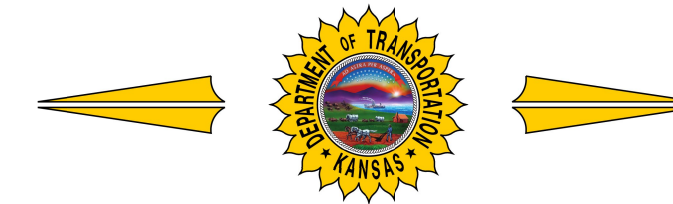


STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	01	135

KDOT PROJECT NUMBER 99-99 KA-5728-01
 FEDERAL PROJECT NUMBER BR-F-A572(801)

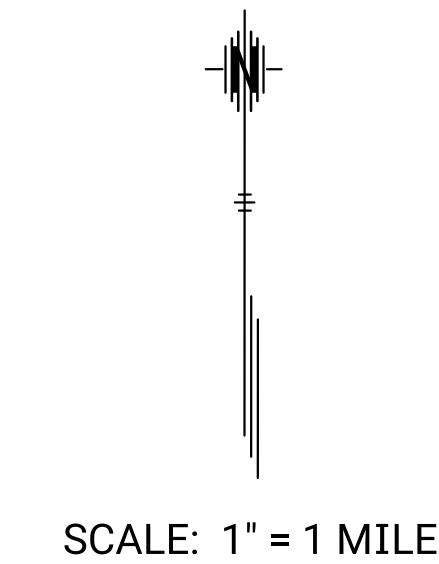
STATE OF KANSAS DEPARTMENT OF TRANSPORTATION



PLAN AND PROFILE OF PROPOSED STATE HIGHWAY

FEDERAL AID PROJECT
 WABAUNSEE COUNTY

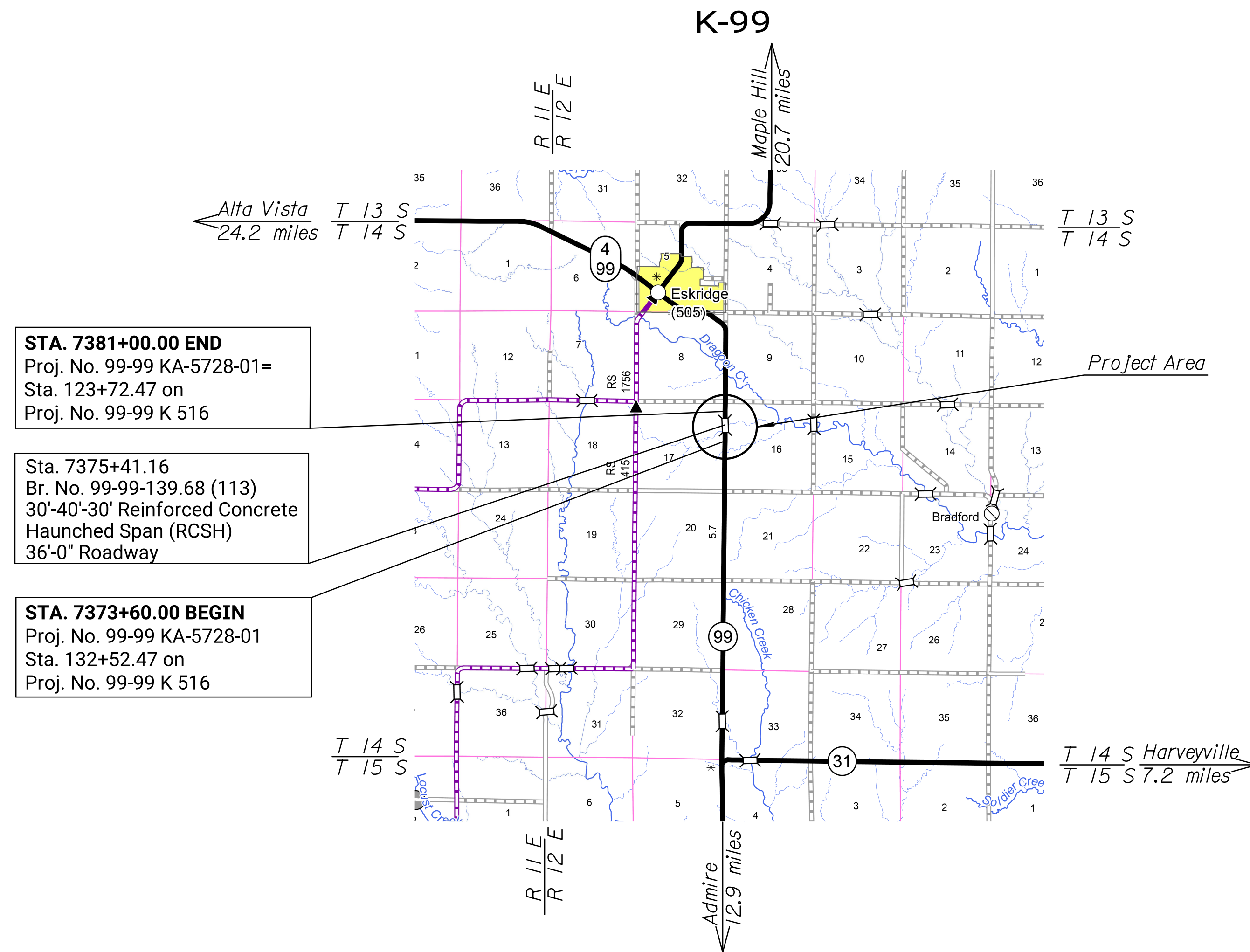
GRADING AND SURFACING (ASPHALT)
 BRIDGE
 SEEDING
 SIGNING



INDEX OF SHEETS

1	TITLE SHEET
2	SIGNATURE SEAL SHEET
3-4	TYPICAL SECTIONS
5	FOUNDATION TREATMENT & COMPACTION DETAIL
6	SALVAGED TOPSOIL
7	GENERAL NOTE
8	PLAN-PROFILE (MAINLINE)
9	PLAN-PROFILE (SHOOFLY DETOUR)
10	RIGHT OF WAY MONUMENT ENSTALLATION DETAIL
11-12	PAVEMENT DETAILS
13	SLOPE DRAIN
14-22	GUARDRAIL
23-26	END SECTIONS
27	PIPE CULVERT SUMMARY
28	DRAINAGE DATA SHEET
29-31.001	CULVERT
32-44	BRIDGE NO. 99-99-139.68 (113)
45-55	DETOUR BRIDGE
56	BRIDGE EXCAVATION
57	STANDARD PILE DETAILS
58	SUPPORTS AND SPACERS FOR REINFORCING STEEL
59-61	CHANNEL REALIGNMENT
62-63	SUMMARY OF QUNATITIES
64	PROJECT SURFACING
65-75	TEMPORARY EROSION AND POLLUTION CONTROL
76	SEEDING
77-84	SIGNING
85	CONSTRUCTION SEQUENCING
86-95	TRAFFIC CONTROL
96-129	CROSS SECTIONS (K-99)
130-135	CROSS SECTIONS (CHANNEL REALIGNMENT)

DATE	2020
DATE	10/2023
DATE	10/2023
DATE	10/2023
BY	J. Bowen
SURVEY	M. R. Remboldt / D. Kahle
CADD TECHNICIAN	A. Zuno-KDOT (Road) / K. Moore-HDR (Bridge)
DESIGNERS	S. Bass (Road) / B. Rognie (Bridge)
SQUAD	



NOTE: Traffic to be carried around construction on shoofly detour.

DESIGN DESIGNATION

AADT (2024) = 750
 AADT (2044) = 850
 DHV = 11%
 D = 65%
 T = 14%
 V = 65 mph
 C of A = None
 Clear Zone = 26 ft.

CONVENTIONAL SIGNS

COUNTY LINE	-----	CENTER LINE OF PROJECT	-----
CITY LIMITS	-----	TERRACE	-----
STATE OR NATIONAL LINE	-----	CULVERTS	-----
TOWNSHIP, SECTION or GRANT LINE	-----	DROP INLET & STORM SEWER	-----
PROPERTY LINE	-----	ACCESS CONTROL	-----
HIGHWAY FENCE	-----	POWER POLE	-----
EXISTING FENCE	-----	TELEPHONE POLE	-----
GUARDRAIL	-----	MARSH	-----
CONSTRUCTION LIMITS	-----	HEDGE	-----
RIGHT OF WAY LINE	-----	TREES	-----
TRAVELED WAY	-----	PROFILE ELEVATION	-----
RAILROADS	-----	STREAM or CREEK	-----

GROSS LENGTH OF PROJECT	740.00	FT. (Includes Equations)
EXCEPTIONS	0.00	FT.
NET LENGTH OF PROJECT	740.00	FT. 0.140 MILES
NET LENGTH OF BRIDGES	102.50	FT. 0.019 MILES
NET LENGTH OF ROAD	637.50	FT. 0.121 MILES

Approved _____ Date _____

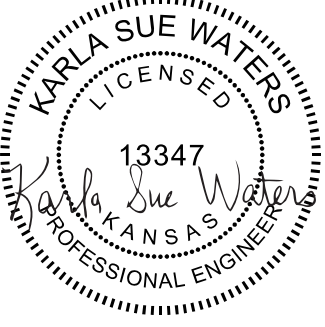

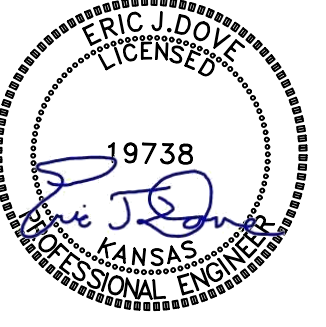
State Transportation Engineer

By: _____

Chief, Bureau of Road Design

KANSAS DEPARTMENT OF TRANSPORTATION

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	02	135

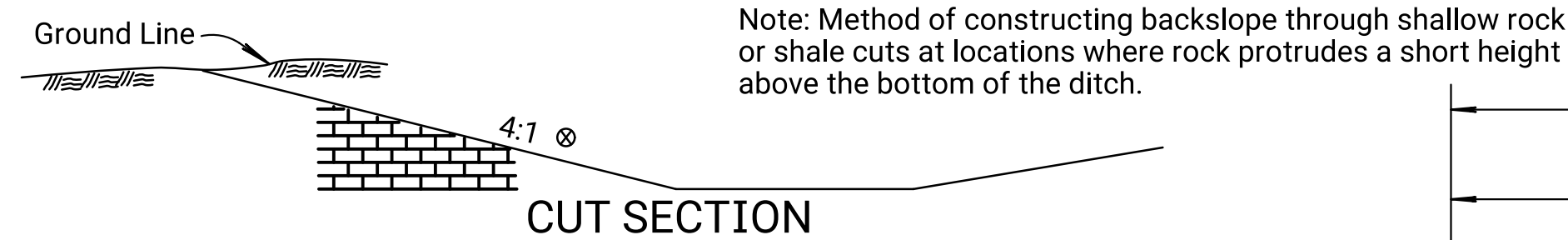
 Jan 11, 2024 Name: Sara Peters Co. Name: KDOT - Traffic Plan Section: Permanent Signing	 Jan 11, 2024 Name: Karla Waters Co. Name: Schwab Eaton Plan Section: Traffic	 Jan 11, 2024 Name: Thaddeus Kosmicki Co. Name: HDR Engineering Plan Section: Bridge	 Jan 11, 2024 Name: Eric Dove Co. Name: HDR Engineering Plan Section: Channel Re-alignment				

Plotted by : August Zuno 1-NOV-2023 16:08
 File : rss048.dgn

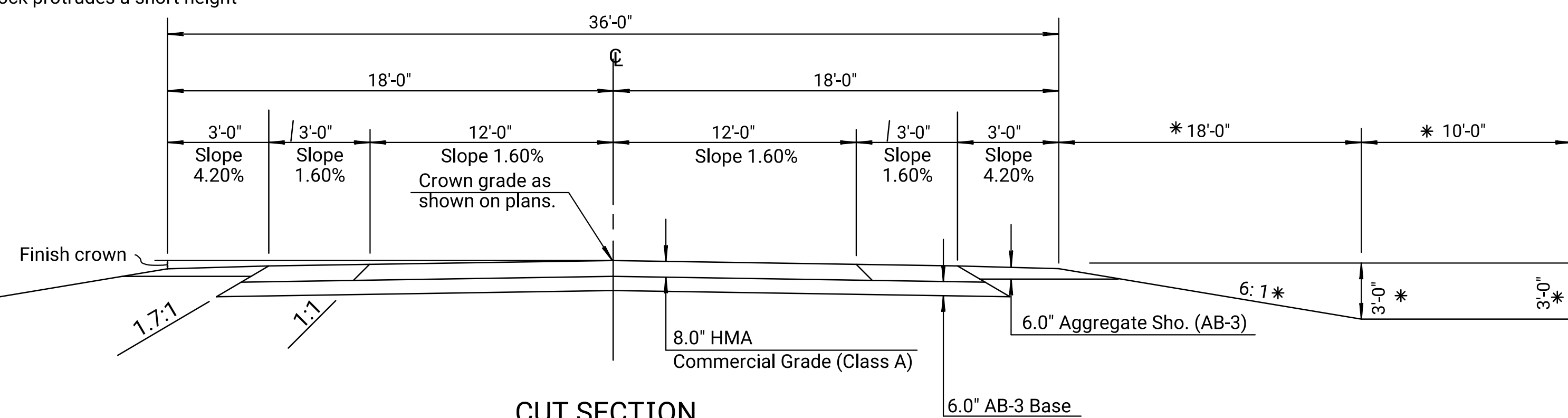
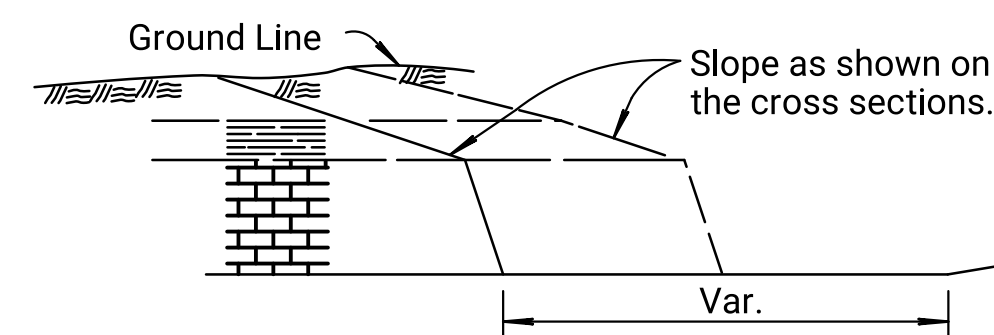
01	01-24-18	Initial Release	A.L.R.	S.W.K.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
Signature Seal Sheet				
RD048				
FHWA APPROVAL		APPD.		
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	
		Scott W. King		

KDOT Graphics Certified

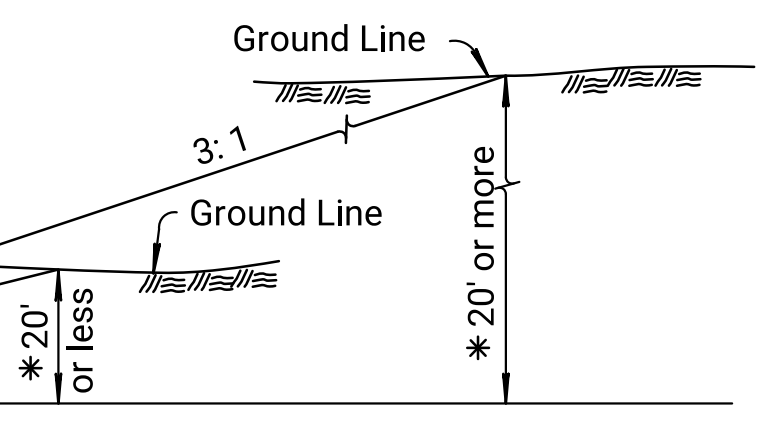
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	03	135



⊗ Slope to be 3: 1 when depth of cut is 20' or more.



CUT SECTION
K-99



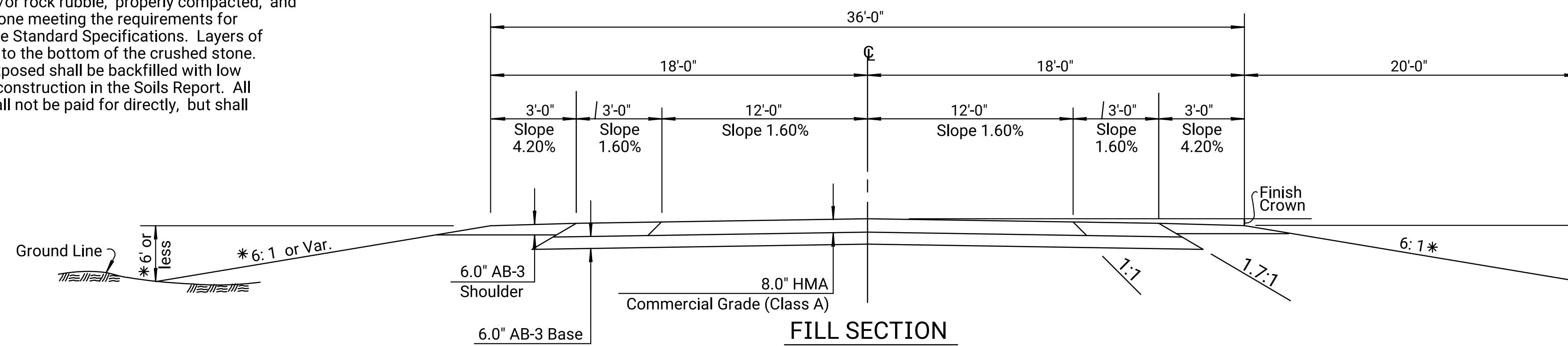
The asphalt portion of the shoulder shall be constructed in conjunction with the roadway surfacing.

* Dimensions and slopes for standard ditches and fills. See plan and cross-sections for variations.

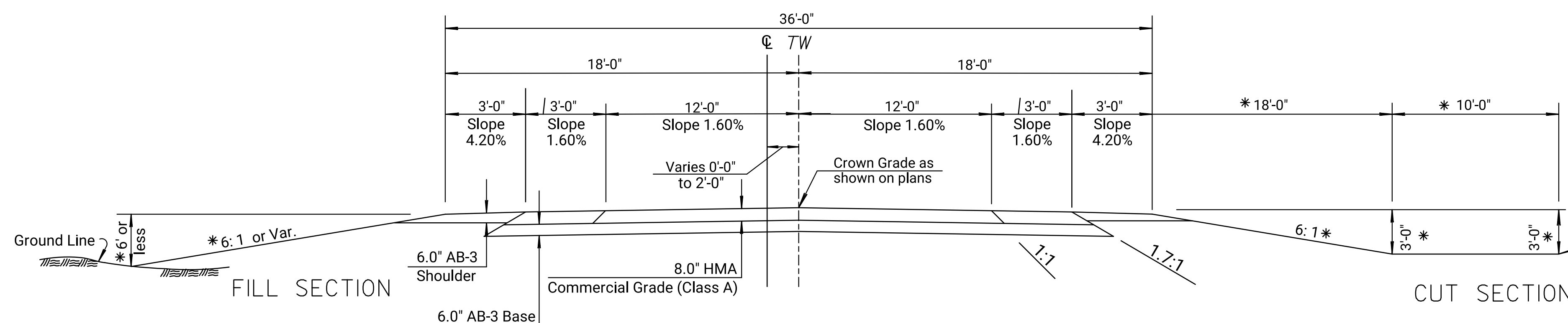
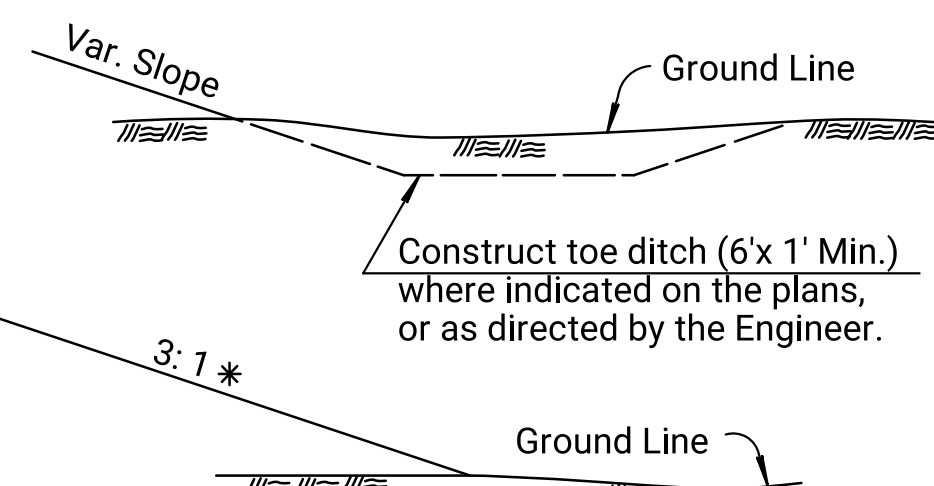
Note: Intersection of all slope lines shall be softened and rounded for pleasing appearance.

Ditch Plugs within the appropriate clear zone shall have side slopes of 10:1 or flatter.

** Overbreakage in limestone or sandstone rock shall be brought to within 8" of the subgrade line with crushed stone, shot rock, and/or rock rubble, properly compacted, and then brought to the subgrade line with crushed stone meeting the requirements for "Crushed Stone for Backfill" in accordance with the Standard Specifications. Layers of earth or shale will not be permitted for backfill up to the bottom of the crushed stone. Overbreakage in shale or in rock where shale is exposed shall be backfilled with low permeability soils listed as useable for subgrade construction in the Soils Report. All materials, equipment, and labor for this work shall not be paid for directly, but shall be subsidiary to other items of the contract.

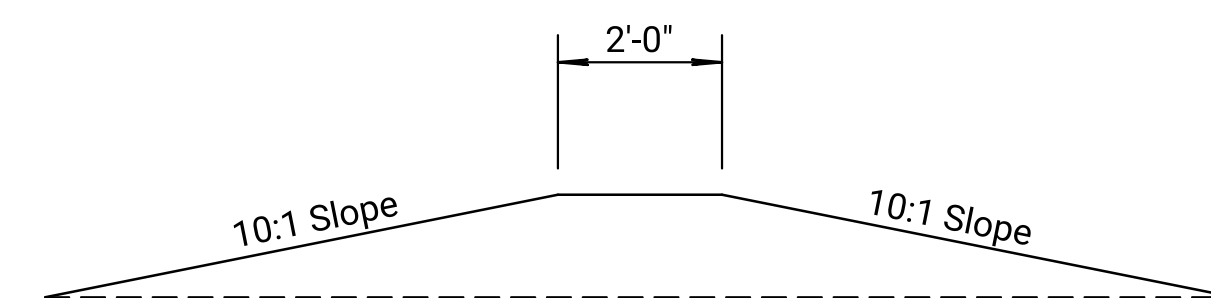


FILL SECTION
K-99

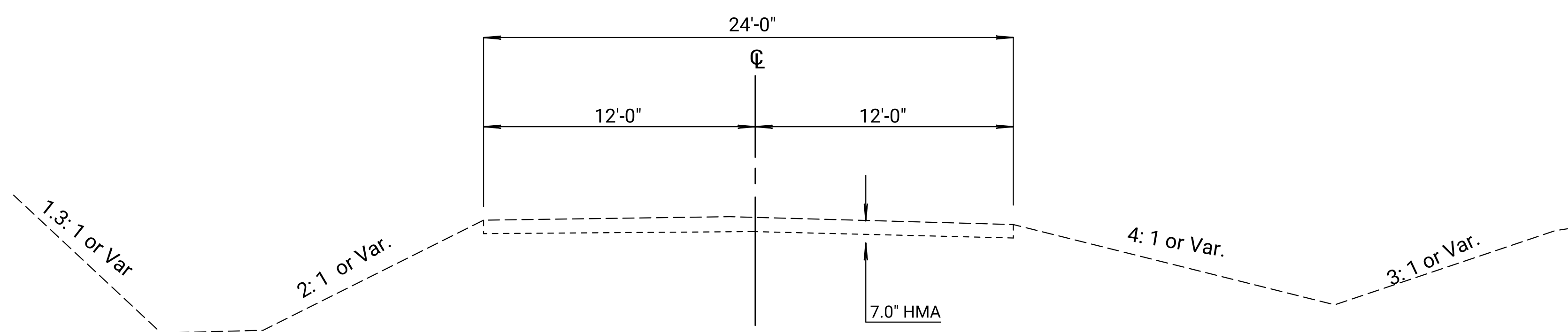


K-99 TW SECTION
STA. 7372+20 to STA. 7373+60

CUT SECTION



SKETCH OF DITCH PLUG



EXISTING TYPICAL SECTION
FOR INFORMATION ONLY

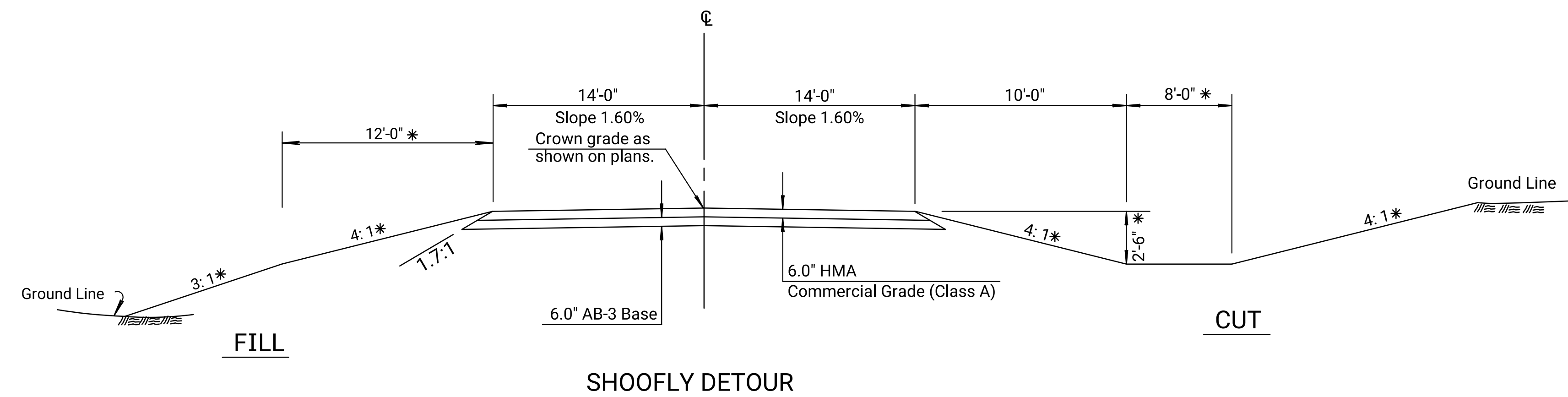
NO.	DATE	REVISIONS	BY	APP'D
21	1-25-13	Removed Slope, Pvm. Edge	S.W.K.	J.O.B.
20	5-20-09	8:1/6:1 over 10' fill mound ent./sd.rd.	S.W.K.	J.O.B.
19	11-10-04	Changed slope labels to percent	S.W.K.	J.O.B.
18	5-10-00	Rev. Ditch Plug Slope 10:1	R.J.S.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

TYPICAL SECTION
GRADING & SURFACING

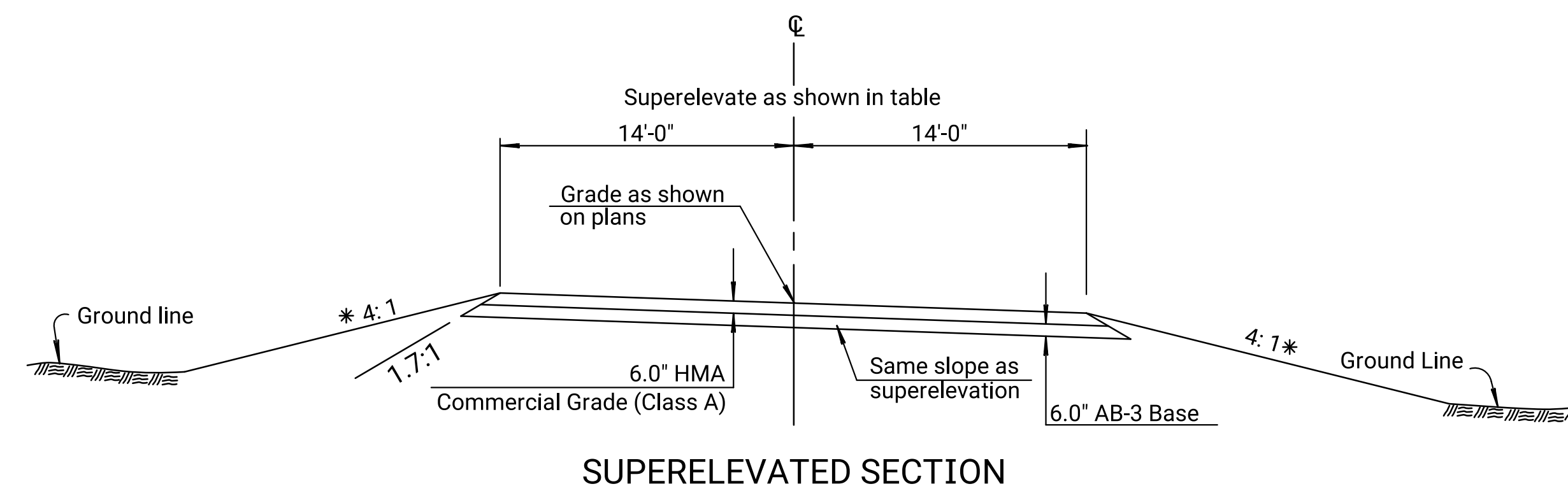
DESIGNED	5-21-2013	APP'D. James O. Brewer
DESIGN CK.	DETAIL CK.	QUAN. CK.
TRACED B.N.B.	TRACE CK. W.L.H.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	04	135

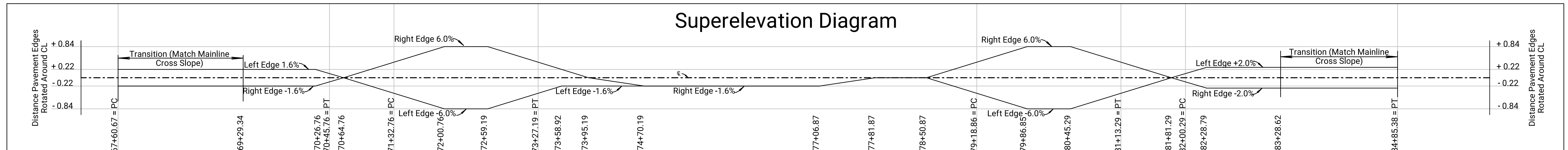


* Dimensions and slopes for standard ditches and fills. See plan and cross-sections for variations.

Note: Intersection of all slope lines shall be softened and rounded for pleasing appearance.



SUPERELEVATED SECTION



Sta. P.I. Curve	Radius	Super %	Transition - (Lin.Ft.)		
			L	A	B
Sta. 69+07.21	500'	R.C.			
Sta. 72+32.70	341'	6	For Superelevation transition, see diagram above.		
Sta. 80+18.80	341'	6			
Sta. 83+46.83	500'	R.C.			

NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

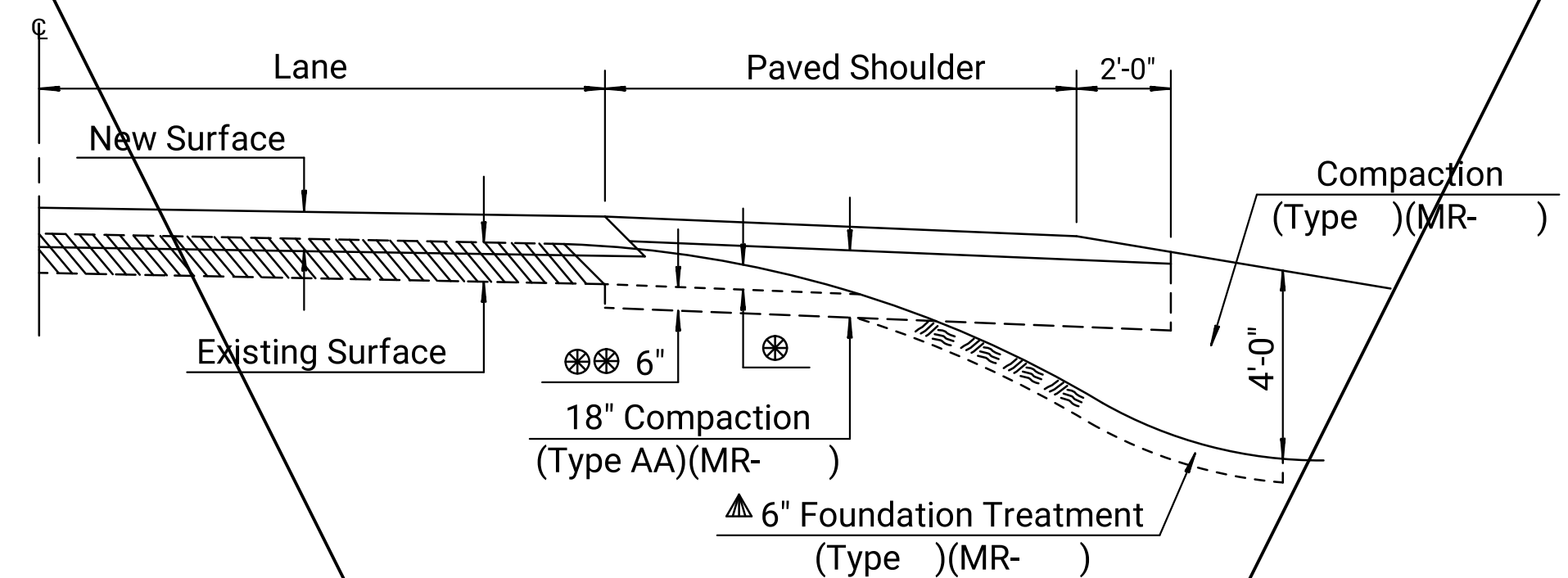
**TYPICAL SECTION
SHOOFLY DETOUR**

RD600

FHWA APPROVAL	5-21-2013	APP'D. James O. Brewer
DESIGNED	DETAILED	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.
		TRACE CK. W.L.H.

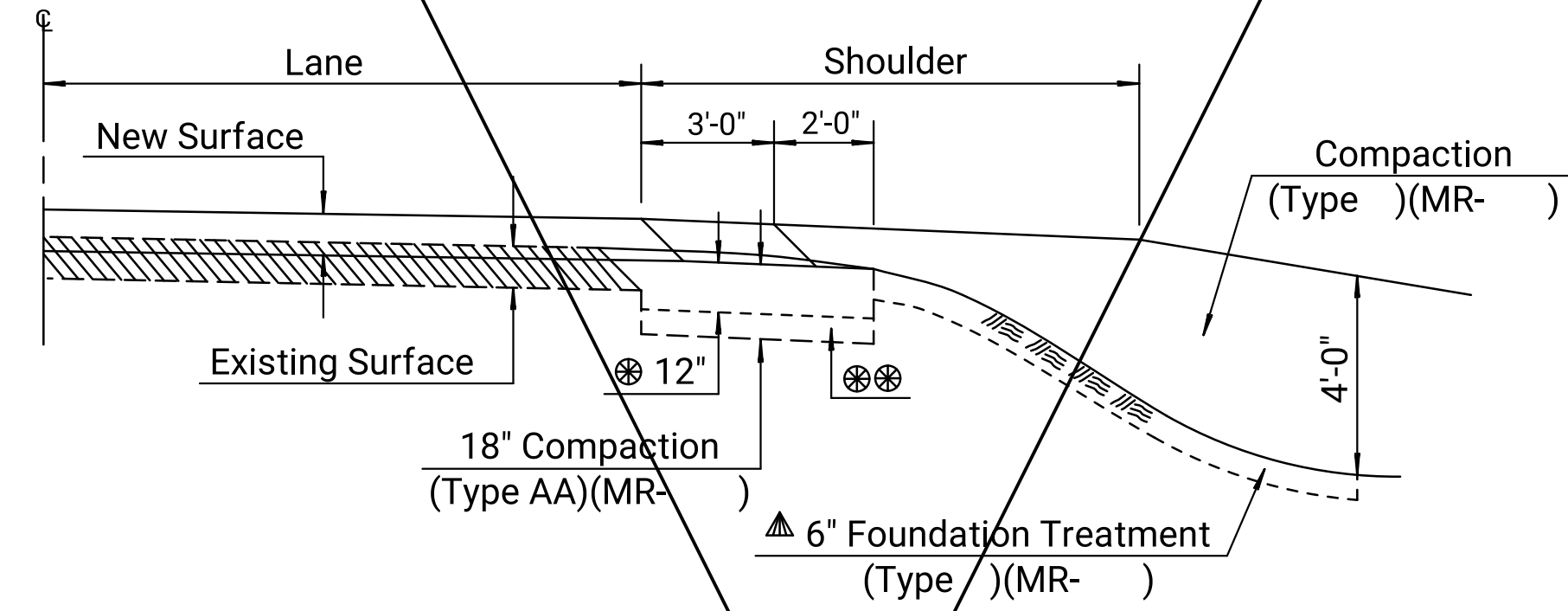
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	05	135

REHABILITATION



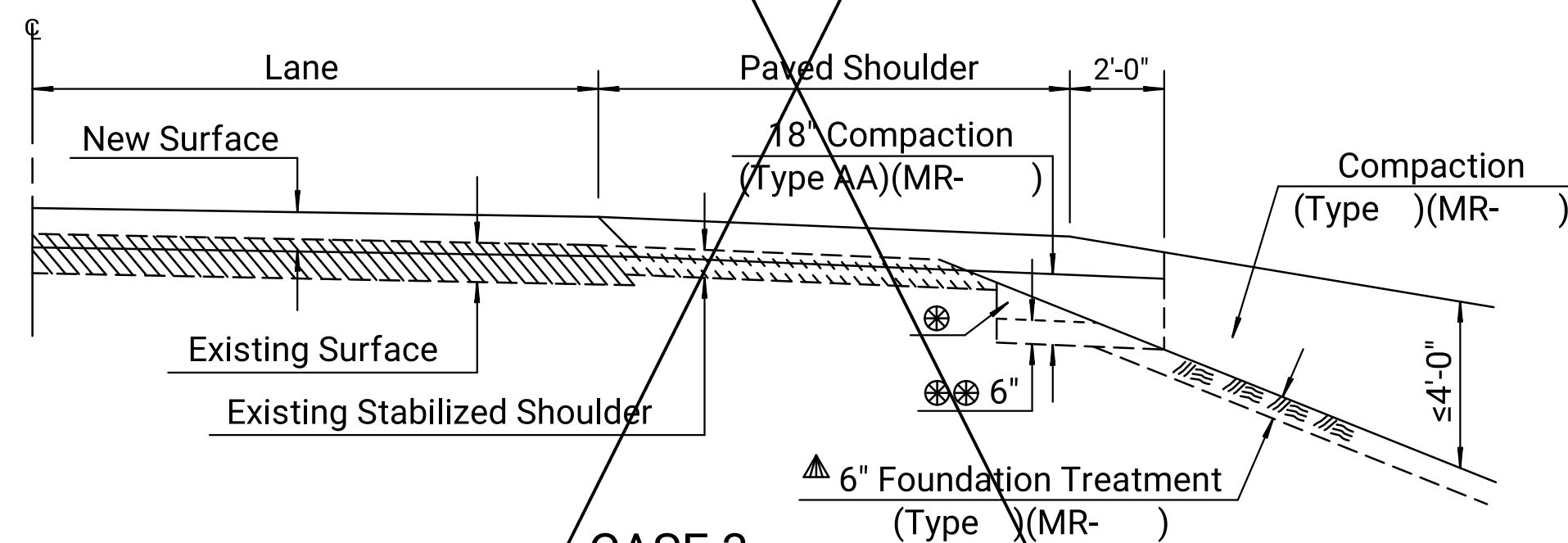
CASE 1

Overlay with Paved Shoulder



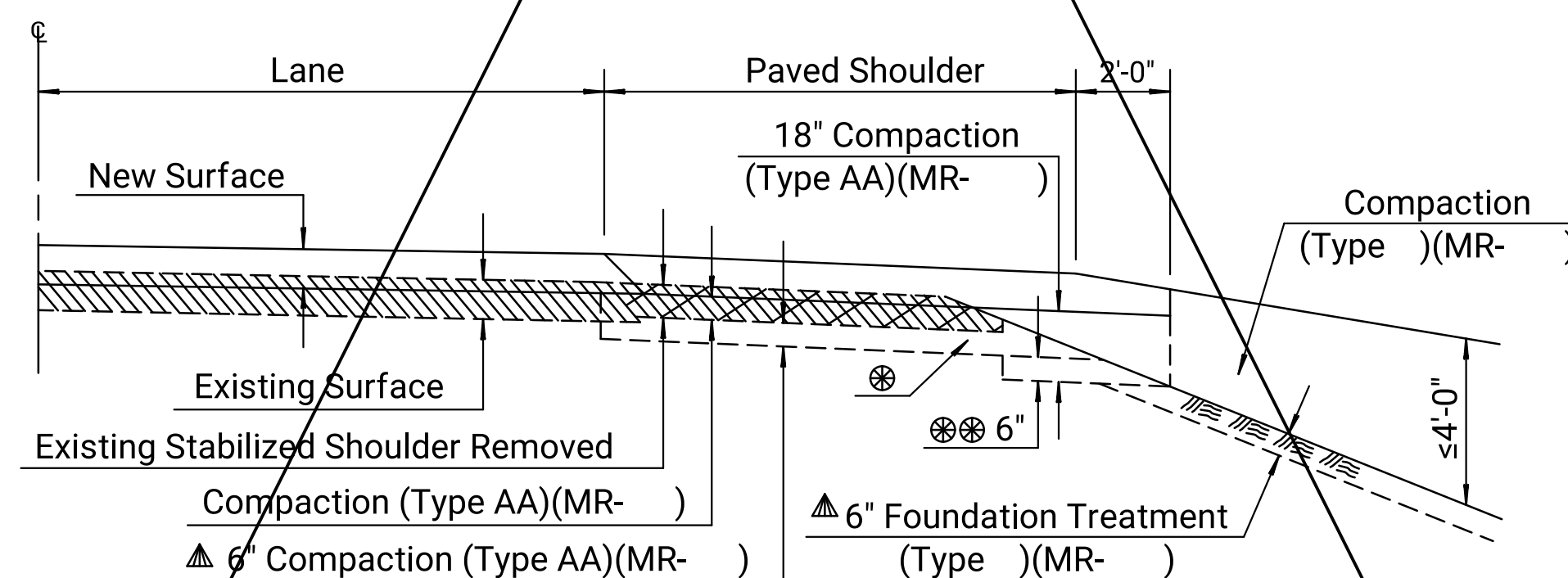
CASE 2

Overlay with Composite Shoulder



CASE 3

Overlay with Existing Paved Shoulder



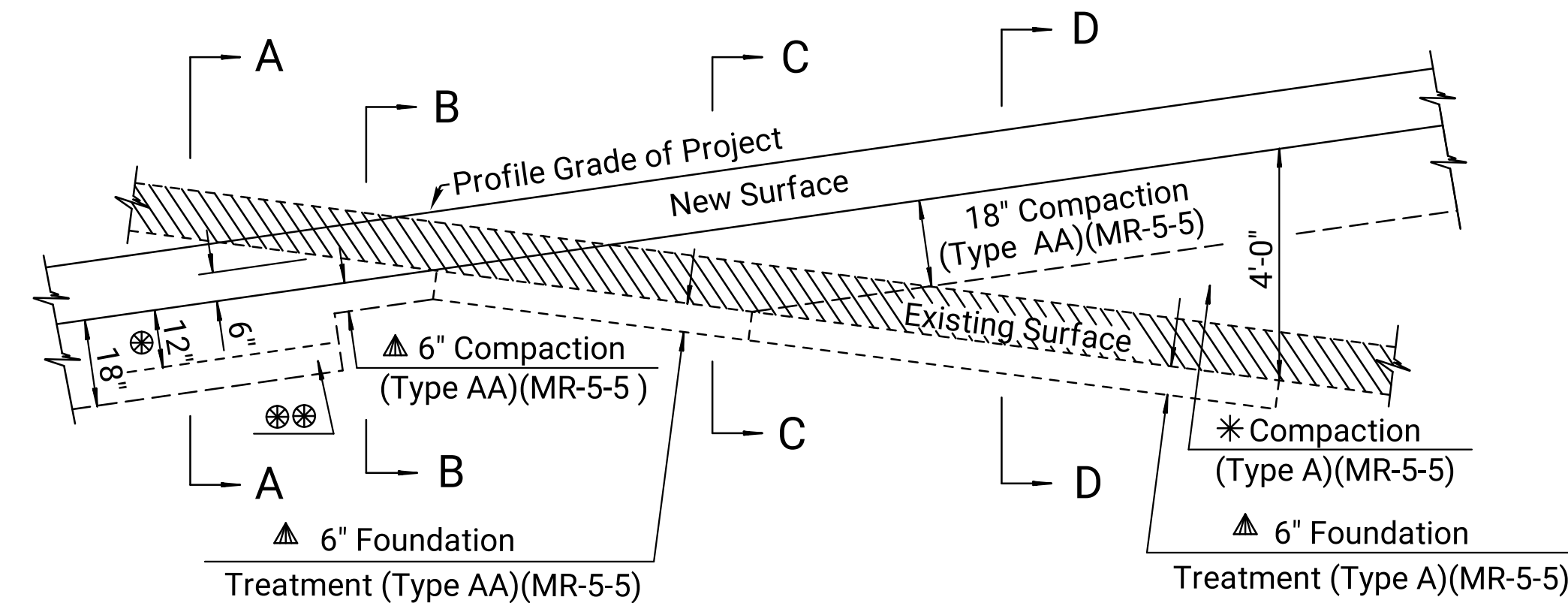
CASE 4

Overlay with Shoulder Replacement

- ⊗ Excavation thru Cuts not Subgraded
- ⊗⊗ The lower 6" of Compaction is subsidiary.
- ▲ Compaction of this material shall be subsidiary.

Note: These are 4 general cases. Specific compaction requirements are determined on a project-by-project basis.

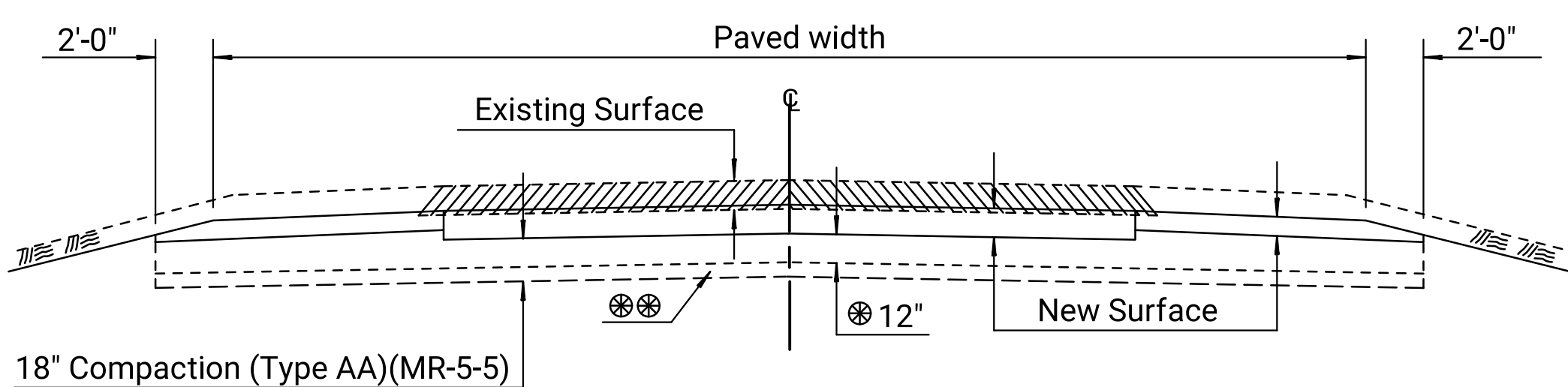
RECONSTRUCTION



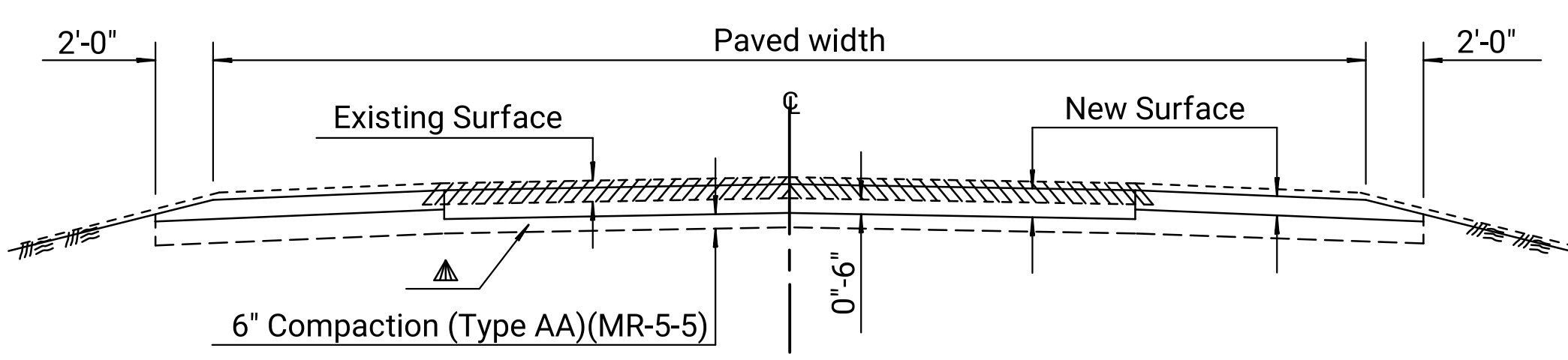
PROFILE

- ⊗ Excavation thru Cuts not Subgraded
- ⊗⊗ The lower 6" of Compaction is subsidiary.
- ▲ Compaction of this material shall be subsidiary.

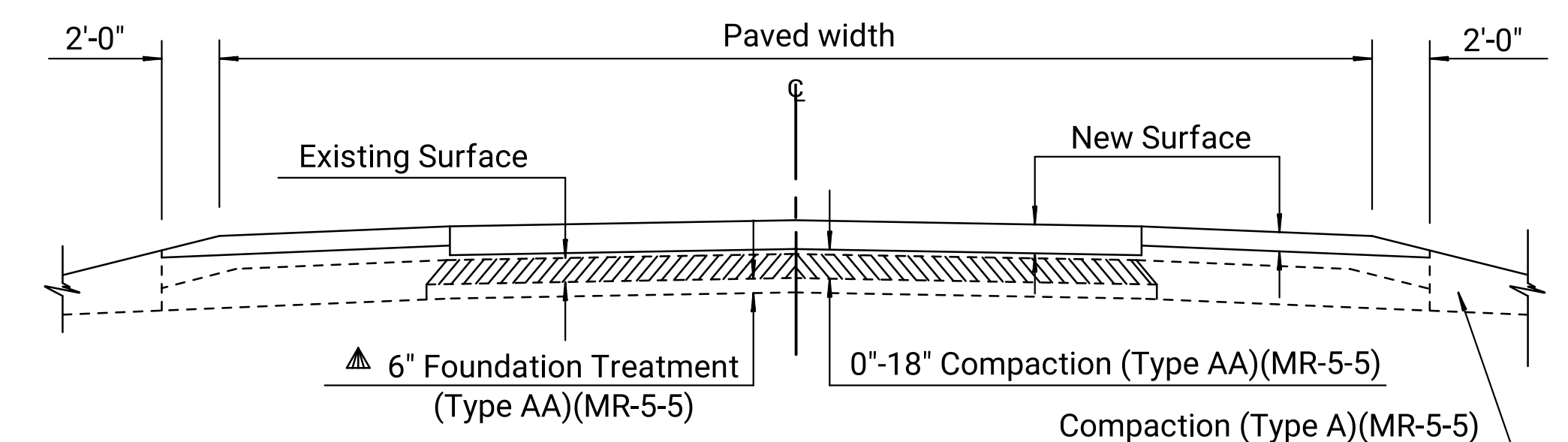
*The compaction requirements for all embankment fill placed between Station 456+44 to 463+37 shall meet Type AA compaction standards, with MR 5-5 moisture control.



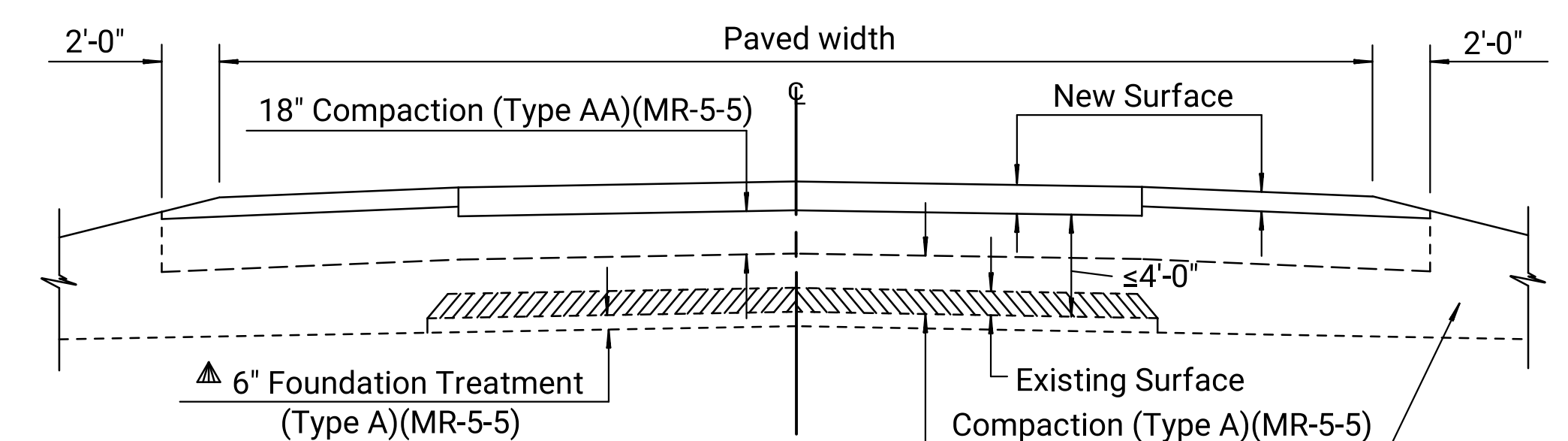
SECTION A-A



SECTION B-B

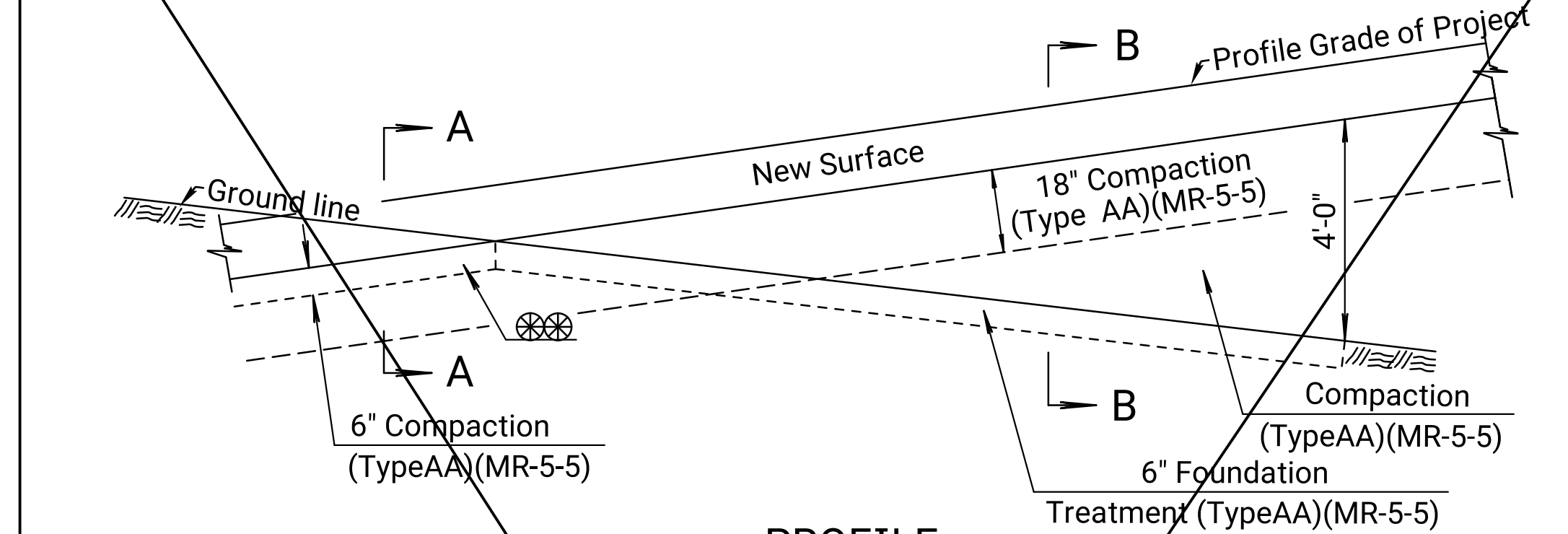


SECTION C-C



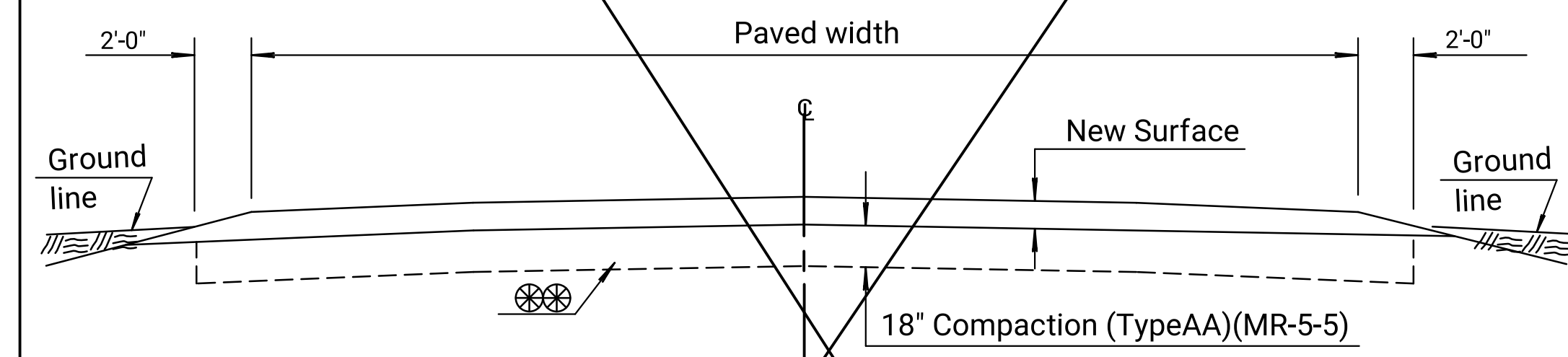
SECTION D-D

SHOOFLY DETOUR

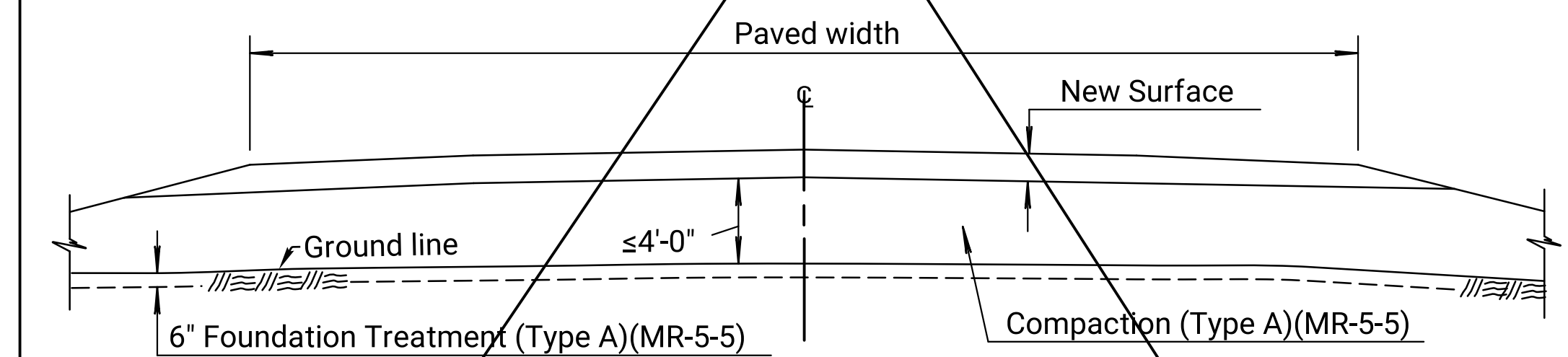


PROFILE

- ⊗⊗ The lower 6" of Compaction is subsidiary.



SECTION A-A



SECTION B-B

General Note

For materials designated to be subgraded, compaction of soils, including shales, designated for backfill refer to Standard Drawing RD605A for details.

Unless otherwise noted on the Plans, compact all embankment, including side roads and entrances.

All compaction under the shoofly will be (Type A) (MR-5-5)

NO.	DATE	REVISIONS	BY	APPD
5	10-17-11	Revised General Note	S.W.K.	J.O.B.
4	1-05-10	Added additional subsidiary comp.	S.W.K.	J.O.B.
3	2-16-05	Redrawn, Rev. Recon. Sec. C-C & D-D	S.W.K.	J.O.B.
2	5-29-98	Revised Reconstruction Section B-B	R.J.S.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

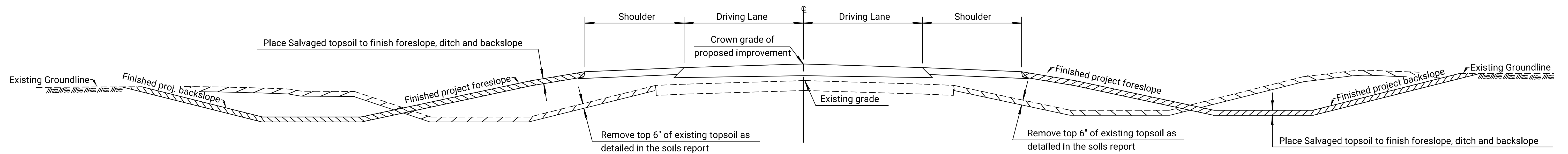
FOUNDATION TREATMENT & COMPACTION OF EARTHWORK

DESIGNED	QUANTITIES	TRACED
APPD. James O. Brewer	QUAN. CK.	TRACE CK. King

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	06	135

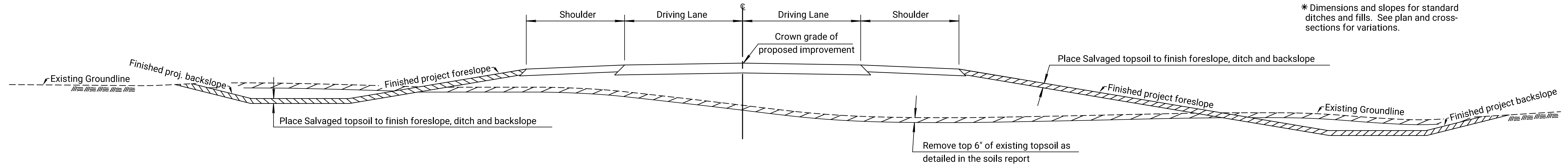
Note to Designer: Acceptable Topsoil locations on a project will be detailed in the Soils Report. The locations will be used in conjunction with the plans to measure a horizontal area in Sq. Yds of "Salvaged Topsoil" within the R/W limits.

GENERAL NOTE
 Adjust the cut and fill sections to accommodate the placement of the salvaged topsoil such that after placement the cross section will be at the final grade as shown on the plans.
 Salvaging, Stockpiling and Placing Topsoil bid as "Salvaged Topsoil" in Square Yards. See KDOT Standard Specifications for details.
 Soften and round the intersection of all slope lines for pleasing appearance.

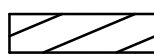
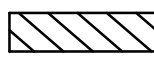


RECONSTRUCTION/REHABILITATION OF EXISTING ROADWAY
 (Removal and Placement of Salvaged Topsoil)

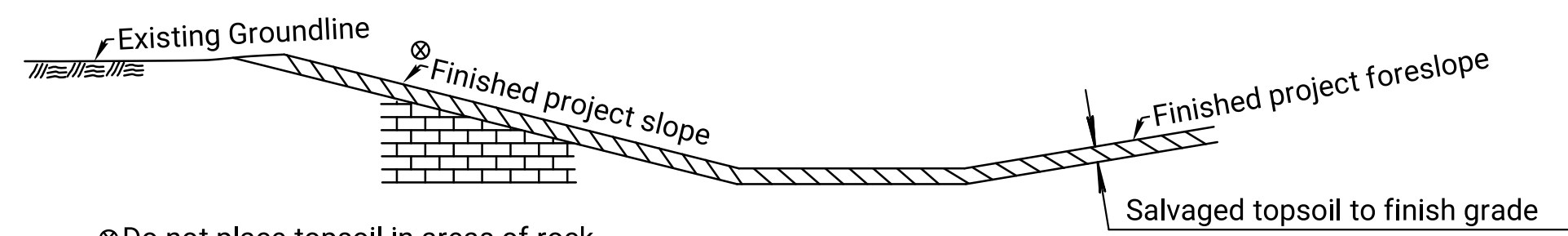
* Dimensions and slopes for standard ditches and fills. See plan and cross-sections for variations.



NEW ROADWAY ALIGNMENT
 (Removal and Placement of Salvaged Topsoil)

LEGEND
 Topsoil to be Salvaged
 Placement of Salvaged Topsoil

Note: Method of showing backslope thru shallow rock or shale cuts at locations where rock protrudes a short height above the bottom of the ditch.



CUT SECTION

⊗ Do not place topsoil in areas of rock that are a 3:1 or steeper slope.

01	12-16-09	Initial Release		S.W.K.	J.O.B.
NO.	DATE	REVISIONS		BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION					
SALVAGED TOPSOIL					
RD599A					
FHWA APPROVAL		APPD.		James O. Brewer	
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	S.W.K.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA5728-01	2023	07	135

GENERAL NOTE

THE GEOLOGICAL INFORMATION SHOWN ON THESE PLANS IS FROM STUDIES MADE IN THE FIELD AND REPRESENTS THE BEST INFORMATION AVAILABLE TO THE KANSAS DEPARTMENT OF TRANSPORTATION.

SOIL FOR EMBANKMENT: ALL SOIL USED IN THE TOP 18 INCHES OF THE EMBANKMENT SHALL CONFORM TO THE FOLLOWING REQUIREMENTS: 10 ≤ PI ≤ 35 AND 20 ≤ LL ≤ 55. SOILS WHICH CONTAIN SUBSTANTIAL ORGANIC MATERIAL, SUCH AS THOSE CLASSIFIED AS OL OR OH ACCORDING TO THE UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D2487) SHOULD NOT BE USED TO CONSTRUCT THE EMBANKMENT OR SUBGRADE, THE ORGANIC MATERIAL MAY BE USED AS SELECT SOIL TO CAP THE SIDESLOPES OF THE EMBANKMENT.

A GROSS VMF OF 0.78 WAS USED TO COMPUTE EARTHWORK QUANTITIES FOR THIS PROJECT. THIS FACTOR INCLUDES QUANTITIES FOR INITIAL CONSOLIDATION AND SETTLEMENT.

ALL BORROW AREA LOCATIONS ADJACENT TO THE RIGHT-OF-WAY, UTILITY POLES MAY BE SET AT THE PERMANENT LOCATIONS PRIOR TO CONSTRUCTION AS APPROVED BY THE ENGINEER PROVIDED A MINIMUM VERTICAL CLEARANCE, IN ACCORDANCE WITH THE NATIONAL ELECTRICAL SAFETY CODE, IS OBTAINED. THE CONTRACTOR WILL BE REQUIRED TO WORK AROUND THESE POLES TO COMPLETE THE WORK.

ALL BORROW TO BE OBTAINED FROM AREAS PROVIDED BY THE CONTRACTOR SHALL BE APPROVED BY THE ENGINEER, BOTH AS TO SUITABILITY OF MATERIAL AND SITE LOCATION. LOCATIONS WHICH, IN THE OPINION OF THE ENGINEER, CONTAIN UNSUITABLE MATERIAL OR WILL LEAVE AN UNSIGHTLY APPEARANCE ON THE PROJECT WILL NOT BE APPROVED.

ALL BORROW/WASTE LOCATIONS SHALL BE SUBMITTED FOR CLEARANCE FROM THE KANSAS HISTORICAL SOCIETY AND THE KANSAS DEPARTMENT OF WILDLIFE AND PARKS PRIOR TO ANY EXCAVATION OR WASTING OF MATERIAL. THE CONTRACTOR IS RESPONSIBLE FOR ACQUIRING ANY PERMITS AND OTHER CLEARANCES THAT ARE REQUIRED.

EXCAVATION SHOWN TO BE WASTED SHALL BE WASTED ON SITES PROVIDED BY THE CONTRACTOR. THESE SITES SHALL BE APPROVED BY THE ENGINEER AS TO SUITABILITY, APPEARANCE, AND SITE LOCATION. LOCATIONS THAT, IN THE OPINION OF THE ENGINEER, WILL LEAVE AN UNSIGHTLY APPEARANCE WILL NOT BE APPROVED.

CHANNELS SHALL BE CUT AT BOX CULVERTS (UNLESS OTHERWISE NOTED) TO FLOW LINE ELEVATIONS AND TO A WIDTH OF ONE FOOT OUTSIDE OF EACH OUTSIDE WALL AND WITH SLOPES 2 TO 1 PRIOR TO CONSTRUCTION OF THE CULVERT.

THE GEOLOGICAL INFORMATION SHOWN ON THESE PLANS IS FROM STUDIES MADE IN THE FIELD AND REPRESENTS THE BEST INFORMATION AVAILABLE TO THE KANSAS DEPARTMENT OF TRANSPORTATION.

ALL TREES, HEDGE ROWS, SHELTER BELTS, AND WOODY SHRUBS NOT SHOWN TO BE REMOVED AND LOCATED BETWEEN THE CONSTRUCTION LIMITS AND THE RIGHT-OF-WAY LINE OR EASEMENT LINE SHALL BE SPARED UNLESS DIRECTED BY THE ENGINEER TO BE REMOVED. ALL TREES WITHIN THE APPROPRIATE CLEAR ZONE SHALL BE REMOVED.

INFORMATION SHOWN IN THE PLANS CONCERNING TYPE AND LOCATION OF UNDERGROUND UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL INCLUSIVE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE TYPE AND LOCATION OF UNDERGROUND UTILITIES AS MAY BE NECESSARY TO AVOID DAMAGES THERETO. ALL KDOT OWNED UTILITIES ARE TO BE LOCATED BY THE CONTRACTOR.

ALL SAW CUTS SHALL BE FULL DEPTH OR AS APPROVED BY THE ENGINEER AND SHALL NOT BE PAID FOR DIRECTLY, BUT SHALL BE SUBSIDIARY TO OTHER BID ITEMS IN THE CONTRACT.

CONTRACTOR TO MAINTAIN ACCESS TO PROPERTY OWNERS AT ALL TIMES AS DIRECTED BY THE ENGINEER.

ADDITIONAL SECTION CORNER REFERENCES AND TIES ARE AVAILABLE UPON REQUEST.

IF THE EXISTING FENCE IS DISTURBED, INCLUDING AT THE DRAINAGE STRUCTURE CONNECTION, IT SHALL BE REPLACED OR RESET AS DETERMINED BY THE ENGINEER. UNLESS OTHERWISE NOTED IN THE PLANS, THIS WORK SHALL BE SUBSIDIARY TO OTHER ITEMS IN THE CONTRACT.

THE ROCA SHALE FORMATION FOUND IN THE PROJECT AREA, WILL BE CLASSIFIED AS NON-DURABLE SHALE. THE NON-DURABLE SHALES SHALL BE MANIPULATED WITH EQUIPMENT AS DIRECTED IN THE STANDARD SPECIFICATIONS.

THE TEMPORARY GUARDRAIL (INCLUDING ALL HARDWARE AND ACCESSORIES) SHALL BECOME THE PROPERTY OF KDOT. THIS TEMPORARY GUARDRAIL SHALL BE STOCKPILED WITHIN THE RIGHT-OF-WAY FOR REMOVAL BY KDOT FORCES.

THE EXISTING K-99 PAVEMENT, PIPE, AND GUARDRAIL TO BE REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND REMOVED FROM THE SITE.

THE TEMPORARY SHOOFLY PAVEMENT SHALL BE MILLED FULL DEPTH AND WILL BECOME THE PROPERTY OF KDOT. THE MILLINGS SHALL BE HAULED BY THE CONTRACTOR TO THE KDOT SUBAREA OFFICE IN ESKRIDGE, LOCATED 1.3 MILES NORTH OF THE PROJECT. TRANSPORTING OF THIS MATERIAL SHALL BE BID AS "TRANSPORTING SALVAGEABLE MATERIAL." TRANSPORTING THIS MATERIAL WILL BE A FEDERALLY NON-PARTICIPATING BID ITEM.

1,000 CUBIC YARDS OF OF EXCESS SOIL MATERIAL FROM THE REMOVAL OF THE SHOOFLY DETOUR SHALL BECOME THE PROPERTY OF KDOT. THIS MATERIAL SHALL BE HAULED BY THE CONTRACTOR TO THE KDOT SUBAREA OFFICE IN ESKRIDGE, LOCATED 1.3 MILES NORTH OF THE PROJECT. TRANSPORTING THIS MATERIAL WILL BE PAID FOR AS "TRANSPORTING SALVAGEABLE MATERIAL." TRANSPORTING THIS MATERIAL WILL BE A FEDERALLY NON-PARTICIPATING BID ITEM. THE REMAINING EXCESS SOIL SHALL BECOME PROPERTY OF THE THE CONTRACTOR AND REMOVED FROM THE SITE.

CONCRETE BLOCKS FROM THE EXISTING BOX STRUCTURE WING EXTENSIONS WILL BECOME THE PROPERTY OF KDOT AND STOCKPILED WITHIN THE PROJECT RIGHT-OF-WAY AS DIRECTED BY THE ENGINEER. ITEMS WILL BE TRANSPORTED BY KDOT FORCES TO THE KDOT SUBAREA OFFICE IN ESKRIDGE, LOCATED 1.3 MILES NORTH OF THE PROJECT. THE CONTRACTOR WILL COORDINATE WITH KDOT SUBAREA OFFICE IN ESKRIDGE TO LOAD THE BLOCKS ONTO KDOT TRAILER TO BE HAULED AWAY BY KDOT FORCES.

01	06-06-23	Initial Release	A.L.R.	D.D.T.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
General Note Sheet				
RD040				
FHWA APPROVAL		APPD		
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	08	135

P.I. Sta. 7392+27.09 = NW Cor. Sec. 16, T14S, R12E = Sta. 112 +41.40 on Proj. 99-99 K 516 (Dated 1928)
 N 481,632.885 E 8,615,436.237
 1. Found $\frac{3}{8}$ " rebar (0.3' deep)
 2. Found nail & wschr. NW side cor. fc. po. 54.5' SSW
 3. Found nail & wschr. top cor. fc. po. 47.7' NW
 4. Top steel cor. fc. po. 44.7' NE

P.I. Sta. 7392+27.09 = NW Cor. Sec. 16, T14S, R12E = Sta. 112 +41.40 on Proj. 99-99 K 516 (Dated 1928)
 N 481,632.885 E 8,615,436.237
 1. Found $\frac{3}{8}$ " rebar (0.3' deep)
 2. Cor. is on Sta. 7392+27.09
 3. Found nail and wschr. in top fc. cor. po. 47.7' NW
 4. To the center of the top steel fc. cor. po. 44.7' NE

BM # 10	Set "T" shaped fc. po. S side of rock @ gate po.	22.29' Lt. @ Sta. 7369+06.99	Elev. 1350.28
BM # 11	Set mag. nail & KDOT wschr. top SE abut. of bridge	16.75' Rt. @ Sta. 7377+78.17	Elev. 1302.52
BM # 12	Set mag. nail & KDOT wschr. top E hdwl. RCB	15.22' Rt. @ Sta. 7384+11.38	Elev. 1306.98
BM # 13	Set RR spike in E face of pow. pole	34.18' Lt. @ Sta. 7387+80.38	Elev. 1317.29

KANSAS DEPARTMENT OF TRANSPORTATION
 PLAN AND PROFILE
 STA. 7367+00 TO STA. 7388+00

P.I. Sta. 7339+07.78 = SW Cor. Sec. 16, T14S, R12E = Sta. 165+66.60 on Proj. 99-99 K 516 Dated 1928
 N 476,313.797 E 8,615,481.392
 1. Found $\frac{3}{8}$ " rebar (0.3' deep)
 2. Found nail & wschr. NE side of pow. pole 35.3' NW
 3. Conc. nail & KDOT wschr. W side utility mrkr. po. 52.2' NE
 4. Conc. nail & KDOT wschr. E side of pow. pole 81.0' SSW
 Traverse Point #100
 N 479,749.363 E 8,615,430.569
 1. Set $\frac{1}{2}$ " x 12" rebar w/ orange KDOT cap (Flush)
 2. 28.09' Lt. of Sta. 7373+43.77
 Traverse Point #101
 N 480,008.622 E 8,615,481.127
 1. Set $\frac{1}{2}$ " x 12" rebar w/ orange KDOT cap
 2. 25.55' Rt. of Sta. 7376+02.41
 Traverse Point #102
 N 480,512.314 E 8,615,416.302
 1. Set $\frac{1}{2}$ " x 12" rebar w/ orange KDOT cap (Flush)
 2. 33.27' Lt. of Sta. 7381+06.84

P.I. Sta. 7365+78.29 = W $\frac{1}{4}$ Cor. Sec. 16, T14S, R12E = Sta. 138+97.60 on Proj. 99-99 K 516 (Dated 1928)
 $\Delta = 0^\circ 23' 23"$ Lt. (No curve)
 N 478,984.271 E 8,615,467.769
 1. Set $\frac{1}{2}$ " x 24" rebar w/ orange KDOT cap
 2. Top telephone pedestal 26.5' E
 3. Line of N/S fence line 30.5' W
 4. In line of fence line to E

P.O.T. Sta. 7369+30.87 = Sta. 135+41.60 on Proj. 99-99 K 516 (Dated 1928)
 N 479,336.827 E 8,615,463.572
 1. Set $\frac{1}{2}$ " x 12" rebar (0.3' deep)
 2. Conc. nail & KDOT wschr. W face gate po. 28.7' W
 3. Spk. & KDOT wschr. top metal "T" po. 32.5' E
 4. Field entrance to west 11.5' S

P.O.T. Sta. 7385+97.44
 N 481,003.279 E 8,615,443.733
 1. Set $\frac{1}{2}$ " x 12" rebar (0.3' deep)
 2. Top N end CMP 49.9' SSE
 3. Conc. & KDOT wschr. E face of pow. pole 58.3' SW
 4. TW field entrance to E 52.0' S
 Traverse Point #103
 N 480,957.627 E 8,615,472.885
 1. Set $\frac{1}{2}$ " x 12" rebar w/ orange KDOT cap (Flush)
 2. 28.61' Rt. of Sta. 7385+51.44

P.I. Sta. 7392+27.09 = NW Cor. Sec. 16, T14S, R12E = Sta. 112 +41.40 on Proj. 99-99 K 516 (Dated 1928)
 N 481,632.885 E 8,615,436.237
 1. Found $\frac{3}{8}$ " rebar (0.3' deep)
 2. Found nail & wschr. NW side cor. fc. po. 54.5' SSW
 3. Found nail & wschr. top cor. fc. po. 47.7' NW
 4. Top steel cor. fc. po. 44.7' NE

PROJECT SURVEY CONTROL

HORIZONTAL CONTROL

Horizontal Datum is NAD83(2011) KRCS Zone 8 Manhattan for project coordinates.

VERTICAL CONTROL

North American Vertical Datum = NAVD88 (GEOID 18)

"Base" is a GPS observation was used for the Datum on this project. The GPS observation was checked using Base "1".

A double set of level runs were run through the BASE and all the benchmarks and traverse points for control on this project.

Datum Bench Mark: "BASE"
 NAVD88 (Computed using GEOID18) Elev. = 1336.247

Ctr. Cor. Sec. 16, T14S, R12E
 N 478,989.725 E 8,618,110.834
 1. Found $\frac{1}{2}$ " x 12" (in rock) w/ KDOT alum. capped rebar (0.1' above ground)
 2. Cor. is 2643.07' Rt. (East) of Sta. 7365+78.29 SE $\Delta = 89^\circ 49' 33"$
 3. Set conc. nail and KDOT wschr. W side fc. cor. po. 0.8' E
 4. Set conc. nail and KDOT wschr. top fc. brace po.

W $\frac{1}{4}$ Cor. Sec. 16, T14S, R12E
 N 478,984.271 E 8,615,467.769
 1. Set $\frac{1}{2}$ " x 24" KDOT alum. capped rebar (0.3' deep)
 2. Cor. is on Sta. 7365+78.29
 3. To the center of the top of telephone pedestal 26.5' E
 30.5' W @ Sta. 7369+06.99

STA. 7373+60.00 BEGIN
 Proj. No. 99-99 KA-5728-01
 Sta. 132+52.47 on Proj. No. 99-99 K 516

Sta. 7376+05.41 Const. Slope Drain (Stone) Rt. See Sh. No. 13, 63

Sta. 7376+29 Eradicate Exist. Ent. Rt. & Remove Exist. 18"x21.60' CMP

STA. 7381+00.00 END
 Proj. No. 99-99 KA-5728-01 = Sta. 123+72.47 on Proj. No. 99-99 K 516

Sta. 7385+20.00 End Construction

Sta. 7384+11.00 Const. Slope Protection (Riprap Stone) (200 lb) Rt. See Sh. No. 63

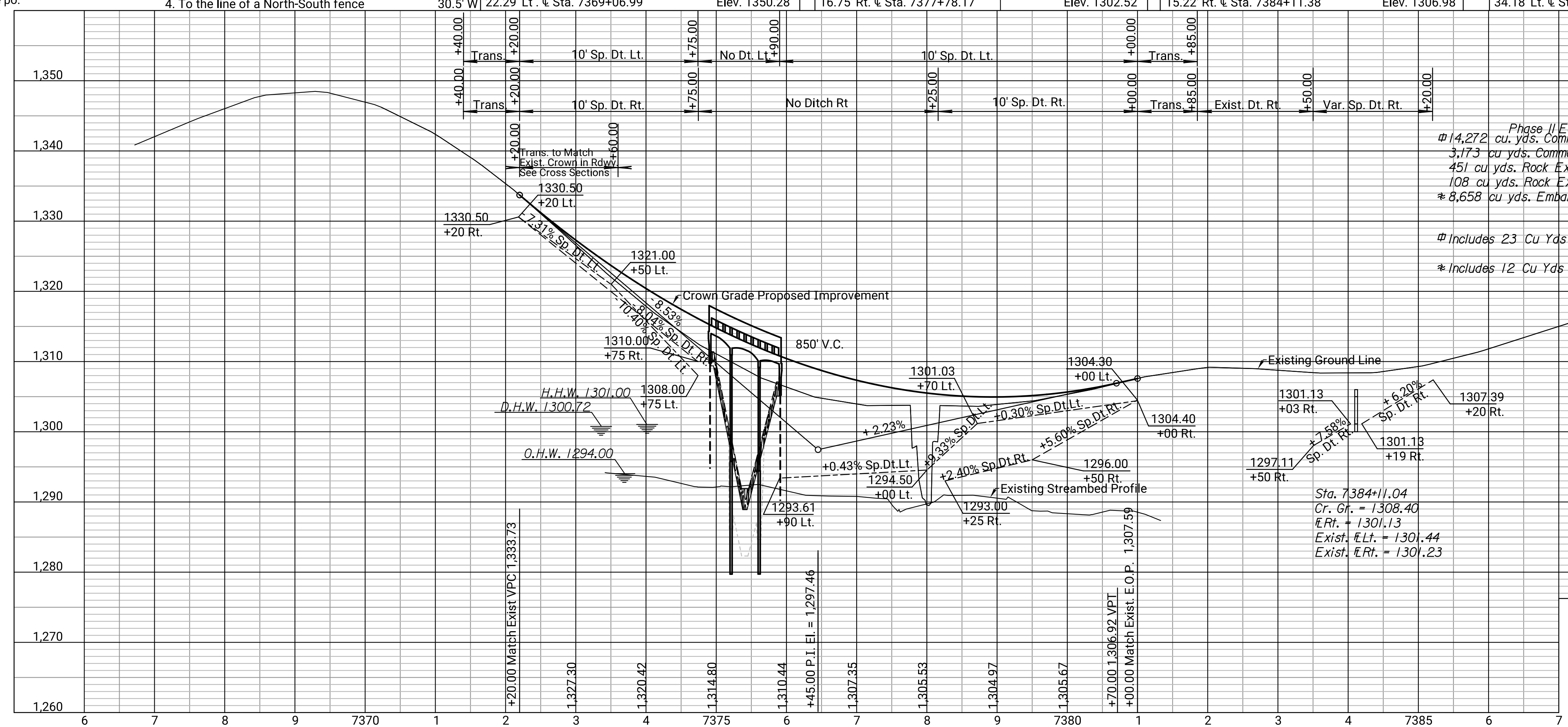
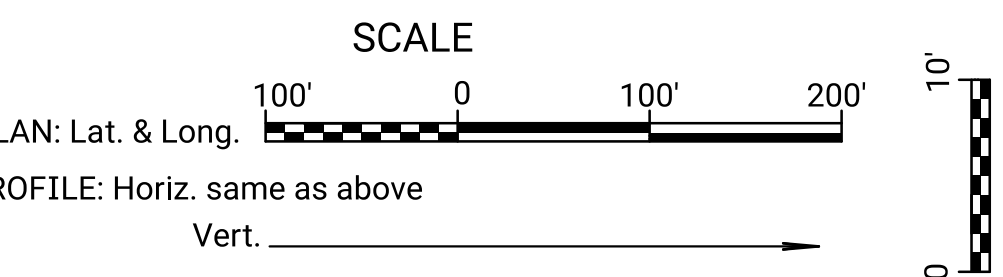
DATE	BY	REFERENCES NOTED	REFERENCES CHECKED
10/2020	D. Kemble		
03/2022	A. Zuno		

UTILITY OWNERS

Gas Line
 City of Eskridge
 Telephone Cable
 Century Link
 Kenny (Integrity Locate)
 913-305-6679

Power
 Everygy
 818 S Kansas Ave
 Topeka, KS 66612
 Scott Schlageck
 785-575-6014

Phase II Earthwork:
 @ 14,272 cu. yds. Common Excavation (VMF=0.78)
 @ 3,173 cu. yds. Common Excavation (To Be Wasted) (VMF=0.78)
 @ 451 cu. yds. Rock Excavation (Existing Pavement-To Be Wasted) (VMF=1.00)
 @ 108 cu. yds. Rock Excavation (Non-Durable Shale-To Be Wasted) (VMF=1.10)
 @ 8,658 cu. yds. Embankment
 @ Includes 23 Cu Yds of Com. Exc. from Ent 7380+70 Lt. (VMF=0.78)
 @ Includes 12 Cu Yds of Emb. from Ent 7380+70 Lt.



