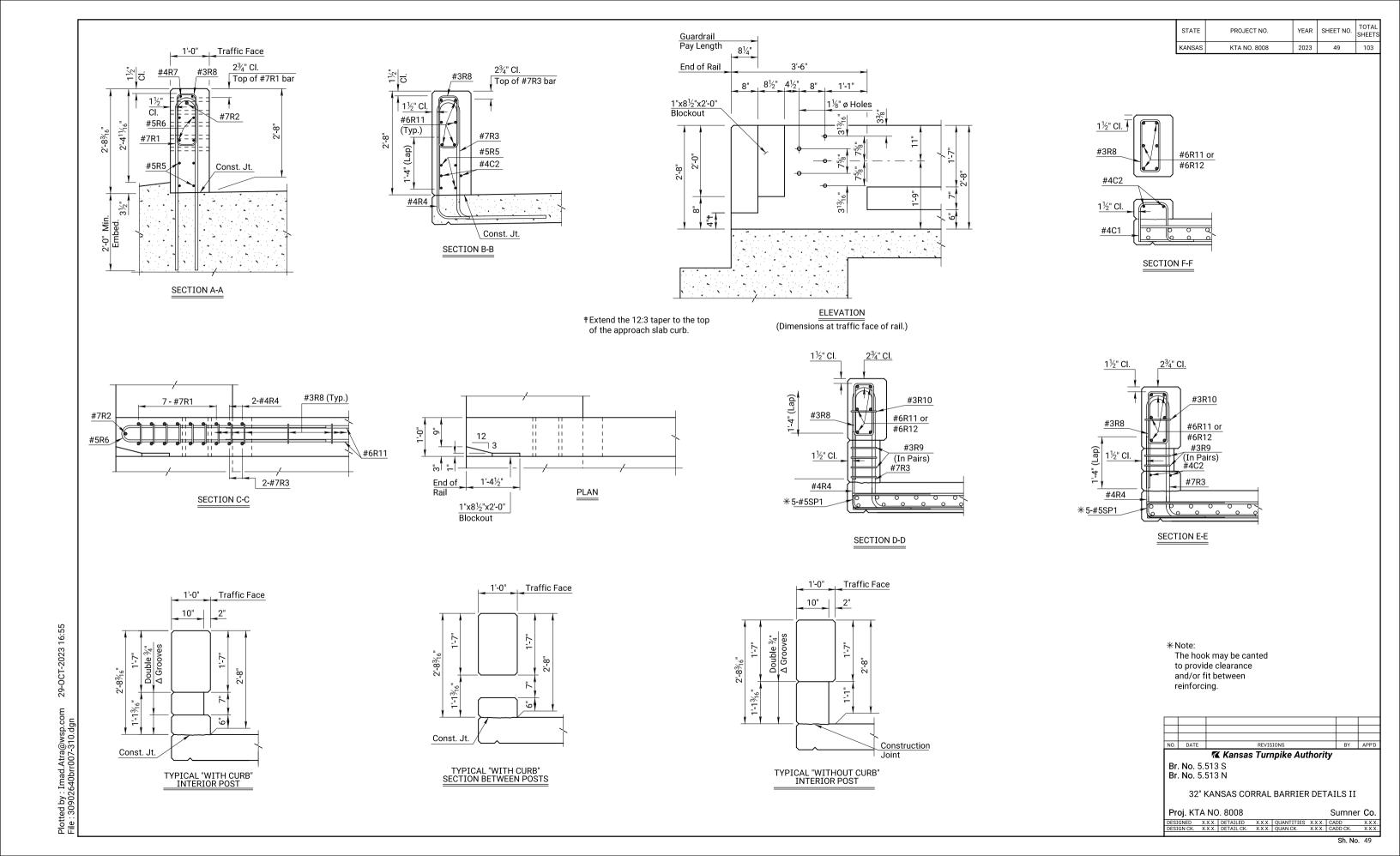


Plotted by : Imad.Atra@wsp.com File : 30902640brr007-150.dgn









	ļ ģ	AD8	#6	32	4'-0"				
	Abutment Diaphra	AD15	#6	28	46'-0"				
	ţΩ								
	l Jer	AD2	#5	8	46'-0"				
	튬	AD5	#5	24	7'-11"				
	Ab	AD6	#5	8	7'-4"				
		AD10	#5	8	9'-7"				
		AD14	#5	8	7"-11"				
		AD16	#5	8	2'-2"				
		AD17	#5	8	2'-9"				
		R11	#6	12	9'-0"	R1	#7	14	9'-3'
		R12	#6	120	9'-8"	R2	#7	2	5'-7'
꿆		R18	#6	10	21'-6"	R3	#7	172	7'-8'
SUPERSTRUCTURE		R19	#6	45	19'-7"				
≌						R5	#5	4	6'-6'
STF		C2	#4	32	7'-8"	R6	#5	4	10'-8
E		R13	#4	4	21'-6"	SP1	#5	105	4'-4'
l H		R15	#4	18	19'-7"	R14	#5	294	6'-6'
5		R20	#4	24	38'-10"	R16	#5	278	6'-0'
	Rail					R17	#5	16	5'-11
	_					R21	#5	294	5'-0'
						R4	#4	172	4'-8'
						R7	#4	2	10'-8
						C1	#4	48	3'-3'
						R8	#3	296	4'-4'
						R9	#3	62	4'-6'
						R10	#3	42	4'-6'
		S4	#7	58	35'-0"	S1	#7	58	14'-2
		S7	#7	29	34'-0"				
		S2	#6	72	33'-8"				
		S3	#6	72	47'-0"				
	Deck	S5	#6	72	20'-0"				
		S6	#6	36	40'-0"				
		SL1	#5	427	33'-11"				
		T1	#5	433	45'-7"				
		B1	#5	432	45'-7"				

BILL OF REINFORCING STEEL
EPOXY COATED - GRADE 60
(LISTED FOR SOUTHBOUND I-35 STRUCTURE
NORTHBOUND I-35 STRUCTURE IDENTICAL)

Mark Size Number Length Mark Size Number Length

AD7

AD9

AD11

AD12 #5

45'-9"

7'-0"

9'-3"

10'-6"

Bent Bars

90 22

12

4

 $\otimes$  See Bending Diagrams.

#5

#5

#5

9'-7"

 $\otimes$ 

 $\otimes$ 

9'-10"

Straight Bars

14

18

6

#8

#8

#8

#8

AD1

AD3

AD4

AD13

			BILL OF REINFORCING STEEL NON-EPOXY COATED - GRADE 60 (LISTED FOR SOUTHBOUND I-35 STRUCTURE NORTHBOUND I-35 STRUCTURE IDENTICAL) Straight Page									
Straight Bars Bent Bars												
		Mark	Size	Number	Length	Mark	Size	Number	Length			
Œ	t											
	Abutment Beam	A1	#6	60	6'-10"	A5	#6	22	7'-1"			
SUB- STRUCTURE	utr Jeal	A2	#6	52	17'-8"							
TR	Abu	A3	#6	6	4'-10"	A4	#5	54	9'-5"			
N.												

 $\otimes$  See Bending Diagrams.

			<ul> <li>* BILL OF REINFORCING STEEL</li> <li>NON-EPOXY COATED - GRADE 60</li> <li>(LISTED FOR NORTHBOUND I-35 STRUCTURE)</li> </ul>									
			Stra	ight Bars	ł		Bei	nt Bars				
		Mark	Size	Number	Length	Mark	Size	Number	Length			
		PB1	#9	18	8'-0"	PB8	#7	36	6'-0"			
		PB9	#9	18	17'-7"	PB15	#7	36	10'-10"			
	Pier Beam	PB2	#7	18	3'-0"	PB5	#5	60	8'-0"			
		PB10	#7	18	3'-0"	PB6	#5	36	7'-6"			
						PB7	#5	30	7'-10"			
JRE		PB3	#6	18	8'-0"	PB13	#5	60	7'-10"			
티		PB4	#6	6	5'-10"	PB14	#5	72	8'-10"			
SUBSTRUCTURE		PB11	#6	24	17'-7"	PB16	#5	36	11'-8"			
STI		PB12	#6	6	11'-0"							
B		PB17	#6	18	9'-8"	P1	#4	48	5'-3"			
S		PB18	#6	108	4'-10"							
	±					l						
	ha	PC1	#9	80	26'-3"	PC4	#4	228	6'-11"			
	S =	PC2	#9	40	20'-3"	PC5	#4	90	6'-11"			
	٥	PC3	#9	120	11'-10"	△ DS1	#4	2	$\otimes$			
	Pier & Drill Shaft	△DC1	#9	64	22'-10"	△DS2	#4	2	$\otimes$			
	Pie	△DC2	#9	64	25'-2"	△ DS3	#4	2	$\otimes$			
		△DC3	#9	64	29'-5"							

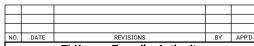
 $\otimes$  See Bending Diagrams.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	KTA NO. 8008	2023	51	103

					L OF REINF -EPOXY CO			`	
			(1		R SOUTHB				
				ight Bars		T		nt Bars	
		Mark	Size	Number	Length	Mark	Size	Number	Length
		IVIGIR	OIZC	Hamber	Length	IVIGIR	OIZC	Hamber	Length
		PB1	#9	18	8'-0"	PB8	#7	36	6'-0"
		PB2	#7	18	3'-0"	PB5	#5	60	8'-0"
	۱٤					PB6	#5	36	7'-6"
	Pier Beam	PB3	#6	18	8'-0"	PB7	#5	30	7'-10"
	E	PB4	#6	6	5'-10"				
SUBSTRUCTURE	🚡	PB18	#6	54	4'-10"	P1	#4	24	5'-3"
E									
Ŋ.									
SST									
SUE									
0,									
	l af	PC1	#9	40	26'-3"	PC4	#4	114	6'-11"
	<u>                                    </u>	PC2	#9	20	20'-3"	PC5	#4	45	6'-11"
		PC3	#9	60	11'-10"	△ DS1	#4	1	$\otimes$
	Pier & Drill Shaft	△DC1	#9	32	22'-10"	△DS2	#4	1	$\otimes$
	Pie	△DC2	#9	32	25'-2"	△ DS3	#4	1	$\otimes$
		△DC3	#9	32	29'-5"				

 $\otimes$  See Bending Diagrams.

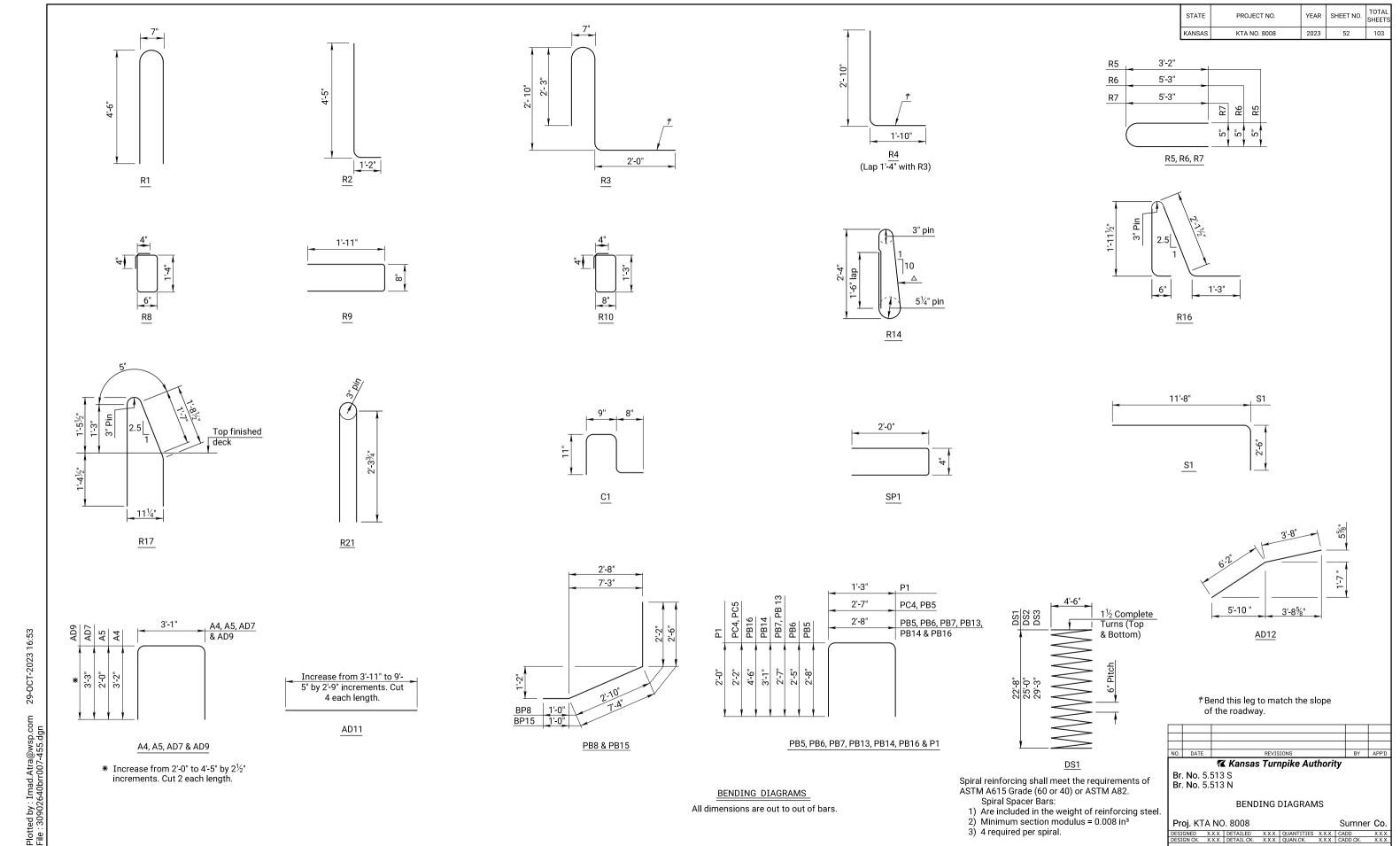
- All NB Exterior Pier bars and all shared
  Interior Pier bars are included
- \*\* All SB Exterior Pier bars are included
- △ All Drilled Shaft reinforcing steel shall be <u>subsidiary</u> to the bid item "Drilled Shaft (60")(Cased)"

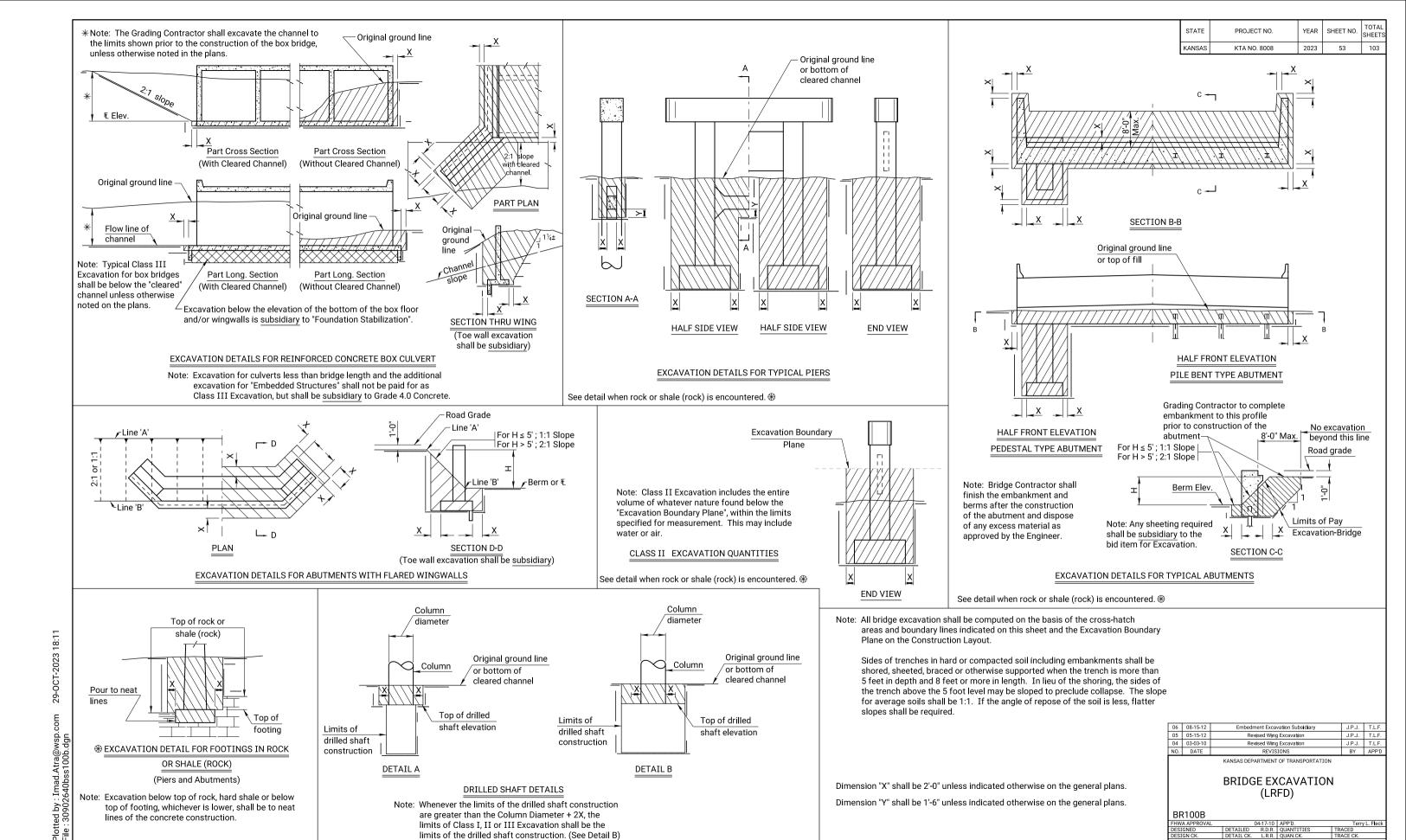


Kansas Turnpike Authority

Br. No. 5.513 S

BAR LIST





PRESTRESSED PILES: Fabricate prestressed concrete pile splices

Method of attachment of pile to build-up may be by any of the

may be by any of the following methods:

4. Provide cored holes for bars as in 3.

piles shall comply with the Standard Specifications.

prestressing strand conforming to ASTM A416, Gr. 270.

and to requirements of the Standard Specifications.

head a minimum of 2'-0".

become a part of the bridge pile system.

the approval of the Engineer.

of strands

Specifications.

Standard Specifications.

in accordance with the Manufacturer's recommendations subject to

methods given in the notes on "Alternate Methods." If mild reinforcing

ALTERNATE METHODS: Method of attachment of a pile to build-up

1. Cut off at least 2'-0" of pile and expose a minimum of 2'-0"

2. Cast 8-#6, or 8-#5 bars (equally spaced) into pile head. All

3. Drill 8 holes in pile head (equally spaced) for installation of

bars shall extend into pile head and project from pile

8 grouted dowel bars of same size and length as in 2.

No bars or strands are to extend from head of pile or build-up

TEST PILES: Drive test piles where called for on the bridge plans.

DRIVING FORMULA: Driving formula shall conform to the Standard

MEASUREMENT AND PAYMENT: Measurement and payment for all

REINFORCEMENT: Use reinforcing steel conforming to ASTM A615,

Grade 60. Hoops and spirals may be either plain or deformed bars.

PRESTRESSING STEEL: Use uncoated seven-wire low relaxation

STEEL PILE: Steel pile shall conform to the requirements of the

PILE POINTS: Pile points shall conform to the dimensions shown

into footing or pile cap unless approved by the Engineer.

The test piles located within the limits of the substructure will

steel is used for attachment, the area shall be no less than that used

Construction as currently used by the Kansas Department of Transportation CONCRETE: Concrete for cast-in-place shall be f'c = 3,500 PSI.

SPECIFICATIONS: Standard Specifications for State Road and Bridge

WELDING: All field welding shall meet the requirements of the Standard Specifications.

Concrete for prestressed shall be f'c = 5,000 PSI.

Use only Shielded Metal Arch Welding SMAW (stick welding) for pile splices.

Use only low hydrogen E7018, 7016, or 7015 series welding rod (electrode) for all welding applications during pile splicing.

New electrodes are to be purchased for each KDOT project. The electrodes shall arrive on the project in factory hermetically sealed containers, opened and labeled with indelible ink in front of the engineer. The label shall include the current date and the project number. If the container seal is questionable or shows signs of damage the electrode is to be dried in an oven at least one hour at a temperature of 700°F to 800°F.

Upon removal from intact hermetically sealed factory packaging or the drying oven the electrode is to be placed in a storage oven with a minimum temperature of 250°F.