P.O.T. Shoofly Sta 65+35.96
N 478,984.271 E 8,615,467.769
1. = ϕ Sta. 7365+78.29
2. Not Set

P.C. Shoofly Sta 67+60.67
N 479,205.967 E 8,615,465.130
1. = ϕ Sta. 7369+00
2. Not Set

P.I. Shoofly Sta 69+07.21
N 479,352.491 E 8,615,463.385
1. = ϕ Sta. 7369+46.54
2. Not Set

P.T. Shoofly Sta 70+45.76
N 479,476.778 E 8,615,541.008
1. 79.10' Rt. ϕ Sta. 7370+69.89
2. Not Set

P.C. Shoofly Sta 71+32.76
N 479,550.569 E 8,615,587.0933
1. 126.06' Rt. ϕ Sta. 7371+43.12
2. Not Set

P.I. Shoofly Sta 72+32.70
N 479,635.333 E 8,615,640.032
1. 180' Rt. ϕ Sta. 7372+27.25
2. Not Set

P.T. Shoofly Sta 73+27.19
N 479,735.236 E 8,615,638.842
1. 180' Rt. ϕ Sta. 7373+27.19
2. Not Set

P.C. Shoofly Sta 79+18.86
N 480,326.894 E 8,615,631.798
1. 180' Rt. ϕ Sta. 7379+18.86
2. Not Set

P.I. Shoofly Sta 80+18.80
N 480,426.825 E 8,615,630.608
1. 180' Rt. ϕ Sta. 7380+18.80
2. Not Set

P.T. Shoofly Sta 81+13.29
N 480,510.304 E 8,615,575.665
1. 126.06' Rt. ϕ Sta. 7381+02.90
2. Not Set

P.C. Shoofly Sta 82+00.29
N 480,582.977 E 8,615,527.836
1. 79.09' Rt. ϕ Sta. 7381+76.18
2. Not Set

P.I. Shoofly Sta 83+46.83
N 480,705.381 E 8,615,447.276
1. = ϕ Sta. 7382+99.52
2. Not Set

P.T. Shoofly Sta 84+85.38
N 480,851.905 E 8,615,445.531
1. = ϕ Sta. 7384+46.06
2. Not Set

P.O.T. Shoofly Sta 87+39.57
N 481,106.076 E 8,615,442.505
1. = ϕ Sta. 7387+00.24
2. Not Set

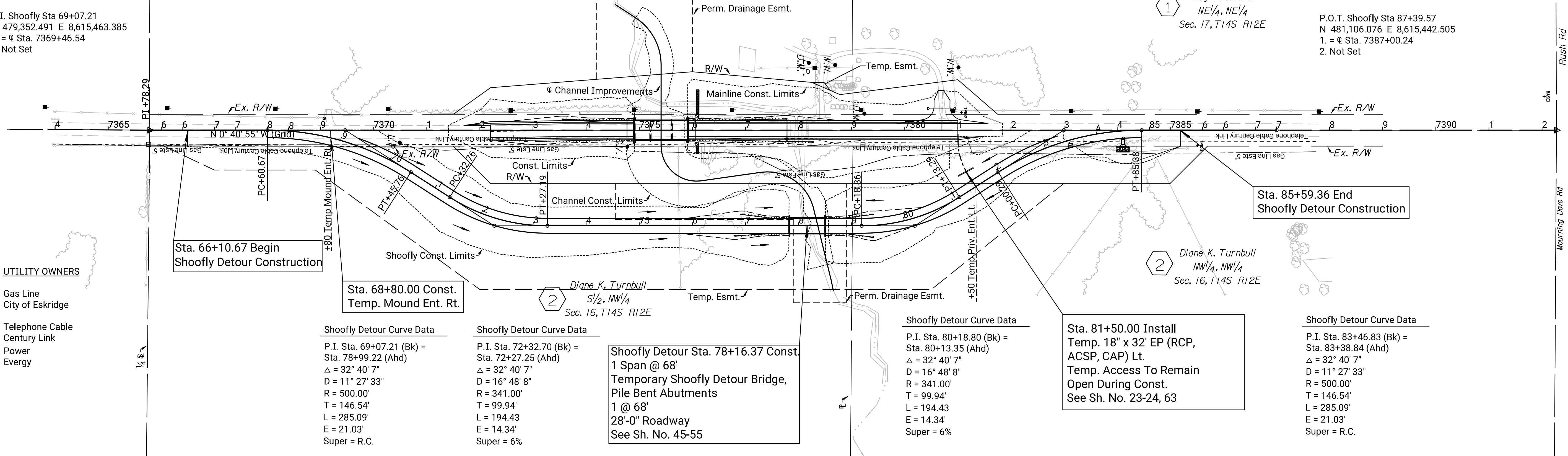
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	09	135

1 Gary D. Kemble
S/2, NE/4
Sec. 17, T14S R12E

1 Gary D. Kemble
NE/4, NE/4
Sec. 17, T14S R12E

2 Diane K. Turnbull
NW/4, NW/4
Sec. 16, T14S R12E

BY	DATE
A. Zuno	2023
D. Kallie	2023
REFERENCES NOTED	REFERENCES CHECKED



Shoofly Detour Curve Data
P.I. Sta. 69+07.21 (Bk) = Sta. 78+99.22 (Ahd)
 $\Delta = 32^\circ 40' 7''$
 $D = 11^\circ 27' 33''$
 $R = 500.00'$
 $T = 146.54'$
 $L = 285.09'$
 $E = 21.03'$
Super = R.C.

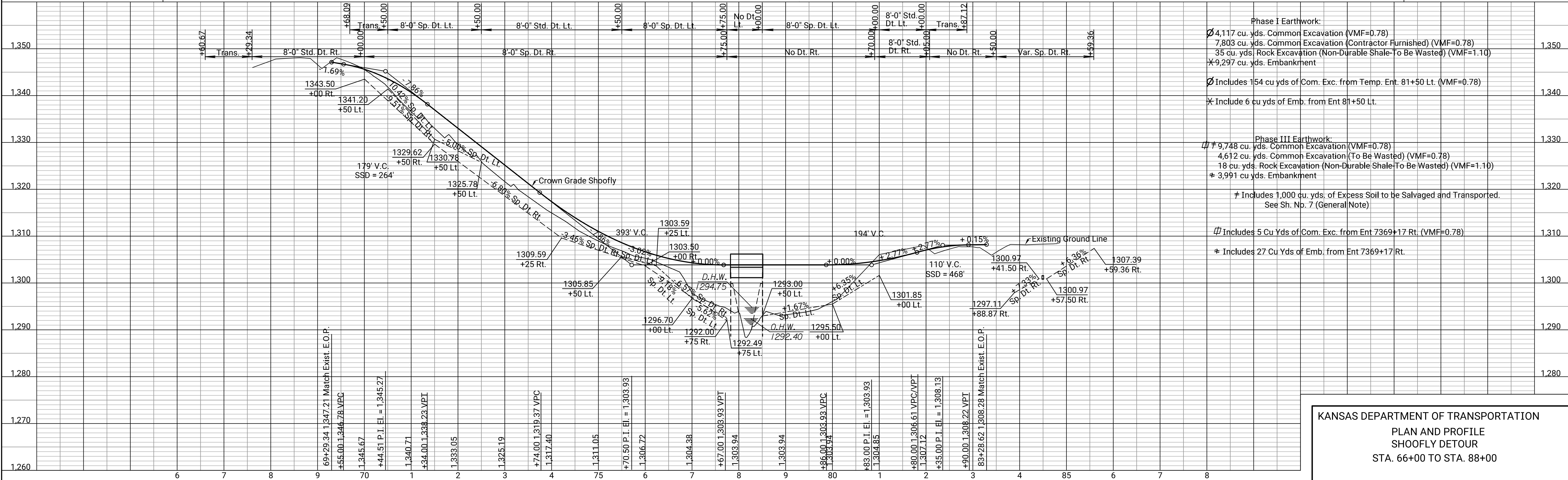
Shoofly Detour Curve Data
P.I. Sta. 72+32.70 (Bk) = Sta. 72+27.25 (Ahd)
 $\Delta = 32^\circ 40' 7''$
 $D = 16^\circ 48' 8''$
 $R = 341.00'$
 $T = 99.94'$
 $L = 194.43'$
 $E = 14.34'$
Super = 6%

Shoofly Detour Sta. 78+16.37 Const.
1 Span @ 68'
Temporary Shoofly Detour Bridge,
Pile Bent Abutments
1 @ 68'
28'-0" Roadway
See Sh. No. 45-55

Shoofly Detour Curve Data
P.I. Sta. 80+18.80 (Bk) = Sta. 80+13.35 (Ahd)
 $\Delta = 32^\circ 40' 7''$
 $D = 16^\circ 48' 8''$
 $R = 341.00'$
 $T = 99.94'$
 $L = 194.43'$
 $E = 14.34'$
Super = 6%

Sta. 81+50.00 Install
Temp. 18" x 32" EP RCP,
ACSP, CAP Lt.
Temp. Access To Remain
Open During Const.
See Sh. No. 23-24, 63

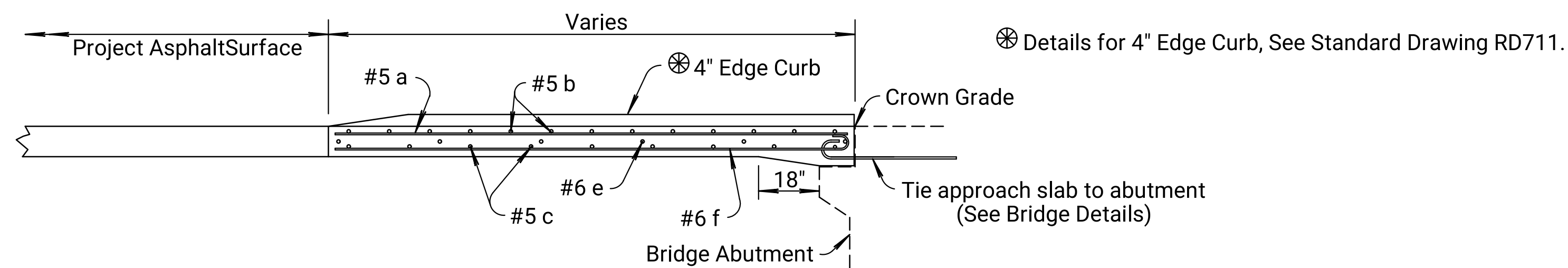
Shoofly Detour Curve Data
P.I. Sta. 83+46.83 (Bk) = Sta. 83+38.84 (Ahd)
 $\Delta = 32^\circ 40' 7''$
 $D = 11^\circ 27' 33''$
 $R = 500.00'$
 $T = 146.54'$
 $L = 285.09'$
 $E = 21.03'$
Super = R.C.



KANSAS DEPARTMENT OF TRANSPORTATION
PLAN AND PROFILE
SHOOFLY DETOUR
STA. 66+00 TO STA. 88+00

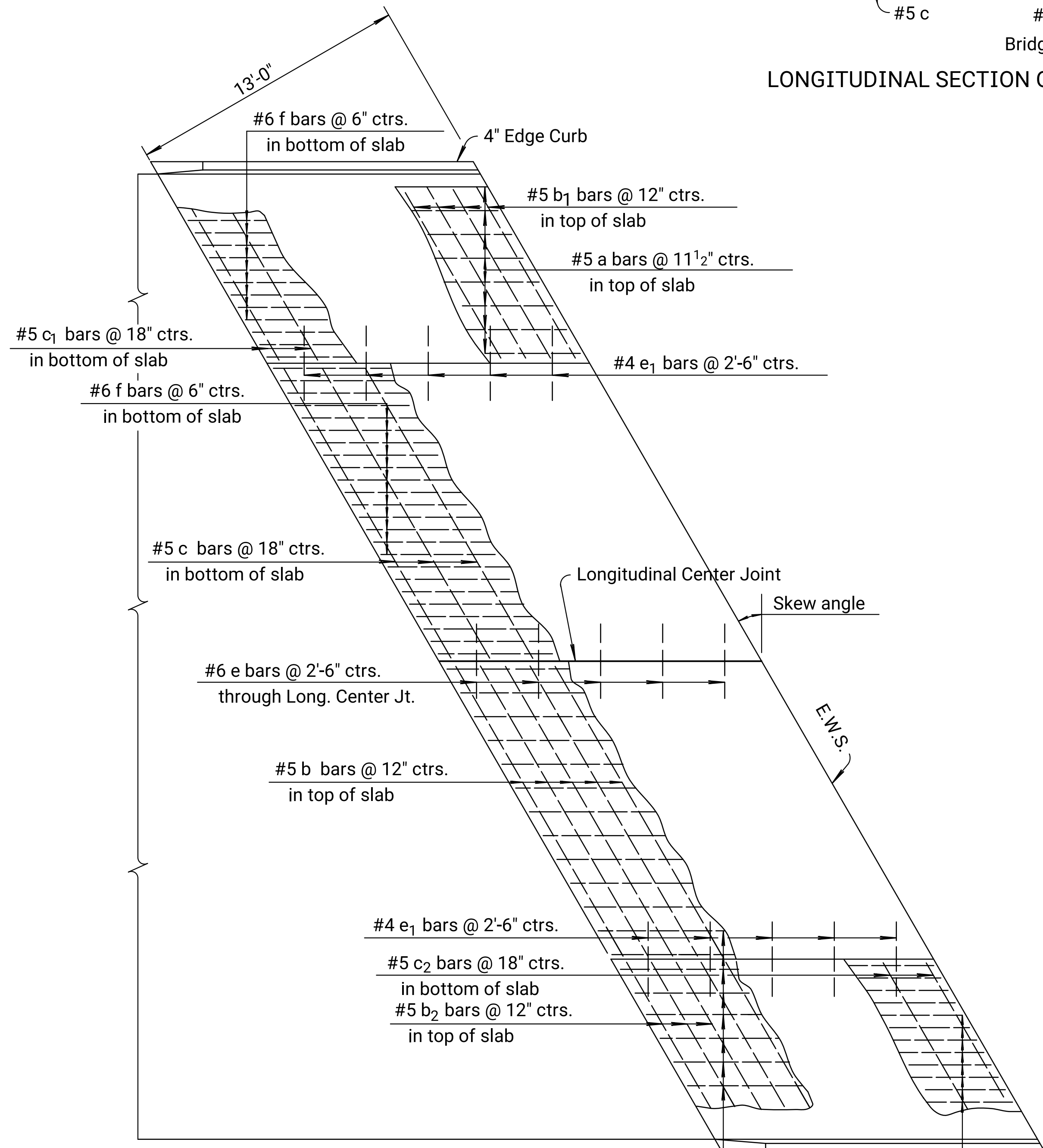
Plotted by: August Zuno 18-JAN-2024 08:59
File: ka572801rpp-02.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	11	135



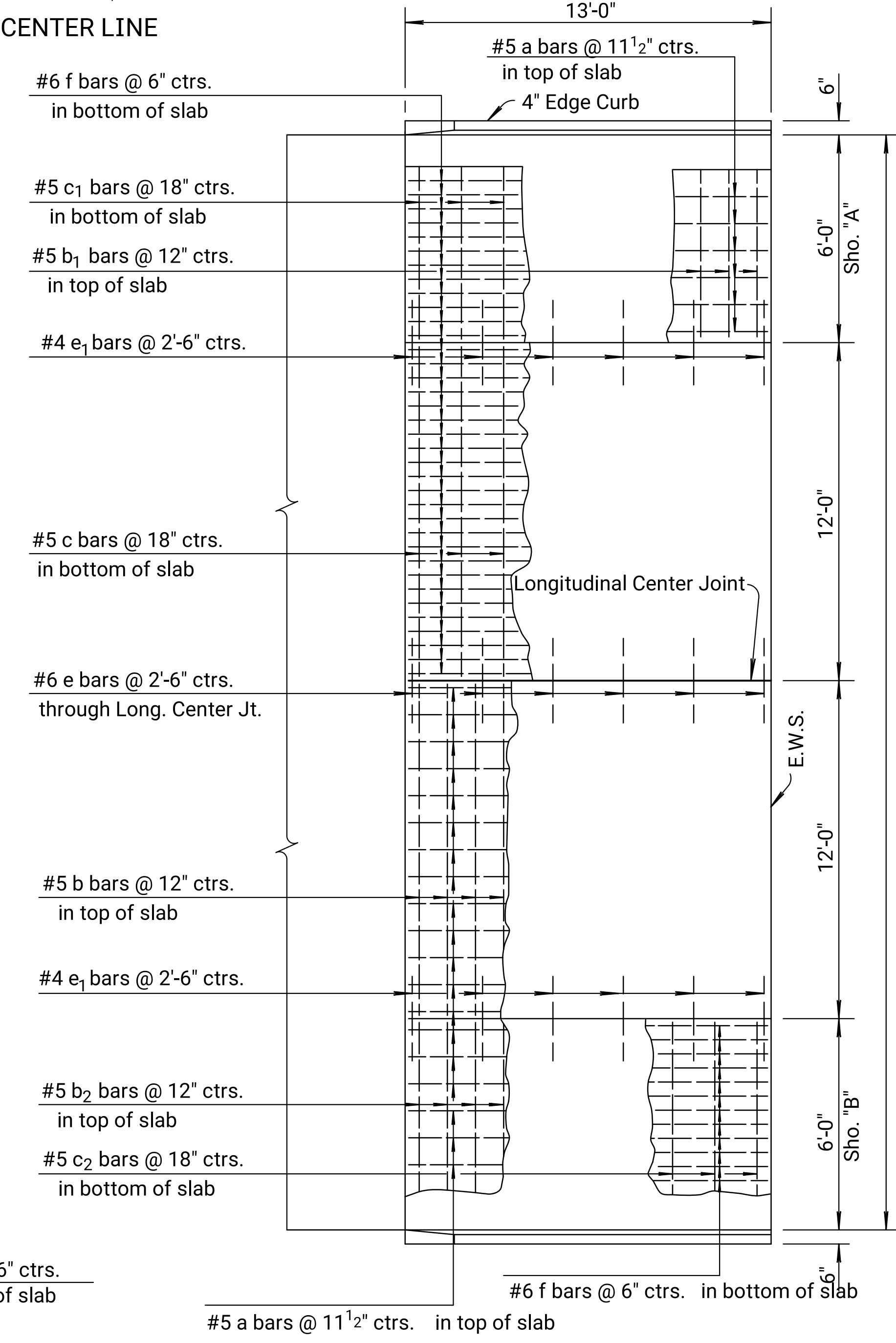
⊗ Details for 4" Edge Curb, See Standard Drawing RD711.

LONGITUDINAL SECTION ON CENTER LINE



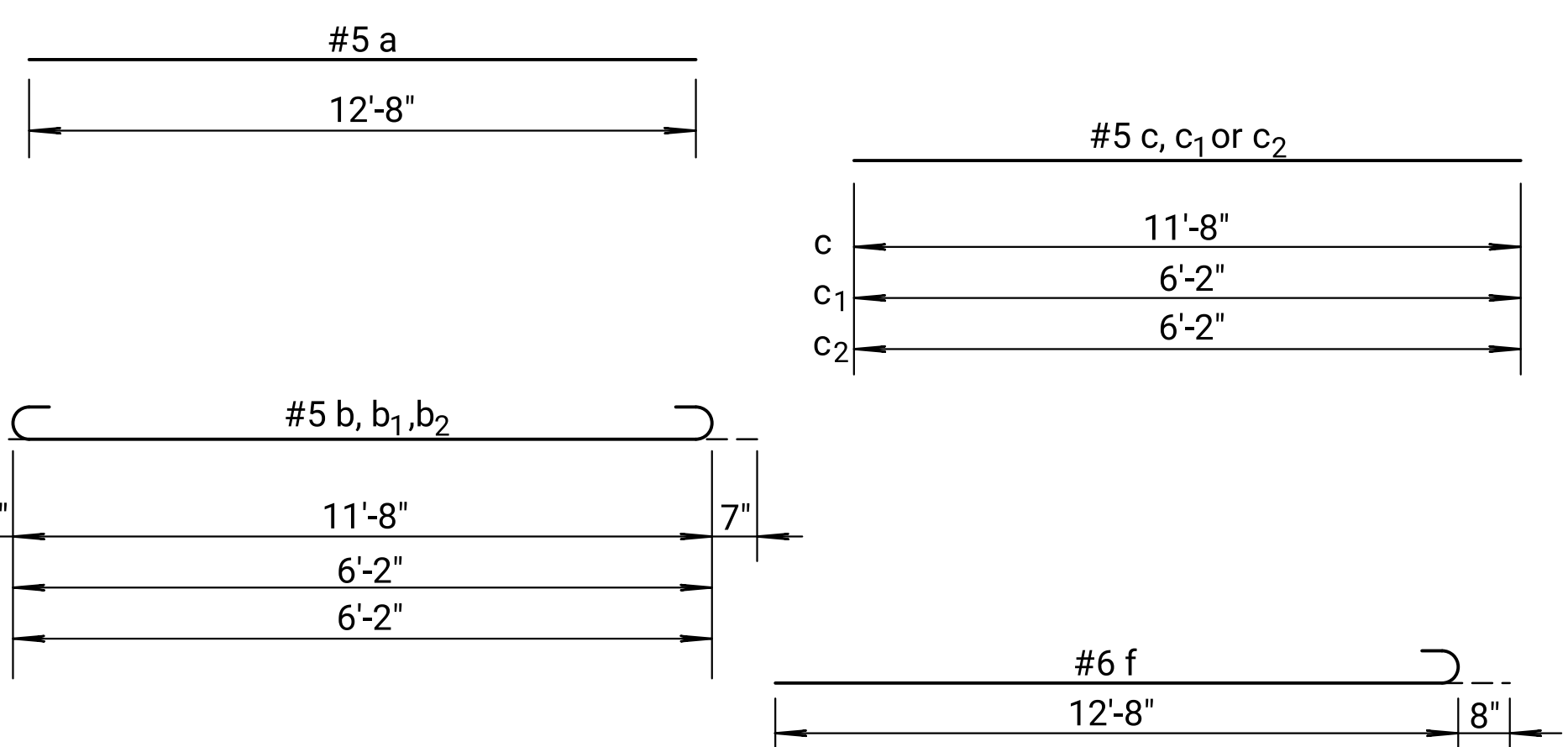
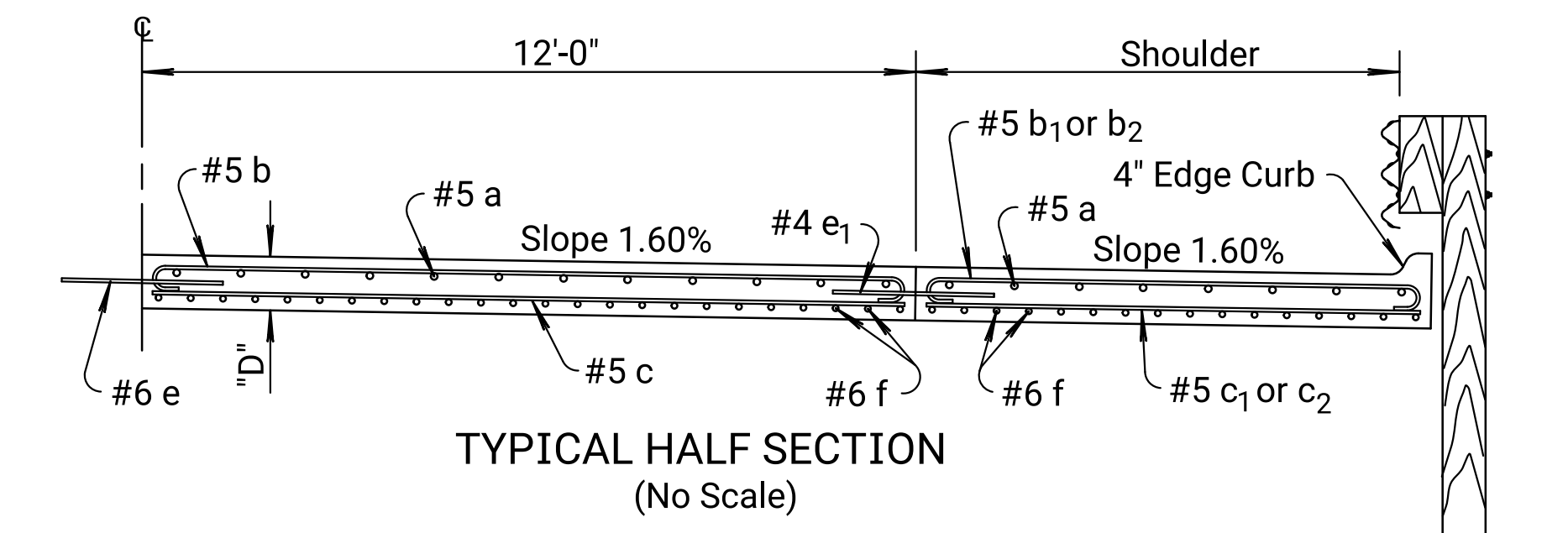
Note: Spacing of longitudinal reinforcing bars is normal to center line.
Spacing of transverse reinforcing bars is parallel to center line.

PLAN FOR SKEWED APPROACH (SKEW ≥ 5°)
(No Scale)



PLAN FOR NORMAL APPROACH
(No Scale)

GENERAL NOTE
Special Concrete Bridge Approach shall be paid for as Sq. Yds. of Concrete Pavement (10" Unif.)(AE)(Br. App.) and includes all work and materials required to construct the approach slab as shown on this sheet.
All work and materials required for installation of joint material shall be subsidiary to this bid item.
At the Contractor's option #4x3'-0" tie bars @ 15" centers may be substituted for the #6 e bars at 2'-6" centers.
All reinforcing steel shall be epoxy coated.
See Standard Drawing RD711 for details of joints and edge curb.
Clearance from the face of concrete for all reinforcing steel shall be 2 inches.
Standard reinforcing bar hooks in accordance with the latest ACI specifications shall be used throughout.



Note: All dimensions are out to out on bars unless noted otherwise.

BENDING DIAGRAMS

BILL OF MATERIALS

BAR SCHEDULE																																	
NORMAL APPROACH						-- ° SKEW						-- ° SKEW																					
Bar	a	b	b ₁	b ₂	c	c ₁	c ₂	e	e ₁	f		a	b	b ₁	b ₂	c	c ₁	c ₂	e	e ₁	f		a	b	b ₁	b ₂	c	c ₁	c ₂	e	e ₁	f	
No.	40	26	13	13	18	9	9	6	12	74		#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#5	#6	#4	#6	
Size	#5	#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6	
Length	12'-8"	12'-10"	7'-4"	7'-4"	11'-8"	6'-2"	6'-2"	3'-0"	3'-0"	13'-4"		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6		#5	#5	#5	#5	#5	#5	#5	#6	#4	#6	
Reinforcing Steel (Grade 60) (Epoxy Coated)	2,950 lbs.										lbs.										lbs.												
Concrete Pavement (10" Unif.)(AE)	53.44 Sq. Yds.										Sq. Yds.										Sq. Yds.												

Note: Quantities listed for one approach slab only. Two required per bridge. Reinforcing steel and joint lengths shown for information only.

09	09-09-09	Revised Reinforcing Steel listing	S.W.K.	J.O.B.
08	05-14-09	Revised General Note	S.W.K.	J.O.B.
07	10-30-08	Added guardrail post detail at curb	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

CONCRETE BRIDGE APPROACH PAVEMENT ADJACENT TO ASPHALT SURFACE

RD715

DESIGNED	06-09-09	APPD.	James O. Brewer
DESIGN CK.	DETAIL CK.	QUANTITIES	TRACED
		QUAN.CK.	TRACE CK.

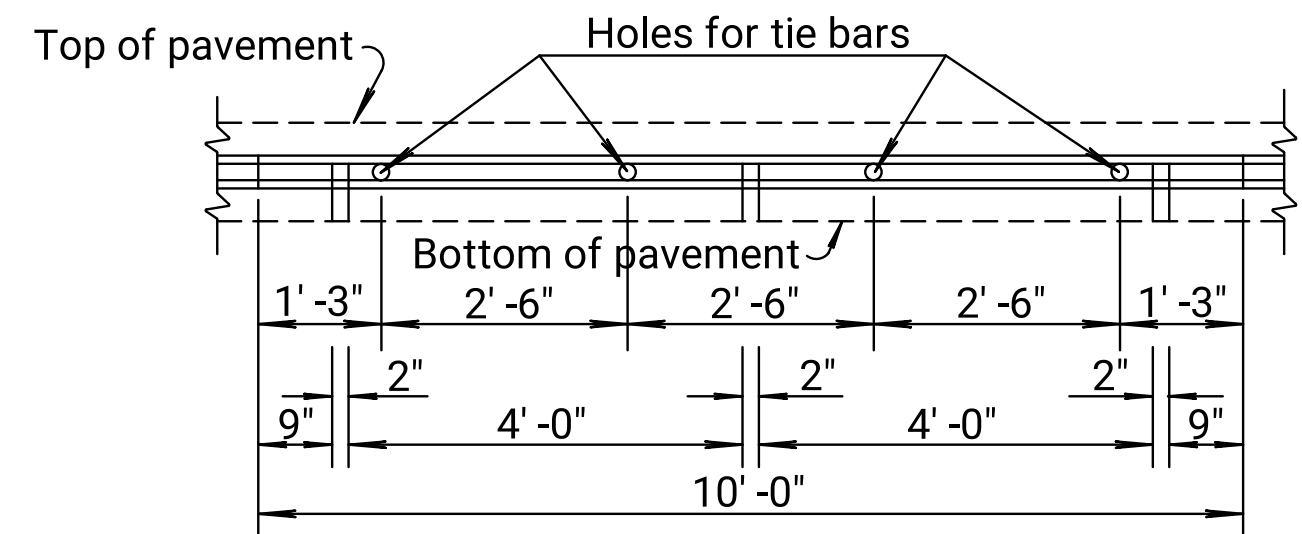
KDOT Graphics Certified 01-04-2024 Sh. No. 11

Note to Designer: The designer shall be responsible for designating pavement thickness and computing reinforcing steel and concrete quantities and dimensions necessary to complete this sheet.

Plotted by: August.Zuno 4-JAN-2024 15:29 File: rss715.dgn

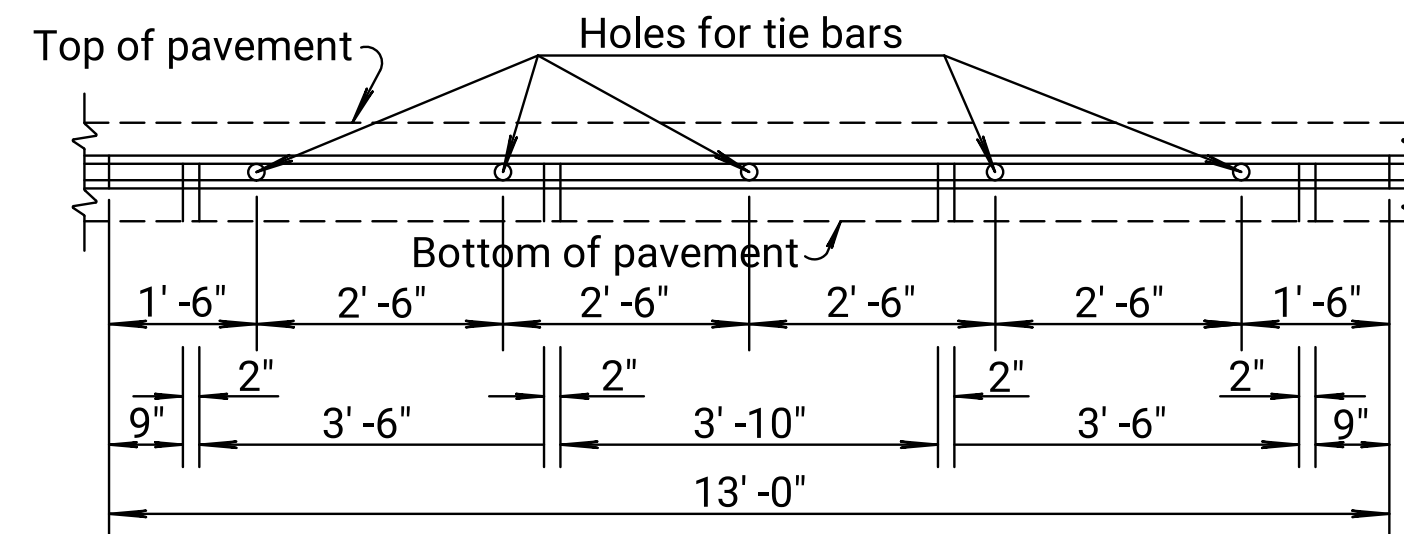
KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	12	135



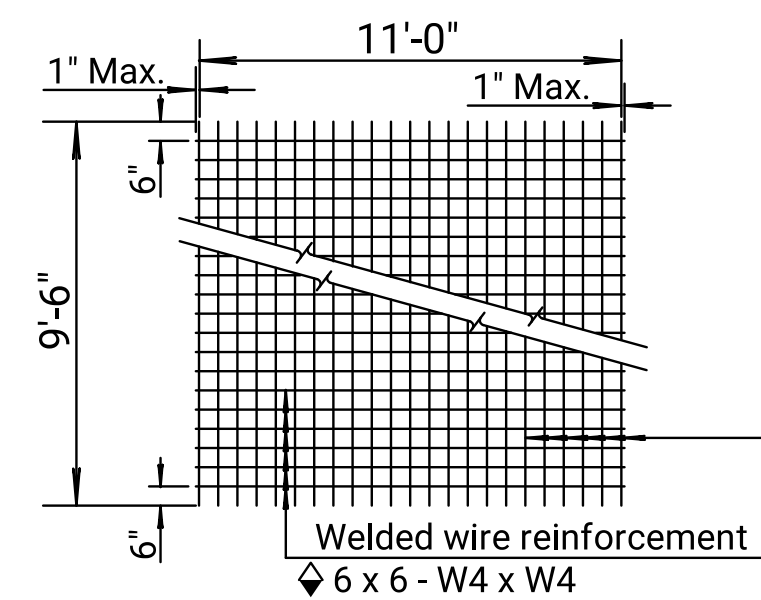
To be used only against forms. Shall not extend through contraction joints.

METAL STRIP FOR LONGITUDINAL CONSTRUCTION JOINT (10'-0")



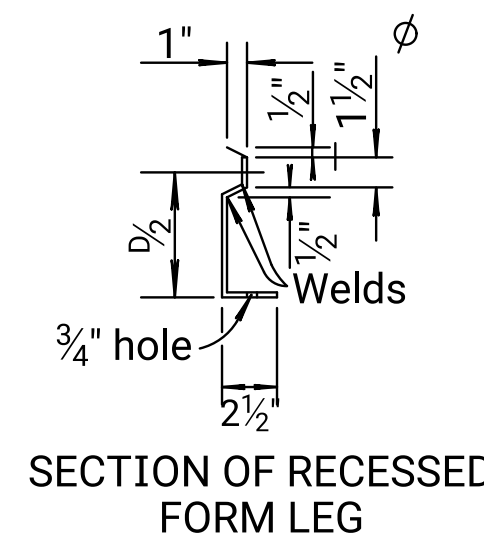
To be used only against forms. Shall not extend through contraction joints.

METAL STRIP FOR LONGITUDINAL CONSTRUCTION JOINT (13'-0")



TYPICAL SHEET OF WELDED WIRE REINFORCEMENT FOR SPECIAL BRIDGE APPROACH PAVEMENT

Note: Epoxy coated #3 bars longitudinally @ 12" ctrs. & #3 bars transversely @ 18" ctrs. may be substituted for each layer of epoxy coated welded wire reinforcement.

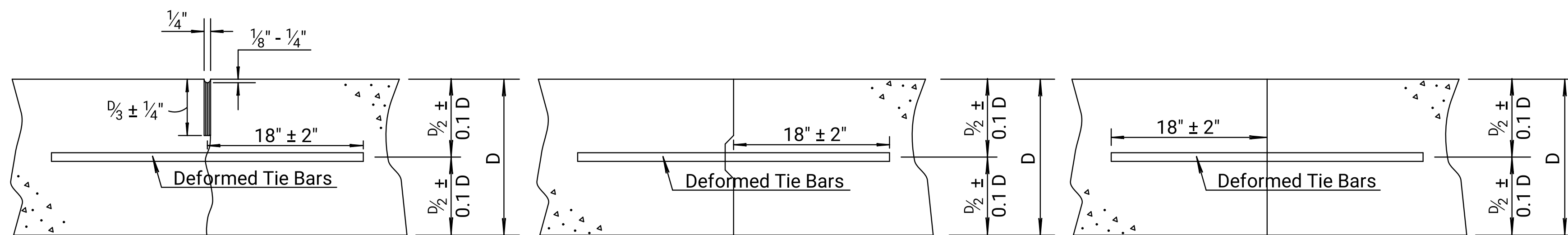


Ø Snap-in leg or other approved designs may be used in lieu of welded leg.



DETAIL OF LAP FOR WELDED WIRE REINFORCEMENT

The lap shall extend beyond the first transverse or bag wire of each sheet. The sheet shall be wired securely at the edges and at intervals not to exceed 2'-6" for the full width of the sheet. Approximate weight of welded wire reinforcement = 58 lbs. per 100 sq. ft. Other methods for fastening the sheets of welded wire reinforcement at the laps may be used with the approval of the Engineer.



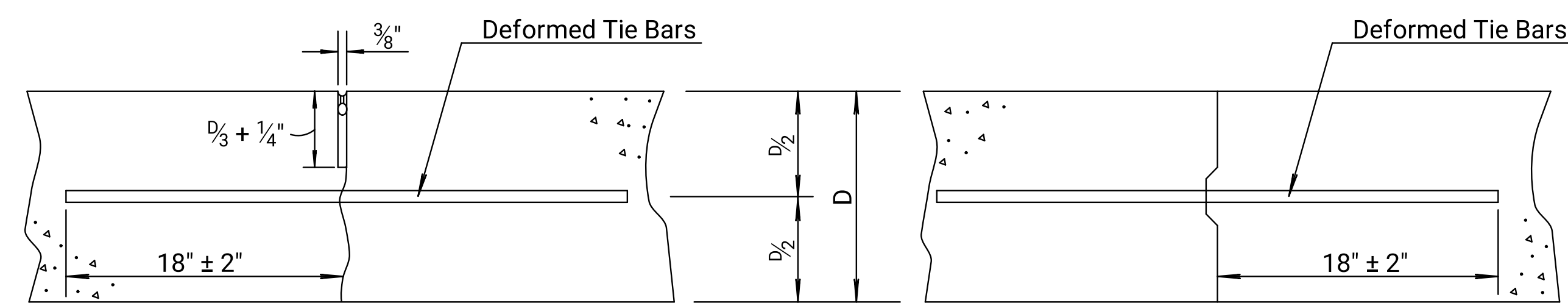
Tied Non-Keyed

Tied Keyed Construction

Tied Butt Construction

LONGITUDINAL JOINTS

Note: For longitudinal construction joints the contractor has the option of using either the keyed or butt type. Place deformed tie bars mid-depth of the shoulder.



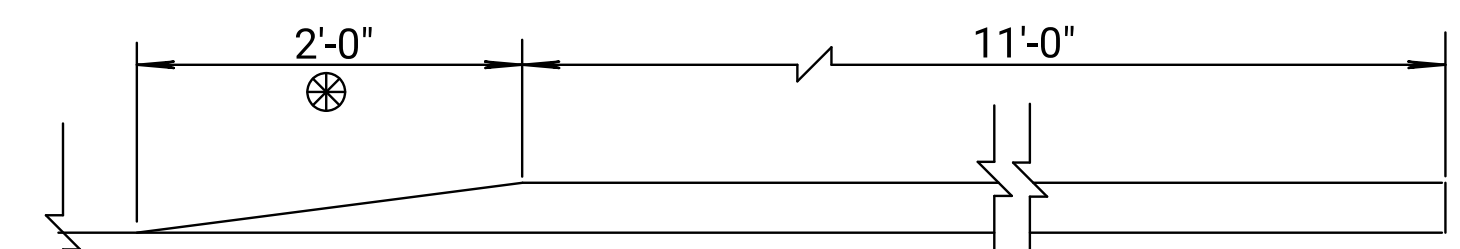
Monolithic Pour

Construction Joint

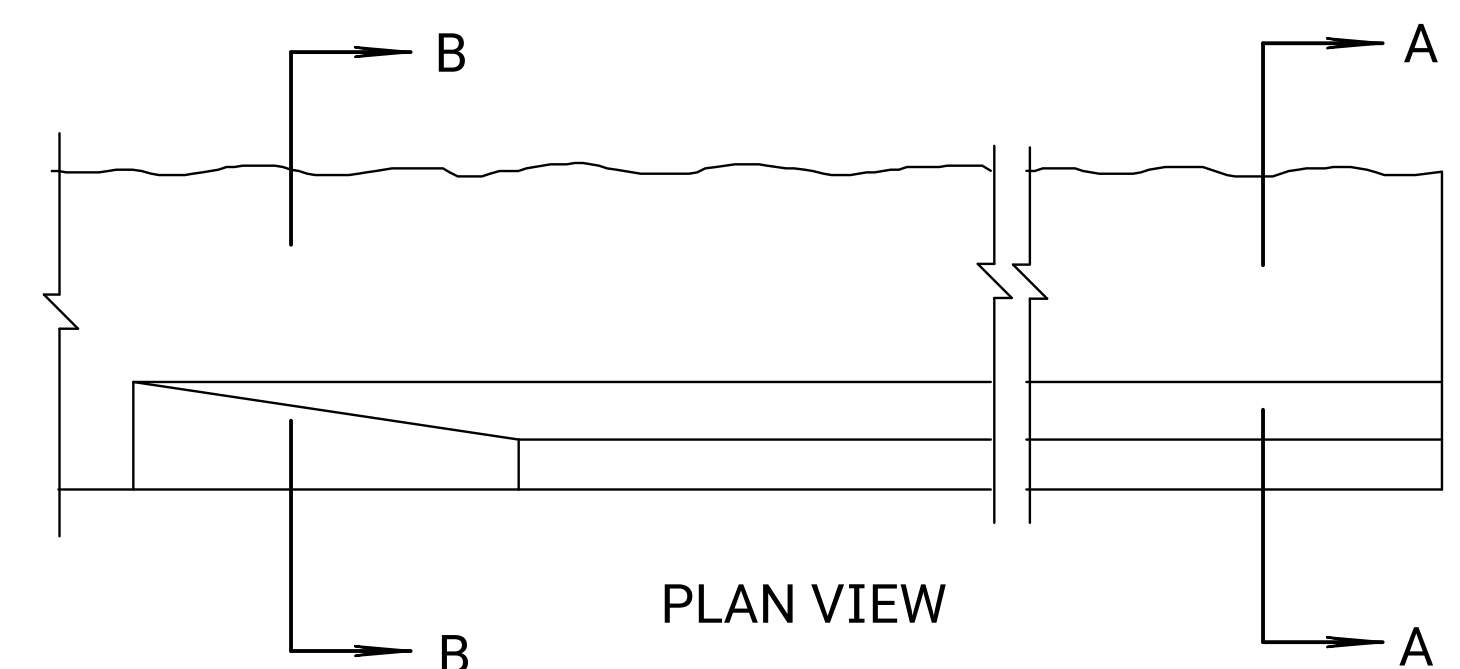
TRANSVERSE JOINTS

Note: A construction joint is required when the concrete placement has been interrupted for a substantial length of time or at the end of a day's placement.

⊗ No 4" Curb transition when adjacent to Flume Inlet.



ELEVATION



PLAN VIEW

4" EDGE CURB DETAIL

GENERAL NOTES

All work shall be done in conformity with the Standard Specifications applicable to the project.

The cost of all bars and joint material shown on this sheet is to be included in the bid price for Concrete Pavement.

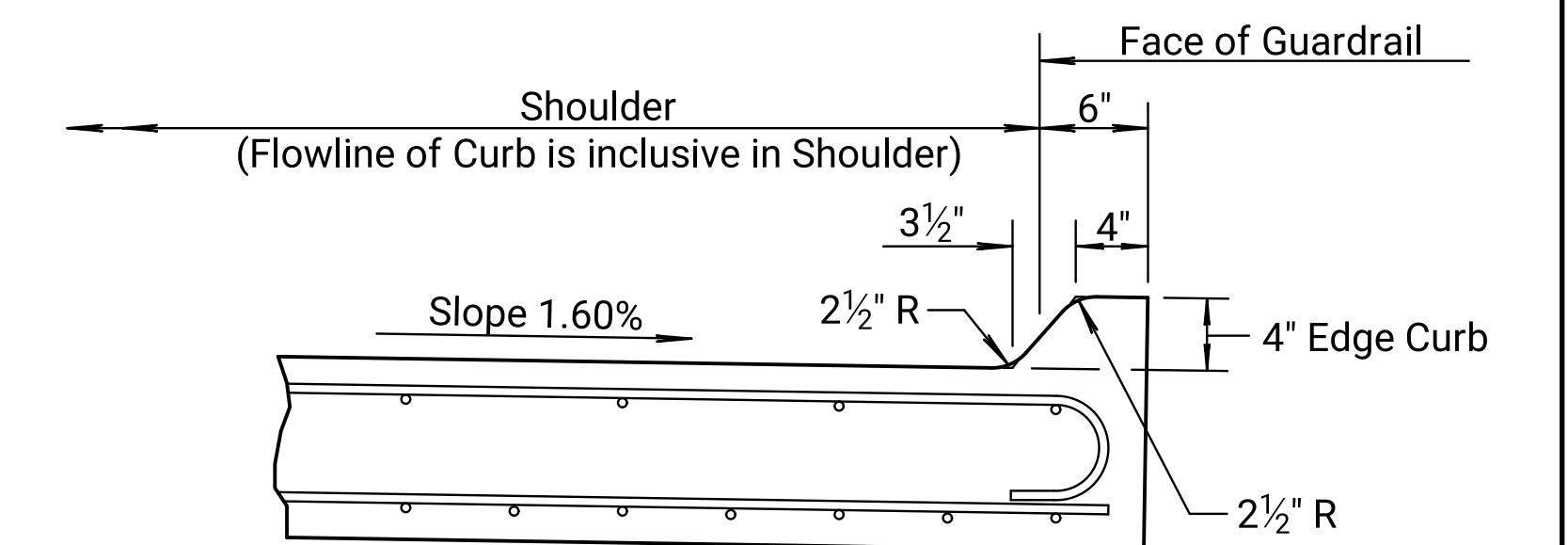
At each planned transverse joint location, a 4 to 6 inch wide strip of the pavement surface shall be protected from the texturing operation to provide a transverse textureless surface centered over the joint sawcut.

All sawed joints on this project shall be filled with sealant in accordance with Standard Specifications.

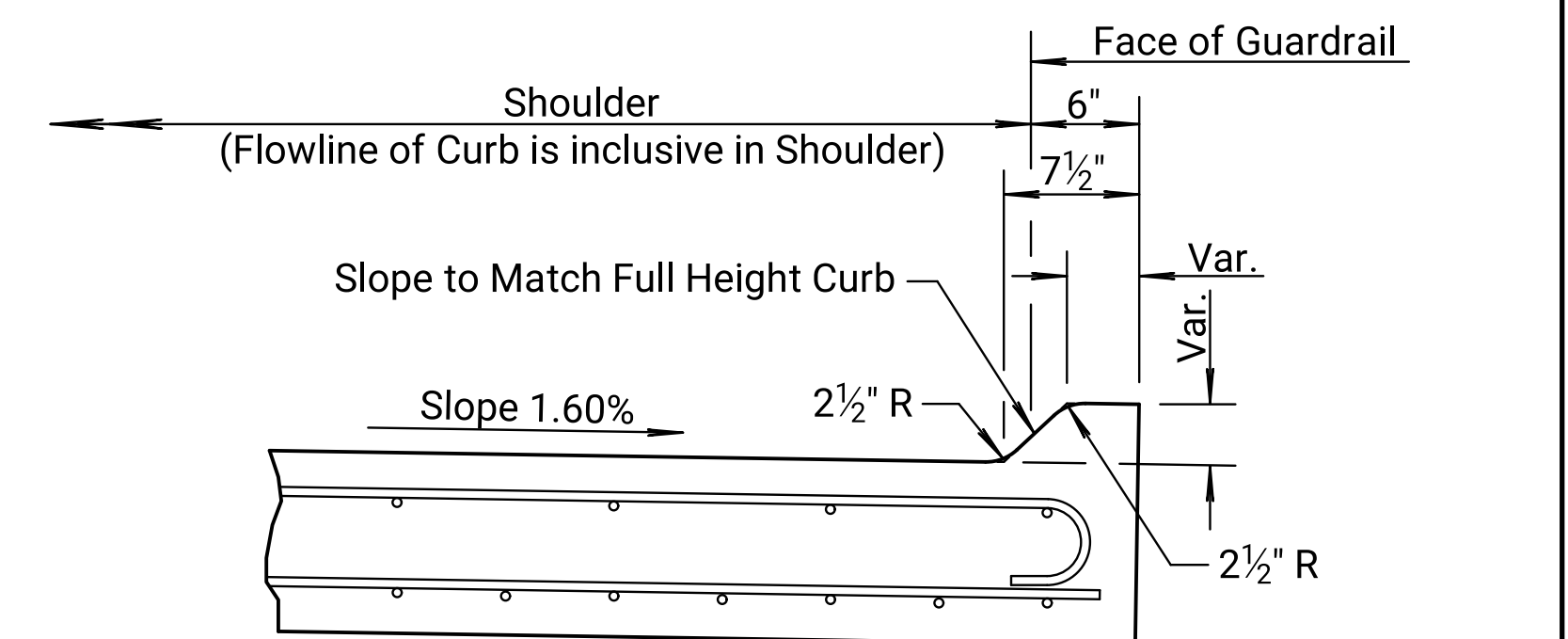
The 4 inch edge curb shall be constructed integral with the approach slab shoulder.

All materials and work required for this construction shall be subsidiary to the concrete approach slab.

Tie bars shall be evenly spaced along the length of the slab and no tie bars shall be within 12" of contraction joint.



SECTION A-A



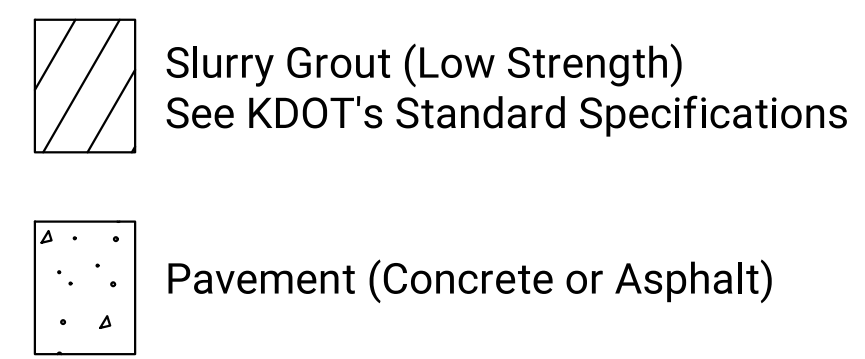
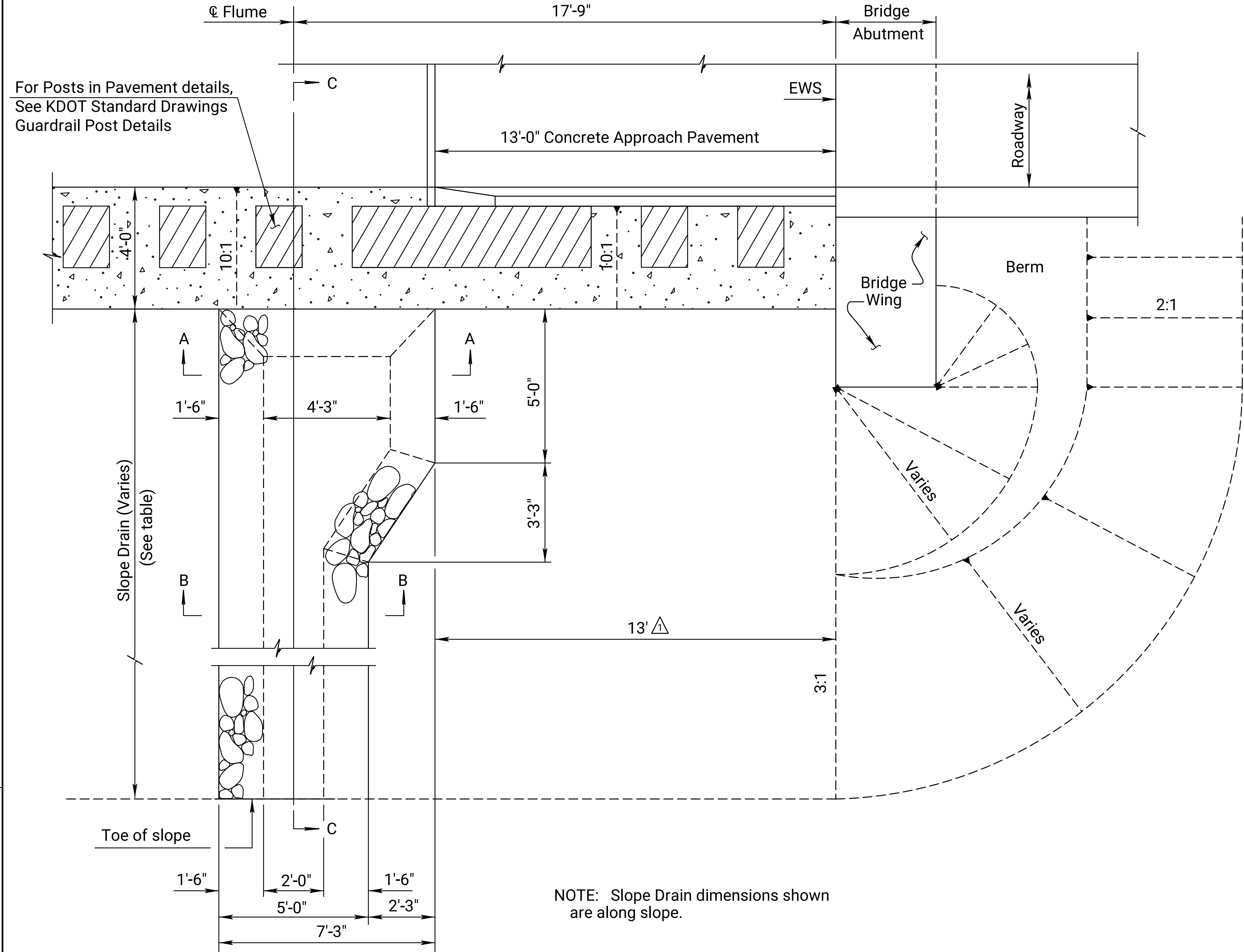
SECTION B-B

13	05-17-13	Revised Note, Longitudinal Joints	S.W.K.	J.O.B.
12	05-14-09	Pres. Relief Jt. to RD712/tie bar lab.	S.W.K.	J.O.B.
11	10-23-08	Revised Sec. A-A and Sec. B-B	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION			
MISCELLANEOUS DETAILS FOR CONCRETE BRIDGE APPROACH PAVEMENT			
RD711			
DESIGNED	10-23-13	APPD.	James O. Brewer
DESIGN CK.	DETAILD	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

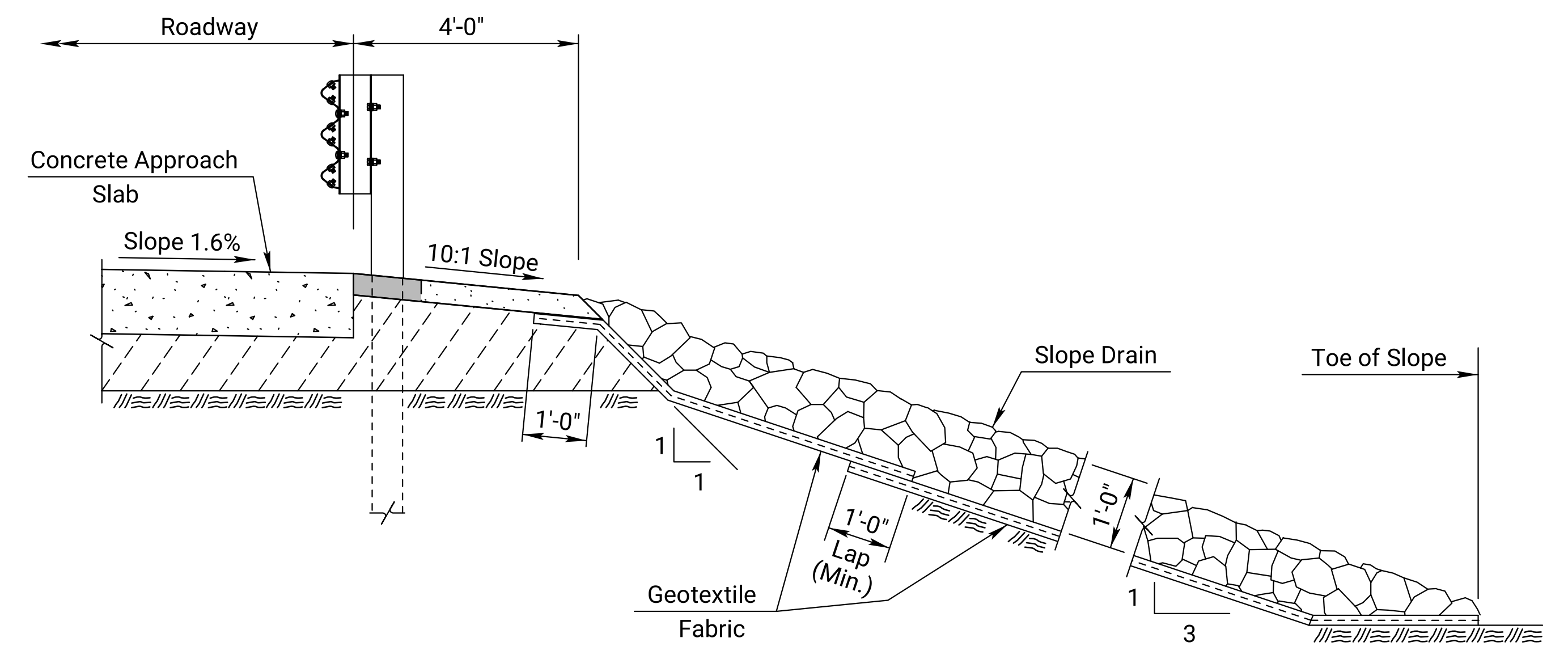
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	13	135

Note to Designer: Use of a stone flume and stone slope drain is intended for small localized drainage (approximately 0.5 cfs.) Width of Slope Drain (Stone) at Section A-A could be widened to accommodate existing conditions. Larger flows should utilize a Concrete Flume and Slope Drain (Concrete/Stone). See KDOT Standard Drawing Flume Inlet and Slope Drain (Concrete/Stone). Depiction is of a Normal Bridge Approach with MGS Thrie Beam. Other abutment types or stiffened guardrail attachments to the bridge rail would need evaluation by the Designer.



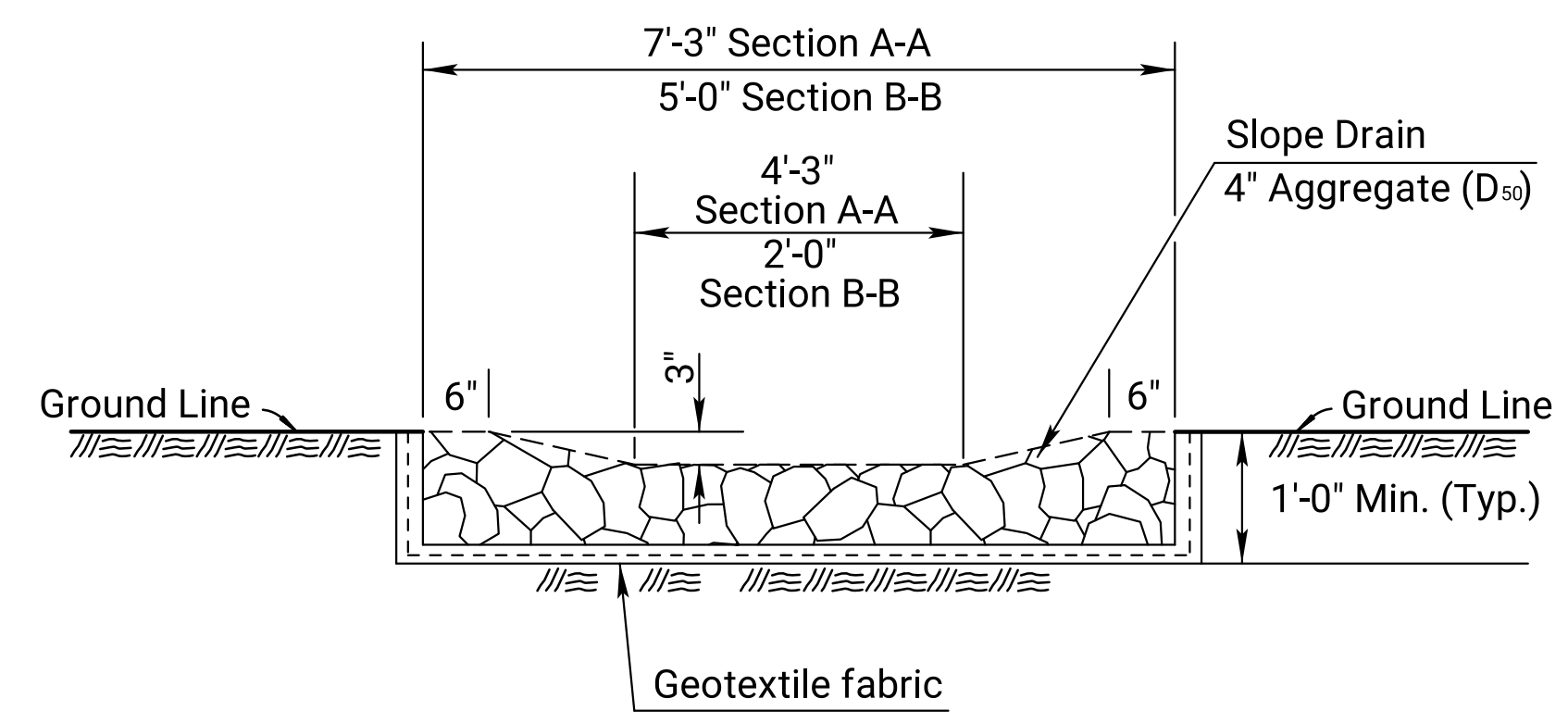
GENERAL NOTES

- Limits of "Slope Drain (Stone)" are as shown on this sheet. Limits may be adjusted as needed at the direction of the Engineer to match ground elevations found at the site.
- Gradation and aggregate for the "Slope Drain (Stone)" shall meet the requirements of stone for Aggregate Ditch Lining and have a D_{50} of 4" unless otherwise noted on the Plans.
- Excavation and grading for placement of Slope Drain and all work and material to install geotextile fabric shall be subsidiary to "Slope Drain (Stone)".
- Slope Drain shall be underlain with geotextile fabric. Fabric damaged or displaced during construction shall be replaced at no cost to KDOT. Fabric shall be installed and secured as recommended by the fabric manufacturer. One (1) copy of the fabric manufacturer's installation procedure shall be submitted to the Engineer. The installation procedure shall show details of the splices, overlaps, and pin layout. Minimum overlap of geotextile shall be 1'-0". Fabric shall be anchored along edges and splices at a maximum of 3'-0" centers with staples or pins (with washers). Interior area of fabric shall be pinned or stapled as recommended by the manufacturer but not more than 5'-0" centers. Pins or staples shall be a minimum of 1'-0" in length of embedment. Geotextile fabric shall meet the requirements of KDOT Specifications.
- The Contractor shall place the rock from the bottom to the top of the slope, as shown. Place the rock in a manner which produces a reasonably well graded mass of rock without segregation of the material sizes. Placement, measurement, and payment shall conform to KDOT Specifications for "Slope Drain (Stone)".



SECTION C-C

NOTE: Slope Drain dimensions shown are along slope.



SECTION A-A, SECTION B-B

For Information Only:
 Slope Drain (Stone) 4" Aggregate (D_{50})
 Section A-A
 0.23 cu. yds. 4" Aggregate (D_{50}) per lin. ft.
 1.03 sq. yds. Geotextile Fabric per lin. ft.
 Section B-B
 0.15 cu. yds. 4" Aggregate (D_{50}) per lin. ft.
 0.80 sq. yds. Geotextile Fabric per lin. ft.

SUMMARY OF QUANTITIES			
Item	Abut. No. 1	Abut. No. 2	
Slope Drain (Stone)	0 L.F.	76 L.F.	
Items subsidiary to Slope Drain (Stone)			
Geotextile Fabric	0 S.Y.	46 S.Y.	

NO.	DATE	REVISIONS	BY	APP'D
3	10/4/07	Bid Item Update	JPJ	KFH
2	8/30/07	Changed to Slope Drain	JPJ	KFH
1	02/14/03	Current Release	RAM	KFH

KANSAS DEPARTMENT OF TRANSPORTATION

SLOPE DRAIN (STONE)

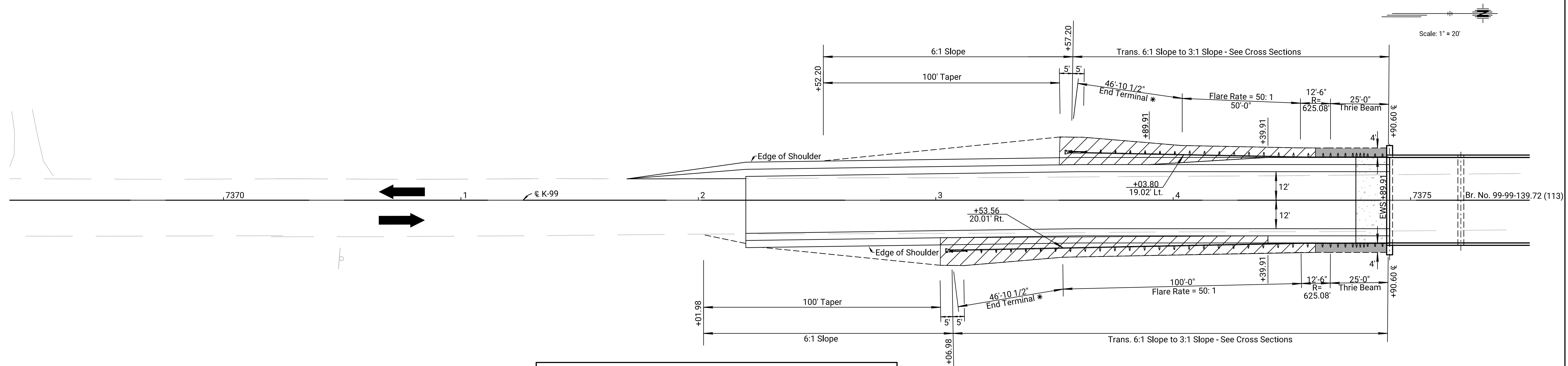
SHEET NO.	OF	SCALE	APP'D
DESIGNED		DETAILED	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.
			CADD CK.

Std. Base File: br103.dgn
 Plotted By: August Zuno
 File: ka5728rss-103-01.dgn
 Plot Date: 18-JAN-2024 09:14

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	14	135

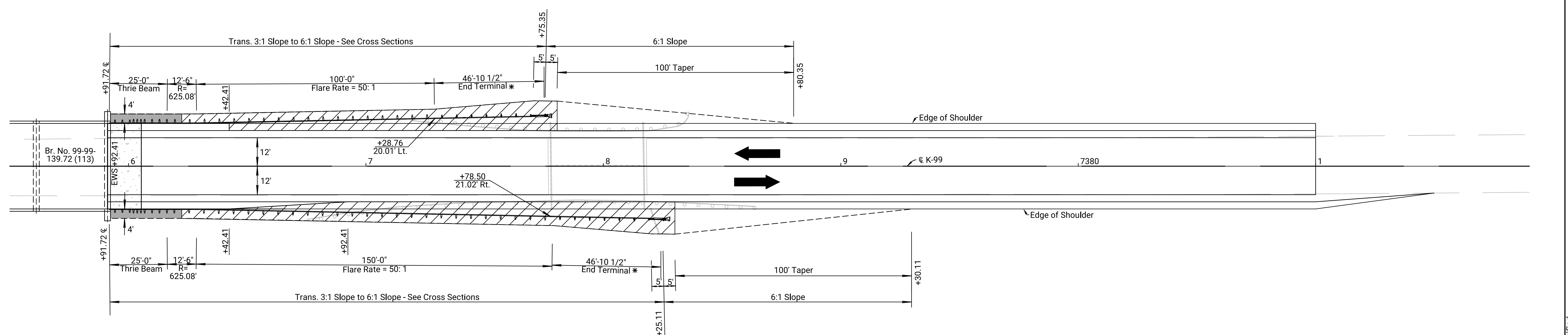
DATE	BY	REFERENCES NOTED	REFERENCES CHECKED



LEGEND

- 4" Asphalt Widening HMA Comm. Grade (Class A)
- 10" Concrete Approach Slab
- AB-3 Shoulder and Guardrail Pad

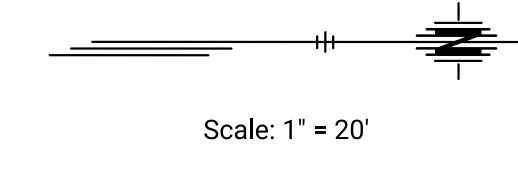
Note: All Stations and offsets are to face of blockouts.
* MGS-MSKT or MGS-SOFTSTOP End Terminal



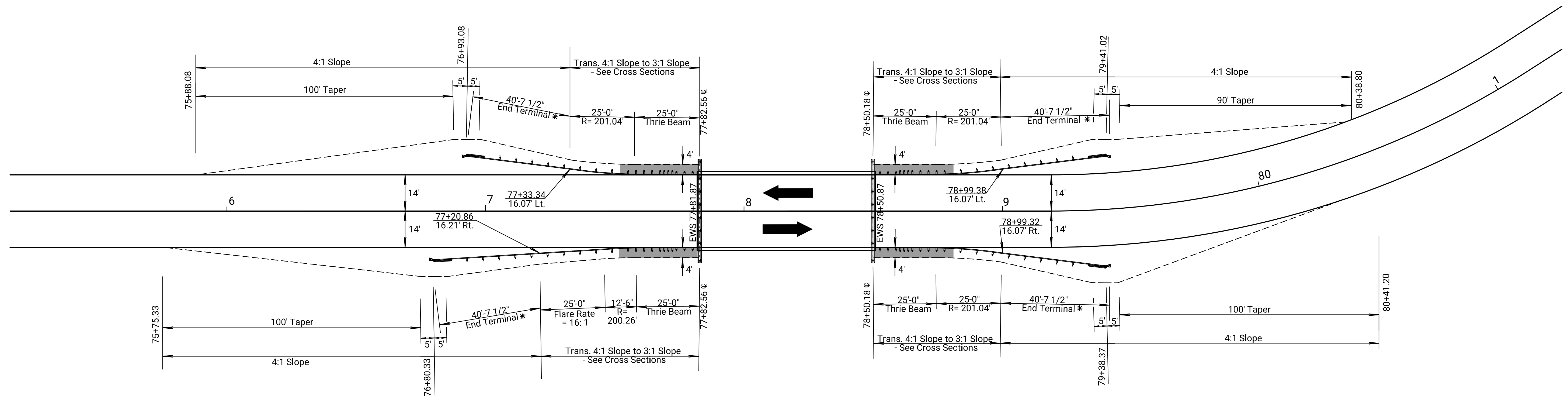
KANSAS DEPARTMENT OF TRANSPORTATION
GUARDRAIL LAYOUT
 K 99 BR. NO. 99-99-139.72 (113)

Plotted by : August Zuno 04-JAN-2024 13:59
 File : ka572801rgr-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	15	135



BY	DATE
REFERENCES NOTED	
REFERENCES CHECKED	



LEGEND

4" Asphalt Widening HMA Comm. Grade (Class A)

Note: All Stations and offsets are to face of blockouts.
 * MGS-FLEAT or MGS-SRT End Terminal

Plotted by : August Zuno 04-JAN-2024 14:02
 File : ka572801rgr-02.dgn

KANSAS DEPARTMENT OF TRANSPORTATION
**SHOOFLY GUARDRAIL
 DETAIL SHEET**

KDOT Graphics Certified

Note to Designer - Design guardrail installations using guidance shown on KDOT's 'Guardrail Typical Alignments' Standard Drawings. 'Flared' guardrail installations are preferred over 'Parallel' or 'Zero Flare' installations. Where 'Flared' or 'Parallel' installations are used, the flare rate of the guardrail end terminal typically matches the flare rate of the remaining guardrail installation. For 'Zero Flare' installations, 'Parallel' guardrail end terminals should be designed using typical flare rates of 50:1 or flatter for the length of the end terminal. However, while 50:1 or flatter flare rates are typical for 'Parallel' guardrail end terminals, these end terminals may be flared as steep as 26:1 or flatter in order to offset the end terminal head as far from the edge of the through traveled lane as practicable.

Plotted by: August Zuno
 File: rrs606.dgn
 5-JAN-2024 07:37

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	16	135

GENERAL NOTES

Install the guardrail end terminals according to the Manufacturer's Installation Manual. The Contractor will furnish a copy of the Manufacturer's Installation Manual to the Engineer prior to the start of the installation.

Use approved steel (preferred) or wood posts provided by the Manufacturer. The guardrail end terminal post type may be independent of the post type used in the remainder of the installation. However, no mixing of post types is permitted in the remaining w-beam and thrie-beam installation.

Use approved polymer (preferred) or wood blockouts provided by the Manufacturer. The guardrail end terminal blockout size and type may be independent of the blockout size and type used in the remainder of the installation. For blockout size and types for the remaining w-beam and thrie-beam portion of the installation see the details shown on KDOT's 'Guardrail Post Details' and 'Guardrail Thrie-Beam Transition Details' Standard Drawings.

Apply retroreflective sheeting to the end terminal impact head before installation.

Tighten all cable anchor assemblies as per the Manufacturer's Installation Manual.

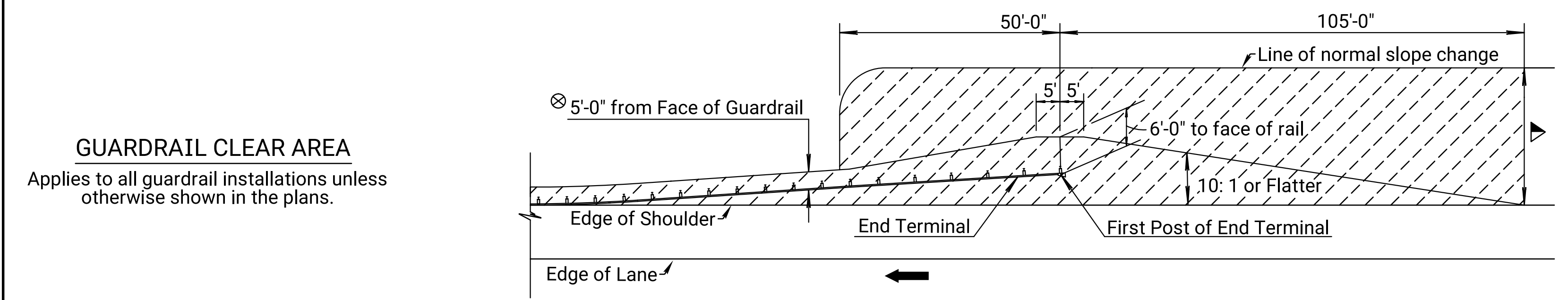
Lap w-beam and thrie-beam guardrail splices, in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final configuration. Lap end terminal splices per the Manufacturer's Installation Manual in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final configuration.

The minimum length of w-beam guardrail required between the thrie-beam transition and the guardrail end terminal is 12'-6" for all installations; unless otherwise stated in the Manufacturer's Installation Manual.

Where pavement with a thickness less than or equal to 8" is encountered during installation, use the details shown on KDOT's 'Guardrail Post Details' Standard Drawings to provide openings in the pavement for the guardrail posts. Where pavement with a thickness greater than 8" or geologic rock is encountered during installation, follow the Manufacturer's Installation Manual for guidance. Where the Manufacturer's Installation Manual does not address pavement with a thickness greater than 8" or geologic rock, contact the manufacturer for instructions or install the guardrail posts as directed by the Engineer.

All work and materials required for w-beam and thrie-beam guardrail installations are paid for under the appropriate bid items for either CGS or MGS guardrail depending on the type of installation.

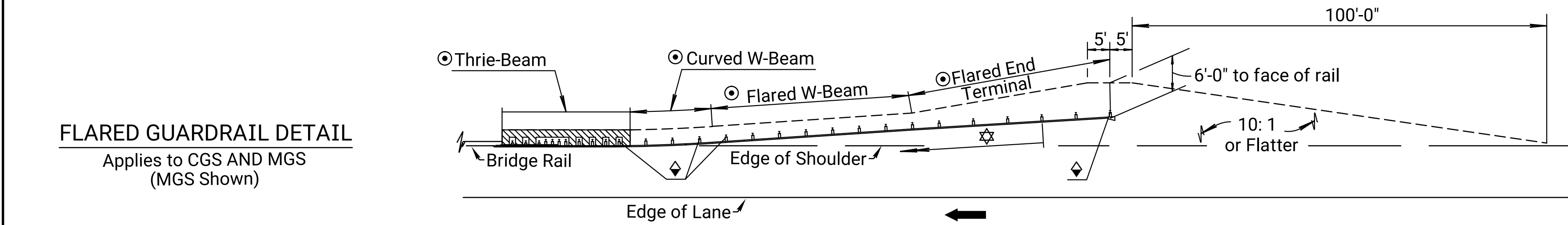
All work and materials required for guardrail end terminal installations are paid for under the bid item for the selected guardrail end terminal. See the table on this sheet for the appropriate end terminal bid item information.



Keep Area Free of Stockpiled Material, Equipment, or Other Obstacles, Such as Temporary Signs, Regardless of Crash Worthiness. This Clear Area Extends 105 Feet in Advance of and 50 Feet behind the First Post of the Guardrail End Terminal and Then, in Order to Maintain Full Post Spacing, Continues 5 Feet behind the Face of the Guardrail through the W-Beam Portion of the Installation as Shown in the 'Guardrail Clear Area' Detail on this Sheet.

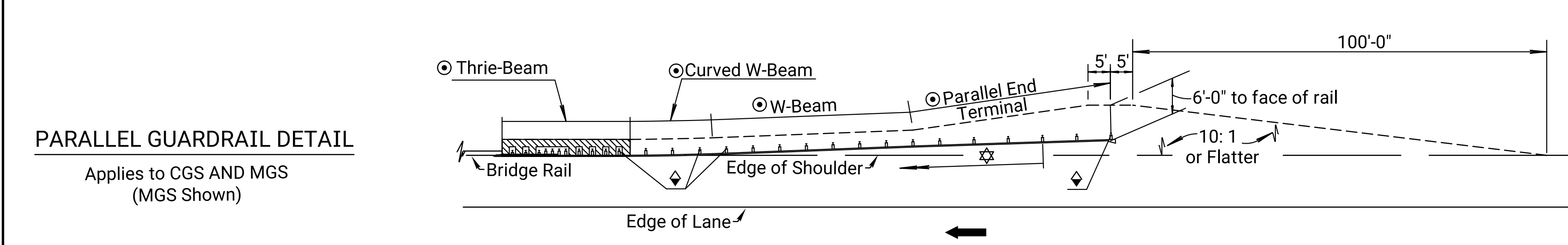
▲ Normal Project Side Slope.

⊗ Deflection Distance for Normal Post Spacing

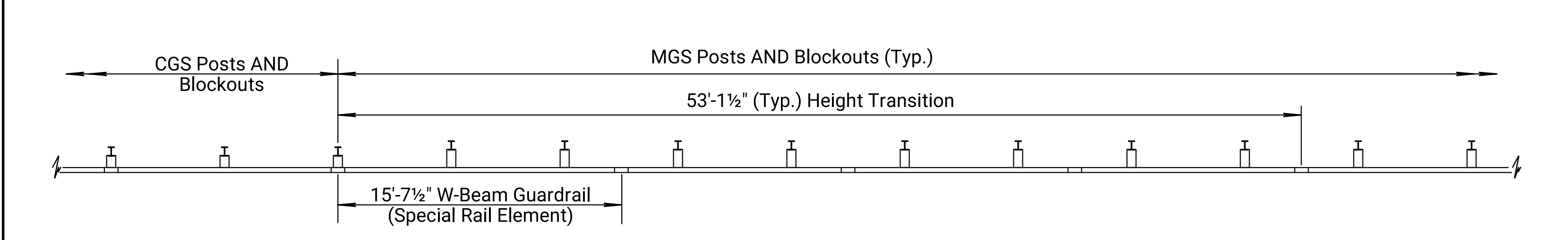


FLARED GUARDRAIL DETAIL
 Applies to CGS AND MGS (MGS Shown)

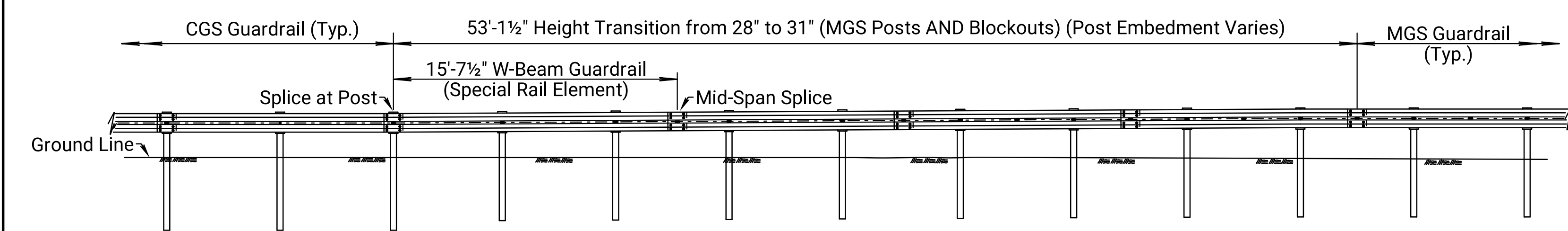
⊙ See Guardrail Layout Sheets for Details
 ◆ On Guardrail Layout Sheets, Show Station AND Offset from the Roadway Alignment to the Face of Post at these Locations.
 ☆ Length of Need (Begins at Post 3)



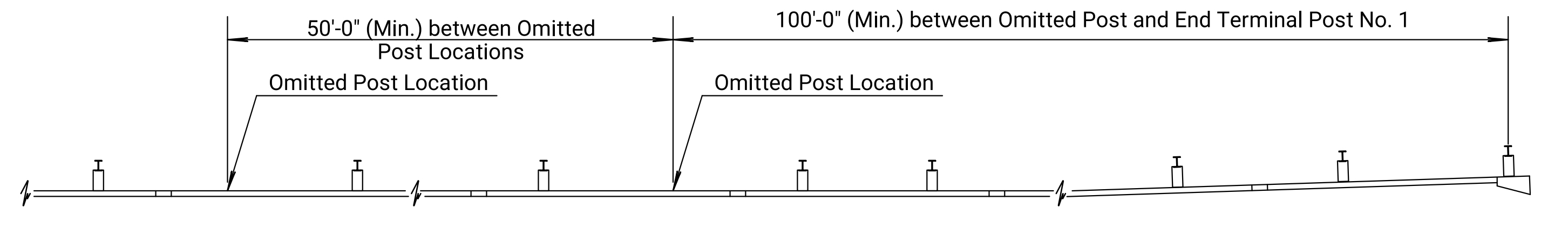
PARALLEL GUARDRAIL DETAIL
 Applies to CGS AND MGS (MGS Shown)



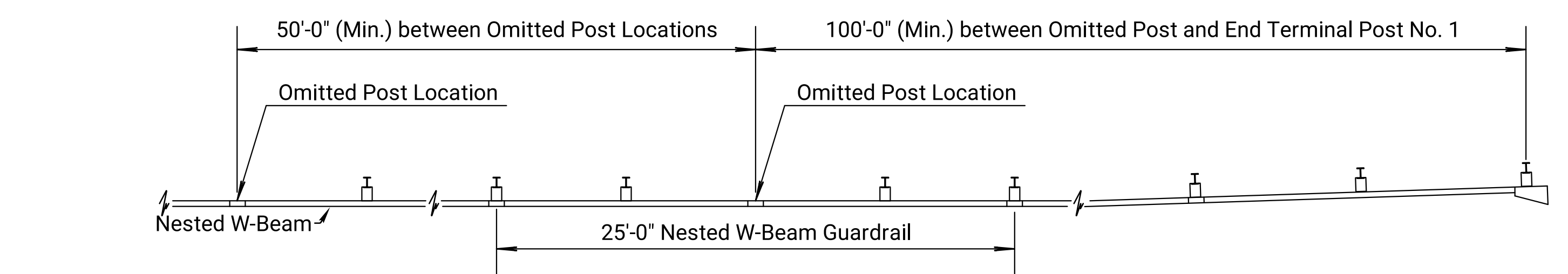
CGS TO MGS TRANSITION DETAILS (PLAN)



CGS TO MGS TRANSITION DETAILS (ELEVATION)



MGS OMITTED POST DETAIL



CGS OMITTED POST DETAIL

MIDWEST GUARDRAIL SYSTEM (MGS) END TERMINALS									
END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (MGS-FLEAT)	Flared	31"	NCHRP 350	Yes	Yes	Yes	Road Systems	40'-7½"	37'-6"
Guardrail End Terminal (MGS-SRT)	Flared	31"	NCHRP 350	Yes	Yes	No	Trinity Industries	40'-7½"	37'-6"
Guardrail End Terminal (MGS-MSKT)	Parallel	31"	MASH	Yes	No	Yes	Road Systems	46'-10½"	46'-10½"
Guardrail End Terminal (MGS-SOFTSTOP)	Parallel	31"	MASH	Yes	No	Yes	Trinity Industries	46'-10½"	50'-9½"

02	09-05-18	ADD. OMITTED POST AND TRANS. DETAILS	A.L.R.	T.T.R.
01	06-05-18	INITIAL RELEASE	A.L.R.	T.T.R.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

GUARDRAIL AUXILIARY DETAILS

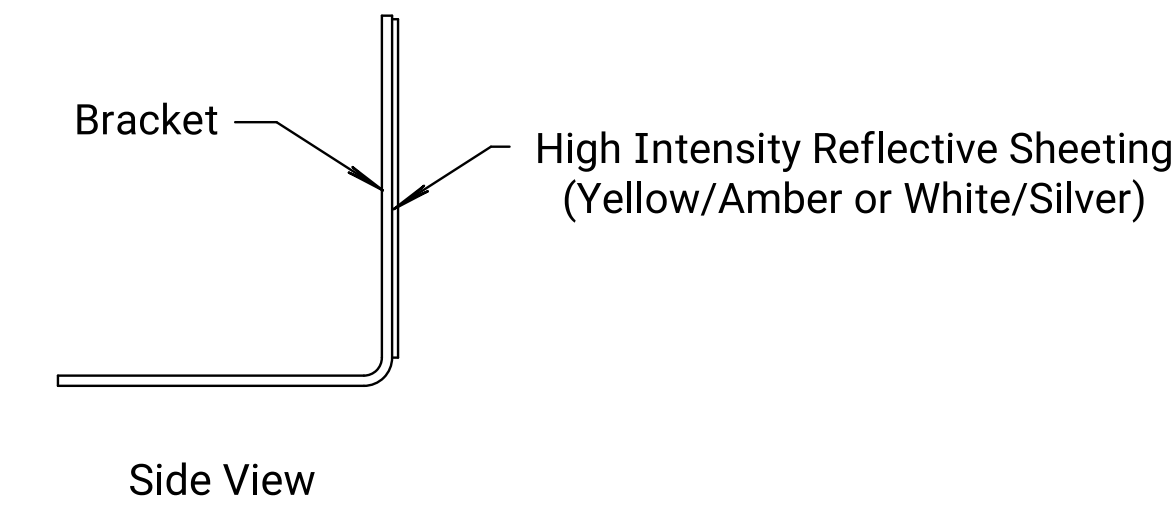
RD606

Scott W. King

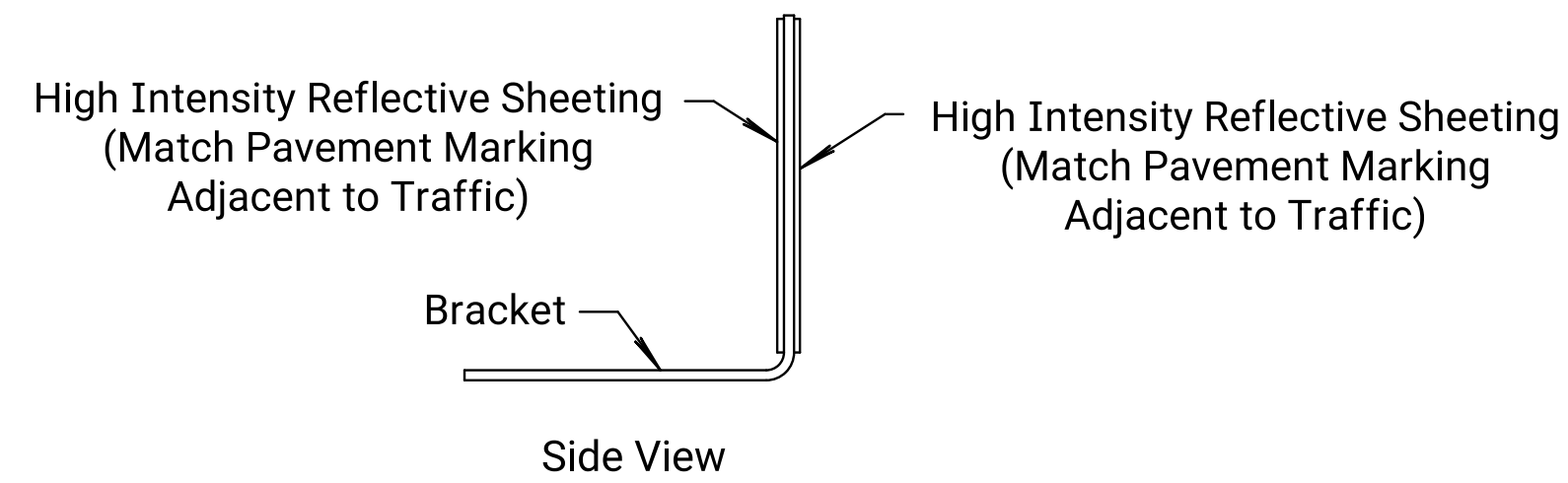
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

KDOT Graphics Certified

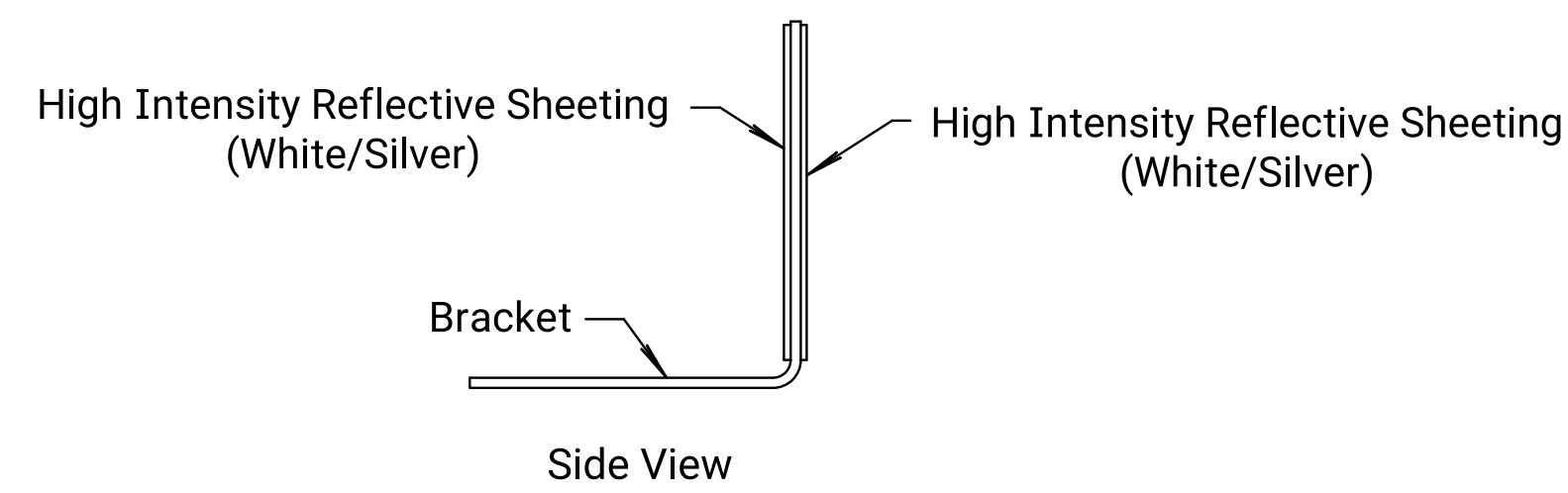
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	17	135



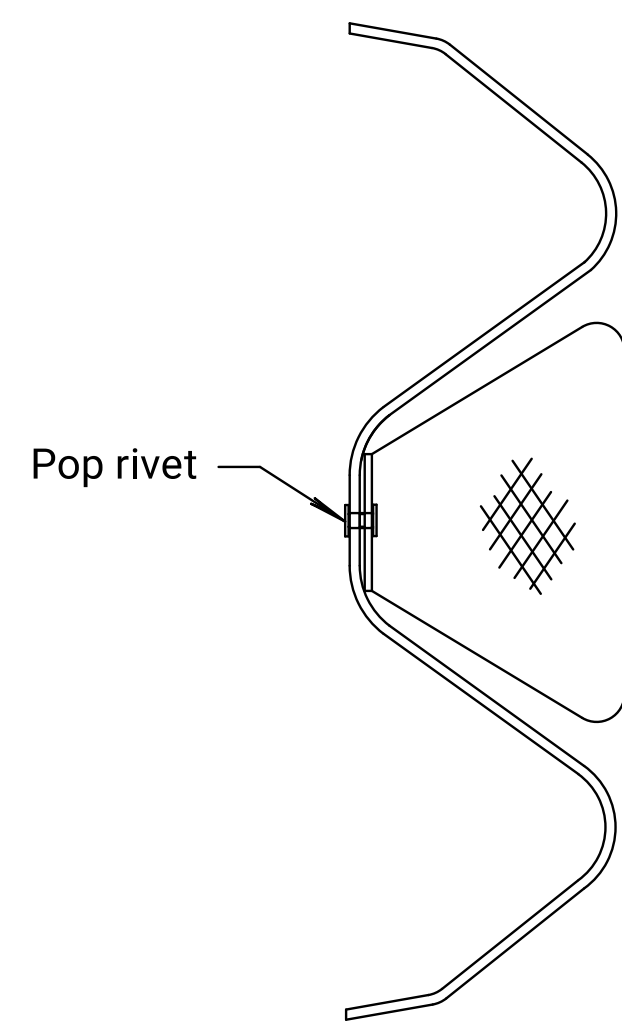
Flexible Marker
One-Way Traffic



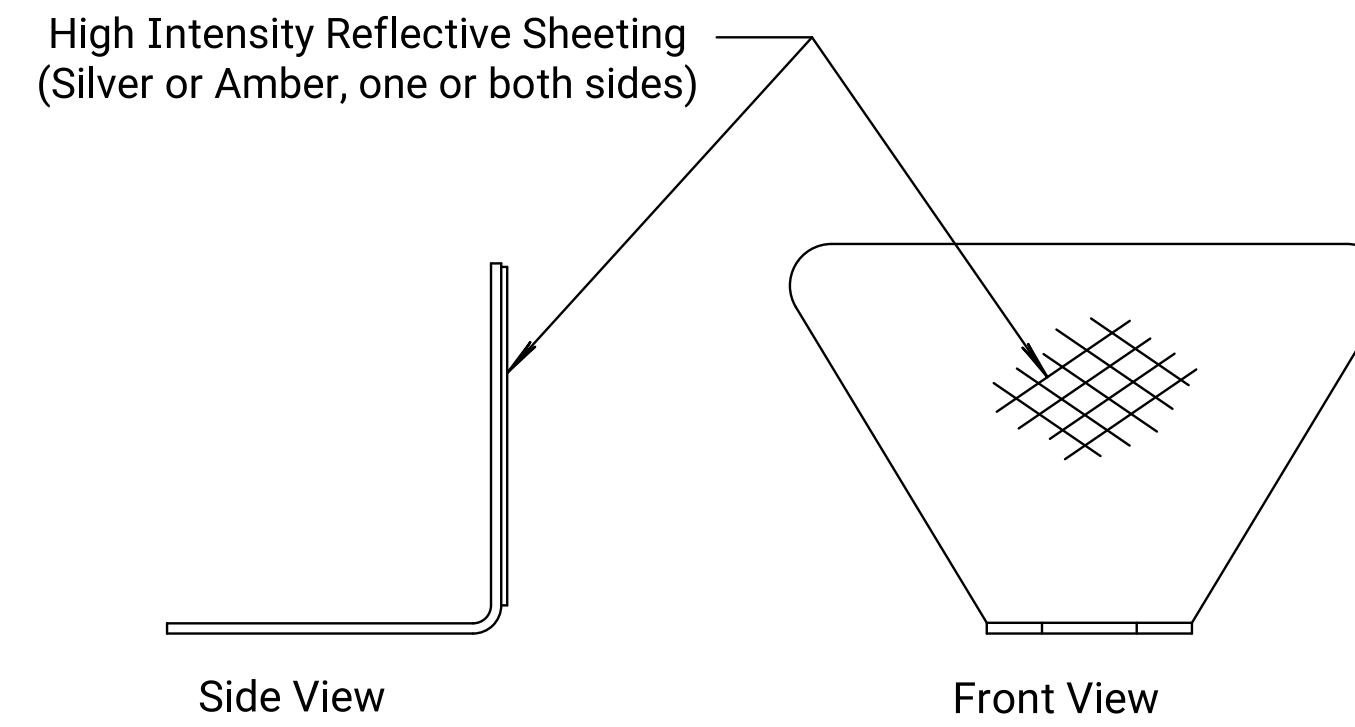
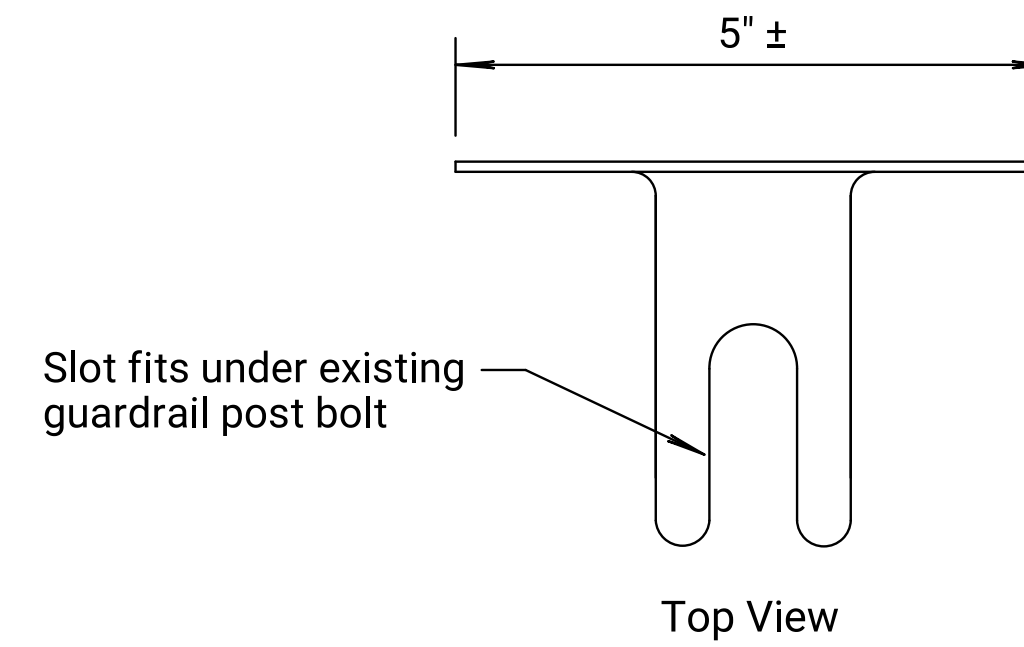
Flexible Marker
Median Locations



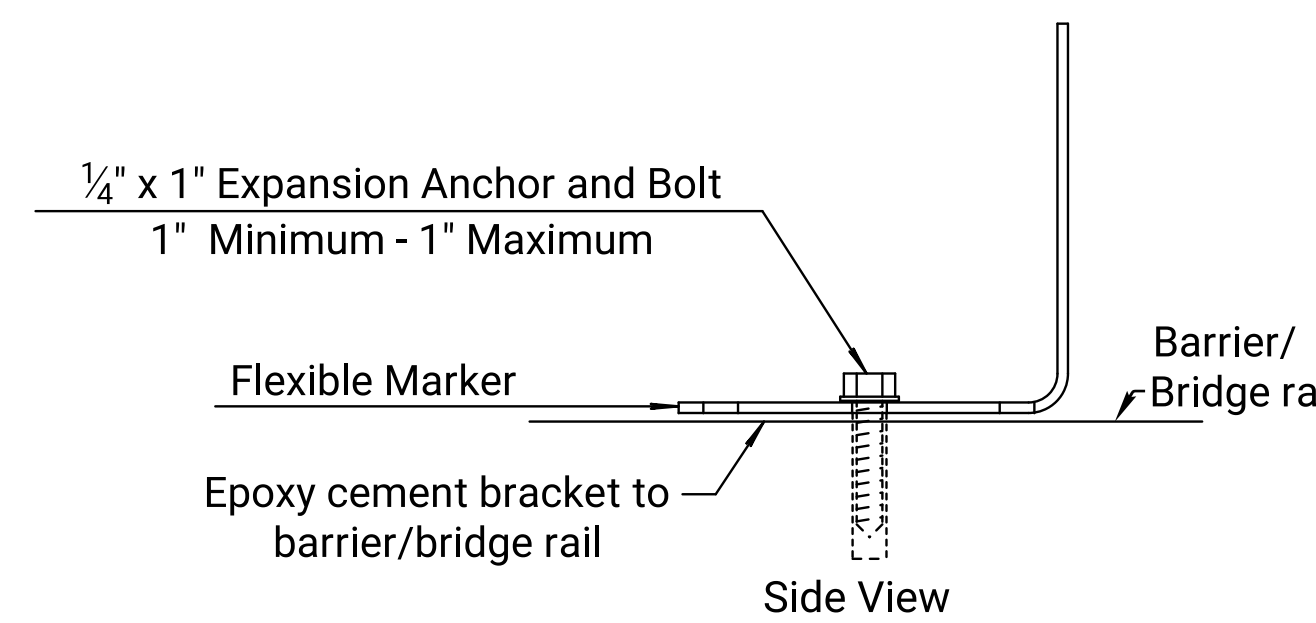
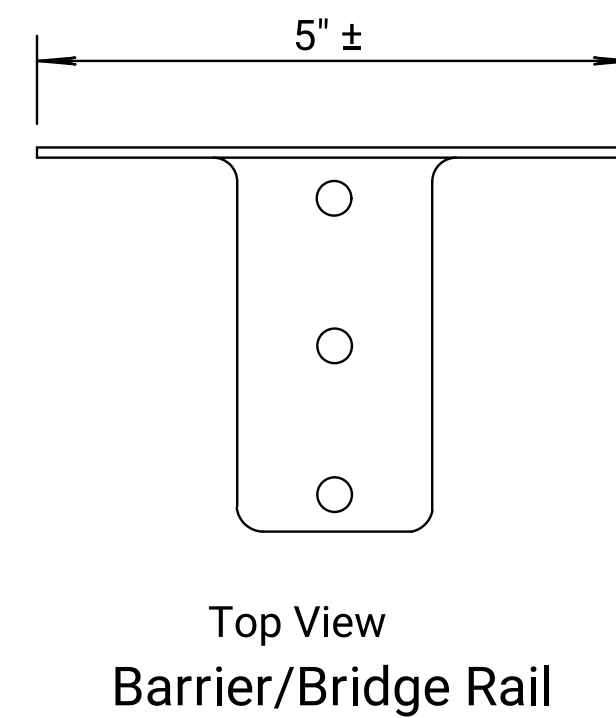
Flexible Marker
Two-Way Traffic



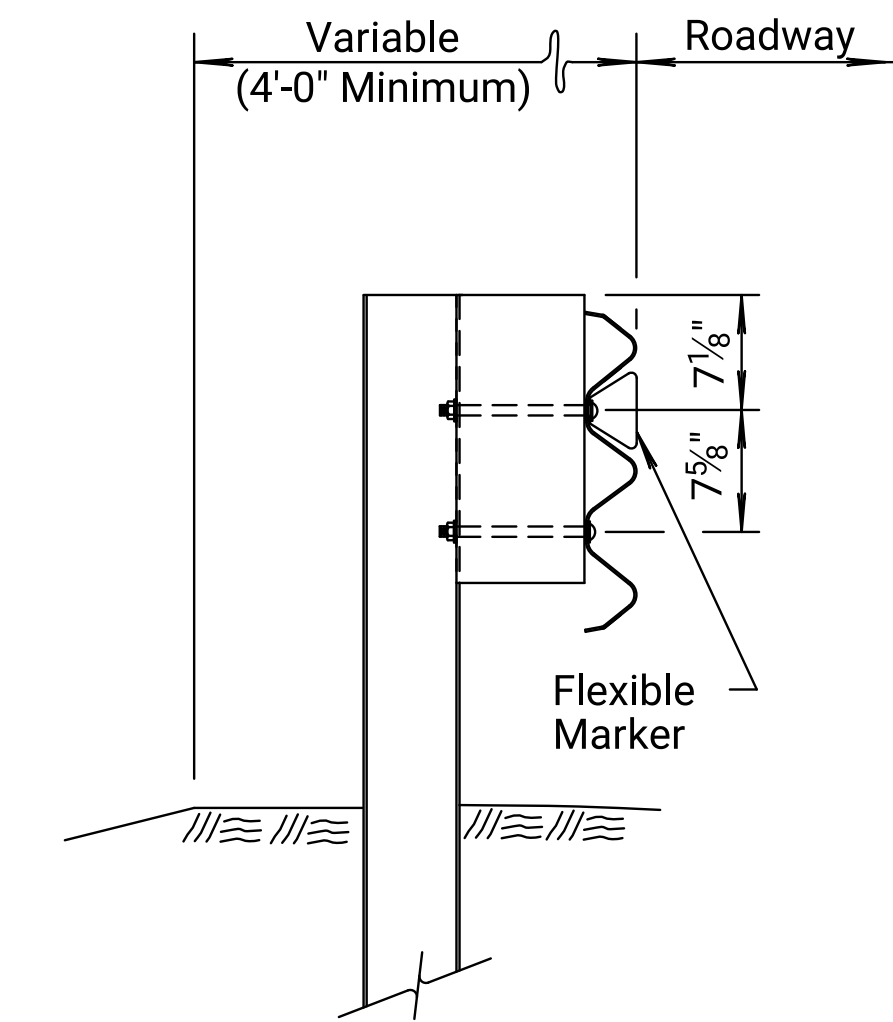
Typical Mounting on W-Beam
Pop rivet attachment to Guardrail when necessary.



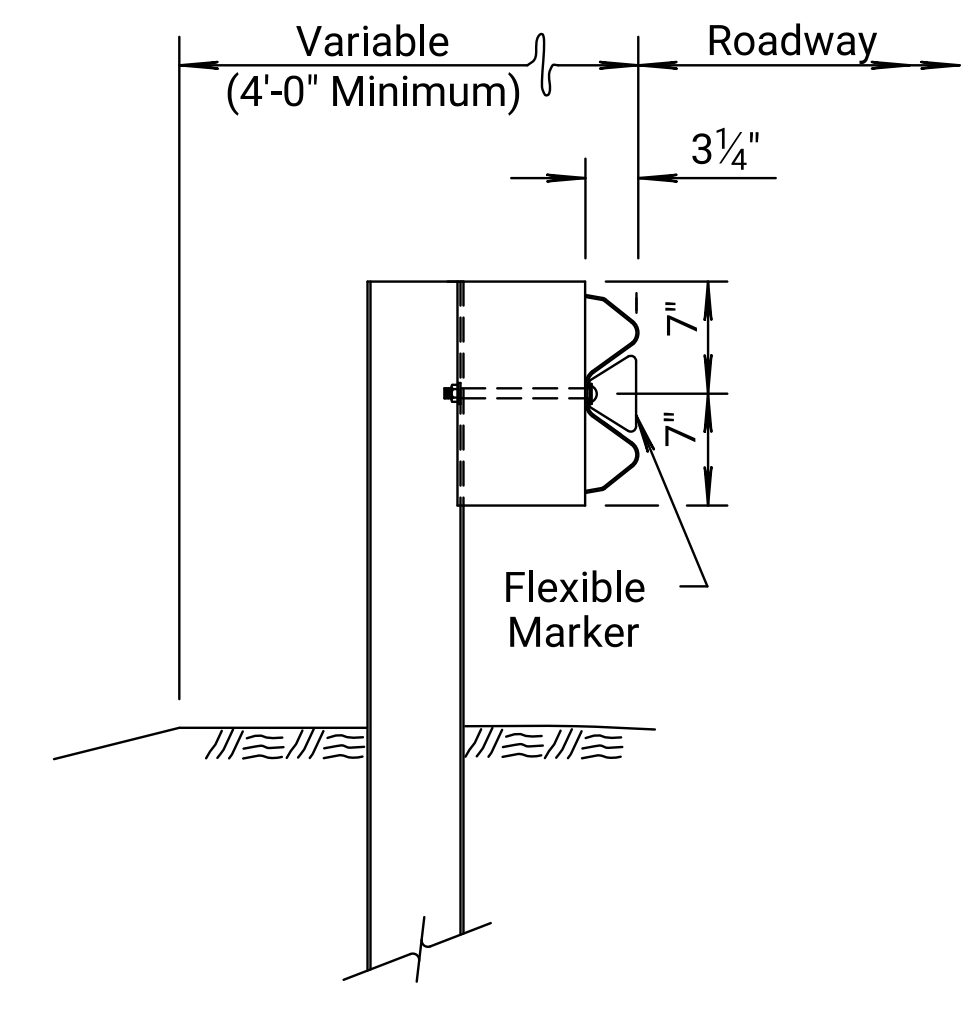
Flexible Guardrail Marker
(High Impact Polycarbonate approx. .085" thick, 5 1/4" x 3")



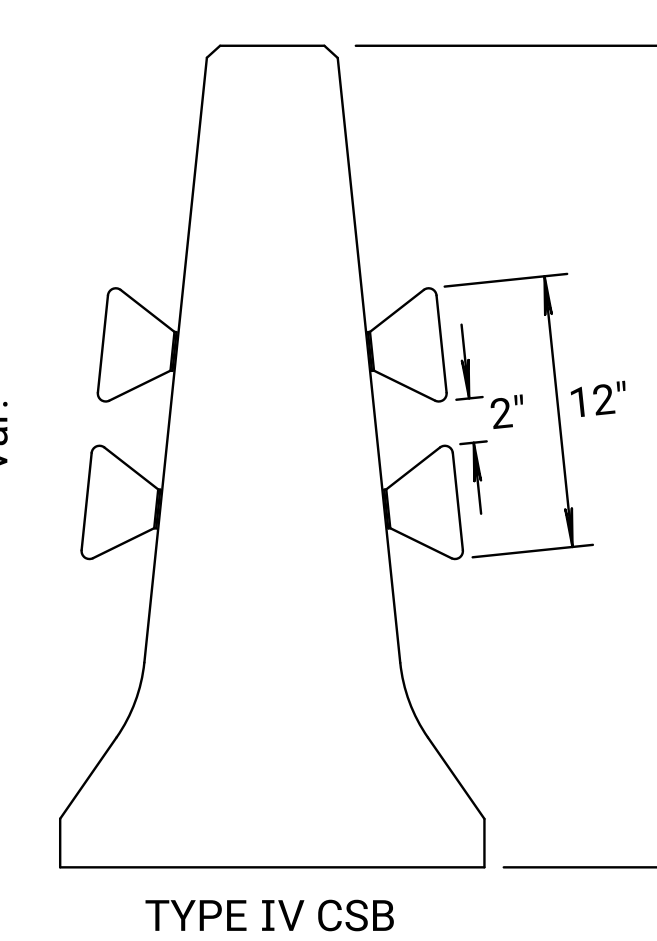
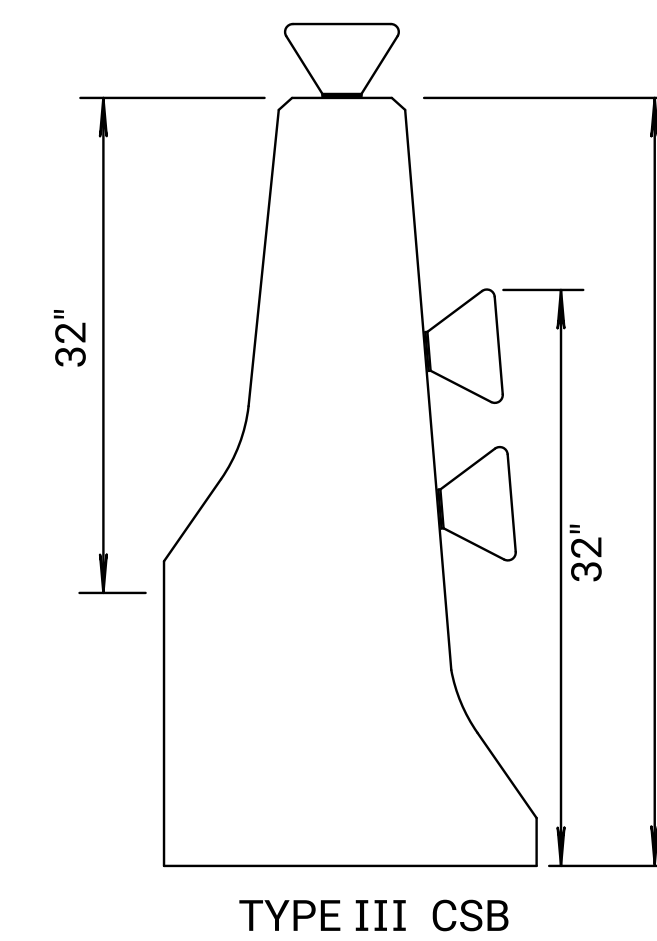
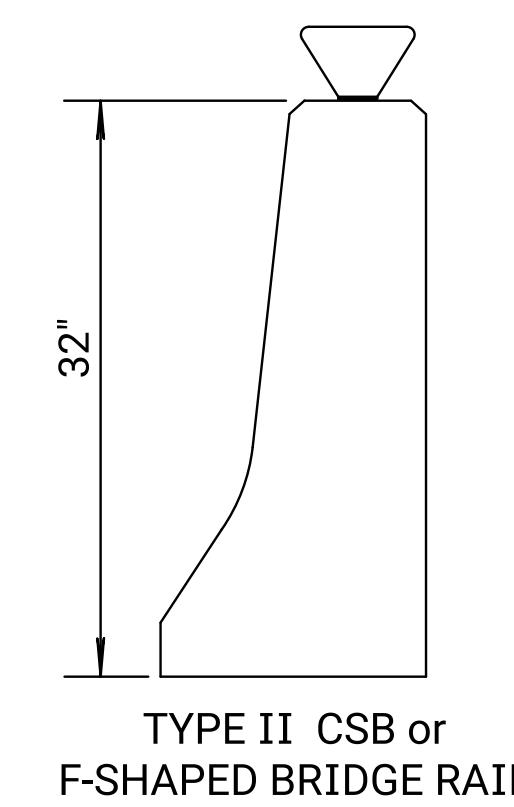
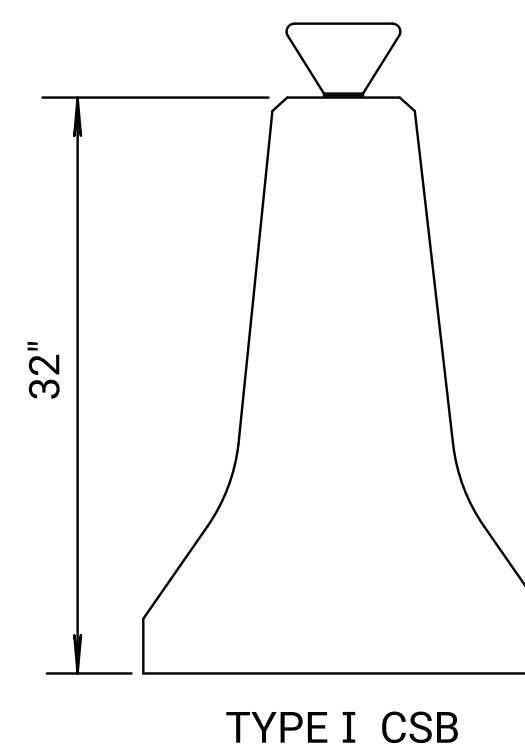
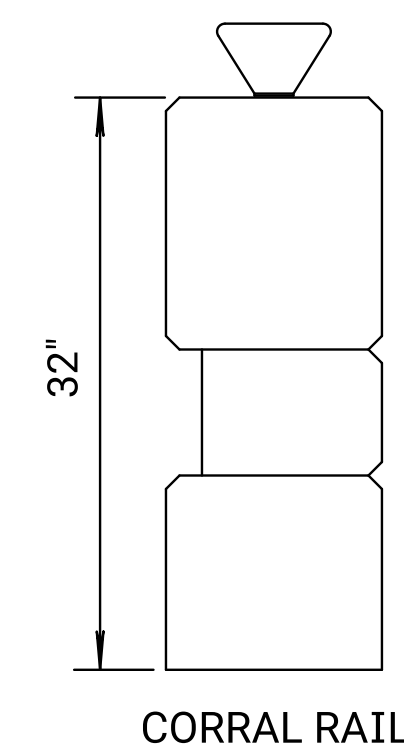
Method of Attaching Flexible
Marker to Barrier/Bridge Rail



THRIE-BEAM GUARDRAIL



W-BEAM GUARDRAIL



TYPICAL BARRIER/BRIDGE RAIL MOUNTING DETAILS

GENERAL NOTES

Install flexible markers on a post behind the guardrail bolt head on the traffic side of guardrail installations at a spacing not to exceed 25'. No marker is installed between the head and post #5 when the guardrail is terminated with a crashworthy end terminal.

Install flexible markers on the top of bridge rails at a spacing not to exceed 50', except for long bridges (greater than 200' long), where spacing may be increased to 100'.

Install flexible markers on the top of concrete safety barrier at a spacing not to exceed 100', except for barrier along a horizontal curve or along ramps and ramp tapers, where spacing is not to exceed 50'.

Where the height of the bridge rail or concrete barrier is greater than 32", mount the flexible markers on the side of the barrier at a height of 32" as shown on this sheet.

For guardrail, bridge rail, or concrete safety barrier located on two-way roadways, use flexible markers with white/silver high intensity reflective sheeting on both sides.

For guardrail located on one-way or divided roadways, use flexible markers with reflective sheeting installed on the approach traffic side of the bracket only. For bridge rail or concrete safety barrier located on the outside edge of one-way or divided roadways, use flexible markers with reflective sheeting installed on the approach traffic side of the bracket only. For bridge rail or concrete safety barrier located in the median, use flexible markers with reflective sheeting installed on both sides of the bracket. Match the color of the marker (yellow/amber or white/silver) to the color of the pavement marking adjacent to the traffic lane.

Use High Impact Polycarbonate Flexible Guardrail Marker with High Intensity Reflective Sheeting or an approved equivalent, see Standard Specifications.

Use zinc or cadmium plated fasteners that comply with Standard Specifications.

Work and materials required for installation of markers on guardrail, bridge rail, or concrete safety barrier are subsidiary to other bid items in the contract.

Install flexible markers for the final (permanent) traffic configuration.

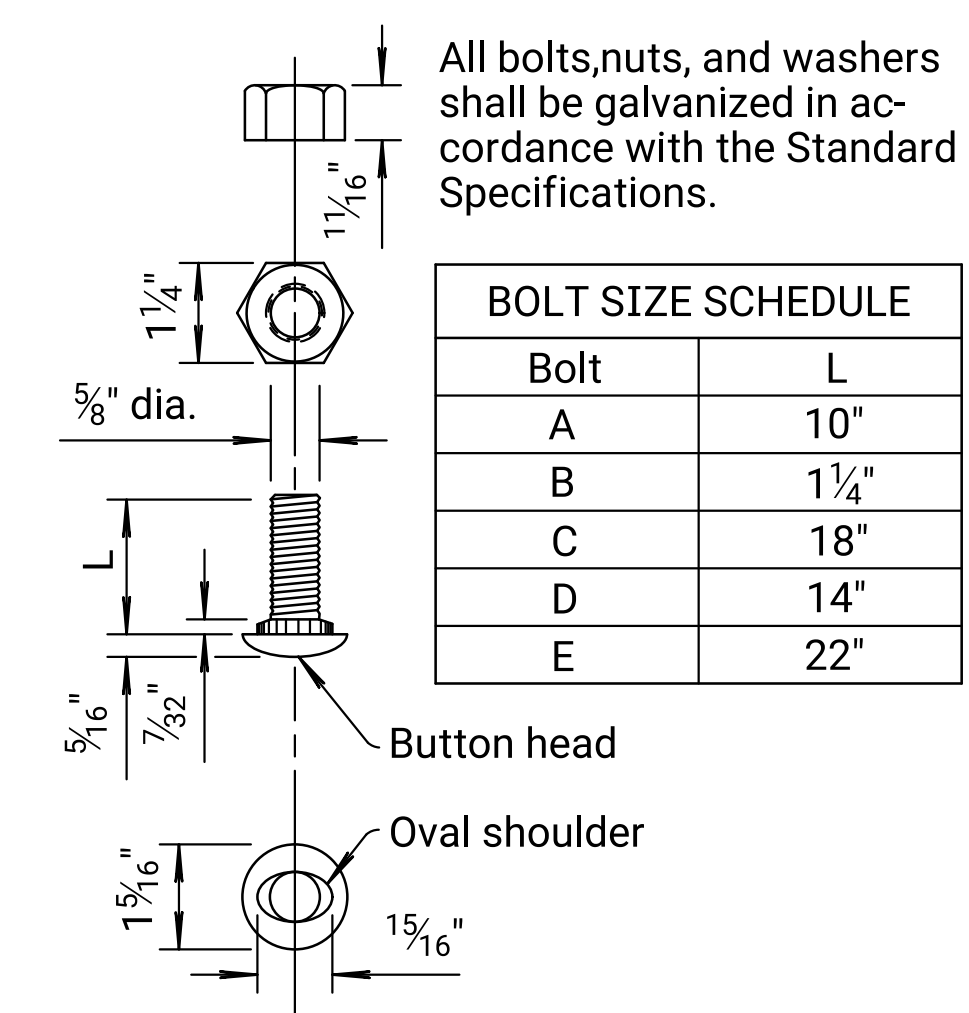
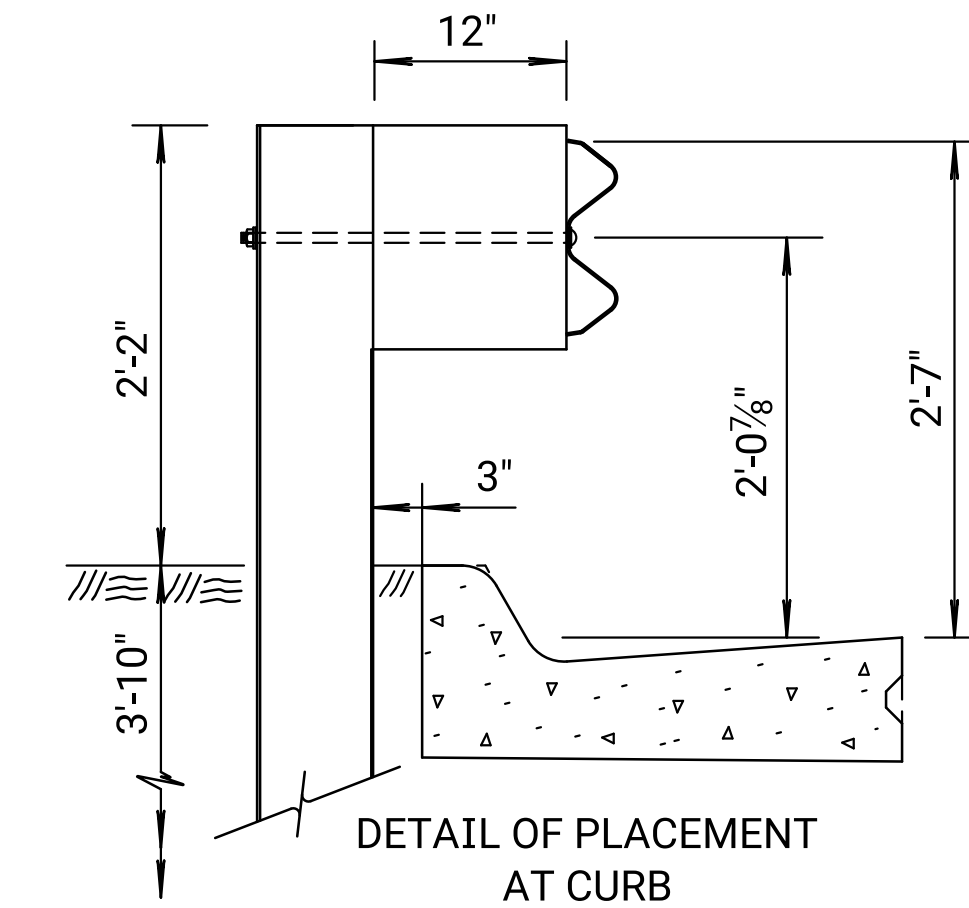
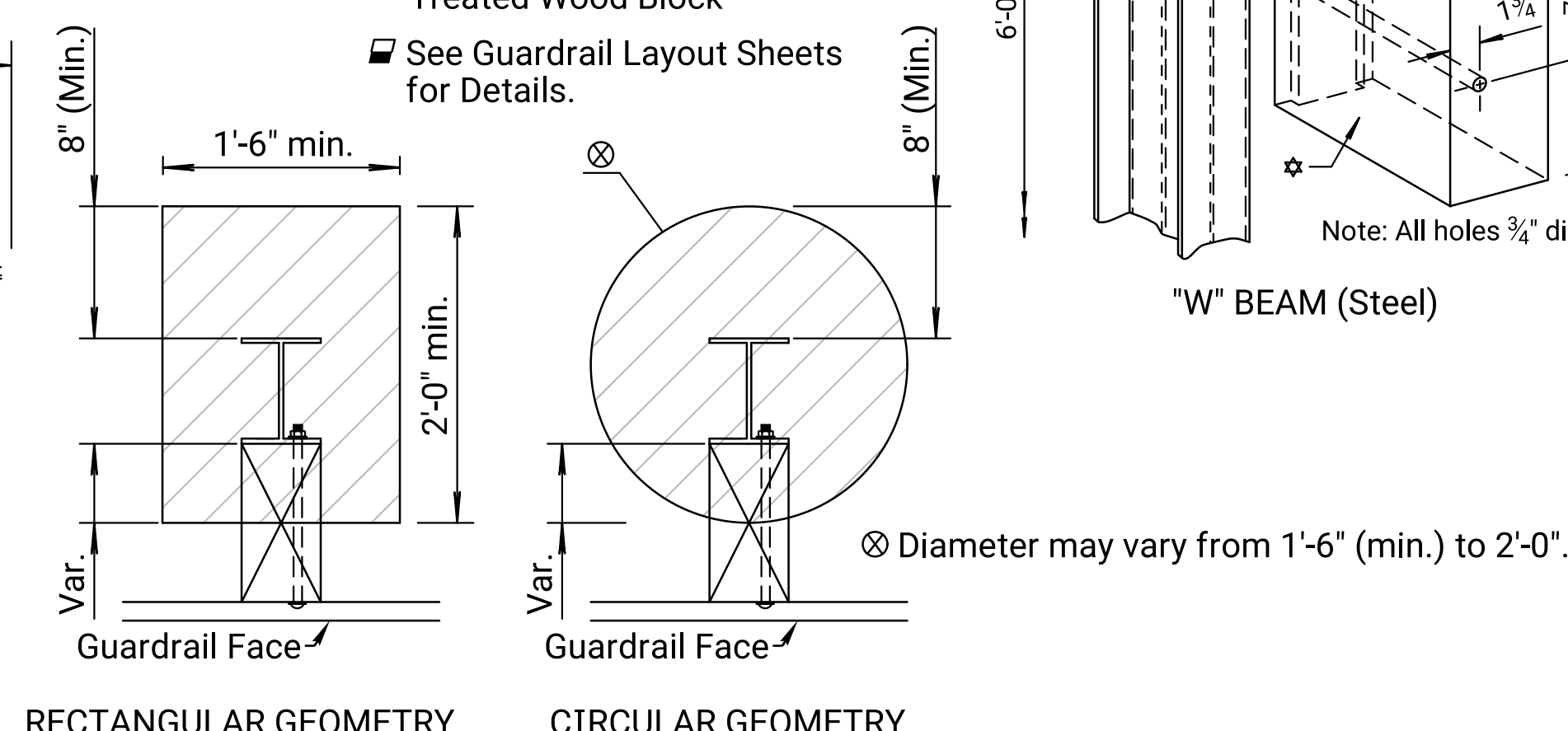
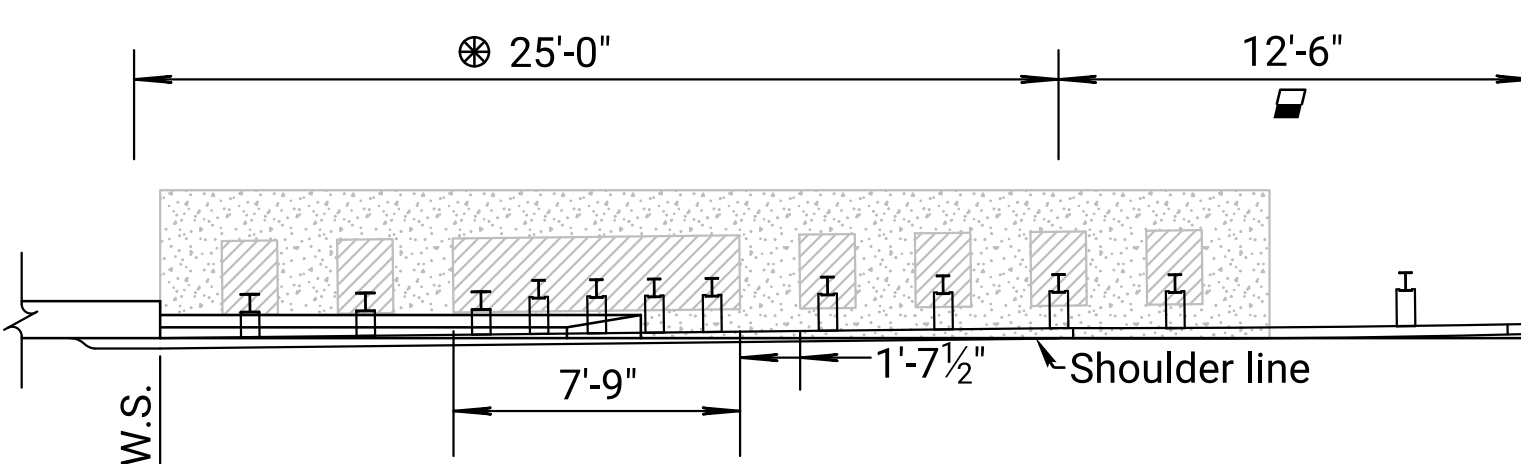
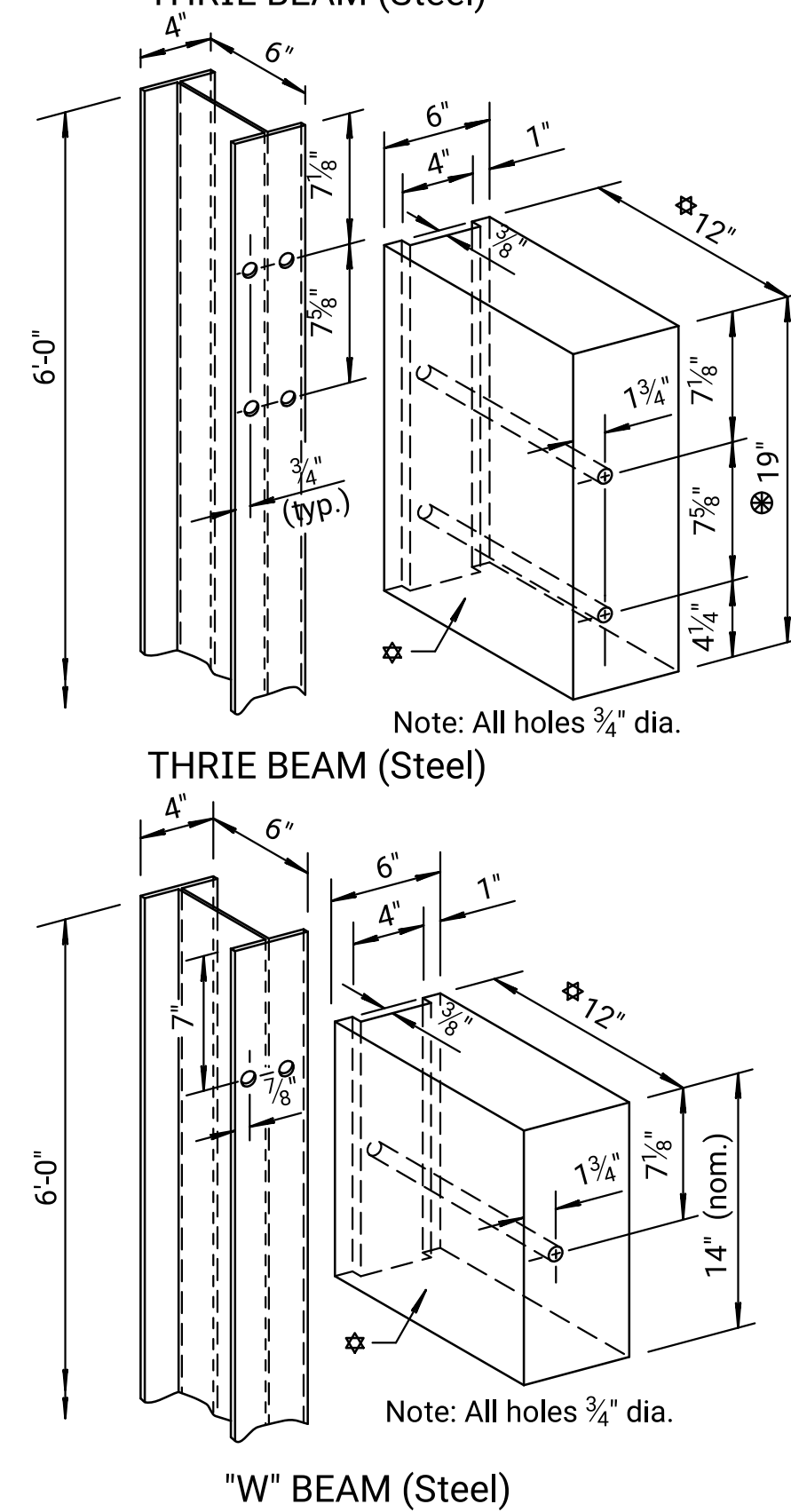
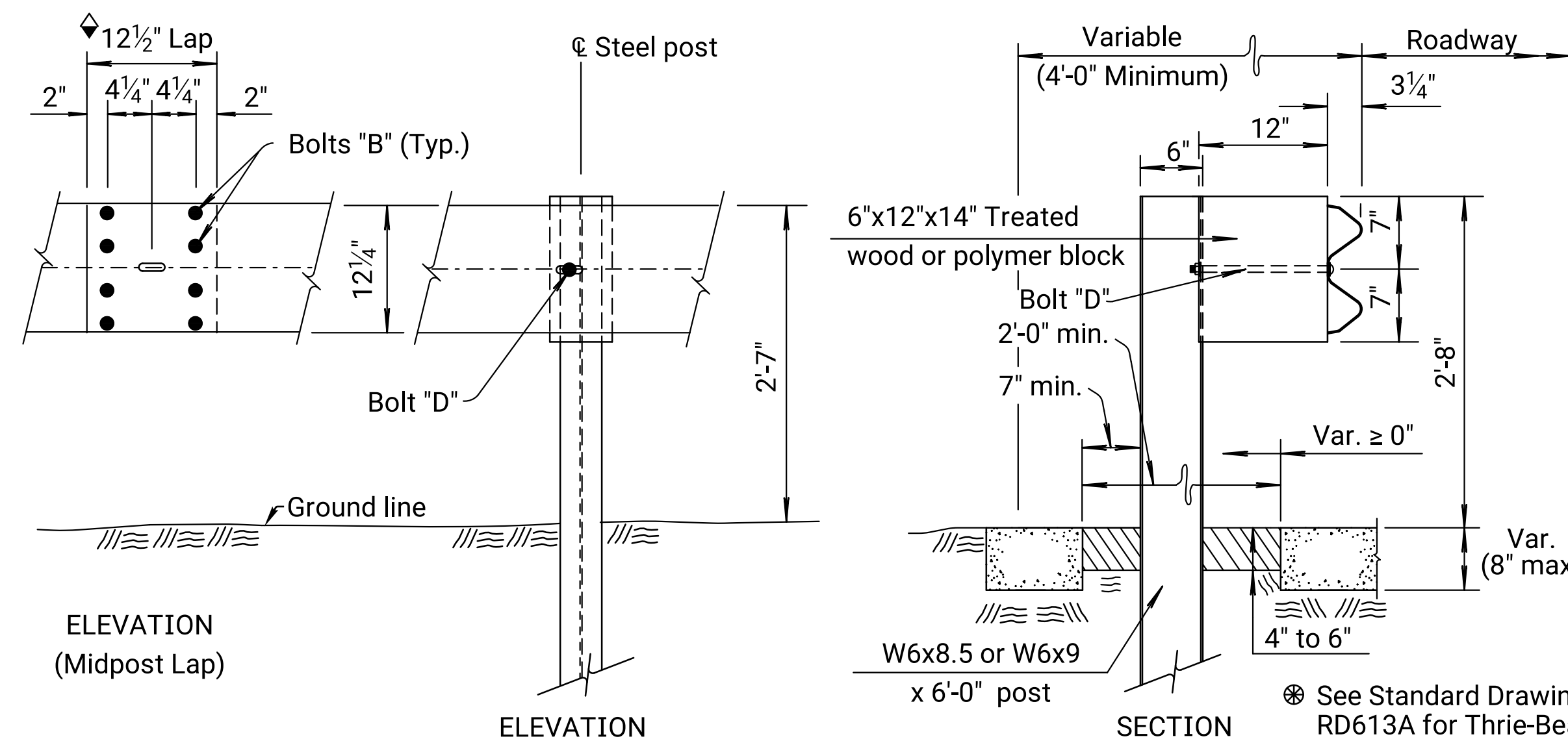
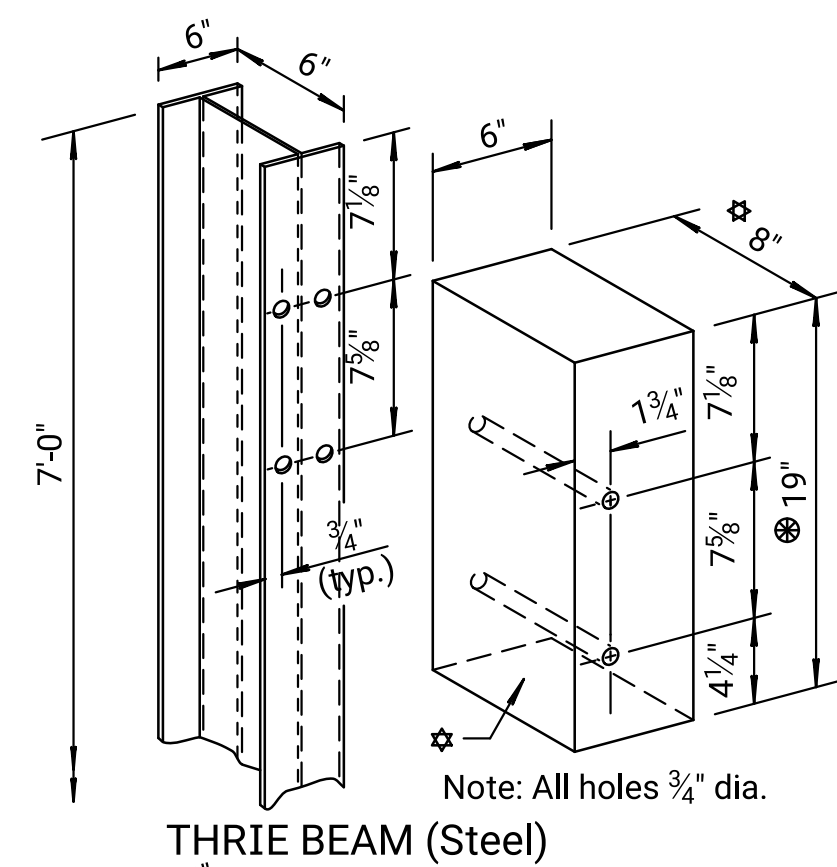
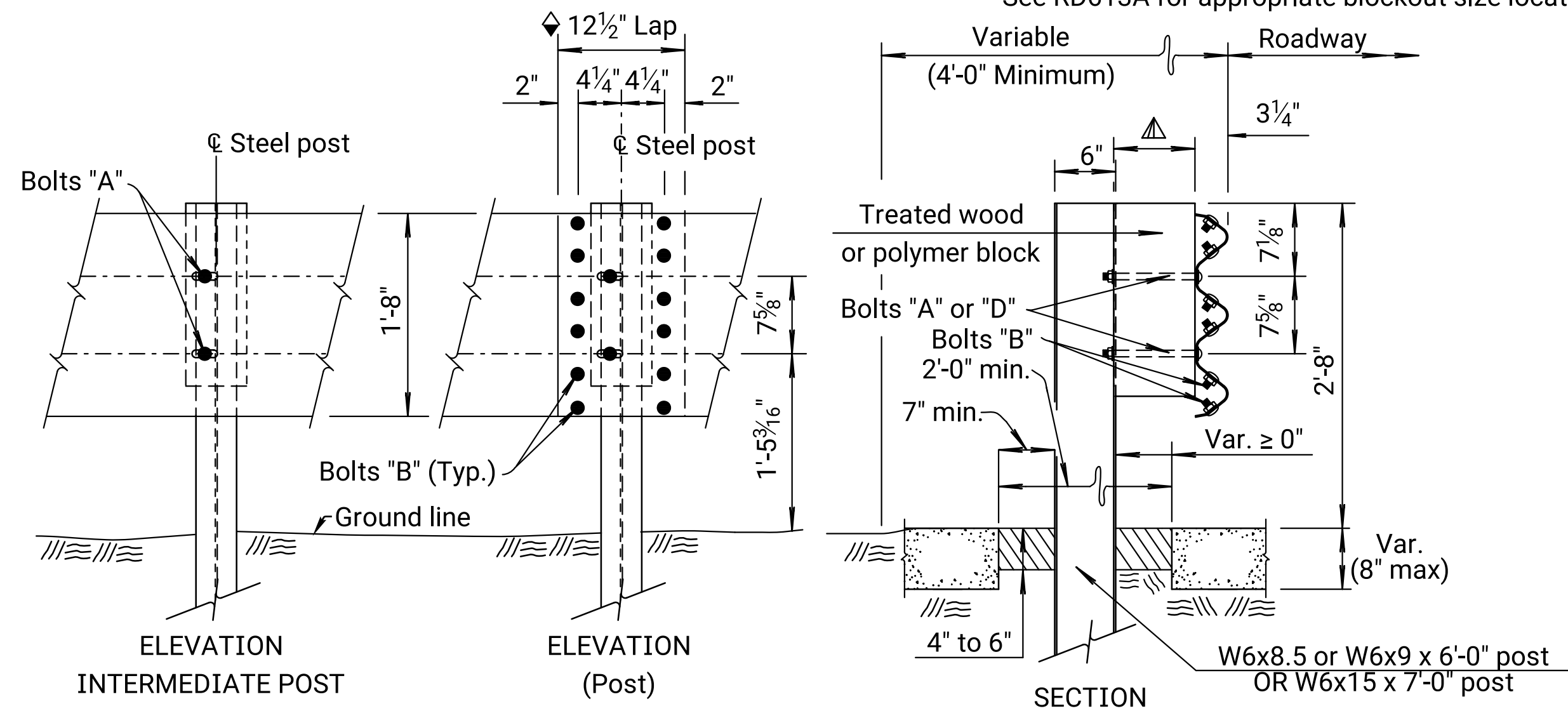
09	09-11-17	Rev. Det. Markers, Rev. Gen. Note	A.L.R.	S.W.K.
08	11-15-10	Revised notes	S.W.K.	J.O.B.
07	12-21-08	AKT marker or approved equal	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION				
MARKER DETAILS FOR GUARDRAIL, BARRIER, AND BRIDGE RAILS				
RD610				
FHWA APPROVAL	03-15-18	APPD.	Scott W. King	
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	18	135

◆ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

▲ See RD613A for appropriate blackout size location.



All bolts, nuts, and washers shall be galvanized in accordance with the Standard Specifications.

BOLT SIZE SCHEDULE	
Bolt	L
A	10"
B	1 1/4"
C	18"
D	14"
E	22"

Note: Measure height of rail from the pavement surface at the curb/pavement joint as shown. A special design is needed when guardrail is not located as detailed. A Type II (laydown) curb & gutter is preferred when guardrail is adjacent to curb.

Notes to Designer: For posts installed in pavement thicker than 8" or posts installed in rock formations refer to AASHTO's Roadside Design Guide for details then revise this drawing and all supporting drawings appropriately.

Plotted by : August.Zuno 5-JAN-2024 07:44 File : rrs611a.dgn

GENERAL NOTES (Steel Posts)

Use grade of steel for steel posts that meet the requirements of the standard specifications.

Hot dip galvanize the posts after fabrication, see standard specifications.

Wood blockouts may be used through the 25'-0" thrie-beam section with wood or polymer blockouts used throughout the remainder of the w-beam installation. The blackout size and material used in the guardrail end terminal may be independent from the remainder of the installation. For wood/polymer blackout requirements see standard specifications.

Use S4S rectangular blockouts for Thrie-Beam/W-Beam installation.

Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations.

Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered that prevents installation of a full length post.

All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made.

NO.	DATE	REVISIONS	BY	APPD
05	09-24-15	Separated Steel/Wood Post Details	S.W.K.	S.W.K.
04	11-08-12	Revised Detail, Posts in Pavement	S.W.K.	J.O.B.
03	08-01-12	Revised Note to Designer	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

GUARDRAIL POST (STEEL) (MGS) DETAILS

RD611A

DESIGNED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.

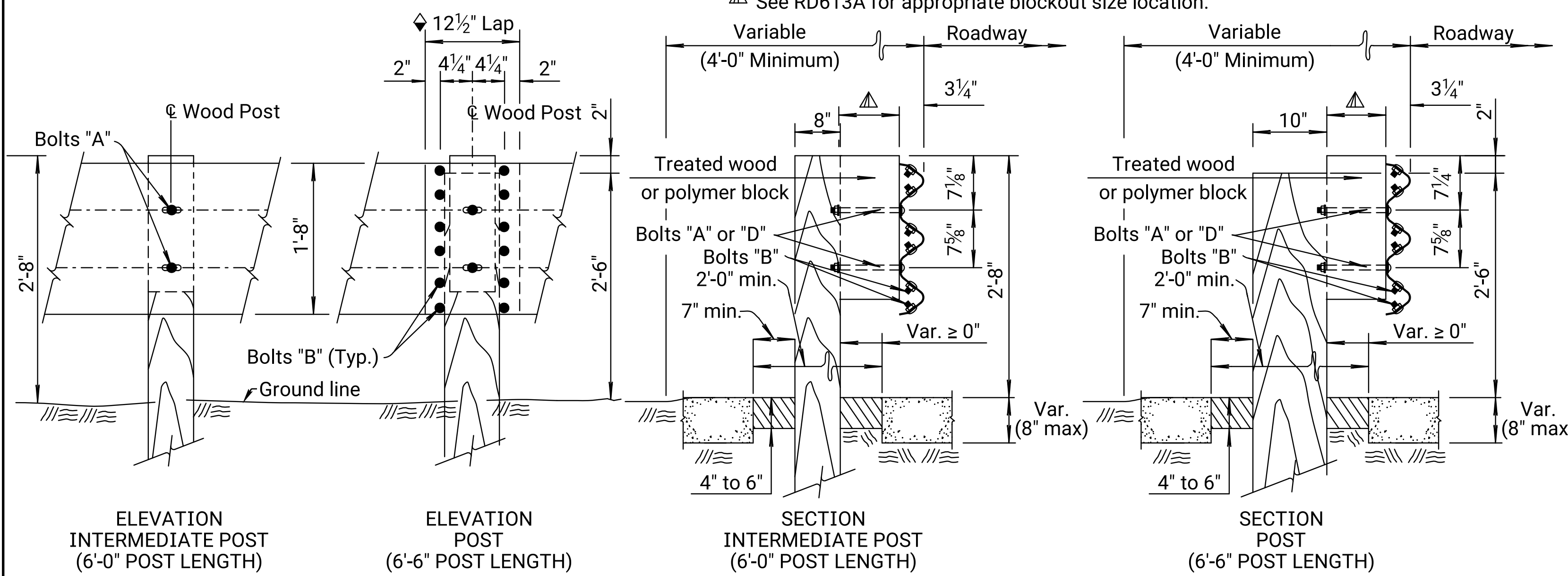
01-29-16 | APPD. Scott W. King

DOT Graphics Certified 01-05-2024 Sh. No. 18

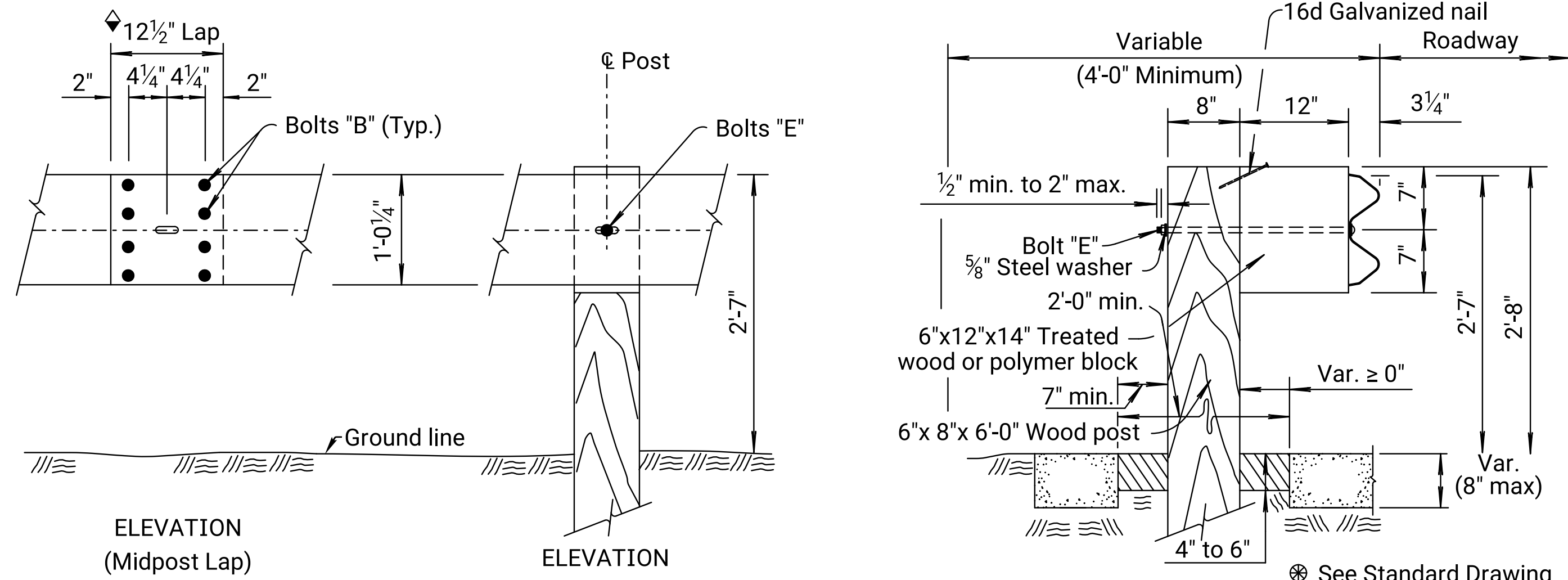
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	19	135

◆ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

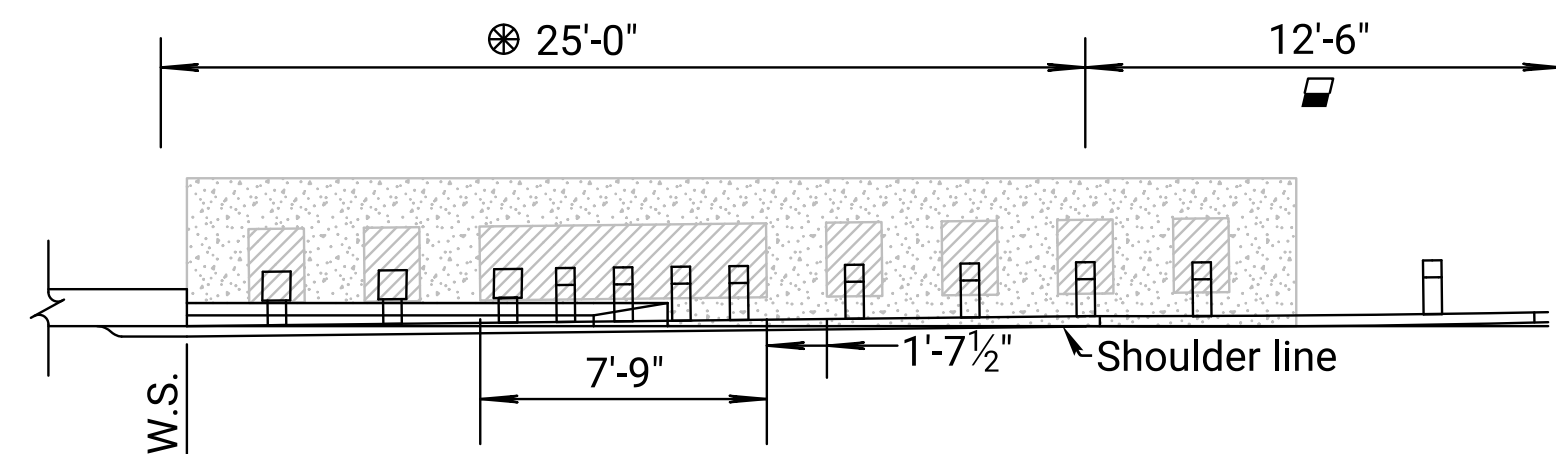
▲ See RD613A for appropriate blockout size location.



THRIE BEAM POST DETAILS/POSTS IN PAVEMENT



W-BEAM (MGS) POST DETAILS/POSTS IN PAVEMENT

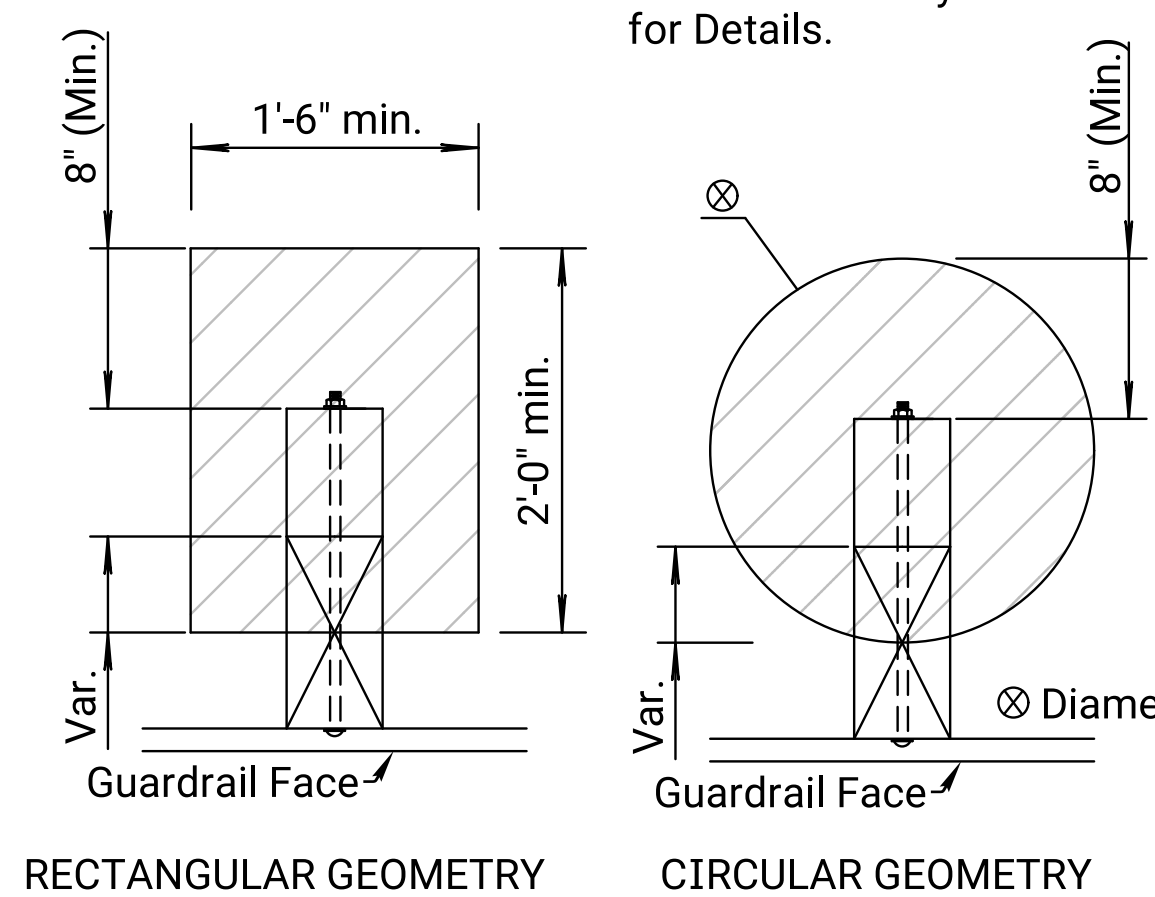


POSTS IN PAVEMENT

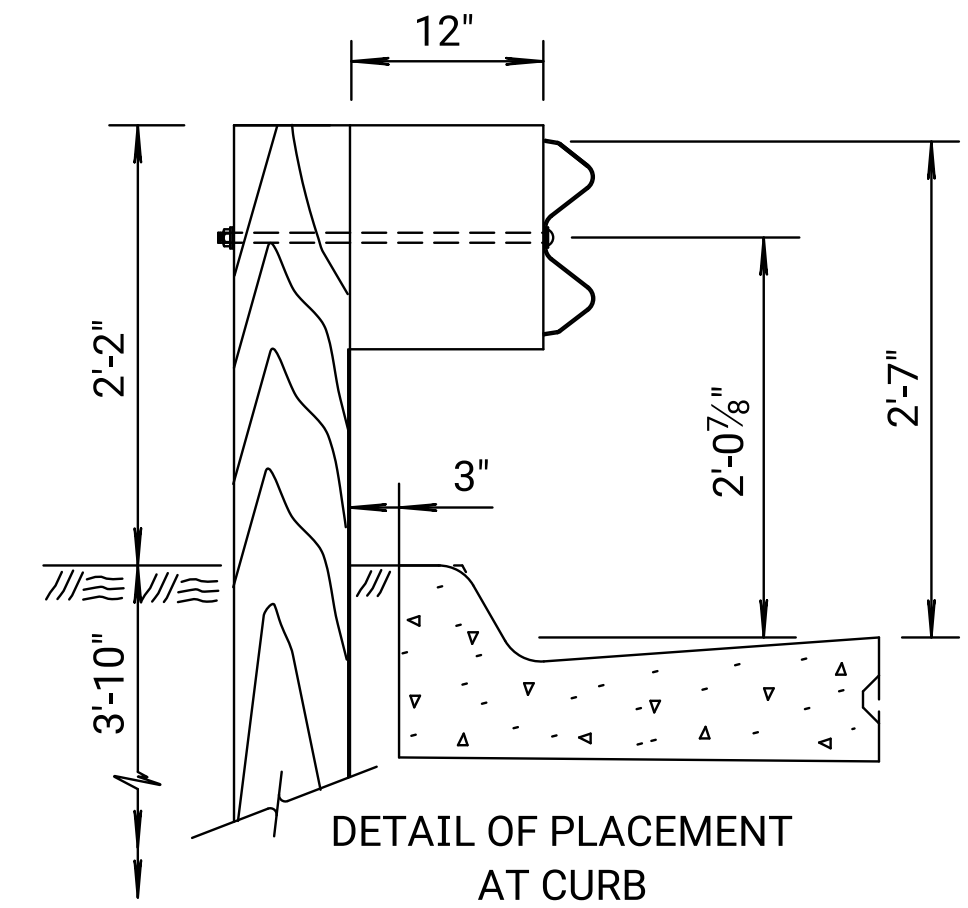
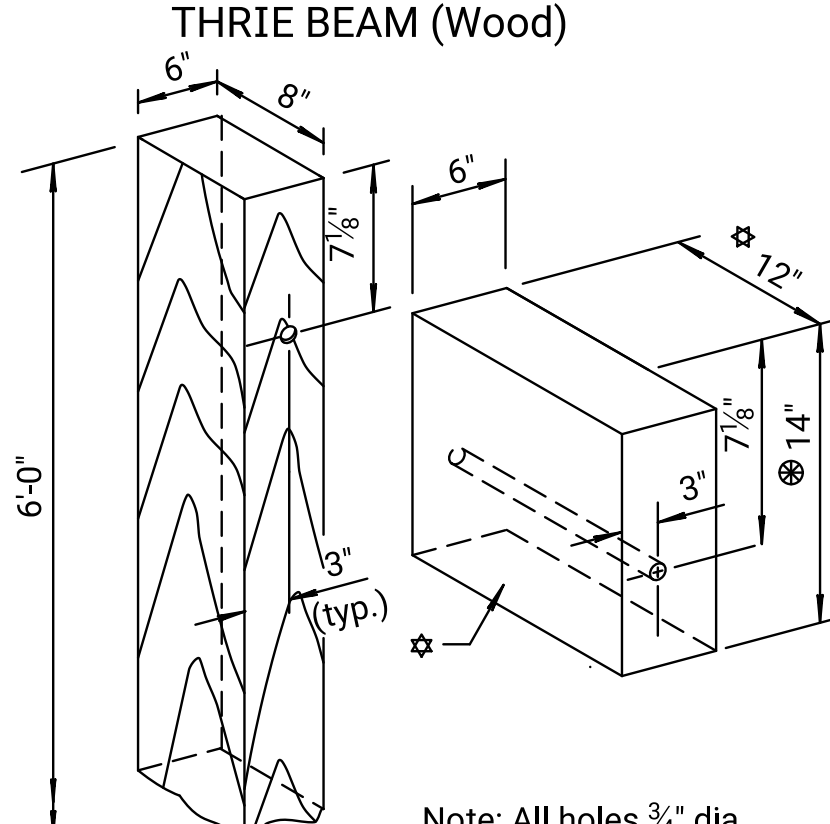
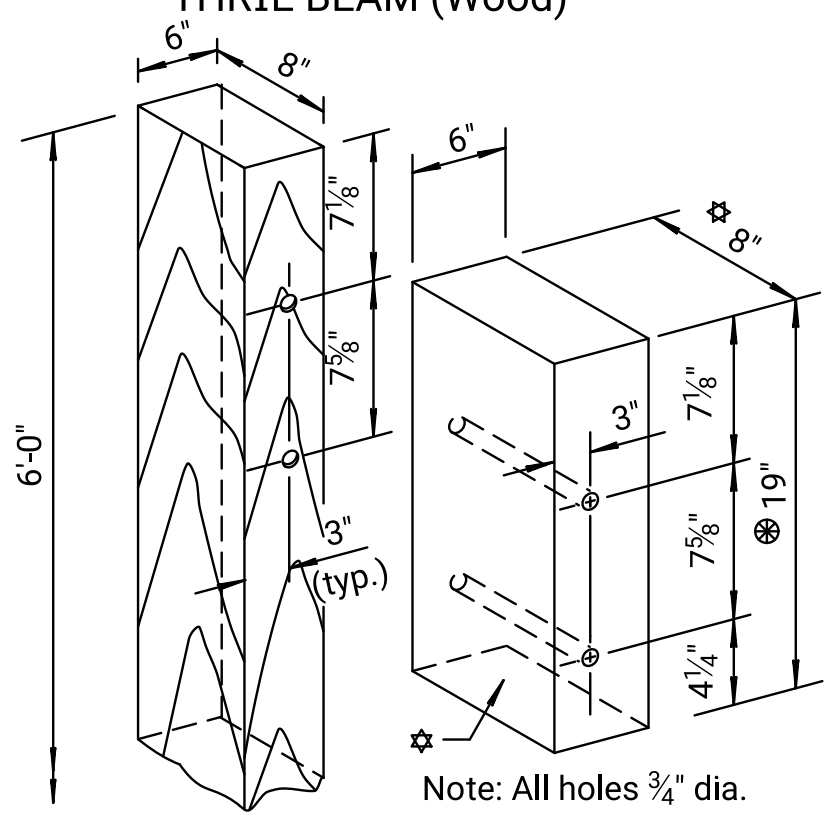
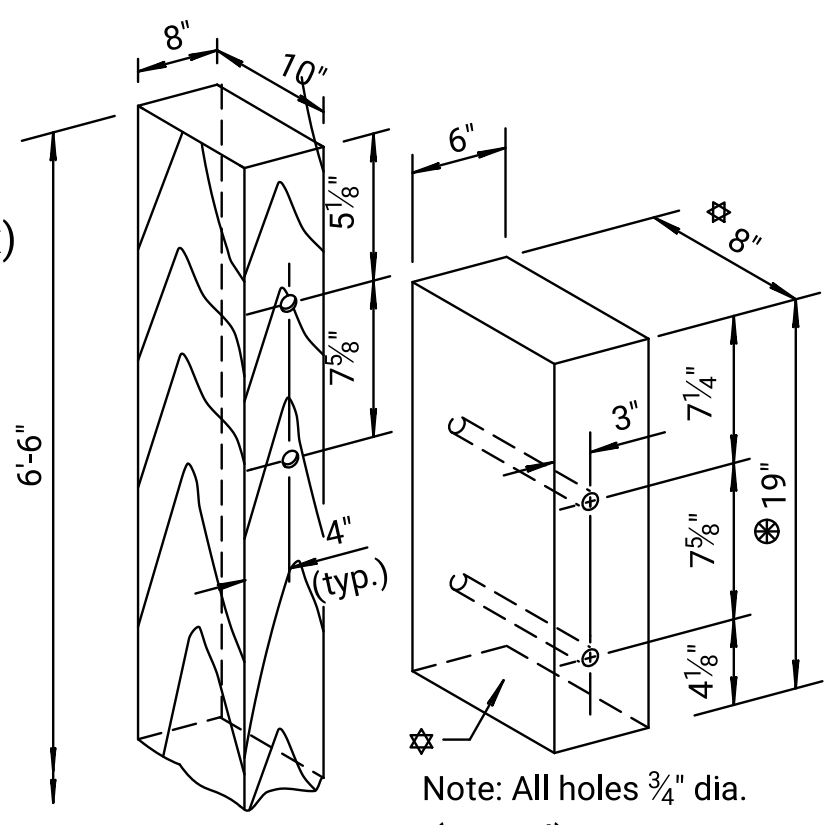
(Not to Scale)

- ▣ Slurry Grout (Low Strength)
See KDOT's Standard Specifications
- ▣ Pavement (Concrete or Asphalt)

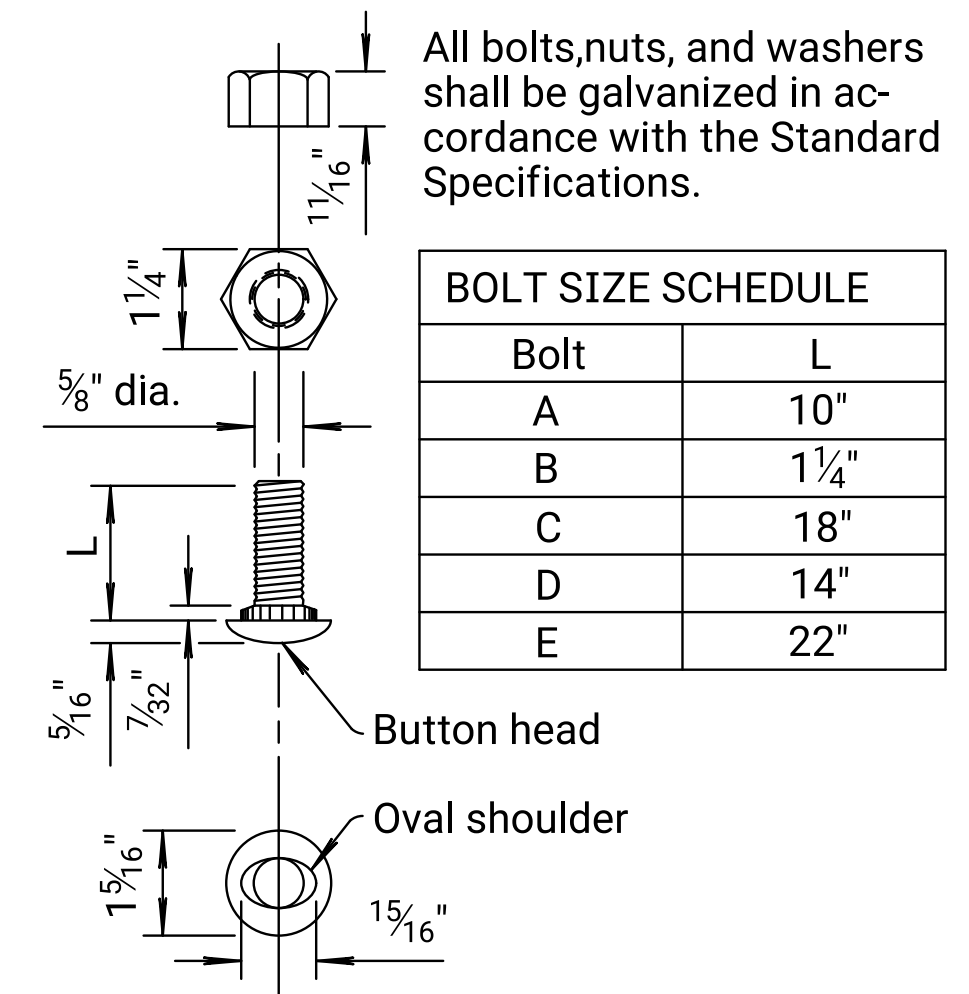
Note: Low Strength Grout must have a 28-day compressive strength of 120 psi or less. All work and materials related to posts in pavement are subsidiary to other guardrail bid items. Rectangular geometry shown in Posts in Pavement detail. Circular geometry, as shown on this sheet, may be used at the Contractor's option.



PLAN (ALTERNATE GEOMETRIES)



Note: Measure height of rail from the pavement surface at the curb/pavement joint as shown. A special design is needed when guardrail is not located as detailed. A Type II (laydown) curb & gutter is preferred when guardrail is adjacent to curb.



BOLT SIZE SCHEDULE	
Bolt	L
A	10"
B	1 1/4"
C	18"
D	14"
E	22"

GENERAL NOTES (Wood Posts)

Give all wood posts and wood blocks a preservative treatment, see standard specifications. Thoroughly saturate all cuts, injuries and bolt holes on wood posts and blocks with preservative. Use only one type of preservative treatment on a project.

Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations.

Wood blockouts may be used through the 25'-0" thrie-beam section with wood or polymer blockouts used throughout the remainder of the guardrail end terminal may be independent from the remainder of the installation. For wood/polymer blockout requirements see standard specifications.

Use S4S rectangular posts/blockouts for Thrie Beam/W-Beam installation. See standard specifications for additional information.

Contractor must notify Engineer at the earliest time when a non-removable man-made object (footing, pipe, etc.) is encountered and prevents installation of a full length post.

All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made.

- See Standard Drawing RD613A for Thrie-Beam Transition Section Blockout hole pattern.
- ★ Non-Metallic (Polymer) or Treated Wood Block
- ▣ See Guardrail Layout Sheets for Details.

Notes to Designer: For posts installed in pavement thicker than 8" or posts installed in rock formations refer to AASHTO's Roadside Design Guide for details then revise this drawing and all supporting drawings appropriately.

Plotted by: August.Zuno@ks.gov
4-JAN-2024 20:13
File: rrs611b.dgn

04		09-24-15	Initial Release	T.T.R.	S.W.K.
NO.	DATE	REVISIONS	BY	APPD	
KANSAS DEPARTMENT OF TRANSPORTATION					
GUARDRAIL POST (WOOD) (MGS) DETAILS					
RD611B					
FHWA APPROVAL		01-29-16	APPD.	Scott W. King	
DESIGNED	DETAILED	QUANTITIES	TRACED		
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		
KDOT Graphics Certified			05-11-2022	Sh. No. 19	

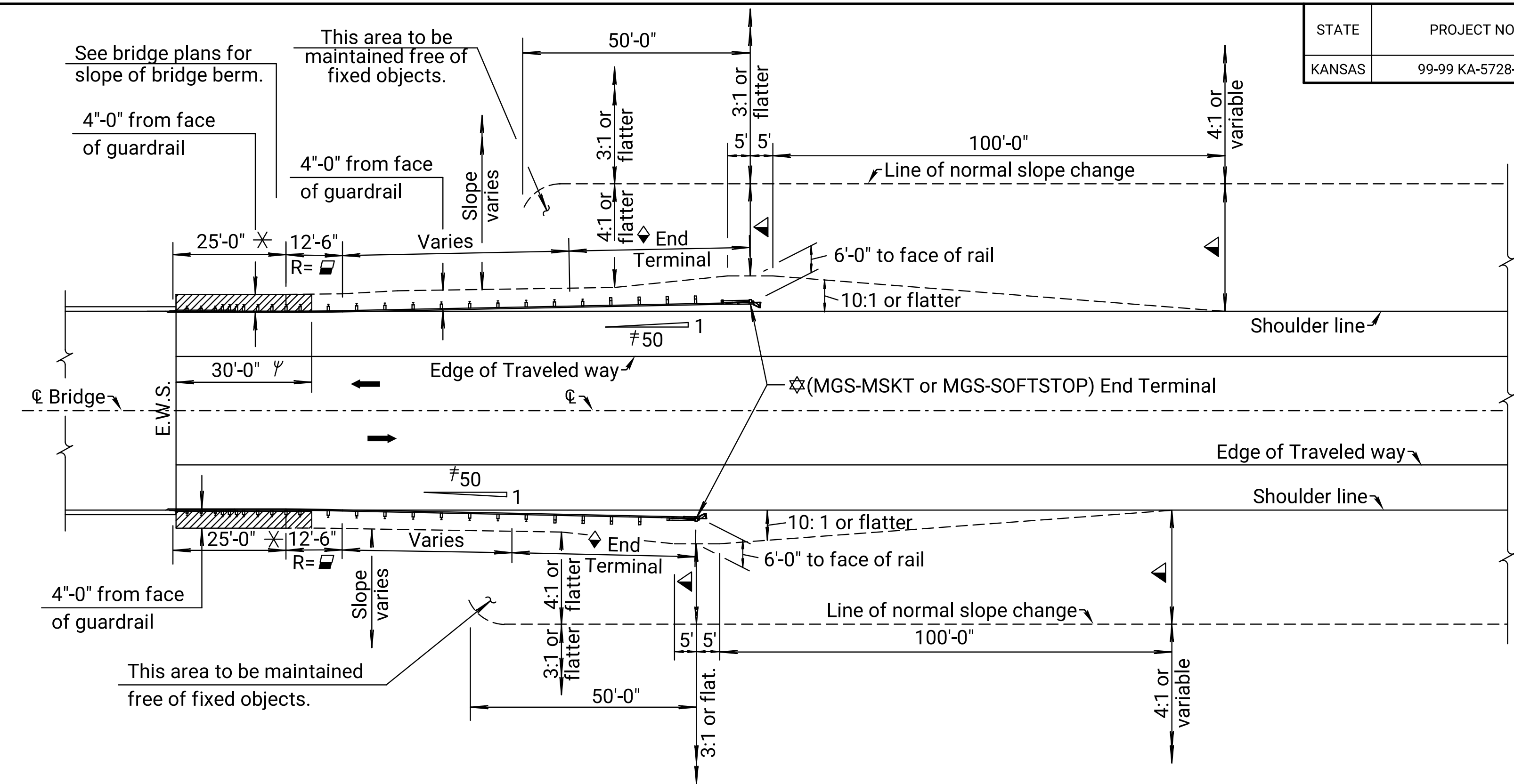
KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	20	135

Notes to Designer: Determine the guardrail length of need using either KDOT's Length of Need Equation or a graphic design approach with an L₁ distance measured from the edge of the area of concern to the P.I. of the curved guardrail section. Combine material for asphalt widening in the plan quantities.