When electrodes are removed from the hermetically sealed container or storage oven and exposed to the atmosphere for less than 4 hours place into the storage oven for at least 4 hours before removing for use.

If electrode is exposed to the atmosphere for 4 hours or more (or 9 hours for moisture resistant electrodes designated with an R in their labeling) then electrode can be dried in a drying oven at a temperature of 450°F to 550°F.

If the electrode is exposed to the atmosphere for 4 hours or more a second time or the rod becomes wet discard rod.

CAST-IN-PLACE SHELLS: Steel shells for cast-in-place concrete piles shall conform to the requirements of the Standard Specifications.

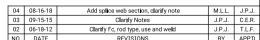
All piles driven without a mandrel shall be of the minimum thicknesses shown. Piles driven with a mandrel shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion and/or buckling due to soil pressure after the mandrel is removed.

Remove, replace or correct to the satisfaction of the Engineer improperly driven, broken or otherwise defective pipe piles. Otherwise drive an additional pile at no extra cost.

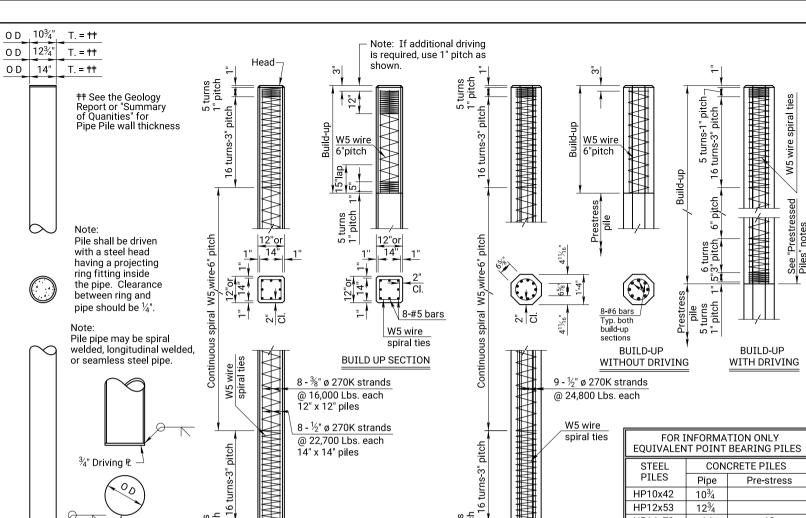
The Contractor shall maintain a light suitable for visual inspection of the pile on the job at all times prior to and during the filling of the pipe.

PAINT: All paint shall comply with the Standard Specifications, or as specified on the plans.

MILL TEST REPORTS: Steel piles test reports and steel shell test reports shall comply with the Standard Specifications.



# STANDARD PILE DETAILS



HP14x73 14 12 HP14x102 14 HP14x117 16

SPLICES: Splices for steel piles and shell piling shall be in accordance with details shown on this sheet and the Standard Specifications.

For integral pile bent abutments and piers, if a pile splice is required, do not locate the pile splice within a region extending 2'-0" above and 10'-0" below the bottom of the concrete web wall. For abutments, locate the pile splice at least 10'-0" below top of fill.

With the approval of the Engineer, one splice per bent may be allowed in the region described above without testing. If additional splices are anticipated, based on the geology, the Contractor prior to driving, will locate the splice so that the splice will not fall within the regions described above.

† For integral pile bent abutments and piers, if a splice is located within the regions described above, then the Contractor will test the welds by Radiograph (RT) test methods. Repair and retest any welds not passing the test(s). Each weld tested will have written confirmation of results. Report these results to the Engineer. This work is not paid for directly, but is subsidiary to "Piles".

> \* Minimum as required by welding process.

Section thru Flange

Cope regions

PILE SPLICE DETAILS

H-Pile Section

Pipe Section

Section A-A

(Thru web)

BG = Backgouge

Weld Symbology Definition

Use grinder to bevel edges of splice as shown in weld symbology and drawing. In addition to bevels, produce clean, bare, and shiny surfaces at and around the splice welding

16" PRESTRESSED

**CONCRETE PILES** 

Lay full penetration root weld from beveled side of splice.

Back gouge root weld from side opposite of root welding application making sure to remove all foreign materials, porous steel, and inclusions from root weld. Finish welding the non beveled side of the splice.

Finish welding beveled side of the splice while removing slag, foreign materials, porous steel, and inclusions in between welding passes, use of a grinder may be needed.

Verify that enough filler metal has been correctly placed in all weld locations to obtain a flush or convex surface with no concavity produced upon completion of the final welds.

Pick-up points 0.21 L 0.21 L 0.58 L DOUBLE POINT PICK-UP PICK-UP POINTS FOR PRESTRESSED PILING

SINGLE POINT PICK-UP

0.3 L

PLAIN ROUND

Length (L)

Pick-up point

18:11

29-0CT-2023

CAST-IN-PLACE CONCRETE PILES

Max. length - 55' single point pick-up Max. length - 80' double point pick-up

Note: Piles shall be marked at Pick-up points to indicate proper points for attaching handling lines.



Tin -

12" OR 14"

**PRESTRESSED** 

CONCRETE PILES

Outside Flange

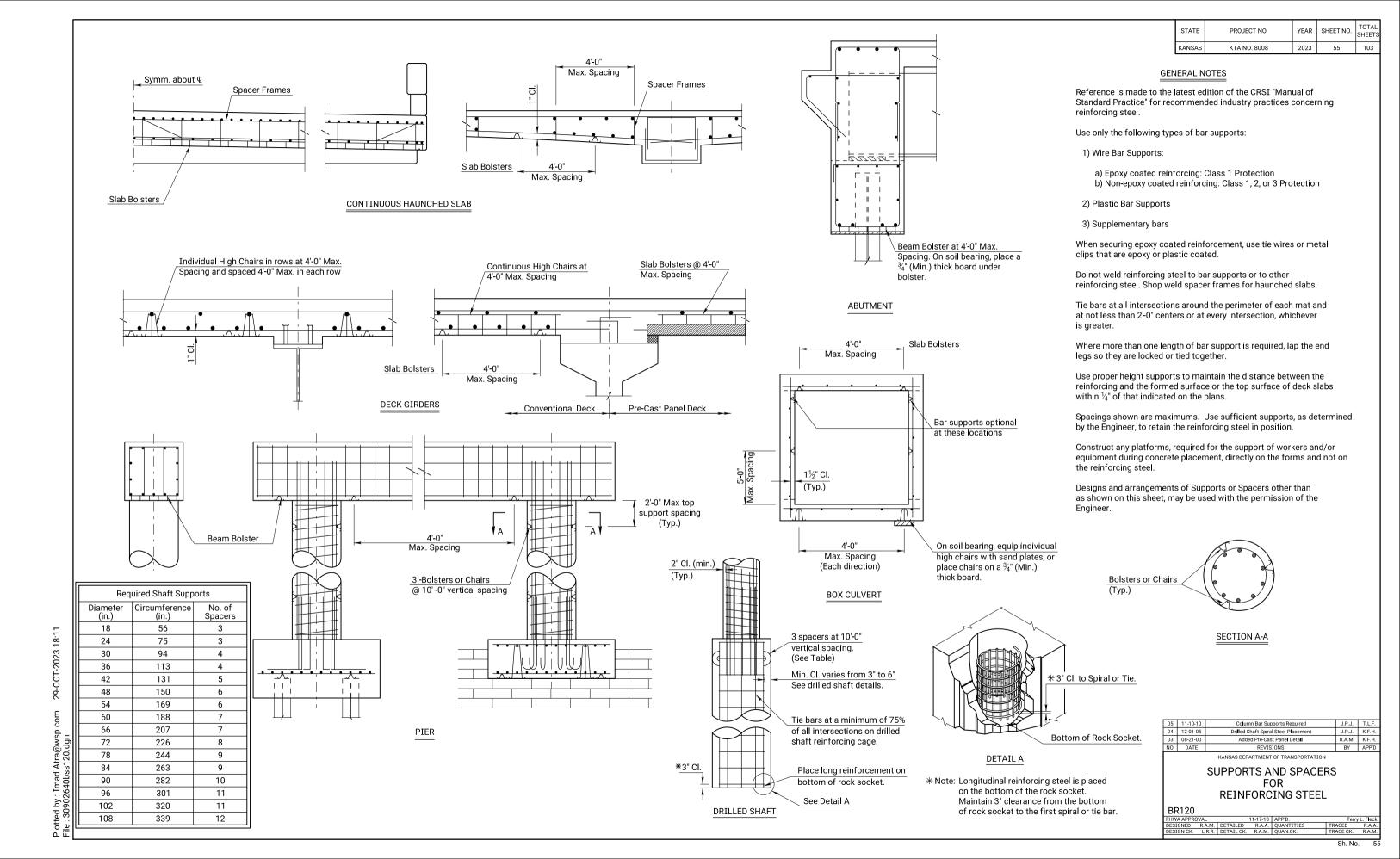
Inside Flange

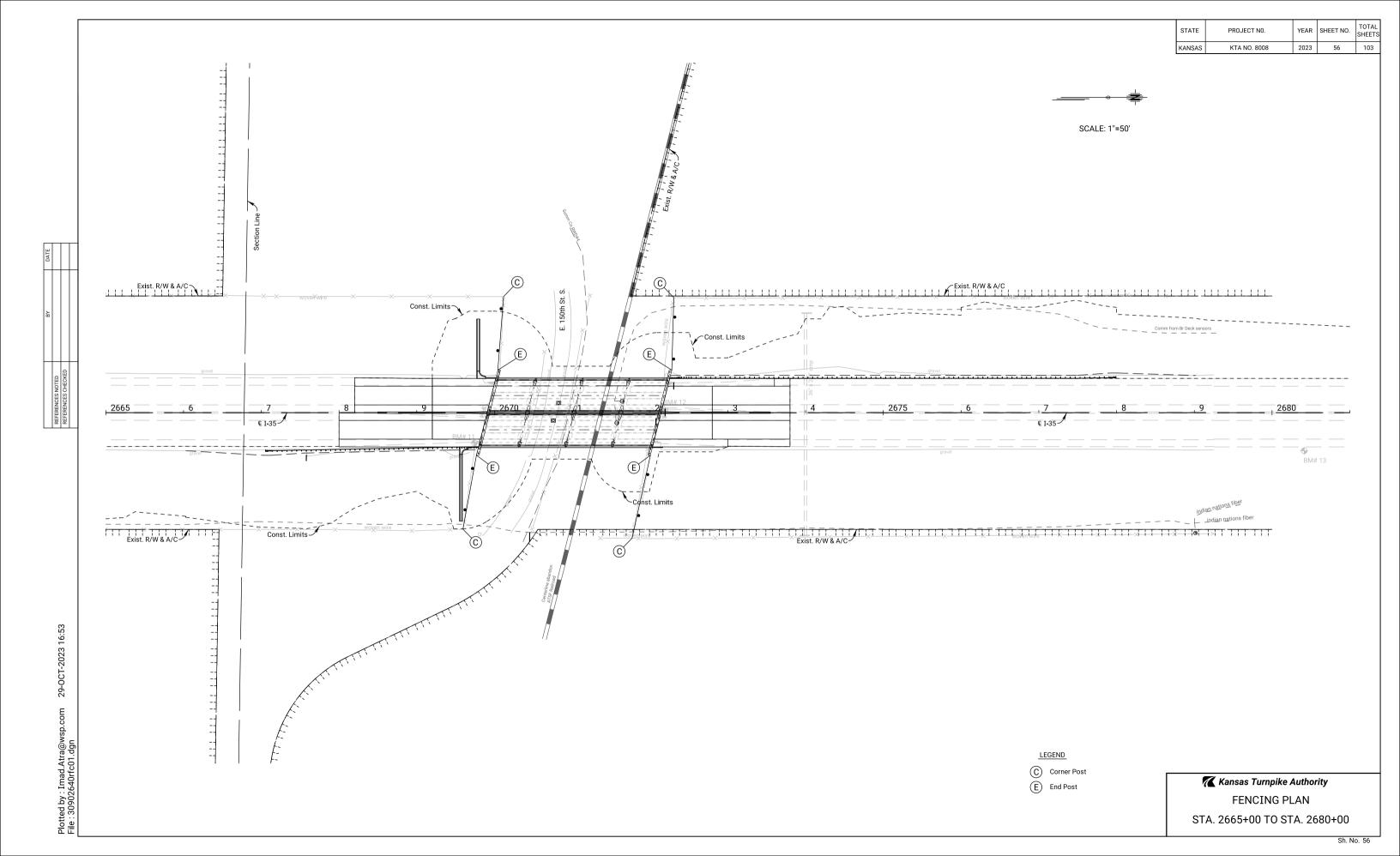
SHELL PILE POINT

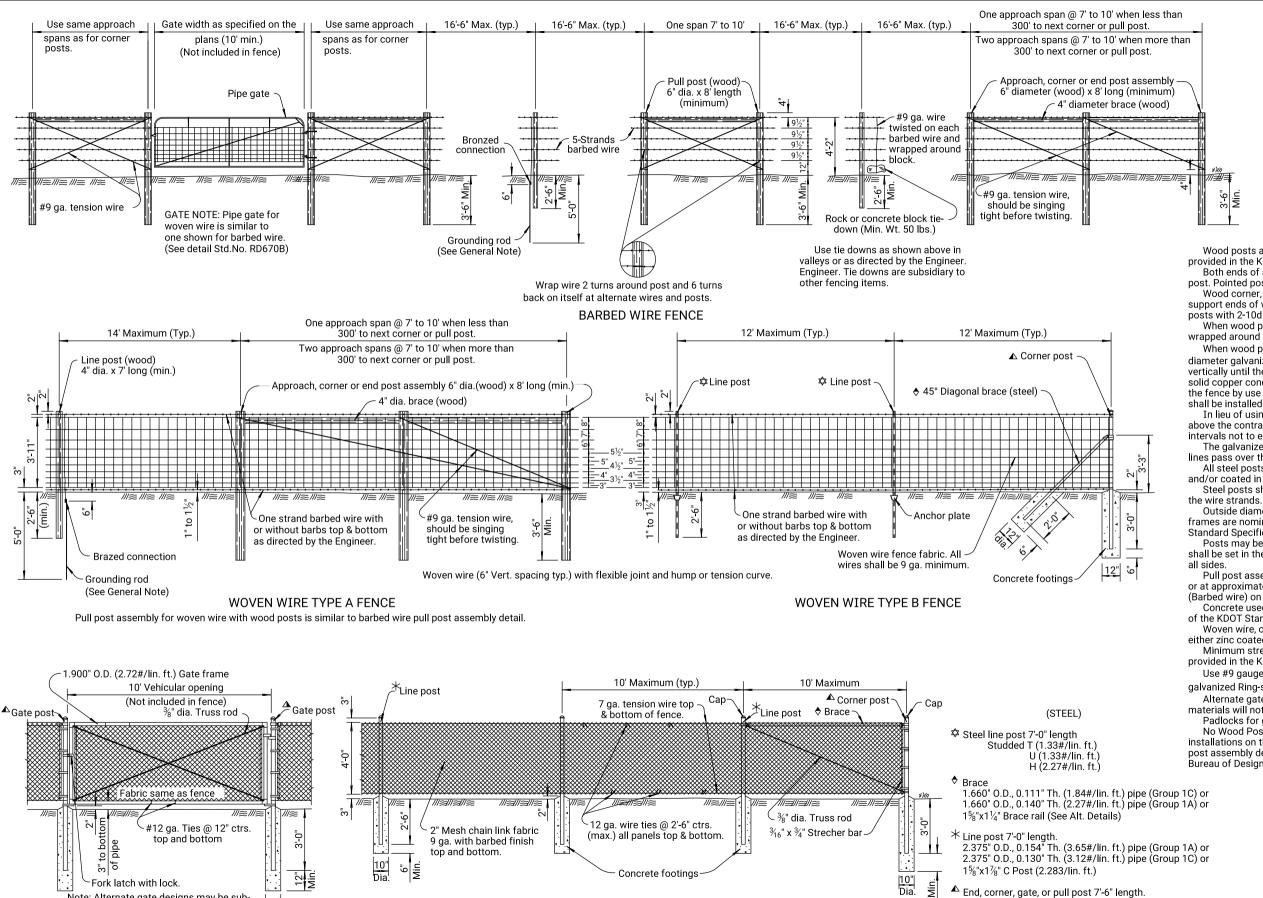
# CAST STEEL PILE POINT

The pile point shall be a one-piece unit of cast steel. Weld pile points in accordance with manufacturer's recommendations to each steel pile before driving.

KANSAS DEPARTMENT OF TRANSPORTATION







CHAIN LINK FENCE

Imad.Atra@wsp. 640RD670A.dgn

Note: Alternate gate designs may be sub-

mitted for approval. Lighter weight

materials will not be approved. **DETAIL of GATE, HINGE & SPECIFICATIONS**  **GENERAL NOTE** 

PROJECT NO.

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Wood posts and braces shall be given a preservative treatment as provided in the KDOT Standard Specifications.

Both ends of all wood posts shall be cut normal to the axis of the post. Pointed posts will not be permitted.

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KANSAS

Wood corner, end, pull and approach posts shall be notched to support ends of wood braces. Wood braces shall be toenailed to the posts with 2-10d nails in each end of the brace.

When wood posts are used, both ends of all tension wires shall be wrapped around the posts twice and stapled in place.

When wood posts are used the fence shall be grounded by a \%" diameter galvanized or copper coated rod five feet long, driven vertically until the top is six inches below the ground surface. A #6 solid copper conductor shall be securely fastened to each element of the fence by use of clamps or other suitable device. Grounding rod shall be installed at intervals of 175' maximum.

In lieu of using the galvanized or copper coated rod as described above the contractor may, at his option, use a steel line post at intervals not to exceed each eighth post.

The galvanized or copper coated rod shall be used where power lines pass over the fence.

All steel posts, braces, fittings, and gate frames shall be galvanized and/or coated in accordance with the Standard Specifications.

Steel posts shall be provided with fasteners prevent slippage of

Outside diameters shown for tubular steel posts, bracing and gate frames are nominal. Weight tolerances shall be as shown in the KDOT Standard Specifications.

Posts may be set by driving or digging. If by digging, the posts shall be set in the center of the hole and the soil tamped securely on

Pull post assembly shall be used at sharp breaks in vertical grade or at approximately 330' centers (Woven & Chain link) or 1320' centers (Barbed wire) on straight runs or as directed by the Engineer.

Concrete used in fence installation shall conform to the requirements of the KDOT Standard Specifications.

Woven wire, chain link fabric, barbed wire and tension wire shall be either zinc coated (galvanized) or aluminum coated.

Minimum strength of barbed wire and tension wire shall be as provided in the KDOT Standard Specifications

Use #9 gauge galvanized staples  $1\frac{1}{2}$ " to  $1\frac{3}{4}$ " long, or #9 guage galvanized Ring-shank staples  $1\frac{1}{2}$ " to  $1\frac{3}{4}$ " long.

Alternate gate designs may be submitted for approval. Lighter weight materials will not be approved.

Padlocks for gates shall be furnished by the State

(A120) 2.875" O.D., 0.203" Th. (5.79#/lin. ft.) pipe (Group 1A)

or 2.875" O.D., 0.160" Th. (4.64#/lin.ft.) pipe (Group 1C)

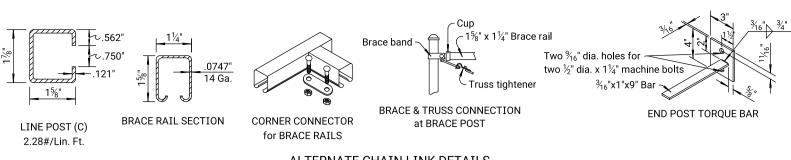
No Wood Posts are allowed for new or reconstruction fence installations on the State Highway System. Shop drawings for steel gate post assembly designs are to be submitted to the State Road Office, Bureau of Design for approval prior to construction

09	11-30-09	Rev. post listing,wood po. restrict.	S.W.K.	J.O.B.
80	11-08-05	Revised brace dimension	S.W.K.	J.O.B.
07	11-02-04	Added assembly to end post label	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

# HIGHWAY FENCE BARBED, WOVEN, & CHAIN LINK

RD670A			
FHWA APPROVAL	12-16-09	APP'D.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.
			_





STATE

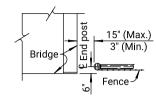
KANSAS

A line post shall be used in the KDOT fence at each private cross fence, and the contractor shall make a temporary connection. This work shall be <u>subsidiary</u> to other bid items.

In general, where needed, use small channel crossing as shown, Type I and Type II Floodgates will be used very seldom.