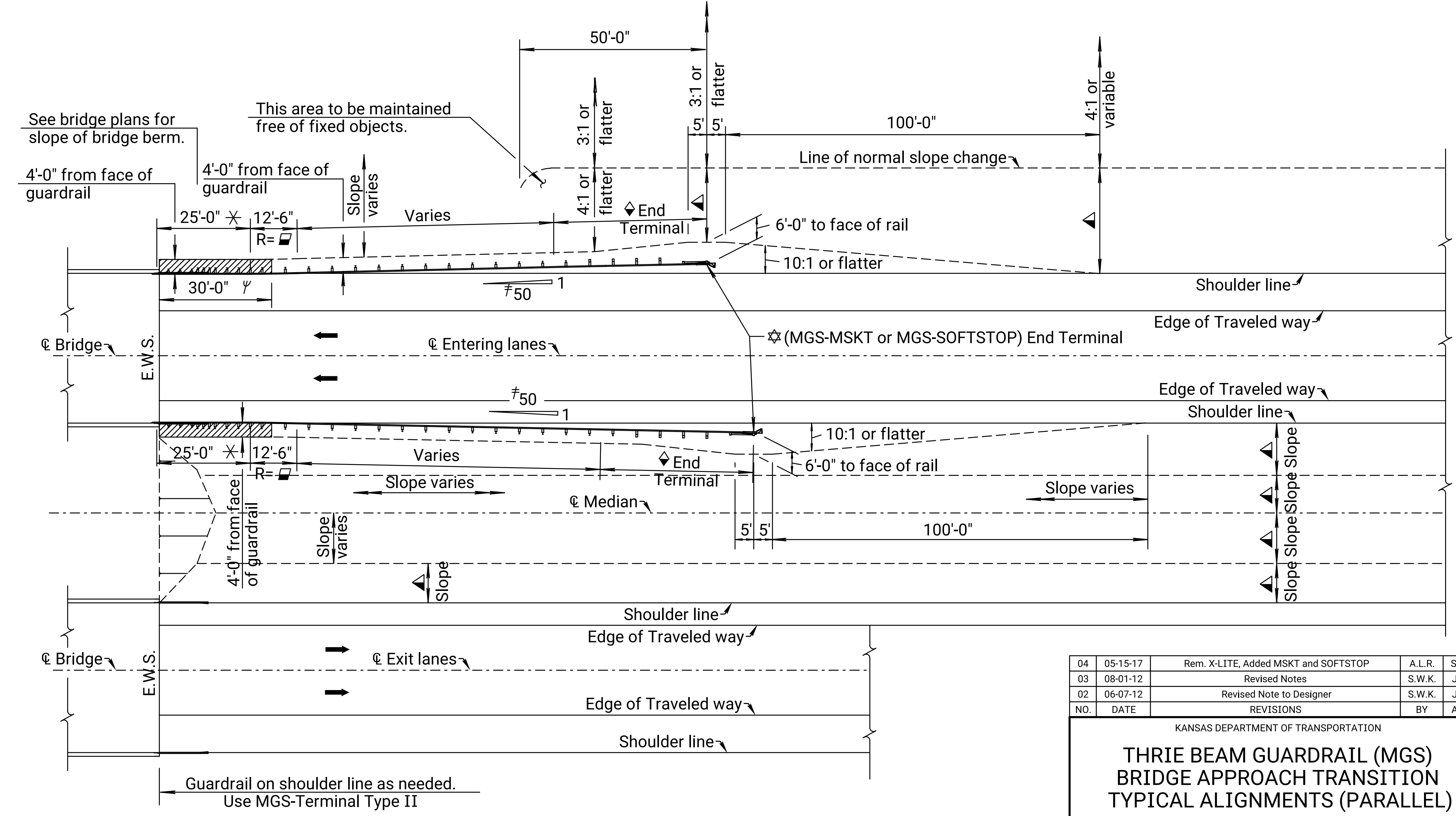
"Parallel" installations are flared at a rate of 50:1. "Zero Flare" installations follow the edge of shoulder.

Plotted by : August.Zuno@ks.gov
File : rss612b.dgn



THRIE BEAM TRANSITION - TWO LANES

- ✱ Thrie Beam Transition. See Std. Drawing RD613A for details and general note.
- ▣ Radius= 625.08'
- ▲ Normal project side slope. See typical sections.
- ◆ See KDOT's 'Guardrail Auxiliary Details' Standard Drawing.
- ∇ 4" Asphalt material placed on 4'-0" embankment widening unless flume inlet and slope drain is constructed. See RD611A for "Post in Pavement" details.
- ≠ Terminate zero flare rate installations with a parallel guardrail end terminal. Typically parallel end terminals are flared at 50:1 over the length of the end terminal, but may be flared up to 26:1 or flatter.
- ✱ The minimum length of w-beam guardrail required between the thrie-beam transition and the guardrail end terminal is 12'-6" for all installations.



THRIE BEAM TRANSITION - FOUR LANES (DIVIDED)

04	05-15-17	Rem. X-LITE, Added MSKT and SOFTSTOP	A.L.R.	S.W.K.
03	08-01-12	Revised Notes	S.W.K.	J.O.B.
02	06-07-12	Revised Note to Designer	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

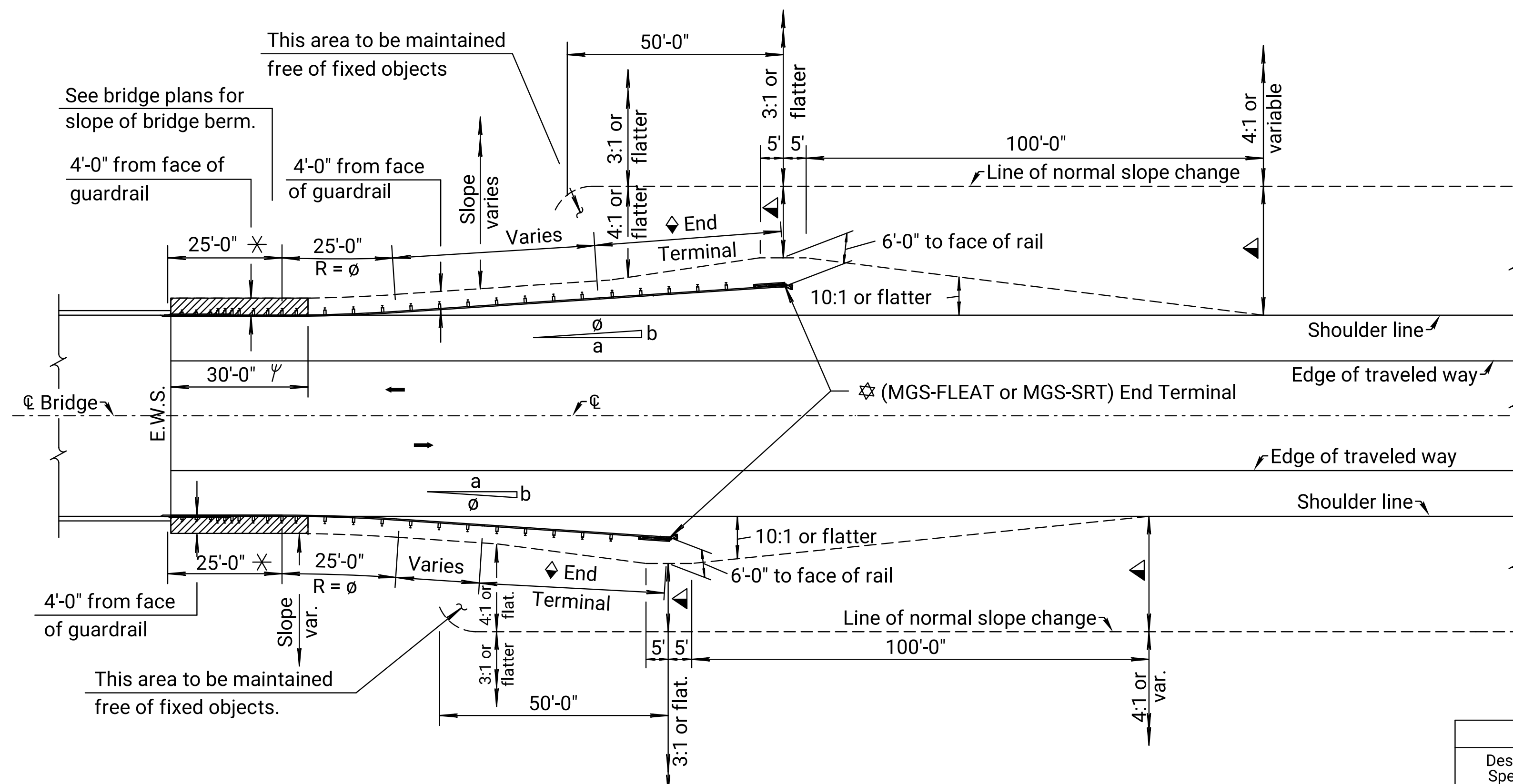
KANSAS DEPARTMENT OF TRANSPORTATION				
THRIE BEAM GUARDRAIL (MGS) BRIDGE APPROACH TRANSITION TYPICAL ALIGNMENTS (PARALLEL)				
RD612B				
FHWA APPROVAL	10-12-17	APPD.	Scott, W. King	
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	21	135

Notes to Designer: Determine guardrail length of need using either KDOT's Length of Need Equation or a graphic design approach with an L₁ distance measured from the edge of the area of concern to the P.I. of the curved guardrail section. Combine materials for asphalt widening in the plan quantities.

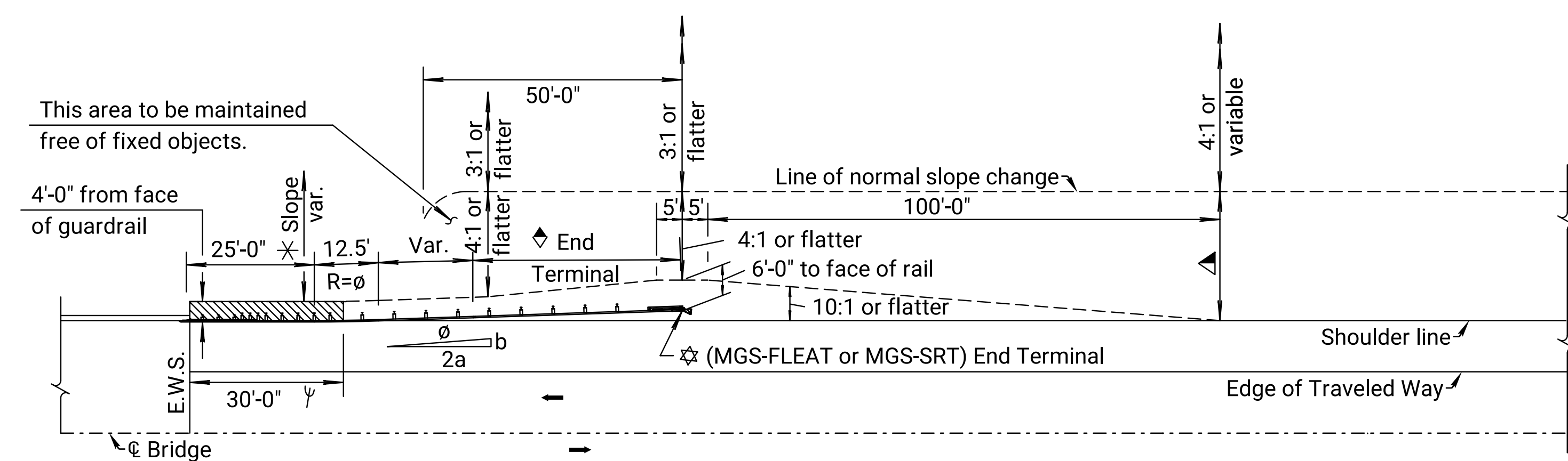
Optional: If approach side is within the shyline, use a flare rate of 2a:b for all quadrants.

Plotted by: August.Zuno@ks.gov 04-JAN-2024 20:14
File: rd612c.dgn



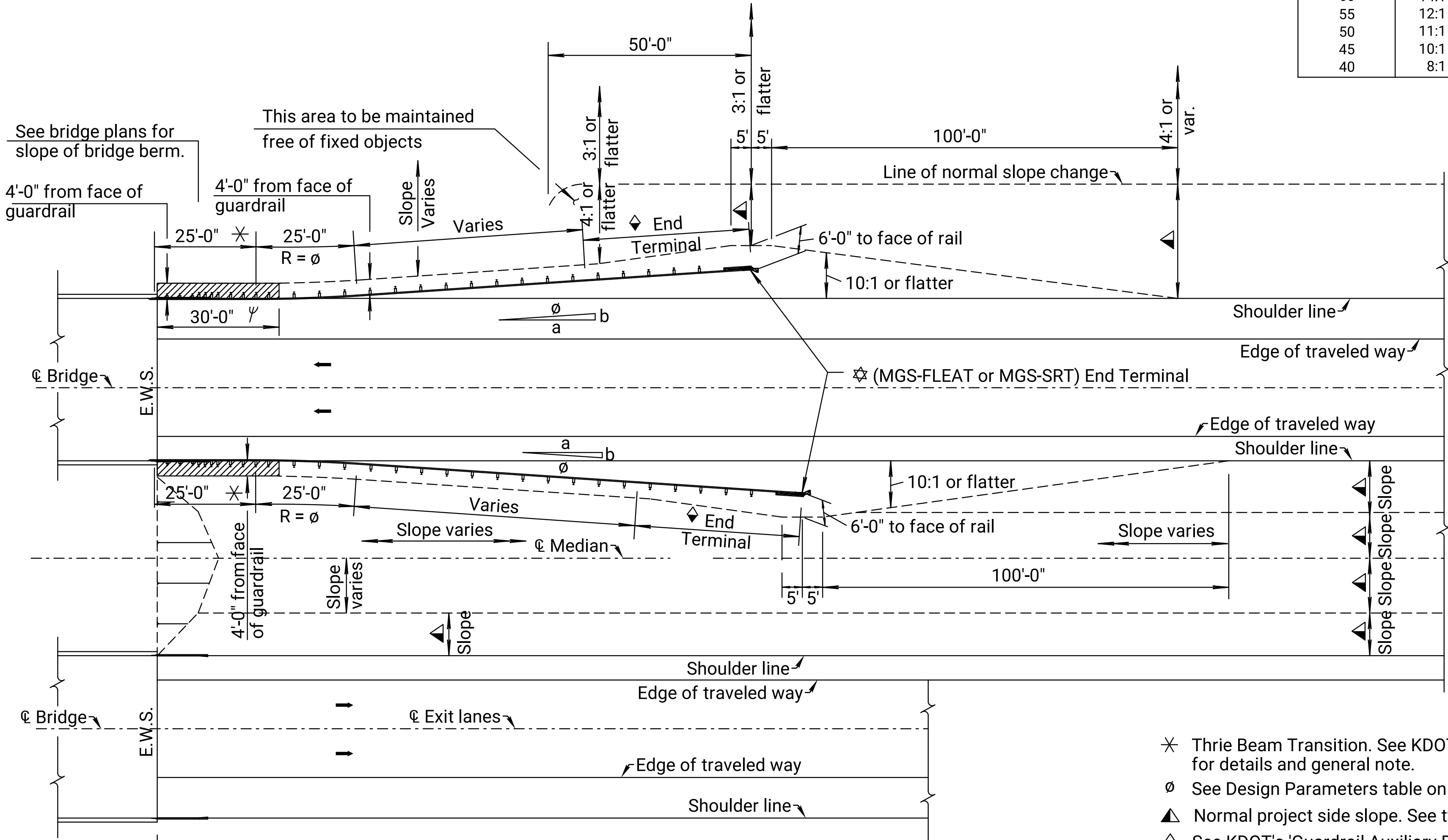
THRIE BEAM TRANSITION - TWO LANES

Note: Use flare rate of a:b and curve length of 25'-0" when guardrail is beyond shyline. Use flare rate of 2a:b and curve length of 12'-6" when guardrail is located inside the shy line.



ALTERNATE TREATMENT - TWO LANES (Flare Rate = 2a:b)

DESIGN PARAMETERS				
Design Speed (mph)	Flare Rate (a:b)	Radius (R)	Flare Rate (2a:b)	Radius (R)
70	15:1	375.55'	30:1	375.14'
60	14:1	350.59'	26:1	325.16'
55	12:1	300.69'	24:1	300.17'
50	11:1	275.76'	21:1	262.70'
45	10:1	250.83'	18:1	225.23'
40	8:1	201.04'	16:1	200.26'



THRIE BEAM TRANSITION - FOUR LANES (DIVIDED)

- * Thrie Beam Transition. See KDOT's 'Thrie Beam Guardrail Transition Details' Standard Drawings for details and general note.
- ∅ See Design Parameters table on this sheet for radius, length of curve and flare rate information.
- ▲ Normal project side slope. See typical sections.
- ◆ See KDOT's 'Guardrail Auxiliary Details' Standard Drawing.
- ∩ 4" Asphalt material placed on 4'-0" embankment widening unless flume inlet and slope drain is constructed. See KDOT's 'Guardrail Post Details' Standard Drawings for "Post in Pavement" details.
- ☆ The minimum length of w-beam guardrail required between the guardrail end terminal and any transition section, including the thrie-beam transition, is 12'-6".

04	06-05-18	Removed Flare-beyond-the-Flare	A.L.R.	T.T.R.
03	05-15-17	Removed X-LITE	A.L.R.	S.W.K.
02	06-07-12	Revised Note to Designer	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

**THRIE BEAM GUARDRAIL (MGS)
BRIDGE APPROACH TRANSITION
TYPICAL ALIGNMENTS (FLARED)**

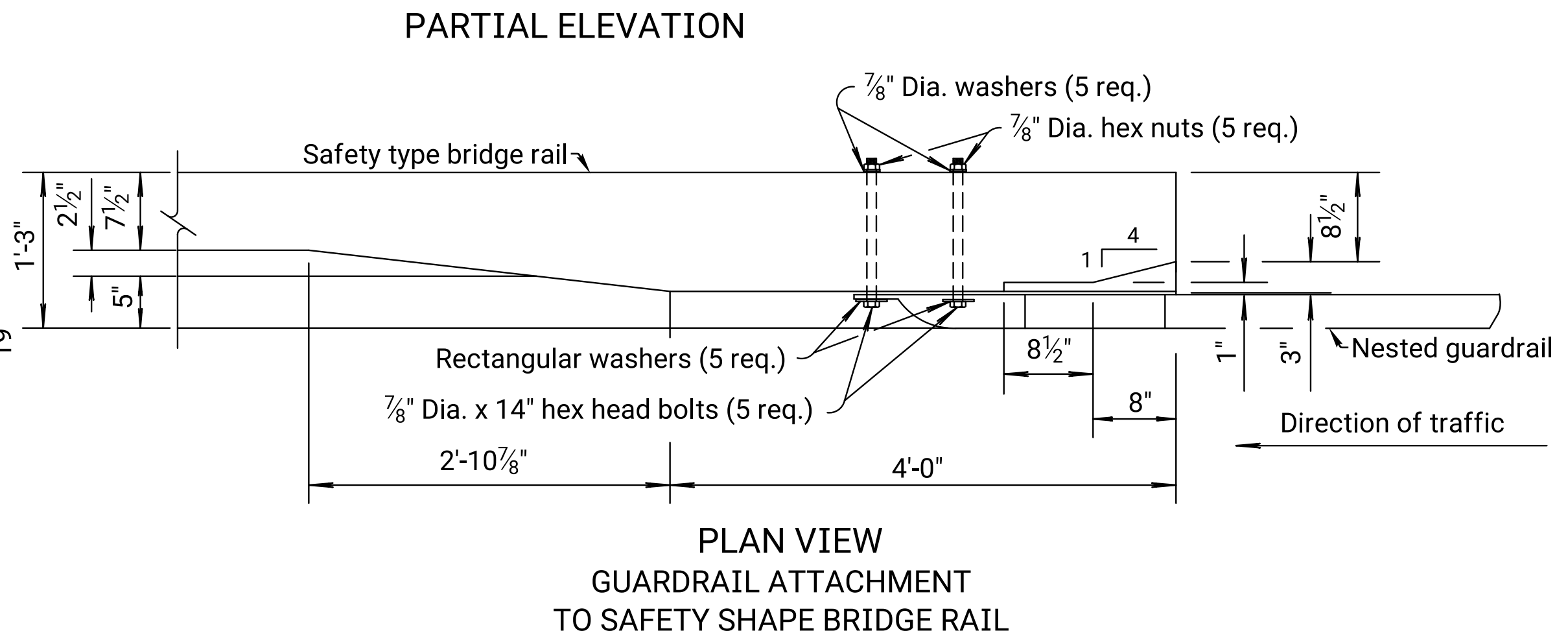
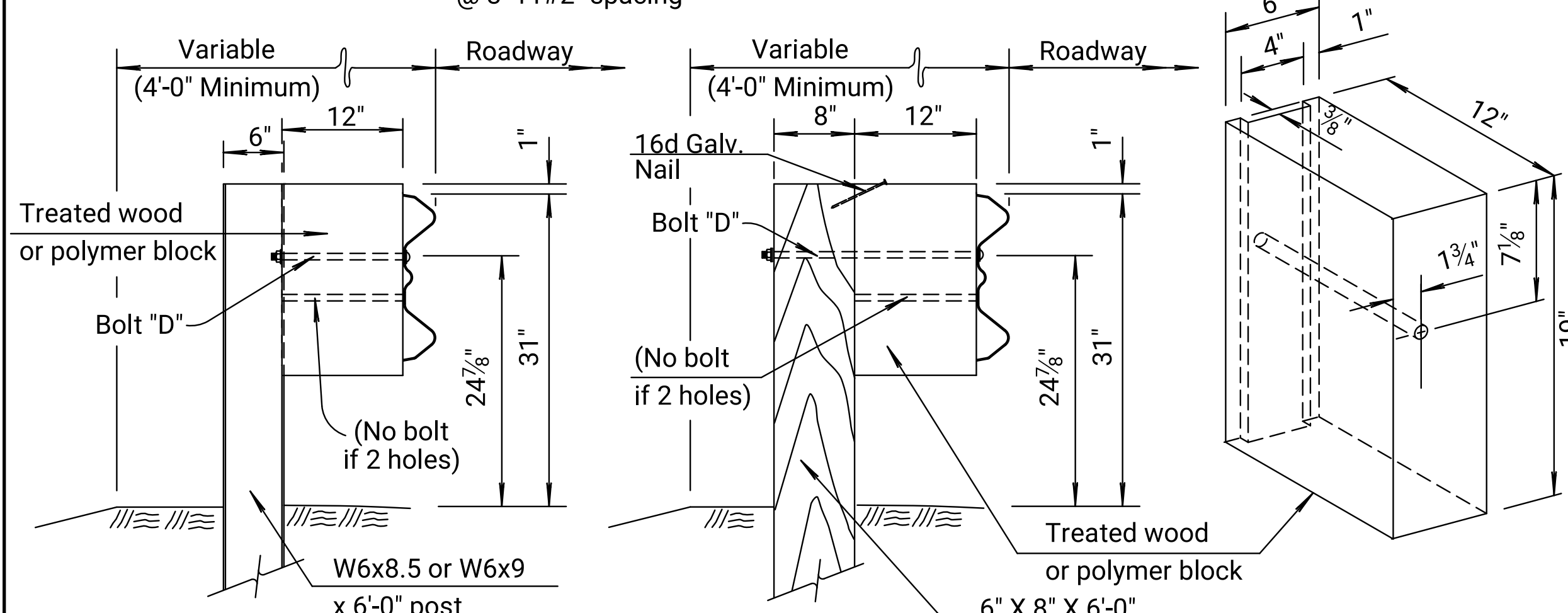
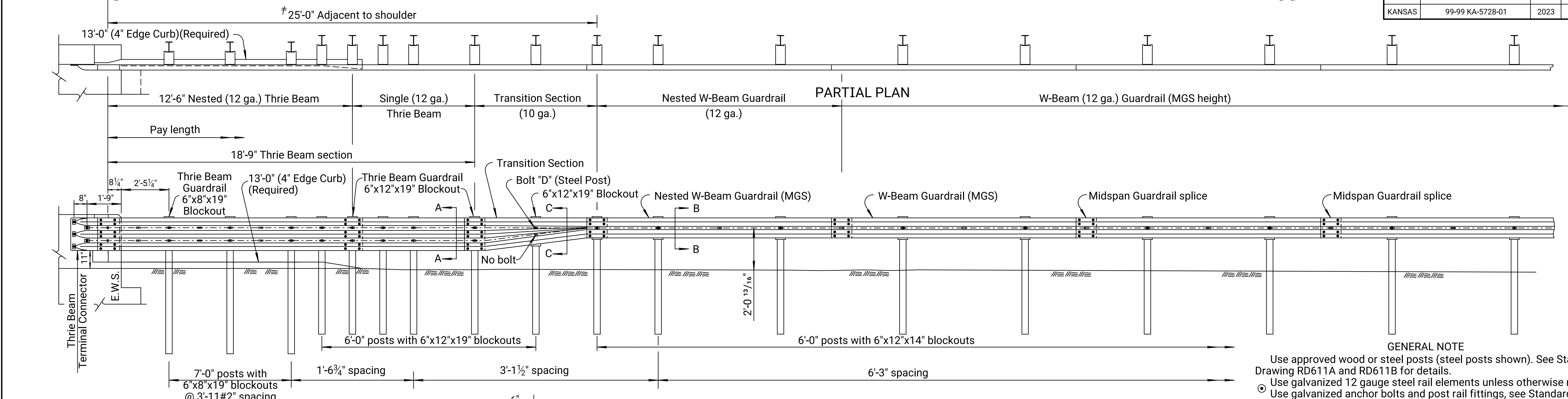
RD612C

DESIGNED	06-19-18	APPD	Scott, W. King
DESIGN CK.	DETAIL CK.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

KDOT Graphics Certified 08-21-2022 Sh. No. 21

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	22	135

Use Steel or Wood Posts (Steel Posts Shown)



GENERAL NOTE

Use approved wood or steel posts (steel posts shown). See Standard Drawing RD611A and RD611B for details.

Use galvanized 12 gauge steel rail elements unless otherwise noted.

Use galvanized anchor bolts and post rail fittings, see Standard Specifications. Supply guardrail parts that are interchangeable with similar parts regardless of source or manufacturer.

Wood blockouts may be used through the 25'-0" thrie-beam section with wood or composite blockouts used throughout the remainder of the w-beam installation. The blockout size and material used in the guardrail end terminal may be independent from the remainder of the installation.

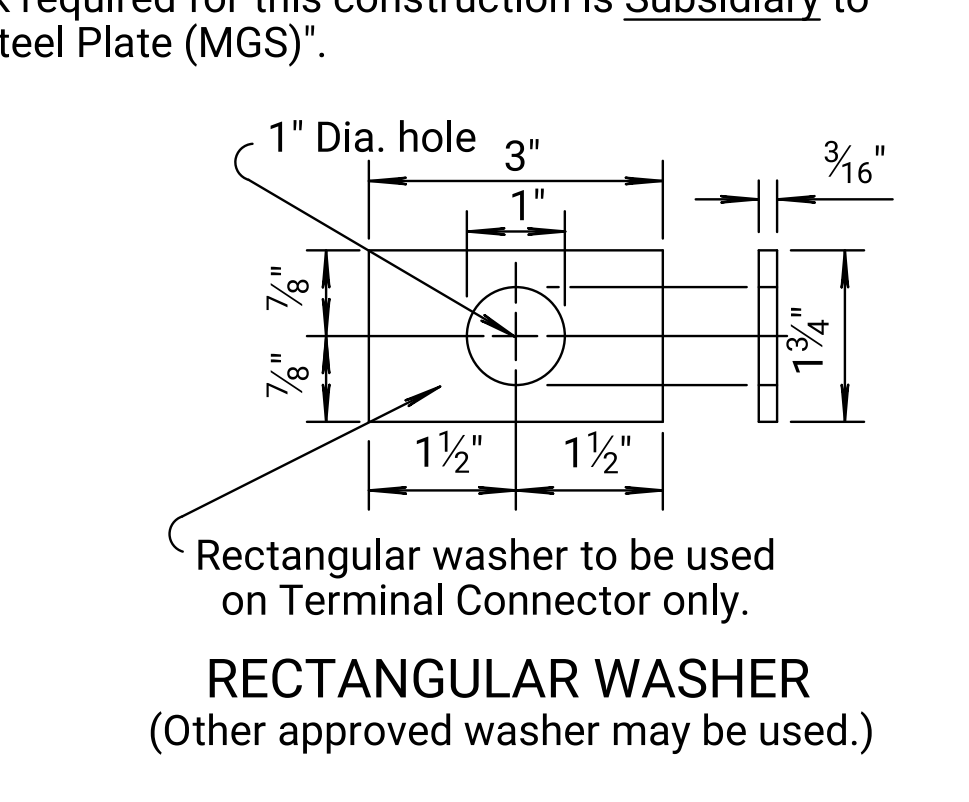
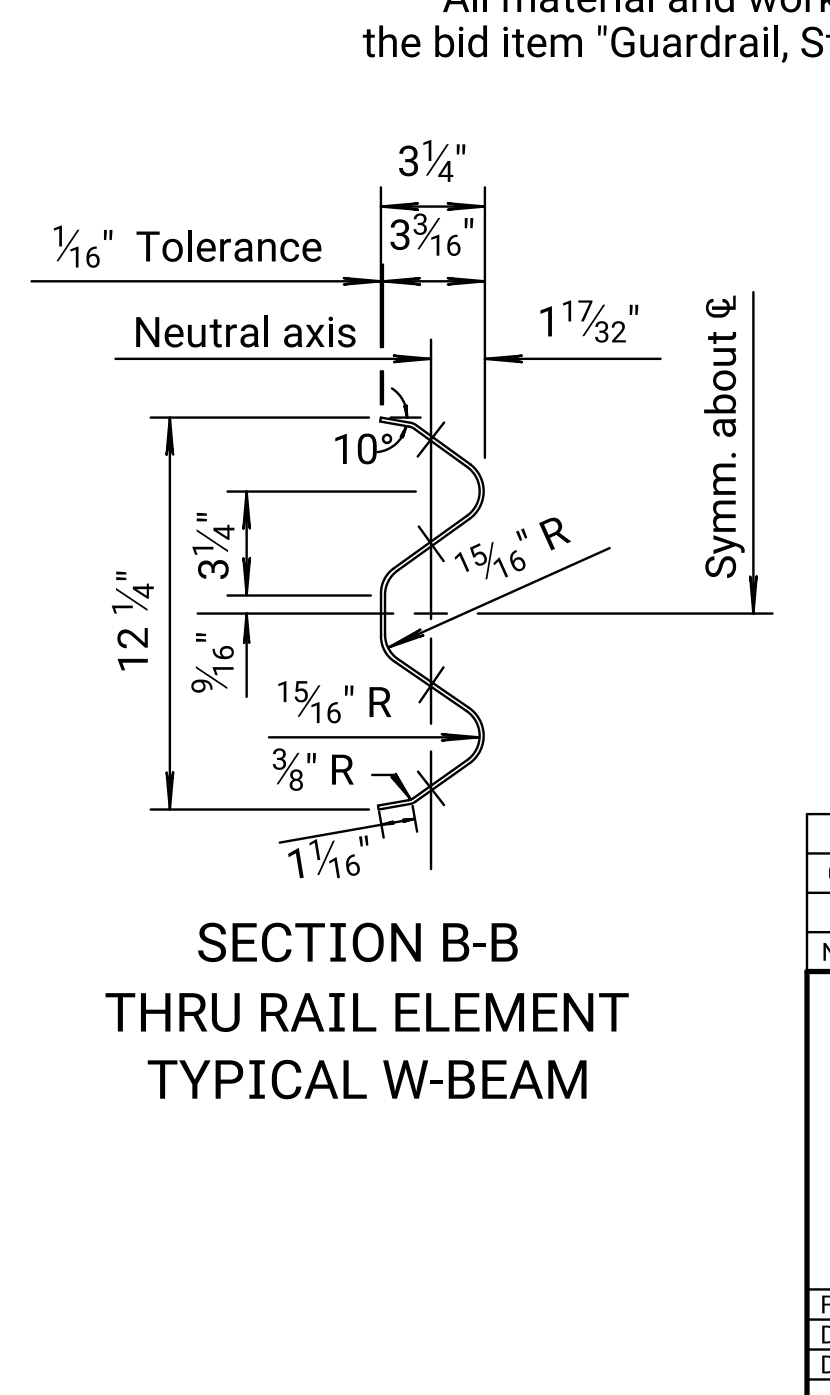
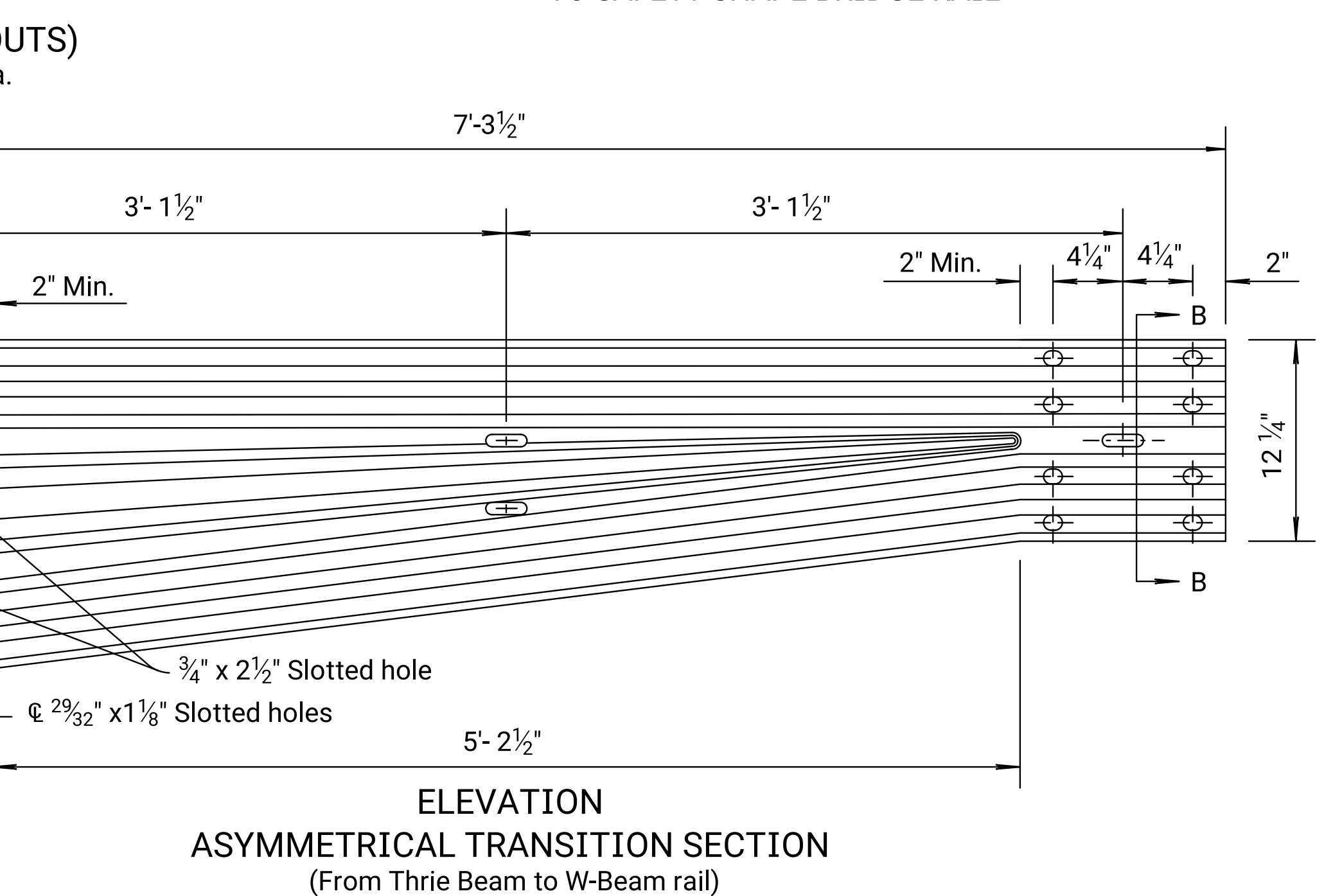
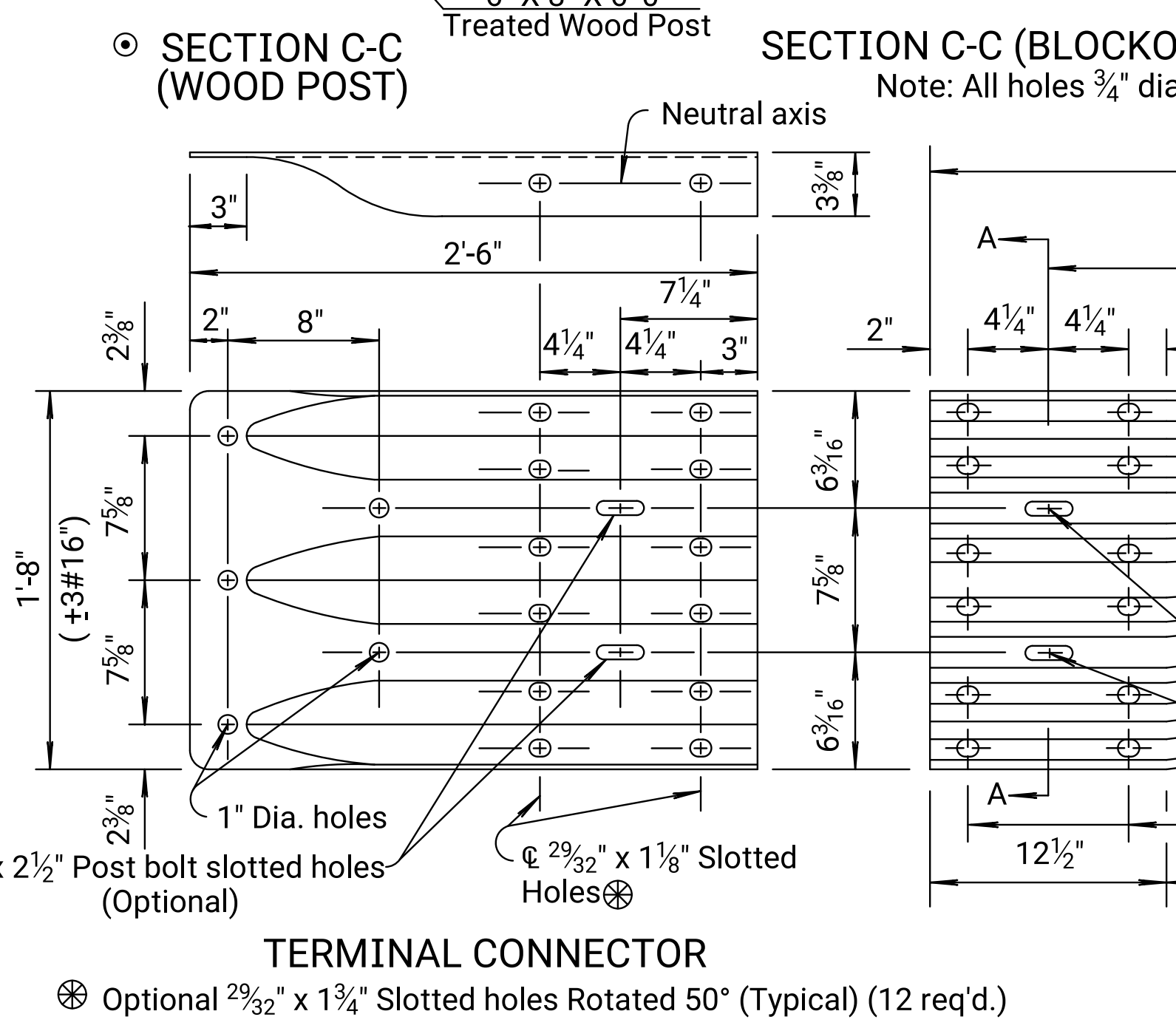
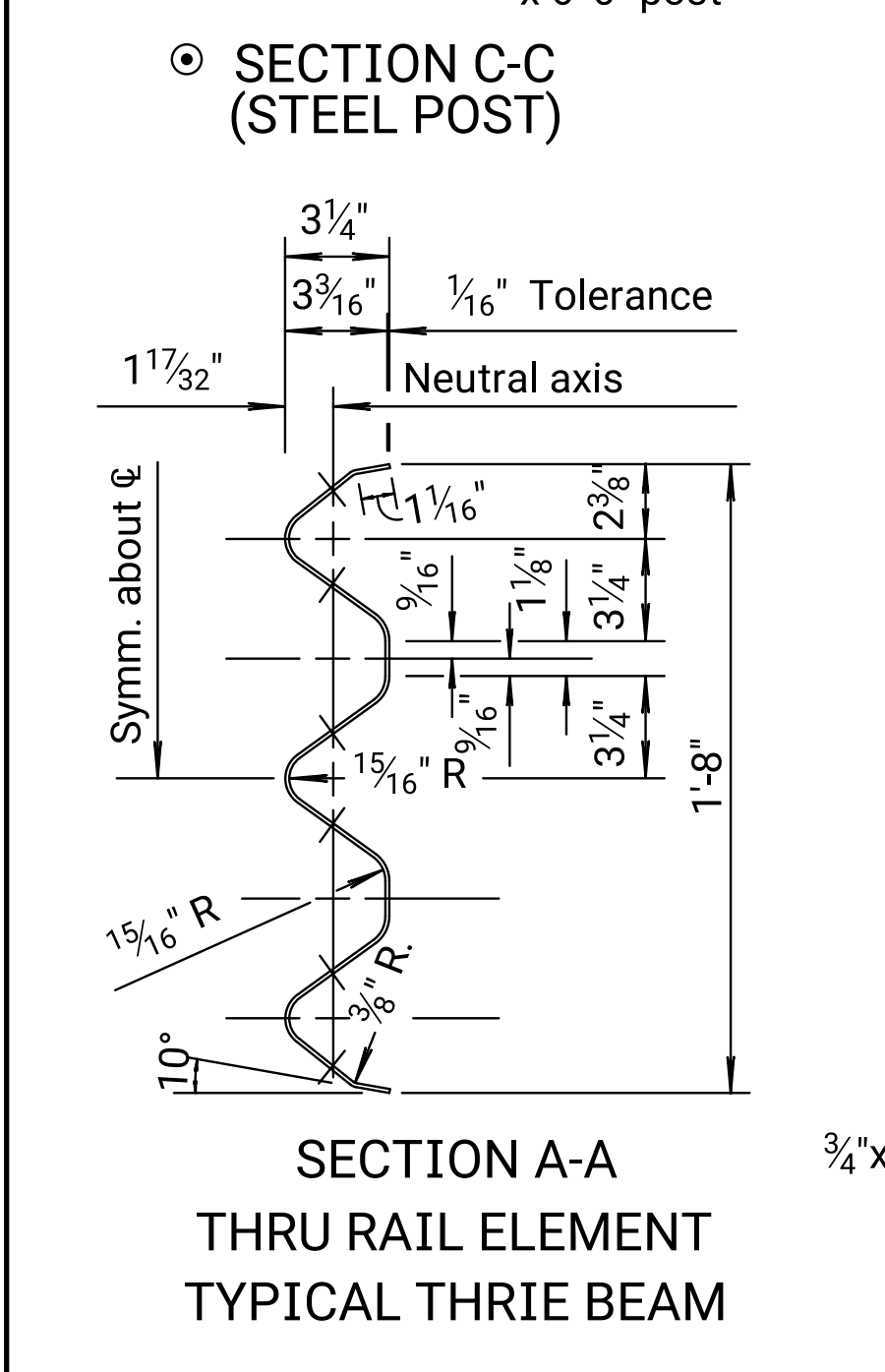
Fabricate Terminal Connector from 10 gauge steel, see Standard Specification. The connector has the same section as thrie beam guardrail. Terminal connector is Subsidiary to the bid item "Guardrail, Steel Plate (MGS)".

Shop bend curve rails when radius is less than 150'.

Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

Bridge to guardrail transition consists of 1- 18'-9" thrie-beam with 1- 12'-6" thrie-beam section nested in back of 18'-9" section (See Layout), 1- Thrie beam to W-beam Asymmetrical transition section, use associated hardware with post sizes and location shown. For the remainder of installation use (MGS) W-beam guardrail with only one post type used within (MGS) guardrail run.

All material and work required for this construction is Subsidiary to the bid item "Guardrail, Steel Plate (MGS)".



NO.	DATE	REVISIONS	BY	APPD
02	02-10-16	Added Detail, Wood Post	T.T.R.	S.W.K.
01	01-25-12	Revised Details, Thrie-Beam	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

DETAILS OF THRIE BEAM to (MGS) GUARDRAIL TRANSITION

RD613A

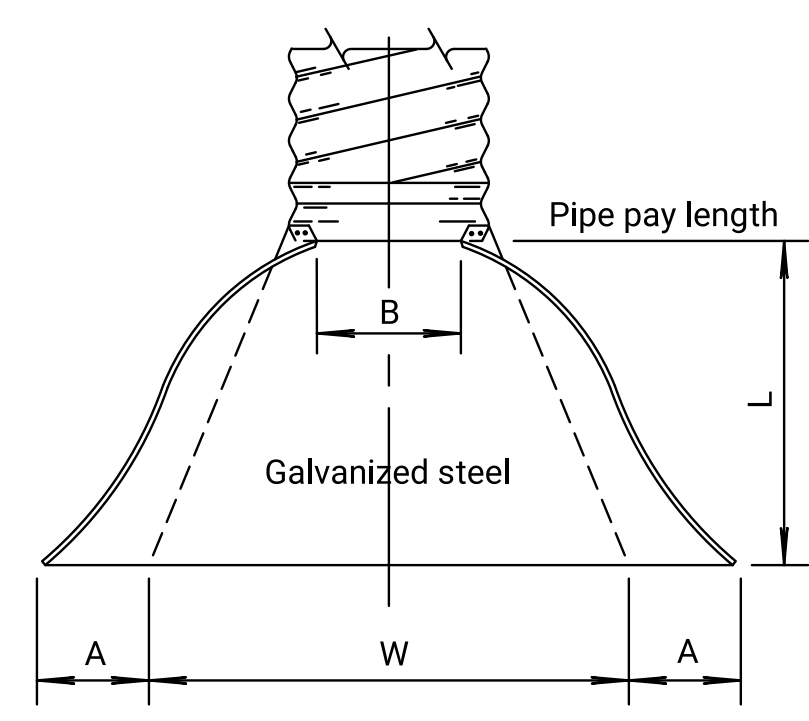
DESIGNED	DATE	APPD.	SCOTT W. KING
DESIGN CK.	DETAIL CK.	QUANTITIES	TRACED
		QUAN. CK.	TRACE CK.

DOT Graphics Certified 11-18-2022 Sh. No. 22

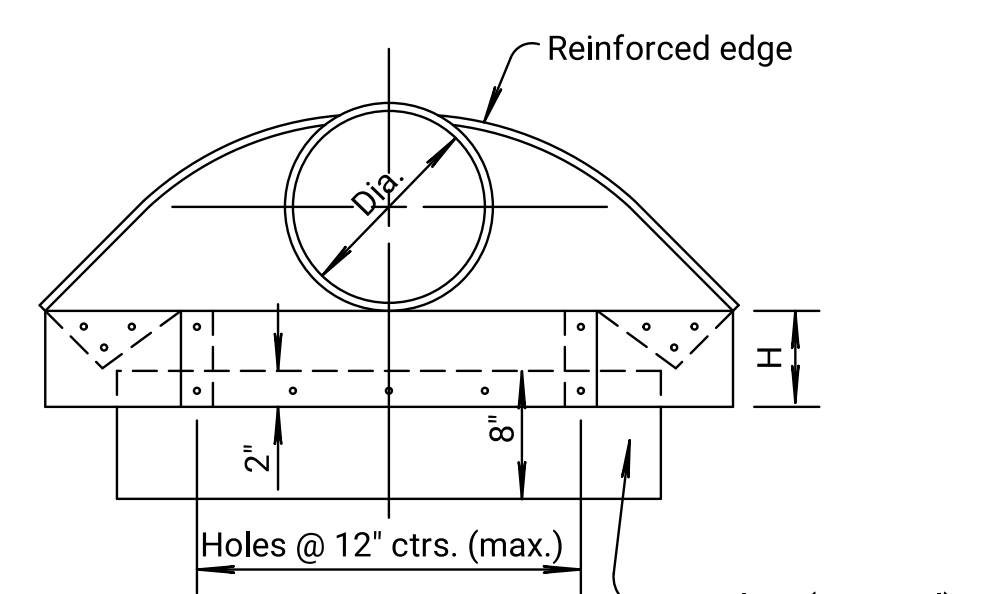
Plotted by: August.Zuno@ks.gov 4-JAN-2024 20:13
File: rss613a.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	23	135

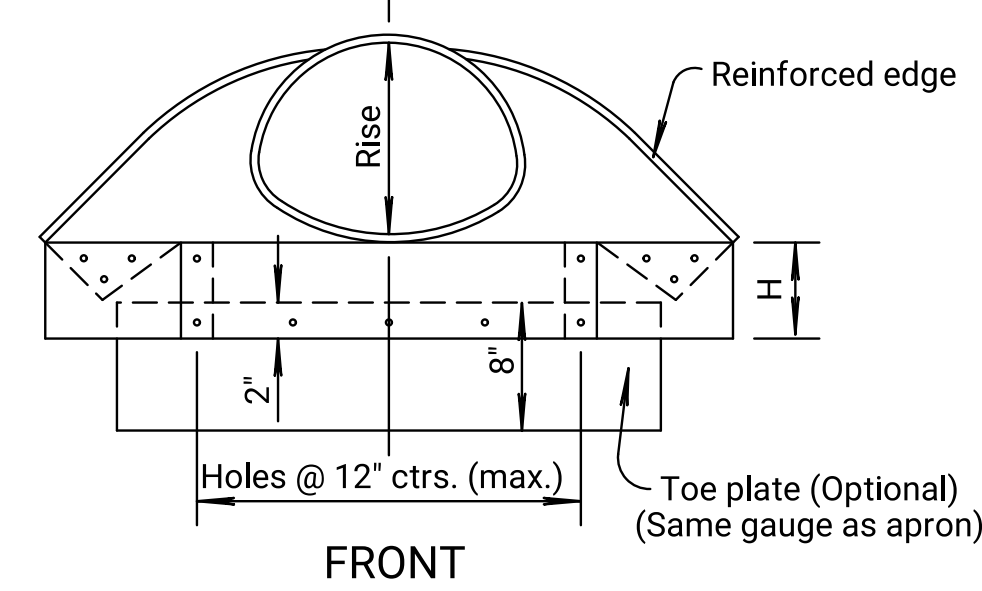
Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.



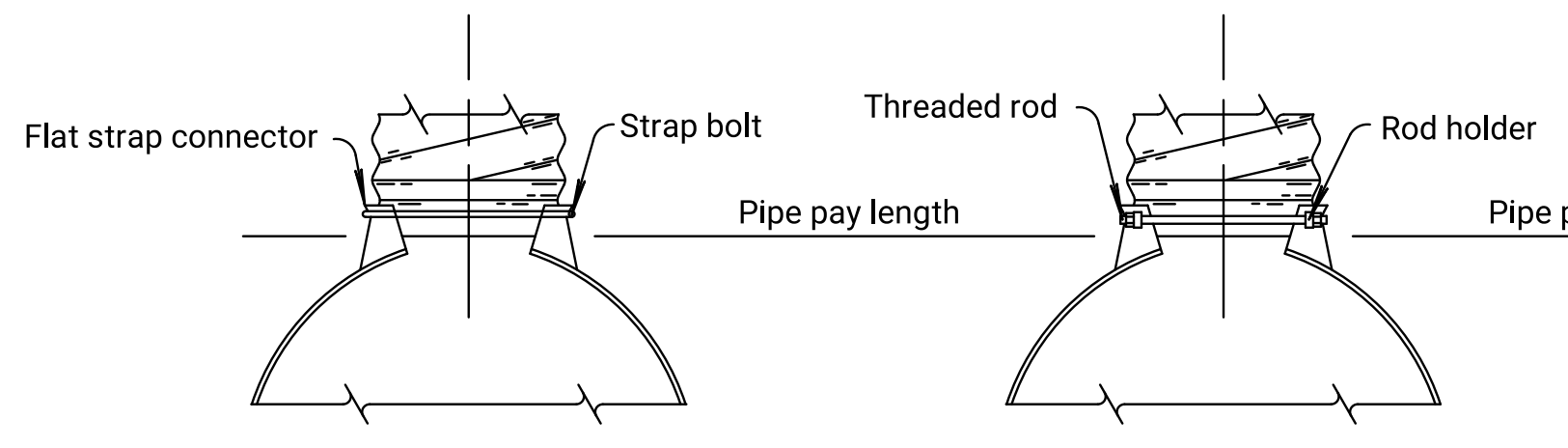
PLAN
(Illustrated with Type #3 Connection)



FRONT

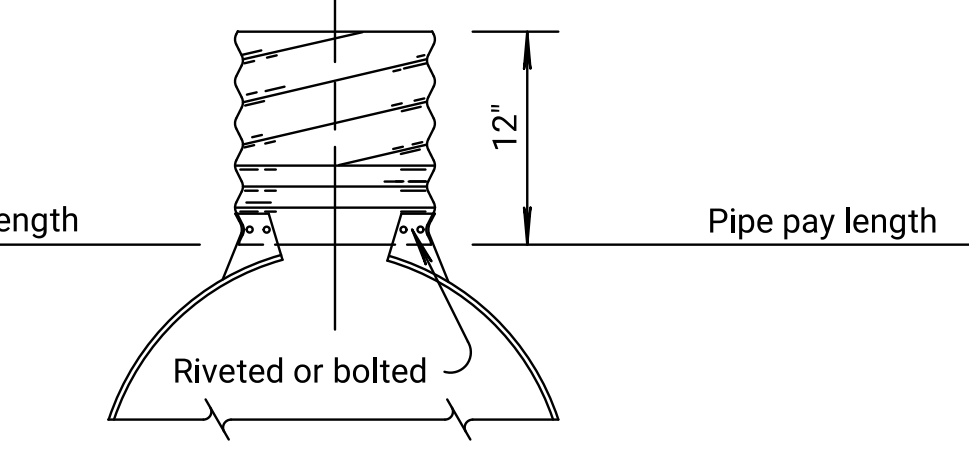


FRONT

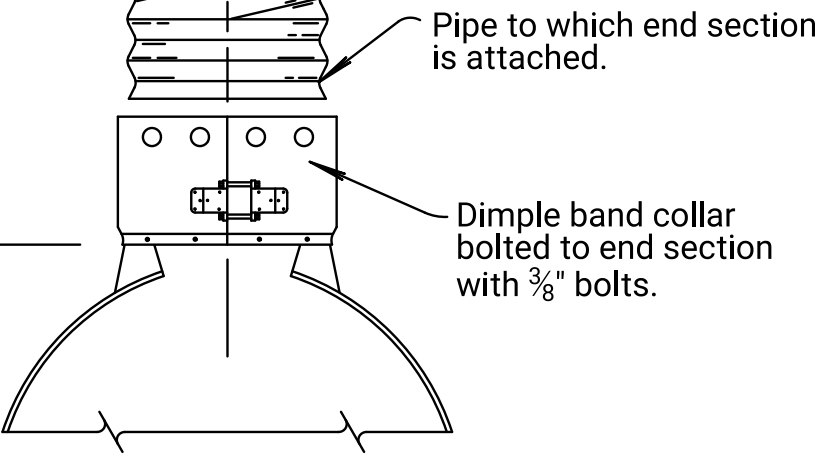


TYPE 1
Available in sizes 12" through 24" only.

TYPE 2
Available in sizes 30" and 36" Round and 17"x13" through 57"x38" Pipe-Arches.

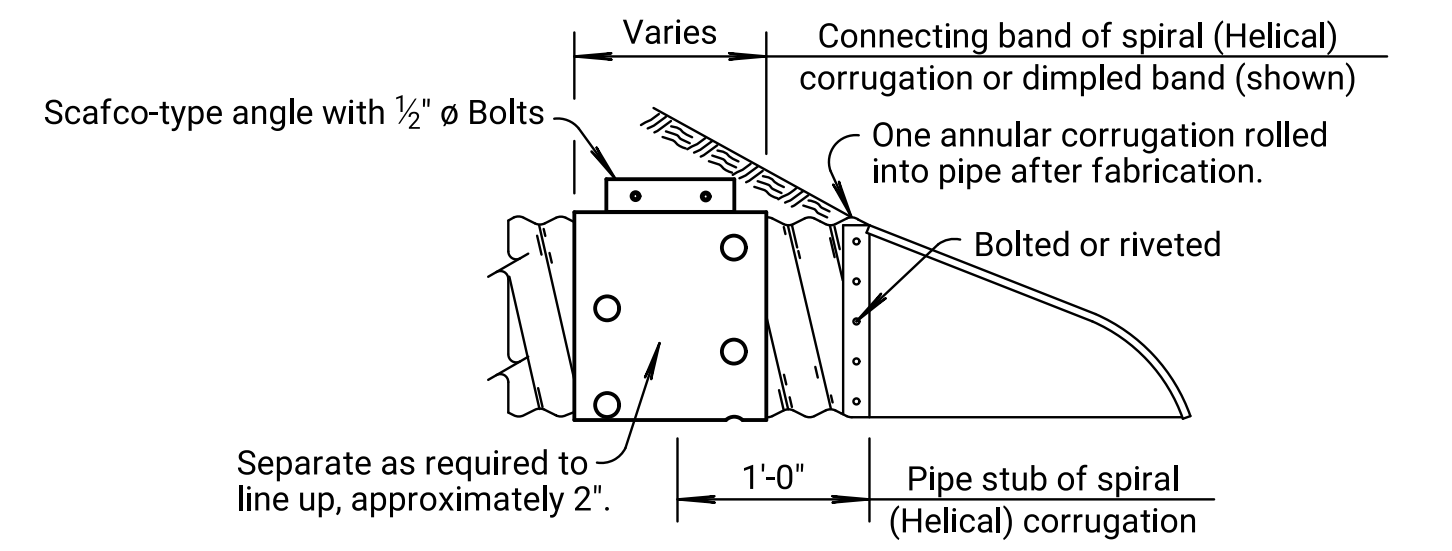


TYPE 3
Available in sizes 42" through 96" Round and 60"x46" through 81"x59" Pipe-Arches.



TYPE 5
Available for all Round and equivalent Pipe-Arch sizes, (Type 1 and Type 2 connections are recommended for the smaller sizes with annular ends).

Note: Type 3 connection may be furnished instead of Type 1 or Type 2 for smaller round or arch pipe.



SPIRAL (HELICAL) CORRUGATION

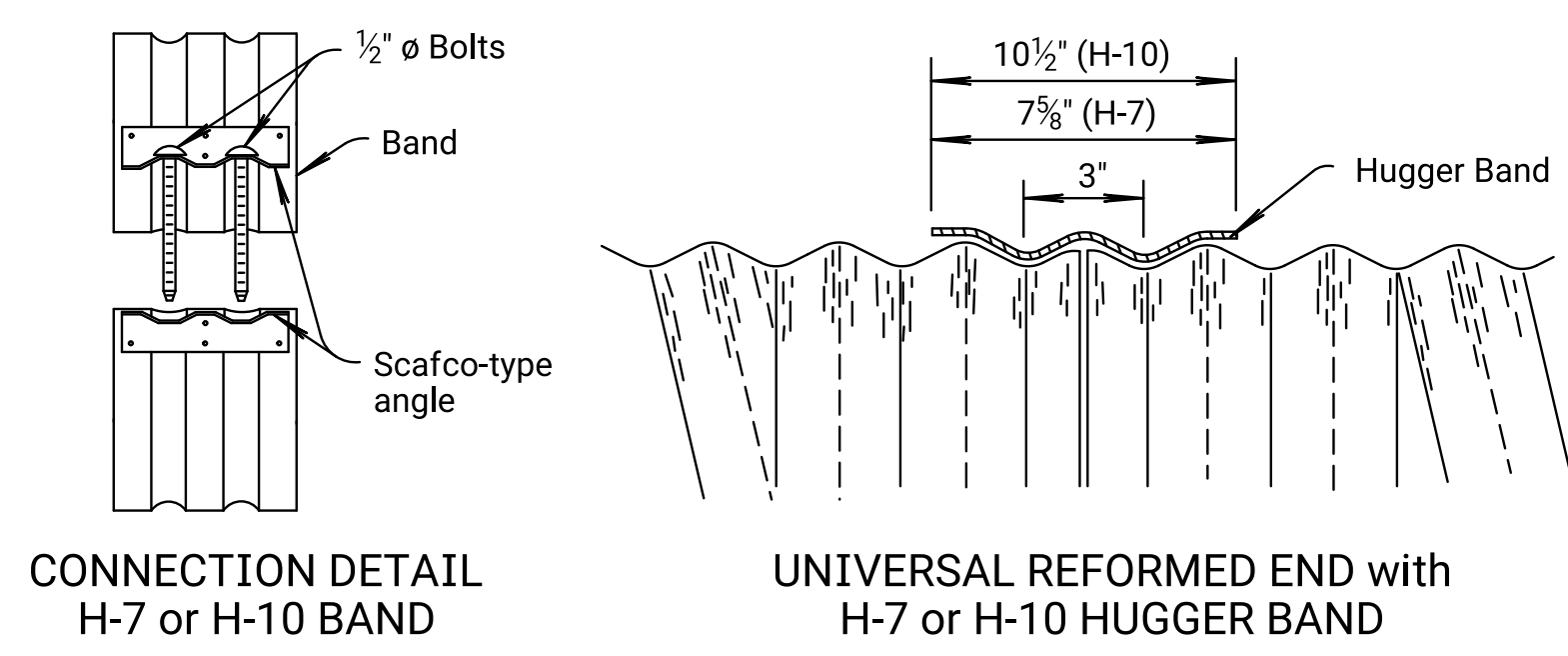
For all sizes of round and arch culvert pipes having Spiral (Helical) corrugations, the end sections and connecting bands shall be as shown above.

Thickness CSP/ACSP	Thickness CAP	Gauge
0.064"	0.060"	16 ga.
0.079"	0.075"	14 ga.
0.109"	0.105"	12 ga.
0.138"	0.135"	10 ga.
0.168"	0.164"	8 ga.

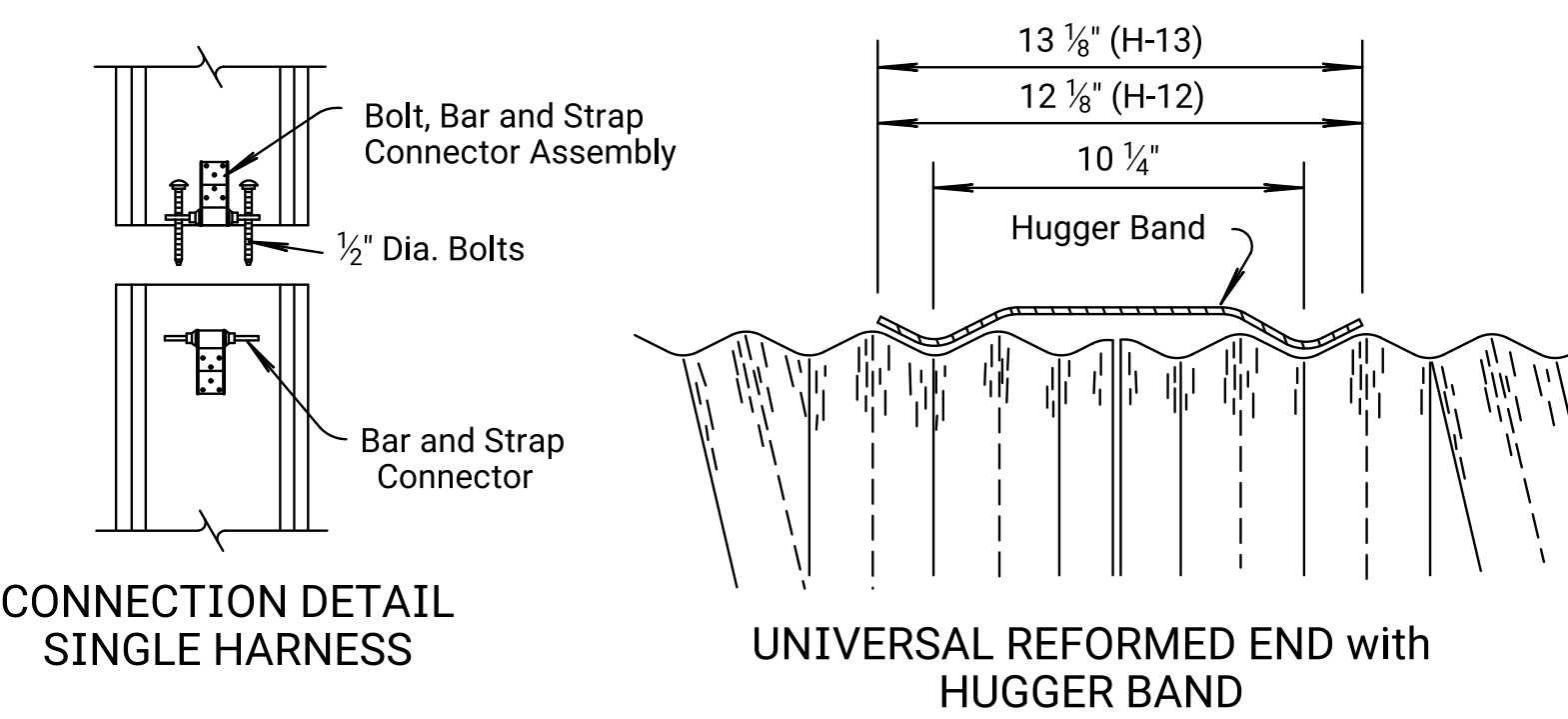
Pipe Dia. (In.)	CS, ACS or CA Gauge	Dimensions in Inches					Approx. Slope
		A (min.)	B (max.)	H (min.)	L (±2")	W (min.)	
12"	16	5	7	6	21	22	2 1/2: 1
15"	16	6	8	6	26	28	2 1/2: 1
18"	16	7	10	6	31	34	2 1/2: 1
21"	16	8	12	6	36	40	2 1/2: 1
24"	16	9	13	6	41	46	2 1/2: 1
30"	14	11	16	8	51	55	2 1/2: 1
36"	14	13	19	9	60	70	2 1/2: 1
42"	12	15	25	10	69	82	2 1/2: 1
48"	12	17	29	12	78	88	2 1/2: 1
54"	12	17	33	12	84	100	2 1/2: 1
60"	12/10	17	36	12	87	112	2: 1
66"	12/10	17	39	12	87	118	2: 1
72"	12/10	17	44	12	87	120	2: 1
78"	12/10	17	48	12	87	130	1 1/2: 1
84"	12/10	17	52	12	87	136	1 1/2: 1
90"	12/10	17	58	12	87	142	1 1/2: 1
96"	12/10	17	58	12	87	144	1 1/2: 1

Bid Designation Sq. Ft.	Nom. W.W. Area Sq. Ft.	Pipe Arch Span & Rise	Dimensions in Inches 2 1/2" x 1/2" Corrugations					Dimensions in Inches 3" x 1" or 5" x 1" Corr.					Approx. Slope		
			CS, ACS or CA Gauge	A (min.)	B (max.)	H (min.)	L (±2")	W (min.)	CS, ACS or CA Gauge	A (min.)	B (max.)	H (min.)		L (±2")	W (min.)
1.0	1.1	17" x 13"	16	5	9	6	20	28						2 1/2: 1	
1.5	1.6	21" x 15"	16	6	11	6	24	34						2 1/2: 1	
2.0	2.2	24" x 18"	16	7	12	6	28	40						2 1/2: 1	
2.5	2.9	28" x 20"	16	7	16	6	32	46						2 1/2: 1	
3.0 or 4.0	4.5	35" x 24"	14	9	16	6	39	58						2 1/2: 1	
5.0 or 6.0	6.5	42" x 29"	14	11	18	7	46	73						2 1/2: 1	
7.0 or 8.5	8.9	49" x 33"	12	12	21	9	53	82						2 1/2: 1	
10.0 or 11.0	11.7	53" x 41"							12	17	26	12	63	88	2: 1
10.0 or 11.0	11.6	57" x 38"	12	16	26	12	62	88							2: 1
12.5 or 14.0	15.6	60" x 46"							12	17	36	12	70	100	2: 1
12.5 or 14.0	14.7	64" x 43"	12	17	30	12	69	100							2: 1
16.5	19.3	66" x 51"							12/10	17	36	12	70	112	1 1/2: 1
16.5	18.1	71" x 47"	12/10	17	36	12	77	112							1 1/2: 1
21.0	23.2	73" x 55"							12/10	17	36	12	77	124	1 1/2: 1
21.0	21.9	77" x 52"	12/10	17	36	12	77	124							1 1/2: 1
25.0	27.4	81" x 59"							12/10	17	44	12	77	136	1 1/2: 1
25.0	26.0	83" x 57"	12/10	17	44	12	77	130							1 1/2: 1
32.0	32.1	87" x 63"							12/10	17	44	12	77	136	1 1/2: 1
36.0	37.0	95" x 67"							12/10	17	44	12	87	160	1 1/2: 1
42.0	42.4	103" x 71"							12/10	17	44	12	87	172	1 1/2: 1
47.0	48.0	112" x 75"							12/10	17	44	12	87	172	1 1/2: 1

(Information listed in these tables are nominal and may vary by manufacturer.)



DETAILS FOR H-7 HUGGER BAND (12" thru 36") or H-10 HUGGER BAND (12" thru 120")



DETAILS FOR H-12 or H-13 HUGGER BAND

Pipe Dia. Inches	Minimum Gauge of Round Pipe				
	2 1/2" x 1/2" Corr. CSP or ACSP	3" x 1" Corr. CSP or ACSP	5" x 1" Corr. CSP or ACSP	2 1/2" x 1/2" Corr. CAP	3" x 1" Corr. CAP
12"	14			16	
15"	14			16	
18"	14			16	
21"	14			16	
24"	14			16	
30"	14			14	16
36"	14			14	16
42"	14			12	16
48"	12	14	16	14	16
54"	12	14	16	14	16
60"	10	14	16	14	16
66"	10	14	16	14	16
72"	10	14	16	14	16
78"	8	14	14	14	14
84"	8	14	14	14	14
90"		14	14	14	14
96"		12	12	12	12
102"		12	12	12	12
108"		12	12	12	12
114"		12	12	12	12
120"		10	10	10	10

Bid Designation Sq. Ft.	Pipe Dimension Span & Rise	Sq. Ft.	Equiv. Round Pipe Diameter	Minimum Gauge of Arch Pipe				
				2 1/2" x 1/2" Corr. CSP or ACSP	3" x 1" Corr. CSP or ACSP	5" x 1" Corr. CSP or ACSP	2 1/2" x 1/2" Corr. CAP	3" x 1" Corr. CAP
1.0	17" x 13"	1.1	15"	14			16	
1.5	21" x 15"	1.6	18"	14			16	
2.0	24" x 18"	2.2	21"	14			16	
2.5	28" x 20"	2.9	24"	14			14	
3.0 or 4.0	35" x 24"	4.5	30"	14			14	
5.0 or 6.0	42" x 29"	6.5	36"	14			12	
7.0 or 8.5	49" x 33"	8.9	42"	14			12	
10.0 or 11.0	53" x 41"	11.7	48"		14			
10.0 or 11.0	57" x 38"	11.6	48"	12			10	
12.5 or 14.0	60" x 46"	15.6	54"		14			14
12.5 or 14.0	64" x 43"	14.7	54"	12			10	
16.5	66" x 51"	19.3	60"		14			14
16.5	71" x 47"	18.1	60"	10			8	
21.0	73" x 55"	23.2	66"		14			14
21.0	77" x 52"	21.9	66"	8				
25.0	81" x 59"	27.4	72"		14	12		12
25.0	83" x 57"	26.0	72"	8				
32.0	87" x 63"	32.1	78"		12	12		12
36.0	95" x 67"	37.0	84"		12	12		12
42.0	103" x 71"	42.4	90"		12	12		10
47.0	112" x 75"	48.0	96"		12	12		8
54.0	117" x 79"	54.2	102"		10	10		
60.0	128" x 83"	60.5	108"		10	10		
67.0	137" x 87"	67.4	114"		10	10		
74.0	142" x 91"	74.5	120"		8	8		

GENERAL NOTE for METAL PIPE
 Culvert "Type" listed may be CSP, ACSP, CAP, RCP, PVCP & PEP within guidelines of KDOT Pipe Policy for geographic location. More than one pipe "Type" may be acceptable for a design location with allowable types listed for each site.
 There shall be no payment for gain in pipe length due to fit of pipe at connecting band.
 When Hugger Bands are used, the H-7 Hugger Band may be used on circular pipes 36" diameter and smaller or pipe arches 42" x 29" and smaller. The H-10 Hugger Band may be used on 12" thru 120" pipe. The H-12 or H-13 Hugger Band are for pipe sizes larger than 36" diameter or 42" x 29" arch pipe.
 Pipe gauge listed in the tables on this sheet are minimum for E=750 p.s.i. soil. Pipe gauge will be determined for each site based on the Design Manual Volume I- Part C Fill Height Tables and shall be listed in the Pipe Culvert Summary. Gauges shown on this Standard Drawing are KDOT minimum and may not be industry minimum gauge.
 In geographic areas that allow CSP (24" or smaller arched or round pipe) for entrance and side road installation with less than 3,000 AADT, 16 gauge ACSP may be substituted for 14 gauge CSP.
 Aluminum or aluminized pipes or end sections shall be coated with an asphaltic paint when in contact with fresh concrete in accordance with the Standard Specifications.

04	09-10-09	Rev. Round and Arch tables, add. Alum.	S.W.K.	J.O.B.
03	01-20-09	Rev. Round and Arch tables, add. Alum.	S.W.K.	J.O.B.
02	04-18-08	Rev. layout, details, tables and notes	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

METAL END SECTION FOR ROUND & ARCH METAL CULVERTS (TYPE I) & PIPE GAUGE TABLES

RD660

FHWA APPROVAL	12-16-09	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

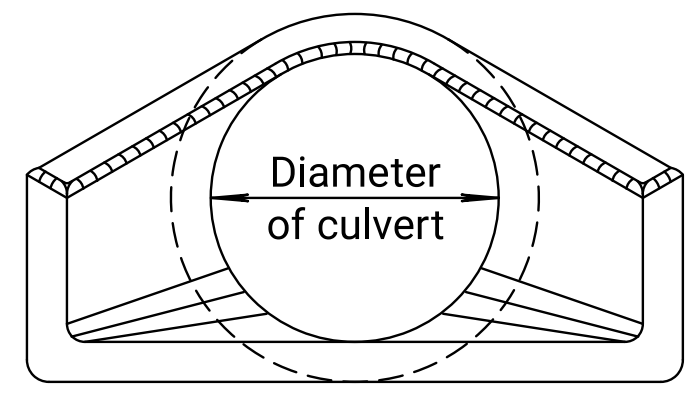
KDOT Graphics Certified 05-16-2022 Sh. No. 23

Plotted by: August.Zuno@ks.gov 4-JAN-2024 20:14
 File: rrs660.dgn

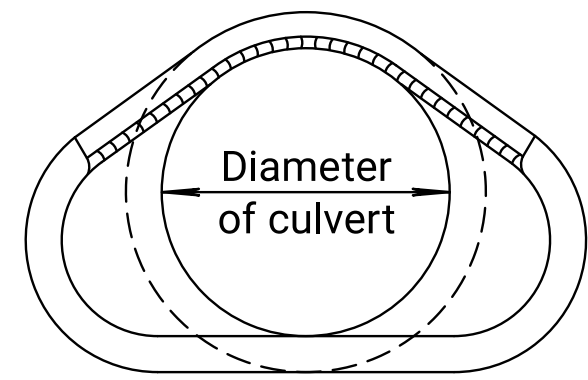
KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	24	135

Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVC, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

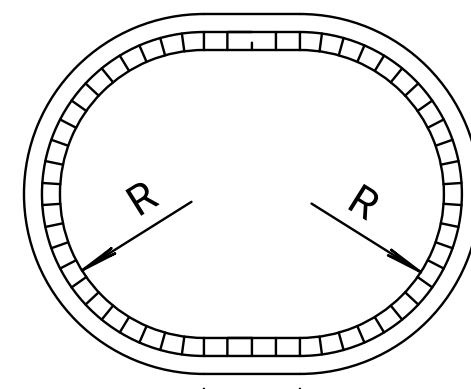
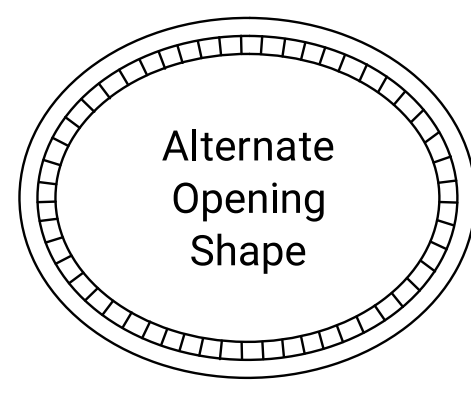


END ELEVATION (TYPE I)

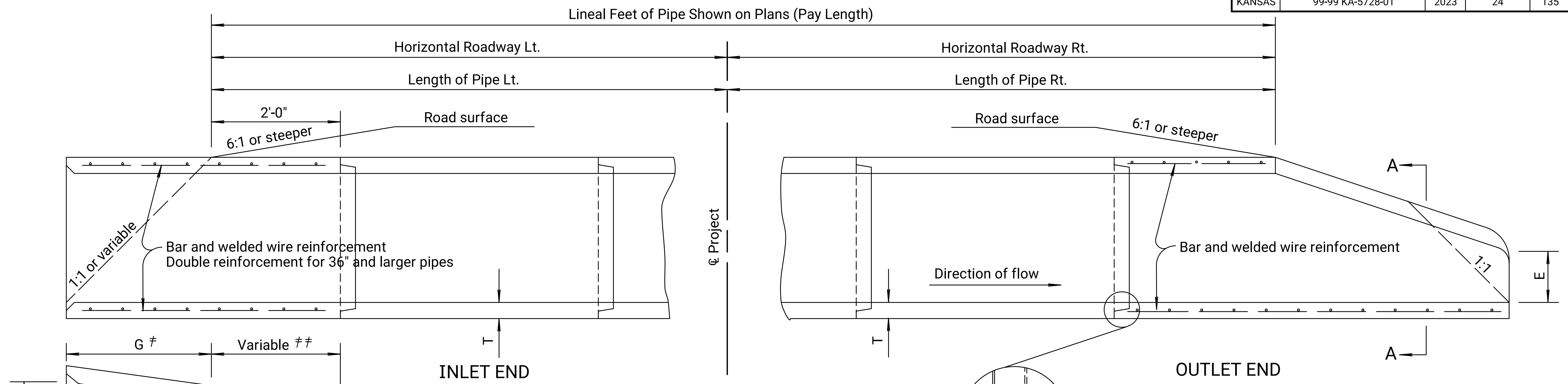


SECTION A-A

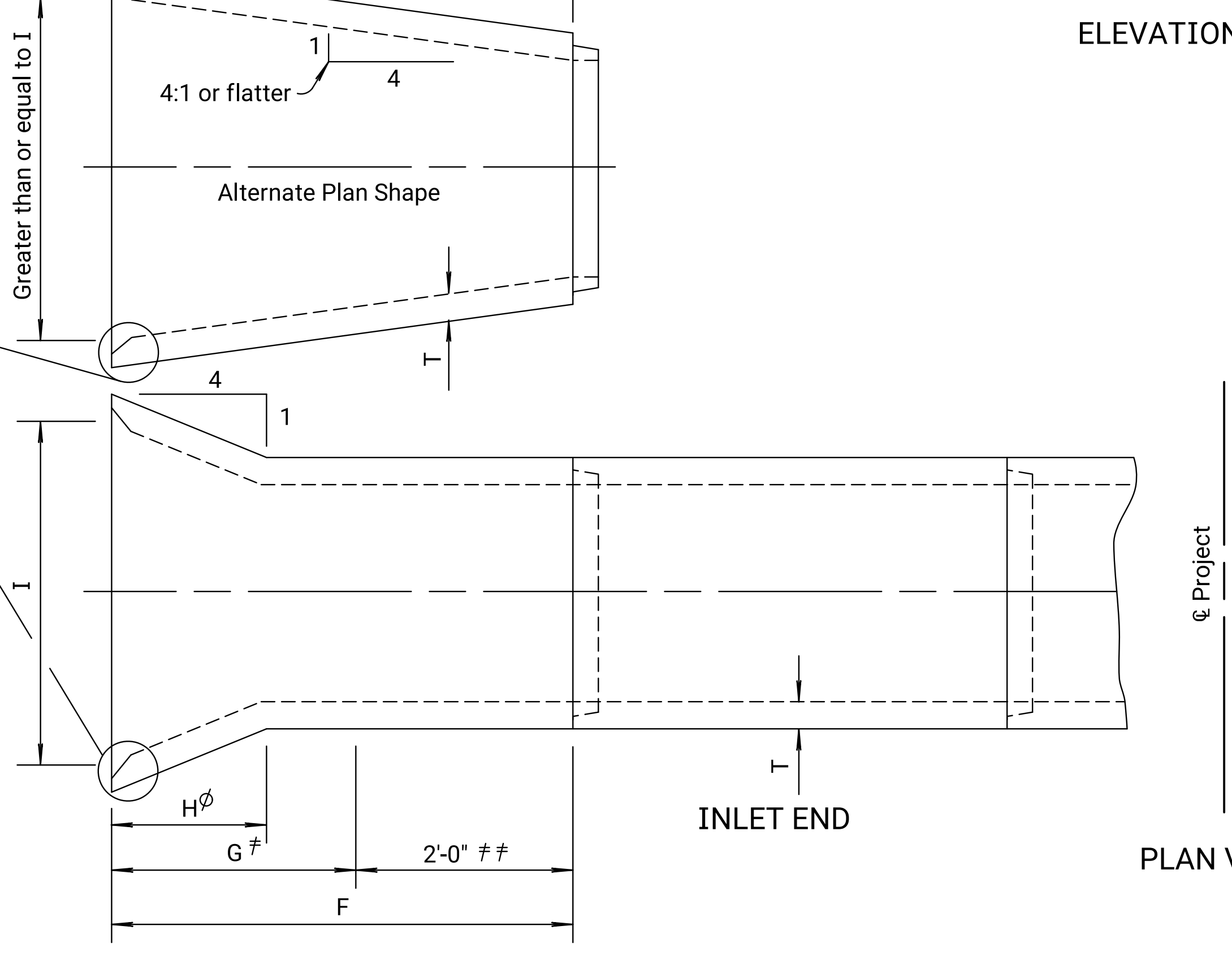
Showing rounding of inside edge of end section.



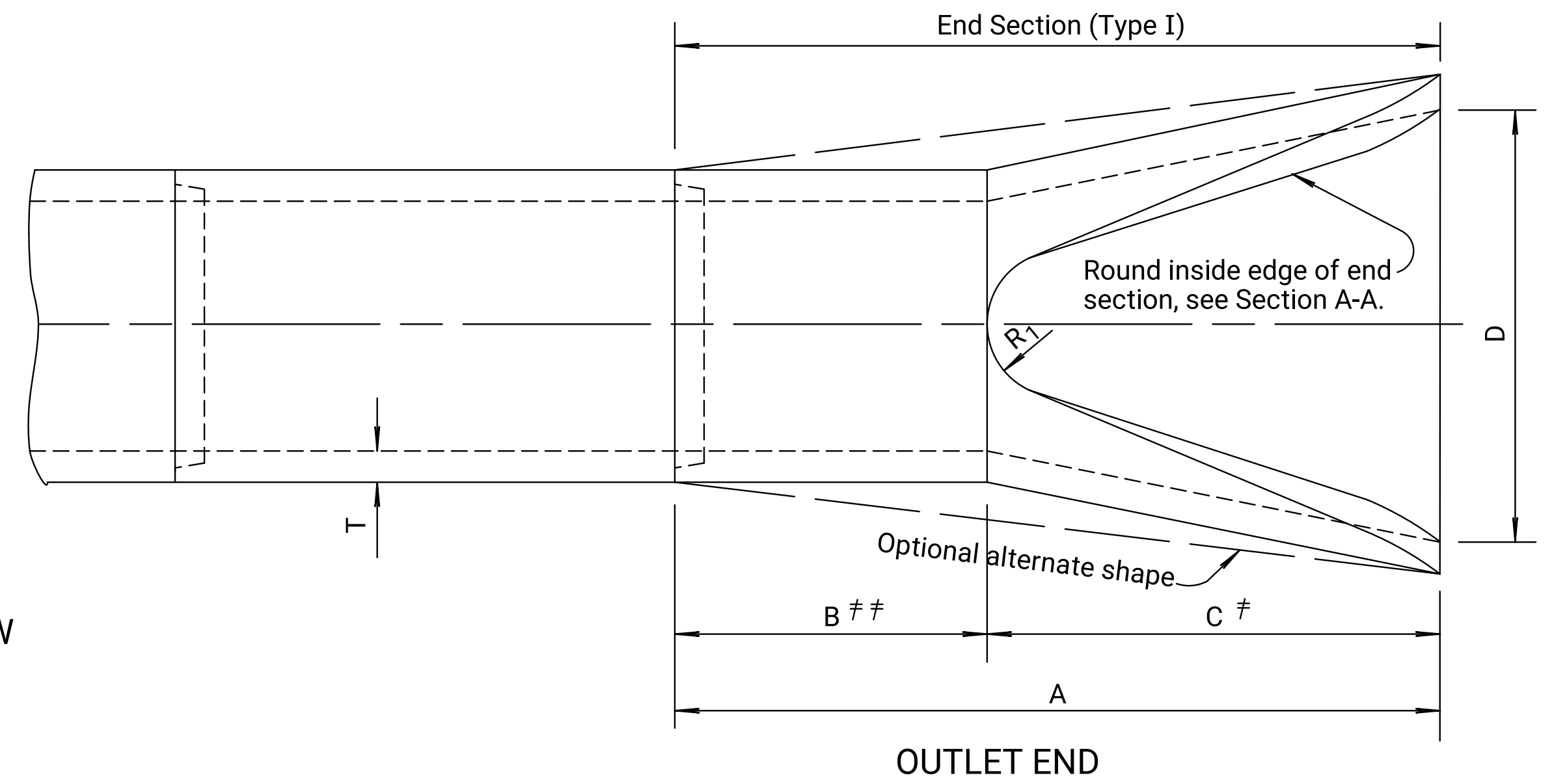
END ELEVATION (TYPE III)



ELEVATION SECTION



PLAN VIEW



OUTLET END

Note: There shall be no payment for gain in length due to joint fit tolerance.

- ∅ Transition to round pipe.
- ≠ Paid for as separate item of End Section, except when structures shall bid as alternates. In that case End Sections shall be subsidiary to bid item. "Drainage Structure No. ".
- ≠≠ Included in pay length of pipe.
- * Minimum waterway area is calculated at the inside of the bevel.

Diam.	A	B≠	C≠	D	E	R:	Slope	T
12"	6'-0 7/8"	4'-0 7/8"	2'-0"	2'-0"	4"	9	3:1	2"
15"	6'-1"	3'-10"	2'-3"	2'-6"	6"	11	3:1	2 1/4"
18"	6'-1"	3'-10"	2'-3"	3'-0"	9"	12	3:1	2 1/2"
24"	6'-1 1/2"	2'-6"	3'-7 1/2"	4'-0"	9 1/2"	14	3:1	3"
30"	6'-1 3/4"	1'-7 3/4"	4'-6"	5'-0"	1'-0"	15	3:1	3 1/2"
36"	8'-1 3/4"	2'-10 3/4"	5'-3"	6'-0"	1'-3"	20	3:1	4"
42"	8'-2"	2'-11"	5'-3"	6'-6"	1'-9"	22	3:1	4 1/2"
48"	8'-2"	2'-2"	6'-0"	7'-0"	2'-0"	22	3:1	5"
54"	8'-2 1/4"	2'-9 1/4"	5'-5"	7'-6"	2'-3"	24	2.4:1	5 1/2"
60"	8'-3"	3'-3"	5'-0"	8'-0"	2'-11"	24	2:1	6"
72"	8'-3"	1'-9"	6'-6"	9'-0"	3'-0"	24	1.86:1	7"
84"	9'-3 1/2"	1'-9"	7'-6 1/2"	10'-0"	3'-0"	24	1.6:1	8"

Diam.	Min. W.W.* Area Sq. Ft.	F	G	H	I	J	K	R	T
24"	4.5	4'-3"	2'-3"	1'-5 1/8"	2'-8"	1 1/2"	8"	1'-0"	3"
30"	7.0	4'-9 1/2"	2'-9 1/2"	1'-9 1/2"	3'-4"	2"	10"	1'-3"	3 1/2"
36"	10.1	5'-4"	3'-4"	2'-1 1/2"	4'-0"	2"	1'-0"	1'-6"	4"
42"	13.7	5'-10 1/2"	3'-10 1/2"	2'-5 7/8"	4'-8"	2 1/2"	1'-2"	1'-9"	4 1/2"
48"	17.9	6'-5"	4'-5"	2'-10 1/8"	5'-4"	3"	1'-4"	2'-0"	5"
54"	22.7	6'-11 1/2"	4'-11 1/2"	3'-2 1/2"	6'-0"	3 1/2"	1'-6"	2'-3"	5 1/2"
60"	28.0	7'-6"	5'-6"	3'-6 7/8"	6'-8"	4"	1'-8"	2'-6"	6"
72"	40.3	8'-7"	6'-7"	4'-3 3/8"	8'-0"	5"	2'-0"	3'-0"	7"
84"	54.8	9'-8"	7'-8"	5'-0 3/8"	9'-4"	6"	2'-4"	3'-6"	8"

Dimensions for alternate shapes shall be equal to or greater than those shown in the table, unless otherwise shown.

Plotted by: August.Zuno@ks.gov 4-JAN-2024 20:15
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02	04-18-08	Added ref. to KDOT Pipe Policy	S.W.K.	J.O.B.
01	04-05-05	Revised reinforcement callout	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

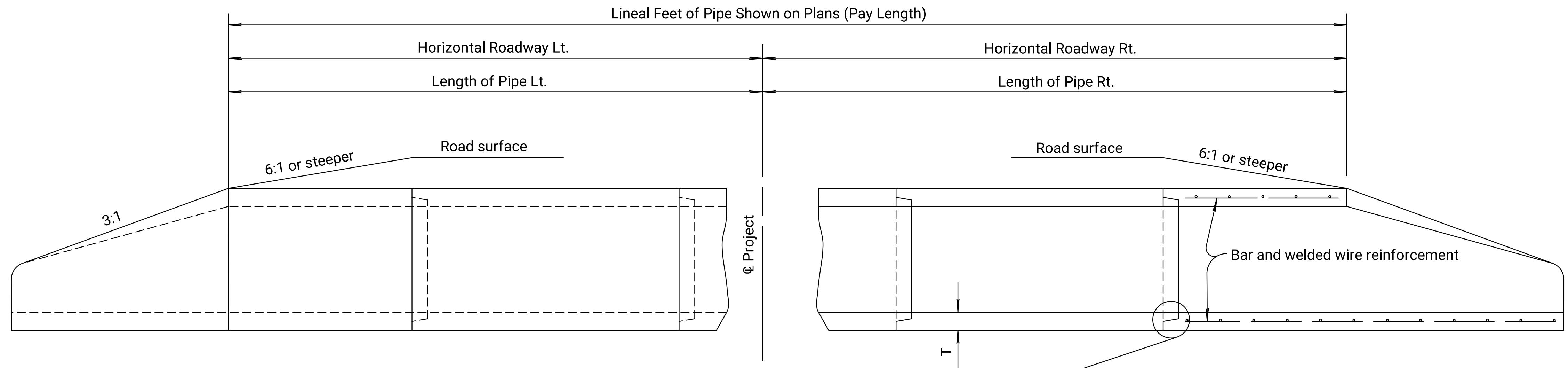
KANSAS DEPARTMENT OF TRANSPORTATION
CONCRETE END SECTIONS FOR CONCRETE PIPES
TYPE I & SIDE TAPERED INLET SECTION (TYPE III)
RD662

FHWA APPROVAL	06-27-08	APPD.	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

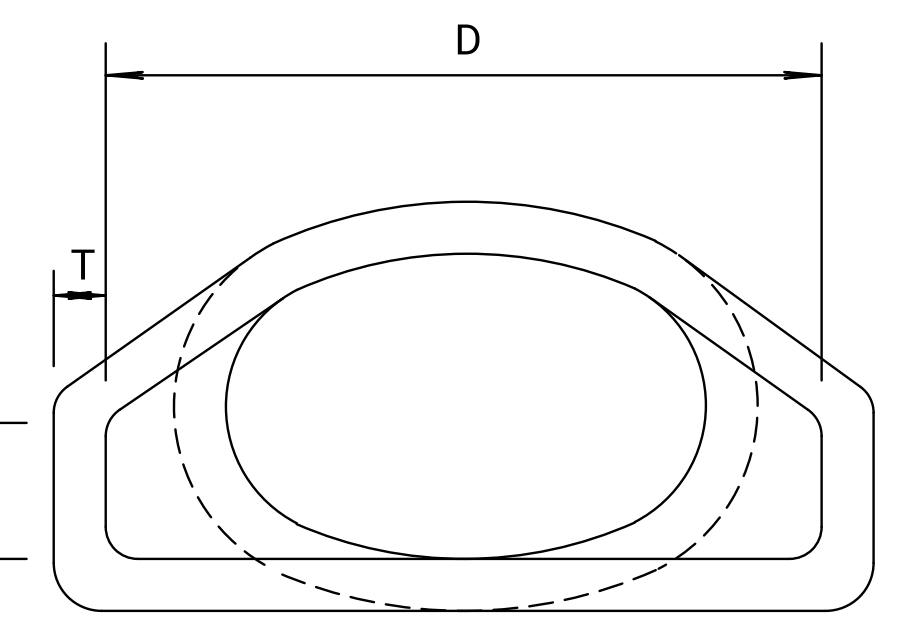
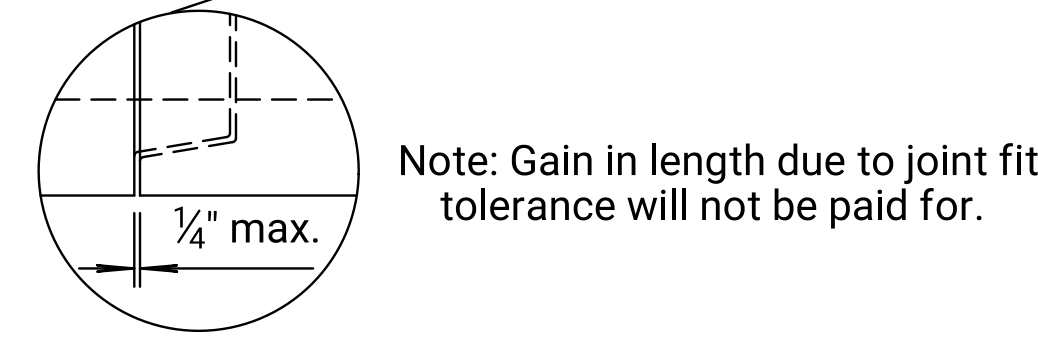
KDOT Graphics Certified 05-16-2022 Sh. No. 24

KDOT Graphics Certified

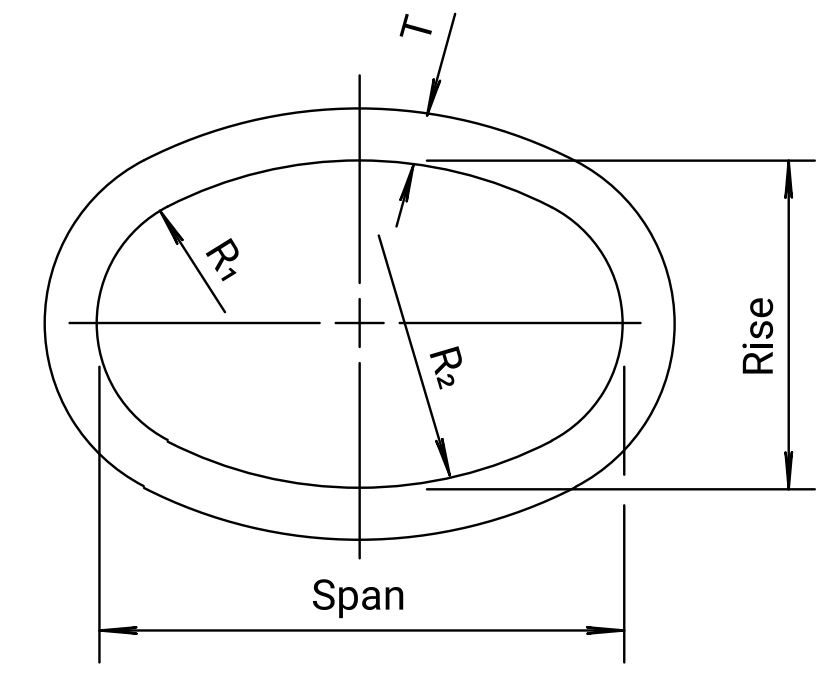
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	25	135



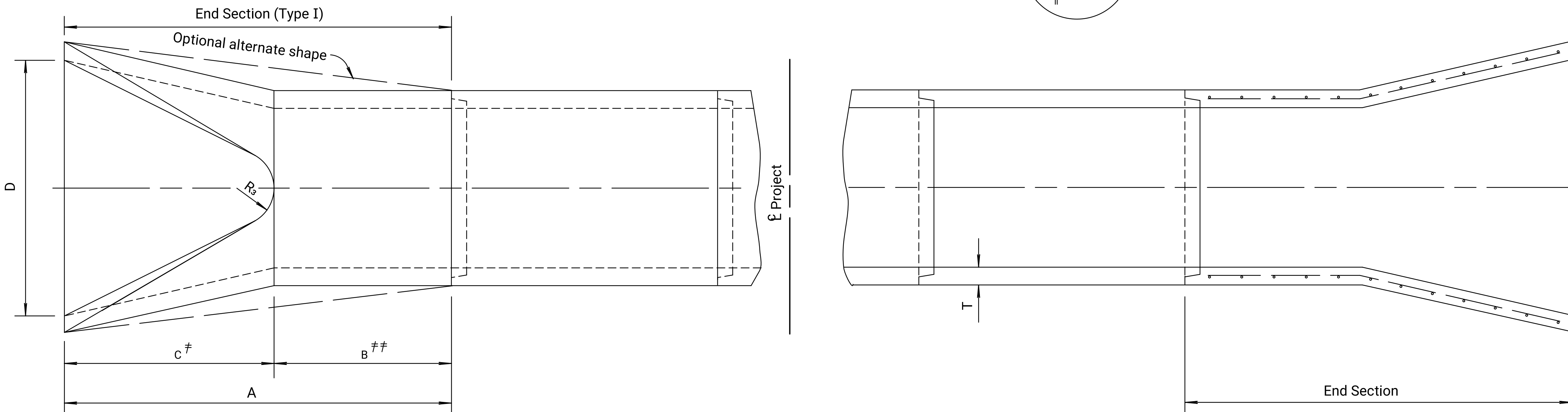
ELEVATION AND SECTION



FLARED END SECTION ELEVATION



END ELEVATION



PLAN AND SECTION

END SECTION (TYPE I) NOMINAL DIMENSIONS													
Bid Designation Sq. Ft.	Equiv. Dia. (in.)	Min. W.W. Area Sq. Ft.	Span	Rise	Overall Length		C †	D	E	R ₁	R ₂	R ₃	T
					A	B ††							
1.0 or 1.5	18"	1.8	23"	14"	6'-0"	3'-9"	2'-3"	3'-0"	8"	6"	20"	6"	2 3/4"
2.0, 2.5 or 3.0	24"	3.3	30"	19"	6'-0"	2'-9"	3'-3"	4'-0"	8 1/2"	8 1/4"	26 1/4"	7"	3 1/4"
4.0	27"	4.1	34"	22"	6'-0"	2'-2"	3'-10"	4'-6"	9"	9 1/4"	29 17/32"	8"	3 1/2"
5.0	30"	5.1	38"	24"	6'-0"	1'-6"	4'-6"	5'-0"	9 1/2"	10 1/4"	32 3/4"	9"	3 3/4"
6.0	33"	6.3	42"	27"	6'-0"	1'-3"	4'-9"	5'-6"	10 3/8"	11 7/16"	36 3/16"	10 1/2"	3 3/4"
7.0	36"	7.4	45"	29"	8'-0"	3'-0"	5'-0"	6'-0"	11 1/4"	12 1/4"	39 1/4"	12"	4 1/2"
8.5	39"	8.8	49"	32"	8'-0"	3'-0"	5'-0"	6'-3"	12"	13 9/16"	42 21/32"	12 1/2"	4 3/4"
10.0	42"	10.2	53"	34"	8'-0"	3'-0"	5'-0"	6'-6"	15 3/4"	14 3/4"	46"	13"	5"
11.0 or 12.5	48"	12.9	60"	38"	8'-0"	3'-0"	5'-0"	7'-0"	21"	16 1/2"	51 3/4"	14"	5 1/2"
14.0 or 16.5	54"	16.6	68"	43"	8'-0"	3'-0"	5'-0"	7'-6"	25 1/2"	18 21/32"	58 13/32"	16"	6"

† Paid for as separate item of "End Sections".
 †† Included in pay length of pipe.

Design of end section shall conform to standard reinforced concrete horizontal elliptical pipe. Slight variations in the dimensions specified will be allowed.

Note: Reinforced concrete pipe extensions are based on the surveyed end of pipe. Replacement of any additional pipe length required due to the removal of the existing end section will not be paid for directly, but will be subsidiary to the bid item "Removal of Existing Structures".

04	05-17-13	Added Note, Pipe Extensions Subsidiary	S.W.K.	J.O.B.
03	04-18-08	Added ref. to KDOT Pipe Policy	S.W.K.	J.O.B.
02	04-06-05	Revised reinforcement callout	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

CONCRETE END SECTION FOR REINFORCED CONCRETE HORIZONTAL ELLIPTICAL PIPE TYPE I

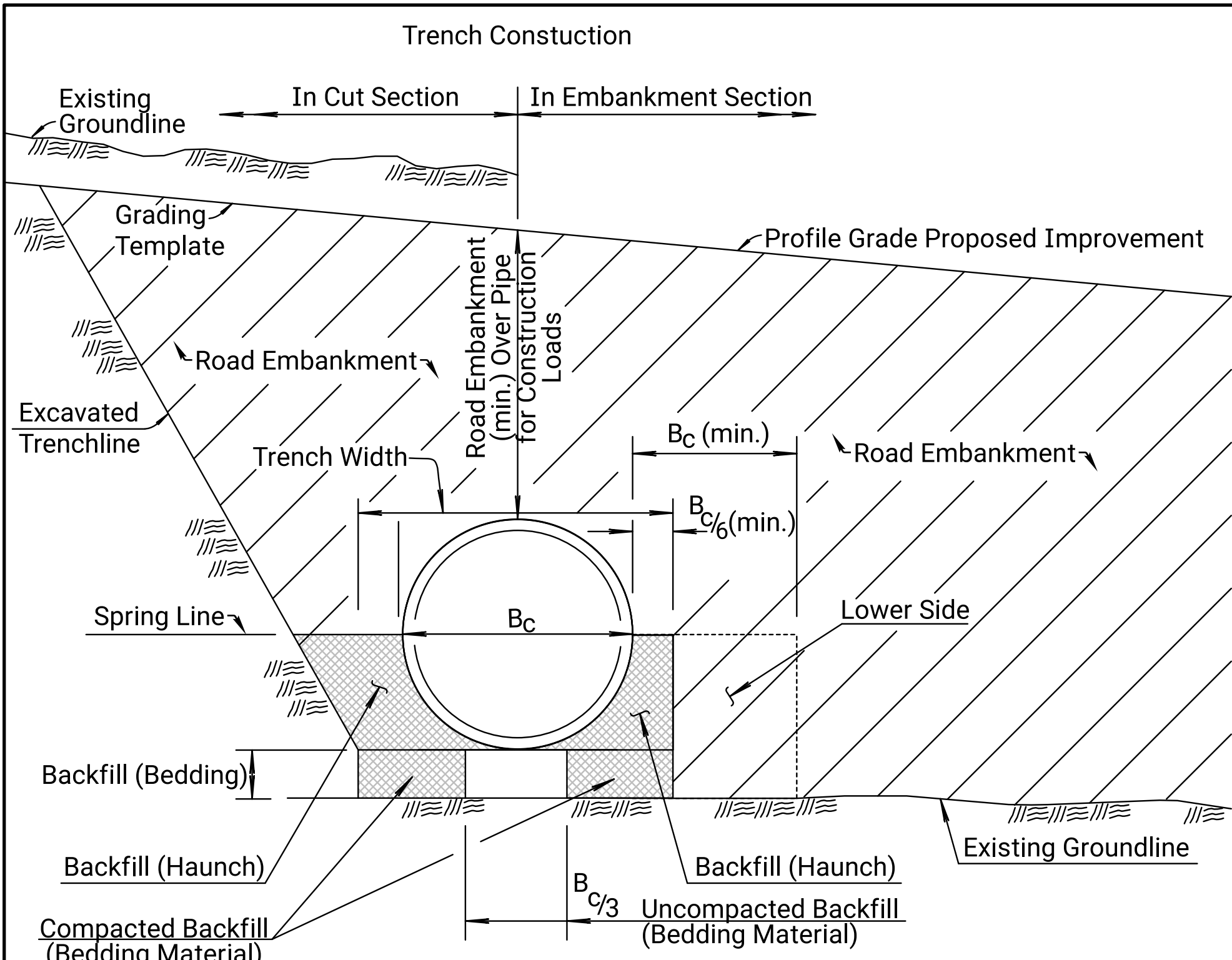
RD663

DESIGNED	09-04-14	APPD.	Jame O. Brewer
DESIGN CK.	DETAIL CK.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVC, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

Plotted by: August.Zuno@ks.gov 04-JAN-2024 20:15
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STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	26	135



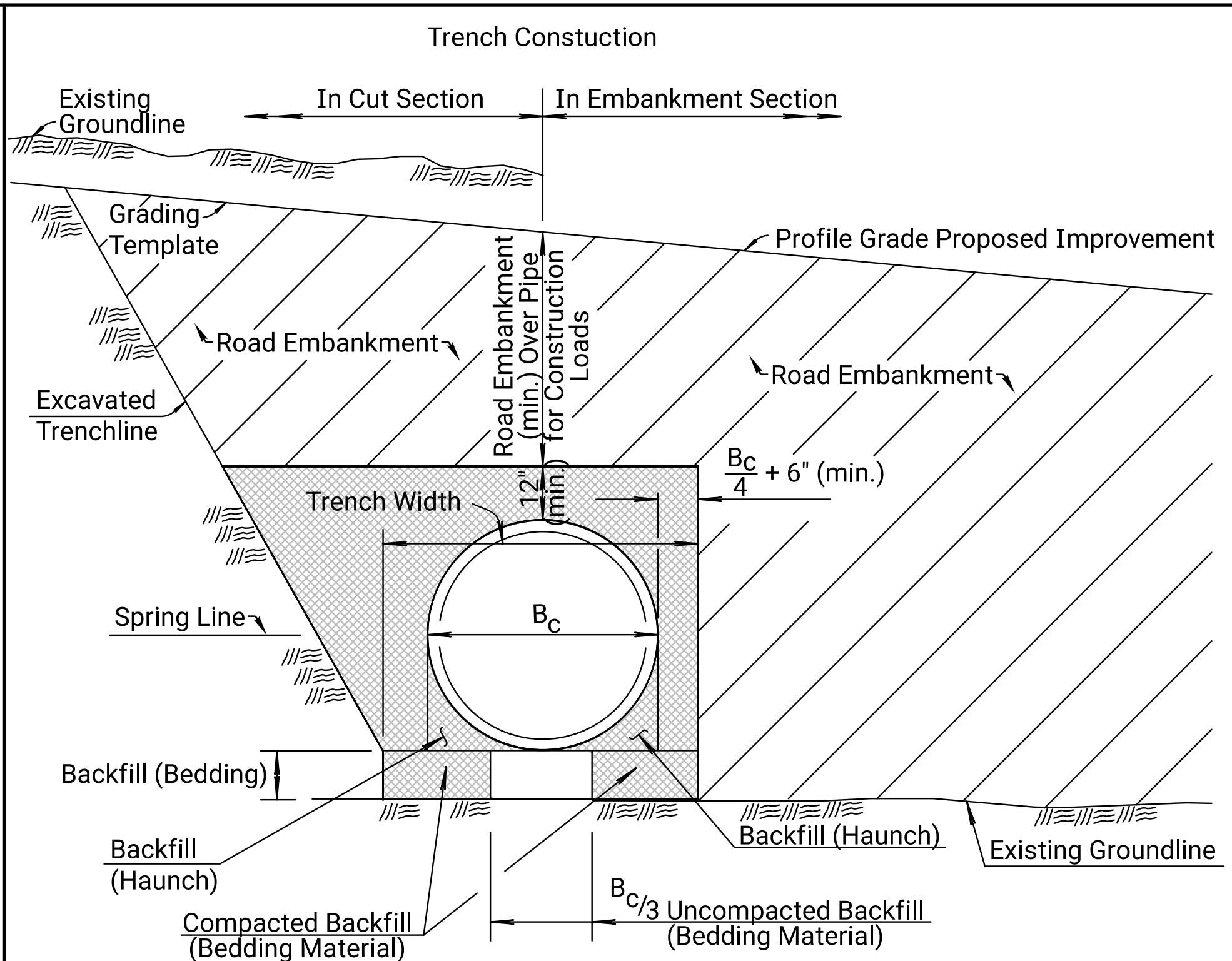
REINFORCED CONCRETE PIPE

- Backfill (Bedding) Thickness: When over soil, use $B_c/24$ " (min. of 4" & max of 24") of PB-2 or PB-3. When over rock, excavate $B_c/12$ " (min. of 6") below the bottom of the pipe and replace with PB-3.
- Backfill Material: See KDOT's Standard Specifications, Aggregates for Backfill in Division 1100.
- Compacted Backfill Material: Use Type B compaction in the haunch areas shown above. Use Type A compaction Backfill (Bedding), unless otherwise specified in the Contract Documents.
- Road Embankment: For compaction See KDOT Road Standard Drawing, Foundation Treatment & Compaction of Earthwork.
- Trench Width: The minimum required trench width is the greater of: $1.33 \times B_c$ or $B_c + 24$ "
- B_c : Outside Pipe Diameter

Pipe Dia. (Inside Diameter)	Axle Load (Tons)			
	<25	25-37.5	37.5-55	55-75
≤ 15"	2'-0"	3'-0"	4'-6"	6'-0"
18"	2'-0"	3'-0"	4'-0"	6'-0"
21" - 24"	1'-6"	2'-6"	4'-0"	6'-0"
27"	1'-6"	2'-6"	4'-0"	5'-6"
30"	1'-0"	2'-6"	4'-0"	5'-6"
33"	1'-0"	2'-0"	4'-0"	5'-6"
36"	1'-0"	1'-6"	4'-0"	5'-6"
42"	1'-0"	1'-6"	3'-6"	5'-6"
48" - 54"	1'-0"	1'-6"	2'-6"	5'-6"
60" - 66"	1'-0"	2'-0"	2'-6"	5'-6"

Pipe Dia. (Inside Diameter)	Axle Load (Tons)			
	<25	25-37.5	37.5-55	55-75
72"	1'-0"	2'-0"	3'-6"	4'-0"
66" - 84"	1'-0"	2'-0"	3'-0"	4'-0"
90"	1'-0"	2'-0"	3'-6"	4'-6"
96"	1'-0"	2'-6"	3'-6"	4'-6"
102"	0'-6"	2'-6"	3'-6"	5'-0"
108"	0'-6"	2'-0"	3'-6"	5'-0"
114"	0'-6"	2'-0"	4'-0"	5'-0"
120" - 132"	0'-6"	2'-0"	4'-0"	5'-6"
144"	1'-0"	2'-6"	4'-6"	5'-6"

Minimum Road Embankment height should be no less than the value in the provided table or the manufacturer's specification.

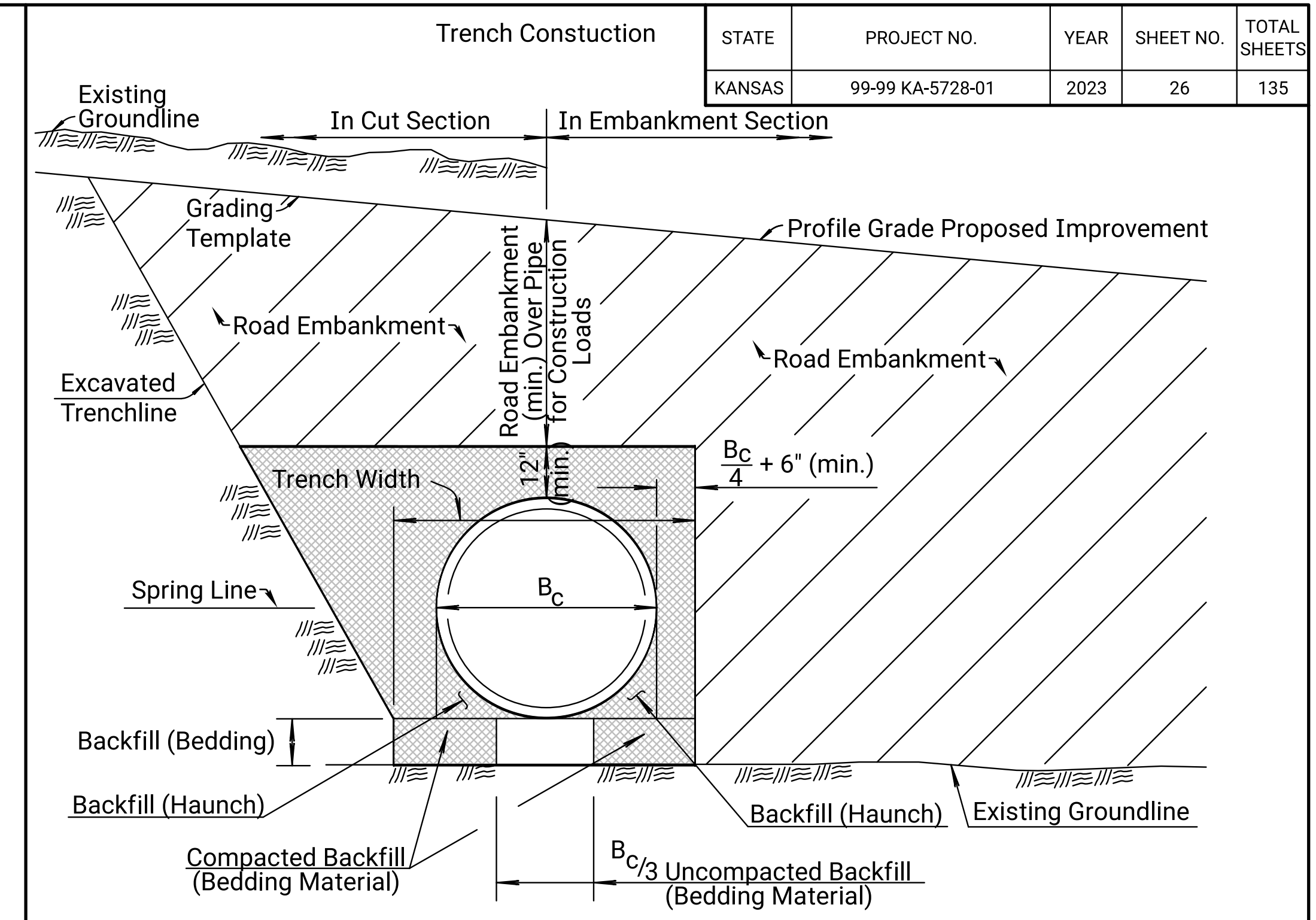


CORRUGATED METAL PIPE

- Backfill (Bedding) Thickness: When over soil, use $\frac{1}{2}$ in./ft. of fill over pipe (min. of 4" & max. of 24") of PB-2 or PB-3. When over rock, excavate 6" below the bottom of the pipe and replace with PB-2 or PB-3.
- Backfill Material: See KDOT's Standard Specifications, Aggregates for Backfill in Division 1100.
- Compacted Backfill Material: See KDOT's Standard Specifications and Pipe Manufacturer's Installation Guidelines.
- Compaction: Use Type B compaction in the haunch areas shown above. Use Type A compaction Backfill (Bedding), unless otherwise specified in the Contract Documents.
- Road Embankment: For compaction See KDOT Road Standard Drawing, Foundation Treatment & Compaction of Earthwork
- Trench Width: The minimum required trench width is the greater of: $1.50 \times B_c + 12$ " or $B_c + 16$ "
- B_c : Outside Pipe Diameter

Pipe Dia. (Inside Diameter)	Axle Load (Tons)			
	<25	25-37.5	37.5-55	55-75
< 42"	2'-0"	2'-6"	3'-0"	3'-0"
48" - 72"	3'-0"	3'-0"	3'-6"	4'-0"
78" - 120"	3'-0"	3'-6"	4'-0"	4'-0"
126" - 144"	3'-6"	4'-0"	4'-4"	4'-4"

Minimum Road Embankment height should be no less than the value in the provided table or the manufacturer's specification.



THERMOPLASTIC PIPE

- Backfill (Bedding) Thickness: When over soil, use $\frac{1}{2}$ in./ft. of fill over pipe (min. of 4" & max. of 24") of PB-2 or PB-3. When over rock, excavate 6" below the bottom of the pipe and replace with PB-2 or PB-3.
- Backfill Material: See KDOT's Standard Specifications, Aggregates for Backfill in Division 1100.
- Compacted Backfill Material: Use Type B compaction.
- Compaction: Hand-held or walk behind compaction equipment is permitted when compacting fill directly above the pipe only when the fill over the pipe is greater than or equal to 12". The use of ride-on compaction equipment is permitted for compacting fill directly above the pipe only when the fill over the pipe is greater than or equal to 36". A prime goal of pipe installation is to manipulate and compact embedment material under the pipe haunches, to achieve full contact of the material with the pipe bottom and fill voids under the pipe.
- Road Embankment: For compaction See KDOT Road Standard Drawing, Foundation Treatment & Compaction of Earthwork.
- Trench Width: The minimum required trench width is $1.50 \times B_c + 12$ ". The maximum trench width is $1.575 \times B_c + 12\frac{1}{2}$ "
- B_c : Outside Pipe Diameter

Pipe Dia. (Inside Diameter)	Axle Load (Tons)			
	<25	25-37.5	37.5-55	55-75
< 36"	2'-0"	2'-6"	3'-0"	3'-0"
42" - 48"	3'-0"	3'-0"	3'-6"	4'-0"
54" - 60"	3'-0"	3'-0"	3'-6"	4'-0"

Minimum Road Embankment height should be no less than the value in the provided table or the manufacturer's specification.

GENERAL NOTES

Do not drop, drag or otherwise handle pipe sections in a manner which may cause damage. Inspect the line and grade before and during placement of compacted backfill and uncompacted backfill materials. For trench installations place pipe in the center of the excavated trench. When installing pipe, place the uncompacted backfill and compacted backfill material in the bedding area to grade, install pipe to grade, place and compact the haunch area up to the spring line of the pipe and complete the backfill as specified in KDOT's Standard Specifications unless otherwise noted in the contract documents. B_c for horizontal elliptical pipe, vertical elliptical pipe, arch pipe, and non bridge-sized concrete box structures will be measured along the horizontal axis; similar to the dimension shown for circular pipe on this sheet. The spring line is a line along the side of the culvert where the tangent to the culvert wall is vertical. It occurs at the widest point in the culvert. Material used for the roadway embankment may be used in lieu of compacted backfill material as approved by the Engineer. The backfill load transmitted to the pipe is directly dependent on the trench width. Where maximum trench widths are not indicated in any of the contract documents, trench widths should be as narrow as possible with side clearance adequate enough to ensure proper compaction of backfill material at the sides of the pipe. The trench width formulas provided can be used as a general guide.

01	05-09-22	Initial Release	A.L.R.	T.T.R.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
PIPE INSTALLATION DETAILS				
RD658				
FHWA APPROVAL		06-08-22 APPD.		
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	
KDOT Graphics Certified 06-10-2022 Sh. No. 26				

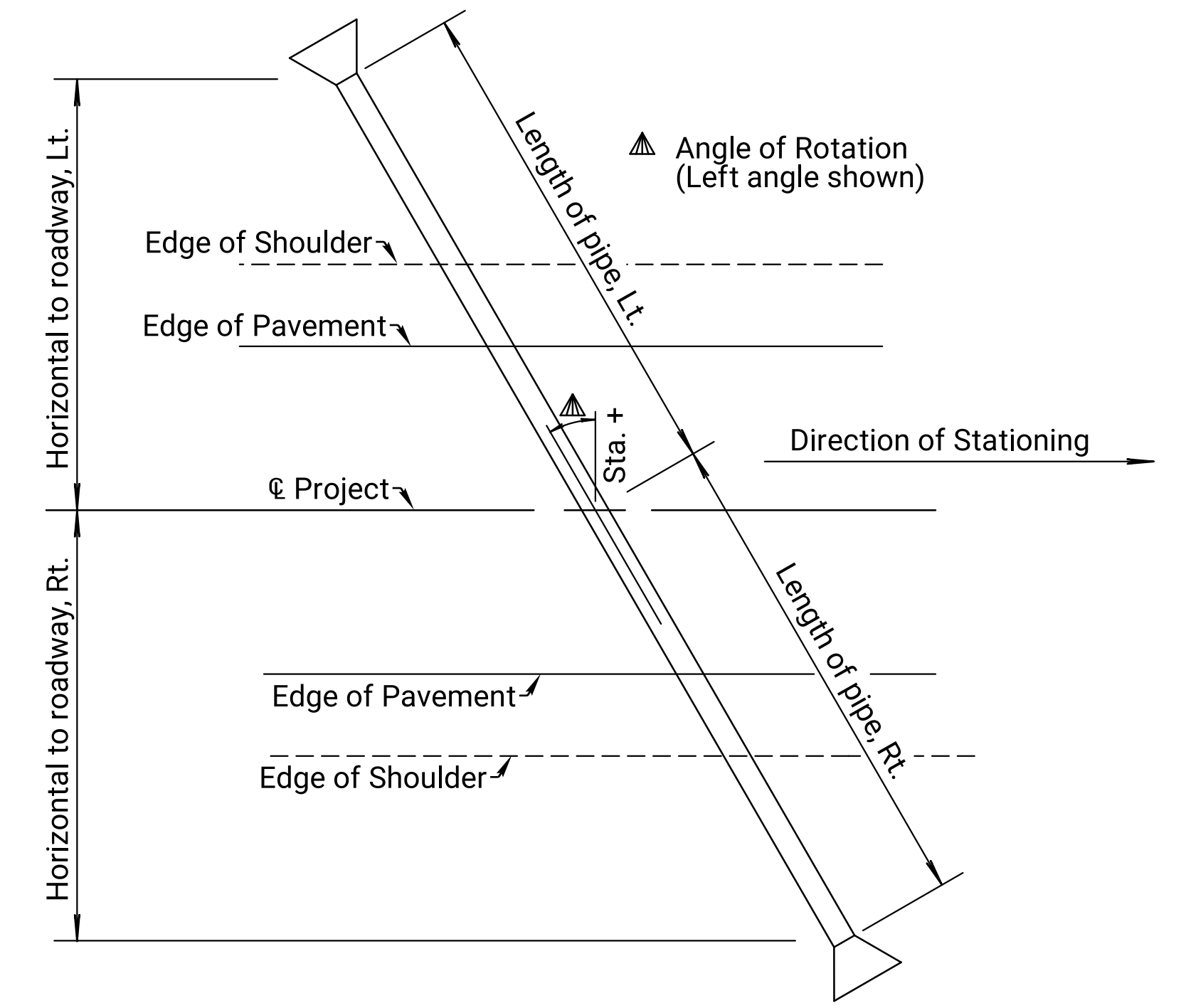
Plotted by: August.Zuno@ks.gov
4-JAN-2024 20:16
File: rsg658.dgn

KDOT Graphics Certified

PIPE CULVERT SUMMARY																				
Station	Type	Size or Bid Designation Sq. Ft.	Crown Grade Elev.	Flow Line		Floor Elev.		Horizontal Roadway		Degree of Rotation	Length of Pipe		Lin. Ft. of Pipe	Height of Fill (max.) Ft.	Concrete Pipe AASHTO Class No.	Pipe Gauge		Pipe Corrugations		Remarks
				Lt.	Rt.	Lt.	Rt.	Lt.	Rt.		Lt.	Rt.				Steel	Alum.	Steel	Alum.	
Mainline 7380+70	E.P.	1.5 ⁰	1306.87	1303.80	1303.94			24	22		24	22	46	1.0	III					
Shoofly 81+50	E.P.	18'	1305.82	1302.81	1303.52			17	15		17	15	32	1.1						Temporary

● Unless otherwise noted, minimum pipe gauge & corrugations to be as shown in RD660. See Summary of Quantities for End Section information.

✱ Only include floor elevations for embedded pipes. See RD668 for details. For structures not embedded, the floor elevations may be omitted.



PLAN
(Showing Rotation about C)

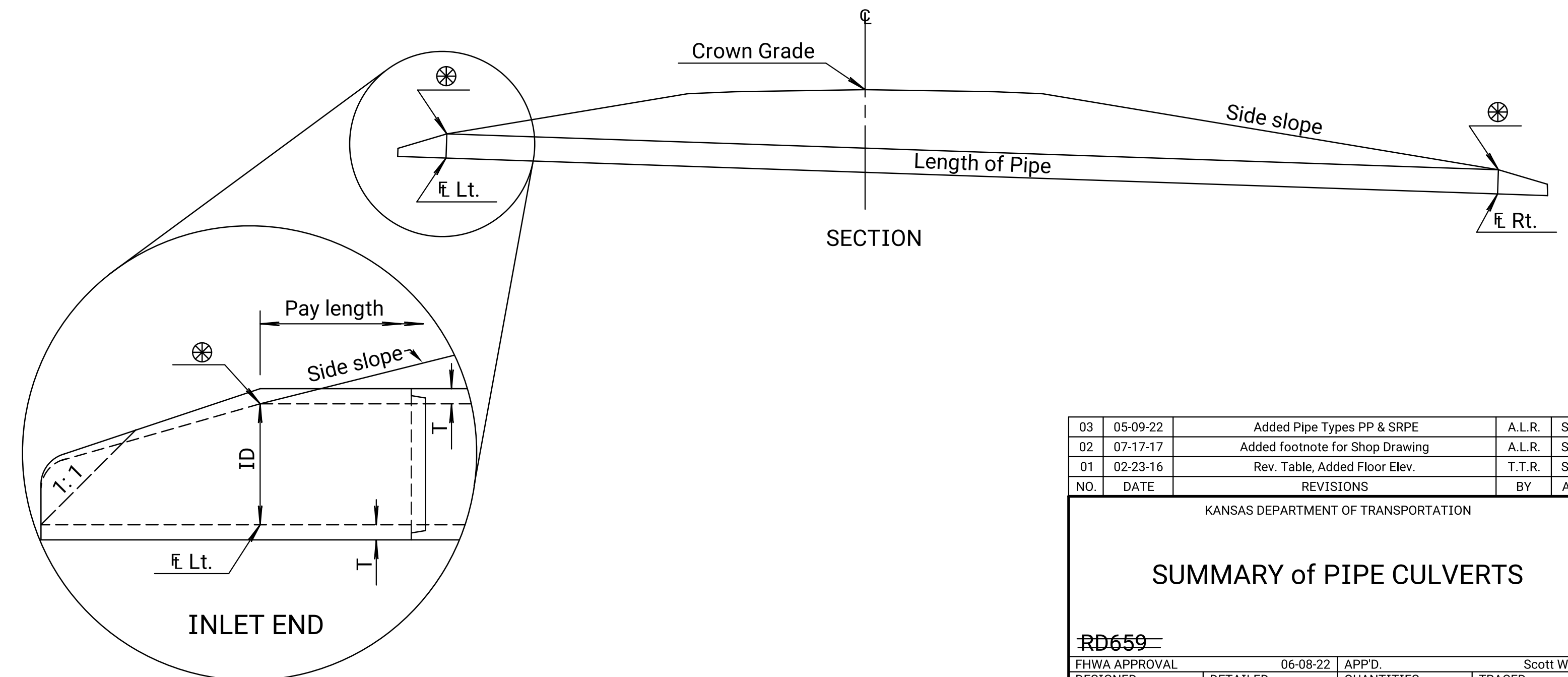
⊗ Design side slope to intersect inside diameter of pipe outside of Clear Zone.

Type	ALLOWABLE LOCATION ▲			
	Mainline	Side Road	Entrance	Storm Sewer Under ML Not Under ML
☆ PVC				
■ PEP				
■ PPP				
⚡ SRPE				
CSP				
ACSP			X	
CAP			X	
RCP			X	

☆ When inside diameter of pipe is 36" or less.
 ▲ Unless otherwise specified in the plans. Some pipe types may not be allowed at a location if the fill height exceeds the maximum allowable or is less than the minimum allowable cover.
 ■ When inside diameter of pipe is 60" or less.
 ⚡ For inside diameter: ≥ 30"

Type	ALLOWABLE END SECTIONS			
	◆ CS	◆ ACS	CA	RC
PVCP				
PEP				
PPP				
SRPE				
RCP				X
ACSP	Provide End Sections of the same material and coating type as the pipe.			
CAP	Provide End Sections of the same material and coating type as the pipe.			
CSP	Provide End Sections of the same material and coating type as the pipe.			

◆ Type IV End Sections are only made of CS or ACS.



NO.	DATE	REVISIONS	BY	APPD
03	05-09-22	Added Pipe Types PP & SRPE	A.L.R.	S.W.K.
02	07-17-17	Added footnote for Shop Drawing	A.L.R.	S.W.K.
01	02-23-16	Rev. Table, Added Floor Elev.	T.T.R.	S.W.K.

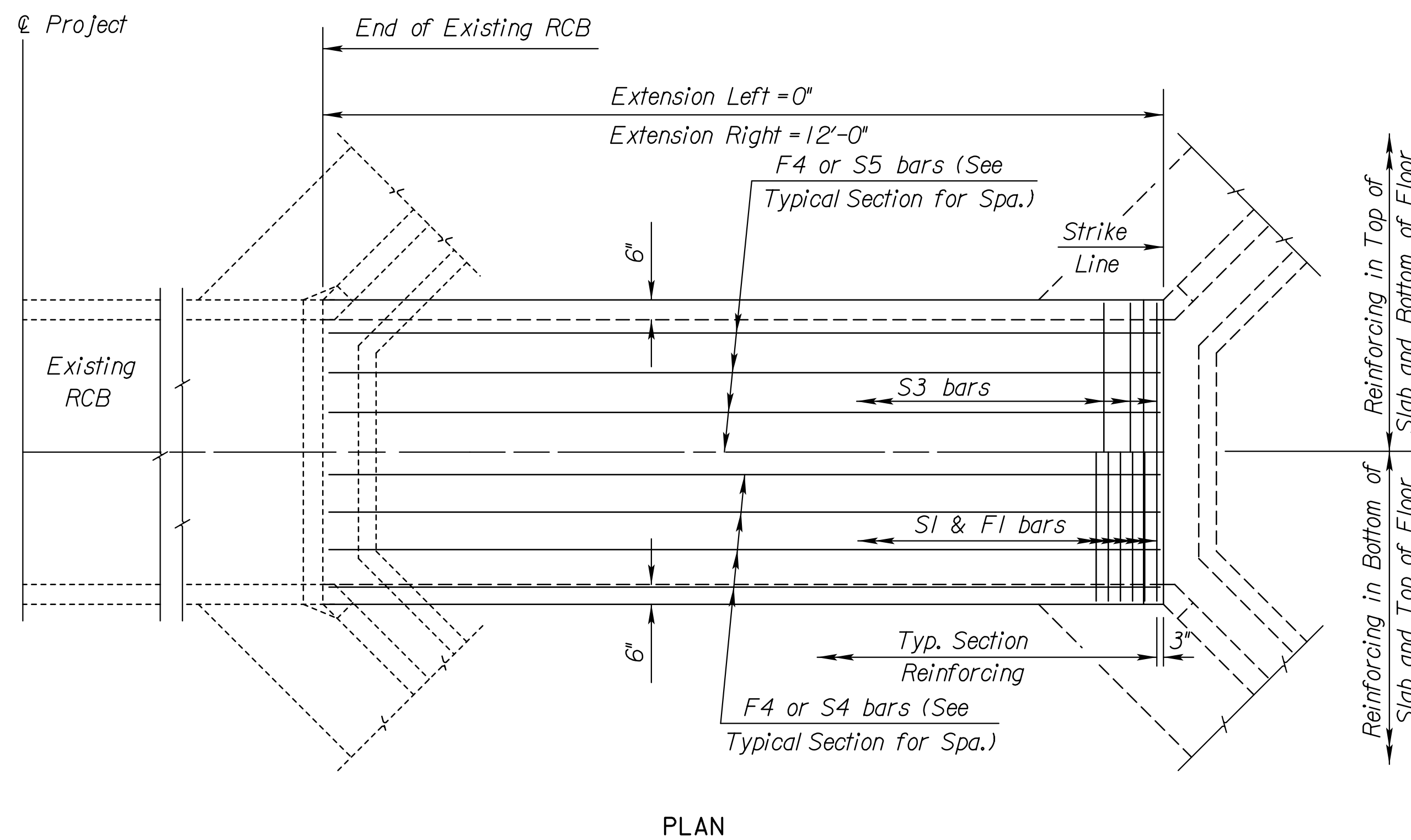
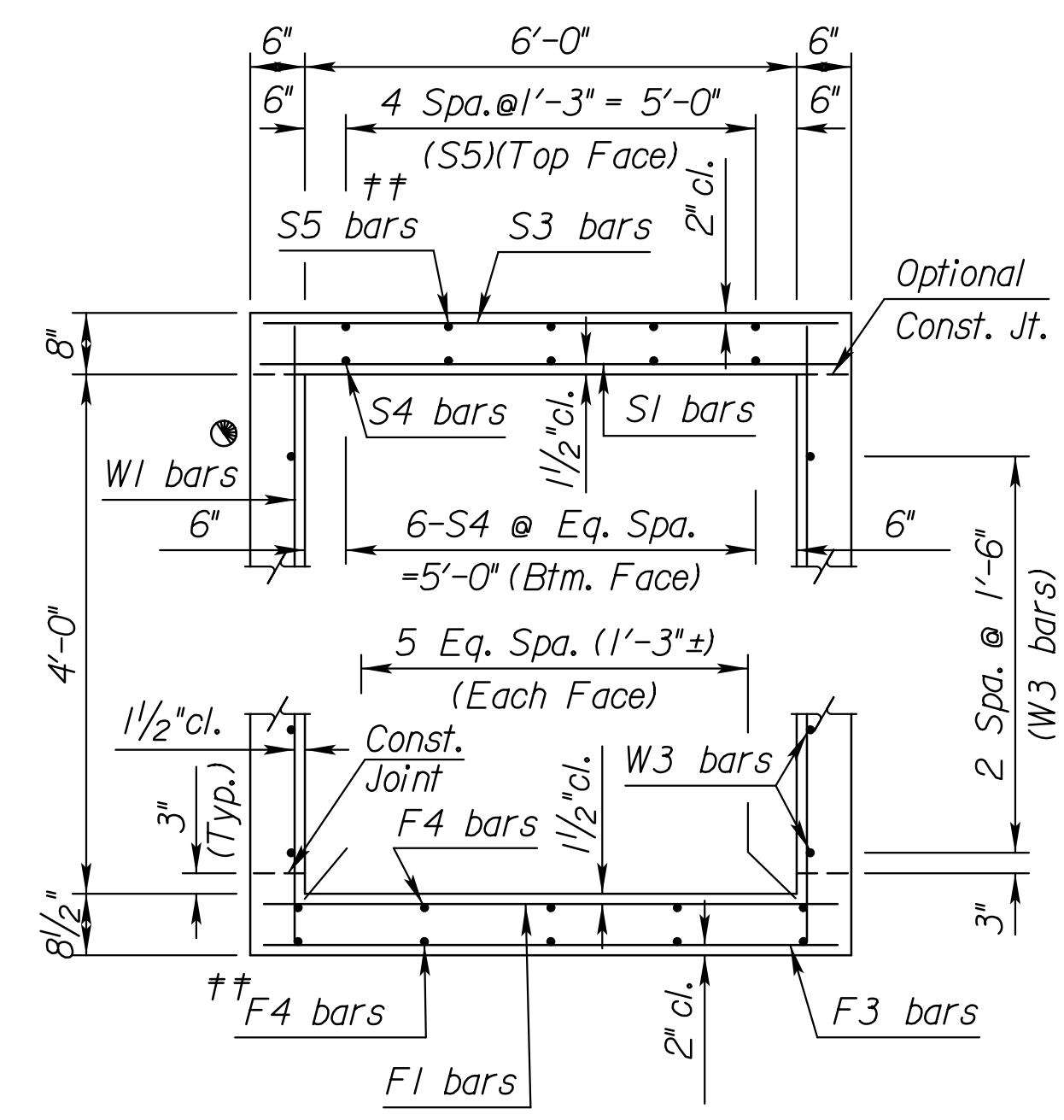
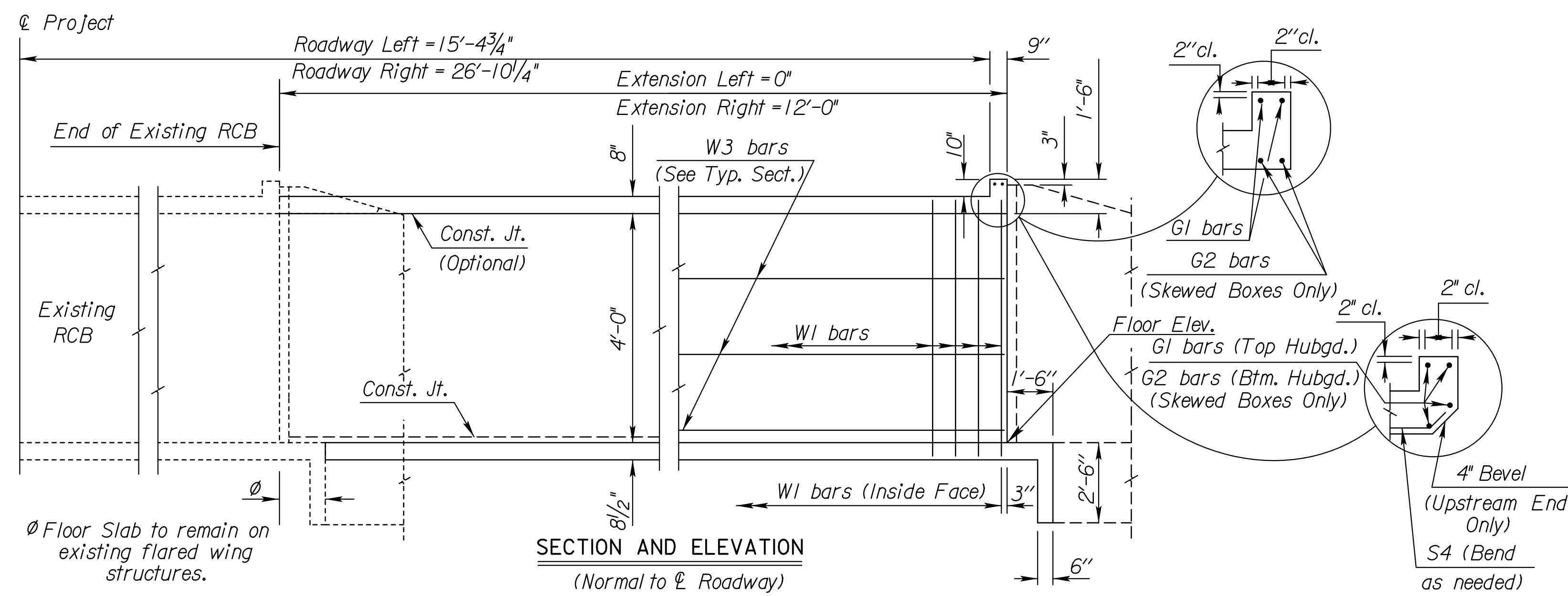
KANSAS DEPARTMENT OF TRANSPORTATION				
SUMMARY of PIPE CULVERTS				
RD659				
FHWA APPROVAL	06-08-22	APPD.	Scott W. King	
DESIGNED	DETAILED	QUANTITIES	TRACED	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

Note to Designer:
 KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVC, PPP, SRPE, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	29	135

VERSION/ID	12/15/2000
CADD YBA	5/4/2020
DATABASE	TJL22
RCB PROGRAM	157
KBOX MODEL ID	
CELL LIBRARY	2/8/2022

06



See RCB Auxiliary Details for Optional Splice.

Note:
S3 bars omitted unless grade box or slab thickness is greater than or equal to 12".

Note:
F3 bars omitted unless floor thickness is greater than or equal to 12".

†† Omit S5 bars when S3 bars are omitted and omit the bottom layer of F4 bars when F3 bars are omitted.

See Standard No. RD 080 for additional details.

Note:
Use only cast-in-place construction at this location.

GENERAL NOTES

DESIGN SPECIFICATION: AASHTO LRFD Spec., 2007 Ed., 2009 Int.
DESIGN LOADING: HL93
UNIT STRESSES: Grade 4.0 Concrete $f'c = 4,000$ p.s.i.
 Reinforcing Steel $f_y = 60,000$ p.s.i.
FILL HEIGHT: Unless otherwise noted, the Design Fill Height is measured from the riding surface at the culvert and includes the surfacing.
CONCRETE: Use concrete conforming to Grade 4.0 Concrete. Bevel all exposed edges with a 3/4" triangular molding. Where Grade 4.0(AE) is specified, place this concrete in the top slab above the Construction Joint.
REINFORCING: Use reinforcing steel conforming to ASTM A615, Grade 60. All dimensions relative to reinforcing steel are to the centerline of the bar unless otherwise noted.
EXCAVATION: Excavation for culverts less than bridge length shall not be paid for directly but shall be subsidiary to Grade 4.0 Concrete. Excavation for RCB bridges shall be paid for as Class III Excavation.
SEAL COURSE: The Engineer may require a seal course. The seal course shall be unreinforced Concrete(Commercial Grade) with a minimum depth of 3 inches or as determined by the Engineer. Concrete for the seal course shall be paid for at the unit price set for Concrete for Seal Course.
FOUNDATION STABILIZATION: The Foundation Stabilization quantity has been calculated to the limits shown on the "RCB Auxiliary Details" sheet. The depth may be increased by the Engineer. The Contractor may under-run Foundation Stabilization under the barrel if founded on firm material and with the Engineer's approval. Use Foundation Stabilization on all wingwalls unless founded on rock or granular material.
QUANTITIES: The quantities shown in the Culvert Summary include apron and/or soil saver quantities when they are required by the plans. Payment for additional quantities that result from including a seal course and/or a floating apron, as a change in the original plans, shall be made at the unit price bid for the various items involved.
GRANULAR BACKFILL (WINGWALLS):
 See the "Auxiliary Details" sheet.
STRIKE LINE: Construct the wingwalls and that portion of the RCB outside the Strike Line level. Construct the wingwall footings with the culvert floor. See the wingwall detail sheets.
BRIDGE BACKWALL PROTECTION SYSTEM: For structures with this bid item in the Summary of Quantities. See the "Auxiliary Details" sheet.

Δ Epoxy Coated Bars

Minimum Splice Lengths	
#4	1'-5"
#5	1'-9"

CULVERT SUMMARY											LRFR RATING FACTORS			
For design purposes ONLY. Do NOT use for Construction											includes any welded wire fabric			
Floor Elev.	Crown Gr. Elev.	Design Fill Ht.	Skew	Wings	Scour Apron	Soil Saver	Concrete			Reinf. Steel (Gr. 60)			HL-93 Loading	
							Barrel (Cu.Yds.)	Wings (Cu.Yds.)	Total (Cu.Yds.)	Barrel (Lbs.)	Wings (Lbs.)	Total (Lbs.)	Inventory	Operating
Ext.Lt. 1301.44	1308.40	2	0	None Flared	Yes	No	0.00	0.00	0.00	0	0	0	1.16	1.50
Ext.Rt. 1301.13							6.22	6.38	12.60	748	677	1425		

BAR SCHEDULE

		Δ F1				Δ F3				Δ F4				Δ S1				Δ S3				Δ S4				Δ S5							
		Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length
Ext.Lt.	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Ext.Rt.	6	7"	23	6'-8"	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	6'-8"	4	6	11'-8"	6	7"	23	6'-8"	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

		Δ K1				Δ K2				Δ W1				Δ W3				Δ G1				Δ G2											
		Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length	Size	Spa.	No.	Length
Ext.Lt.	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Ext.Rt.	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	4	9"	32	5'-0"	N/A	N/A	N/A	N/A	4	6	11'-8"	N/A	N/A	N/A	N/A	N/A	5	2	6'-8"	N/A	N/A	N/A	N/A	N/A

SUMMARY OF QUANTITIES	
Concrete (Grade 4.0)	10.4 C.Y.
Concrete (Grade 4.0(AE))	2.2 C.Y.
Bridge Backwall Protection System	11 S.Y.
Reinforcing Steel (Gr. 60)	680 Lbs.
Reinforcing Steel (Gr. 60)(Epoxy Coated)	750 Lbs.
Class III Excavation	C.Y.
Foundation Stabilization	6 C.Y.
Concrete for Seal Course (Set)	1 C.Y.
Granular Backfill (Wingwalls)	12 C.Y.

NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Sta. 7384+11.04				
SINGLE 6 ft x 4 ft RCB 12.0 ft EXT. RT.				
BR 1.6.4 P		Wabaunsee Co.		
FHWA APPROVAL		10-20-10 APP'D		Terry L. Fleck
DESIGNED	DETAILED	QUANTITIES	CADD	
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.	

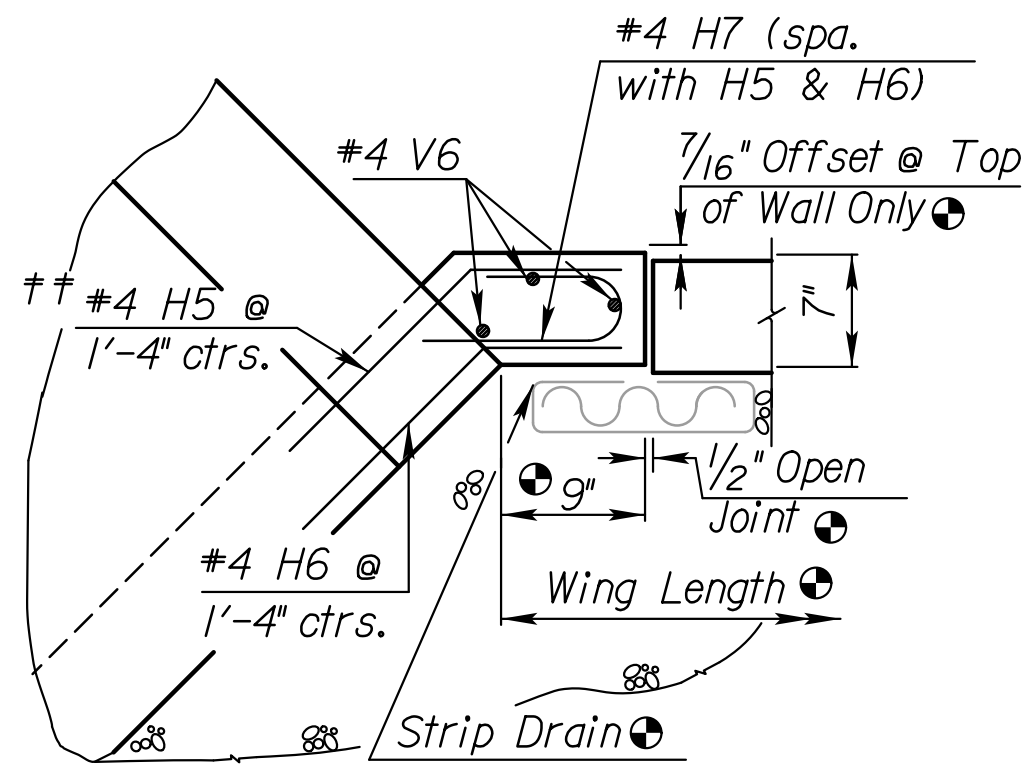
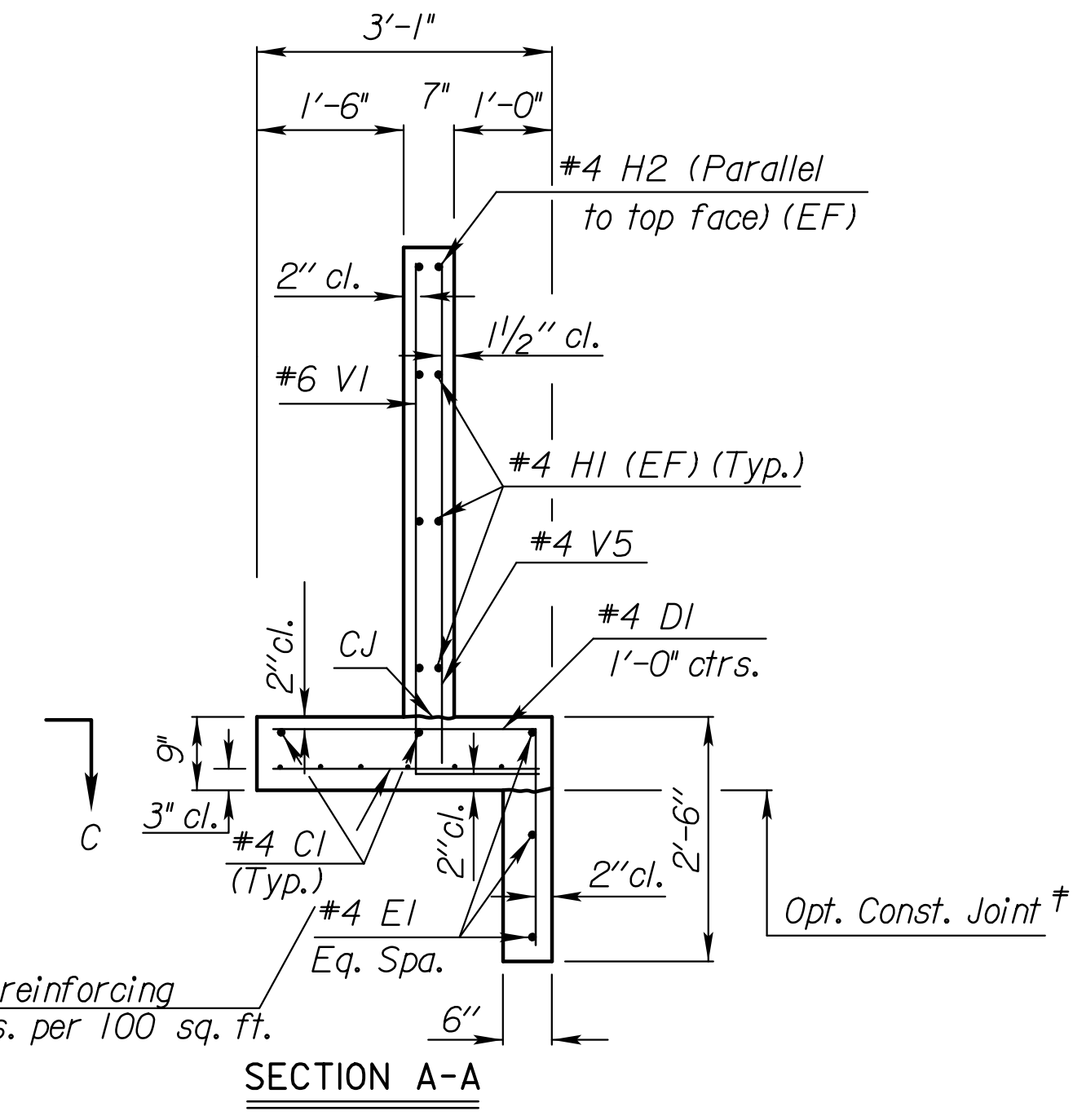
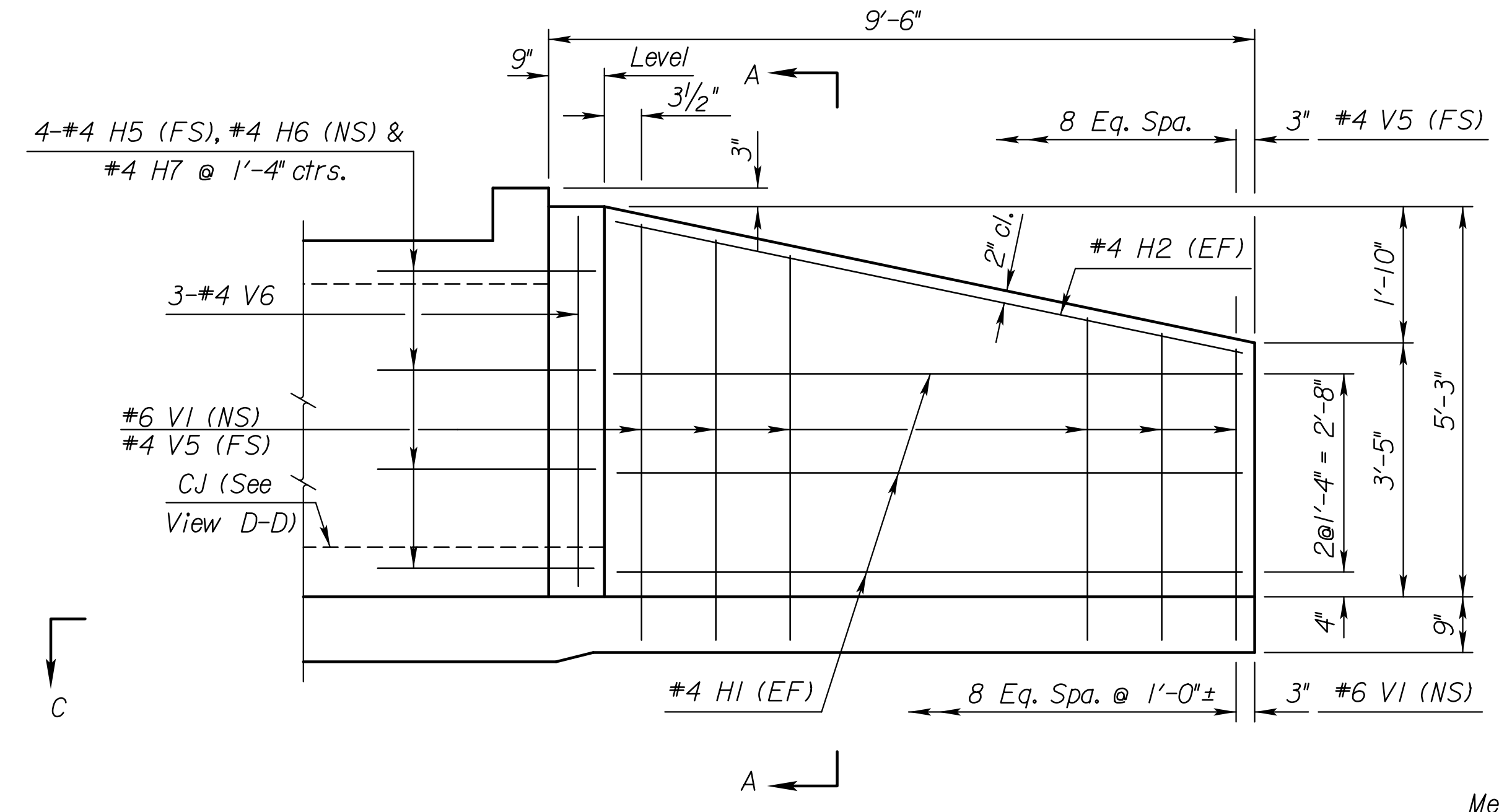
Plotted By: August Zuno
 File: KA-5728-01_RCBBox/AZ.dgn
 Plot Date: 5-JAN-2024 10:20
 Plot Location: Road

KDOT Graphics Certified

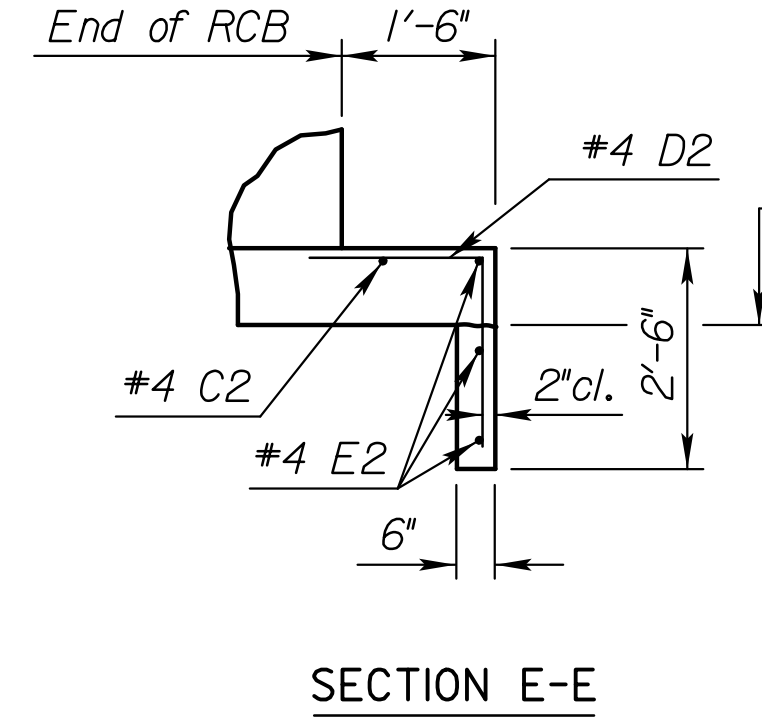
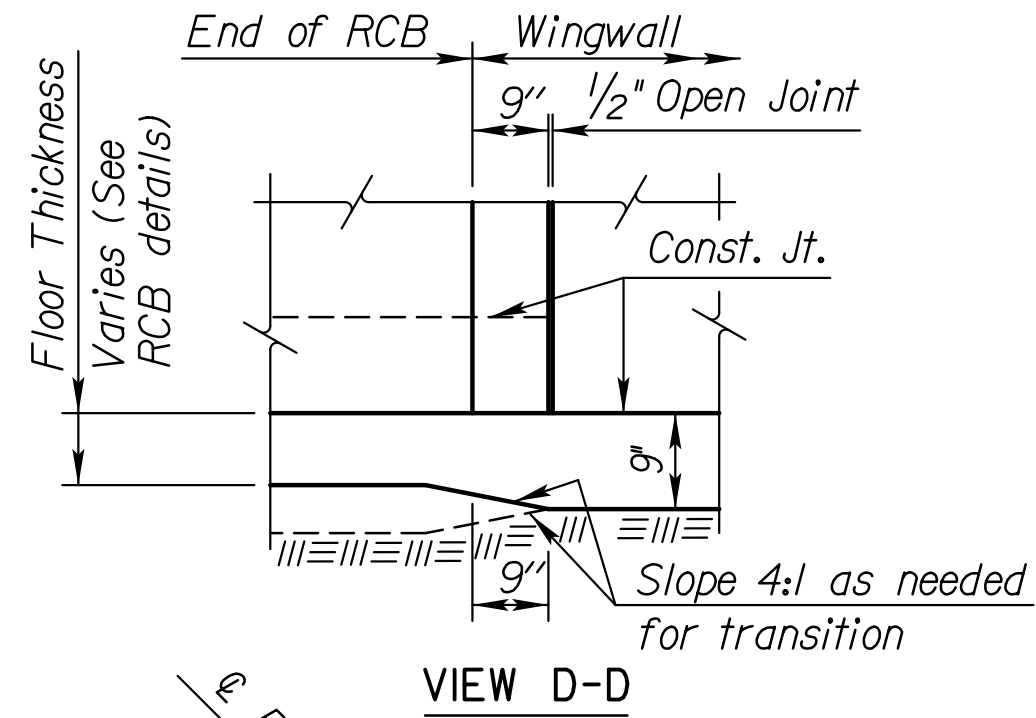
GENERAL NOTES

DESIGN SPECIFICATION: AASHTO LRFD Spec., 2007 Ed., 2009 Int.
DESIGN LOADING: HL93
UNIT STRESSES: Grade 4.0 Concrete; $f'_c = 4,000$ p.s.i.
 Reinforcing Steel; $f_y = 60,000$ p.s.i.
CONCRETE: Grade 4.0 Concrete shall be used throughout. Bevel all exposed edges with a $\frac{3}{4}$ " triangular mauling.
REINFORCING: All reinforcing shall conform to ASTM A615, Grade 60. Welded Wire Reinforcement shall conform to ASTM A185. All dimensions relative to reinforcing steel shall be to centerline of bar unless otherwise noted. Welded Wire Reinforcement shall be classified as pounds of reinforcing and included in the total quantity for the bid item Reinforcing Steel (Gr. 60).
QUANTITIES: Wingwall Quantities include all quantities outside the neat lines of the box, excluding the hubguard.
APRON: A 5" concrete slab shall be constructed between the downstream wings in locations subject to scour only when specified on the plans or by the Engineer.
BACKFILL MATERIAL: Use Granular Backfill material meeting the requirements of SB-1, SB-2, SCA-1, SCA-2. Backfill all wings to limits shown on the "RCB Auxiliary Sheet".
FILTER FABRIC: Separate in-situ material from granular backfill with approved filter fabric complying with Section 1710. Filter Fabric is subsidiary to "Granular Backfill".
FOUNDATION STABILIZATION: Use Foundation Stabilization on all wingwalls unless founded on rock or granular material.

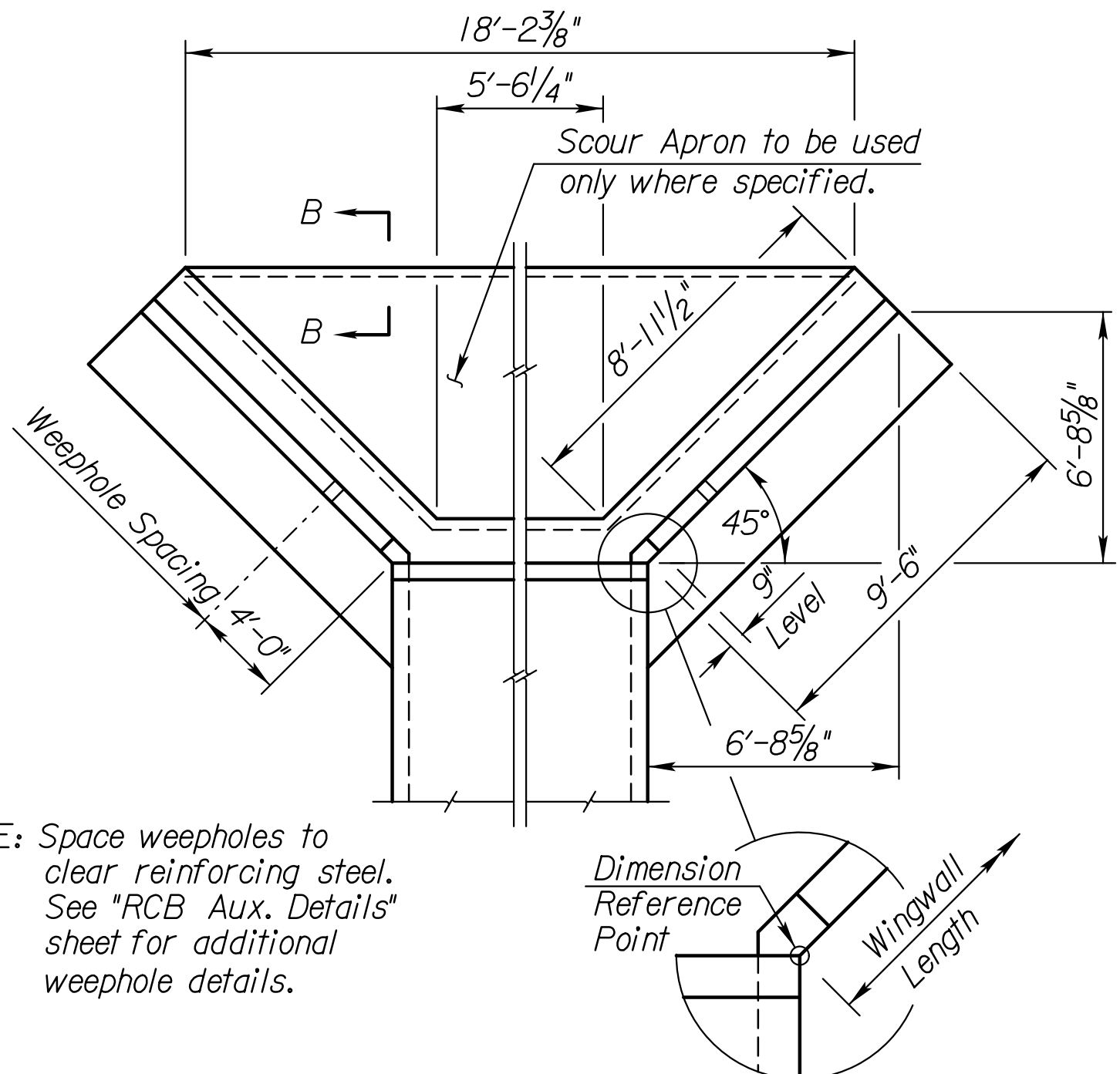
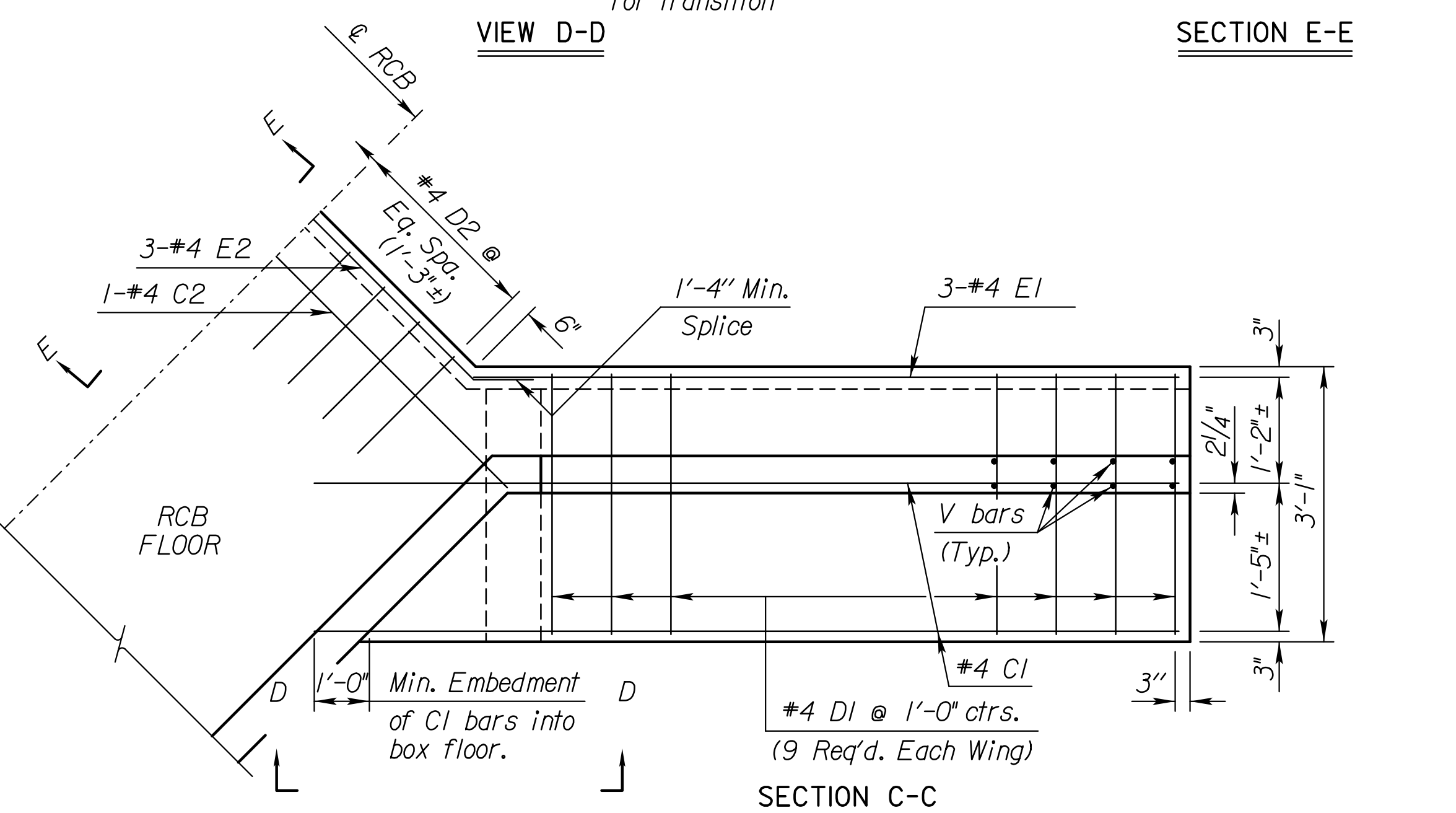
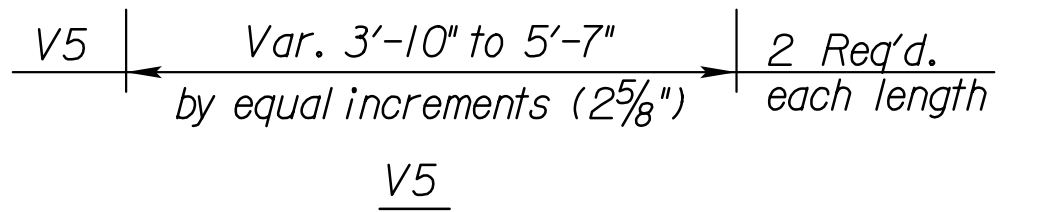
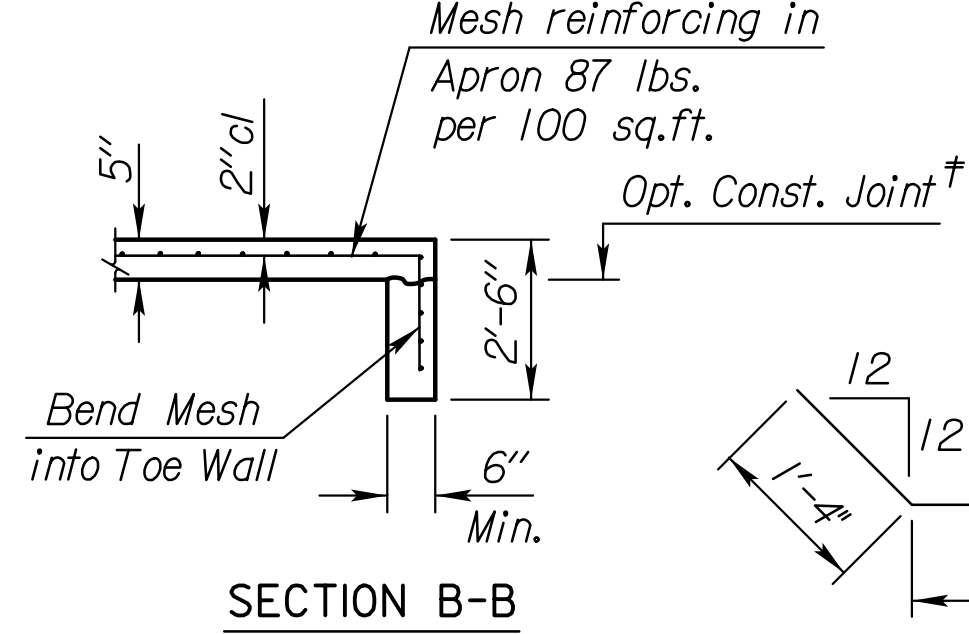
Typical both wings



† NOTE: Const. Jt. may be used at Contractor's option when approved by the Engineer. DI bars or mesh may be spliced thus: Minimum overlap shall be 1'-3". No increase in quantities or cost shall be allowed when Contractor elects this option.



NOTE:
 EF = Each Face
 NS = Near Side
 FS = Far Side
 CJ = Const. Joint



NOTE: Space weepholes to clear reinforcing steel. See "RCB Aux. Details" sheet for additional weephole details.

Quantities listed below are included in the Summary of Quantities shown on the RCB details.

	WINGWALL QUANTITIES (One End Only)	
	Foundation Stabilization	Concrete (Gr. 4.0)
Wingwalls	1.80 (C.Y.)	4.54 (C.Y.)
Apron	1.23 (C.Y.)	1.84 (C.Y.)
Soil Saver	0.00 (C.Y.)	0.00 (C.Y.)
Reinforcing Steel (Gr. 60)		521 Lbs.
Welded Wire Fabric (Wings)		51 Lbs.
Welded Wire Fabric (Apron)		105 Lbs.
Granular Backfill (Wingwalls)		12.00 C.Y.
Filter Fabric (subsidiary)		18.00 S.Y.

BENDING DIAGRAM
(All dimensions are out to out of bars.)
 †† Bend in Field

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
 Sta. 7384+11.04
FLARED WINGWALLS
 4 ft Rise (0*SKEW)

BR 10.00.04 Wabaunsee Co.
 FHWA APPROVAL 10-20-10 APP'D Terry L. Fleck
 DESIGNED DETAILED QUANTITIES CADD
 DESIGN CK. DETAIL CK. QUAN. CK. CADD CK.

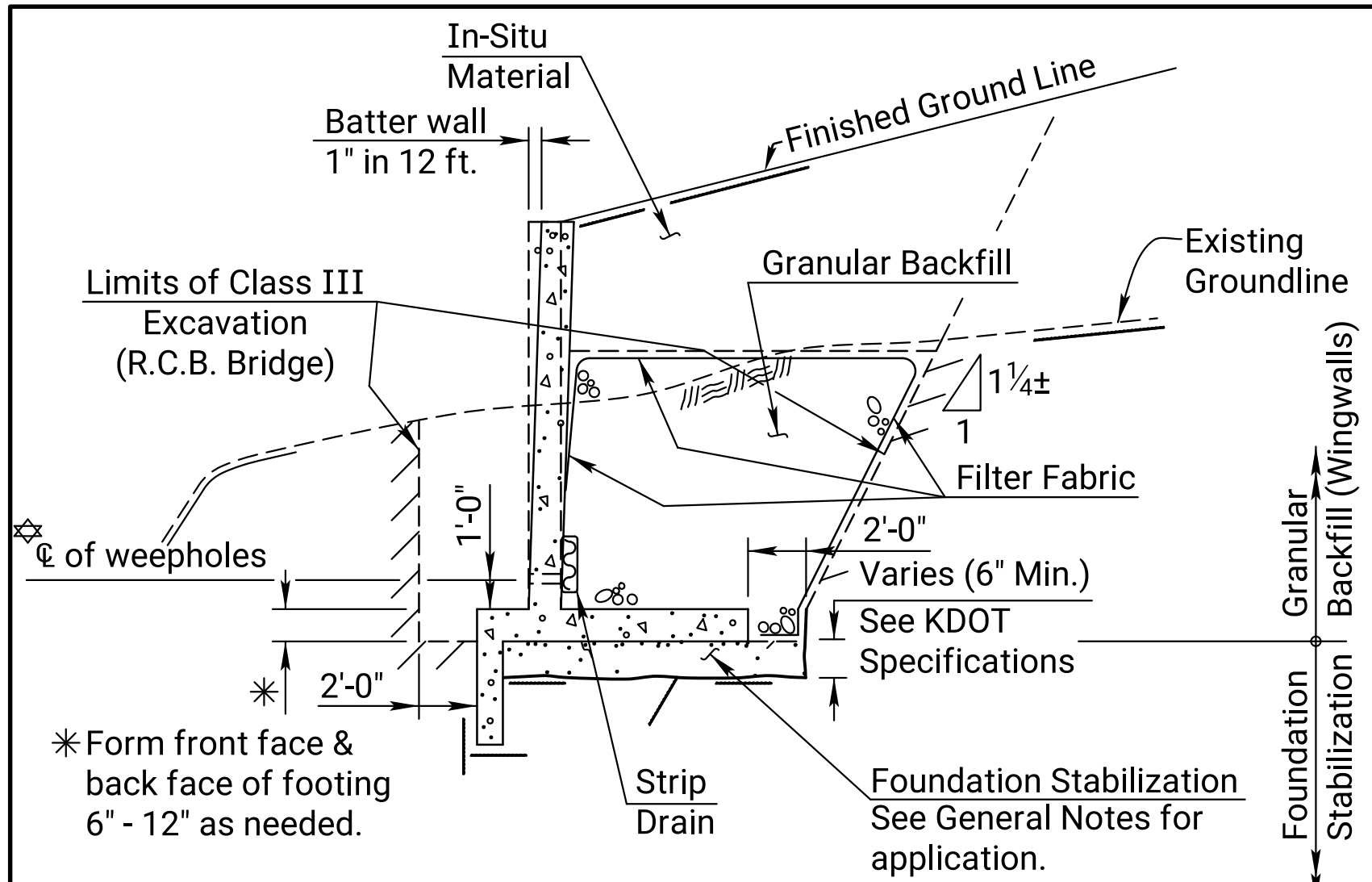
NOTE: Reinforcing Bar List is for both wings at one end of box only.