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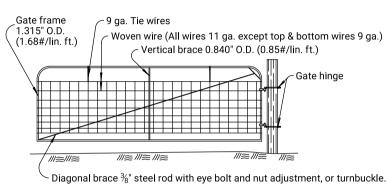
# **FENCE DETAILS** AT BRIDGE ABUTMENTS

(Use appropriate post and brace for fence type, dimensions are common for all fence types.)

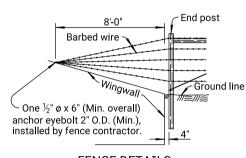
· 45° Diagonal brace (steel) 1.660" O.D. 0.083" Th. @ 1.40#/lin.ft. pipe (Group 1C)

Note: See Standard Drawing RD670A for steel post requirements.

# ALTERNATE CHAIN LINK DETAILS



DETAIL of GATE, HINGE & SPECIFICATIONS (for Barbed & Woven Fence)



**FENCE DETAILS** AT DRAINAGE STRUCTURES (Type A, B, or Barbed wire fence.)

# BARBED WIRE FENCE STEEL POST (ALTERNATE)

Intermediate span

(14'-0" Maximum

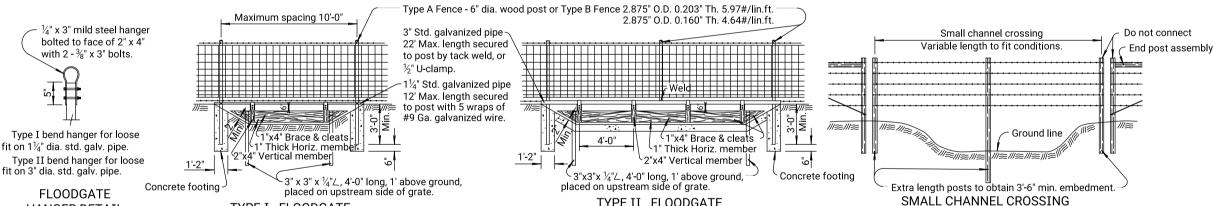
Steel posts may be used in lieu of wood posts as shown above.

Anchor plate

▲ End, corner or pull post (steel)

End or corner spans

(10'-0" Maximum)

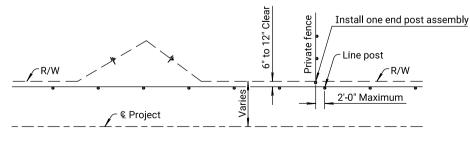


FLOODGATE HANGER DETAIL

TYPE I FLOODGATE

TYPE II FLOODGATE (Grouted stone or concrete lined ditch.

The above sketch is typical only and can be varied to fit existing conditions. Small channel crossings shall be included in lin. ft. of fence. All extra materials and labor within the small channel crossing shall be <u>subsidiary</u> to lin. ft. of fence.

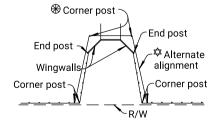


# TYPICAL INSTALLATION DIAGRAM

Note: Right of Way fence shall generally be set parallel to and 6" to 12" clear from the Right of Way line.

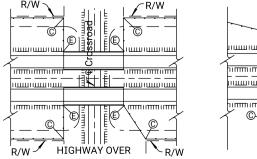
The alignment layouts as shown are typical, but are not representative of all situations that may occur. Construction may be varied, as required to meet field conditions and/or as directed by the Engineer.

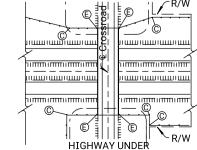
The access control fence shall be attached to the private fence end post assembly using leader wires or staples.



# FENCE ALIGNMENT AT UNDERPASS OR **BOX DRAINAGE STRUCTURE**

Alternate alignment may be used at deep underfill culverts, as directed by the Engineer.





FENCE ALIGNMENT AT BRIDGE ABUTMENTS

♦ Where fence installation over a drainage structure is located within the clear zone, horizontal bracing at the corner posts will not be permitted. An alternate design utilizing diagonal bracing shall be provided.

07	07-28-09	Revised Steel size listing	S.W.K.	J.O.B.				
06	11-02-04	Revised General Note	S.W.K.	J.O.B.				
05	05-30-02	Removed KDOT ownership sign.	S.W.K.	J.O.B.				
NO.	DATE	REVISIONS	BY	APP'D				
	KANSAS DEPARTMENT OF TRANSPORTATION							

# **INSTALLATION DETAILS** BARBED, WOVEN, & CHAIN LINK

RD670B			
FHWA APPROVAL	12-16-09	APP'D.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.

EARTHWORK										
STATION to STATION				EXCAV		COMP.	THRU NOT SUB			
		сом	MON	* ROCK		CONTRACTOR FURNISHED		TYPE B MR-90	COMMON EXC.	TYPE AA MR-0-5
		CU. YDS.	VMF	CU. YDS.	VMF	CU. YDS.	VMF	CU. YDS.	CU. YDS.	CU. YDS.
I-35 Phase 1 (Nor	thbound Lanes)									
2664+03.02	2673+80	446	0.87	201	1.00	3,444	0.87	3,384	274	274
I-35 Phase 2 (Sou	ithbound Lanes)									
2668+20	2681+20	890	0.87	190	1.00	3,361	0.87	3,698	252	252
I-35 South Crosso	over									
2641+60	2649+00								342	342
I-35 North Crosso	over									
2689+00	2696+40								342	342
TOTALS		1,336		391		6,805		7,082	1,210	1,210

\* All rock excavation is existing pavement removal assumed 10" thick and is to be wasted.

*	* REMOVAL OF EXISTING STRUCTURES (FOR INFORMATION ONLY)					
STATION	to STATION	SIDE	REMARKS			
2641+60.00	2649+00.00	Œ.	740.00' Existing Median Barrier			
2667+59.22	2669+73.12	Rt.	214.69' Existing Guardrail			
2668+50.10	2669+82.18	Œ.	Existing Median Barrier			
2669+29.87		Rt.	Concrete Flume			
2669+60.61		Lt.	Concrete Flume			
2669+59.39	2669+74.95	Rt.	111.40' Fence			
2670+06.16	2670+11.61	Lt.	102.07' Fence			
2670+38.39	2670+64.46	Œ.	127.55' Fence			
2671+76.91	2671+96.83	Rt.	118.36' Fence			
2672+18.07	2672+30.55	Lt.	109.01' Fence			
2672+22.91 2673+54.63		Ę.	Existing Median Barrier			
2672+31.77	2674+45.85	Lt.	214.79' Existing Guardrail			
2689+00.00	2689+00.00 2696+40.00 © 740.00' Existing Median Barrier					
	•					

\* The listing shown may not be complete.

FLUME INLET & SLOPE DRAIN					
STATION SIDE FLUME INLET			SLOPE DRAIN (STONE) LIN. FT.		
2669+57.00	Rt.	1	86.0		
2669+79.00	Lt.	1	67.0		
TOTA	LS	2	153.0		

	GUARDRAIL					
			STEEL PL	ATE (MGS)		
STATION to STATION		SIDE LIN. FT	LIN. FT.	мgs-мsкт	REMARKS	
Br. No. 5.5.133 NB						
2667+08.02 2669+79.85 Rt.		225.00	1	Parallel		
Br. No. 5.5.133 SB						
2672+24.89 2667+96.75 Lt.		525.00	1	Zero Flare		
	TOTALS			2		

PERMANENT PRECAST CONCRETE MEDIAN BARRIER (TYPE 1F)					
STATION to STATION S:			NUMBER OF UNITS	LIN. FT.	REMARKS
2641+60.00	2649+00.00	Œ.	37	740.0	
2668+50.10	2669+90.10	Œ	7	140.0	
2672+14.63 2673+54.63 © 7				140.0	
2689+00.00 2696+40.00 © 37			37	740.0	
TOTAL				1760.0	

Cast New Barrier using details on Sh. No's. 23 - 26. Field Verify and adjust station ranges to match existing barrier joints.

IMPACT ATTENUATOR ABSORB 350 (TL-3) (TEMPORARY)					
STATION	SIDE	EACH	REMARKS		
2641+60.00	Ę.	1	Phase 1		
2641+60.00	Œ.	'	Phase 2		
2649+00.00	Ę.	1	Phase 1		
2649+00.00	Œ.	'	Phase 2		
2667+79.64	Lt.	1	Phase 1		
2672+03.19	Rt.	'	Phase 1		
2668+72.80	Lt.	1	Phase 2		
2674+15.69	Rt.	'	Phase 2		
2689+00.00	Œ.	1	Phase 1		
2689+00.00	Ē.	'	Phase 2		
2696+40.00	Ę.	1	Phase 1		
2696+40.00 €			Phase 2		
TOTA	L	6			

RUMBLE STRIPS (MILLED) (ASPHALT)					
			STA	ΓΙΟΝ	
STATION t	o STATION	NORTH	BOUND	SOUTHBOUND	
İ		INSIDE	OUTSIDE	INSIDE	OUTSIDE
2641+60.00	2649+00.00	7.4 7.4			
2668+00.00	2669+90.48	1.9	1.8	1.7	1.8
2672+14.26 2673+80.00		1.7	1.8	1.6	1.6
2689+00.00 2696+40.00		7.4		7.4	
TOTALS		18.4	3.6	18.1	3.4

FENCE					
			W	OVEN WIRE FEN	ICE
STATION to STATION		SIDE	LIN. FT.	CORNER POST EACH	END POST EACH
2669+59.39	2669+76.72	Rt.	96.6	1	1
2670+04.23	2670+11.61	Lt.	94.8	1	1
2671+76.91	2672+00.50	Rt.	109.2	1	1
2672+28.01 2672+30.55		Lt.	93.3	1	1
		393.9	4	4	

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KANSAS	KTA NO. 8008	2023	59	103

RECAPI	TULATION C	F BRIDGE QUANTITIES
BRIDGE NUMBER	STATION	SEE SHEET NO.
5.5.133 NB	2671+02	27
5.5.133 SB	2671+02	27

TIEM
Contractor Construction Staking
Mobilization   1

For Surfacing Quantities, See Sheet No. 60
For Erosion Control Quantities, See Sheet No. 63
For Seeding Quantities, See Sheet No. 68
For Pavement Marking Quantities, See Sheet No. 70
For Traffic Control Quantities, See Sheet No. 83
For Temp. Conc. Safety Barrier Quantities, See Sheet No. 84

02	01-14-08	Rem. Drainage Structure summary	S.W.K.	J.O.B.
01	01-09-91	Detailed on CADD	R.J.S.	J.O.B.
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

# SUMMARY OF QUANTITIES

RD050

FHWA APPROVAL	05-28-08	APP'D.	James 0	. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	S.W.K.

#### GENERAL NOT

On surfacing projects, the 6" of Compaction Type AA, shown for the center portion on the roadbed, is for the purpose of restoring the original Compaction Type AA which may have been lost since grading operations. The exact locations of this Compaction Type AA, which will be required, is to be determined by the Engineer at the time of construction. This work shall be paid under the bid item "Compaction of Earthwork (Type AA)(MR- )".

Over all structures, unless otherwise directed by the Engineer, where the top of the hubguard is level with or above the finished shoulder grade, the earth cover over the structure slab shall be removed and backfilled with \_\_\_\_\_\_ materia as directed by the Engineer. The removal of this material will be subsidiary.

The \_\_\_\_ material used to backfill over the structure shall be paid for at the prices shown in the contract.

The earth shoulders shall be compacted full depth (Type -MR ) except, when ordered by the Engineer, the top 3" shall be left uncompacted for seeding.

All side roads and house entrances shall be surfaced with \_\_\_\_\_\_ to the R/W line as indicated on the detail. All side roads and house entrances with existing asphalt surface shall be surfaced with \_\_\_\_\_\_ at least to the R/W line or to the end of construction, as directed by the Engineer. Each mailbox turnout (ON PROJECTS WHERE STABILIZED SHOULDERS ARE NOT SPECIFIED) shall be surfaced with \_\_\_\_\_\_ to the limits shown on the detail.

Surfacing material (SA-\_\_\_\_\_) shall be used for surfacing house entrances and side roads (\_\_\_\_\_C.Y./SQ. YD.) beyond the limits of the asphalt surface to the limits of construction as determined by the Engineer.

The thickness of side road and entrance surfacing may be increased to the same thickness as the stabilized shoulder within the approximate limits of the shoulder.

On projects which specify both asphalt base and surface course materials, side roads, house entrances and mailbox turnouts may be surfaced with both materials at the contractors option, with the approval of the Engineer.

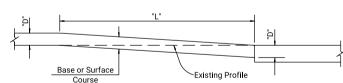
Quantities for aggregate for shoulders, AS-1, are calculated on the basis of 150 lbs. per cu. ft. Quantities for stabilized base course, AB-3, are calculated on the basis of 1 56 lbs. per cu. ft. Weight/cu. ft. includes moisture allowed by specification.

The base course shall be constructed to the plan thickness as shown.

Thicknesses indicated for all construction which is paid for on a weight or volume basis are approximate and may vary to correct for unevenness in the foundations or for other normal unevenness encountered in placement operations.

A tack coat of SS-1HP shall be provided between each lift of all base courses and surface courses and under the first lift of base or surface courses when they are placed on an existing asphalt, brick, or concrete surface, when so ordered by the Engineer and at the rate designated by him. Quantities are included for these tacks calculated at the rate of 0.06 gal. /sq. yd.

Asphalt Material quantities are calculated on the basis of 8.328 lbs. per gal. Shoulder rumble strips will not be constructed as part of this project.



TYPICAL PROFILE AT GRADE CONTROL POINTS

The Contractor shall cut the subgrade in accordance with this profile at all grade control points, i.e.; existing pavements, grade bridges and R.R. crossings, also at changes in thickness of base or surface courses. Corresponding dimensions of "D" and "L" shall be as given in the table below. The work of cutting the subgrade and disposing of excess excavated material shall be subsidiary to other items in the contract.

	TABLE OF DIMENSIONS										
				ADLL	01 01	IVILIAO	10140				
D	L	D	L	D	L	D	L	D	L	D	L
1"	25'	3"	75'	5"	125'	7"	175'	9"	225'	11"	275'
2"	50'	4"	100'	6"	150'	8"	200'	10"	250'	12"	300'

SUMMARY OF QUANTITIES								
ITEM	COMM. GR. (CLASS A) (2")	COMM. GR. (CLASS A) (2")(70-28)	COMM. GR. (CLASS A) (8")		SURF. MAT. (AB-3) (4")	AGG. BASE (AB-3) (8")	AGG. BASE (AB-3) (10")	MILLING (2")
	TONS	TONS	TONS	TONS	TONS	SQ. YD.	SQ. YD.	SQ. YD.
NB Lanes								
Sta. 2668+00 - 2669+20	14.20	* 38.10						333.5
Sta. 2669+20 - 2669+90.48		27.20	108.80	113.75		490.7		
Sta. 2672+14.26 - 2672+80	20.25	28.80	115.10	60.20		392.3		
Sta. 2672+80 - 2673+80		* 31.45						275.1
SB Lanes								
Sta. 2668+20 - 2669+20	23.35	* 31.90						279.2
Sta. 2669+20 - 2669+90.48		30.75	122.85	50.75		389.8		
Sta. 2672+14.26 - 2672+80	14.25	25.30	101.05	84.20		413.1		
Sta. 2672+80 - 2673+80		* 32.10						280.8
Crossovers A & B								
Sta. 2641+60 - 2649+00	247.10			668.65			1,229.6	2,020.3
Crossovers C & D								
Sta. 2689+00 - 2696+40	247.10			668.65			1,229.6	2,020.3
GR Surfacing NB Lanes								
Sta. 2666+03.03 - 2669+79.41					* 72.90			
GR Surfacing SB Lanes								
Sta. 2672+25.32 - 2679+01.75					* 79.35			
TOTALS	566.25	245.60	447.80	1,646.20	152.25	1,685.9	2,459.2	5,209.2

<sup>\*</sup> Quantity has been increased 5% for Contingency.

Surfaced Roadbed Shoulder Line M.B. Turnout ♦ Width shall be 8' or shoulder width, whichever is greater. SECTION A-A Note: The face of Mail Roy should be no closer to the roadway than the edge of the shoulder. Align with edge of turnout when turnout width is greater than shoulder width. 40' Shoulder Line Mail Box ← Direction of Traffic Edge of Surfacing- ℙroject

✓ A --DETAIL FOR SURFACING OF MAIL BOX TURNOUTS for Side Roads 24' for Entrances "W" R/W Thickness as shown in General Note Variable slope approx. Variable slope approx 50' or as available Typical drainage structure -Rad. Pt. 32.69' E.P Ditch Shoulder Line Edge of Surface Edge of Surface

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YEAR SHEET NO.

60

2023

WITH DRAINAGE STRUCTURE

UNIT

Tons

Tons

Sq. Yds. Sq. Yds. M. Gal.

TOTAL

2.660

2.460

RECAPITULATION OF QUANTITIES

ITEM

HMA Commercial Grade (Class A

Surfacing Material (AB-3)

Aggregate Base (AB-3)(8")
Aggregate Base (AB-3)(10")
Water (Aggregate Base)(Set Price)

HMA Commercial Grade (Class A)(70-2

MOUND ENTRANCE OR SIDE ROAD

DETAIL FOR SURFACING OF SIDE ROADS & HOUSE ENTRANCES

▲ 8:1 Slope at the appropriate clear zone shall apply to all mound entrances and mound side roads to 10' fill height. Normal Slope (but not steeper than 6:1) for over 10' fill height.

 Normal Slope (but not steeper than 6:1) at approximate 
 Structure or appropriate clear zone width.

♣ On side roads and entrances which slope toward the highway, a low point approx. 6" deep shall be constructed to divert surface drainage into the highway ditch, unless otherwise shown on the plans.

12	01-10-07	Changed bituminous to asphalt	S.W.K.	J.O.B.			
11	08-30-06	Changed tack type/rate	S.W.K.	J.O.B.			
10	03-24-05	Revised compaction, tack type/rate	S.W.K.	J.O.B.			
NO.	DATE	REVISIONS	BY	APP'D			

KANSAS DEPARTMENT OF TRANSPORTATION

# SUMMARY OF QUANTITIES (Surfacing)

 RD051

 FHWA APPROVAL
 09-06-06 | APPD.
 James O. Bre

 DESIGNED
 DETAILED
 QUANTITIES
 TRACED

 DESIGNED
 DETAILED
 QUANTITIES
 TRACED

lb/cu ft	Aggregate Base (AB-3)		
	lb/cu ft	lb/cu ft Aggregate Base (AB-3)	Ib/cu ft Aggregate Base (AB-3)

RATES OF APPLICATION

ITEM

HMA Commercial Grade (Class A

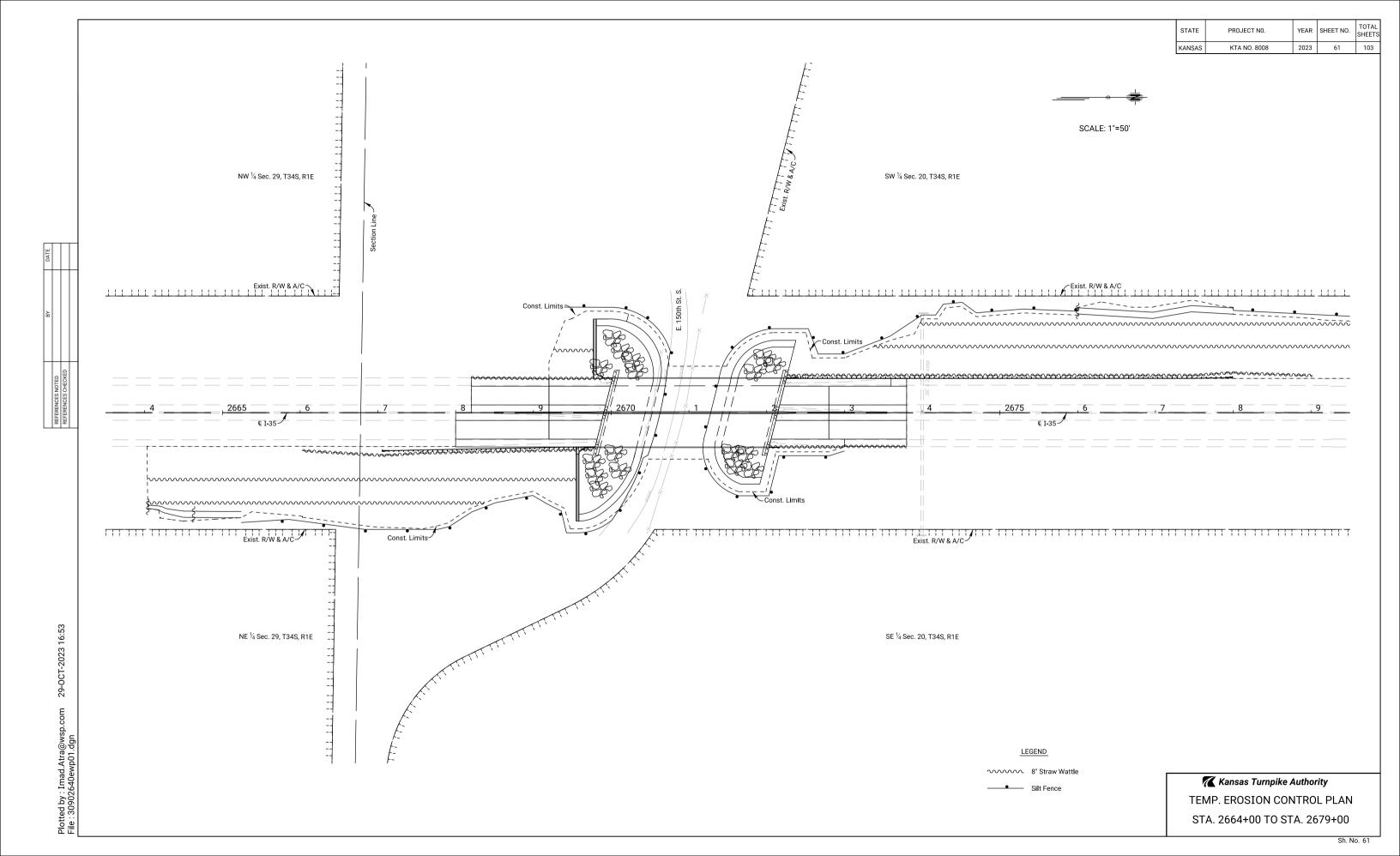
†	Computed at the rate of
††	Computed at the rate of

RATE UNIT

145 lb/cu ft

CE CK.