0° Skew	# See Bending Diagram														
	No.	#4C1	#4D1	#4E1	#4C2	#4D2	#4E2	#6V1	#4H1	#4H2	#4H5	#4H6	#4H7	#4V5	#4V6
Length	11'-7"	4'-11"	8'-9"	6'-8"	5'-2"	8'-6"	*	8'-4"	8'-6"	1'-10"	2'-1"	1'-9"	*	5'-0"	

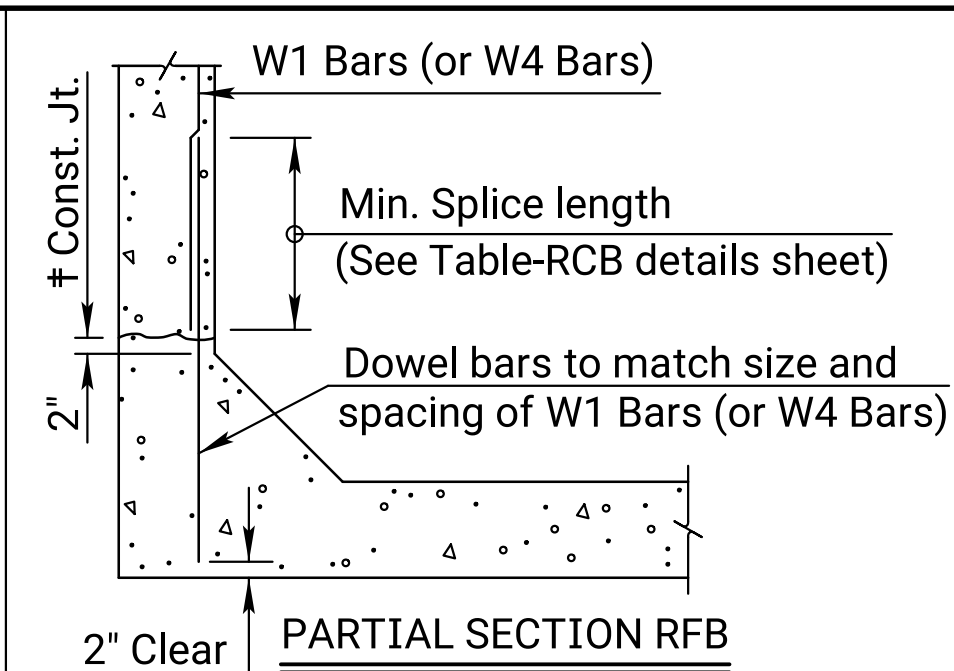
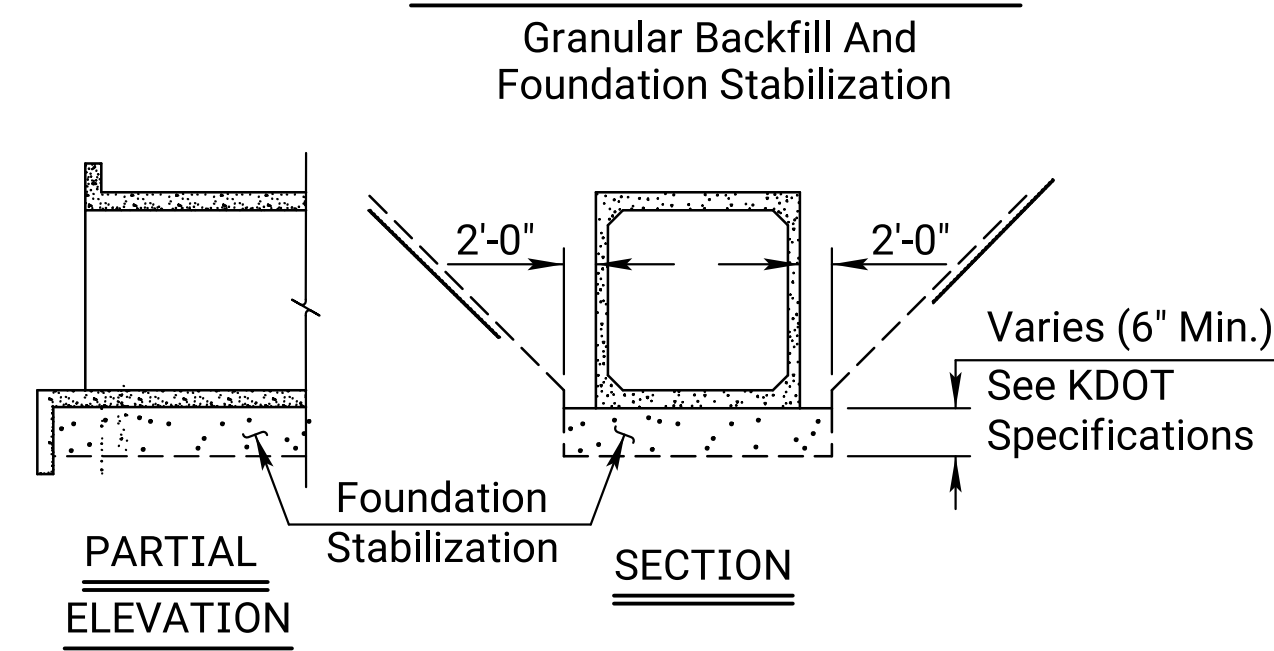
Plotted By: August Zuno Plot Location: Road
 File: KA-5728-01_RCBWing\JAZ.dgn
 Plot Date: 8-JAN-2024 08:18

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	31	135



SECTION A-A THRU WINGWALL

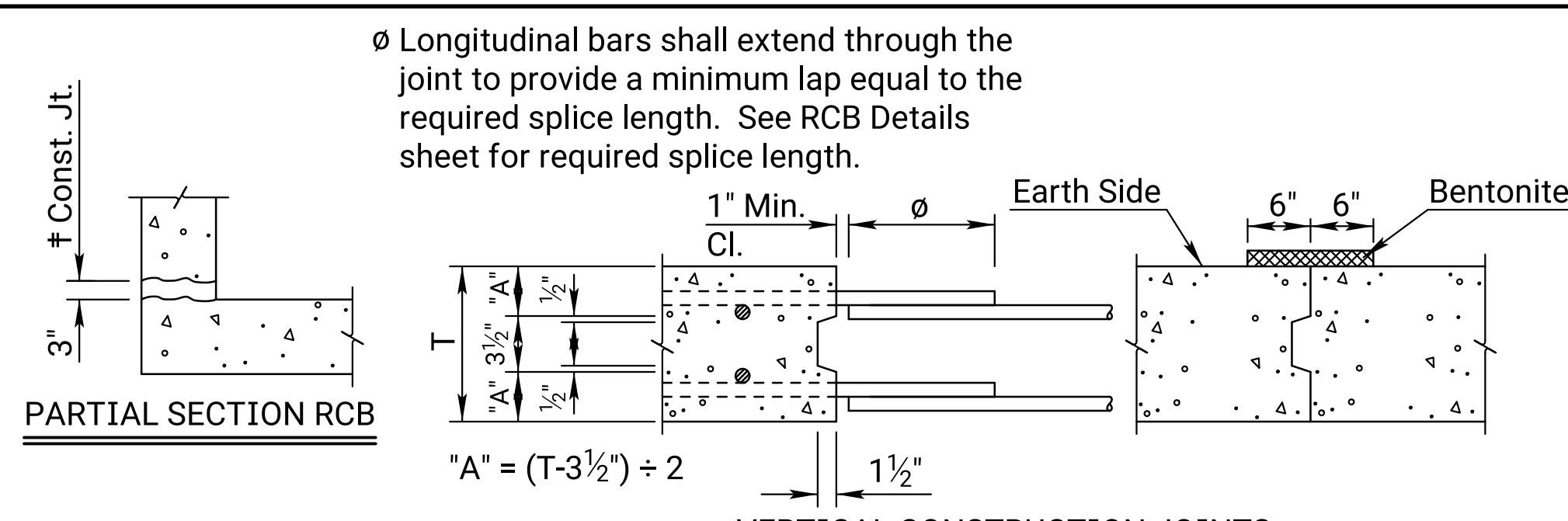


OPTIONAL BAR DETAIL

The Contractor shall have the option of using Dowel Bars to match vertical wall bars as shown, however no allowance will be made for additional steel required for bar laps.

OPTIONAL COLD JOINT

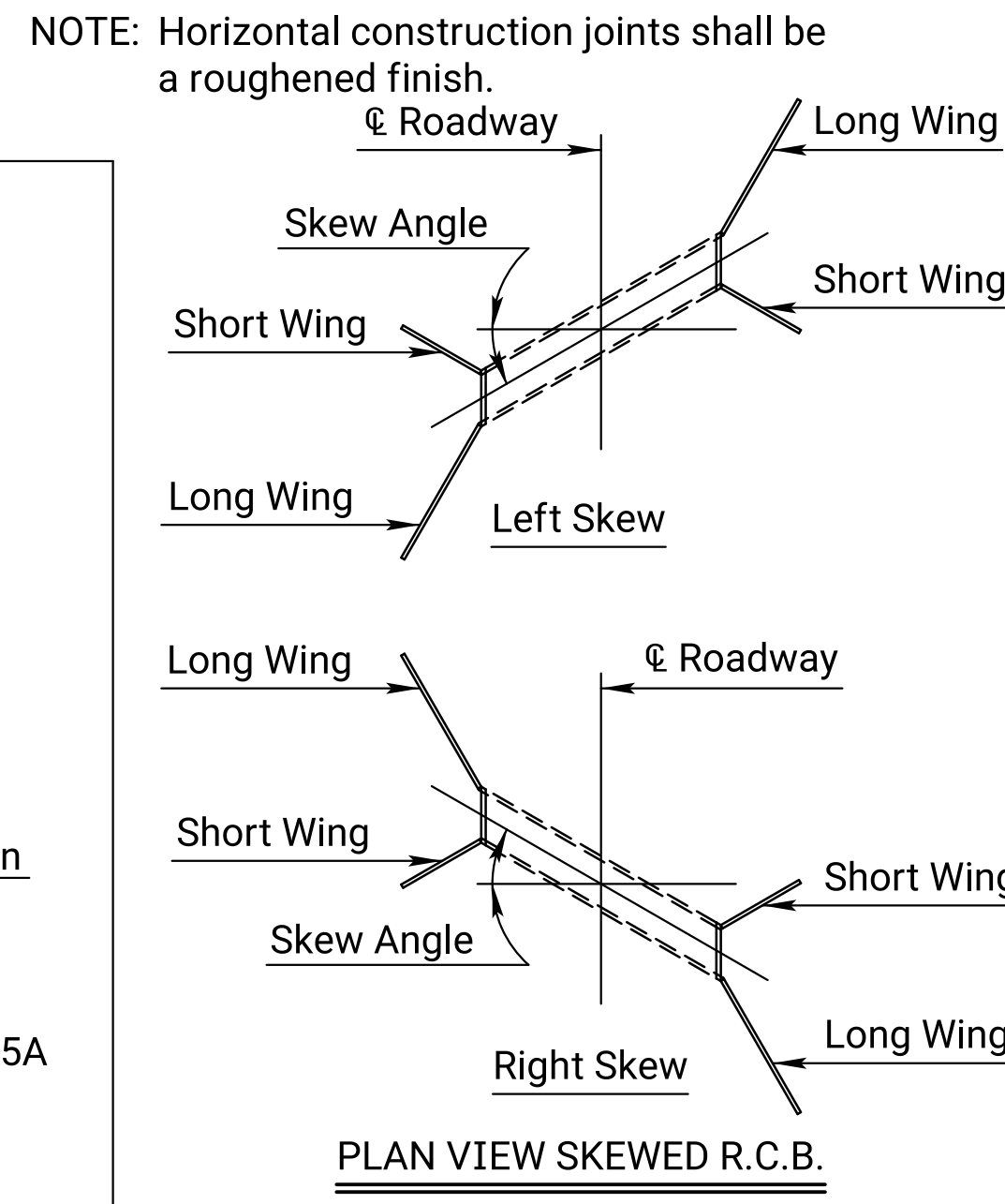
The Contractor has the option of placing the Lower Horizontal Construction Joint at the top of the slab for RCB's or at the top of the fillet for RFB's.



VERTICAL CONSTRUCTION JOINTS

NOTE: Vertical construction joints shall be perpendicular to the longitudinal axis of the RCB and shall be placed at any location as needed for construction and as approved by the Engineer.

NOTE: Barrel Construction Joints located in a median with less than 5 ft. of fill or at locations specified by the Engineer, shall be protected by a bentonite based system as shown. Place the bentonite on the exterior walls and top slab. See requirements for bentonite in the KDOT Specifications for "Bridge Backwall Protection System". Material and installation of the bentonite system shall be subsidiary to the bid item "Grade 4.0 Concrete".



PLAN VIEW SKEWED R.C.B.

GENERAL NOTES

Foundation Stabilization:
The depth of Foundation Stabilization may be increased by the Engineer. The Contractor may under-run Foundation Stabilization under the barrel if founded on firm material and with the Engineer's approval. Use Foundation Stabilization on all wingwalls unless founded on rock or granular material.

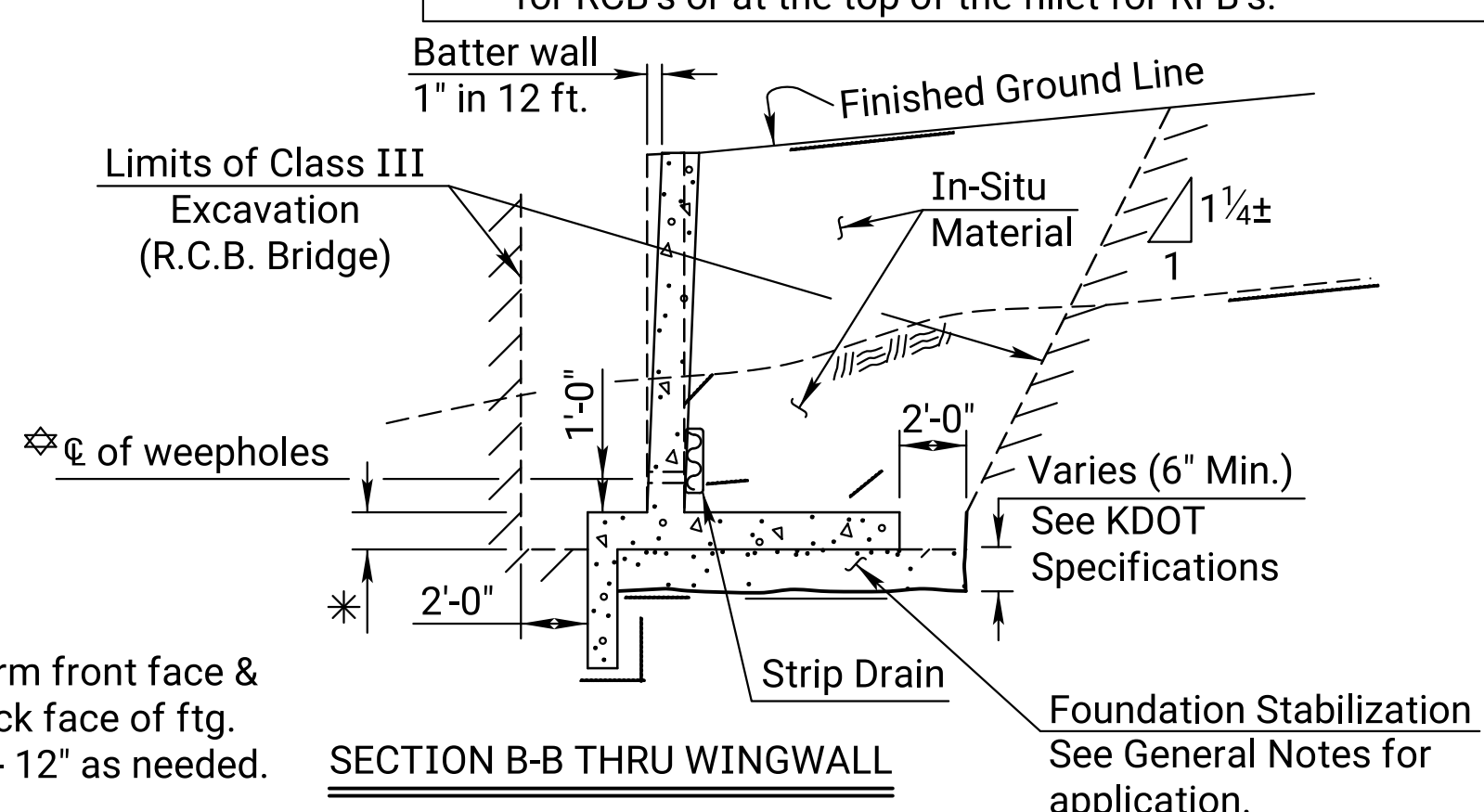
Granular Backfill (Wingwalls):
1. Granular Backfill shall be used to backfill behind wingwalls to the limits shown in the WINGWALL PLAN and Elevation detail. Granular Backfill construction may be used separately or combined with Foundation Stabilization as directed by the Engineer.
2. Measurement for the bid item, "Granular Backfill (Wingwalls)", shall be measured in Cubic Yards to the theoretical limits as shown.
3. Material for Granular Backfill (Wingwalls) shall conform to the requirements of SB-1, SB-2, SCA-1 or SCA-2.
4. Consolidate Granular Backfill using hand equipment only. Avoid over consolidation.
5. Use filter fabric complying with Section 1710. Use only within the limits of Granular Backfill to separate from the In-Situ Material. Filter Fabric is subsidiary to "Granular Backfill".

In-Situ Backfill (Wingwalls):
1. Use any material found within the project limits except Highly Plastic Clay(s) or organic material. The material is subsidiary to "Granular Backfill".
2. Use Type "B" Compaction.
3. Use only hand or walk behind equipment for Compaction.

Wingwall Drainage:
1. All wingwalls with weepholes shall have horizontal and vertical wingwall drainage as shown. Strip drains will be used. See KDOT Specifications for "Abutment Strip Drains" for strip drain requirements.
2. Construction and materials for wingwall drainage, including weepholes, and strip drain shall be subsidiary to the bid item, "Grade 4.0 Concrete". Weepholes may be a formed opening or corrugated polyethylene tubing.

Wingwall Foundation Stabilization:
The Foundation Stabilization quantity has been calculated to the limits shown in the Section Thru Wingwall. The depth may be increased by the Engineer. Use Foundation Stabilization on all wingwalls unless founded on rock or granular material.

Seal Course:
Seal Course consisting of 3" min. of Commercial Grade Concrete shall be constructed to the limits directed by the Engineer. No reinforcing in the floor of the slab or wall footing shall be placed until the Seal Course has gained sufficient strength to permit working upon it without injury.



SECTION B-B THRU WINGWALL

* Form front face & back face of ftg. 6" - 12" as needed.

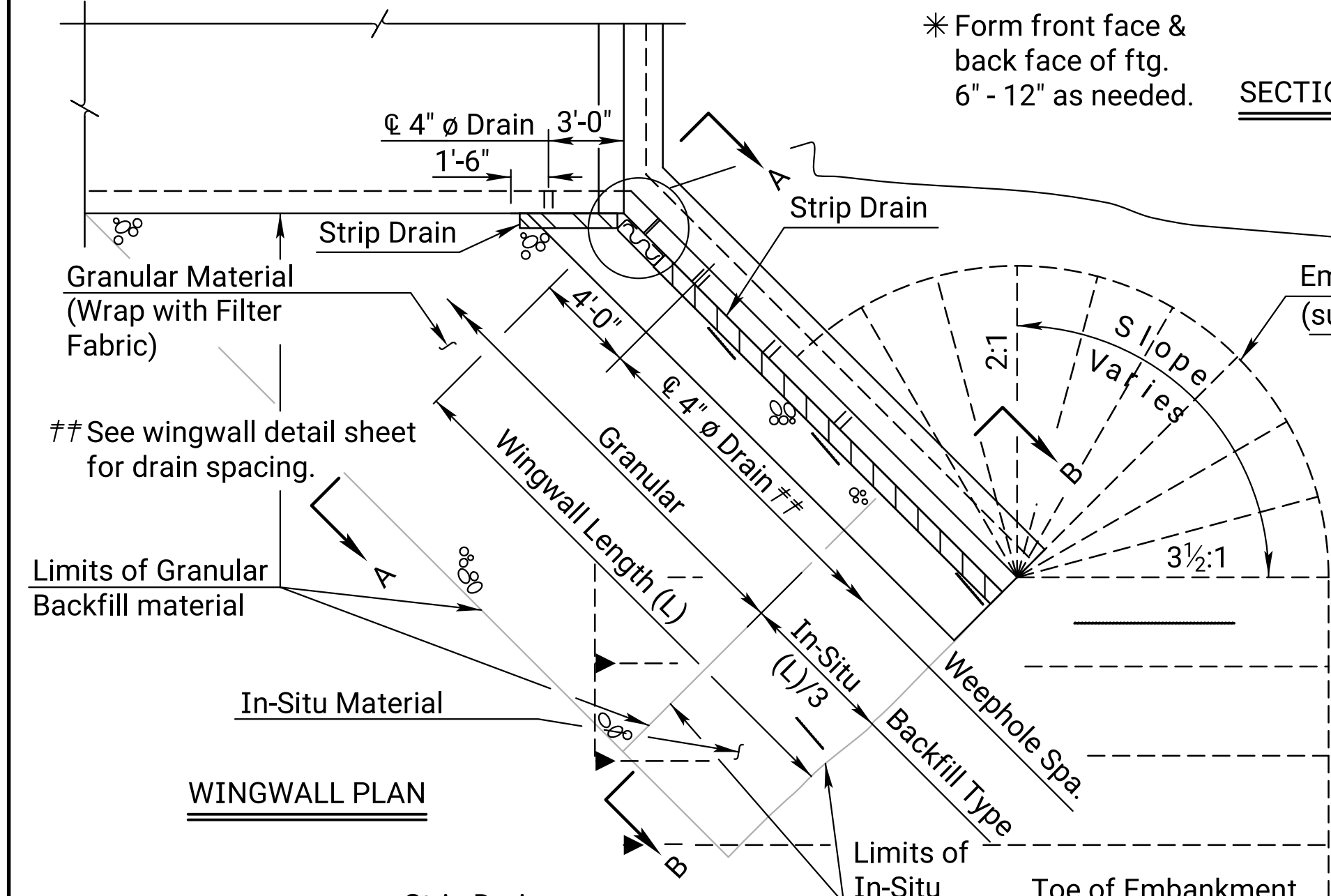
* Weep holes will be adjusted up by the amount of embedment shown on BR025A if the structure is embedded.

Embankment required (subsidiary to Grade 4.0 Conc.)
Extend filter fabric backing and lap a minimum of 4". Wrap all sides and ends.

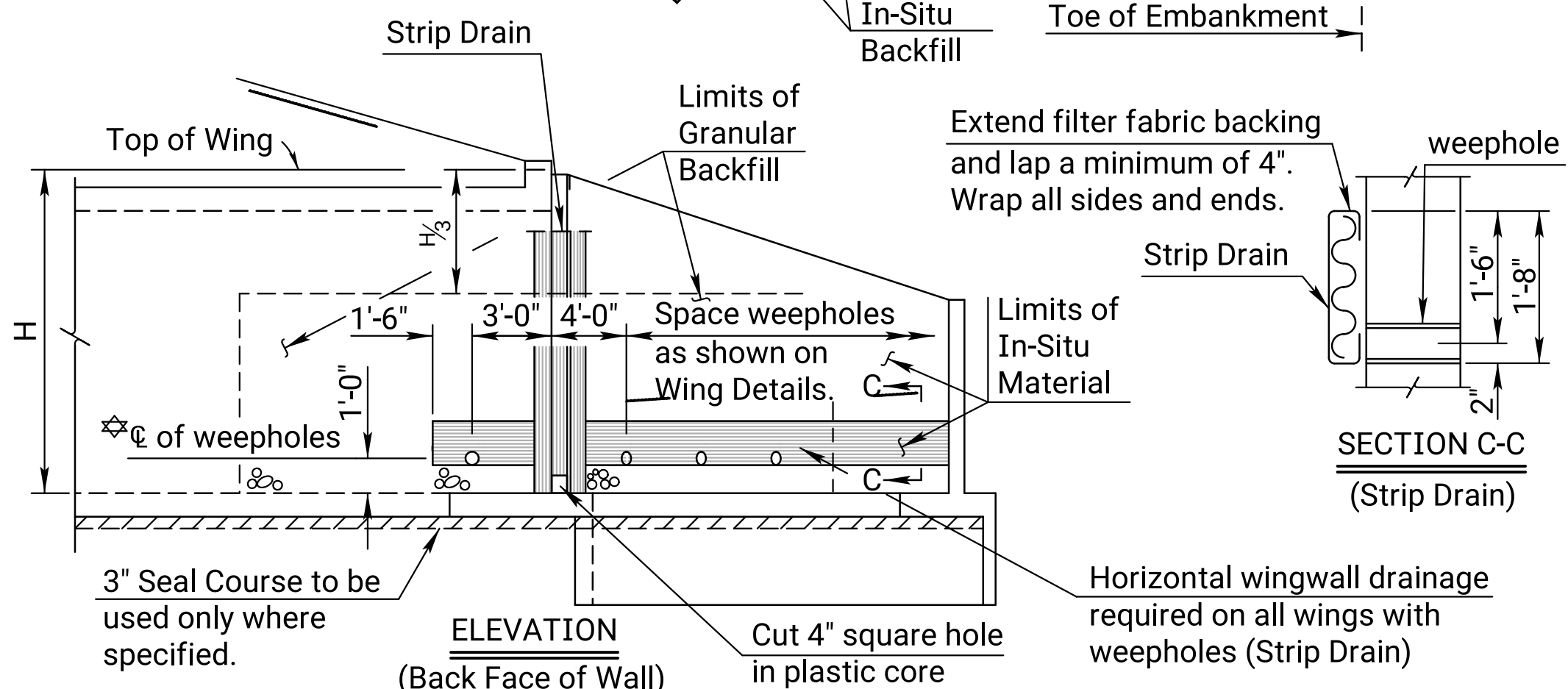
Strip Drain (Extend to top of footing). Cut 4" square hole in plastic core at top of footing)

VERTICAL STRIP DRAIN

GENERAL NOTES



WINGWALL PLAN



SECTION C-C (Strip Drain)

Extend filter fabric backing and lap a minimum of 4". Wrap all sides and ends.

Strip Drain

weephole

Strip Drain

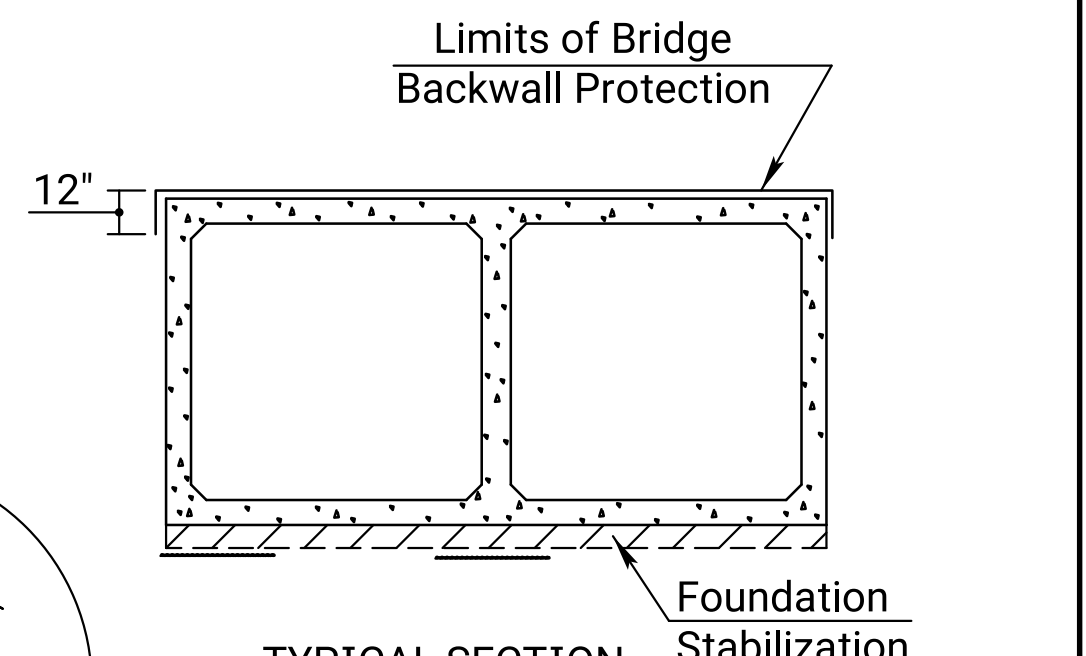
weephole

Strip Drain

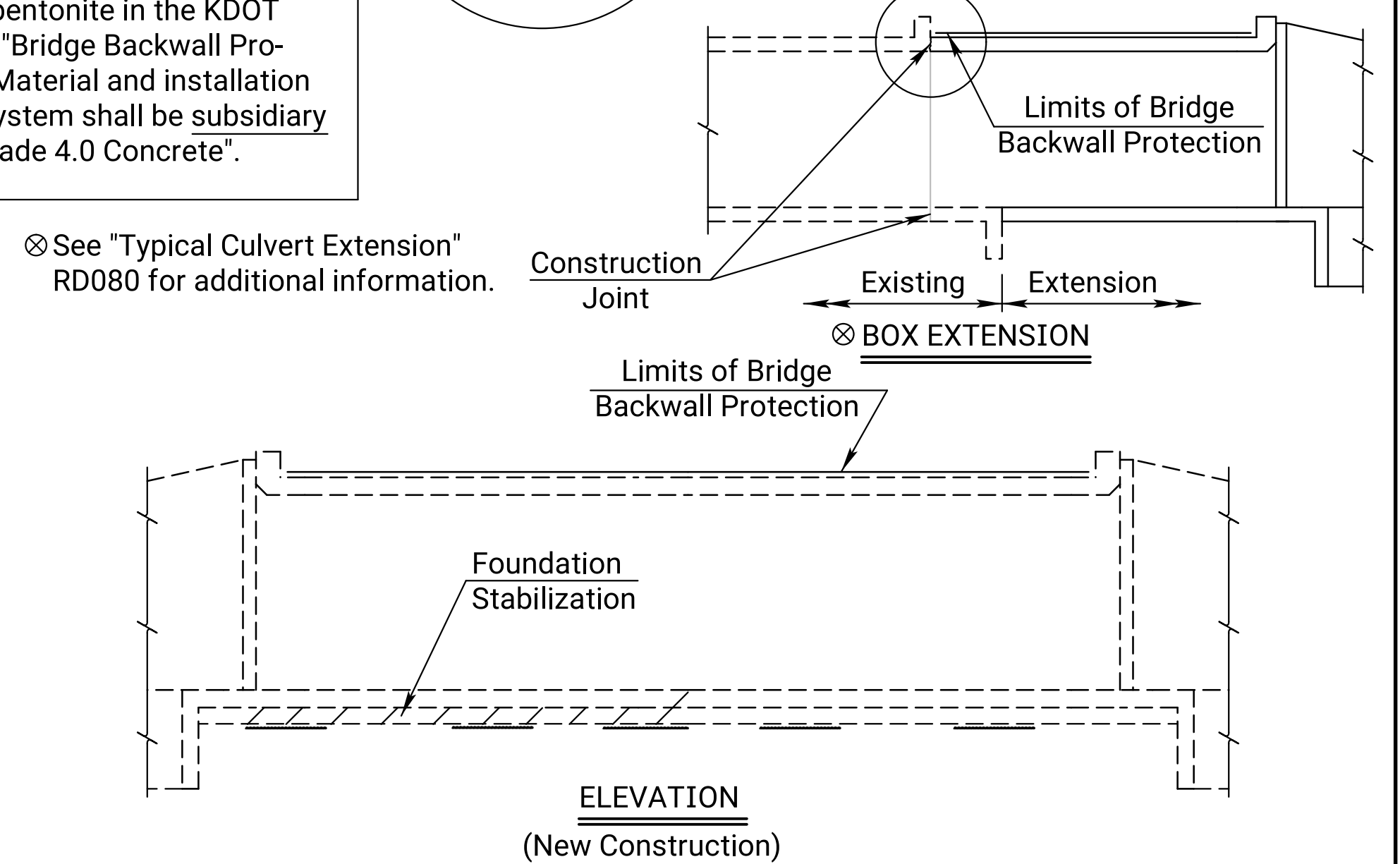
weephole

Strip Drain

weephole



TYPICAL SECTION



ELEVATION (New Construction)

GENERAL NOTES

Bridge Backwall Protection System:
1. For all structures with less than or equal to 2'-0" of fill, apply a "Bridge Backwall Protection System" from Section 1700 to the limits shown. Do not place hot mix asphalt directly on this material.
2. Use a "non-coal tar" material from KDOT's approved list.
3. Protect this material from damage during backfilling. Repair at no additional cost.
4. When the Plans show hot mix asphalt placed directly on the slab, substitute "Pavement Water Proofing" material from Section 800, this material shall be subsidiary. Wait 28 days after top slab is completed before applying this material.

NO.	DATE	REVISIONS	BY	APPD
07	02-06-18	Filter Fabric Clarification	M.L.L.	J.P.J.
06	06-07-17	Filter Fabric Modification	J.P.J.	J.P.J.
05	10-11-13	Granular Backfill Limits	J.P.J.	L.R.R.

KANSAS DEPARTMENT OF TRANSPORTATION

RCB AUXILIARY DETAILS (LRFD)

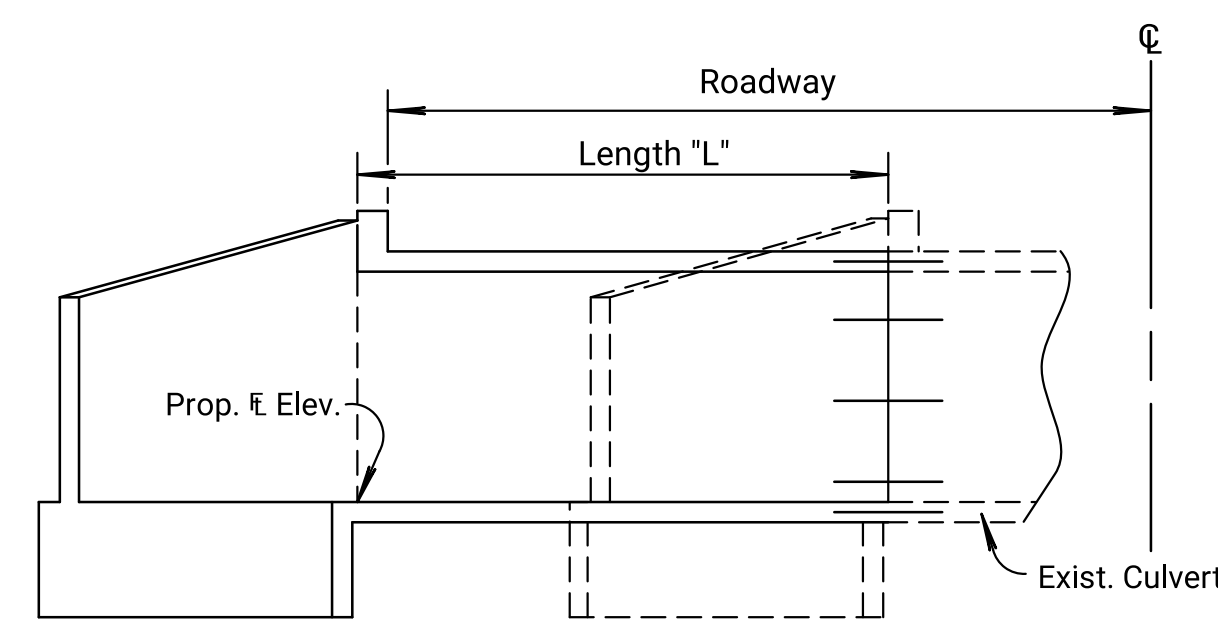
DESIGNED	DETAILD	QUANTITIES	TRACED	R.A.A.
J.P.J.	J.P.J.	J.P.J.	J.P.J.	J.P.J.
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	J.P.J.
J.S.R.	J.P.J.	J.P.J.	J.P.J.	J.P.J.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	31.001	135

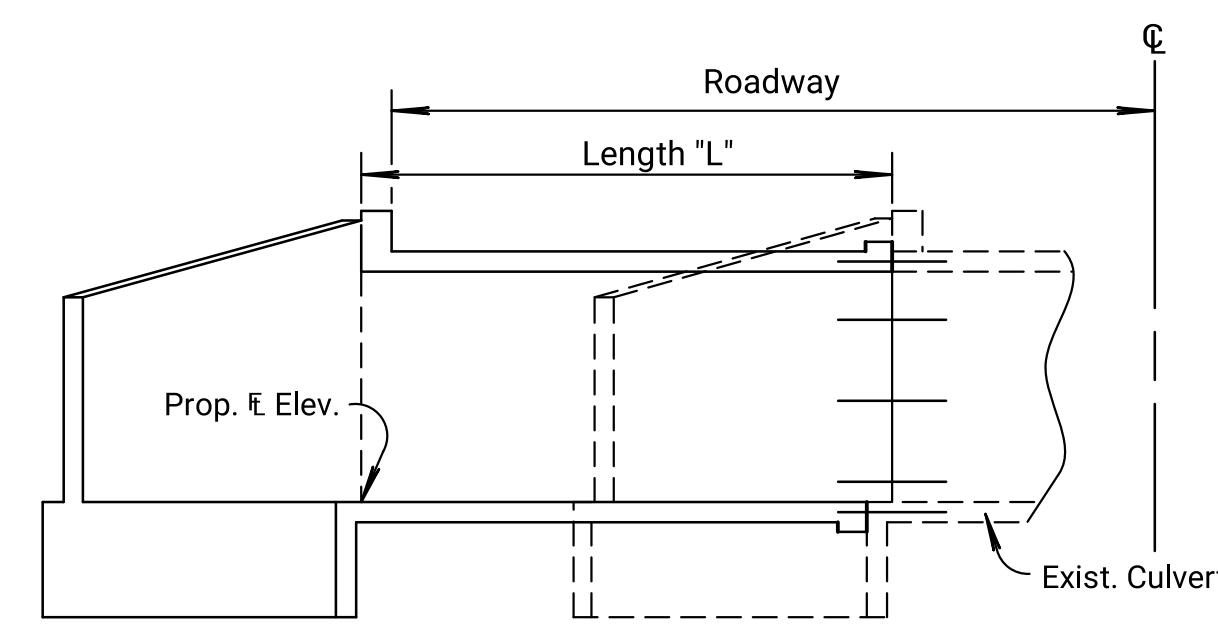
GENERAL NOTE
Dimensions of existing structures shall be checked in the field prior to starting the new construction. Interior walls of multiple box extensions need not be the same thickness as the existing walls.
All existing concrete surfaces adjacent to new concrete shall be thoroughly cleaned by brushing, and soaked with water immediately prior to placing the new concrete.
All work and material necessary for installing the dowel bars shall be subsidiary to the bid item "Reinforcing Steel".
Grouting of bars shall meet the Standard Specifications of the Kansas Department of Transportation. Locate dowel bars near the center of walls and slabs.
For non-skewed boxes "s₁" and "f₁" bars shall be placed at 1/2 normal spacing for the first 18".

Straight Wing Extensions
Remove existing wings and wing aprons. Remove top of hubguard if necessary to clear new construction. A minimum of 24" length of the existing wing and floor steel shall be left intact and shall be cleaned and straightened to bond into the new concrete. Dowels (#5 deformed bars) shall be inserted across the top of box as shown in sketch. Butt extensions against existing culvert. This work shall be subsidiary to the bid item "Grade 4.0 Concrete".

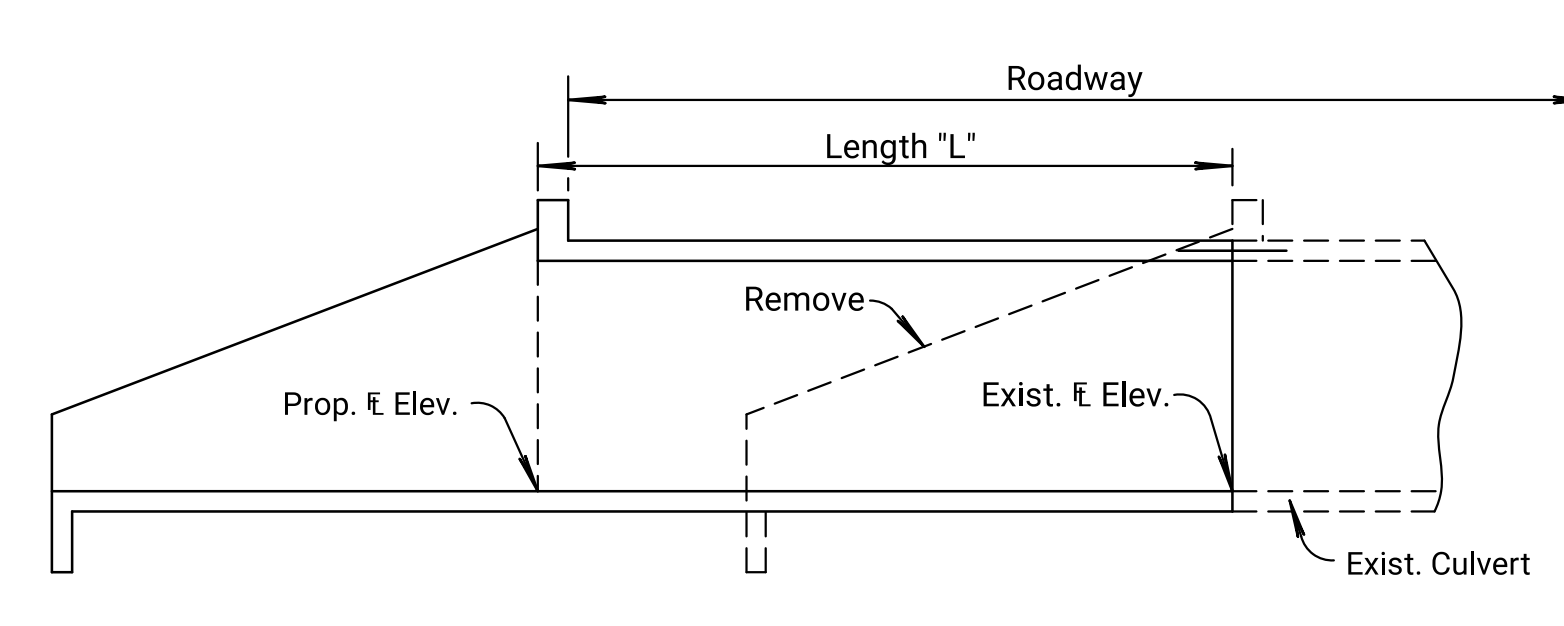
Flared Wing Extensions
Remove top of hubguard, if necessary, to clear new construction. Dowels (#5 deformed bars) shall be inserted in top, bottom, sides and intermediate walls, as shown in sketch. Butt extensions against existing culvert.
If the existing wingwall has an open vertical joint that interferes with the installation of the dowels, remove part of the wingwall to clear construction. This work shall be subsidiary to the bid item "Grade 4.0 Concrete".



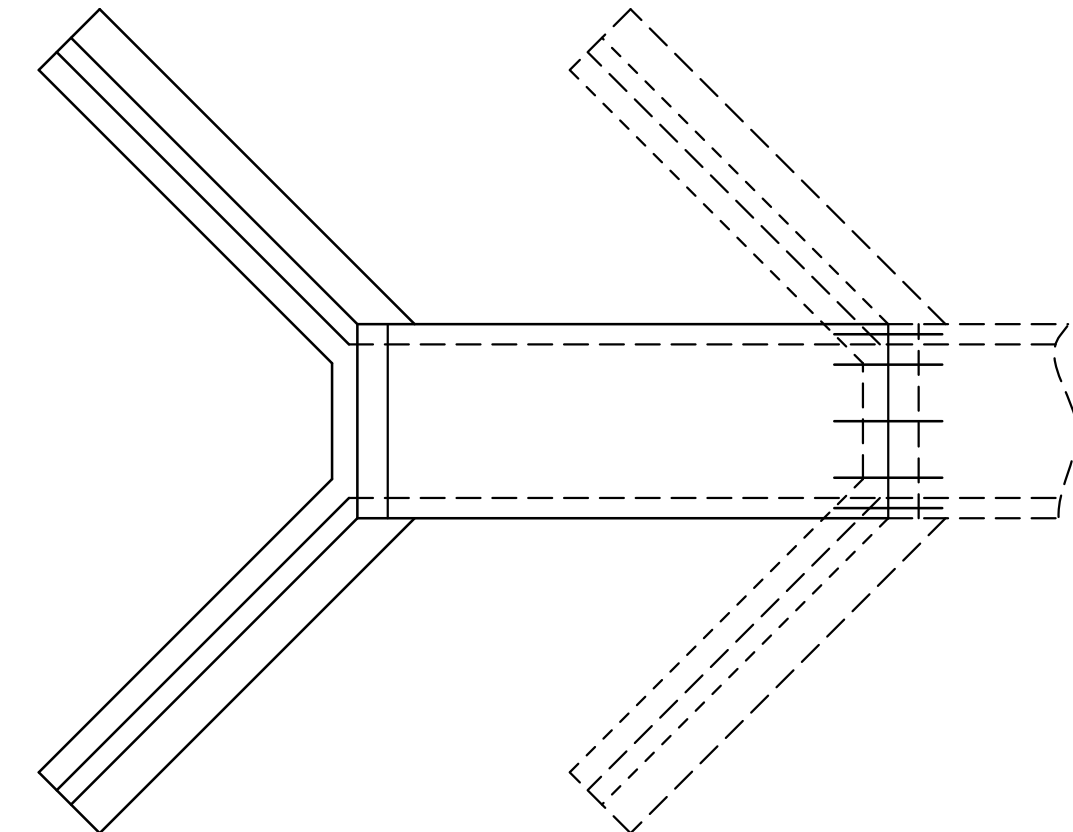
TYPICAL SECTION
(Flared Wings)



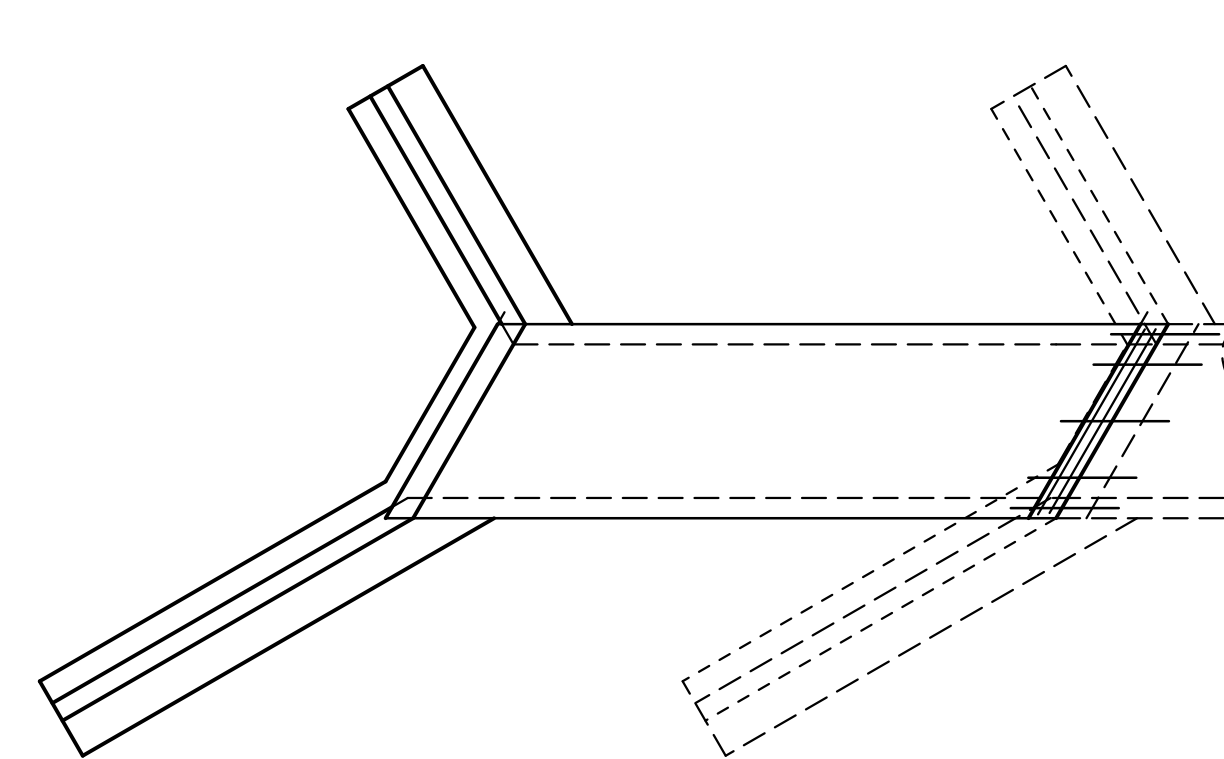
TYPICAL SECTION
(Skewed Structure)



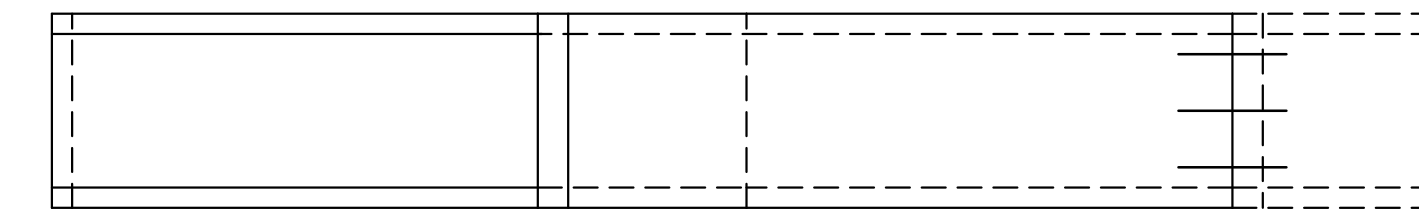
TYPICAL SECTION
(Straight Wings)



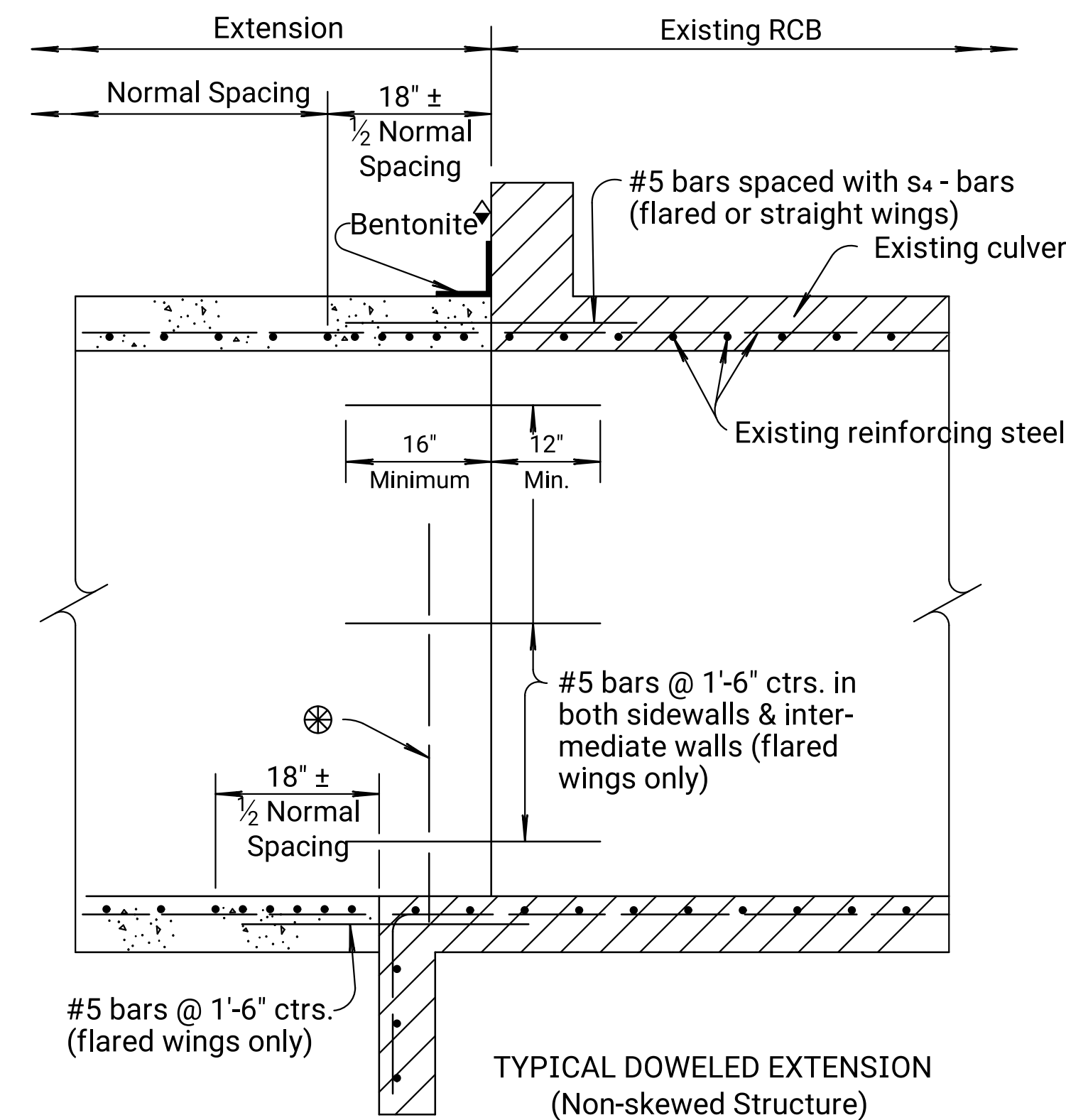
TYPICAL PLAN
(Flared Wings)



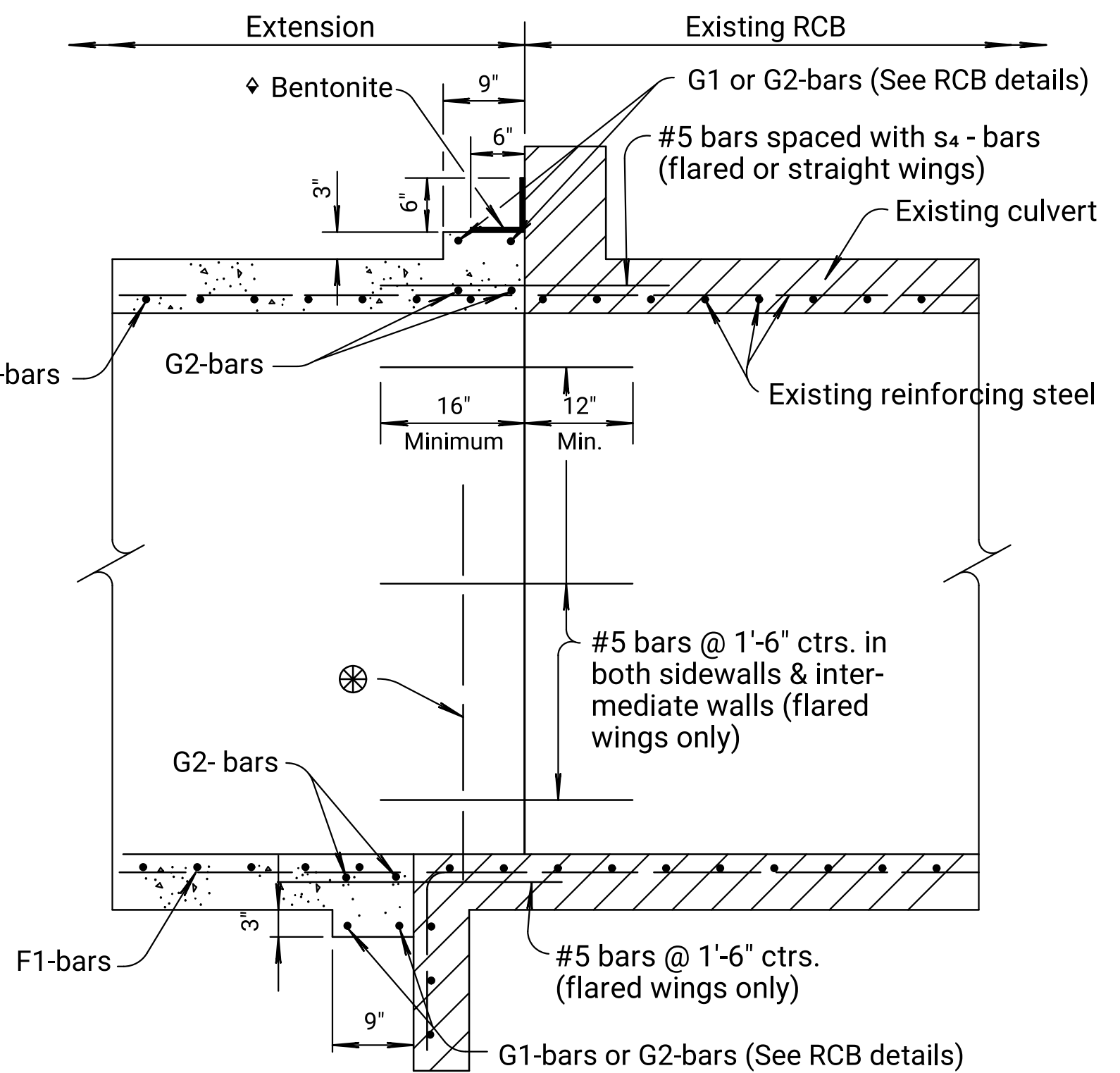
TYPICAL PLAN
(Skewed Structure)



TYPICAL PLAN
(Straight Wings)



TYPICAL DOWELED EXTENSION
(Non-skewed Structure)

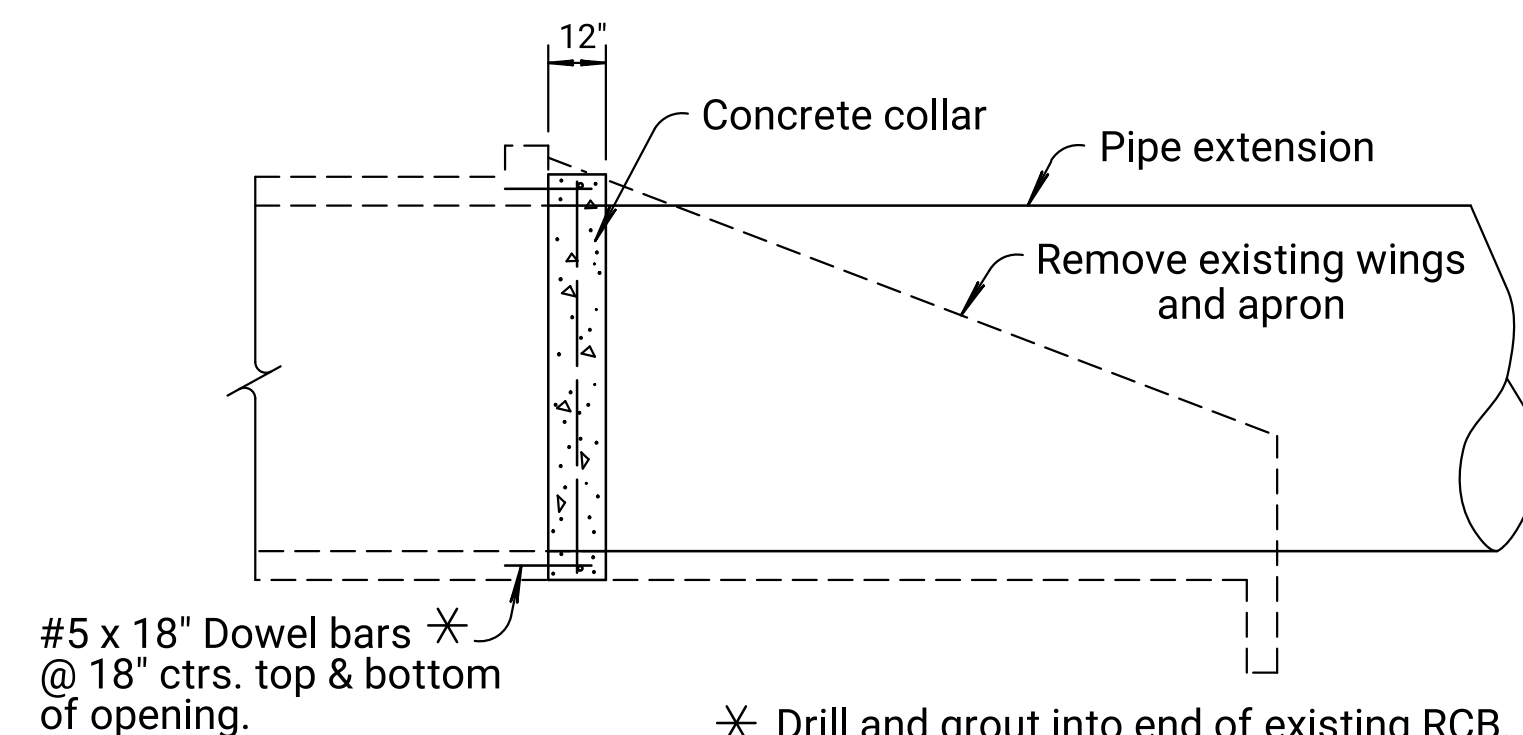


TYPICAL DOWELED EXTENSION
(Skewed Structure)

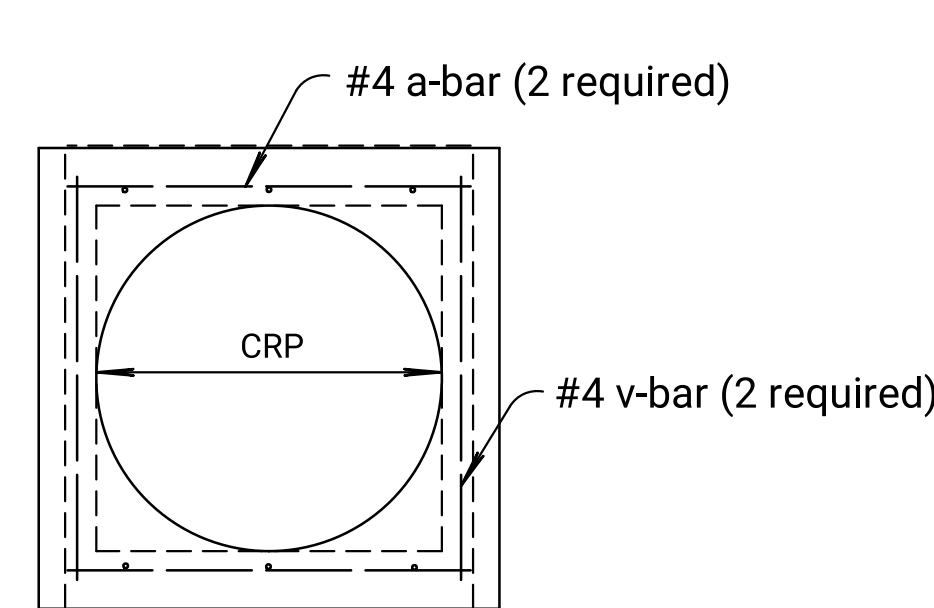
NOTE: The joint between the RCB extension and the existing structure shall be protected by a bentonite based system as shown when the following conditions exist:
1. Fill depth over the joint is 2 feet or less.
2. Lateral location of the joint is 20 feet or less from edge of pavement.
3. RCB span is equal to or greater than 8 feet.
The bentonite shall be placed on the exterior walls and top slab and shall conform with the requirements of the Special Provision for "Bridge Backwall Protection System".
All materials and labor required for this work shall be Subsidiary to the bid item "Grade 4.0 Concrete".

Quantities on this sheet are included in Summary of Quantities, Sh. No. 62

Station	Size Exist. R.C.B.	Roadway		Length "L"		Conc. (Grade 4.0) Cu. Yds.	Conc. (Grade 4.0) (AE) Cu. Yds.	Reinf. Steel (Gr. 60) (Epoxy Coated) Lbs.	Reinf. Steel (Gr. 60) Lbs.	Foundation Stabilization Cu. Yds.	Remarks
		Lt.	Rt.	Lt.	Rt.						
7384+11.04	6' x 4'	15'-4"	26'-10"	0'-0"	12'-0"	10.4	2.2	680	750	6	
TOTAL						10.4	2.2	680	750	6	



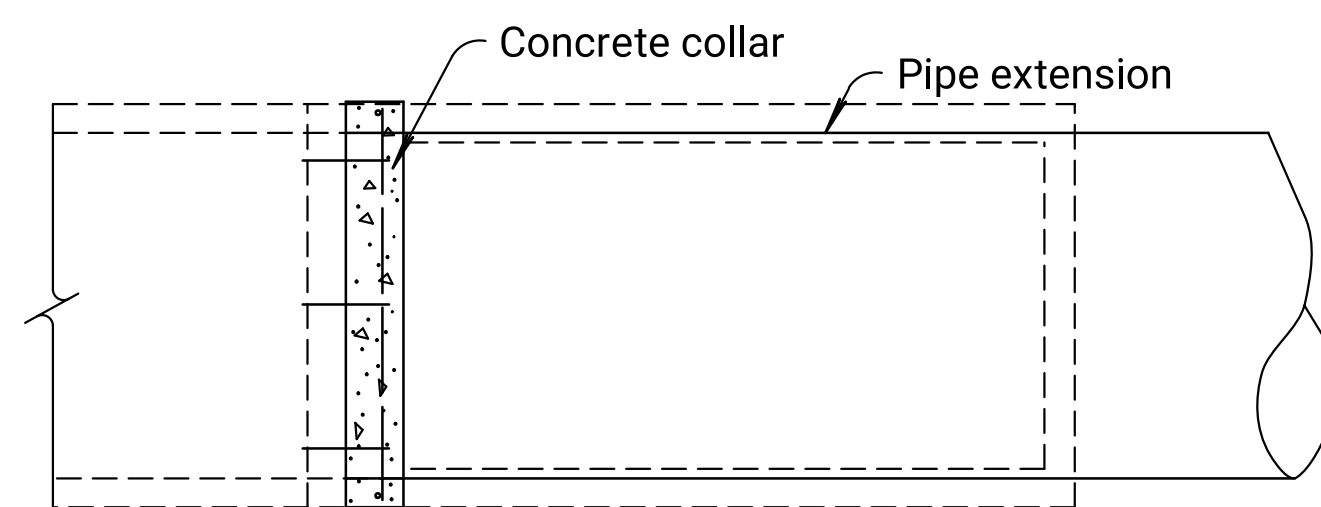
SIDE VIEW



END VIEW

CONSTRUCTION OF COLLAR

NOTE: All Labor, Equipment, Materials, and Incidentals necessary to complete the concrete collar shall not be paid for directly but shall be considered Subsidiary to the individual pipe bid item.



TOP VIEW

Station	Size Exist. R.C.B.	Size C.R.P. Extension	Ext. Length		End Section	
			Lt.	Rt.	Lt.	Rt.

NO.	DATE	REVISIONS	BY	APPROVED
08	05-04-05	Class to Grade Conc., notes & details	S.W.K.	J.O.B.
07	04-18-01	Revised General Note	R.J.S.	J.O.B.
06	12-05-00	Rev. General Note flared wing ext.	R.J.S.	J.O.B.

TYPICAL
CULVERT EXTENSIONS

RD080

DESIGNED	QUANTITIES	TRACED
DESIGN CK.	QUAN. CK.	TRACE CK.

Item Location	SUMMARY OF QUANTITIES													
	Excavation		Concrete		Reinforcing Steel		*Piles (Steel)	Drilled Shaft (48") (Cased)	Sonic Test (Drilled Shaft) (Set Price)	Core Hole (Investigative)	Bridge Backwall Prot. System	Abutment Strip Drain	Slope Protection (Riprap Stone)	Bridge Deck Grooving
	Class I Cu. Yds.	Class II Cu. Yds.	(Grade 4.0) (AE) (SW) Cu. Yds.	(Grade 4.0) (AE) Cu. Yds.	(Grade 60) (Epoxy Coated) Lbs.	(Grade 60) Lbs.	(HP12x53) Lin. Ft.	Lin. Ft.	Each	Lin. Ft.	Sq. Yds.	Sq. Yds.	Cu. Yds.	Sq. Yds.
Abutment No. 1	59	-	**	-	**	-	155	-	-	-	23	20	449	-
Pier No. 1	11	-	**	7.5	-	1510	-	64	-	55	-	-	-	-
Pier No. 2	8	3	**	7.4	-	1490	-	60	-	52	-	-	-	-
Abutment No. 2	59	-	**	-	**	-	130	-	-	-	23	20	453	-
Substr. Total	137	3	-	14.9	-	3000	285	124	-	107	46	40	902	-
Superstr. Total	-	-	282.4	-	68430	-	-	-	-	-	-	-	-	364
Total	137	3	282.4	14.9	68430	3000	† 285	124	1	107	46	40	902	364

** Quantities are included in the Superstr. Total Quantity.

† Summary of Piling
Abutment No. 1 5 @ 31 ft.
Abutment No. 2 5 @ 26 ft.

* NOTE: Only steel pile HP 12x53 shall be used on this project

CONTRACTOR CONSTRUCTION STAKING: Contractor Construction Staking for clear span bridges requires two independent surveys. See KDOT Specifications.

GENERAL NOTES

EXISTING STRUCTURE: Plans of the existing structure are on file and available for inspection by qualified bidders at the State Bridge Office, KDOT, Eisenhower State Office Building, 700 SW Harrison St., Topeka, KS.

SLOPE PROTECTION (Riprap Stone): Place Slope Protection (Riprap Stone) to the limits and thicknesses shown on the plans or as directed by the Engineer. Use (Light 200 LB) as described in Division 1100 placed to the limits shown in the plans.

CAMBER: Provide camber as shown on the Camber Diagram unless the Contractor uses either long span steel beam falsework (concrete dead load deflection greater than 1/4") or timber falsework with greater than 12'-0" clear span. If either case exists, submit falsework plans that show the additional required camber.

EMBANKMENT: Complete the embankment at the abutments as shown on the Bridge Excavation sheet prior to driving the abutment piling or commencing with the abutment footing excavation.

DRIP LINE PROTECTION: Place a 10 foot wide mat of geotextile under the rock embankment on the berm and berm slopes and centered on the drip lines of the slab.

FALSEWORK PLANS: A licensed Professional Engineer shall design the falsework details. Details shall bear the seal of a licensed Professional Engineer. Submit electronic plans conforming to Section 105 of the Standard Specification with details in compliance with KDOT Specifications to the Field Engineer for review.

BRIDGE EXCAVATION: Elevation 1295.50 shall designate the Excavation Boundary Plane of Class I and Class II Excavation; Class I above the plane, Class II below the plane. See the Bridge Excavation sheet for the limits of pay excavation.

CONCRETE: Superstructure concrete is bid as Concrete (Grade 4.0)(AE)(SW). Substructure concrete is bid as Concrete (Grade 4.0)(AE). The Contractor may use Concrete (Grade 4.0) in the footings. Bevel all exposed edges of all concrete with a 3/4" triangular molding, except as otherwise noted on the plans. Construction Joints are optional with the Contractor, but if used, place only at locations shown, or at locations approved by the Engineer.

FALSEWORK INSPECTION: This project has falsework plan requirements which are considered "Category 2" by KDOT specifications. If falsework deficiencies or variations from the approved and sealed plans are found, the falsework design Engineer of Record will provide written approval of the changes. If for the convenience of the Contractor the falsework becomes "Category 1" by the use of non-typical supports; then the inspection and review requirement of "Category 1" will be fully enforced, but at no cost to the State. "Category 2" falsework inspection is not paid for directly, but is subsidiary to other bid items.

BACKFILL COMPACTION: Compact backfill at the abutments.

PILING: Piles shall be driven to penetrate the Long Creek Limestone Member. Driving shall stop when in the opinion of the Engineer additional driving may damage the piling. Drive all piling to the Pile Driving Formula Load of:

Abutment No. 1 52 Tons
Abutment No. 2 52 Tons

REINFORCING STEEL: All reinforcing steel dimensions are to the centerline of bars unless otherwise noted. All reinforcing steel, except the spiral bars, shall conform to the requirements of ASTM A615, Grade 60. Spiral bars may meet the requirements of either ASTM A615 (GR. 40 or 60) or AASHTO M32, and are included in the bid item "Reinforcing Steel (Gr. 60)".

FALSEWORK PLANS AND SHOP DRAWINGS: Use the U.S. Customary system of units on falsework plans and shop drawing details.

As a minimum drive each pile to the load and penetration, but in no case shall the pile be driven to more than 110% of Pile Driving Formula Driving Load. At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Engineer may request that the Pile Driving Analyzer (PDA) equipment be used.

Where non-coated bars come in contact with epoxy coated bars, they need not be coated.

FALSEWORK: Leave the falsework in place for the entire unit until 15 days after the last concrete pour for the unit or longer as directed by the Engineer.

CORRAL RAIL: Build the corral rail after the falsework is struck.

PILING SPLICE LOCATION: Integral pile splice locations and weld testing criteria for Abutments No. 1 & 2 will follow the "Standard Pile Details" Sheet (BR110).

ABUTMENT STRIP DRAIN: See the General Notes on the "Abutment Strip Drain" sheet.

COLUMN CONSTRUCTION: Cure the drilled shaft footing as required by the KDOT Specifications before beginning the column construction (placing resteel or formwork). Do not place cast in place shear bolts, coil inserts or other devices used as falsework support in the column without the approval of the Engineer. Do not remove the column formwork without the approval of the Engineer. Curing shall continue after the formwork is removed as required by the KDOT specifications.

BRIDGE BACKWALL PROTECTION SYSTEM: See the General Notes on the "Abutment Strip Drain" sheet.

BROKEN CONCRETE: Waste the broken concrete from the existing bridge on sites provided by the Contractor and approved by the Engineer.

CONSTRUCTION LOADS: Limited traffic is permitted on the new sub-deck, one-course deck or any concrete overlay during the curing period, keep any exposed deck wet during the curing period. See KDOT Specifications Section 710 Tables 710-1 & 710-2 for additional information.

GEOTECHNICAL REPORT: The geotechnical report and geotechnical report addendum (Dated May 2023) and the geotechnical information shown on the plans is the best information available. The report is available for inspection by qualified bidders at the State Bridge Office, KDOT, Eisenhower State Office Building, 700 SW Harrison, Topeka, KS.

SLAB ELEVATIONS: The Contractor shall record elevation readings on the "Slab Elevations" sheet in the table at locations designated by a "(2)" and submit the sheet to the Engineer.

REMOVAL OF EXISTING STRUCTURE: Removal of existing structure is included in the bid item, "Removal of Existing Structures", Lump Sum. All materials removed from the existing structure shall become the property of the Contractor. Remove this material from the site.

INDEX TO BRIDGE DRAWINGS	
Sheet No.	Drawing
32	General Notes and Quantities
33	General Notes
34	Contour Map
35	Construction Layout
36	Engineering Geology
37	Abutment Details
38	Abutment Strip Drain
39	Pier Details
40	Superstructure Details (1 of 2)
41	Superstructure Details (2 of 2)
42	Slab Elevations
43	32" Kansas Corral Rail (Without Curb)
44	Bill of Reinforcing Steel and Bending Diagrams
45	Detour Bridge Plans
	Standard
56	Bridge Excavation (LRFD)
57	Standard Pile Details
58	Supports and Spacers for Reinforcing Steel

DESIGN DATA

DESIGN SPECIFICATIONS: Superstructure (Reinforced Concrete Haunch Slab Design): AASHTO Specifications, 2007 Edition and latest Interim Specifications. Load and Resistance Factor Design.

Substructure: AASHTO Specifications, 2020 (9th) Edition and latest Interim Specifications. Load and Resistance Factor Design.

DESIGN LOADING: HL-93

Design Dead Load includes an allowance of 15 psf for a future wearing surface.

UNIT STRESSES:

Concrete (Grade 4.0)	f'c = 4.0 ksi
Concrete (Grade 4.0)(AE)	f'c = 4.0 ksi
Concrete (Grade 4.0)(AE)(SW)	f'c = 4.0 ksi
Reinforcing Steel (Grade 60)	fy = 60 ksi
Steel Pile	fy = 50 ksi

LRFD DESIGN PILE LOAD:

Design Loading (Tons/Pile)	Strength I	Service I	Phi
Abutment No. 1	52	36	0.60
Abutment No. 2	52	36	0.60

LRFD DESIGN DRILLED SHAFT LOAD:

Design Loading (Tons/Shaft)	Strength I	Service I	End Bearing Phi	Side Friction Phi
Pier No. 1	270	210	0.5	0.5
Pier No. 2	270	210	0.5	0.5

TRAFFIC DATA	
AADT (2024)	750
AADT (2044)	850
DHV	11%
D	65:35
T	14%

LFD & LRFR RATING FACTORS		
Rating Level	Inventory	Operating
Truck HS-20 (36T)	1.93	3.23
Type HET (110T)		1.31
2002 LFD Rating, 17th Edition AASHTO		
HL-93 Loading	1.70	2.20
2020 Manual for Bridge Evaluation		

3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 99-99-139.68 (113) Sta. 7375+41.16

GENERAL NOTES AND QUANTITIES
K-99 OVER DRAGOON CREEK DRAINAGE
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO. OF SCALE	APP'D
DESIGNED ASF DETAILED JAH QUANTITIES ASF CADD JAH	
DESIGN CK. TKI DETAIL CK. TKI QUAN. CK. TKI CADD CK. TKI	

Plotted By: JHOVERSO
 File: c:\pwworking\ventra\01\43371674\k572801\br113-01.dgn
 Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	33	135

GENERAL NOTES

CONCRETE PLACING SEQUENCE: The sequence of placing concrete in the slab and curbs shall be as shown, or the Contractor may submit an alternate placing sequence for review. Submit the alternate placing sequence to the Engineer at the Preconstruction Conference. Include the proposed rate of concrete placement in C.Y./h, the plant capacity, placement direction, construction joint location, a description of the equipment used in placing the concrete, proposed admixtures, and the quantity of concrete in each placing segment. Any additional cost for the Contractor's alternate plan of placing concrete, including admixtures, shall be at the Contractor's expense and shall be considered subsidiary to the bid item. "Concrete (Grade 4.0)(AE)(SW)". Approval of the Contractor's alternate sequence is required prior to placement of concrete in the deck.

PIER BEAM CONSTRUCTION: Cure the columns as required by the KDOT Specifications before beginning the pier beam construction (placing resteel or formwork). Do not drill and grout bolts or other devices into the columns for falsework support unless approved by the Engineer. Cure the columns as required by the KDOT Specifications before beginning to place the superstructure concrete.

TEMPERATURE: The design temperature for all dimensions is 60°F.

QUANTITIES: Items not listed separately in the Summary of Quantities are subsidiary to other items in the proposal.

DIMENSIONS: All dimensions shown on the design plans are horizontal dimensions unless otherwise noted. Make necessary allowances for roadway grade and cross slope.

CONSTRUCTION JOINTS: The construction joints shown are optional with the Contractor. If used, place the construction joints only at locations shown or at locations approved by the Engineer.

DRILLED SHAFTS: Construct the drilled shafts using the cased method. A permanent casing is required. All excavation, concrete, reinforcing steel, pipes for Sonic Testing, casings, labor, and incidentals necessary to complete the shaft as shown on the details and as directed by KDOT Specifications shall be included in the bid item "Drilled Shafts (48")(Cased)". Use Grade 4.0 Concrete in the drilled shaft. In no case shall the bottom of the drilled shaft be placed higher than the elevation shown unless otherwise directed by the KDOT Geologist.

Drill an Investigative Core Hole at the location(s) shown on the plans. See KDOT Specifications.

If the location of the top of the shaft is such that the casing cannot be overtopped to remove concrete impurities, provide extra casing length to over-pour the concrete in the shaft and chip back to the plan elevation of the top of the shaft.

If the permanent casing is to be corrugated metal pipe (CMP) then it will be galvanized.

SONIC TESTING: Equip all drilled shafts with piping to allow sonic testing to be done. Install pipes at locations shown on the plans. All wet pours will be tested. Also, the Engineer has the option to require sonic, non-destructive, integrity testing at any location of concern. Sonic testing shall be paid for at the unit price set for "Sonic Test" (Drilled Shaft) (Set Price). If the sonic testing indicates defective concrete in the shaft, the Engineer will measure the first sonic test for payment, and the Contractor is responsible for subsequent sonic testing of that shaft. Report test results directly to KDOT's Chief Geologist. No work will be done above the top of drilled shaft without the approval of the Chief Geologist.

INVESTIGATIVE CORE HOLES: Please contact the Topeka Regional Geology Office when the schedule for the investigative core holes have been established so that a member of the staff may be on site when the work is being performed.

BRIDGE DECK GROOVING: After the bridge deck has cured, transversely groove the deck in accordance with KDOT Specifications. For phased construction groove each completed phase before opening to traffic. Align the grooves from each adjacent phase across the bridge deck without jogs or discontinuities. For skewed bridges all grooving will be perpendicular to the centerline of the bridge.

DEMOLITION PLANS: This is a Category A Demolition. Submit detailed Demolition Plans to the Field Engineer per KDOT Specifications. No Demolition work will begin without approved Demolition Plans. A Licensed Professional Engineer is not required.

ASBESTOS INFORMATION: Samples of this structure were tested to determine the amount of Asbestos Containing Materials (ACM) present in the components. The results are listed below:

Concrete (Abutment, Deck)	0%
Date of Report	01/18/2022

For any result above greater than 1%, abatement shall be performed according to KDOT Specifications. Results less than 1% require no abatement.

CAUSEWAY: If the Contractor chooses to build a causeway for bridge construction purposes, the Contractor shall obtain any required U.S. Army Corps of Engineers Section 404 Permit, Kansas Department of Agriculture Permit, Kansas Department of Health and Environment Section 401 Permit, Kansas Department of Wildlife Parks and Tourism Permit or any other permit required by law for causeway construction. Obtain the permit in a timely manner so as not to delay the completion of the project.

Plotted By: JHOVERSO Plot Location:
 File: c:\pwworking\central\01\43371674\k4572801\br113-02.dgn
 Plot Date: 01-08-24

3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	

KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 99-99-139.68 (113) Sta. 7375+41.16
GENERAL NOTES
K-99 OVER DRAGOON CREEK DRAINAGE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
		QUANTITIES	ASF
		TKI	CADD
		TKI	CADD
		TKI	TKI

P.I. Sta. 7339+07.78 = SW Cor. Sec. 16, T14S, R12E=
P.I. Sta. 165+66.60 on Proj. 99-99 K 516 Date 1928
N 476,313.797 E 8,615,481.392
1. Found 3/8" rebar (0.3' deep)
2. Found nail & wshr. NE side of pow. pole
3. Conc. nail & KDOT wshr. W side utility mrkr. po.
4. Conc. nail & KDOT wshr. E side of pow. pole
5. In TW of K-99 Hwy.

35.3' NW
52.2' NE
81.0' SSW

P.I. Sta. 7365+78.29 = W/4 Cor. Sec. 16, T14S, R12E=
Sta. 138+97.60 on Proj. 99-99 K 516 (Date 1928)
 $\Delta = 0^{\circ}23'23''$ Lt. (No curve)
N 478,984.271 E 8,615,467.769
1. Set 1/2" x 24" rebar w/ orange KDOT cap
2. \odot Top telephone pedestal
3. Line of N/S fence line
4. In line of fence line to E
5. In TW of K-99 Hwy.

26.5' E
30.5' W

Gary D. Kemble
S/2, NE/4
Sec. 17, T14S R12E

Sta. 7375+41.16, Construct
Br. No. 99-99-139.68 (113)
30'-40'-30'
Reinforced Concrete Slab, Continuous
and Parabolic Haunched (RCSH)
36'-0" Roadway

P.O.T. Sta. 7369+30.87=Sta. 135+41.60 on
Proj. 99-99 K 516 (Date 1928)
N 479,336.827 E 8,615,463.572
1. Set 1/2" x 12" rebar (0.3' deep)
2. Conc. nail & KDOT wshr. W face gate po.
3. Spk. & KDOT wshr. top metal "T" po.
4. Field entrance to west
5. TW of K-99 Highway

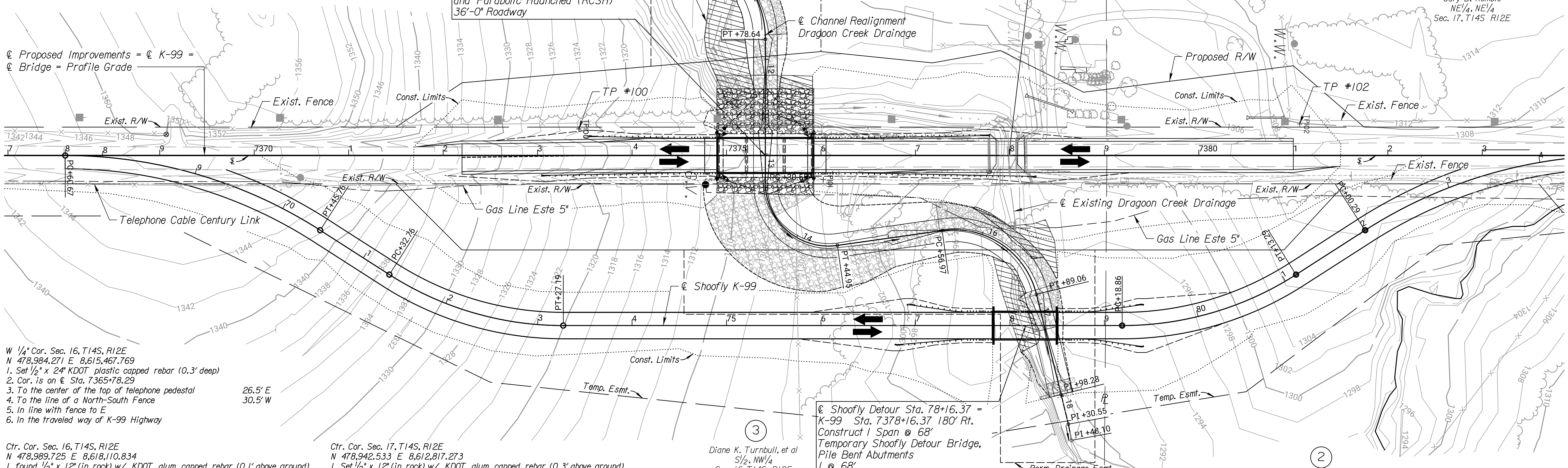
28.7' W
32.5' E
11.5' S
3.0' E

Sta. 7377+97.47, Remove
Br. No. 99-99-139.73 (072)
41'-6"
Steel Beam Simple Span (SBMS)
29.1' Roadway

P.I. Sta. 7392+27.09=NW Cor. Sec. 16, T14S, R12E =
Sta. 112+41.40 on Proj. 99-99 K 516 (Date 1928)
N 481,632.885 E 8,615,436.237
1. Found 3/8" rebar (0.3' deep)
2. Found nail & wshr. NW side cor. fc. po.
3. Found nail & wshr. top cor. fc. po.
4. \odot Top steel cor. fc. po
5. TW of K-99 Highway

54.5' SSW
44.7' NW
44.7' NE
4.0' E

Gary D. Kemble
NE/4, NE/4
Sec. 17, T14S R12E



\odot Proposed Improvements = \odot K-99 =
 \odot Bridge = Profile Grade

W 1/4 Cor. Sec. 16, T14S, R12E
N 478,984.271 E 8,615,467.769
1. Set 1/2" x 24" KDOT plastic capped rebar (0.3' deep)
2. Cor. is on \odot Sta. 7365+78.29
3. To the center of the top of telephone pedestal
4. To the line of a North-South Fence
5. In line with fence to E
6. In the traveled way of K-99 Highway

26.5' E
30.5' W

Ctr. Cor. Sec. 16, T14S, R12E
N 478,989.725 E 8,618,110.834
1. found 1/2" x 12" (in rock) w/ KDOT alum. capped rebar (0.1' above ground)
2. Cor. is 2643.07' Rt. (East) of \odot Sta. 7365+78.29 SE $\Delta = 89^{\circ}46'33''$
3. Set conc. nail and KDOT wshr. W side fc. cor. po. 0.8' E
4. Set conc. nail and KDOT wshr. top fc. brace po.
5. Set cor. on W side fc cor. po.
6. In line with fences North-South and East

Ctr. Cor. Sec. 17, T14S, R12E
N 478,942.533 E 8,612,817.273
1. Set 1/2" x 12" (in rock) w/ KDOT alum. capped rebar (0.3' above ground)
2. Cor. is 2650.82' Lt. (West) of \odot Sta. 7365+78.29 SW $\Delta = 89^{\circ}23'24''$
3. Set conc. nail and KDOT wshr. W side 30' cedar
4. To line of newer fence North
5. In line with fences North-West
6. At Northwest side fence corner post

1.5' N
5.0' W

Diane K. Turnbull, et al
S/2, NW/4
Sec. 16, T14S R12E

\odot Shoofly Detour Sta. 78+16.37 =
K-99 Sta. 7378+16.37 180' Rt.
Construct 1 Span @ 68'
Temporary Shoofly Detour Bridge,
Pile Bent Abutments
1 @ 68'
28'-0" Roadway

47.7' NW
44.7' NE
4.0' E

Perm. Drainage Esmt.