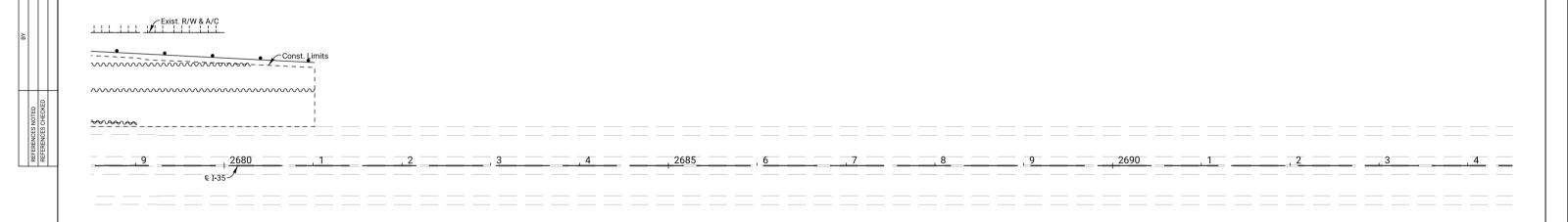
Plotted by : Imad.Atra@wsp.com 29-0CT-20:



STATE PROJECT NO. YEAR SHEET NO. TOTAL SHEETS
KANSAS KTA NO. 8008 2023 62 103

----

SCALE: 1"=50'



Exist. R/W & A/C

LEGEND

8" Straw Wattle

Silt Fence

Kansas Turnpike Authority
TEMP. EROSION CONTROL PLAN
STA. 2679+00 TO STA. 2694+00

FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P<sub>2</sub>O<sub>5</sub>, K<sub>2</sub>O listed in Summary of Quantities will be acceptable

- \* N = Nitrogen Rate of Application
- \*\* P2O5 = Phosphorous Rate of Application
- \*\*\* K<sub>2</sub>O = Potassium Rate of Application

The Contractor will be required to finish areas of excavation, borrow and embankment in accordance with the specifications. Areas that require installation or construction of temporary water pollution control items will be finished in reasonable close conformity to the alignment, grade and cross section shown on the plans or as established by the Engineer.

Plotted by : Imad.Atra@wsp.com File : 30902640LA852A.dgn

CLT = Construction Limit Tract. This area is defined by the entire disturbed area of the project that requires seeding and erosion control measures to be placed. Any impervious areas (i.e. pavement, gravel, riprap, etc.) shall not be included in this

Slope = Defined by the area of the project that requires Class 1 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

Channel = Defined by the area of the project that requires Class 2 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

#### **GENERAL NOTES**

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded, and mulched. Soil preparation shall conform to the Standard Specifications.

Temporary seeding shall be done during any time of the year that the soil can be cultivated. After the temporary seeding has been completed on the entire project, permanent seeding shall be done during the normal seeding season.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching materials is generally as follows:

 $1\frac{3}{4}$  -  $2\frac{1}{4}$  Tons per Acre =  $1\frac{1}{2}$ " loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	KTA NO. 8008	2023	63	103

Seed, fertilizer and mulch quantities for information only. Items to be paid as Temp Seeding - LSUM

	St			SEEDING / EROSION CONTROL C	QUANTITIES	
P.L.S. RA	TE/ ACRE	AC	RES	BID ITEM	QUANTITY	UNIT
CLT	SL/CH	CLT	SL/CH	BID ITEM	QUANTITI	ONT
150		2.6		Temporary Fertilizer (16-20-0)	390.0	LB
80		2.6		Temporary Fertilizer (15-30-15)	208.0	LB
45		2.6		Temporary Seed (Grain Oats)	117.0	LB
45		2.6		Temporary Seed (Sterile Wheatgrass)	117.0	LB
				Soil Erosion Mix		LB
			Erosion Control (Class I, Type C)		SQ YD	
				Eroslon Control (Class 2, Type E)		SQ YD
				Sediment Removal (Set Price)	1	CU YD
				Synthetic Sediment Barrier		LF
				Temporary Berm (Set Price)	1	LF
				Temporary Ditch Check (Rock)		CU YE
				Temporary Inlet Sediment Barrier		EACH
				Temporary Sediment Basin		CU YD
				Temporary Slope Drain		LF
				Temporary Stream Crossing		EACH
				Biodegradable Log (9")		LF
				Biodegradable Log (12")		LF
				Biodegradable Log (20")		LF
				Straw Wattle (8")	3,842	LF
				Filter Sock (12")		LF
				Filter Sock (18")		LF
				Geotextile (Erosion Control)		SQ YE
				Silt Fence	2,107	LF
				SWPPP Design	- Lump Sum	LS
				SWPPP Inspection		EACH
	† †		1	Water Pollution Control Manager		EACH
900 lbs /	acre	2.6	1	Mulch Tacking Slurry	<del>- 2,340</del>	LB
2 tons / a	cre	2.6	İ	Mulching	7.8	TON
				Water (Erosjon Control) (Set Price)	1	MGAL

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. See Permanent Seeding Summary of Seeding Quantities sheet LA850 for further details.

Geotextile (Erosion Control) shall be removed prior to placement of permanent slope protection.

Regreen and Quick Guard are the approved sterile wheatgrass products.

† If the total disturbed area of the project, not just the seeding area, is 1 acre or more, then these bid items must be included

\*\*\*\* List size of material.

The amount of mulch and mulch tacking slurry in the bid quantities is estimated. (Acres of Seeding X 1.5 X 2 Tons/Acre). The estimated quantity includes mulching associated with both temporary and permanent seeding operations. The total mulch and mulch tacking slurry required shall be determined in the field. The bid item for mulching and mulch tacking slurry shall be paid for according to the Standard Specifications.

Quantities for all erosion control items are estimated to give full flexibility for compliance with the NPDES permit. Final quantities will be determined in the field

SOIL EROSION MIX						
PLS RATE	NAME	QTY (lb)				
	Total (lb)					

The Soil Erosion Mix is to be placed under the Class 1 and/or Class 2 erosion control

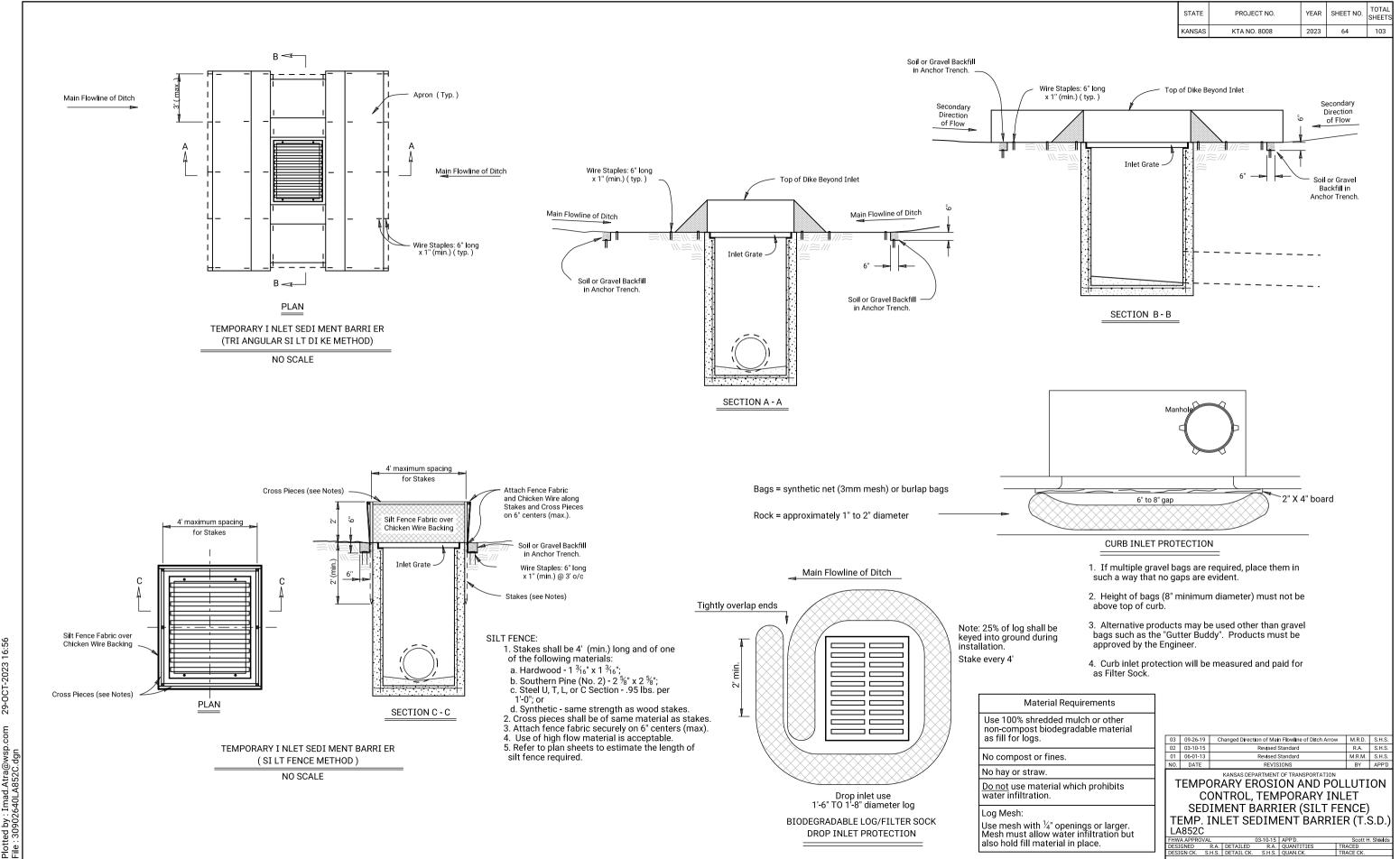
The Soil Erosion Mix consists of the Shoulder Area of the Permanent Seed Mix used on the project.

NO.	DATE	REVISIONS	BY	APP'D		
01	06-01-17	Revised Standard	M.R.D.	S.H.S.		
02	12-01-17	Revised Standard	M.R.D.	S.H.S.		
03	08-03-20	Added Note	M.R.D.	M.L.		

KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY EROSION AND POLLUTION CONTROL

852A	
APPROVAL	01-26-18



18" (min.) diameter Biodegradable Log Section

18" (min.) diameter

Alternative Staking (Optional)

Biodegradable Log Section

Downstream Apron

(Optional)

Downstream Apron

(Optional)

Direction of Flow

Direction of Flow

SECTION A - A

ALT. DETAIL

OPTIONAL

29-0CT-2023 16:56

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#### SILT FENCE BARRIER

NO SCALE

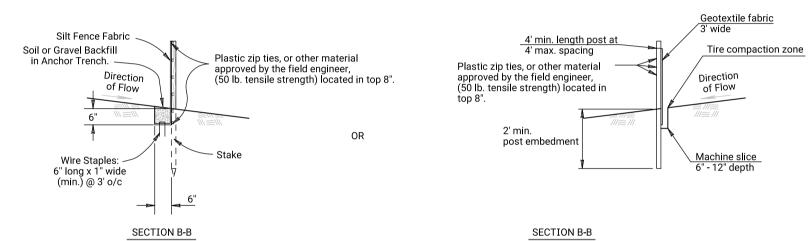
4' (max.)

BIODEGRADABLE LOG SLOPE INTERRUPTIONS

OR Filter Sock

Stakes (typ.)

TYPICAL ELEVATION



# INSTALLATION NOTES

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	KTA NO. 8008	2023	65	103

### SILT FENCE:

- 1. Stakes shall be 4' (min.) long and of one of the following materials:
- a. Hardwood 1 <sup>3</sup>/<sub>16</sub>" x 1 <sup>3</sup>/<sub>16</sub>":
- b. Southern Pine (No. 2)  $2\frac{5}{8}$ " x  $2\frac{5}{8}$ ";
- c. Steel U, T, L, or C Section .95 lbs. per 1'-0"; or
- d. Synthetic same strength as wood stakes.
- 2. Attach fence fabric with 3 zip ties within the top 8" of the fence
  Alternate attachment methods may be approved by the Engineer on aperformance basis.
- 3. Use of high flow material is acceptable.
- 4. Refer to plan sheets to estimate the length of silt fence required.

### BIODEGRADABLE LOG OR FILTER SOCK

- 1. Place biodegradable logs or filter sock tightly together minimum overlap of 18".
- 2. Wood stakes shall be 2" x 2" (nom.).
- 3. Refer to plan sheets to estimate length of biodegradable log and filter sock required.
- 4. Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.
- 5. Length of stakes should be 2 times the height of the log at a minimum with minimum ground embedment equal to the height of the log / sock.

Biodegradable Log or Filter Sock Slope Interruptions

Blodegradable Log of Filter Gook Glope Titterraptions									
	PRODUCT								
		9" Sediment Log or 8" Filter Sock (ft)	12" Sediment Log or 12" Filter Sock (ft)	20" Sediment Log or 18" Filter Sock (ft)					
ınt	≤4H:1V	40	60	80					
Gradient	3H:1V	30	45	60					
Slope G									
S									

	BIODEGRADABLE LOG MATERIAL						
	LOW FLOW	HIGH FLOW					
9"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber					
12"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber					
18"-20"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber					

Deviations should be approved by the Field Engineer.

### GENERAL NOTES

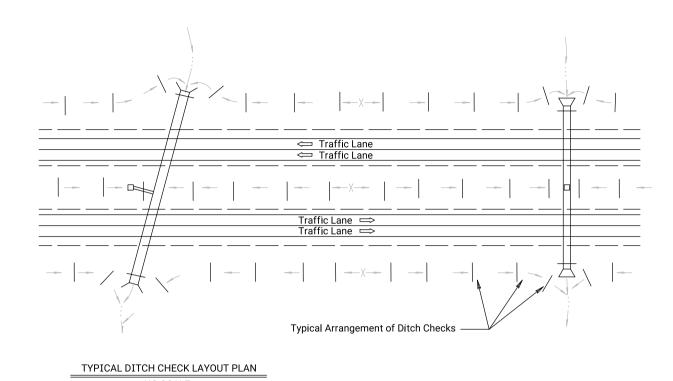
- 1) Slope interruptions shall be placed along contour lines, with a short section turned upgrade at each end of the barrier.
- 2) The maximum length of the slope interruptions shall not exceed 250 feet, and the barrier ends need to be staggered.
- Interruptions damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired immediately by Contractor at no additional cost to KDOT.
- Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

03	06-28-16	Revised Standard	R.A.	S.H.S.		
02	03-01-15	Revised Standard	R.A.	S.H.S.		
01	06-01-13	Revised Standard	M.R.M.	S.H.S.		
NO.	DATE	REVISIONS	BY	APP'D		

TEMPORARY EROSION AND
POLLUTION CONTROL
SLOPE INTERRUPTIONS
BIODEGRADABLE LOG / SILT FENCE

LA8520		5117107	,DLL	200,01	- '	LITOL	
FHWA APPRO	VAL		09-14-16	APP'D.		Scott H.	Shi
DESIGNED	S.H.S.	DETAILED	R.A.	QUANTITIES		TRACED	
DESIGN CK.	S.H.S.	DETAIL CK.		QUAN.CK.		TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	KTA NO. 8008	2023	66	103



	IOLOG SPACING
DITCH & SLOPE (%)	SPACING INTERVAL (FEET)
1.0	125
2.0	60
3.0	40
4.0	30
5.0	25
NOTE: Use this space except Rock Ditch Ch	

18" FILTER SOCK				
CHECK S	SPACING			
DITCH Q SLOPE (%)	SPACING INTERVAL (FEET)			
1.0	110			
2.0	55			
3.0	35			
4.0	25			
5.0	20			
NOTE: Use this spac				

NO SCALE	

# GENERAL NOTES

- The choice of ditch check methods is at the option of the Contractor.
- Use only rock checks in situations where the ditch slope is 6 percent or greater.
- Ditch checks damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired by Contractor at no extra cost to KDOT.

Plotted by : Imad.Atra@wsp.com File : 30902640LA852E.dgn

03	08-10-16	Revised Standard	R.A.A.	S.H.S.
02	06-28-16	Revised Standard	R.A.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APP'D
		MANAGE DED ADTRACTIC OF TO AN ODODT ATTOM		

KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY EROSION AND POLLUTION CONTROL DITCH CHECKS

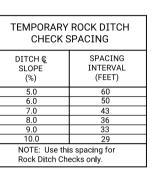
.A852E	
WA APPROVAL	

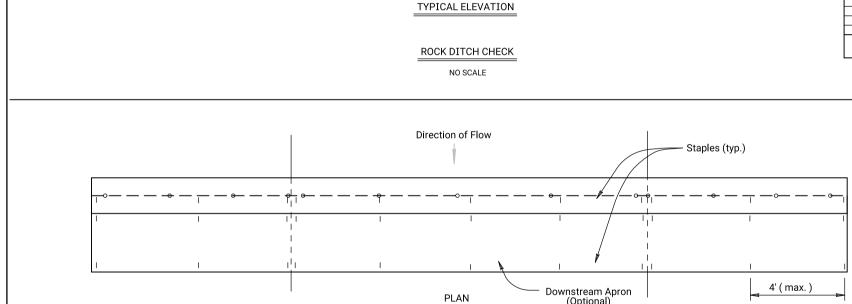
LASSZI	=					
FHWA APPRO	VAL		09-14-16	APP'D.	Scott H	. Shields
DESIGNED	S.H.S.	DETAILED	R.A.A.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN.CK.	TRACE CK.	S.H.S.





- 1. Rock shall be clean aggregate, D50-6" and aggregate filler.
- 2. Place rock in such manner that water will flow over, not around ditch check.
- 3. Do not use rock ditch checks in clear zone.
- 4. Excavation: The ditch area shall be reshaped to fill any eroded areas. Prior to placement of the rock, the ditch shall be excavated to the dimensions of the Rock Ditch Check and to a minimum depth of 6" (150mm). After placement of the rock, backfill and compact any over-excavated soil to ditch grade. This work shall be subsidiary to the bid item Temporary Ditch Check (Rock)
- 5. Aggregate excavated on site may be used as an alternate to the 6" rock, if approved by the Engineer.
- 6. The Engineer may approve the use of larger aggregates for the downstream portion of the check when conditions warrant
- 7. When the use of larger rock is approved, D50-6" rock will be placed between the larger aggregate and the aggregate
- 8. Aggregate filler will be placed on the upstream face of the ditch check. Aggregate filler will comply with Filter Course Type I, Division 1114.





— 6" ( min. )

10'

SECTION A - A

2000000

4' ( max. )

Ground Level

(Optional)

→> B

Stakes (typ.)

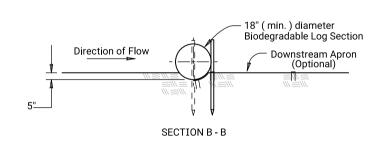
TYPICAL ELEVATION

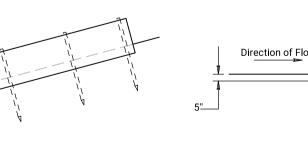
Aggregate Filler

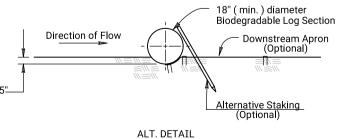
Direction of Flow

29-0CT-2023 17:02

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OPTIONAL

BIODEGRADABLE LOG DITCH CHECK

OR Filter Sock Ditch Check NO SCALE

# BIODEGRADABLE LOG DITCH CHECK NOTES

- 1. Use as many biodegradable log sections as necessary to ensure water does not flow around end of ditch check.
- 2. Overlap sections a minimum of 18".
- 3. Stakes shall be wood or steel according to Section 2114 of the Standard Specifications. Length of stakes shall be a minimum of 2 x the diameter of
- 4. Use Erosion Control (Class 1) (Type C) as the downstream apron when required.
- 5. A downstream apron is required when directed by the Engineer. Apron material will be paid at the contract unit price.
- 6. Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.

03	11-19-20	Revised Standard	M.R.D.	M.L.
02	08-10-16	Revised Standard	R.A.A.	S.H.S.
01	10-21-15	Revised Standard	R.A.A.	S.H.S.
NO.	DATE	REVISIONS	BY	APP'D

TEMPORARY EROSION AND POLLUTION CONTROL ROCK DITCH CHECKS **BIODEGRADABLE LOG DITCH CHECKS** 

LA852G

NATIVE WILDFLOWER MIX 1						
PLS RATE		NAME	QTY (lb)			
0.3		Butterfly Milkweed	, ,			
0.3		Common Milkweed				
0.3		Black Eyed Susan				
0.5		Blanket Flower				
0.5		False Sunflower				
0.5		Lance-Leaf Coreopsis				
0.2		Maximilian Sunflower				
0.1		New England Aster				
0.2		Pinnate Prairie Coneflower				
0.2		Plains Coreopsis				
0.3		Purple Coneflower				
0.3		Upright Prairie Coneflower				
0.3		Dames Rocket				
0.3		Lemon Mint				
0.2		Pitcher Sage				
0.2		Wild Bergamot				
1.0		Illinois Bundleflower				
0.2		Common Evening Primrose				
0.1		Hoary Verbena				
0.8		Purple Prairie Clover				
0.3		Roundhead Lespedeza				
3.0		Showy Partridge Pea				
0.2		White Prairie Clover				
	10.3	Total (lb)				

NATIVE WILDFLOWER MIX 2							
PLS RATE	NAME	QTY (lb)					
0.3	Butterfly Milkweed						
0.3	Black Eyed Susan						
0.5	Black Sampson Coneflower						
1.0	Blanket Flower						
0.2	Maximilian Sunflower						
0.2	Plains Coreopsis						
0.2	Upright Prairie Coneflower						
0.2	Western Yarrow						
0.3	Lemon Mint						
0.4	Pitcher Sage						
1.5	Illinois Bundleflower						
0.2	Common Evening Primrose						
1.0	Blue Wild Indigo						
0.4	Leadplant						
0.4	Purple Prairie Clover						
0.3	White Prairie Clover						
7.4	Total (lb)						

Package and deliver the wildflower seed separately from the grass seed mix. Package and deliver the Tall Drop Seed separately from the grass seed and the wildflower mix. Place the grass seed (except Tall Drop Seed) in the large seed box and drill (cover) seed  $\frac{1}{6}$ ". Place the wildflower seed in a separate seed box and drill (cover) seed  $\frac{1}{6}$ " maximum. Place the Tall Drop Seed in a separate (third) seed box and place the seed (using the seed drill) on the soil surface.

OPTION: Broadcast Tall Drop Seed on the soil surface.

COOL SEASON GRASSES	WARM SEASON GRASSES & WILDFLOWERS
February 15 thru April 20	November 15 thru June 1
August 15 thru September 30	
SPECIES	SPECIES
Bluegrasses	Bermuda Grass
Brome Grasses	Big Bluestem
Canada Wildrye	Blue Grama
Fescues	Buffalo Grass
Prairie Junegrass	Indiangrass
Ryegrasses	Little Bluestem
Sterile Wheatgrass	Sand Bluestem
Tall Dropseed	Sand Dropseed
Western Wheatgrass	Sand Lovegrass
	Side Oats Grama
	Switchgrass
	Wildflower Mixes

SODDING SEASONS										
COOL SEASON GRASSES	WARM SEASON GRASSES									
March 1 thru April 15 September 1 thru November 15	May 15 thru September 1									
SPECIES	SPECIES									
Bluegrass Sod	Buffalo Grass Sod									
Fescue Sod										

If the soil is workable, the Engineer may allow placement of sod between November 15 and March 1. If sod is placed during this time, maintain the sod until 20 days after the beginning of the spring sodding season.

When the area to be seeded is less than 1 acre, seed the area any

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	KTA NO. 8008	2023	68	103

#### **GENERAL NOTES**

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded and mulched. Soil preparation shall conform to the Standard Specifications except as noted below.

All borrow areas shown on the plans are to be fertilized, seeded, and mulched. However, operation in borrow areas where crops are growing may be omitted when requested by the owner.

If temporary cover has provided stable slopes with no erosion, seed the permanent grasses into the existing cover. If there has been erosion that requires repair prior to seeding, then it may be necessary to regrade the area, resulting in bare ground.

FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N,  $P_2O_5$ ,  $K_2O$  listed in Summary of Seeding Quantities will be acceptable.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching material is generally as follows:

 $1\frac{3}{4} - 2\frac{1}{4}$  Tons per Acre =  $1\frac{1}{2}$ " loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

Seed and fertilizer quantities for information only. Items to be paid as Seeding - LSUM

				SI	UMMA	RY OF	SEEDING QUANTITIES		
OLU DD	RATE	L.S. Z/ACRE	OLU DD	ACF	RES		BID ITEM	QUANTITY	UNIT
SHLDR 250	OTHER		SHLDR 0.9	OTHER			Fertilizer (13-13-13)	225.0	lb
0.5			0.9				Seed (Blue Grama Grass) (Lovington)	0.5	lb lb
4.5			0.9				Seed (Buffalo Grass) (Treated)	4.1	lb lb
0.5			0.9	1			Seed (Sand Dropseed Grass)	0.5	lb lb
7			0.9				Seed (Sand Diopseed Grass)  Seed (Side Oats Grama Seeds) (El Reno)	6.3	lb lb
45			0.9				Seed (Tall Fescue) (Endphyte Free)	40.5	lb
6			0.9				Seed (Western Wheatgrass) (Barton)	5.4	lb
Ť			0.15				Cook (Trocton Timeatgrass) (Santon)		
							Mulching *		

SHLDR = Seeded with the Shoulder Mix. Typically 15 feet for 2-lane roads and 30 feet for 4-lane roads. Includes outside roadsides, turfed portions of shoulders, and turfed portion of the median.

OTHER = Seeded with the "Other" Mix. Designated as all other turf areas, except the Shoulder. Usually includes a Native Wildflower Mix.

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. All disturbed areas shall be seeded, fertilized and mulched at the listed rate per acre. The acres are estimated.

Refer to the Standard Specifications, Division 900, Section 904 'Seeding', and Section 907 'Sodding', for the seeding and sodding seasons.

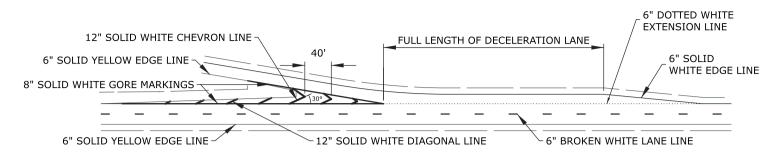
\* See LA852A for mulching quantity. The quantity of mulch is estimated (Acres of Seeding X 1.5 X 2 Tons/Acre). The total mulch required shall be determined in the field. The bid item for mulching shall be paid for according to the Standard Specifications.

02	11-25-20	Updated Seeding / Sodding Periods Charts	M.R.D.	M.L.
01	08-03-20	Revised Standard	M.R.D.	S.H.S.
NO.	DATE	REVISIONS	BY	APP'D

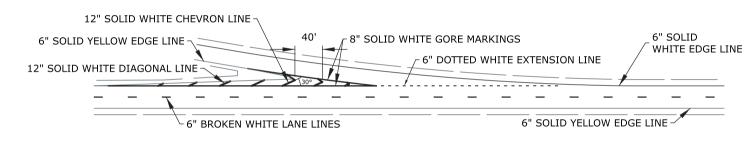
KANSAS DEPARTMENT OF TRANSPORTATION

# PERMANENT SEEDING SUMMARY OF SEEDING QUANTITIES

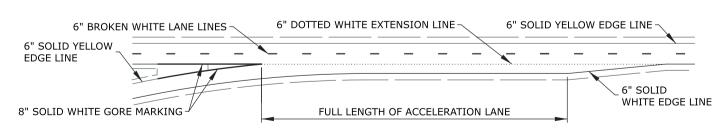
LA850			
FHWA APPROVAL	05-06-19	APP'D.	Mervin Lare
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.



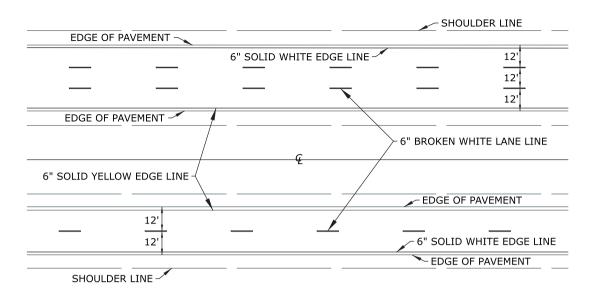
TYPICAL DECELERATION EXIT RAMP



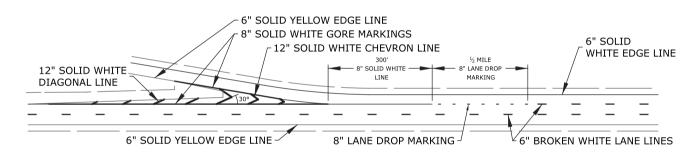
TYPICAL TAPERED EXIT RAMP



TYPICAL ACCELERATION RAMP



# TYPICAL LANE LINE AND EDGE LINE MARKINGS FOR FOUR LANE AND SIX LANE DIVIDED HIGHWAYS

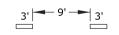


TYPICAL LANE DROP



TYPICAL SPACING FOR DOTTED EXTENSION LINES, UNLESS OTHERWISE NOTED ON PLANS.

TYPICAL SPACING FOR BROKEN LINES UNLESS OTHERWISE NOTED ON PLANS.



TYPICAL SPACING FOR LANE DROP. **UNLESS OTHERWISE** NOTED ON PLANS.

LONGITUDINAL PAVEMENT MARKING LINES SHALL BE OFFSET A MINIMUM OF 2" FROM LONGITUDINAL PAVEMENT JOINTS.

AT RAMP TERMINALS WITH CROSS-ROADS, WRAP 6" EDGE LINES AROUND RADII.

ON NON I, US, AND K ROUTES, 4" EDGE LINES MAY BE INSTALLED. 6" EDGE LINES ARE NOT REQUIRED ON NON I, US, AND K ROUTES.

2	5/25/12	Dotted Extension Lines and Lane Drop Lines	B.A.H.	B.D.G.
1	7/26/05	New FHWA Approval Date	J.F.F.	B.D.G.
NO.	DATE	REVISIONS	BY	APP'D

TYPICAL PAVEMENT MARKING DETAILS FOR MULTI-LANE DIVIDED ROADWAYS

TE307

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	KTA NO. 8008	2023	70	103

					SU	MMA	ry of	PAV	EMEN	IT MA	RKIN	GS										
L	LOCATION	4" Solid WHITE Edge Line	6" Solid WHITE Edge Line	6" Broken WHITE Lane Line	6" Broken WHITE Lane Line (PCP)	6" Dotted WHITE Extension Line	6" Broken WHITE Lane Drop Line	6" Solid WHITE Lane Line	8" Broken WHITE Lane Drop Line	8" Solid WHITE Gore Line	8" Dotted WHITE Extension Line	12" Solid WHITE Diagonal Line	Solid WHITE	WHITE	24" Solid WHITE Type II Crosswalk Line	24" Solid WHITE Stop Line	4" Solid YELLOW Edge Line	4" Solid YELLOW Double Line	4" Solid YELLOW Line	4" Broken YELLOW Line	6" Solid YELLOW Edge Line	12" Solid YELLOW Diagonal Line
NB I-35																						
	05 - 2695+48.33			8,143																	8,143	
26684	3+00 - 2673+80		580																			
00.105																						
SB I-35	2+55 - 2717+05			7,450																	7,450	<del></del>
	3+20 - 2673+80		560	7,430																	7,450	<del></del>
20001	3+20-20/3+60		300																			<del></del>
																						-
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														-								<del></del>
																						-
																						-
	TOTALS		1,140	3,899																	15,593	

ITEMS	TOTAL	UNITS		
PAVEMENT MARKING (MULTI-COMPONENT)(WHITE)(4")		FT		
PAVEMENT MARKING (MULTI-COMPONENT)(WHITE)(6")	5,039	FT		
PAVEMENT MARKING (MULTI-COMPONENT)(WHITE)(8")		FT		
PAVEMENT MARKING (MULTI-COMPONENT)(WHITE)(12")		FT		
PAVEMENT MARKING (MULTI-COMPONENT)(YELLOW)(4")		FT		
PAVEMENT MARKING (MULTI-COMPONENT)(YELLOW)(6")	15,593	FT		
PAVEMENT MARKING (MULTI-COMPONENT)(YELLOW)(12")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(WHITE)(4")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(WHITE)(6")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(WHITE)(8")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(WHITE)(12")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(YELLOW)(4")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(YELLOW)(6")		FT		
PAVEMENT MARKING (THERMOPLASTIC)(YELLOW)(12")		FT		
PAVEMENT MARKING (EPOXY)(WHITE)(4")		FT		
PAVEMENT MARKING (EPOXY)(WHITE)(6")		FT		
PAVEMENT MARKING (EPOXY)(WHITE)(8")		FT		
PAVEMENT MARKING (EPOXY)(WHITE)(12")		FT		
PAVEMENT MARKING (EPOXY)(YELLOW)(4")		FT		
PAVEMENT MARKING (EPOXY)(YELLOW)(6")		FT		
PAVEMENT MARKING (EPOXY)(YELLOW)(12")		FT		
PAVEMENT MARKING (INTERSECTION GRADE)(WHITE)(12")		FT		
PAVEMENT MARKING (INTERSECTION GRADE)(WHITE)(24")		FT		
PAVEMENT MARKING (INTERSECTION GRADE)(YELLOW)(12")		FT		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(WHITE)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(WHITE)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(WHITE)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(WHITE)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(WHITE)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(US-SHIELD)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(K-SHIELD)( )		EACH		
PAVEMENT MARKING SYMBOL (INTERSECTION GRADE)(I-SHIELD)( )		EACH		
PAVEMENT MARKING (PATTERNED COLD PLASTIC)(WHITE)(6")		FT		
PAVEMENT MARKING (PATTERNED COLD PLASTIC)(WHITE)(8")		FT		
PAVEMENT MARKING (PATTERNED COLD PLASTIC)(WHITE)(12")		FT		

SUMMARY OF WORD & SYMBOL MARKINGS																				
LOCATION	4	4	1	4	4	Ė	STOP	ONLY	X-ING	SCHOOL	10	435	24	400	13	\$ 4	<b>\$</b>	\$ 1	3	<b>&gt;</b>
<u> </u>																				
TOTALS																				

WORDS & SYMBOLS SHALL CONFORM TO THE LATEST EDITION OF "STANDARD ALPHABETS FOR HIGHWAY SIGNS AND PAVEMENT" MARKINGS" PRINTED BY THE U.S. DEPARTMENT OF TRANSPORTATION, FEDERAL HIGHWAY ADMINISTRATION.

PRIOR TO COMMENCEMENT OF PAVEMENT MARKING WORK THE ENGINEER WILL ESTABLISH THE LIMITS FOR "NO PASSING" ZONES. THESE LIMITS SHALL BE USED FOR THE LOCATION OF "NO PASSING" LINES AND FOR THE COMPUTATION OF ACTUAL MARKING QUANTITIES FOR THIS LINE

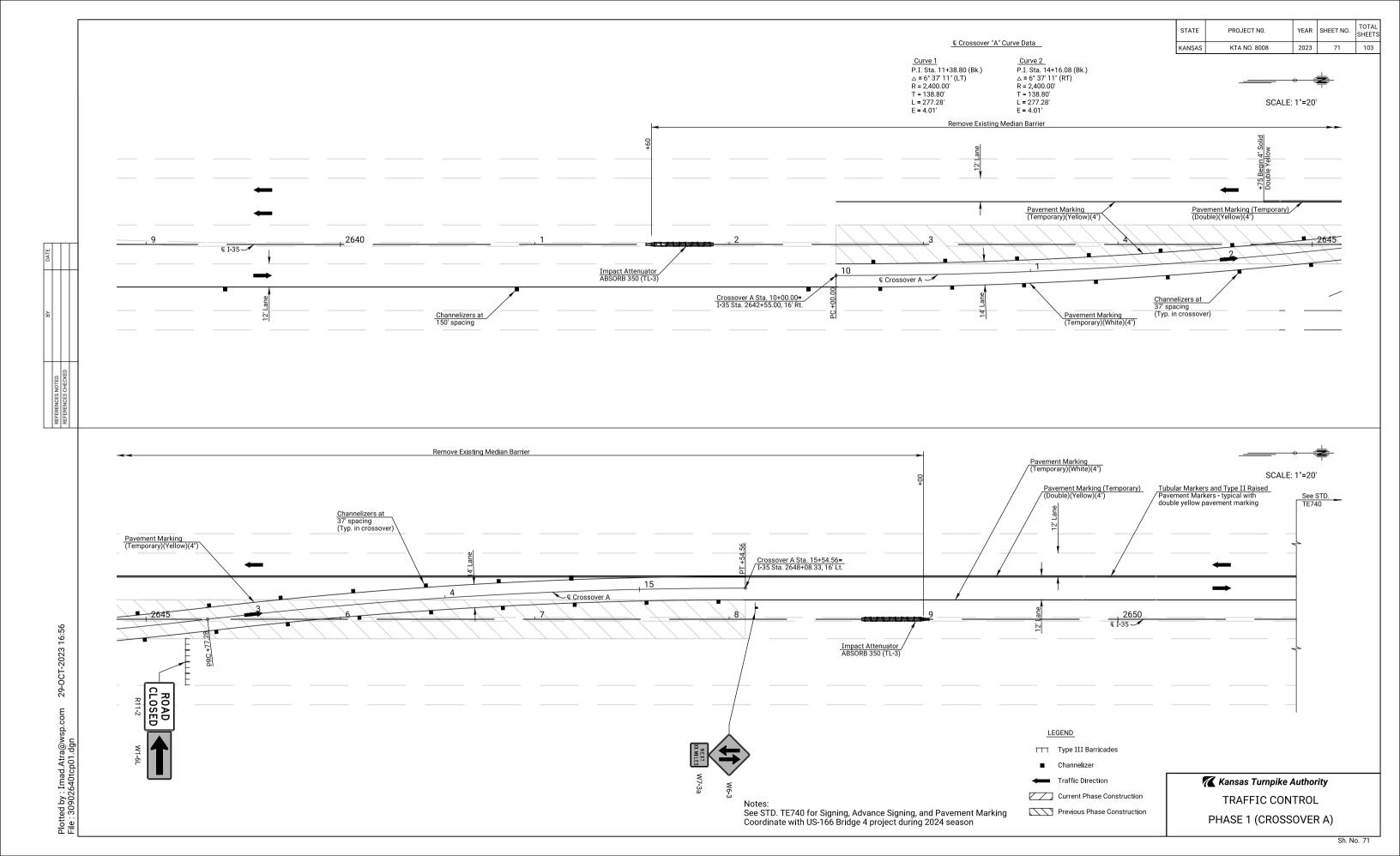
2	5/25/12	Added Line Types, Symbols, and Shields	B.A.H.	B.D.G.
1	7/26/05	New FHWA Approval Date	J.F.F.	B.D.G.
NO.	DATE	REVISIONS	BY	APP'D