Diane K. Turnbull
NW/4, NW/4
Sec. 16, T14S R12E

Shoofly Detour Curve Data
P.I. Sta. 69+07.21
 $\Delta = 32^{\circ}40'7''$
D = 11'27'33"
R = 500.00'
T = 146.54'
L = 285.09'
E = 21.03'
Super = R.C.

Shoofly Detour Curve Data
P.I. Sta. 72+32.70
 $\Delta = 32^{\circ}40'7''$
D = 16'48'08"
R = 341.00'
T = 99.94'
L = 194.43'
E = 14.34'
Super = 6%

Shoofly Detour Curve Data
P.I. Sta. 80+18.80
 $\Delta = 32^{\circ}40'7''$
D = 16'48'08"
R = 341.00'
T = 99.94'
L = 194.43'
E = 14.34'
Super = 6%

Shoofly Detour Curve Data
P.I. Sta. 83+46.83
 $\Delta = 32^{\circ}40'7''$
D = 11'27'33"
R = 500.00'
T = 146.54'
L = 285.09'
E = 21.03'
Super = R.C.

N 1/4 Cor. Sec. 17, T14S, R12E
N 481,648.799 E 8,618,080.040
1. Found 3/8" capped rebar LS (0.8' deep)
2. Cor. is 2643.85' Rt. (East) of \odot Sta. 7392+27.09 SE $\Delta = 89^{\circ}39'46''$
3. Found spike and wshr. in top fc. cor. po.
4. Set nail and KDOT wshr. in top strand steel fc. p.
5. To the traveled way of East-West road
6. In line with fence to South

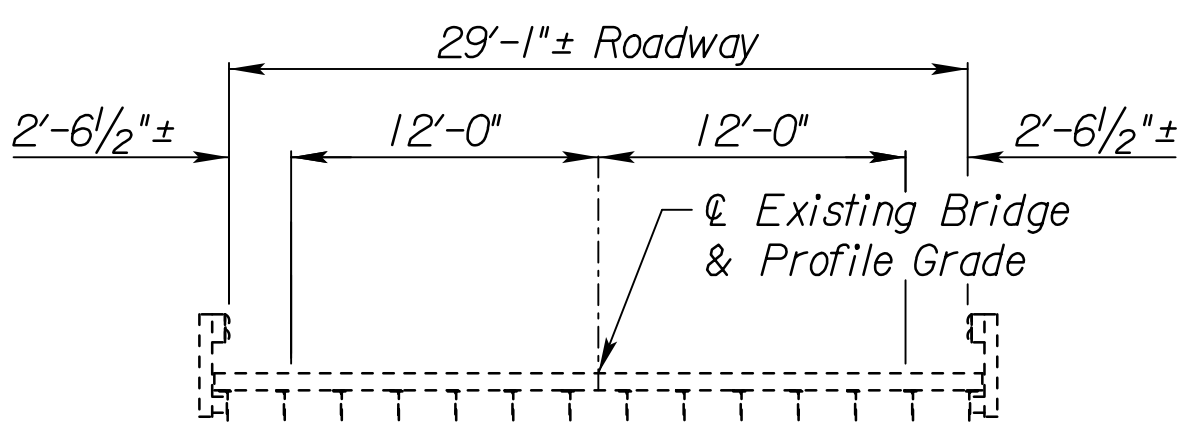
20.4' S
25.9' NNE
3.0' S

N 1/4 Cor. Sec. 17, T14S, R12E
N 481,593.554 E 8,612,810.003
1. Set 1/2" x 24" KDOT alum. capped rebar (0.3' deep)
2. Cor. is 2626.53' Lt. (West) of \odot Sta. 7392+27.09 SE $\Delta = 89^{\circ}49'27''$
3. Set conc. nail and KDOT wshr. in side fc. cor. po.
4. Set conc. nail and KDOT wshr. in side fc. gate po.
5. To the traveled way of East-West road
6. In line with fence to North

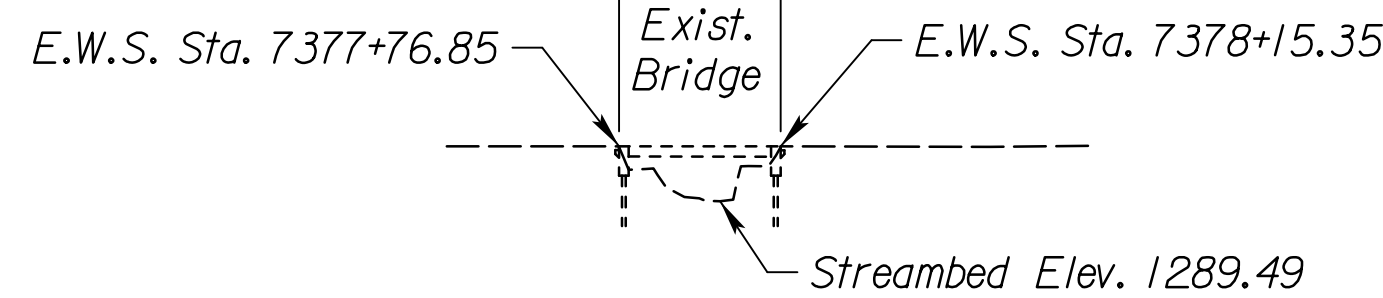
SCALE
50' 0 50' 100'

PLAN: Lat. & Long.

Utility Owners
Everygy
818 So. Kansas Ave
Topeka, KS 66612
Scott Schlageck
785-575-6014
Telephone Cable
Century Link
Kenny (Integrity Locate)
913-305-6679
Gas Line
City of Eskridge
785-449-2621



EXISTING TYPICAL SECTION



EXISTING BRIDGE WATERWAY OPENING
Looking Upstream
Total Waterway Opening = 293 S.F.
Historic High Water Elev. 1301.00 (Unknown)
Overtopping Elevation (Sta. 7378+73) Elev. 1303.63

HISTORIC HIGHWATER

Land owner, Gary Kemble, has not seen overtopping of the K-99 bridge over Dagoon Creek Drainage that they have knowledge of the site. Date of highest level observed in unknown. HHW Elev. 1301.00

Note:
All information is for K-99 unless otherwise noted.

Vertical Datum: NAVD 88 (Geoid 18)

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	34	135

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 99-99-139.68 (113) Sta. 7375+41.16

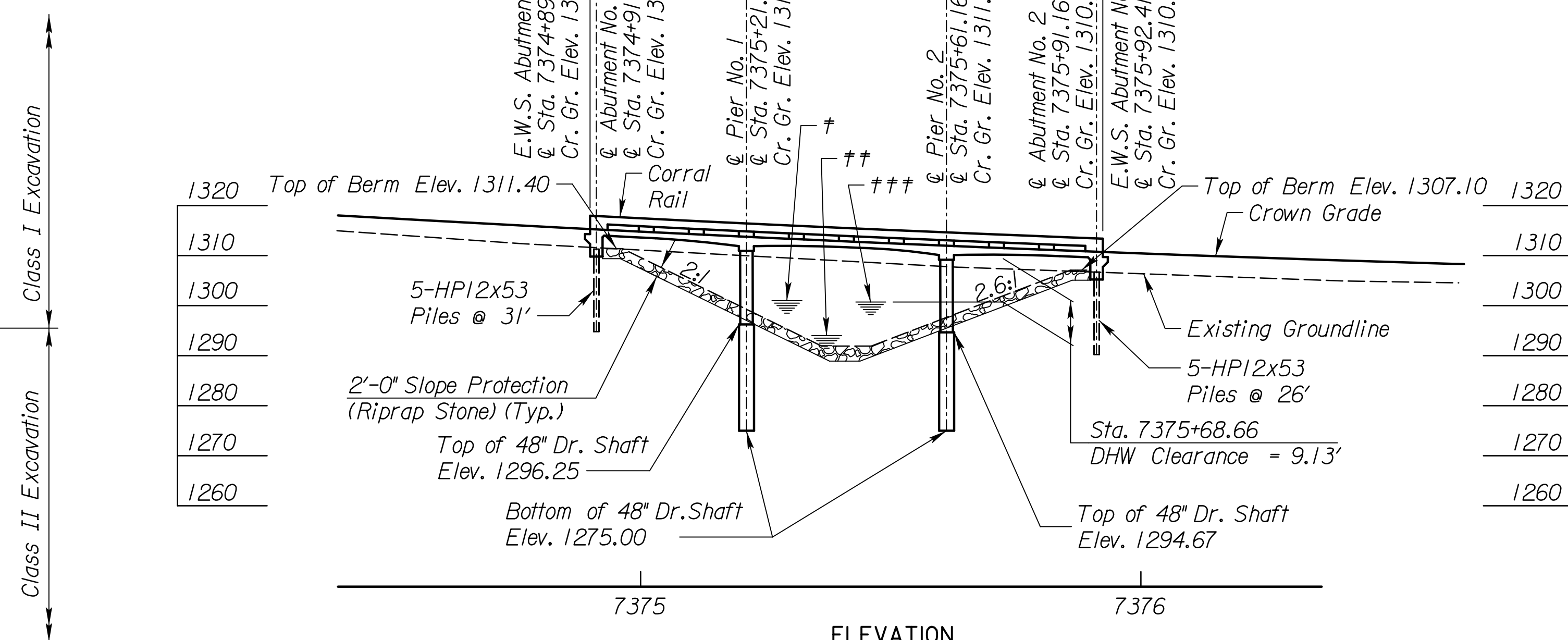
CONTOUR MAP
K-99 OVER DRAGON CREEK DRAINAGE
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
		QUAN. CK.	ASF
		CADD CK.	TKI
		CADD CK.	TKI

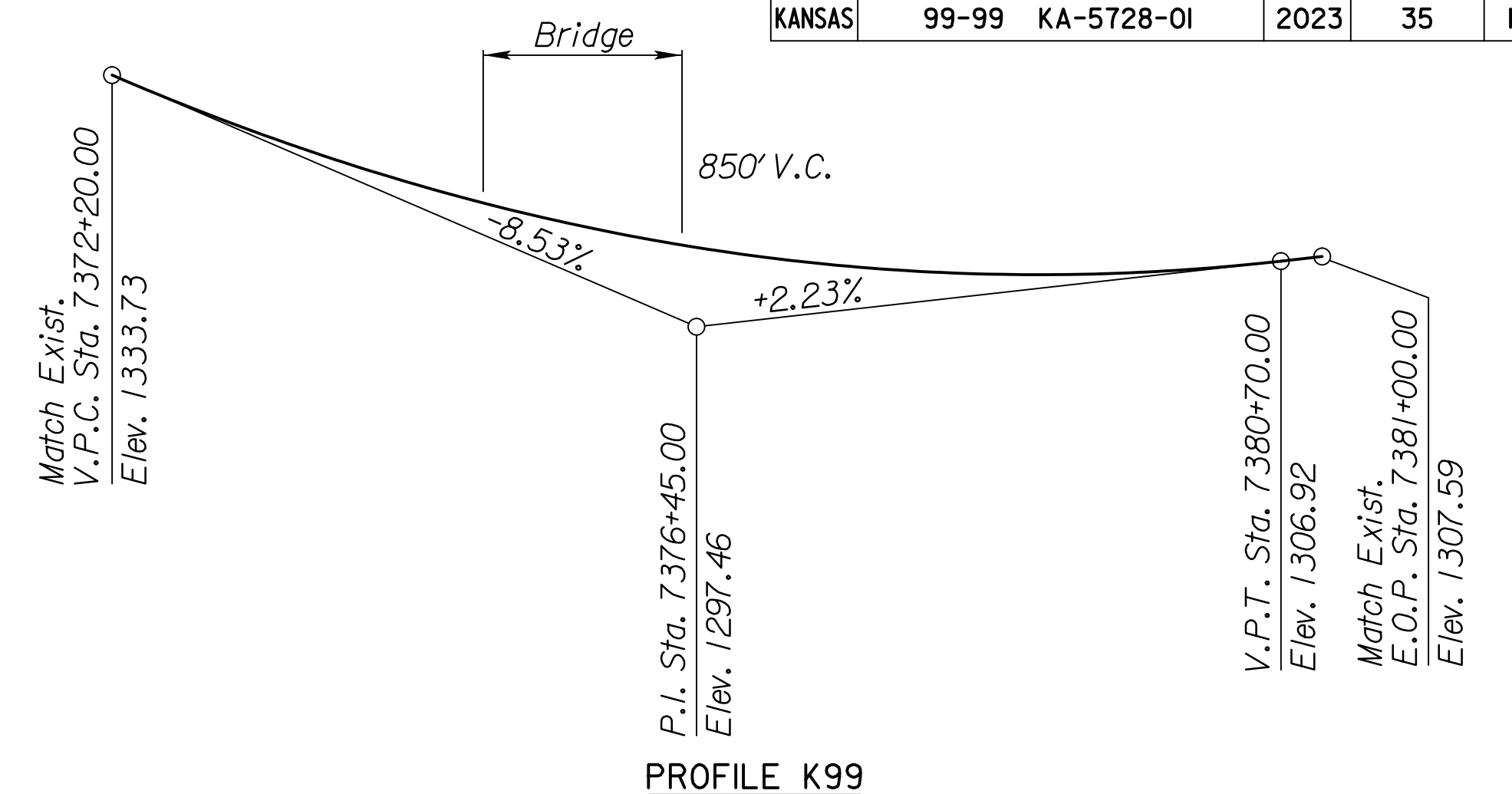
Plotted By: JHOVERSO
File: c:\working\central\01\43371674\k4572801\br113-03.dgn
Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	35	135

† H.H.W. Elev. 1301.00
 †† O.H.W. Elev. 1294.00
 ††† D.H.W. Elev. 1300.72



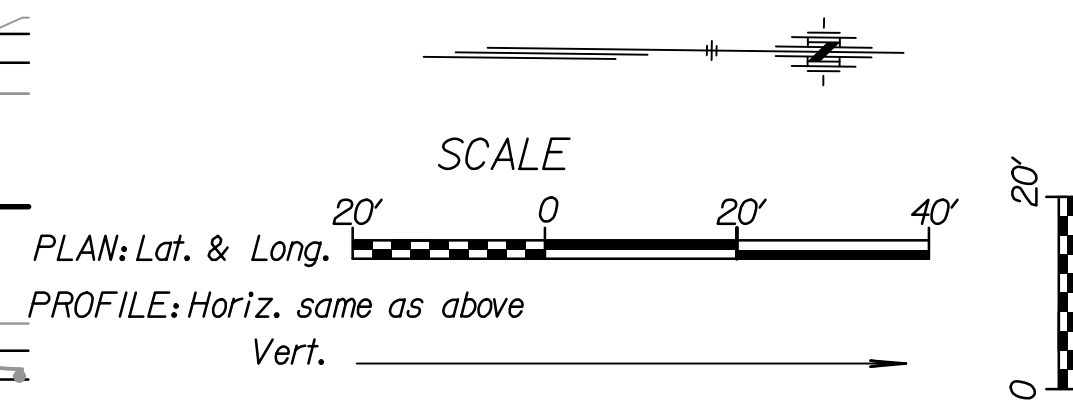
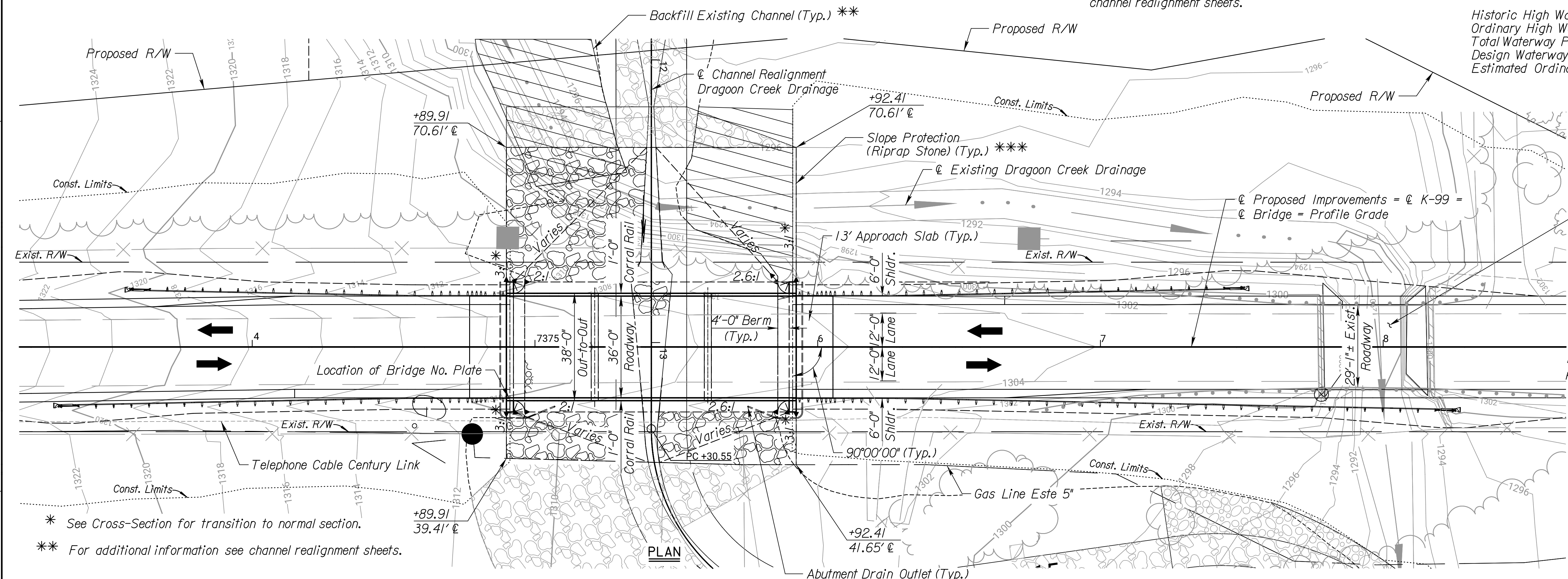
ELEVATION
 30' - 40' - 30'
 Reinforced Concrete Slab, Continuous and Parabolic Haunched (RCSH)
 Pile Bent Abutments, Column Bent Piers
 36'-0" Roadway



DRAINAGE DATA

Drainage Area	1.2	Sq. Mi.
Design Frequency	50	Years
Design Discharge (Q_{des})	2,090	cfs
Design High Water Elevation	1300.72	Ft.
Change in Design Backwater	-1.41	Ft.
Design Backwater Elevation	1301.91	Ft.
Overtopping Elevation (Sta. 7379+00)	1304.97	Ft.
Overtopping Discharge	4,700	cfs
Overtopping Frequency	>500	Years
Discharge at Q100	2,550	cfs
Change in Backwater at Q100	-1.10	Ft.
Backwater Elevation at Q100	1302.89	Ft.
Historic High Water Elevation	1301.00	Ft.
Ordinary High Water Elevation	1294.00	Ft.
Total Waterway Provided	1,010	Sq. Ft.
Design Waterway Provided	230	Sq. Ft.
Estimated Ordinary High Water Discharge	250	cfs

*** Slope protection limits shown are for bridge slope protection only. For additional information outside of bridge slope protection limits see channel realignment sheets.



3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
Br. No. 99-99-139.68 (I13) Sta. 7375+41.16				
CONSTRUCTION LAYOUT				
K-99 OVER DRAGOON CREEK DRAINAGE				
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.				
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	AS	DETAILED	JAH	QUANTITIES
DESIGN CK.	TKI	DETAIL CK.	TKI	QUAN. CK.
			TKI	CADD CK.

BM#10 Set "T" shaped fc. po. S side of rock @ gate po. 22.29' Lt. & Sta. 7369+06.99 Elev. 1350.28
 BM#11 Set mag. nail & KDOT wshr. top SE abut. of bridge 16.75' Rt. & Sta. 7377+78.17 Elev. 1302.52
 BM#12 Set mag. nail & KDOT wshr. top E hdlw. RCB 15.22' Rt. & Sta. 7384+11.38 Elev. 1306.98
 BM#13 Set RR spike in E face of pow. pole 34.18' Lt. & Sta. 7387+80.38 Elev. 1317.29

Vertical Datum: NAVD 88 (Geoid 18)

Plotted By: JHOVERSO
 File: c:\pwworking\ventra\0\1\4337674\k572801\br113-04.dgn
 Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	36	135

PILING: Piles shall be driven to penetrate the Long Creek Limestone Member. Driving shall stop when in the opinion of the Engineer additional driving may damage the piling. Drive all piling to the Pile Driving Formula Load of:

Abutment No. 1 52 Tons
Abutment No. 2 52 Tons

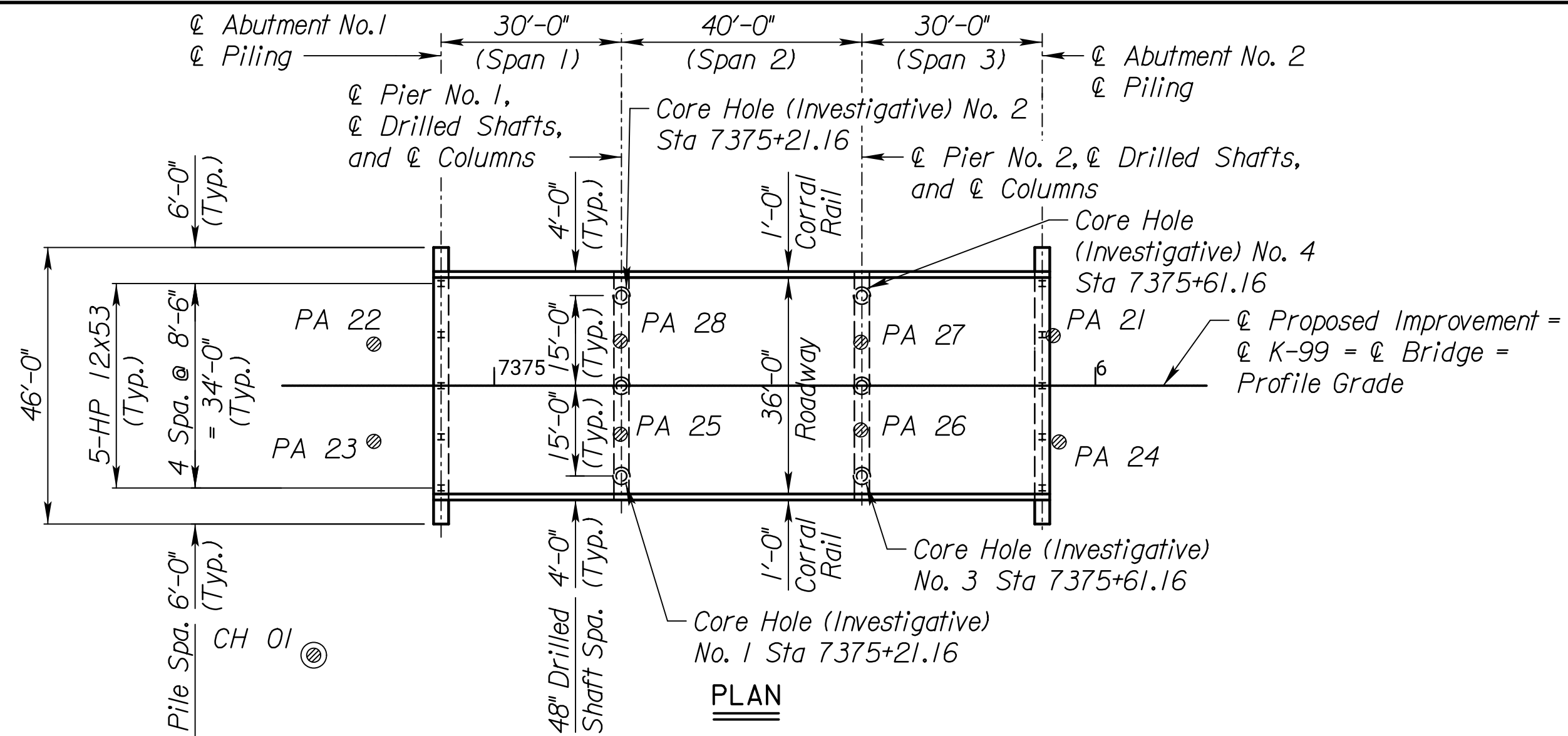
As a minimum drive each pile to the load and penetration, but in no case shall the pile be driven to more than 110% of Pile Driving Formula Driving Load. At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Engineer may request that the Pile Driving Analyzer (PDA) equipment be used.

DRILLED SHAFTS: Construct the drilled shafts using the cased method. A permanent casing is required. All excavation, concrete, reinforcing steel, pipes for Sonic Testing, casings, labor, and incidentals necessary to complete the shaft as shown on the details and as directed by KDOT Specifications shall be included in the bid item "Drilled Shafts (48") (Cased)". Use Grade 4.0 Concrete in the drilled shaft. In no case shall the bottom of the drilled shaft be placed higher than the elevation shown unless otherwise directed by the KDOT Geologist.

Drill an Investigative Core Hole at the location(s) shown on the plans. See KDOT Specifications.

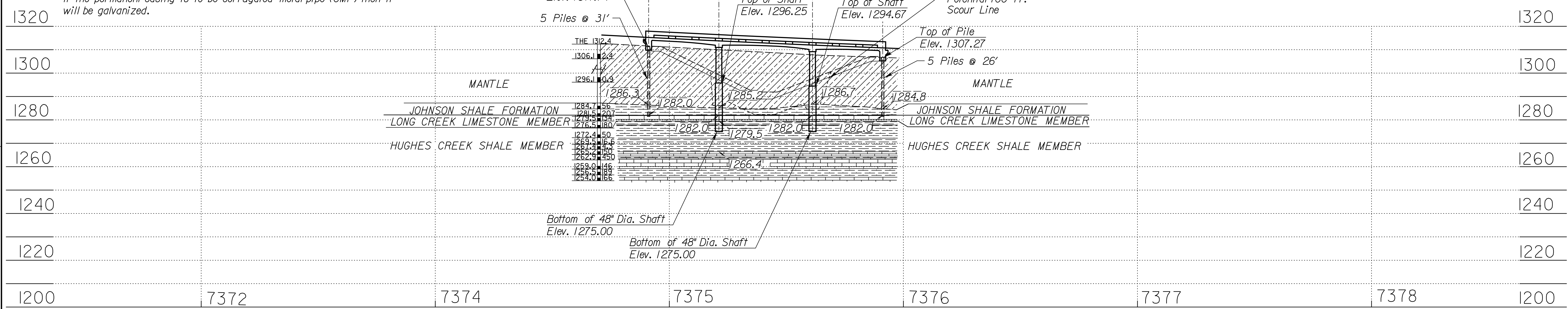
If the location of the top of the shaft is such that the casing cannot be overtopped to remove concrete impurities, provide extra casing length to over-pour the concrete in the shaft and chip back to the plan elevation of the top of the shaft.

If the permanent casing is to be corrugated metal pipe (CMP) then it will be galvanized.



Investigative Core Holes:
Please contact the Topeka Regional Geology Office when the schedule for the investigative core holes have been established so that a member of the staff may be on site when the work is being performed.

All piles shall be Gr. 50 Steel HP12x53.



Plotted By: JHOVERSO
 File: c:\pwworking\central\01\43371674\kds72801\br113-05.dgn
 Plot Date: 01-08-24

STANDARD		GEOLOGIC		SYMBOLS	
	Clay or Underclay		Caliche		Weathered Shale
	Silty Clay		Silty or Clayey Shale		Sandstone
	Silt		Limy Shale		Shaly Sandstone
	Sand		Black or Fissile Shale		Gypsum bed
	Gravel		Sandy Shale		Dolomite
	Boulders		Gypsiferous Shale		Bentonite
			Cross-bedded Sandstone		Loess
			Mortarbed		Coal
			Cherty Limestone		Siltstone
			Sandy Limestone		Chalk
			Wavy limestone		Chalky limestone

SOUNDINGS

- Core drill
- Power auger
- Hand tools
- Cone (CPT) penetrometer
- Shelby tube
- Water level

02/2023

PROFILING

Elevation Tons/sq. ft.
Elevation Blows/ft.

UNCONFINED COMPRESSION TEST
STANDARD PENETRATION TEST

Graphic representation of Cone Penetration Test in N60

CONE (CPT) PENETROMETER TEST Scale: N60

NOTE: Soundings shown on these plans are taken from notes obtained in the field and represent the best information available. Logs of these soundings are provided with the bid documents, or are available from the Kansas Department of Transportation in Topeka for inspection by interested and qualified bidders.

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 99-99-139.68 (113) Sta. 7375+41.16

ENGINEERING GEOLOGY
K-99 OVER DRAGOON CREEK DRAINAGE

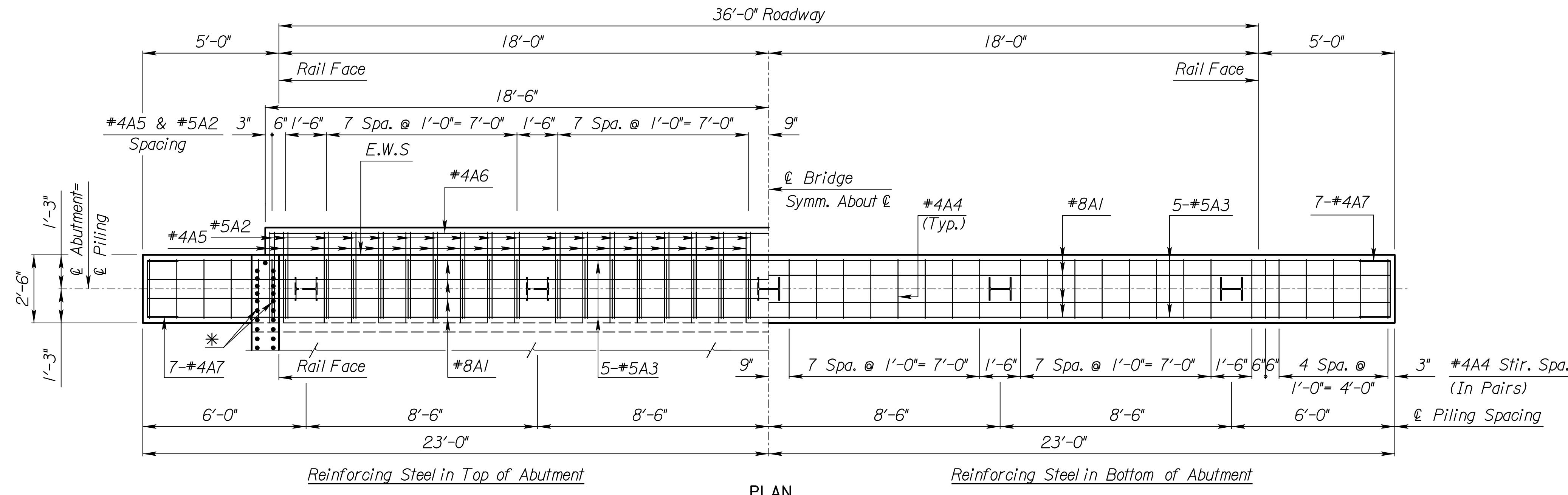
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
QUANTITIES	TKI	QUAN. CK.	TKI
DESIGN CK.	TKI	DETAIL CK.	TKI

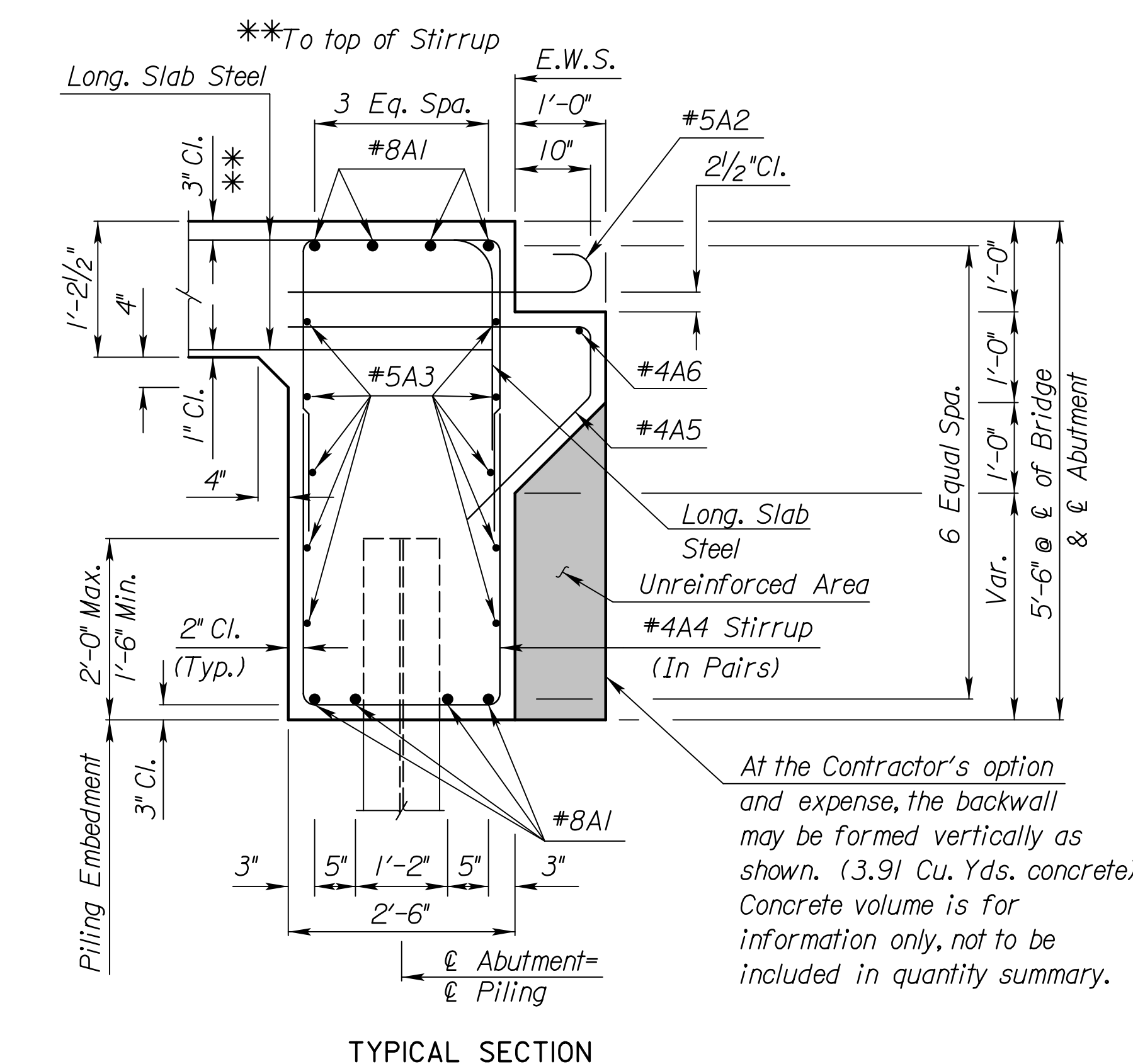
KDOT Graphics Certified 01-05-2024 Sheet No. 36

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	37	135

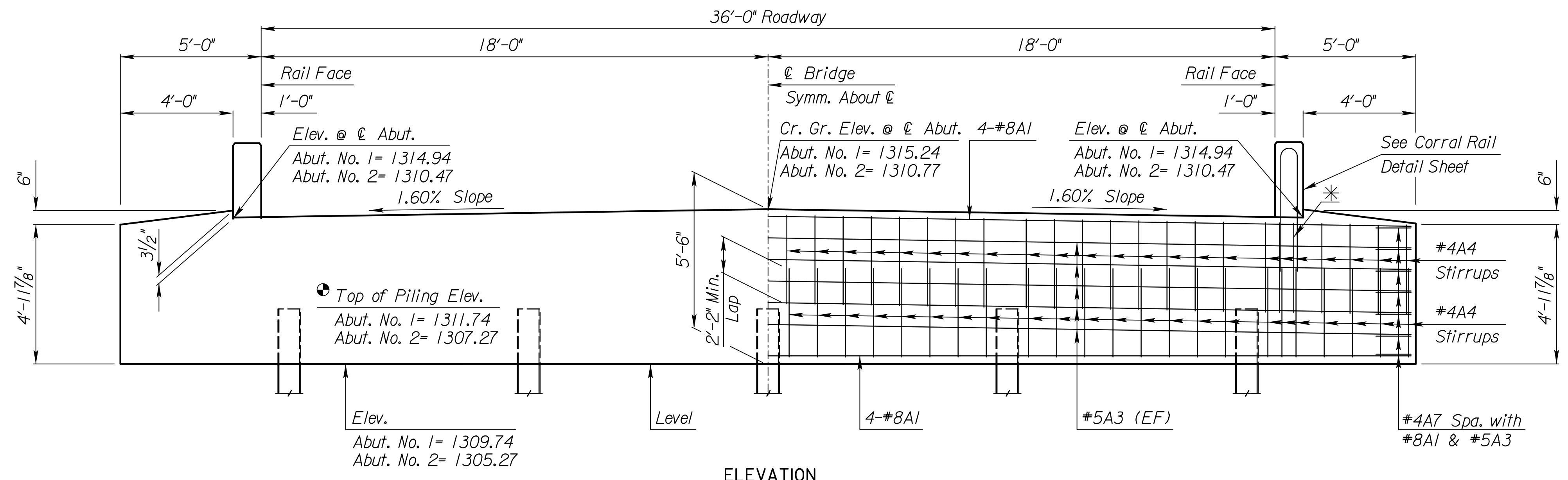
IsLab-17601502.dgn
 LHD
 Roadway Width = 36'-0"
 Skew and Direction = 0
 Number of Piles = 5



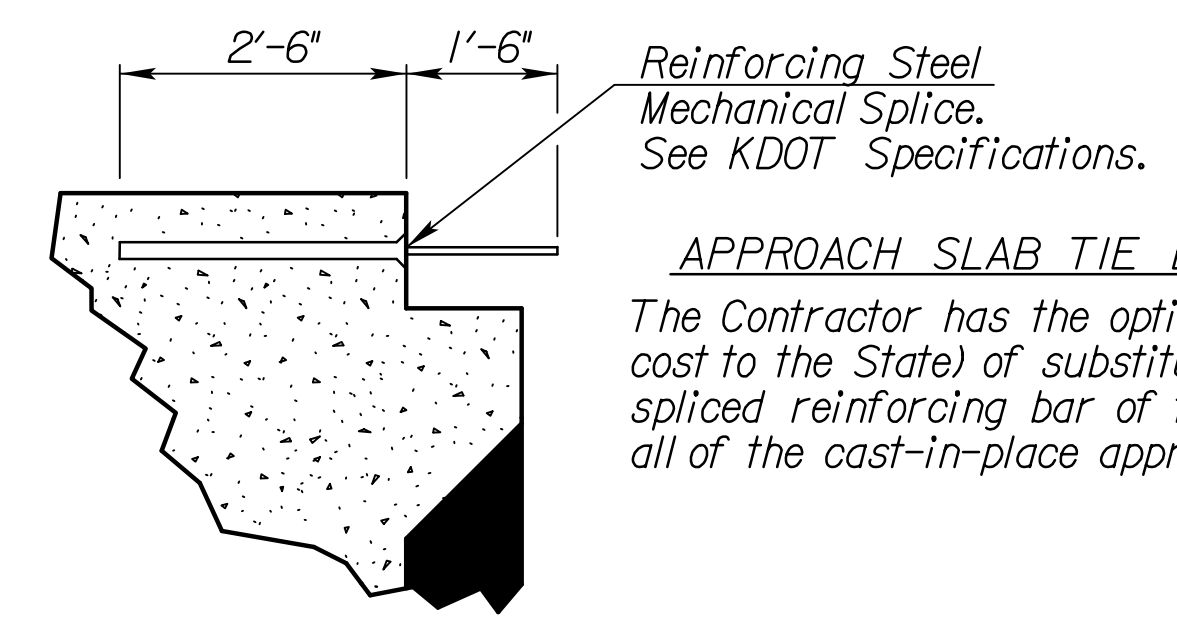
PLAN



TYPICAL SECTION



ELEVATION
(Along Abutment)



APPROACH SLAB TIE BAR OPTION

The Contractor has the option (at no additional cost to the State) of substituting a mechanically spliced reinforcing bar of the same size for any or all of the cast-in-place approach slab tie bars.

*Adjust stirrup to avoid conflict with rail bars.

Note: Top of piling elevations are based on 2'-0" maximum embedment.

Legend
 EF= Each Face

Plotted By: JHOVERSO
 File: c:\pwworking\ventral\01\143371674\k4572801\br113-06.dgn
 Plot Date: 01-08-24

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 99-99-139.68 (113) Sta. 7375+41.16
 ABUTMENT DETAILS
 K-99 OVER DRAGOON CREEK DRAINAGE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
QUANTITIES	JAH	ASF	CADD
TKI	QUAN. CK.	TKI	CADD CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	38	135

GENERAL NOTES

ABUTMENT STRIP DRAIN: The Bridge Contractor shall excavate to the limits shown on the Bridge Excavation sheet, grade the bottom of the backfill area, place the strip drain, and place the perforated pipe, the outlet pipe, the CMP, and the backfill.

BRIDGE BACKWALL PROTECTION SYSTEM: Apply a Bridge Backwall Protection System to the approach side of the abutments and the wings in accordance with KDOT Specifications and the manufacturer's recommendations. Cover the abutments and wings to the limits shown on the details. Prior to backfilling, repair any damage done to the system at no charge to the state.

Place perforated pipe next to the strip drain. Use non-perforated pipe outside the limits of the strip drain. Enclose the perforated pipe with the extension of the filter fabric.

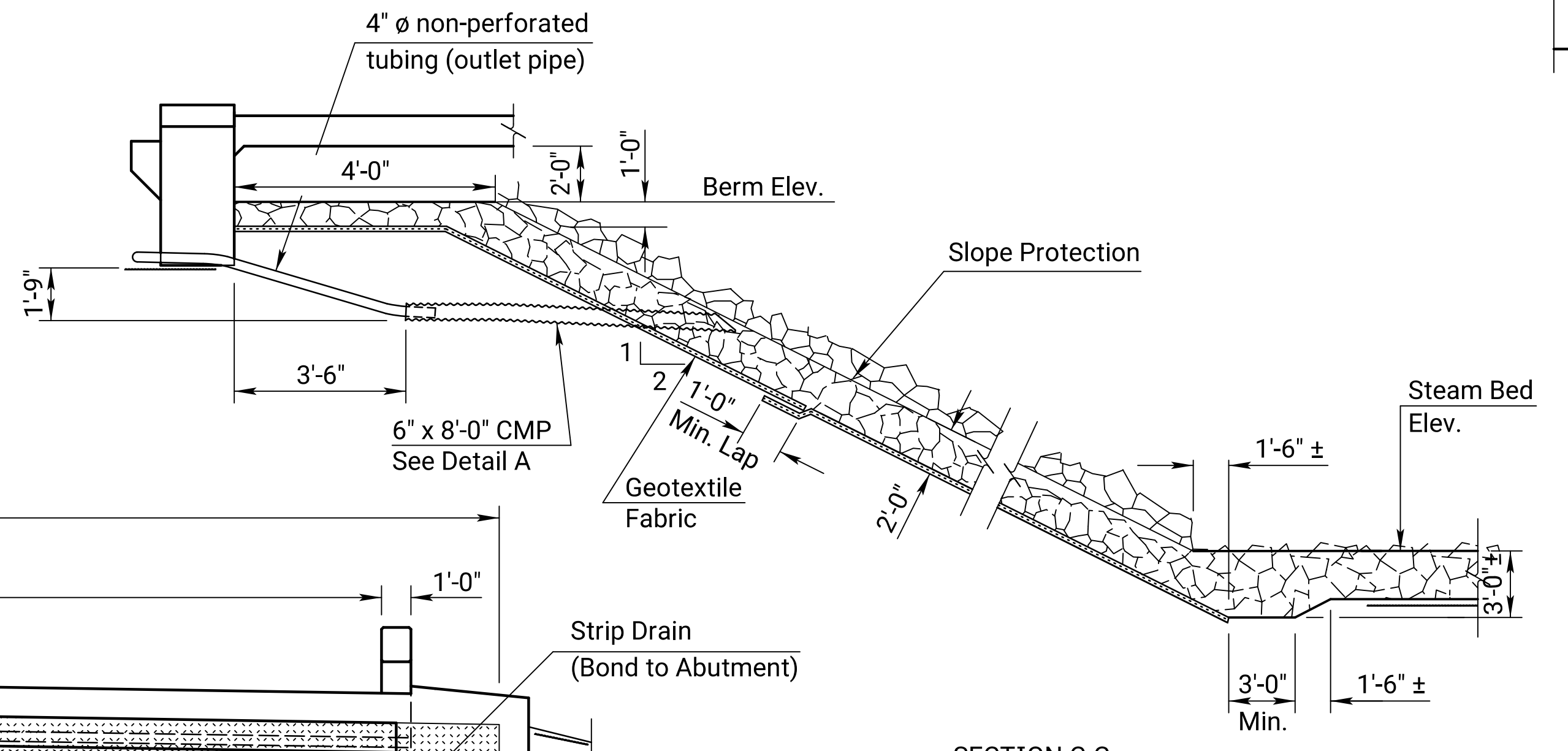
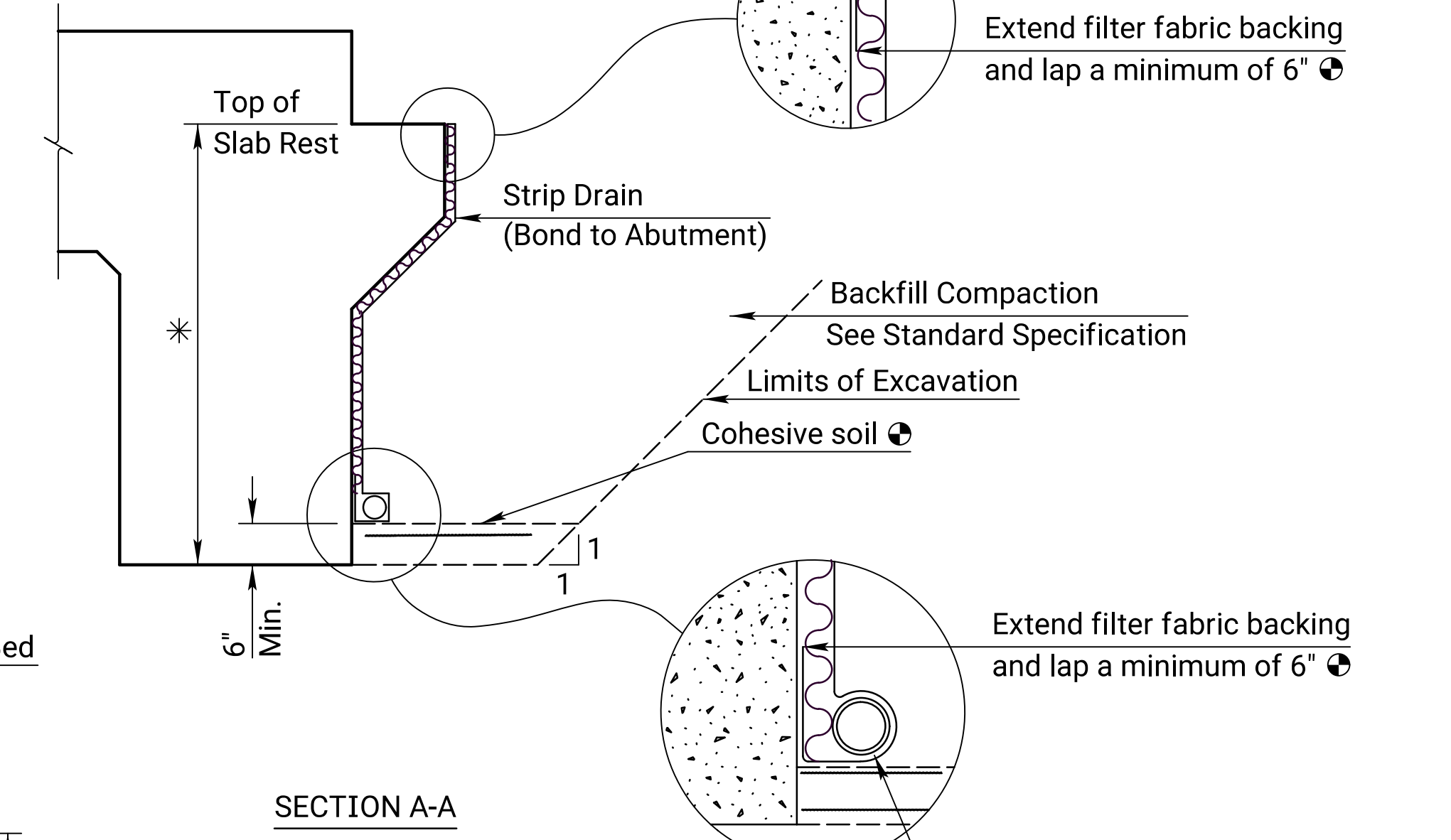
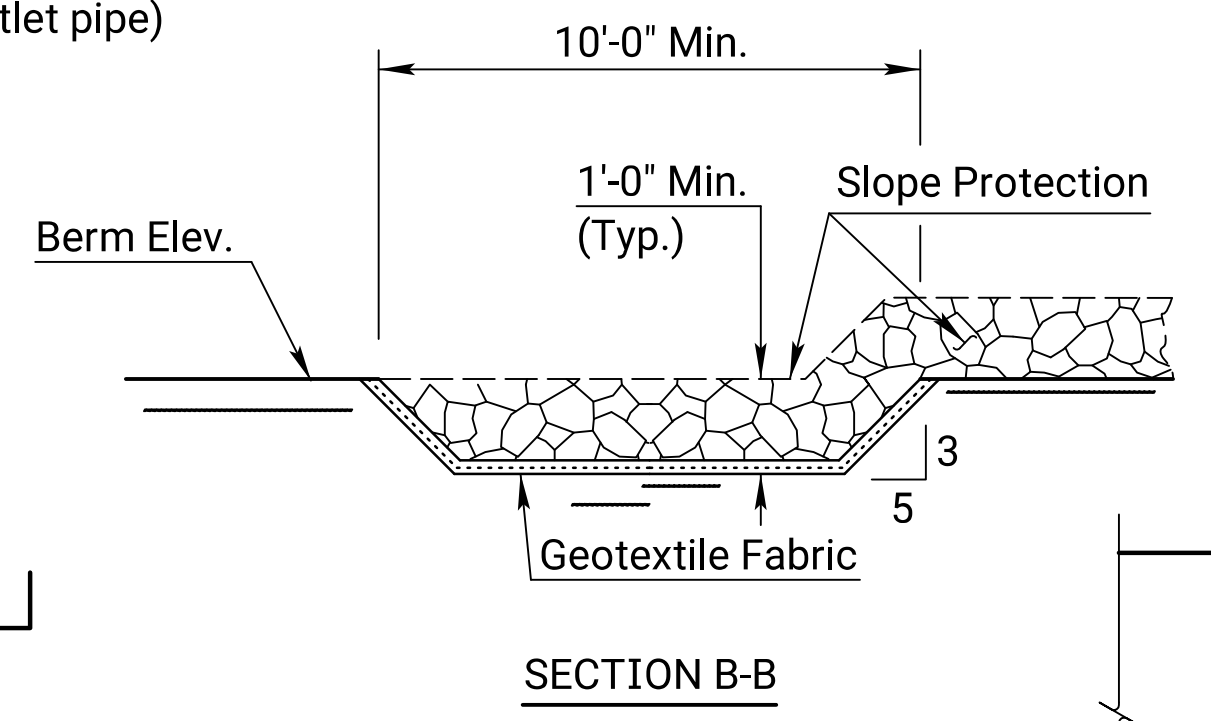
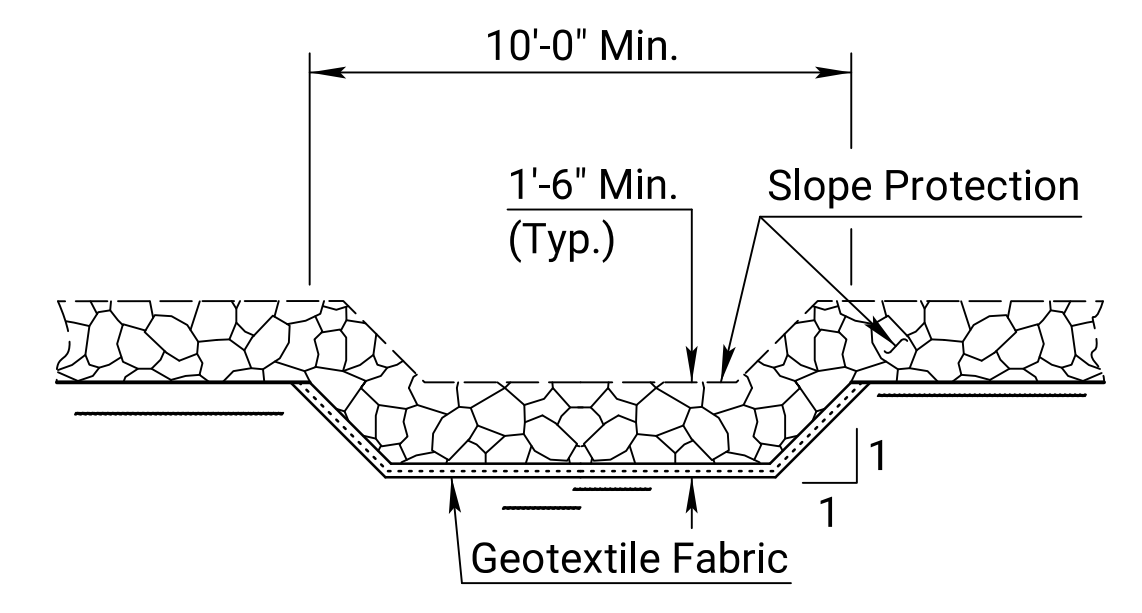
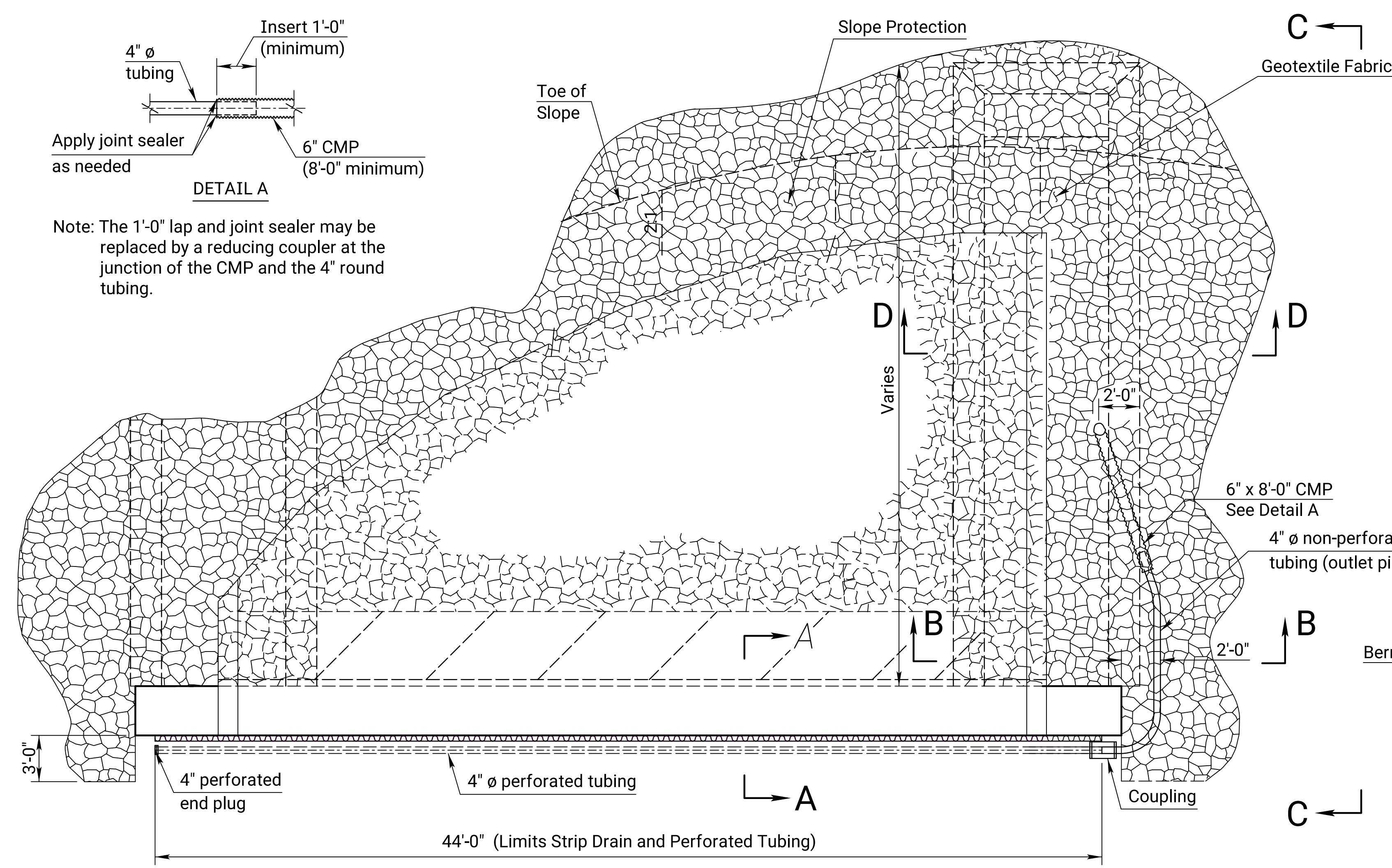
Compact the abutment backfill. See the KDOT Specifications.

Perforated pipe and non-perforated outlet pipe shall be corrugated polyethylene tubing conforming to the KDOT Specifications.

Fit the CMP end section with a ¼" galvanized mesh screen to prevent the entrance of rodents. Seal the joint between the outlet pipe and the end section with a joint sealer.

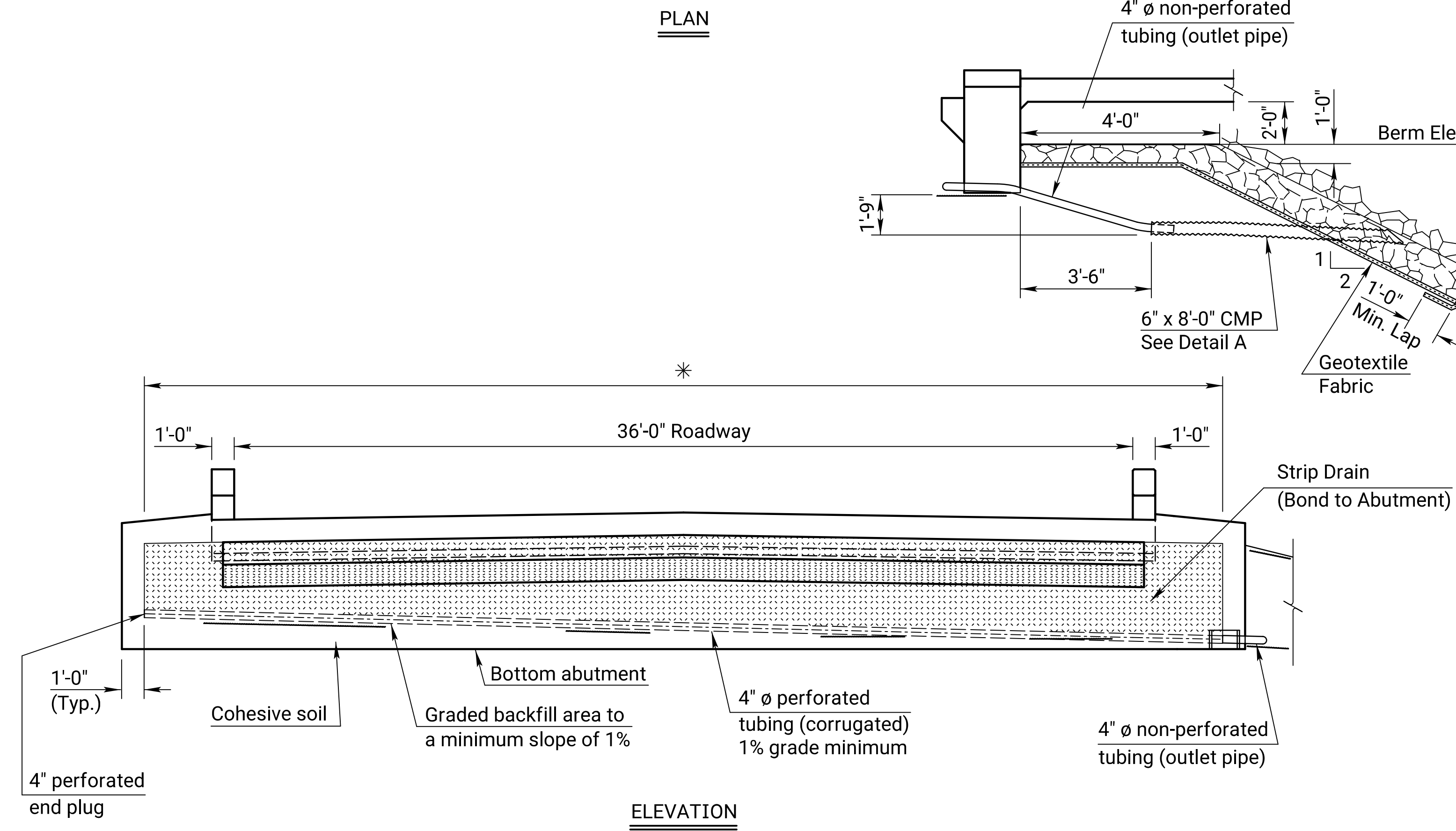
Place the outlet pipe on the downstream side of structures over streams and as shown or noted on other crossings (See the "Construction Layout" sheet).

COHESIVE SOILS: Grade the bottom surface of the excavated area to drain as shown. Backfill this area with a cohesive type of soil. The soil will have a Unified Soil Classification of CL, CH, ML or MH according to ASTM D2487 Classification System with a minimum plasticity index of 13. Compact the material to Type A, MR-90 specifications. If the plasticity index cannot be met, add and mix Bentonite to the soil prior to placement and compaction so that the PI ≥ 13.



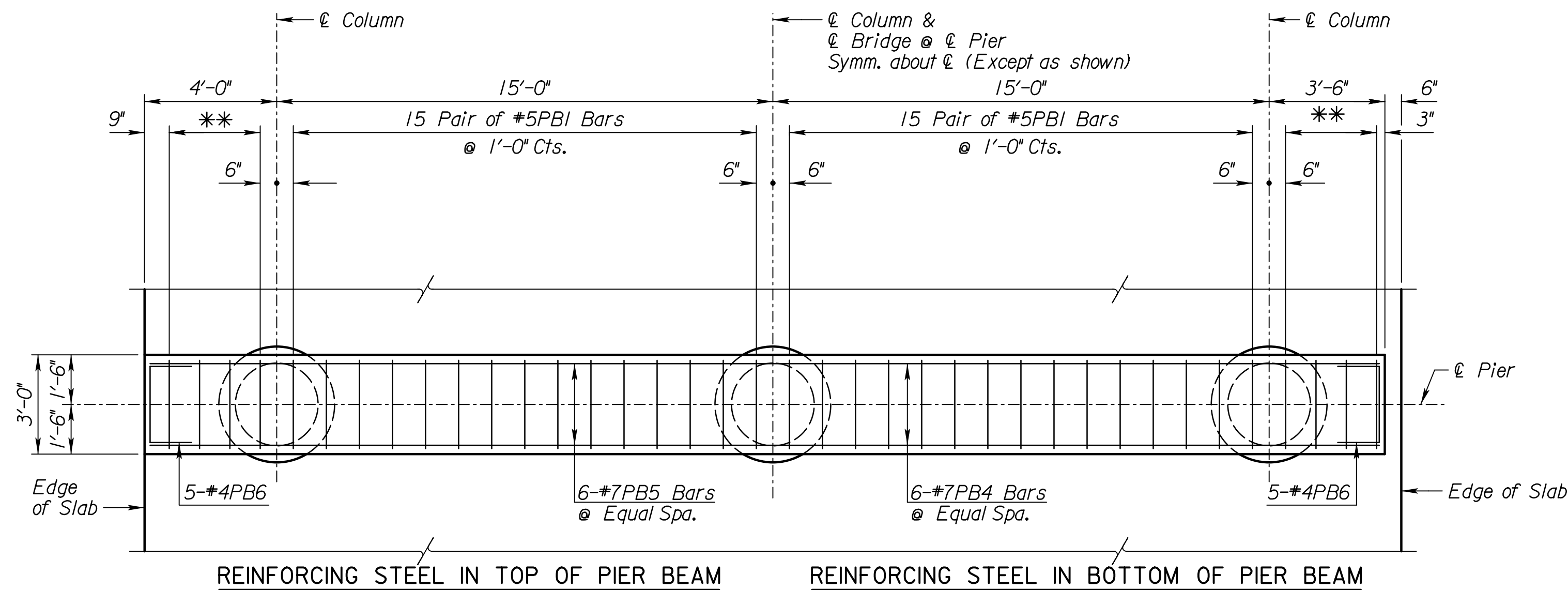
Note: The toe shall extend the entire width of the Slope Protection.

SUMMARY OF QUANTITIES (2 Abutments)	
Abutment Strip Drain	40 Sq. Yds.
Bridge Backwall Protection System	46 Sq. Yds.
Items subsidiary to Strip Drain	
4" ø Perforated Pipe	88 Lin. Ft.
4" ø Outlet Pipe	20 Lin. Ft.
6" ø CMP	16 Lin. Ft.
Items subsidiary to Slope Protection	
Geotextile Fabric	213 Sq. Yds.

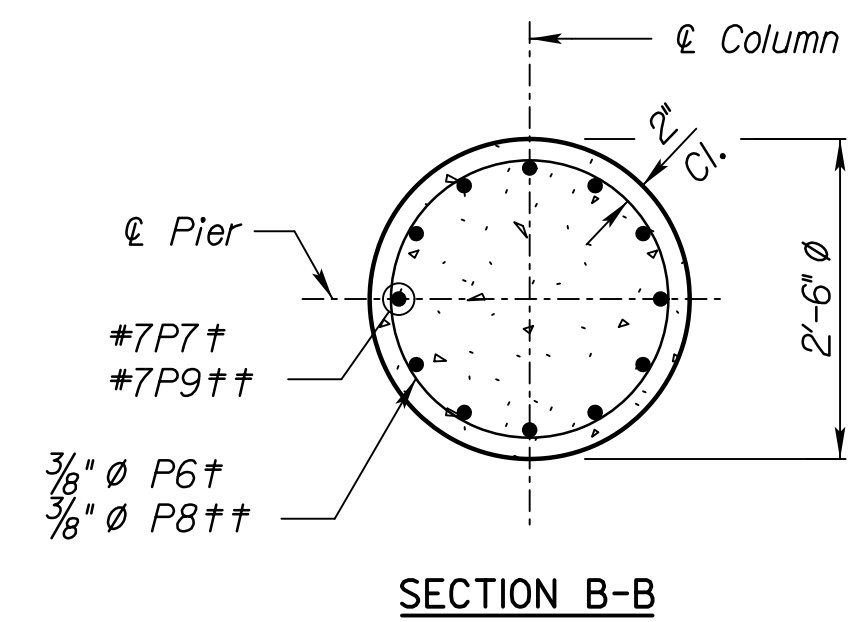


3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 99-99-139.68 (II3)			Sta. 7375+41.16		
ABUTMENT STRIP DRAIN					
K-99 OVER DRAGOON CREEK DRAINAGE					
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	ASF	DETAILED	JAH	QUANTITIES	ASF
DESIGN CK.	TKI	DETAIL CK.	TKI	QUAN. CK.	TKI
			TKI	CADD CK.	JAH

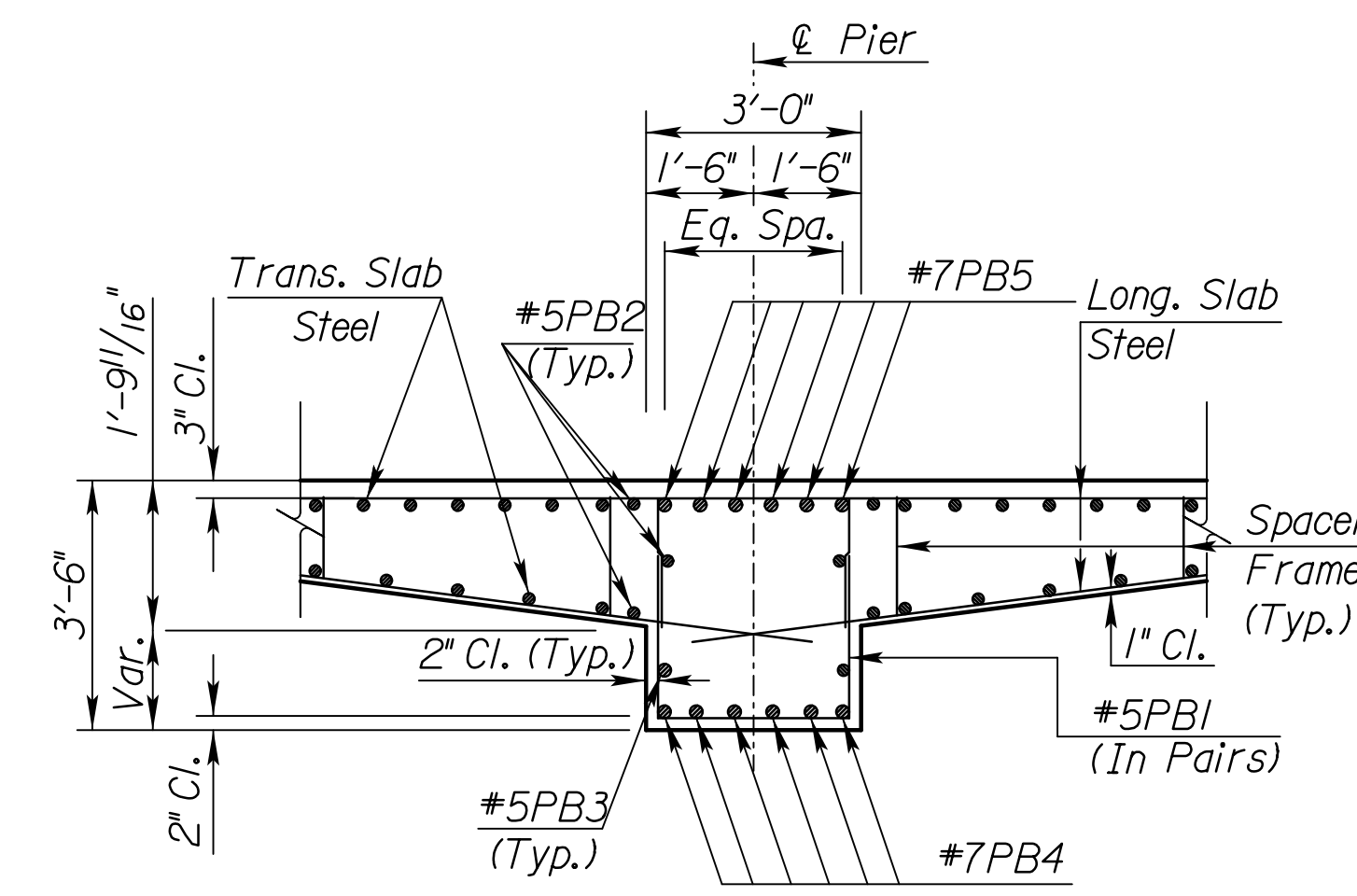
Plotted By: JHOVERSO
 File: c:\pwworking\ventra\0\1\43371674\k572801\br113-07.dgn
 Plot Date: 01-08-24



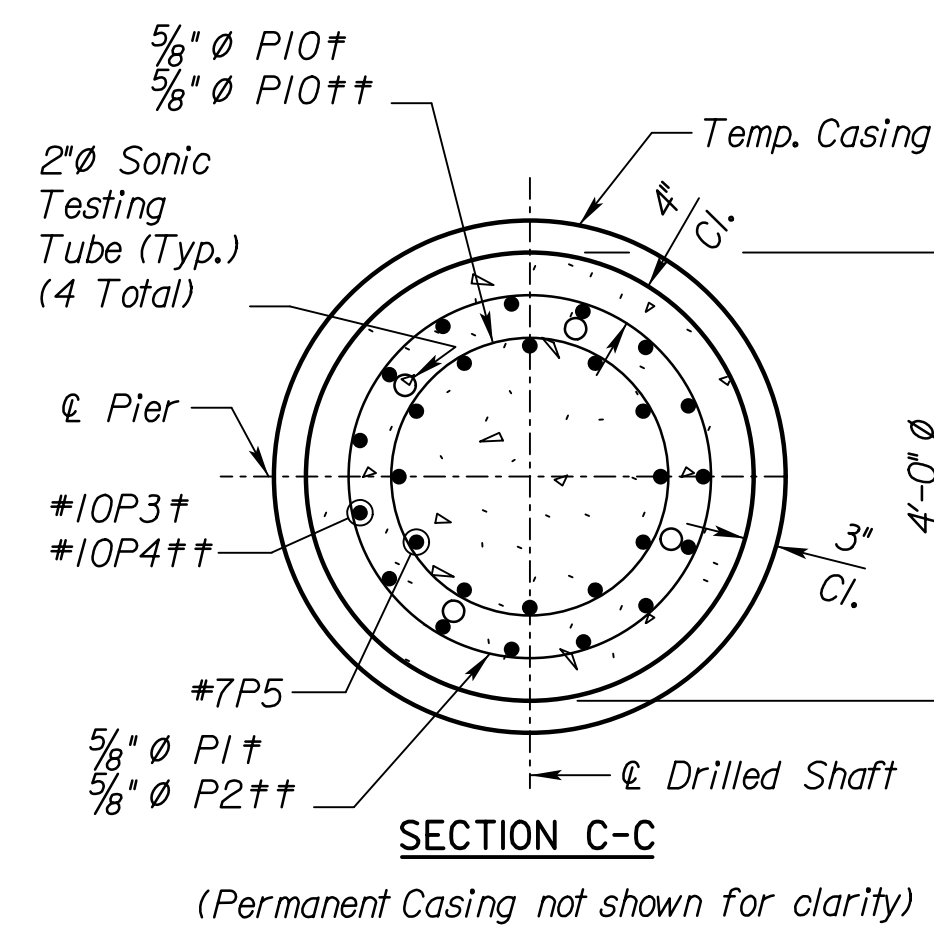
PLAN



SECTION B-B



SECTION A-A



SECTION C-C

(Permanent Casing not shown for clarity)

Notes:
Pier Beam Reinforcement shall be epoxy coated and is included in the Superstructure Quantities.

Column Reinforcement shall be non-coated and is included in the Substructure Quantities.

Concrete and reinforcement in the drilled shaft shall not be paid for directly, but shall be included in the bid item "Drilled Shaft (48") (Cased)".

Reinforcing Steel in the drilled shaft shall be non-coated.

Construct the drilled shafts using the cased method. A Permanent casing is required.

* - Perpendicular to ℓ Bridge.

+ - Pier No. 1.

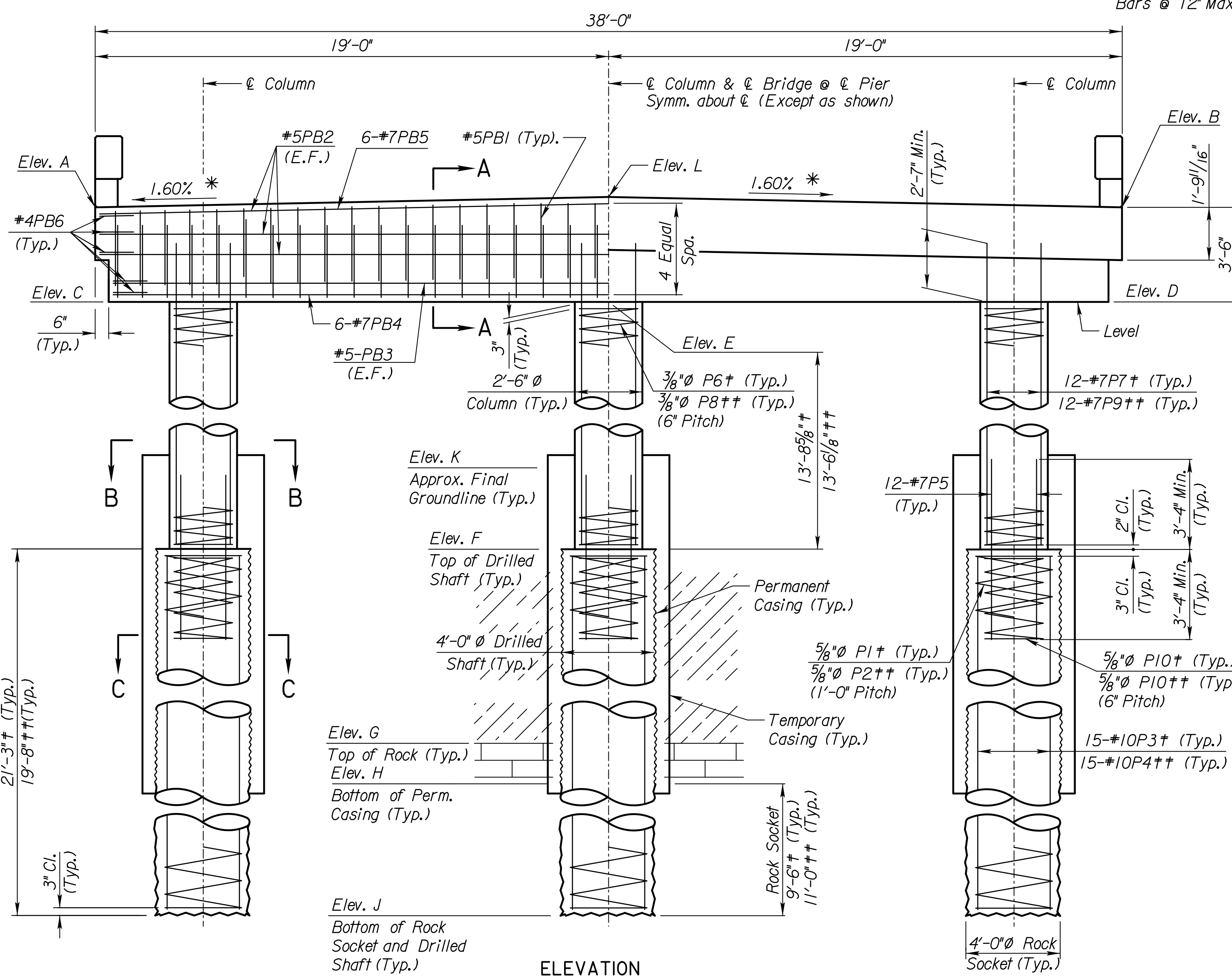
++ - Pier No. 2.

E.F. = Each Face.

Note:
For type of Sonic Testing Tubes, see KDOT Specifications and the Special Provisions. Sonic Testing Tubes shall be spaced equally and extended 1'-0" above top of shaft.

DRILLED SHAFT SUMMARY		
Design Loads		
Pier No.	Maximum Drilled Shaft Load: Tons	Factored Resistance (Rr)
1	270	331
2	270	331

Drilled Shaft Note:
Maximum Drilled Shaft Load is based on Strength I Loads-no Impact.



ELEVATION

TABLE OF ELEVATIONS AT ℓ PIER		
Location	Pier No. 1	Pier No. 2
A	1313.47	1311.68
B	1313.47	1311.68
C	1309.97	1308.18
D	1309.97	1308.18
E	1309.97	1308.18
F	1296.25	1294.67
G	1285.20	1286.70
H	1284.50	1286.00
J	1275.00	1275.00
K	1299.27	1297.64
L	1313.77	1311.98

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 99-99-139.68 (113) S+a. 7375+41.16

PIER DETAILS
K-99 OVER DRAGOON CREEK DRAINAGE
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
QUANTITIES	ASF	CADD	JAH
TKI	QUAN. CK.	TKI	CADD CK.

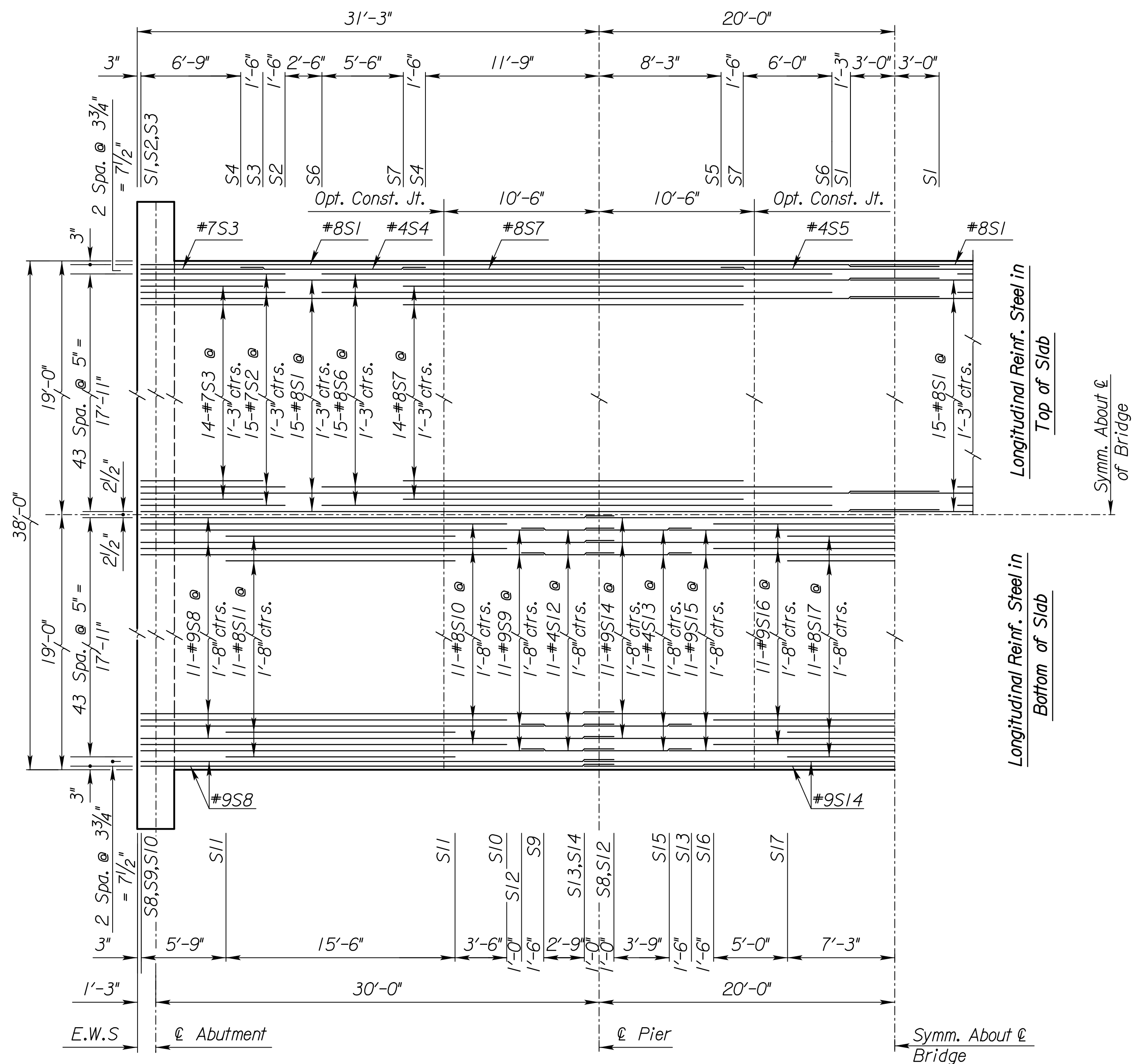
Plotted By: JHOVERSO
File: c:\pwworking\ventral\0\1\43371674\k6572801\br113-08.dgn
Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	40	135

Plot 2	Longest Span Length = 40'
roadway width = 36'	Total No. of Spans = 3
Skew and Direction = 0	Loading = HL-93
Routing Type = Corral	

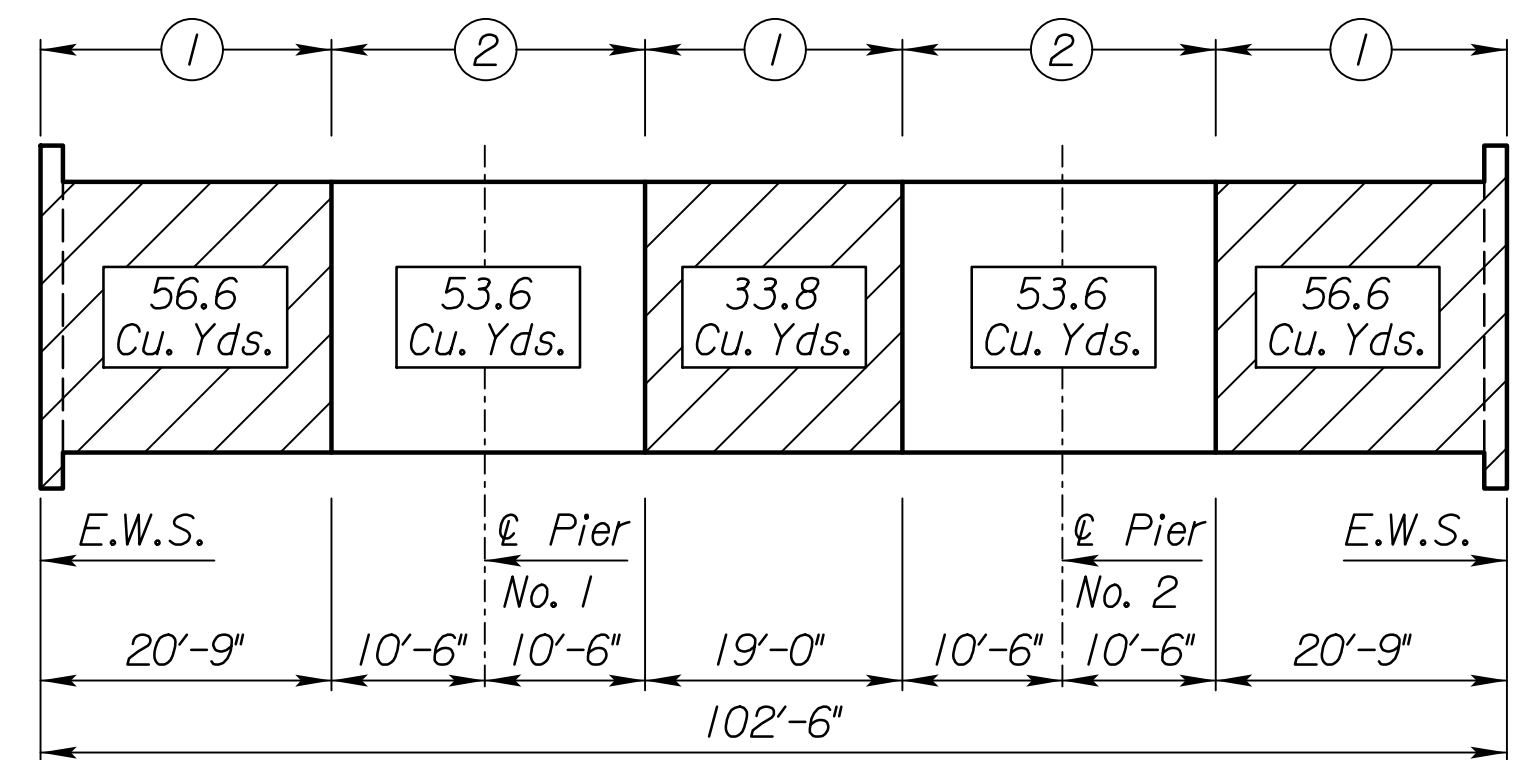
LFD & LRFR RATING FACTOR	
Truck	3
HS-20	1.93
HT	1.31
HL-93	1.70
LRFR	2.20

Note to designer: Do not remove this information



HALF PLAN

Note: See longitudinal section for transverse reinforcing steel.