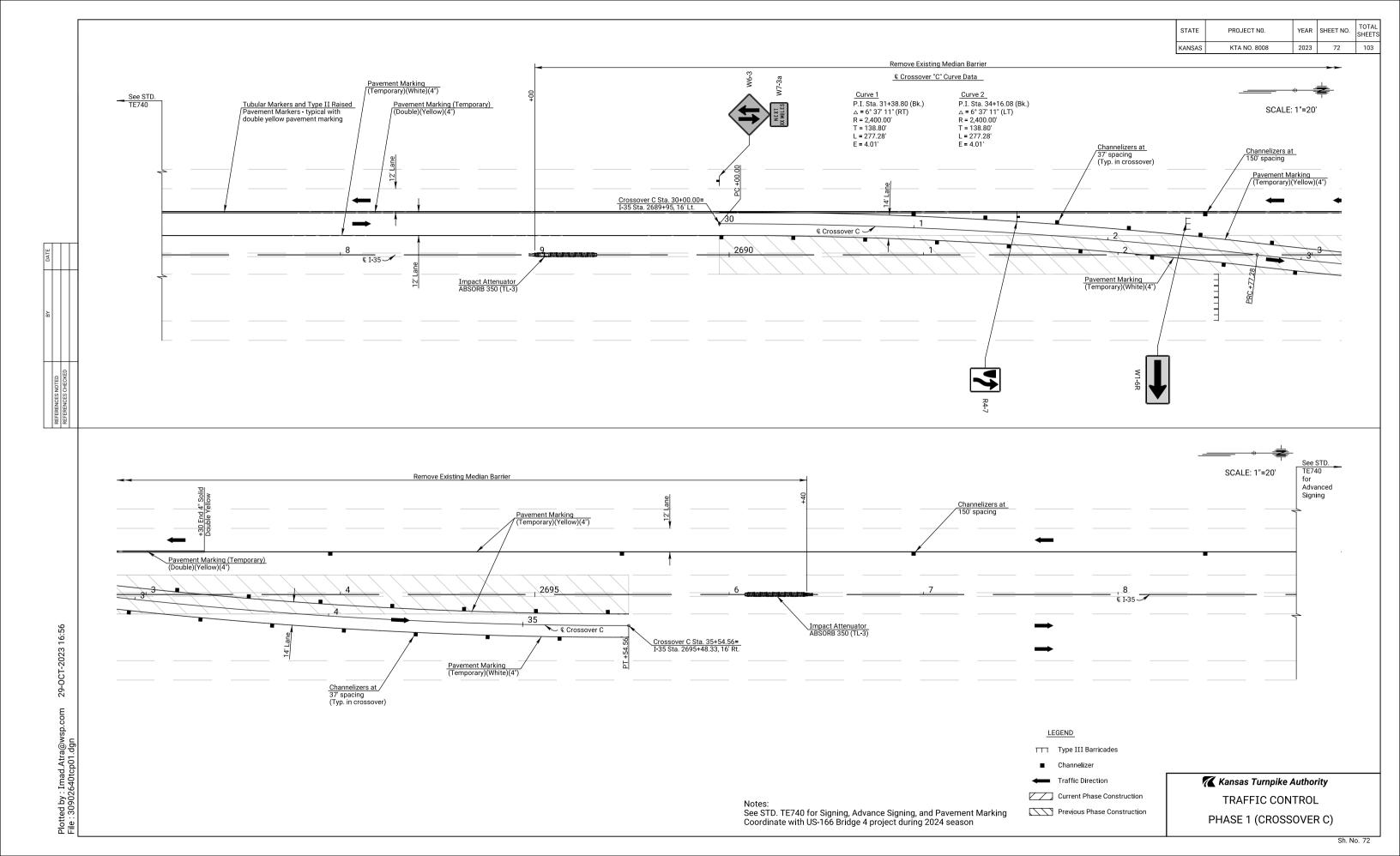
KANSAS DEPARTMENT OF TRANSPORTATION SUMMARY AND RECAPITULATION OF PAVEMENT MARKING QUANTITIES

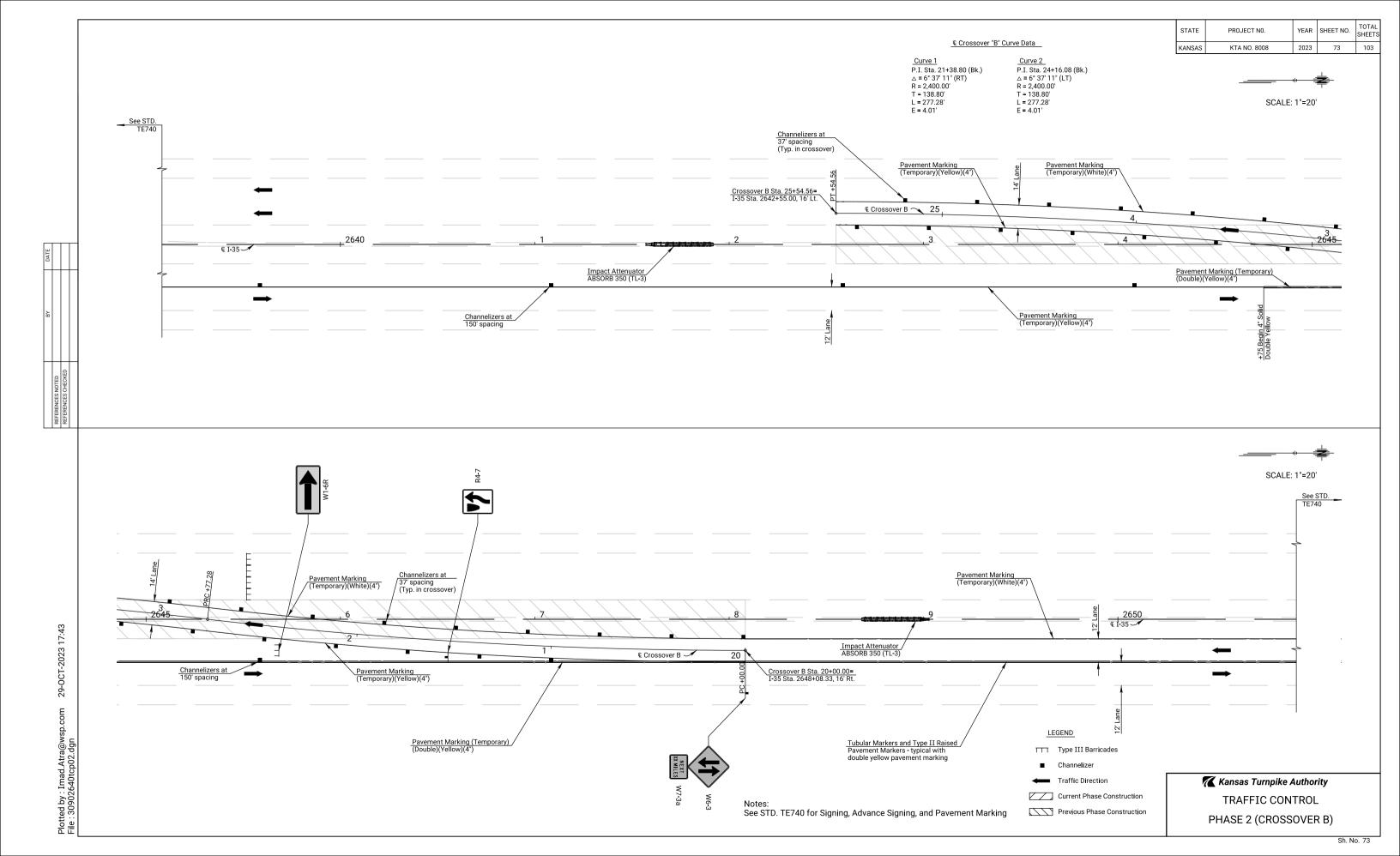
TE311

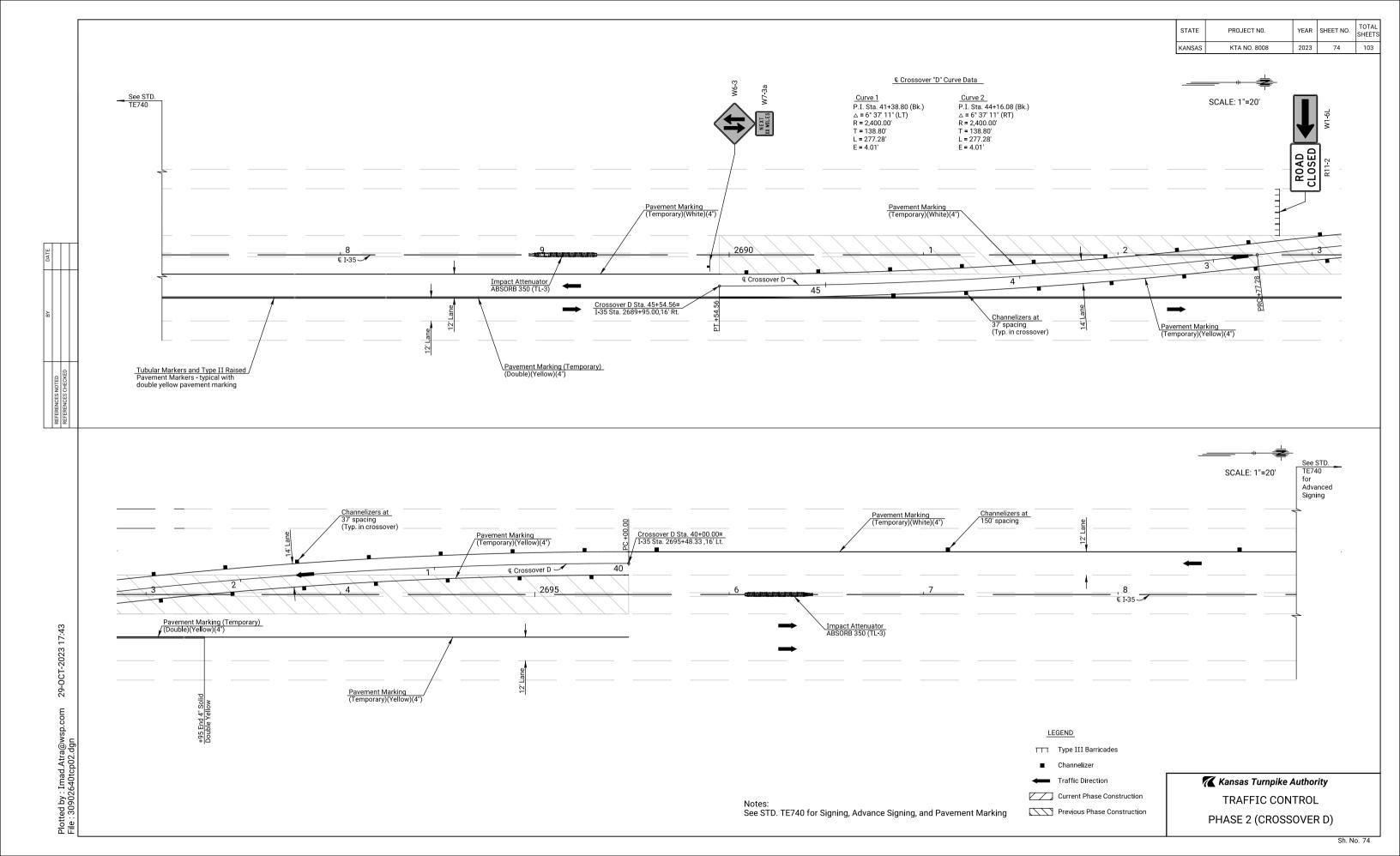
NOTE: ALL TOTALS REFLECT ACTUAL QUANTITY OF PAVEMENT MARKING MATERIALS REQUIRED.

NOTE: FOR SPECIFIC PAVEMENT MARKING DETAILS AND DIMENSIONS SEE PLAN SHEETS









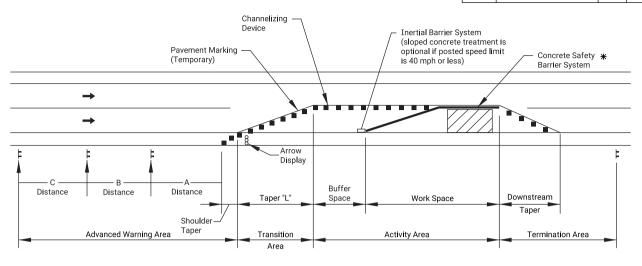
3) Consideration should be made to separate pedestrian and, if needed, bicycle movements from both work site activity and vehicular traffic. Unless a reasonable safe route that does not involve crossing the roadway can be provided, pedestrians should be appropriately directed with advance signing that encourages them to cross to the opposite side of the roadway. In urban and suburban areas with high vehicular traffic volumes, these signs should be placed at intersections (rather than midblock locations) so that pedestrians are not confronted with midblock work sites that will induce them to attempt skirting the work site or making a midblock crossing.

4) When existing pedestrian facilities are disrupted, closed, or relocated, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility.

5) When the driving surface open to traffic is milled or is a temporary surface made of loose material, or when directed by the engineer a W8-15 (Grooved Pavement) or W8-7 (Loose Gravel) sign shall be used on mainline approaches. This sign should be placed a "C" distance after the W20-1 (Road Work Ahead) sign. A W8-15p motorcycle plague shall be used to supplement the W8-15 or W8-7 signs. All signs shall be displayed as long as the condition is present.

6) Alternative temporary rumble strip options may be available. Please contact the Temporary Traffic Control Unit for more information at 785-296-1179 or 785-296-1183.

STATE	PROJECT NO.	YEAR		TOTAL SHEETS	
ANSAS	KTA NO. 8008	2023	75	103	ı



#### TYPICAL WORK ZONE COMPONENTS

\*When concrete barrier system is used, portable channelizing devices are not needed along the tangent barrier section.

#### Minimum advance warning sign spacing (in feet):

· · · · · · · · · · · · · · · · · · ·			•
SPEED (MPH) *	Α	В	С
URBAN (40 MPH OR LOWER)	100	100	100
URBAN (45 MPH OR HIGHER)	350	350	350
RURAL (55 MPH OR LOWER)	500	500	500
RURAL (60 MPH OR HIGHER)	750	750	750
EXPRESSWAY/FREEWAY	1000	1500	2640

#### \* Posted speed prior to work starting

The minimum spacing between signs shall be no less than 100', unless directed by the engineer.

The spacing between any signs may be increased beyond the minimum values in the table above as approved by the engineer in order to maximize visibility.

#### Taper Formulas:

L = WS for speeds of 45 MPH or more

 $L = WS^2/60$  for speeds of 40 MPH or less

Where: L = Minimum length of taper in feet

S = Numericial value of posted speed prior to work starting in MPH

W = Width in offset feet

Shifting Taper=1/2 L Shoulder Taper=1/3 L

#### Channelizer Placement:

- (1) The spacing between devices in transition area (taper) should not exceed a distance in feet equal to 1/2 the posted speed limit in mph prior to work starting
- (2) The spacing between devices in the advanced warning area and the activity area should not exceed a distance in feet equal to two times the posted speed limit in mph prior to work starting.
- (3) Channelizing devices shall be placed for optimum visibility, normally at right angles to the traffic flow.
- (4) Place directional indicator barricades in series to direct traffic onto the new path. The arrow sign should not be visible to opposing traffic.
- (5) Alternating diagonal orange and white striping must slope downward in the direction traffic is expected to pass.

#### **Buffer Space**

SPEED (MPH) *	20	25	30	35	40	45	50	55	60	65	70	75
LENGTH (ft)	115	155	200	250	305	360	425	495	570	645	730	820

#### \* Posted speed prior to work starting

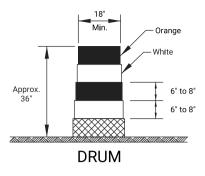
Neither work activity nor storage of equipment, vehicles, or material should occur in the buffer space. When a protection vehicle is placed in advance of the work space, only the space upstream of the vehicle constitutes the buffer space.

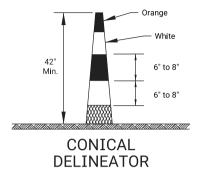
If temporary concrete safety barrier system is used to separate approaching traffic from the work space, the barrier system shall be considered part of the activity area. A full lane width should be available throughout the length of the buffer space. See typical work zone components above.

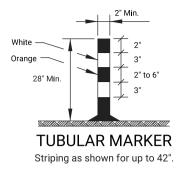
02	03-13-18	W8-15p usage changed to Shall	R.W.B.	E.K.G.						
01	08-18-15	Channelizer spacing info	R.W.B.	K.E.						
NO.	DATE	REVISIONS	BY	APP'D						
	KANSAS DEPARTMENT OF TRANSPORTATION									

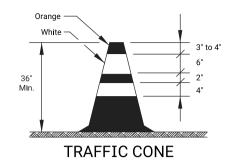
#### TRAFFIC CONTROL **GENERAL NOTES**

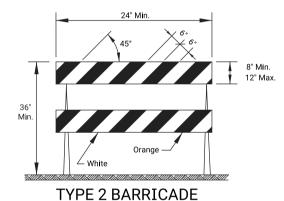
TE700					
FHWA APPRO	DVAL		03-13-18	APP'D.	Eric Kocher
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.



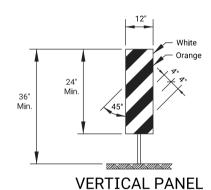




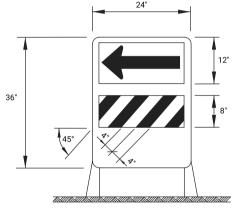




For rails less than 36" long, 4" wide stripes may be used. All stripes shall slope downward to the traffic side for channelization.

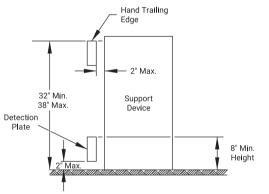


The stripes shall slope downward to the traffic side for channelization.



### **DIRECTION INDICATOR BARRICADE**

The stripes shall slope downward in the direction traffic is to pass. The direction indicator barricade shall be used in series to direct the motorist into the intended lane of travel.



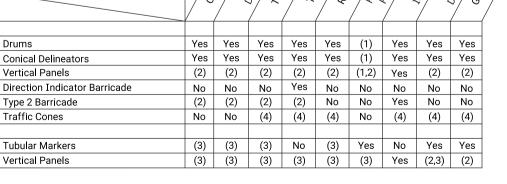
#### PEDESTRIAN CHANNELIZER

- 1. Support device shall not project beyond the detection plate into the pathway.
- 2. Hand trailing edges and detection plates are optional for continuous walls.
- 3. Interconnect pedestrian channelizers to prevent displacement
- and to provide continuous guidance through or around work.

  4. Alternate pathways shall be firm, stable, and slip resistant.
- 5. Treat height differentials > 1/2" in the surfaces of alternate
- paths with a firm, stable, and slip resistant temporary ramp having a slope of 12:1 or flatter and having a width equal to the alternate path.
- 6. Use alternating orange/white on interconnected devices.

Item	Location	\_\d	S.900.85.01.67.8	Viver Signs	"Yents"	Par Pars	Squ. H680't	/ead '	Lead Lead	Solvies (Solvies)	7 Sy
Portable											
	Drums	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes	
	Conical Delineators	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes	
	Vertical Panels	(2)	(2)	(2)	(2)	(2)	(1,2)	Yes	(2)	(2)	
	Direction Indicator Barricade	No	No	No	Yes	No	No	No	No	No	
	Type 2 Barricade	(2)	(2)	(2)	(2)	No	No	Yes	No	No	
	Traffic Cones	No	No	(4)	(4)	(4)	No	(4)	(4)	(4)	
Fixed											
	Tubular Markers	(3)	(3)	(3)	No	(3)	Yes	No	Yes	Yes	
	Vertical Panels	(3)	(3)	(3)	(3)	(3)	(3)	Yes	(2,3)	(2)	

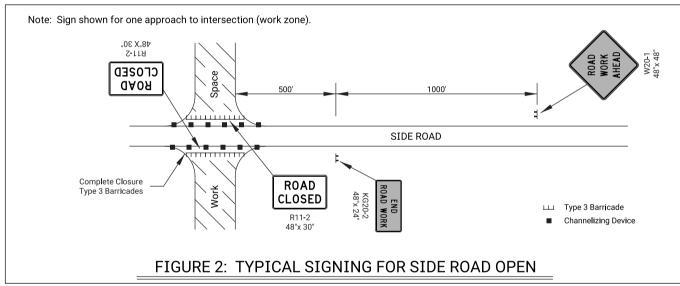
- (1) Not allowed on centerline delineation along freeways or expressways.
- (2) The stripes shall slope downward to the traffic side for channelization.
- (3) May be used upon the approval of the engineer.
- (4) Daytime operations only.

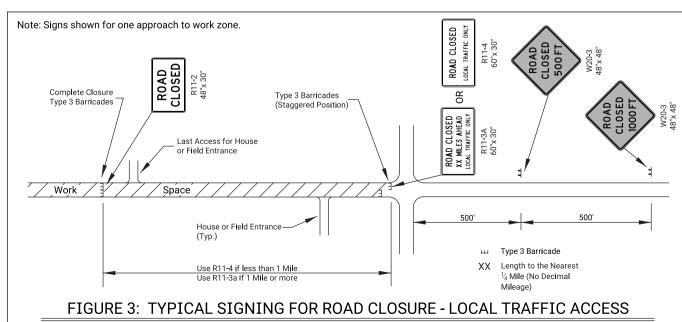


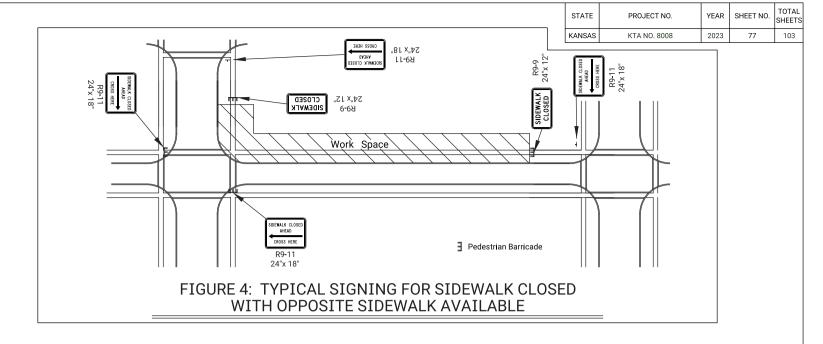
REVISIONS KANSAS DEPARTMENT OF TRANSPORTATION

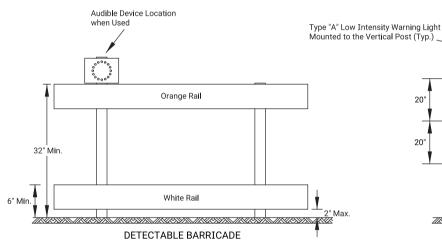
> TRAFFIC CONTROL CHANNELIZING DEVICES

FHWA APPROVAL
DESIGNED L.E.R. DETAILED R.W.B. QUANTITIES
DESIGN CK. DETAIL CK. QUAN.CK.

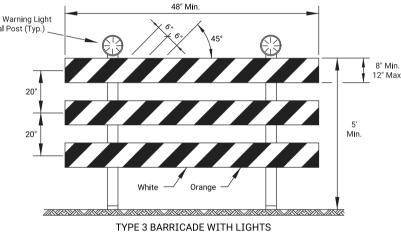








- 1. Support device shall not project beyond the detection plate into the pathway.
- 2. Barricades shall be used to close the entire width of the pathway.
- 3. Do not use warning lights on pedestrian barricades.
- 4. Do not use warning lights on audible devices.



Approved signs mounted on Type 3 barricades should not cover more than 50% of the top two rails or 33% of the total area of the three rails.

When barricades are placed end-to-end or staggered, a Type "A" low intensity warning light shall be mounted to the vertical post near each outside corner of the end barricades.

#### ROAD CLOSED GENERAL NOTES

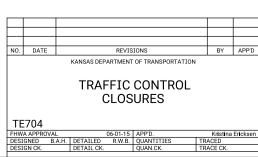
As shown in Figure 1, at the point where thru traffic must detour and local traffic can proceed to the location where the roadway is completely closed, the R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) or R11-4 (ROAD CLOSED LOCAL TRAFFIC ONLY or ROAD CLOSED TO THRU TRAFFIC) sign shall be used with Type 3 barricades (winged position), placed on the shoulders of roadway.

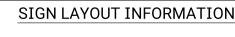
As shown in Figure 3, when local traffic must be allowed access into the work zone, Type 3 barricades shall be longitudinally staggered to maintain the appearance of a closed roadway. A second line of end-to-end Type 3 barricades shall be placed just beyond the last access point in the work zone, to completely close the roadway.

The R11-4 (ROAD CLOSED TO THRU TRAFFIC or ROAD CLOSED LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is less than 1 mile.

The R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is 1 mile or greater.

The words "BRIDGE OUT" (or BRIDGE CLOSED) may be substituted for the words "ROAD CLOSED" on the R11-3a or R11-4 sign where applicable.





END ROAD WORK KG20-2

Std. Size Expwy/Freeway

> 6" C 48"x 24"

WAIT FOR

Std. Size Expwy/Freeway

PILOT CAR 6" C KG20-5 48"x 24"

WORK ZONE KM4-20

NEXT

X MILES

W7-3a

Std. Size Expwy/Freeway 3" C 6" C 48"x 12" 24"x 6"

Mileage to be Determined

by the Engineer.

W8-7

GROOVED

PAVEMENT

W8-15

LOOSE

**GRAVEL** 

Std. Size Expwy/Freeway 30"x 24"

Std. Size

Expwy/Freeway

48"x 48"

Std. Size

Expwy/Freeway

8" D

48"x 48"

Std. Size Expwy/Freeway 48"x 48"

UNEVEN W8-11

Std. Size Expwy/Freeway 8" D 48"x 48"

W8-17

SHOULDER DROP-OFF

Std. Size Expwy/Freeway 30"x 24"

W8-17P (Optional)

NB US-75 CLOSED FOLLOW DETOUR

Std. Size 6" C

Expwy/Freeway

SP-01 (Special Sign)

US-75 CLOSED NORTH OF Topeka FOLLOW DETOUR

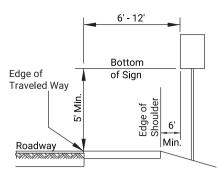
Std. Size Uppercase: 6" C

Expwy/Freeway

Lowercase: 4.5" C

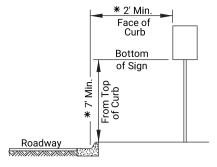
Uppercase: 10" D Lowercase: 8" D

All city names and street names on special signs and destination signs must have upper and lower case letters.



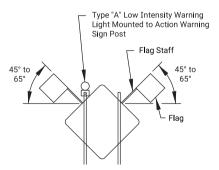
#### **RURAL**

- 1) Ground-mounted signs shall be mounted at a minimum height of 5' measured from the bottom of sign to the near edge of the pavement.
- 2) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- 3) The height of the secondary sign mounted below another sign may be 4' measured from the bottom of the sign to the near edge of the pavement. Signs shall not overlap each other.



#### **URBAN**

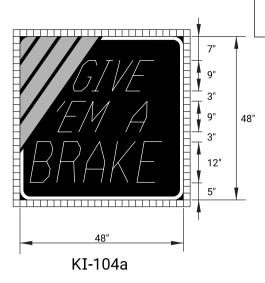
- 1) Signs shall be mounted at a minimum height of 7' measured from the bottom of sign to the near edge of the pavement.
- 2) Neither portable nor permanent sign supports should be located on sidewalks or areas designated for pedestrian or bicycle traffic.
- 3) Signs mounted lower than 7' should not project more than 4" into pedestrian
- 4) The height from of the secondary sign mounted below another sign may be 6' measured from the bottom of sign to the near edge of the pavement. Signs shall not overlap each other.
- 5) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- \* 6) Pedestrian detour signing shall be a minimum of 2' measured from the top of the pedestrian pathway to the bottom of the sign and shall not protrude into the walkway nor shall it project beyond the back of curb.



When the sign width is equal to or greater than 9', three or more wood posts may be used with a minimum of 4' between the centerline of each post. All signs less than 9' in width shall use a maximum of two wood

In the case of hitting rock when driving posts

- 1. Shift the sign location. Do not violate minimum sign spacing.
- 2. With the engineer's approval, use acceptable alternative sign stands.



Sign Number	GIVE EM A BRAKE
Width x Height	4'-0" x 4'-0"
Border Width	1.0"
Corner Radius	4.0"
Stripe Width	3.0"
Mounting	Ground
Background	Type: Non-Reflective
	Color: Black
Legend/Border	Type: Reflective
	Color: White
Legend Font	Dutch 801 Roman SWC
	25 Degree Slant
Stripes	Type: Reflective
	Color: Orange

PROJECT NO.

KTA NO. 8008

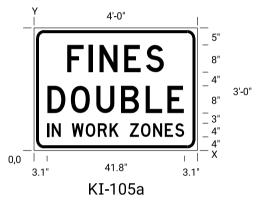
STATE

KANSAS

YEAR SHEET NO. TOTAL SHEETS

78

2023



Sign Number	FINES DOUBLE			
Width x Height	4'-0" x 3'-0"			
Border Width	0.9"			
Corner Radius	3.0"			
Mounting	Ground			
Background	Type: Reflective			
	Color: White			
Legend/Border	Type: Non-Reflective			
	Color: Black			

#### Dimensions in inches

Spacings are to start of next letter

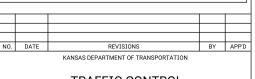
Y FONT											HT LEN						
23.0	$\boxtimes$	F	I	N	Е	S	$\boxtimes$										8.0
D	9.7	6.4	3.2	7.3	6.4	5.4	9.7										28.6
11.0	$\boxtimes$	D	0	U	В	L	E	$\times$									8.0
D	3.9	6.9	7.5	7.3	7.3	6.4	4.9	3.9									40.3
4.0	$\boxtimes$	I	N	$\boxtimes$	W	0	R	K	$\supset$	Ζ	0	N	Ε	S	$\boxtimes$		4.0
D	3.1	1.6	2.7	3.2	4.3	3.8	3.6	2.8	3.2	3.4	3.8	3.6	3.2	2.7	3.1		41.8

#### Notes:

Typically, there are two sets of informational signs installed per project: one for each direction of traffic.

Install signs a minimum of 500' in advance of the road work ahead sign. The engineer may designate a more appropriate location if conditions dictate.

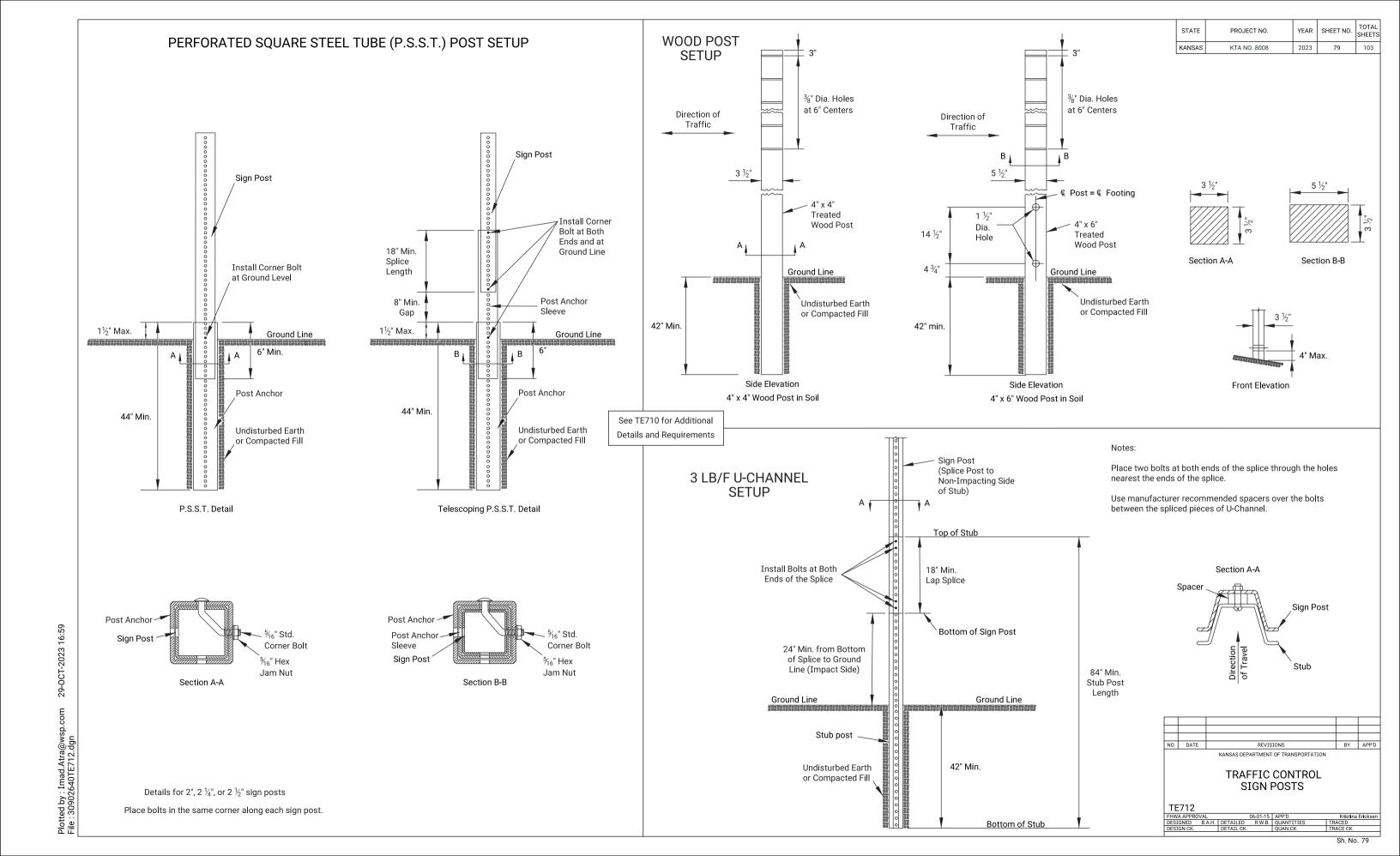
The informational signs are not to interfere with the traffic control signs for the project.

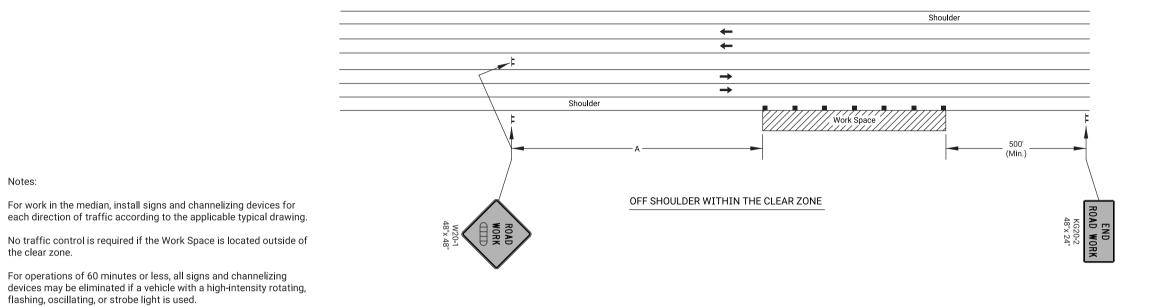


#### TRAFFIC CONTROL SIGN INFORMATION

FHWA APPROVAL 06-01-15 APP'D.
DESIGNED R.W.B. DETAILED R.W.B. QUANTITIES
DETAIL.CK. QUAN.CK.

SP-02 (Special Sign)





# Shoulder $\rightarrow$ Shoulder 500' (Min.) ON SHOULDER

- st Omit taper if paved shoulder is less than 8' wide.
- \* \* Eliminate W7-3a if shoulder is closed for less than 2 miles.

X Length to the Nearest Whole Mile	Χ	Length to the	Nearest	Whole Mile
------------------------------------	---	---------------	---------	------------

Plotted by : Imad.Atra@wsp.com File : 30902640TE722.dgn

Notes:

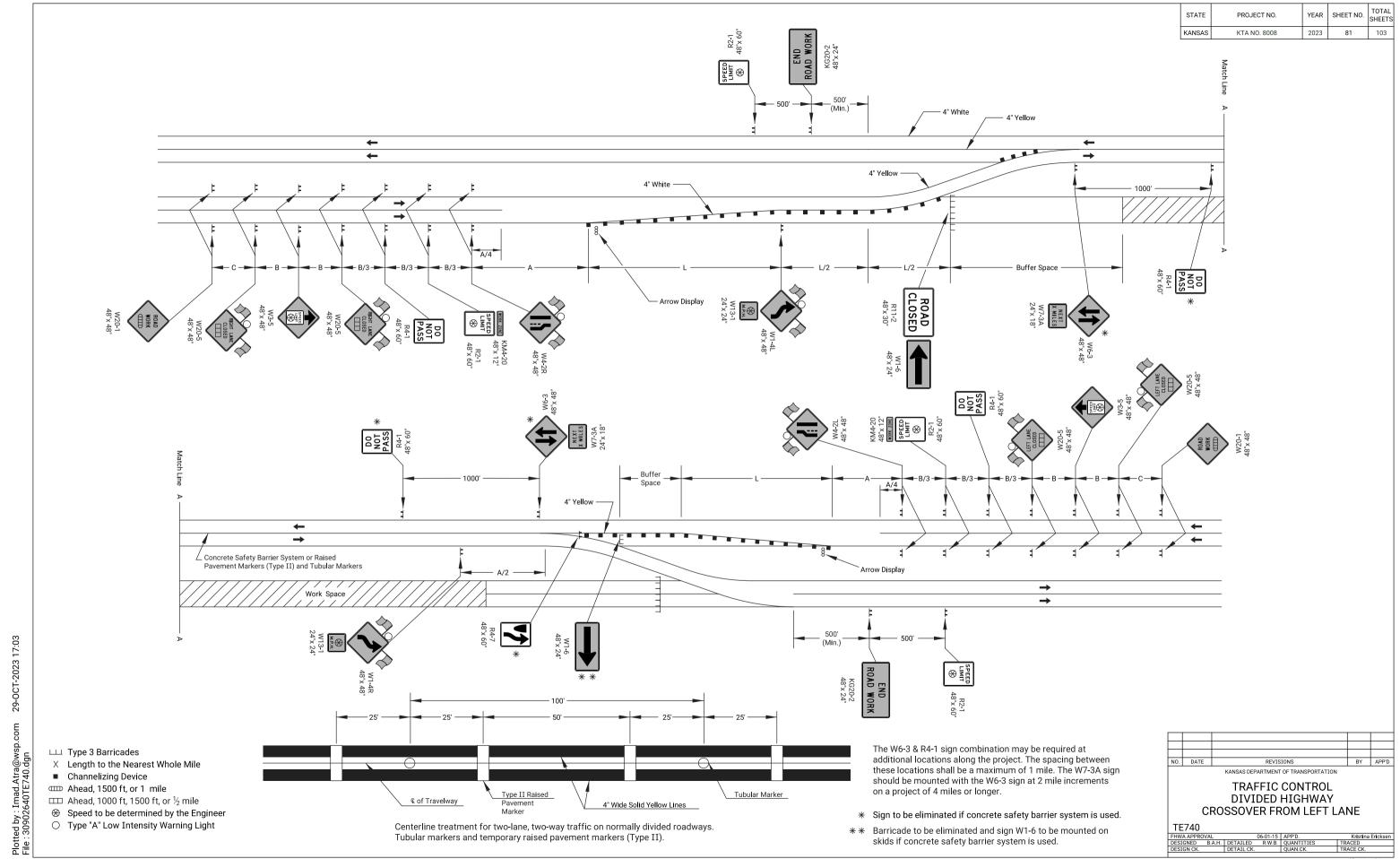
the clear zone.

- Channelizing Device
  Ahead, 1500 ft, or 1 Mile
- ☐☐ Ahead, 1000 ft, 1500 ft or ½ Mile



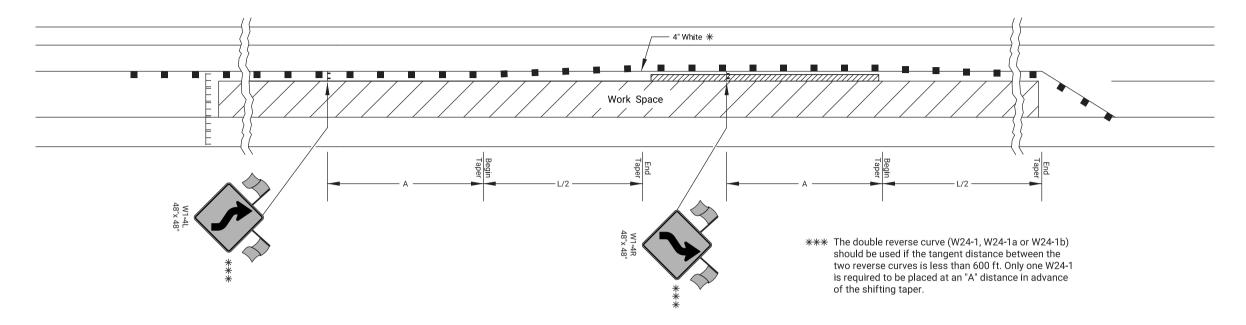
TRAFFIC CONTROL SHOULDER WORK DIVIDED ROADWAY

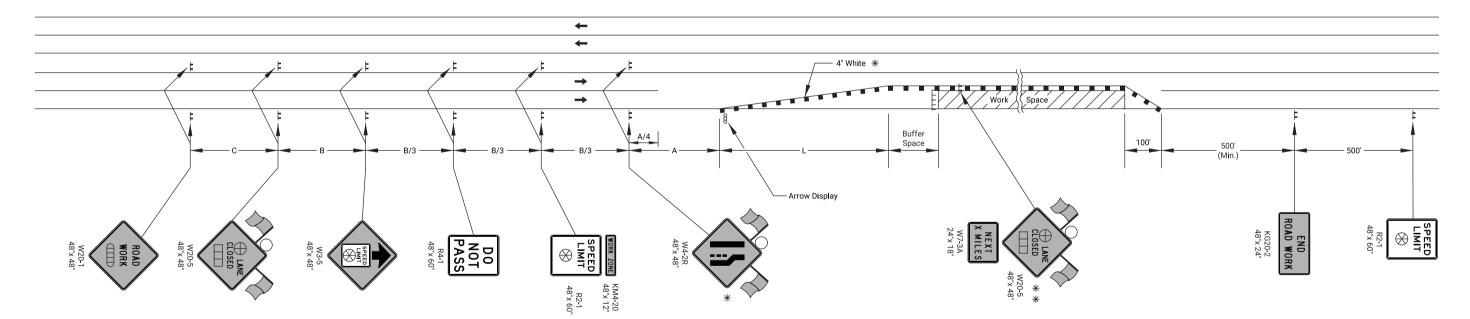
TE722



### SHIFTING TAPER DETAIL

Add signs and devices as shown for work inside a closed lane that extends near to (or into) the open traffic lane.





29-0CT-2023 16:59

- X Length to the Nearest Whole Mile
- Channelizing Device
- Ahead, 1500 ft, or 1 mile
- $\square$  Ahead, 1000 ft, 1500 ft, or  $\frac{1}{2}$  mile
- ⊕ Right or Left
- $\otimes$  Speed to be determined by the Engineer
- Type "A" Low Intensity Warning Light

- \* For left lane closures use W4-2L and yellow edge line along channelizing devices.
- \* \* The W20-5 (\(\oplus \text{Lane Closed}\)) and W7-3A (Next X Miles) signs should be placed at 2 mile increments on a project of 4 miles or longer.

Left-side signs shall be omitted for a four-lane undivided highway.

One flagger should be stationed within each multi-lane roadway activity area where work is in a closed lane adjacent to traffic and not separated by a concrete safety barrier system.

01	R.W.B.	E.G.K.							
NO.	DATE	REVISIONS	BY	APP'D					
KANSAS DEPARTMENT OF TRANSPORTATION									

## TRAFFIC CONTROL LANE CLOSURE ON MULTILANE HWY

TE744						
FHWA APPRO	VAL		03-13-18	8 APP'D. Enic Ko		
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED	
DESIGN CK.		DETAIL CK.		QUAN.CK.	TRACE CK.	

Work Zone Signs (0 to 9.25 Sq.Ft.)

Work Zone Signs (9.26 to 16.25 Sq.Ft.)

Work Zone Signs (16.26 Sq.Ft. & Over)

Recapitulation of Quantities

Quantity

Unit

Each Per Day

Each Per Day

Each Per Day

## SUMMARY OF TRAFFIC CONTROL DEVICES (EACH)

Work Zone Sign (Special)						
Sign No. 16.25 Sq.Ft. & Less 16.26 Sq.Ft. & Ov						

	Temporary Pavement Marking							
	Item	Unit	Pha	Total				
	item	Offic	Phase 1	Phase 2	local			
**	4" Solid White (Type I)	Lin. Ft.	8146	6646	14792			
**	4" Solid Yellow (Type I)	Lin. Ft.	2954	2954	5908			
**	4" Solid Double Yellow (Type I)	Lin. Ft.	9710	9710	19420			
	Temp. Raised Pvmt. Marker (Type II)	Each	86	86	172			

\*\* Paid for as Pavement Marking (Temporary) 4" Solid (Type I)

For the 2024 construction season, coordinate traffic control for NB approach and lane closure with KTA Contract 8011 Bridge Raising at MM 4. This may be accomplished by omitting, covering and/or removing approach signing for the northbound closure and continuing the NB lane closure from MM 4 thru this project. Omit/remove/cover SB End Road Work sign as appropriate during this same time period.

### SUMMARY OF TRAFFIC CONTROL DEVICES (EACH PER DAY)

\* Quantity most used on the project at any one time

	Work Zo	one Signs *	
Sign No.		Size - Sq.Ft. 9.26-16.25	
	0-9.25		16.26 & Over
W20-7		2	
KG20-2	2		
KM4-20	4		
R2-1			6
R4-1			6
R4-7			1
R11-2		1	
W1-4L		1	
W1-4R		1	
W1-6	2		
W3-5		4	
W4-2L		2	
W4-2R		2	
W6-3		2	
W7-3A	2		
W13-1	2		
W20-1		4	
W20-5		8	
W21-5A		1	
W21-5B		1	
		· ·	
		-	
		-	
		1	

Barrio	cades *	Cha	nnelizing De	vices *
Type 3 (4' to 12') Pedestrian		Fixed	Portable	Pedestrian
9		43	115	

Lighted Devices *	
Work Zone Warning Light (Type "A" Low Intensity)	32
Work Zone Warning Light (Red Type "B" High Intensity)	
Arrow Display	2
Portable Changeable Message Sign	2



12-1-23 Revised PCMS Quantity. Added Note.

work Zone Signs (16.26 Sq.Ft. & Over)		Each Per Day
Work Zone Barricades (Type 3 - 4' to 12')		Each Per Day
Work Zone Barricades (Pedestrian)		Each Per Day
Channelizer (Fixed)		Each Per Day
Channelizer (Portable)		Each Per Day
Channelizer (Pedestrian)		Each Per Day
Work Zone Warning Light (Type "A" Low Intensity)		Each Per Day
Work Zone Warning Light (Red Type "B" High Intensity)		Each Per Day
Arrow Display		Each Per Day
Portable Changeable Message Sign		Each Per Day
Pavement Marking (Temporary)		
4" Solid (Type I)	401	Sta./Line
4" Solid (Type II)		Sta./Line
4" Broken (8.0') (Type I)		Sta./Line
4" Broken (8.0') (Type II)		Sta./Line
4" Broken (3.0') (Type I)		Sta./Line
4" Broken (3.0') (Type II)		Sta./Line
4" Dotted Extension (Type I)		Sta./Line
4" Dotted Extension (Type II)		Sta./Line
Solid (Line Masking Tape)		Sta./Line
Broken (Line Masking Tape)		Sta./Line
Symbol (Type I)		Each
Symbol (Type II)		Each
Flexible Raised Pavement Marker (4" Broken (8.0'))		Sta./Line
Flexible Raised Pavement Marker (4" Broken (3.0"))		Sta./Line
Pavement Marking Removal	30,100	
	30,100	Lin. Ft. Each
Work Zone Sign (Special) (16.25 Sq. Ft. & Less) Work Zone Sign (Special) (16.26 Sq. Ft. & More)		Each
Rigid Raised Pavement Marker (Type I)		Each
Rigid Raised Pavement Marker (Type I)  Rigid Raised Pavement Marker (Type II)	172	Each
	1/2	
Traffic Signal Installation (Temporary)		Lump Sum
Traffic Control (Initial Set Up)		Lump Sum
Traffic Control	Lump Sum	Lump Sum
Flagger (Set Price)	1	Hour

KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL SUMMARY OF DEVICES **RECAPITULATIONS OF QUANTITIES** 

				011	MAADY OF OUANITITIES			
				SU	MMARY OF QUANTITIES	1		
LOCATION (ROUTE)	STATION (OR STATION RANGE)	SIDE	FLARE RATE (WHERE APPLICABLE)	TEMPORARY	TEMPORARY-RELOCATE	TEMPORARY-INSTALL ONLY	UNIT	COMMENTS
I-35	2667+79.64 - 2669+91.46	Lt.	15:1	212.5'			17	Phase 1
I-35	2668+72.80 - 2669+84.62	Lt.	15:1	112.5'			9	Phase 1
I-35	2672+03.19 - 2674+15.69	Rt.			212.5'		17	Phase 2
South Crossover	2641+60.00 - 2649+00.00	Ę.				740.0	59	Provided by KTA for winter shutdown
North Crossover	2689+00.00 - 2696+40.00	Œ.				740.0	59	Provided by KTA for winter shutdown
South of Bridge Medians	2668+50.10 - 2669+90.10	Œ.				140.0	11	Provided by KTA for winter shutdown
North of Bridge Medians	2672+14.63 - 2673+74.63	Œ.				140.0	11	Provided by KTA for winter shutdown
TOTALS			325.0	212.5	1760.0			

The quantity reported does not include the 3%" gap between 12'-6" sections of barrier. The 3%" gap will not be included in the pay length for Concrete Safety Barrier (Type F3) (Temporary). See the Summary of Quantities on Sheet No. XX for Recap of Temporary Concrete Safety Barrier and End Treatments.

Design Parameters				
Design Speed (mph)	Flare Rate (a:b)			
70	15:1			
60	14:1			
55	12:1			
50	11:1			
45	10:1			
40	8:1			
30	7:1			

Note: The flare rates listed here apply only to temporary concrete safety barrier installations. See temporary concrete safety barrier layouts included in the plans for variations. Typical alternate flare rates may be used as approved by the Engineer.

01	02-11-15	Initial Release	K.E.K.	S.W.K.
NO.	DATE	REVISIONS	BY	APP'D

SUMMARY OF QUANTITIES TEMPORARY CONCRETE SAFETY BARRIER AND END TREATMENTS

YEAR SHEET NO. TOTAL SHEETS

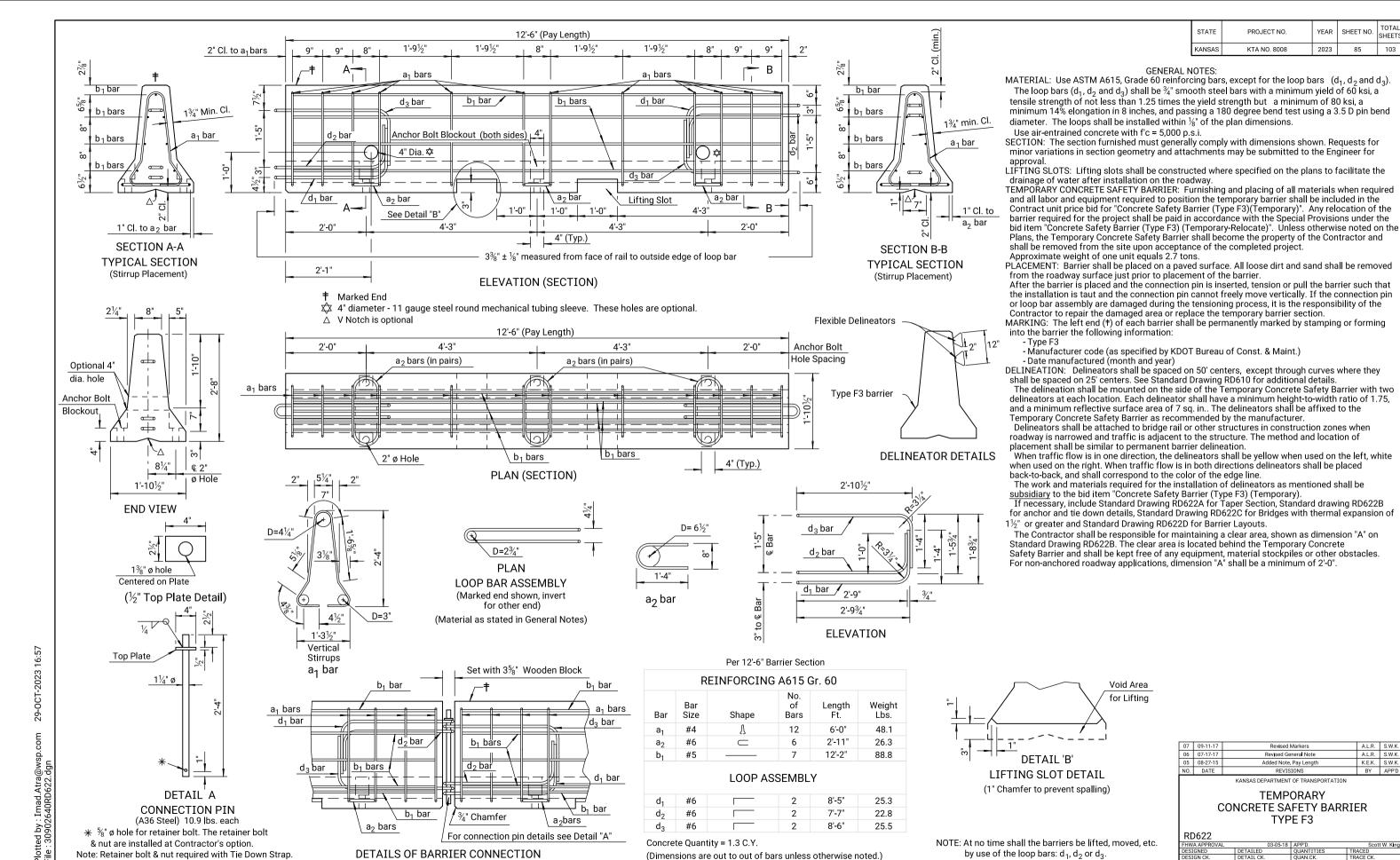
2023 84

STATE

KANSAS

PROJECT NO.

KTA NO. 8008



**GENERAL NOTES:** INSTALLATION: Holes into the pavement to anchor the concrete safety barrier may be drilled after positioning barrier. When anchoring with 3 bolts on traffic side, install barrier with through anchor bolt where possible, use grouted anchor bolts where through bolt can't be used. Do not drill into or otherwise damage support beams, girders, or expansion joints. All work & materials required for the installation of the anchors are subsidiary to the bid item "Concrete Safety Barrier"

UTILITIES & STRUCTURES (Stakes): Verify buried utilities & structures within stake depth. if conflicts between stake & buried elements exist, up to 2 stakes maximum in a single barrier may be omitted if adjacent barriers have 3 stakes each.

ANCHORAGE: Use galvanized grouted anchor bolts, through anchor bolts, nuts & washers that meet standard specifications. Install 3 anchor bolts or asphalt pins per barrier on the traffic side except on transition barrier as shown.

BARRIER REMOVAL: Completely remove all anchor systems. Remove grouted or drop-in anchor system by drilling the anchor with a core barrel 2x the diameter of the insert. Core to a depth equal to the installed depth & remove the core, prepare the hole by removing dust & debris. Fill hole with material that meets KDOT Pre-qualified "Non-shrink grouts for grouting anchor bolts & reinforcing into previously poured concrete". Follow the manufacturer's procedures for mixing, hole preparation & curing. To fill through bolt anchor or screw-in anchor system, remove & completely fill the hole using instructions for drop-in

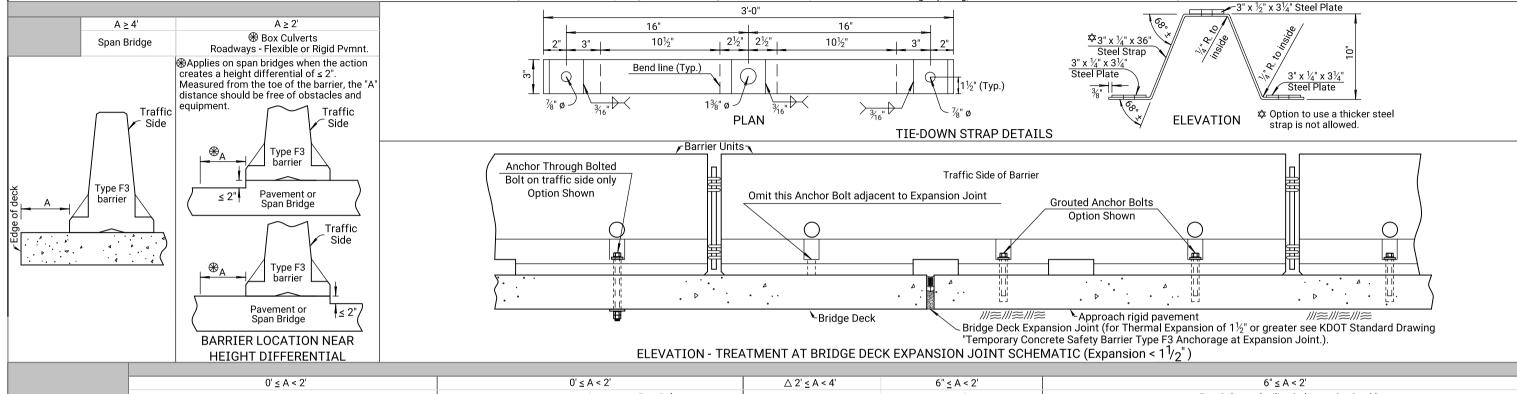
anchors except no coring is required.

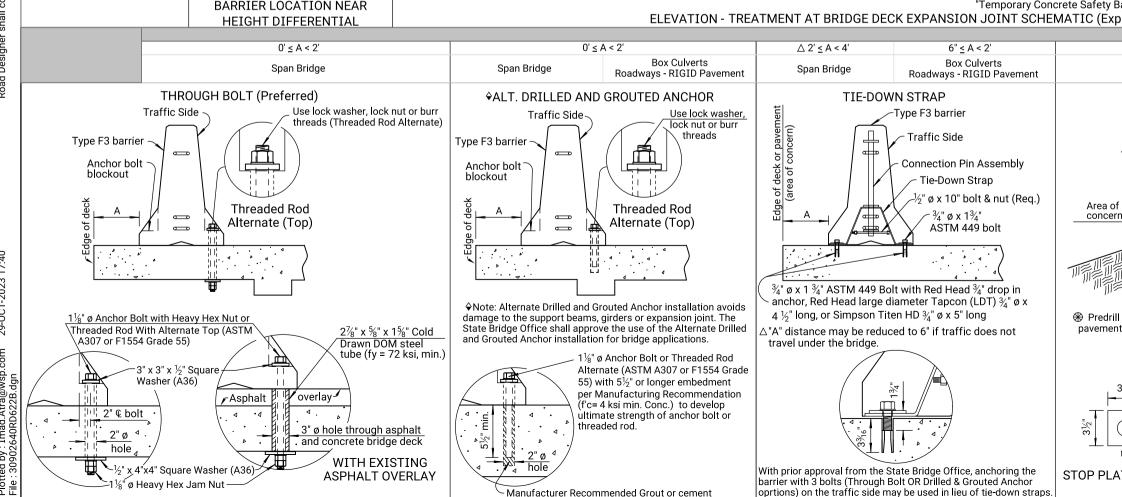
For removed or relocated barrier on flexible pavement, fill stake holes completely with hot or cold asphalt patch material. Work & materials required to remove & patch anchor holes are subsidiary to the bid item "Concrete Safety Barrier".

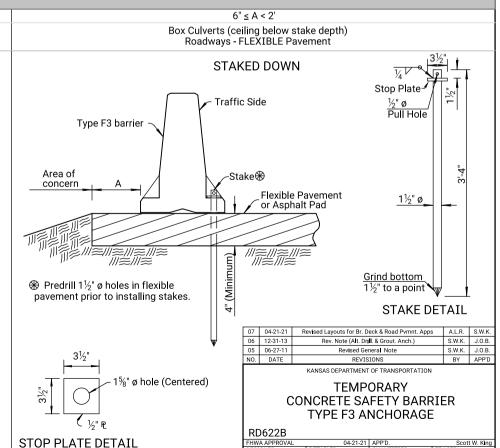
TEMPORARY BARRIERS: Temporary Barriers shown in the details of this drawing are not allowed for permanent installations.

See KDOT Standard Drawing "Temporary Concrete Safety Barrier Type F3 Transition Layouts" for transition details between anchored & free-standing barriers. See KDOT Standard Drawing "Temporary

Concrete Safety Barrier Type F3" for details & quantities not shown on this sheet. SIGNING: For sign spacing, traffic control device details & reference notes, see Index of Sheets.







YEAR SHEET NO.

86

2023

STATE

KANSAS

PROJECT NO.

KTA NO. 8008