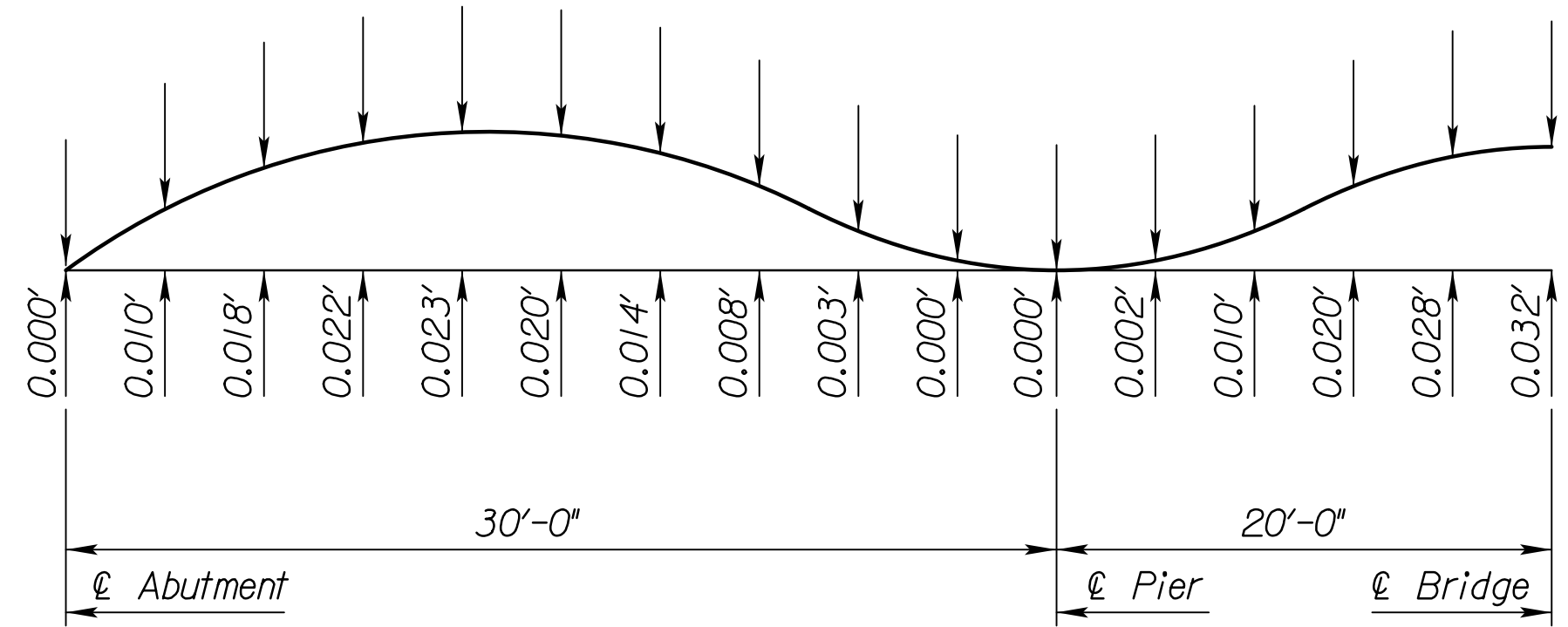
CONCRETE PLACING SEQUENCE DIAGRAM

CONCRETE PLACING SEQUENCE

When long span steel beams having a concrete dead load deflection greater than 1/4" are used or when timber falsework with greater than 12'-0" clear span is used, follow the placing sequence shown. Segmental, combined or continuous pours are allowed, but stop a discontinuous pour at a construction joint short of a pier.

When timber falsework with 12'-0" or less clear span is used, the Contractor, subject to the approval of the Engineer, may use a continuous pour or may discontinue the pour at any construction joint shown.

The Contractor may place the corral rail continuously from one end of the bridge to the other.



DEAD LOAD CAMBER DIAGRAM AT TENTH POINTS

Long Term Deflections = Initial Deflections x 3.5
 (Initial Deflections Based on $E_c = 3.644 \times 10^6$ p.s.i.)
 (camber values in feet)

Note: 1.0 & 4.0 pts. are taken at Δ of abutments
 2.0 & 3.0 pts. are taken at Δ of piers

Top of Form Elevation at 10th Points, (ft.)															
1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5
1314.06	1313.91	1313.77	1313.62	1313.47	1313.28	1313.06	1312.81	1312.53	1312.22	1311.88	1311.95	1311.97	1311.93	1311.84	1311.69
2.6	2.7	2.8	2.9	3.0	3.1	3.2	3.3	3.4	3.5	3.6	3.7	3.8	3.9	4.0	
1311.49	1311.22	1310.90	1310.54	1310.10	1310.16	1310.20	1310.21	1310.20	1310.15	1310.07	1309.96	1309.84	1309.71	1309.59	

Note: Elevations are taken at Crown Grade. Note: The change in elevation from Crown Grade to the Edge of Slab is -0.304'

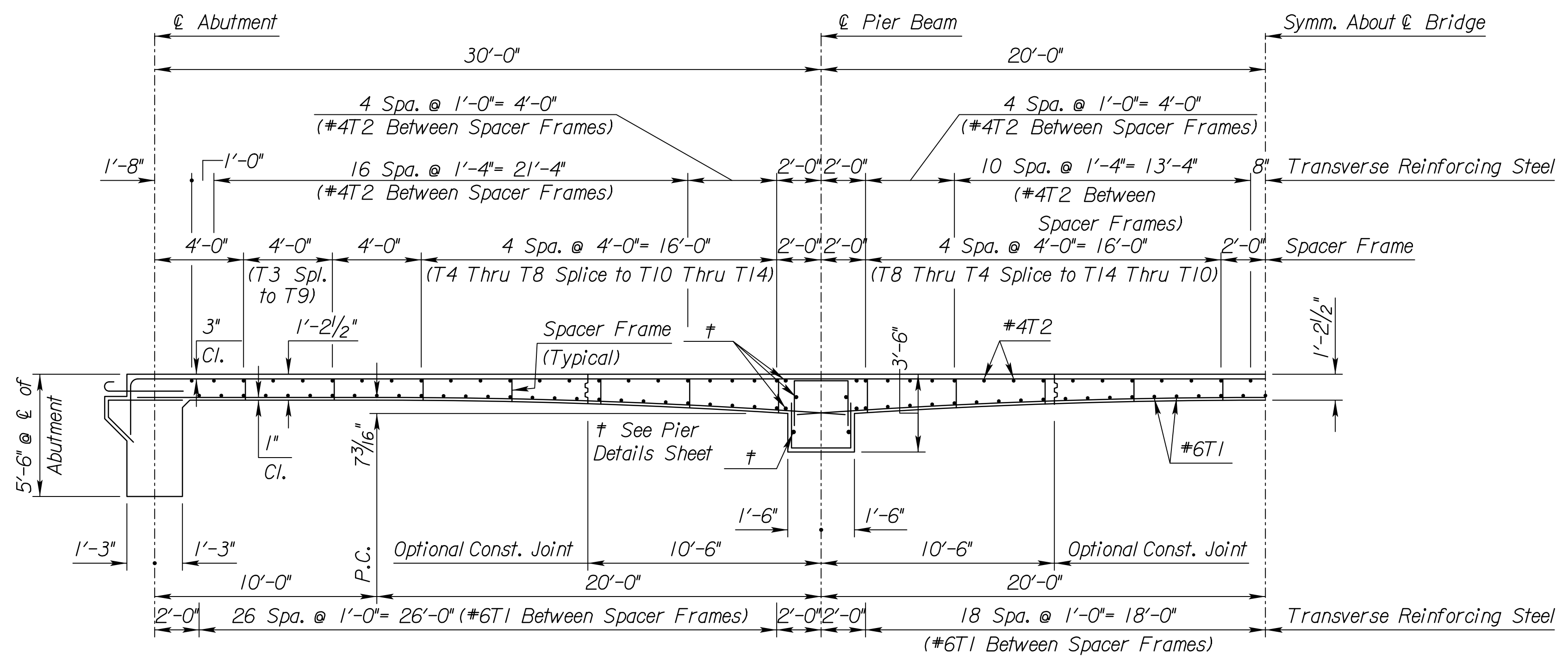
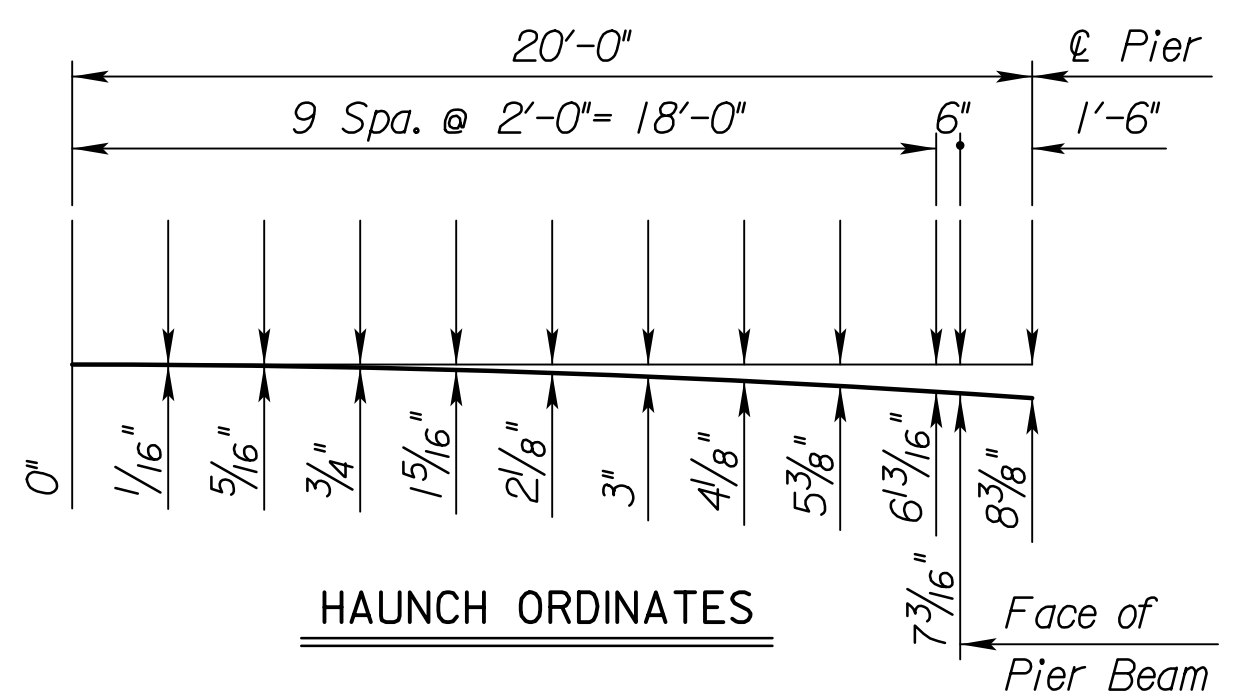
Plotted By: JHOVERSO
 File: c:\pwworking\central\4337674\k572801\br113-09.dgn
 Plot Date: 01-08-24

3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION Br. No. 99-99-139.68 (113) Sta. 7375+41.16 SUPERSTRUCTURE DETAILS (1 OF 2) K-99 OVER DRAGOON CREEK DRAINAGE Proj. No. 99-99 KA-5728-01 Wabaunsee Co.				
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	ASF	DETAILED	JAH	QUANTITIES
DESIGN CK.	TKI	DETAIL CK.	TKI	QUAN. CK.
			ASF	CADD
			TKI	CADD

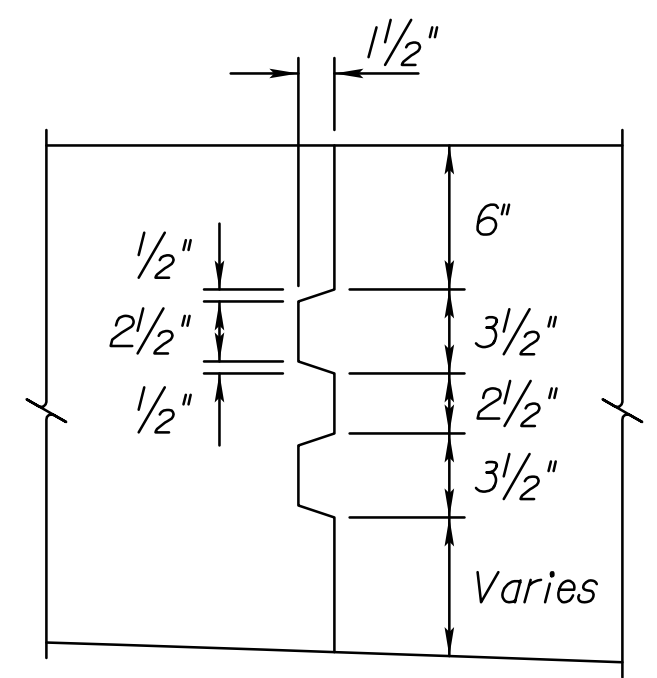
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	41	135

Plot 3
 Longest Span Length = 40'
 Total No. of Spans = 3
 Railing Type = Corral
 Roadway Width = 36'
 Skew and Direction = 0
 Loading = HL-93

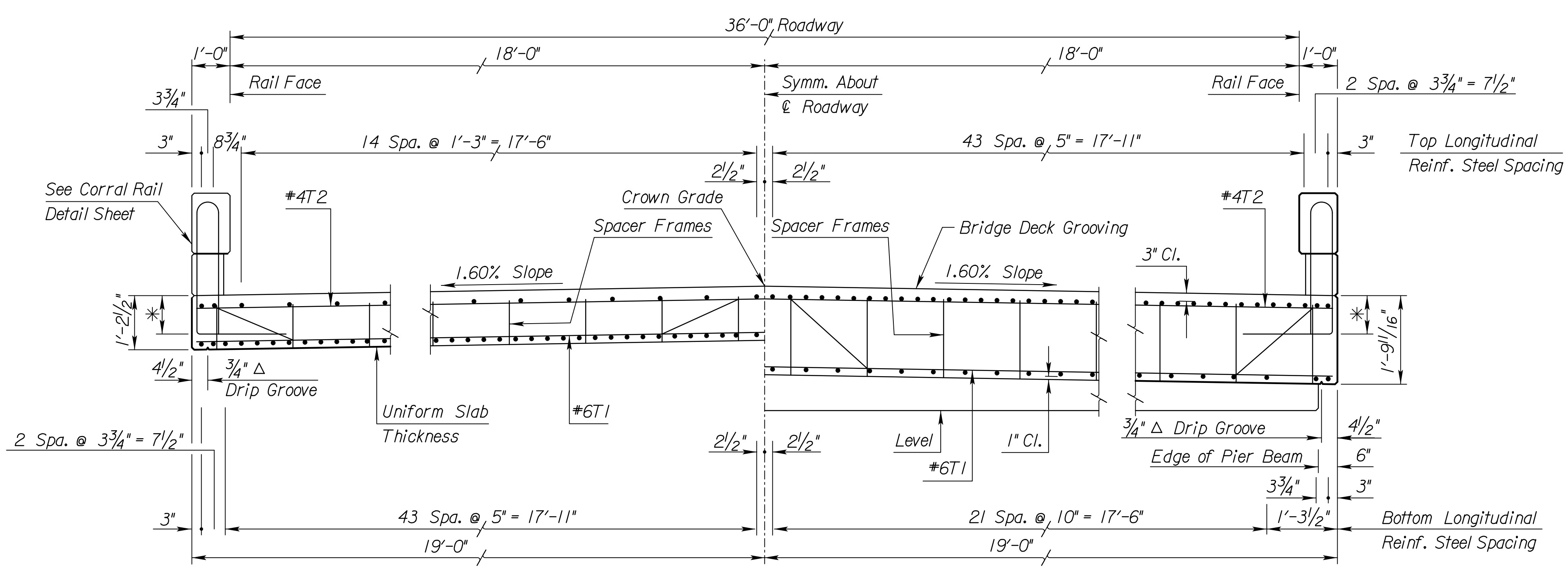
Plot Location:
 File: c:\pwworking\central\01\4337674\k4572801\br113-10.dgn
 Plot Date: 01-08-24



HALF LONGITUDINAL SECTION ALONG C BRIDGE



TRANSVERSE SLAB CONSTRUCTION JOINT (Optional)



Half Section Near Mid-Span TYPICAL SECTION OF SLAB Half Section at Face of Pier Beam

* See Corral Rail Detail Sheet.

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 99-99-139.68 (113) Sta. 7375+41.16
 SUPERSTRUCTURE DETAILS (2 OF 2)
 K-99 OVER DRAGON CREEK DRAINAGE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
		QUANTITIES	ASF
		TKI	CADD
		TKI	CADD
		TKI	CADD

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	42	135

SLAB ELEVATIONS

		SLAB ELEVATIONS													
		Formwork				Screed			Thickness			Deck Profile			
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Survey	Station	† Location	Transverse Location	Estimated Falsework Crush	Target Elevation TOF	Actual Elevation TOF	TOF Variance (QA/QC)	Target Screed El. = TOC El.	Actual Bottom of Screed Elevation Prior to Pour	Screed Variance (QA/QC)	Plan Deck Thickness	Measured Deck Thickness	Deck Thickness Variance (QA/QC)	Plan TOC El.	Actual TOC El. Optional Survey
	(1)(16)	(13)	(13)	(inch) (1)(4)	(1)(6)	(2)	(± inch) (2)(5)	(1)(6)	(2)	(± inch) (2)(7)	(inch) (1)	(inch) (2)(8)	(± inch) (2)(9)	(1)	Date: (3)
A	7374+91.16	℄ Brg. of Abut. #1	Left Fascia	/	/	/	/	1314.94	/	/	/	/	/	1314.94	/
			Crown Gr.	/	/	/	/	1315.24	/	/	/	/	1315.24	/	
			Right Fascia	/	/	/	/	1314.94	/	/	/	/	/	1314.94	/
B	7374+92.41	Interior Face of Abut. #1	Left Fascia	/	1313.67	/	/	/	/	/	14 1/2"	/	/	1314.88	/
			Crown Gr.	/	1313.97	/	/	/	14 1/2"	/	/	1315.18	/		
			Right Fascia	/	1313.67	/	/	/	14 1/2"	/	/	1314.88	/		
C	7375+03.16	4/10 Point from Abut. #1	Left Fascia	1/4	1313.16	/	/	1314.38	/	/	14 9/16"	/	/	1314.34	/
			Crown Gr.	1/4	1313.47	/	/	1314.68	/	/	14 9/16"	/	1314.64	/	
			Right Fascia	1/4	1313.16	/	/	1314.38	/	/	14 9/16"	/	1314.34	/	
D	7375+19.66	Span #1 Face of Pier Beam	Left Fascia	1/4	1311.75	/	/	/	/	/	21 11/16"	/	/	1313.54	/
			Crown Gr.	1/4	1312.05	/	/	/	21 11/16"	/	/	1313.84	/		
			Right Fascia	1/4	1311.75	/	/	/	21 11/16"	/	/	1313.54	/		
E	7375+21.16	℄ Brg. of Pier #1	Left Fascia	/	/	/	/	1313.47	/	/	/	/	/	1313.47	/
			Crown Gr.	/	/	/	/	1313.77	/	/	/	/	1313.77	/	
			Right Fascia	/	/	/	/	1313.47	/	/	/	/	1313.47	/	
F	7375+22.66	Span #2 Face of Pier Beam	Left Fascia	1/4	1311.61	/	/	/	/	/	21 11/16"	/	/	1313.39	/
			Crown Gr.	1/4	1311.91	/	/	/	21 11/16"	/	/	1313.70	/		
			Right Fascia	1/4	1311.61	/	/	/	21 11/16"	/	/	1313.39	/		
G	7375+41.16	Midpoint of Span #2	Left Fascia	1/4	1311.39	/	/	1312.60	/	/	14 1/2"	/	/	1312.55	/
			Crown Gr.	1/4	1311.69	/	/	1312.90	/	/	14 1/2"	/	1312.85	/	
			Right Fascia	1/4	1311.39	/	/	1312.60	/	/	14 1/2"	/	1312.55	/	
H	7375+59.66	Span #2 Face of Pier Beam	Left Fascia	1/4	1309.95	/	/	/	/	/	21 11/16"	/	/	1311.74	/
			Crown Gr.	1/4	1310.26	/	/	/	21 11/16"	/	/	1312.05	/		
			Right Fascia	1/4	1309.95	/	/	/	21 11/16"	/	/	1311.74	/		
I	7375+61.16	℄ Brg. of Pier #2	Left Fascia	/	/	/	/	1311.68	/	/	/	/	/	1311.68	/
			Crown Gr.	/	/	/	/	1311.98	/	/	/	/	1311.98	/	
			Right Fascia	/	/	/	/	1311.68	/	/	/	/	1311.68	/	
J	7375+62.66	Span #3 Face of Pier Beam	Left Fascia	1/4	1309.83	/	/	/	/	/	21 11/16"	/	/	1311.61	/
			Crown Gr.	1/4	1310.13	/	/	/	21 11/16"	/	/	1311.92	/		
			Right Fascia	1/4	1309.83	/	/	/	21 11/16"	/	/	1311.61	/		
K	7375+79.16	4/10 Point from Abut. #2	Left Fascia	1/4	1309.77	/	/	1310.98	/	/	14 9/16"	/	/	1310.94	/
			Crown Gr.	1/4	1310.07	/	/	1311.29	/	/	14 9/16"	/	1311.24	/	
			Right Fascia	1/4	1309.77	/	/	1310.98	/	/	14 9/16"	/	1310.94	/	
L	7375+89.91	Interior Face of Abut. #2	Left Fascia	/	1309.31	/	/	/	/	/	14 1/2"	/	/	1310.52	/
			Crown Gr.	/	1309.61	/	/	/	14 1/2"	/	/	1310.82	/		
			Right Fascia	/	1309.31	/	/	/	14 1/2"	/	/	1310.52	/		
M	7375+91.16	℄ Brg. of Abut. #2	Left Fascia	/	/	/	/	1310.47	/	/	/	/	/	1310.47	/
			Crown Gr.	/	/	/	/	1310.77	/	/	/	/	1310.77	/	
			Right Fascia	/	/	/	/	1310.47	/	/	/	/	1310.47	/	

Pour Dates (2)	
	Deck
	Left Rail (13)
	Right Rail (13)

Survey Data (1)(11)	
Bench Mark No.	Elevation
10	1350.28
11	1302.52
12	1306.98
13	1317.29

Crown Grade Profile (1)(12)	
7376+45.00	VPI Station
1297.46	VPI Elevation
-8.53	G1 %
2.23	G2 %
850.00	L in Stations

Slab Thickness (1)		Span Data (1)	
14 1/2	Uniform Depth (inch)	HL-93	Design Loading
7 3/16	Haunch Depth @ Face of PB (inch)	30	Span #1 (ft)
		40	Span #2 (ft)
1/16	Haunch Depth @ 0.4 Point (inch)	3	Clear Cover (inch)

Roadway Data (1)(10)(13)	
38	Deck Width (ft) (14)
-1.6	% Slope Left (±)
-1.6	% Slope Right (±)
00:00:00	Skew (dd:mm:ss)

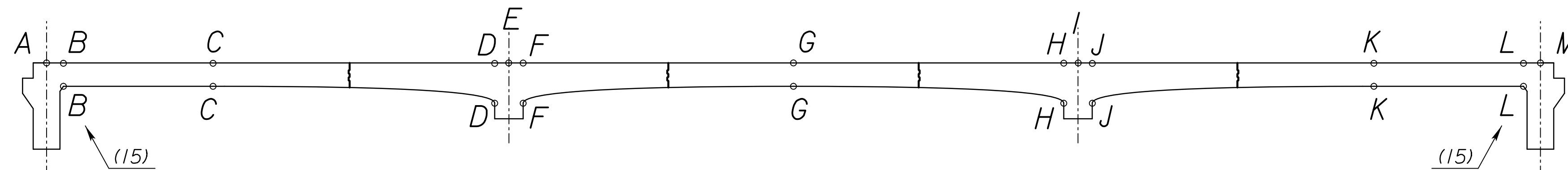
Camber (1)(17)	
0.023	Span #1 0.4 Point (ft)
0.031	Span #2 Midspan (ft)

- (1) By the Design Engineer
- (2) By the Contractor
- (3) By Request
- *(4) Estimated crush for typical falsework. Revise estimate if/when more accurate information becomes available.
- (5) (col 7 - col 6)x12
- (6) Crush (Take Up) and camber must be included
- (7) (col 10 - col 9)x12
- (8) (col 10 - col 7)x12
- (9) (col 13 - col 12)
- (10) If transition falls on the bridge, then enter "Varies" for the % Slope
- (11) From "Construction Layout" sheet
- (12) If bridge is not on the vertical curve, enter Abutment #1 ℄ bearing elevation from the "Construction Layout" sheet. Represent a change in grade with G1 only.
- (13) Looking Up-Station
- (14) Out-to-Out
- (15) Ignore Fillet
- (16) Non-skewed bridges only require ℄ stations.
- (17) Ignore theoretical camber at face of pier beams.

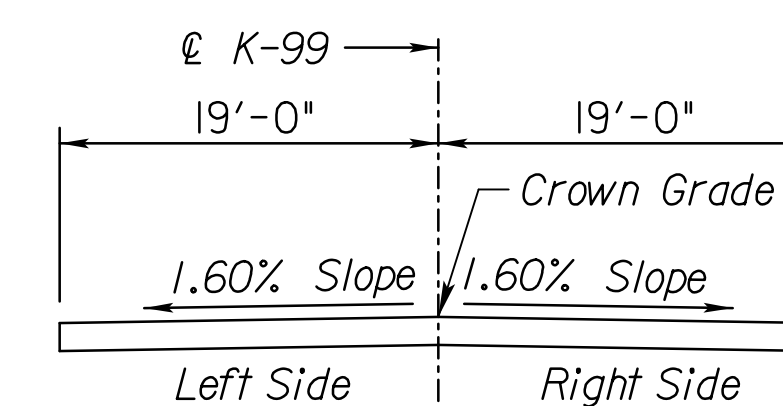
† Stationing shown increasing

NOTE: The Contractor will submit a completed copy of this table to the Field Engineer to be inserted into the As-Built plan set.

* It is assumed that piling have been driven to design bearing and checked by ENR formula (QA/QC). No allowance for pile settlement is included in crush.



ELEVATION OF SLAB



TYPICAL SECTION (Looking Up-Station)

Legend
 TOF = Top of Formwork
 TOC = Top of Concrete
 QA = Quality Assurance
 QC = Quality Control

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

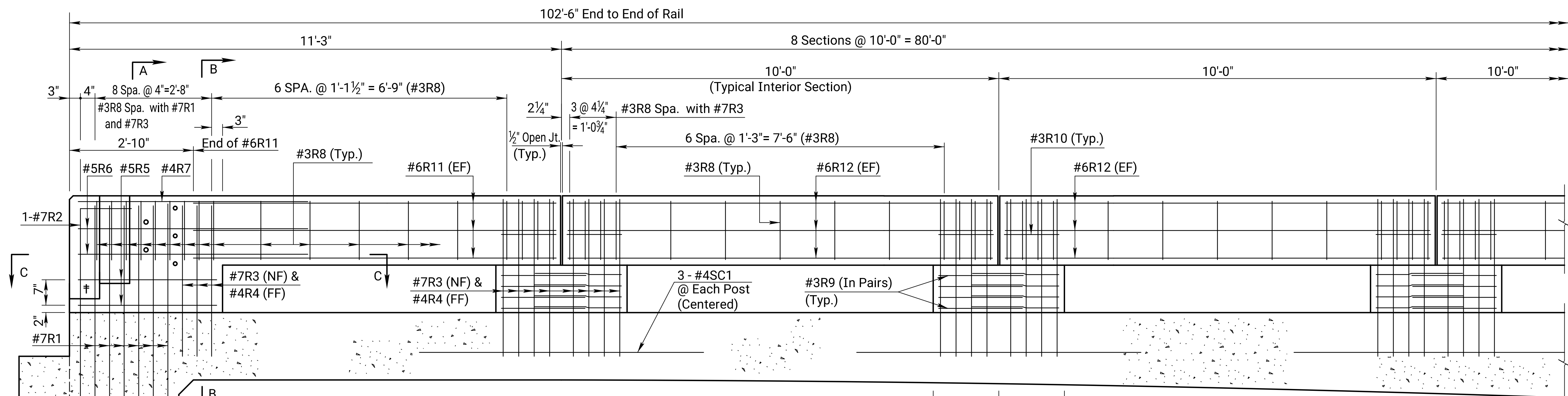
KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 99-99-139.68 (113) Sta. 7375+41.16

SLAB ELEVATIONS
 K-99 OVER DRAGOON CREEK DRAINAGE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

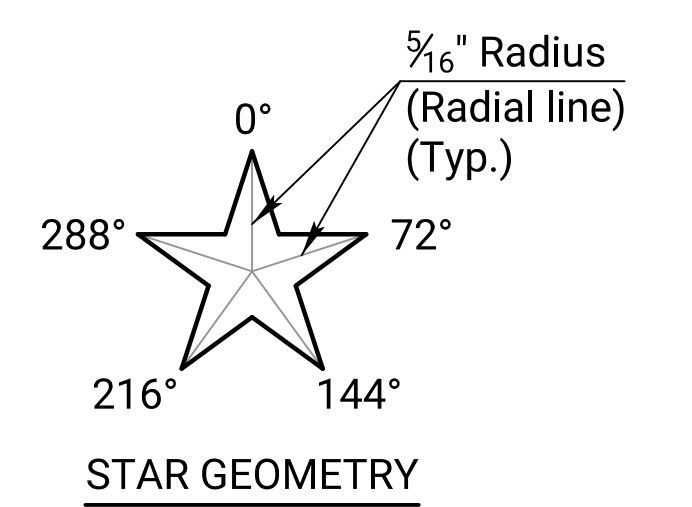
SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
		QUANT. CK.	ASF
		CADD CK.	TKI
		CADD CK.	TKI

Plotted By: JHOVERSO
 File: c:\pwworking\central\01\4337674\k6572801\br113-11.dgn
 Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	43	135



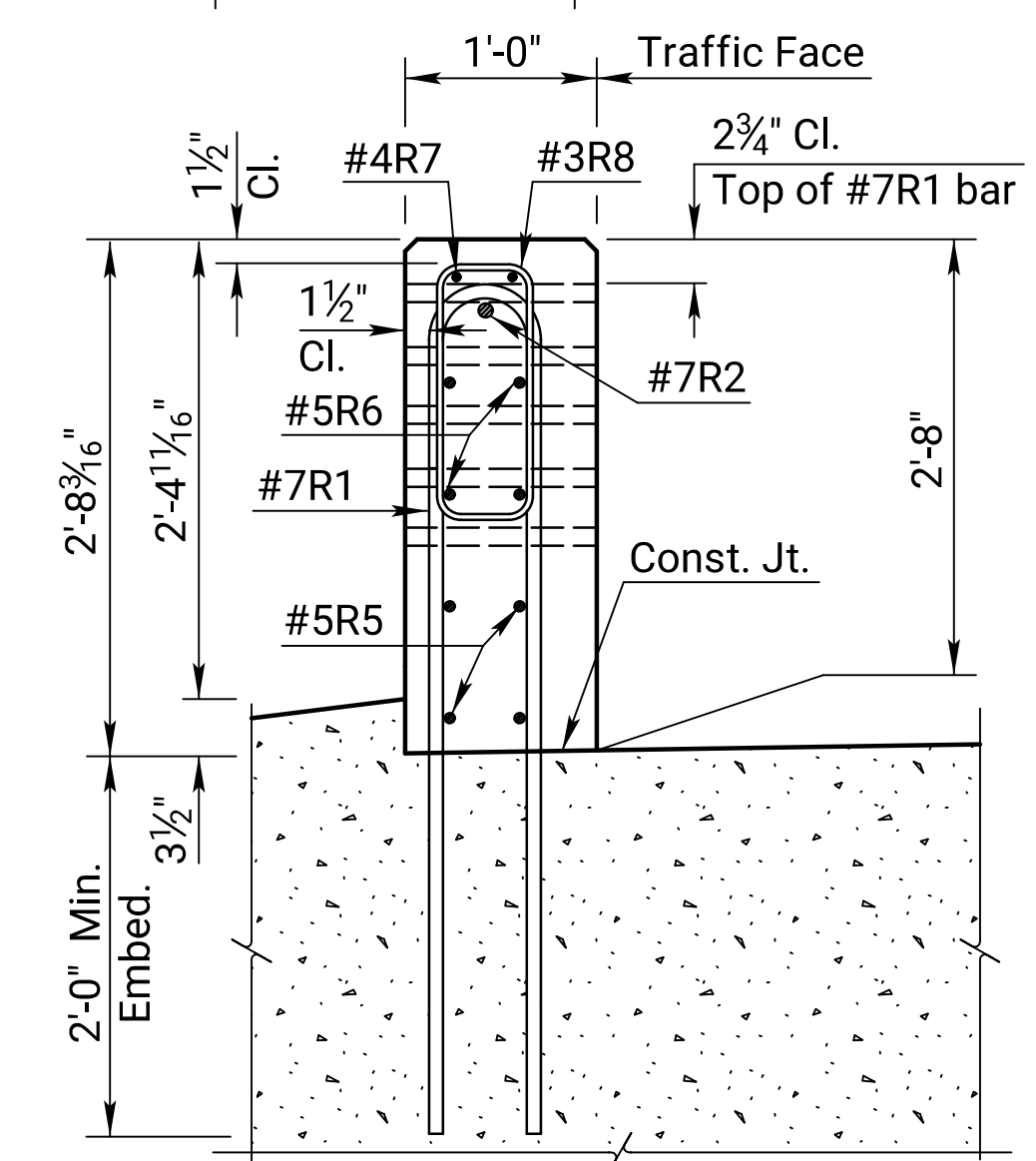
LEGEND
 NF = Near Face
 FF = Far Face
 EF = Each Face



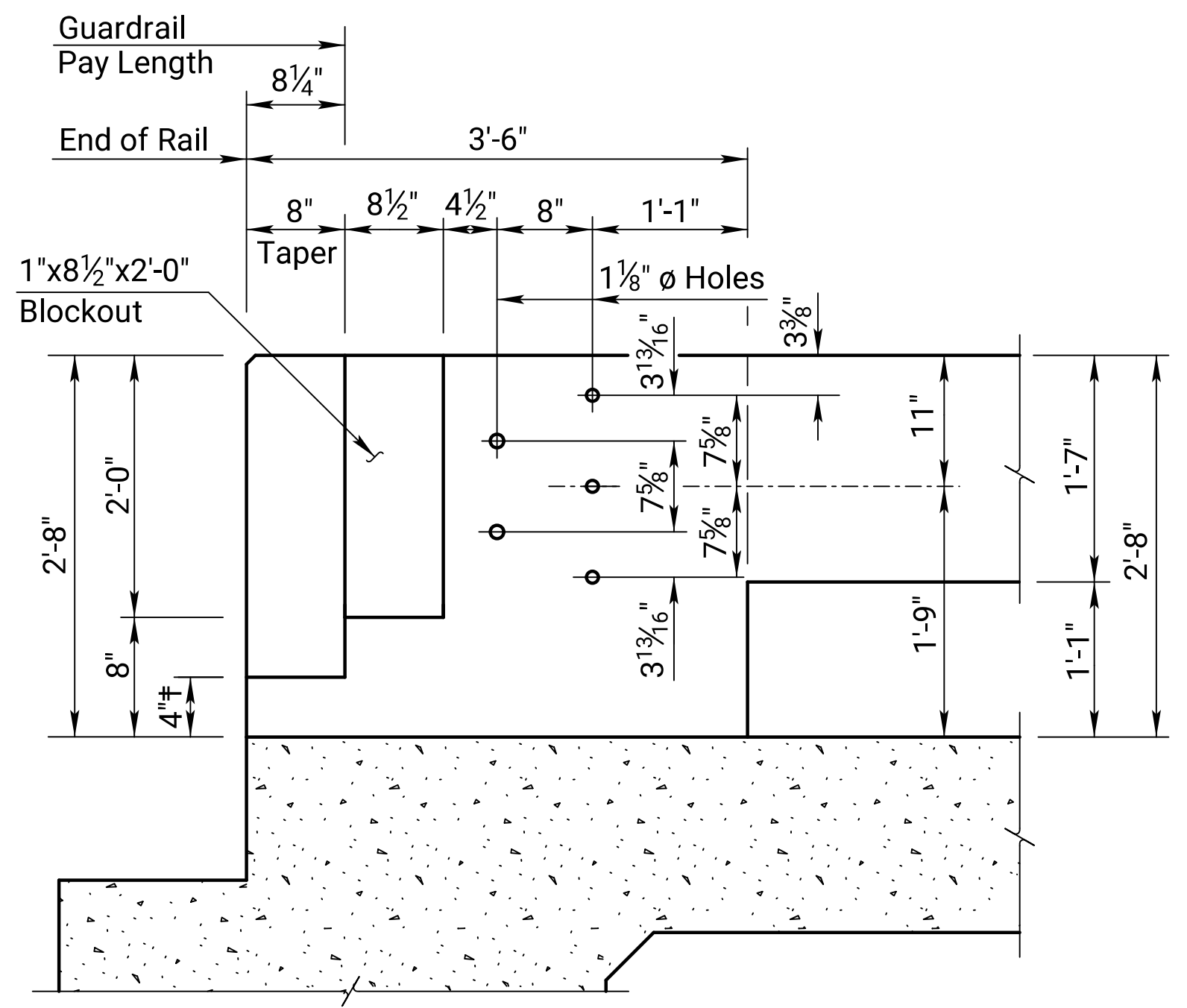
† Extend the 12:3 taper to the top of the approach slab curb.

PARTIAL ELEVATION
(Along Traffic Face)

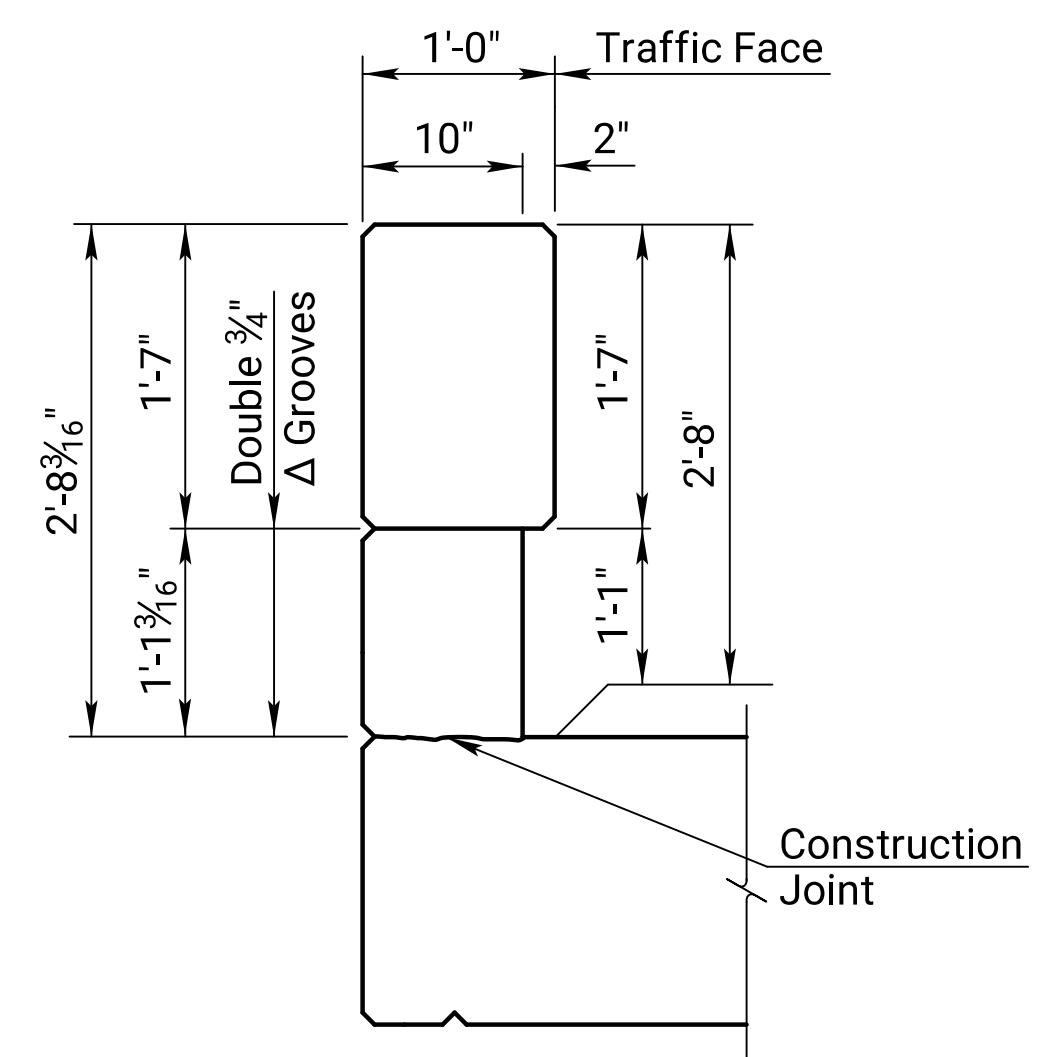
Typical Interior Post



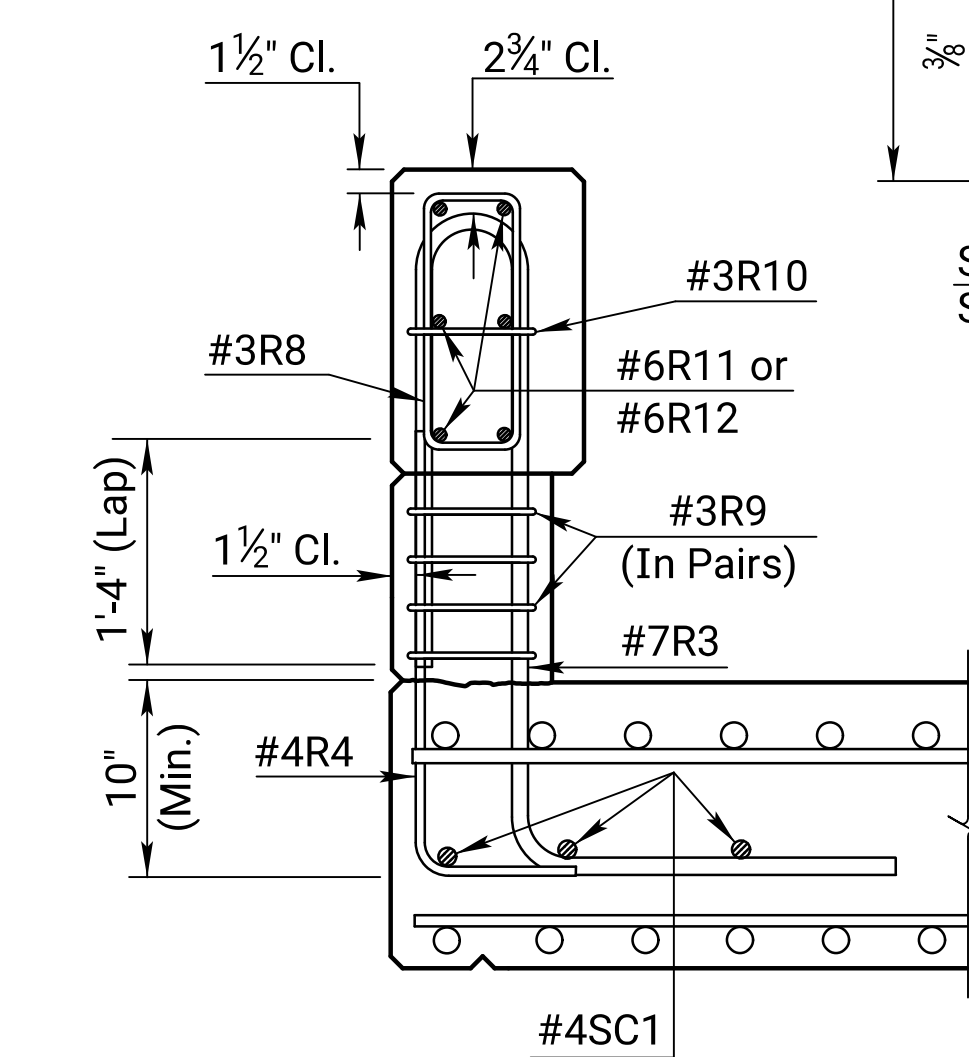
SECTION A-A



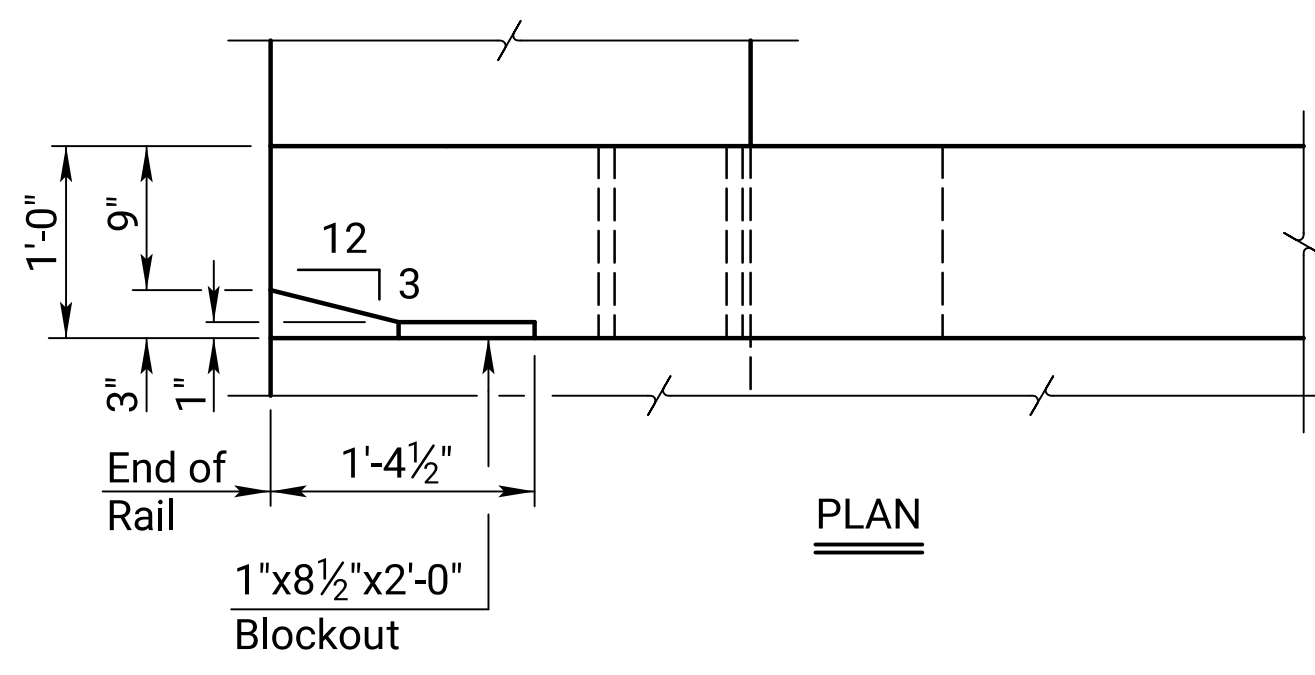
ELEVATION
(Dimensions at traffic face of rail.)



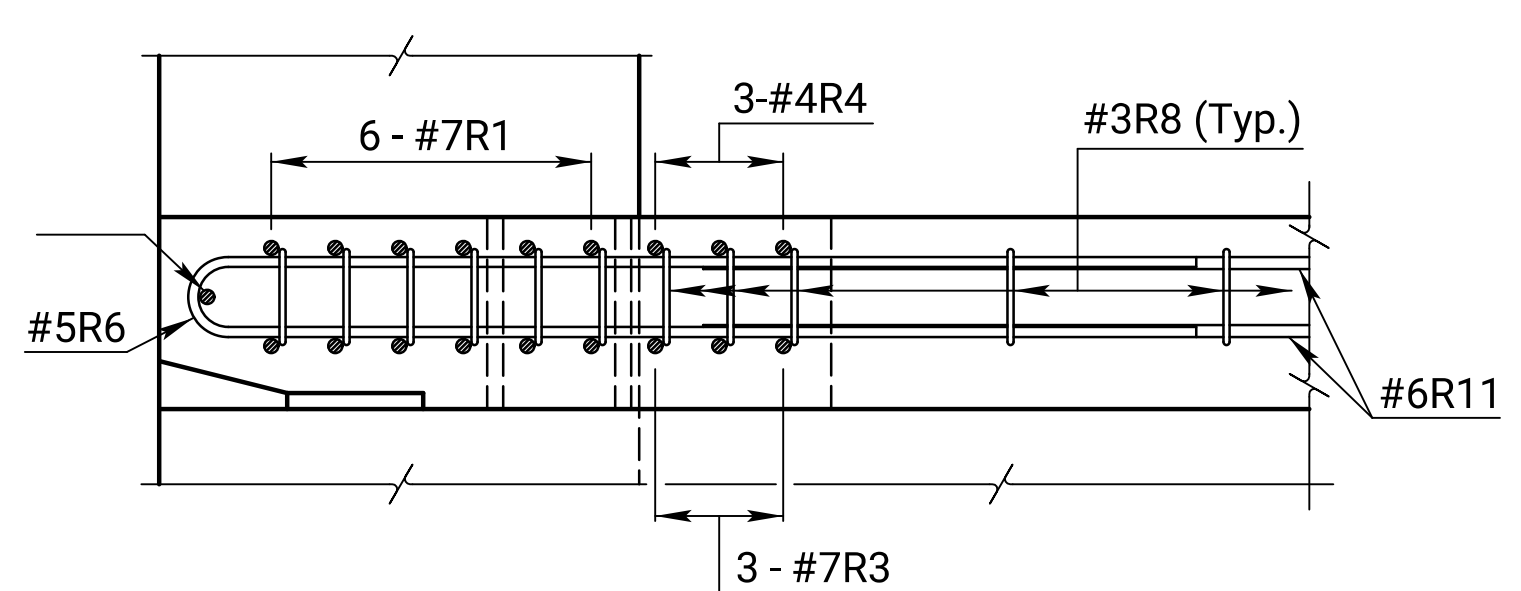
TYPICAL INTERIOR POST



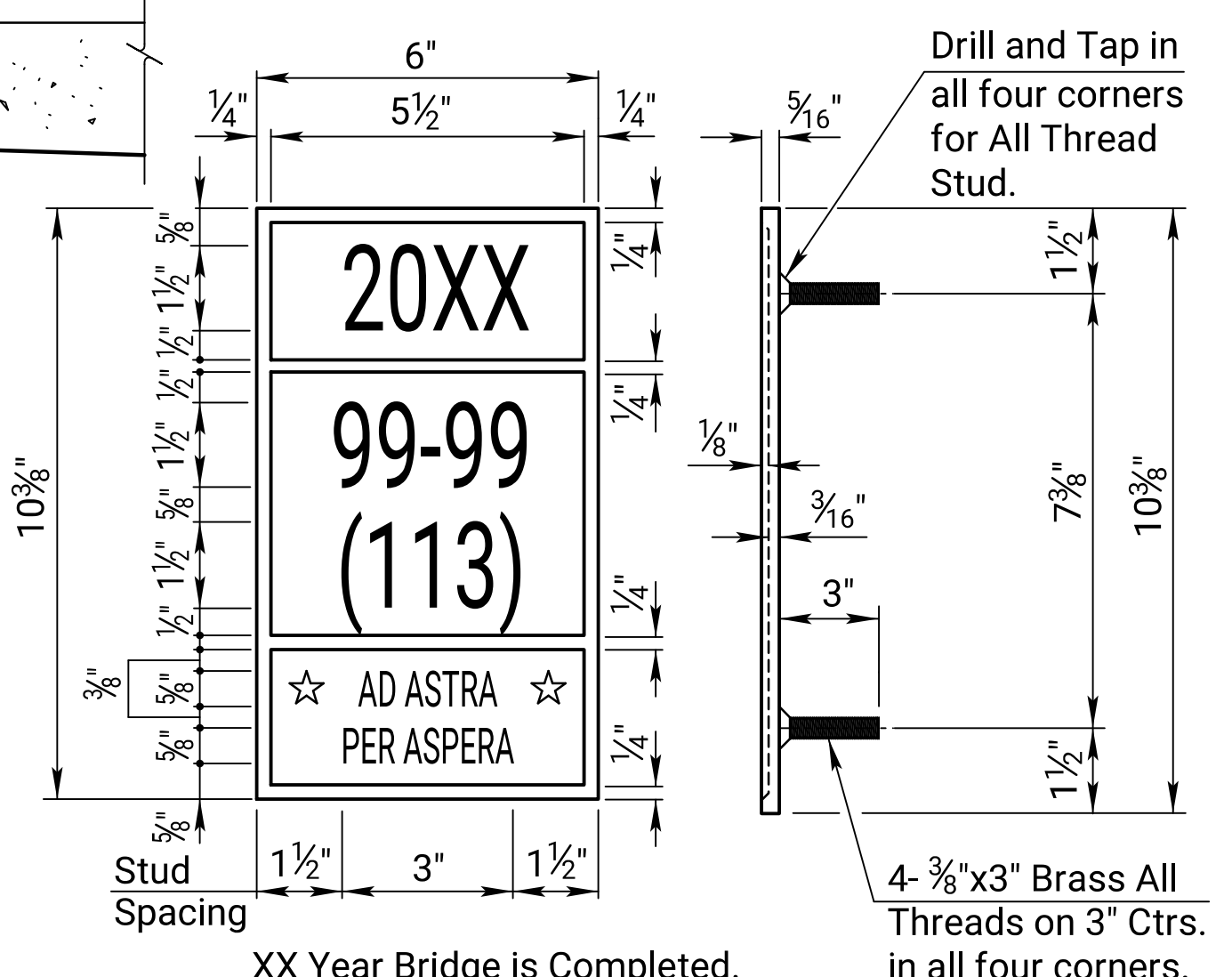
SECTION THRU POST



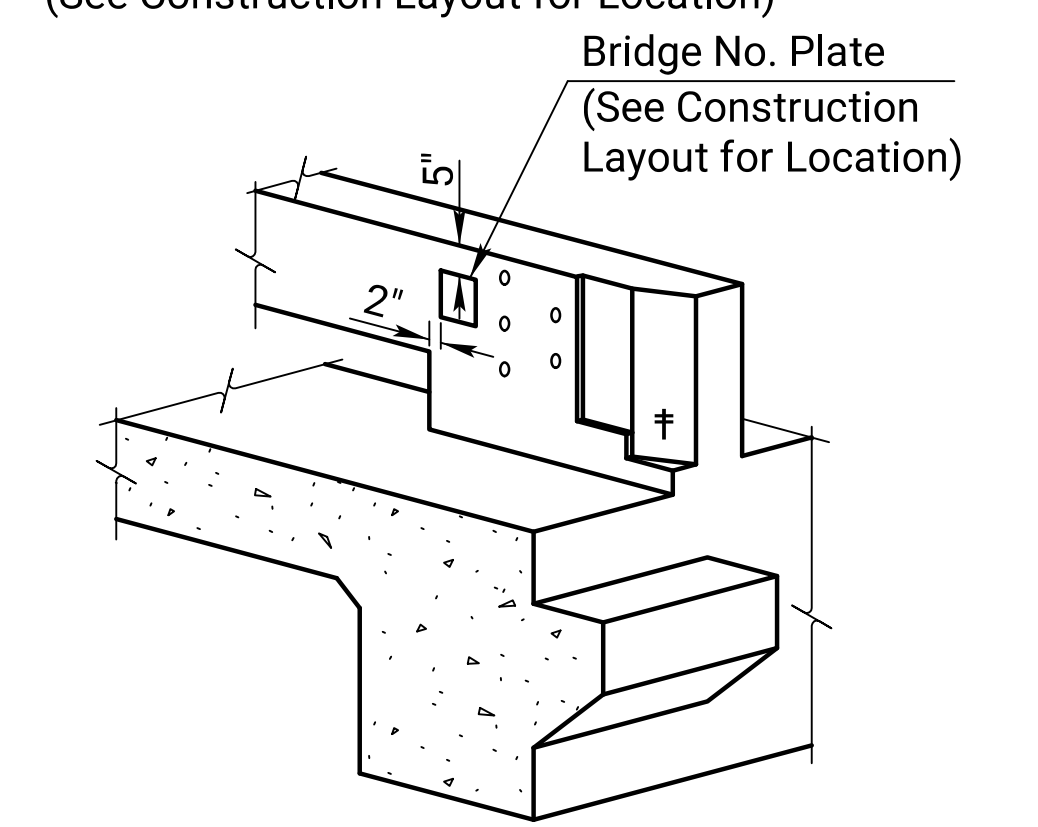
PLAN



SECTION C-C



BRIDGE NUMBER PLATE
(1 Required)
(See Construction Layout for Location)



BRIDGE NUMBER PLATE PLACEMENT DETAIL

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 99-99-139.68 (113) S+a. 7375+41.16
 32" KANSAS CORRAL RAIL (WITHOUT CURB)
 K-99 OVER DRAGOON CREEK DRAINAGE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	ASF	DETAILED	JAH
DESIGN CK.	TKI	DETAIL CK.	TKI
QUANTITIES	JAH	ASF	CADD
TKI	QUAN. CK.	TKI	CADD CK.

Plotted By: JHOVERSO
 File: c:\pwworking\ventral\0\1\43371674\k572801\br113-12.dgn
 Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	44	135

BILL OF REINFORCING STEEL
Epoxy Coated - Grade 60

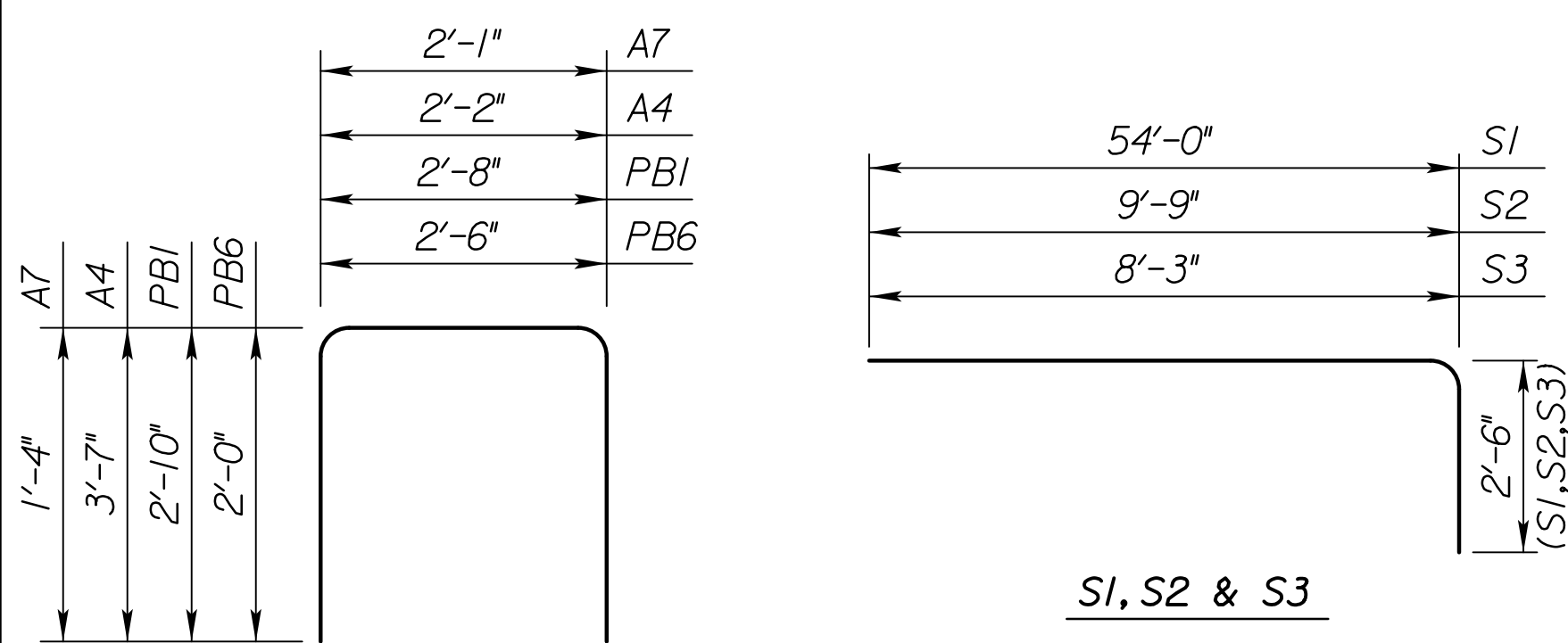
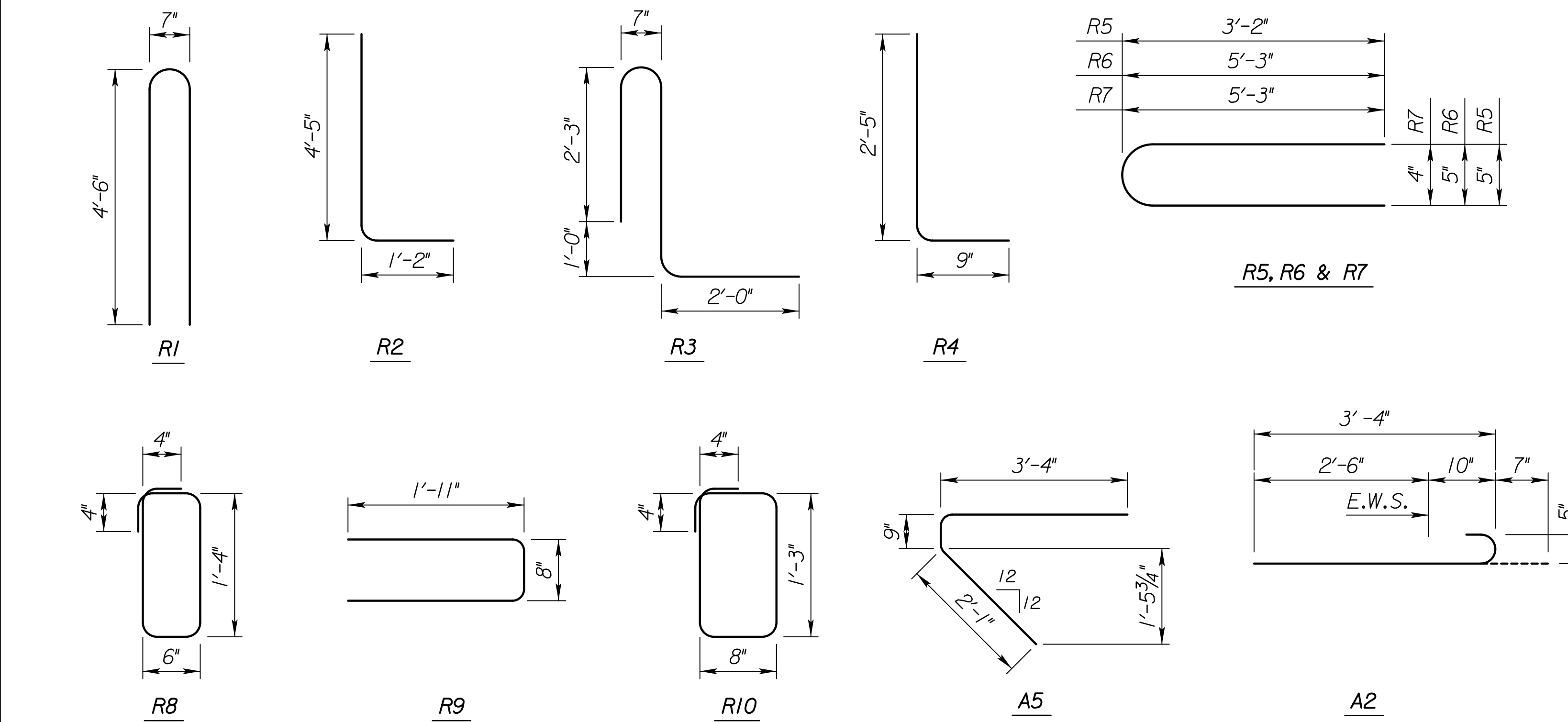
Straight Bars		Bent Bars					
Mark	Size	Number	Length	Mark	Size	Number	Length
S8	#9	52	32'-0"	S1	#8	64	56'-6"
S9	#9	44	27'-3"				
S14	#9	26	42'-0"	R1	#7	24	9'-3"
S15	#9	22	30'-6"	R2	#7	4	5'-7"
S16	#9	22	24'-6"	R3	#7	156	7'-9"
				S2	#7	60	12'-3"
A1	#8	16	45'-8"	S3	#7	60	10'-9"
S6	#8	60	34'-6"				
S7	#8	60	23'-0"	A2	#5	72	3'-11"
S10	#8	44	24'-9"	R5	#5	8	6'-6"
S11	#8	44	15'-6"	R6	#5	8	10'-8"
S17	#8	22	14'-6"				
				A4	#4	184	9'-4"
R11	#6	24	8'-3"	A5	#4	72	6'-2"
R12	#6	96	9'-8"	A7	#4	28	4'-9"
T1	#6	67	37'-8"	R4	#4	156	3'-2"
				R7	#4	4	10'-8"
A3	#5	20	45'-8"				
				R8	#3	280	4'-4"
A6	#4	2	36'-8"	R9	#3	144	4'-6"
S4	#4	4	12'-6"	R10	#3	36	4'-6"
S5	#4	2	23'-6"				
S12	#4	44	6'-3"	T3-T14			
S13	#4	44	7'-3"				
T2	#4	50	37'-8"				
SC1	#4	54	6'-6"				
PB4	#7	12	36'-8"	PBI	#5	144	8'-4"
PB5	#7	12	37'-8"				
				PB6	#4	20	6'-6"
PB2	#5	12	37'-8"				
PB3	#5	4	36'-8"				

⊗ See Bending Diagram

BILL OF REINFORCING STEEL
Non-Epoxy Coated - Grade 60

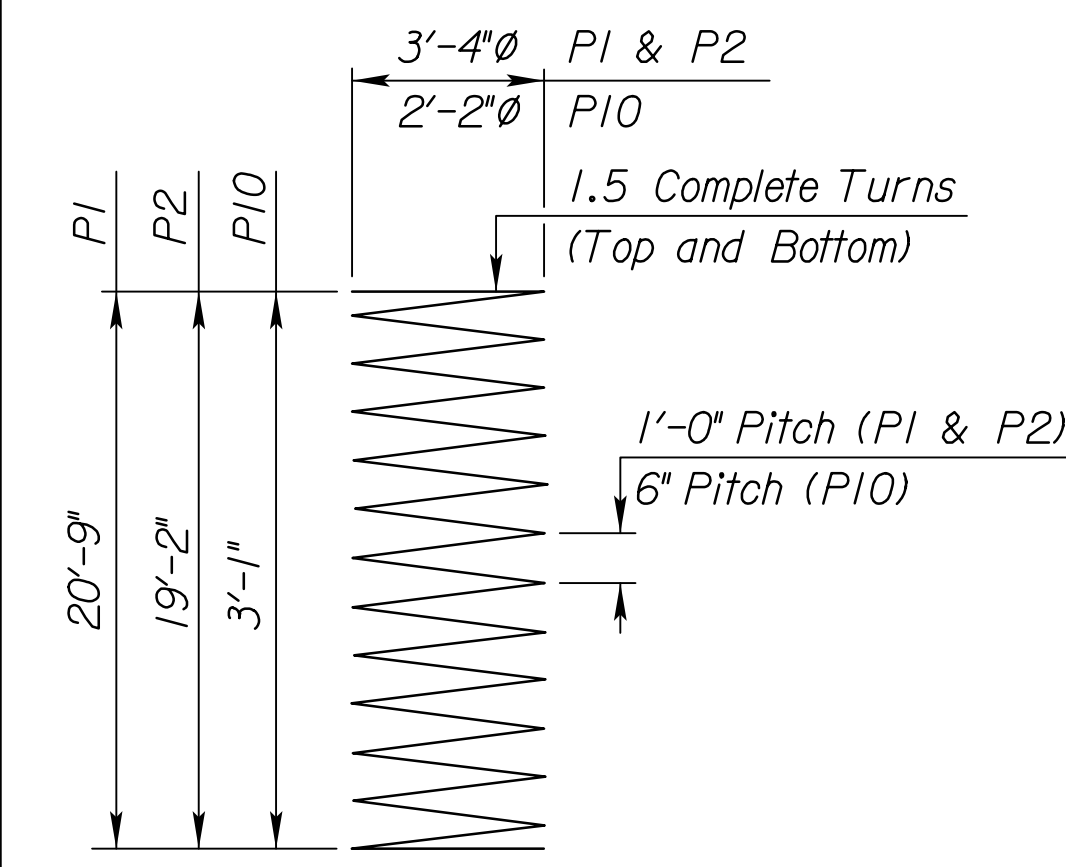
Straight Bars				Bent Bars			
Mark	Size	Number	Length	Mark	Size	Number	Length
P3	#10	45	20'-9"	P1	5/8"∅	3	⊗
P4	#10	45	19'-2"	P2	5/8"∅	3	⊗
				P10	5/8"∅	6	⊗
P5	#7	72	6'-8"				
P7	#7	36	16'-2"	P6	3/8"∅	3	⊗
P9	#7	36	16'-0"	P8	3/8"∅	3	⊗

⊗ See Bending Diagram
Note: P1, P2, P3, P4, P5 and P10 bars are all considered part of the drilled shafts

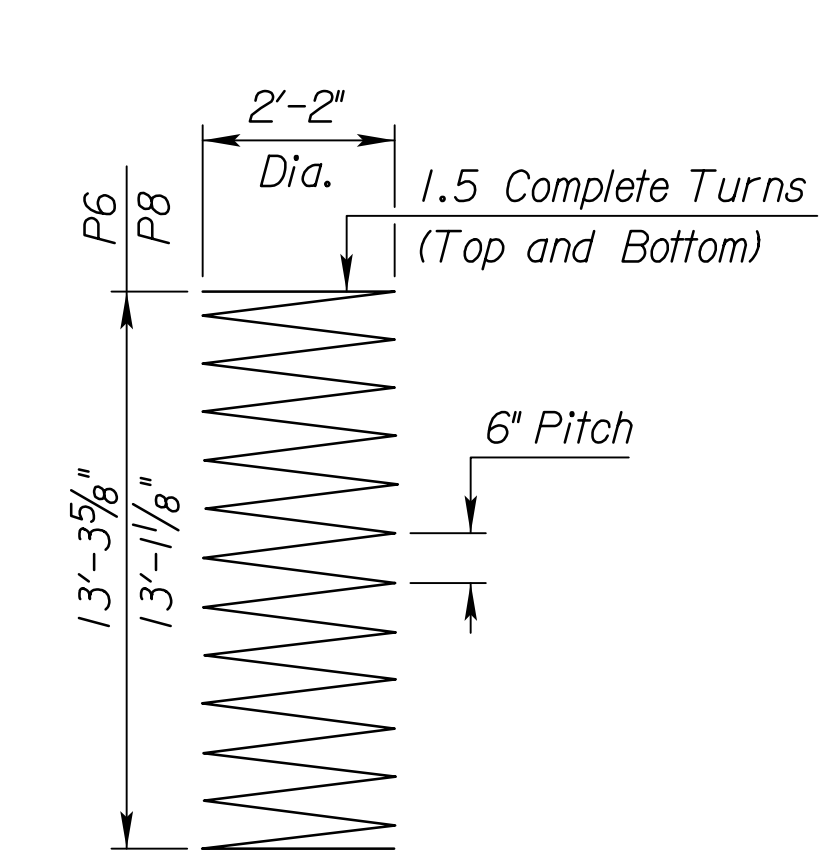


A4, A7, PBI & PB6

S1, S2 & S3



P1, P2 & P10



P6 & P8

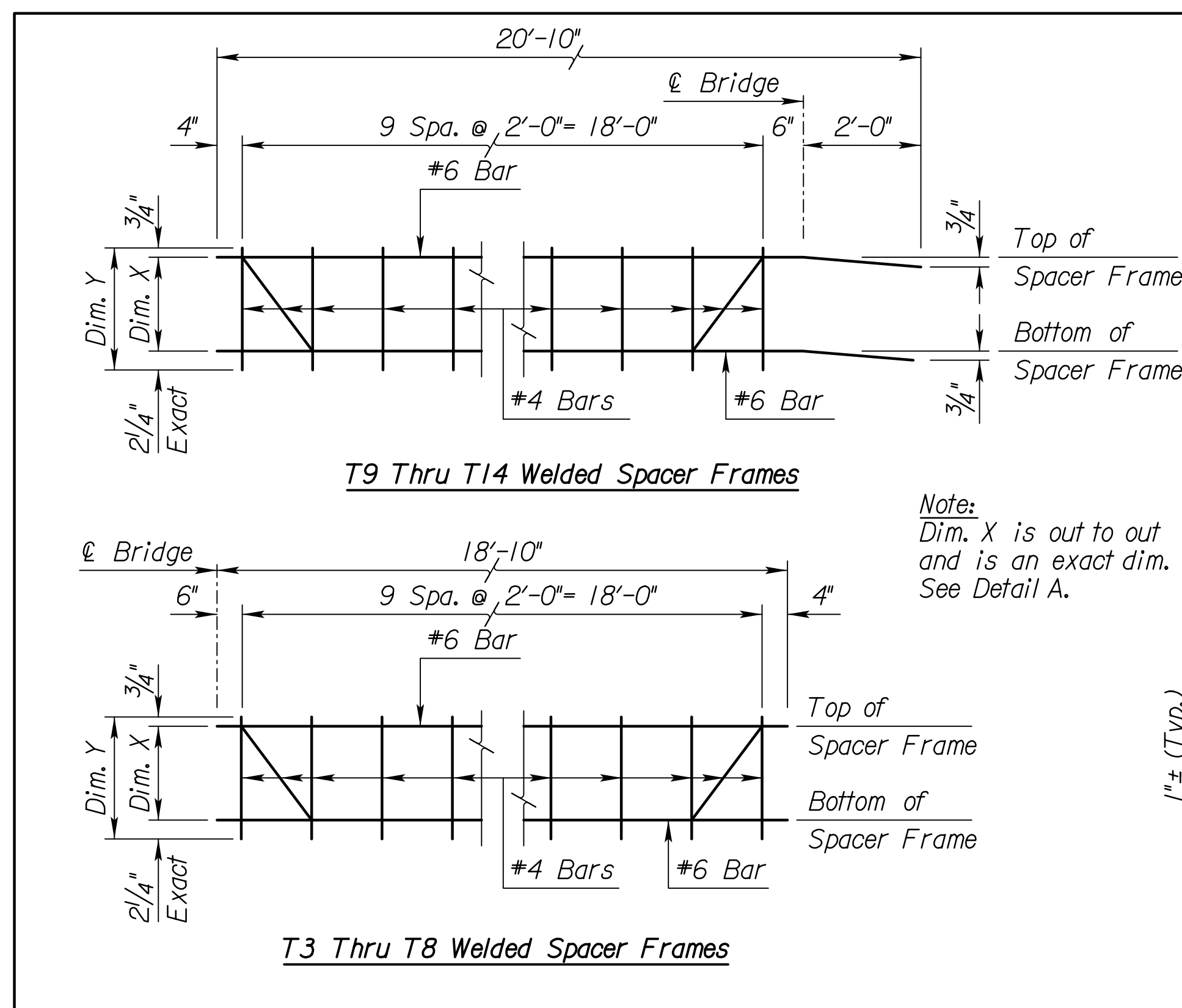
(5/8" ∅ smooth or deformed bar)

(3/8" ∅ smooth or deformed bar)

Spiral reinforcing shall meet the requirements of ASTM A615 Grade (60 or 40) or ASTM A82.

Spiral Spacer Bars:

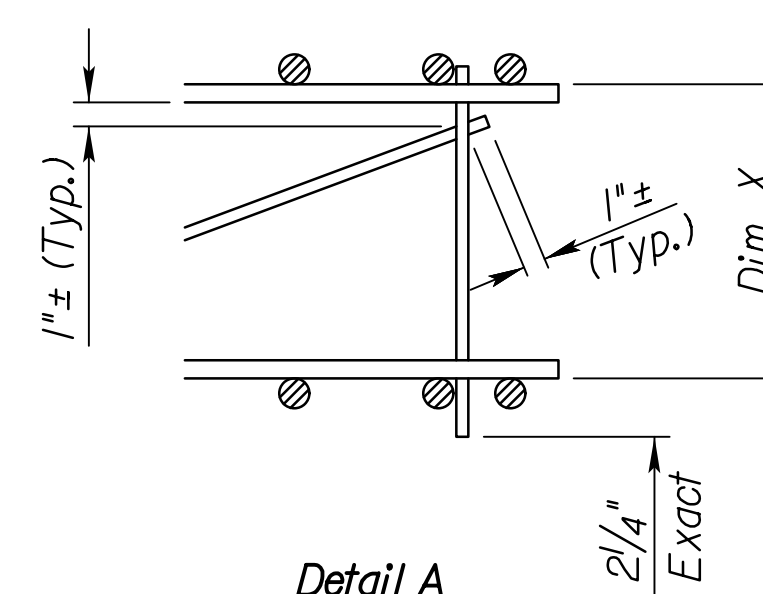
- Are included in the weight of reinforcing steel.
- Minimum section modulus = 0.008 in³
- 4 required per spiral.



⊗ SPACER FRAMES
(Epoxy Coated)

Mark	No. Ea.	Dim. X	Dim. Y
T3, T9	4	8 1/8"	11 1/8"
T4, T10	4	8 3/16"	11 3/16"
T5, T11	4	8 7/8"	11 7/8"
T6, T12	4	10 1/4"	1'-1 1/4"
T7, T13	4	1'-0 1/4"	1'-3 3/4"
T8, T14	4	1'-2 5/16"	1'-5 5/16"

Weight of spacer frames included in the weight of reinforcing steel.



BENDING DIAGRAMS
(All dimensions are out to out of bars.)

Plotted By: JHOVERSO
File: c:\pwork\kentrall\01\43371674\46572801\br113-13.dgn
Plot Date: 01-08-24

3			
2			
1			
NO.	DATE	REVISIONS	BY APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 99-99-139.68 (113) Sta. 7375+41.16
BILL OF REINFORCING STEEL
AND BENDING DIAGRAMS
K-99 OVER DRAGOON CREEK DRAINAGE
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO. OF	SCALE	APP'D
DESIGNED	ASF DETAILED	JAH QUANTITIES
DESIGN CK.	TKI DETAIL CK.	TKI QUAN. CK.

KDOT Graphics Certified 01-08-2024 Sheet No. 44

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	45	135

GENERAL NOTES

ERECTION AND REMOVAL OF ROLLED BEAM DETOUR BRIDGE: The Lump Sum bid item, "Erection and Removal of Rolled Beam Detour Bridge", shall consist of loading and unloading, transportation, erection, maintenance, repair of damaged galvanized coating, and dismantling all detour bridges used on this project. It shall also include inventories, detour bridge site preparation, labor, tools, equipment and all incidentals necessary to complete the work.

Seventy-five (75) percent of the Lump Sum bid price shall be paid when the erection of the structure has been completed. The remaining twenty-five (25) percent shall be paid when the structure has been dismantled and all parts to be returned have been properly inventoried and stockpiled at the storage site listed on the Plans.

REFIT ROLLED BEAM DETOUR BRIDGE: This shall be subsidiary to "Erection and Removal of Rolled Beam Detour Bridge" and consists of Contractor furnished bridge items as listed in Column No. 4 of the Inventory Sheet. These items consist of steel piles, pre-drilled pile holes and replacement items which represent damaged or missing bridge parts from existing spans. The replacement parts are preliminary, being based upon an estimate of the stockpiled items to be supplied by KDOT.

FURNISH ROLLED BEAM DETOUR BRIDGE: The Lump Sum bid item, "Furnish Rolled Beam Detour Bridge", shall consist of the Contractor furnished bridge items as listed in Column No. 5 of the Inventory Sheet. These items will furnish one or more complete bridge spans.

SITE PREPARATION: The Contractor shall complete the channel excavation, if any, and the embankment at the detour bridge prior to driving piling or any substructure construction. Site preparation includes any excavation, backfill or compaction of backfill required. Upon removal of the detour bridge, any fill placed in the channel by the Contractor shall be removed and the stream channel shall be reshaped to its natural lines as directed by the Engineer.

BACKFILL COMPACTION: Backfill compaction shall be required at the abutments, as directed by the Engineer.

GALVANIZING: All structural steel furnished by the Contractor shall be hot-dipped galvanized after fabrication in accordance with ASTM A-123 Specifications. Any warpage caused by the galvanizing process shall be straightened.

The 7/8" diameter beam flange to rocker plate (Drill and Tap) bolts and the 17" barrier unit bolts along with their nuts and washers shall be hot-dipped galvanized in accordance with ASTM A153, Class C, Specifications.

Bridge floor panels shall be galvanized after the panels have been fabricated in accordance with plan details.

WELDING: Material and construction shall conform to the Kansas Department of Transportation Specifications. The shop drawings shall show the prequalified welds to be used in the fabrication.

TIMBER: The Contractor shall furnish the untreated timber. It shall remain the property of the Contractor after the completion of the project.

DECK PANELS: It is recommended that only one panel type be used per span.

GEOTECHNICAL REPORT: The geotechnical report addendum (Dated May 2023) and the geotechnical information shown on the plans is the best information available. The report is available for inspection by qualified bidders at the State Bridge office, KDOT, Eisenhower State Office Building, 700 SW Harrison, Topeka, KS.

CONNECTIONS: All structural steel and barrier unit field connections shall be made with ASTM A-325 high strength bolts, heavy hex nuts and hardened washers under the turned elements. The timber shall be connected to the abutment backwalls with lag screws and ASTM A307 button head bolts.

All bolts shall be tightened, at a minimum, to a snug tight fit. Snug tight is defined as a tightness attained by a few impacts of an impact wrench or the full effect of a man using an ordinary spud wrench. See the Maintenance Note. The high strength bolts used in the N-Connection of the bridge deck panels to the beams shall be tightened to KDOT specification by the turn-of-the-nut method. The N-Connection is a friction connection which requires a high clamping force.

Only the 7/8" diameter beam flange to rocker plate (drill and tap) bolts and the 17" barrier unit bolts along with their nuts and washers shall be retained by KDOT. These are listed as Item No. 23 and 24 on the Inventory Sheet. The Rocker Plates shall remain attached to the beam flanges after the bridge is disassembled.

All bolts, nuts, washers and lag screws (Items No. 25 through 33) not mentioned in the Galvanizing Note may or may not be galvanized. They shall become the property of the Contractor upon completion of the job and shall be removed from the construction site.

PILE BENTS: The Contractor shall construct the pile bents as detailed. Bracing is required if 10 foot or more of piling is exposed above ground. The bracing shall meet the minimum requirements as detailed. The Contractor shall provide the Engineer with details for approval of his proposed method of bracing. The piling and bracing shall remain the property of the Contractor. Piling shall be removed to a minimum depth of 1 foot below finished ground line or as specified in the plans. The C15 channels, bearings and bearing plates shall remain the property of KDOT.

PILING: Due to shallow bedrock at this location, it is recommended that H-Piles be set within pre-drilled holes. Pre-drilling will encounter the Johnson Shale Formation, the Long Creek Limestone Member, and the Hughes Creek Shale Member. Temporary casing should be used to ensure that the pre-drilled hole remains stable after being drilled. H-pile should be placed in the hole and driven to bearing after pre-drilling. Once sufficient bearing has been achieved, the annular space around the pile should be backfilled with concrete up to the top of bedrock. From the top of bedrock the annular space should be backfilled with sand or other loose material to within 2 to 3 feet of the surface according to Standard Specification 704.4c.

Where possible, these piling shall be driven to the minimum computed bearing value of 55 Tons.

In using the pile driving formula of the KDOT Specifications, the Engineer shall not allow the Contractor to drive the pile MORE THAN 82.5 TONS.

If additional piling is required due to a change in pile tip elevation, this piling shall be provided by the Contractor without change in compensation.

PILE AND BERM ELEVATIONS: See Substructure Details sheet for top of pile and berm elevations.

Containers used to store parts at the Storage site must have holes drilled or punched in the bottom of these containers to provide free and total drainage. Steel parts must not be stored in containers that will hold water.

TRANSPORTATION: The Contractor shall transport to the site all parts and materials required to construct the detour bridge. All parts supplied by KDOT may be picked up from the following Receiving Site(s):

KDOT Mixing Strip at the US-50/K-177 Intersection (NE Quadrant), approximately 3 miles south of Project No. 177-9 KA-4430-01 Br 9(076)

Upon completion of the project, the Contractor shall transport the parts supplied by the Contractor and by KDOT to the following Storage Site:

KDOT Mixing Strip to the east of the intersection of K-31 and K-99 on the north side of K-31, south of Project No. 99-99 KA-5728-01.

Where they shall remain the property of KDOT. Exceptions are as noted in the Post Construction Inventory Listing.

Transportation shall include the loading and unloading of these parts and materials.

MAINTENANCE: The Contractor shall be responsible for normal maintenance and periodic inspection to insure tightness of the connections and the safe operation and structural integrity of the detour bridge for the duration of the project.

The Contractor may elect to use specialty nuts instead of the standard heavy hex nut for the grid decking to girder connection. This connection, with standard heavy hex nut, tends to loosen under traffic and requires periodic maintenance. Periodic inspection is required regardless of nut type used.

INTERCHANGEABILITY: Similar parts are designed to be interchangeable throughout the structure.

REPAIR OF GALVANIZED COATING: Areas of damage to the galvanized coating on structural steel or floor panels shall be painted with a zinc rich paint as directed by the Engineer.

CONCRETE: Concrete (4.0)(AE) shall be used in the Barrier Curb.

REINFORCING STEEL: Epoxy coated reinforcing steel, Grade 60, shall be used in the barrier curbs. Epoxy coating shall be in accordance with Kansas Specifications.

STRUCTURAL STEEL: The rolled beams shall meet AASHTO M270 (Grade 36) Specifications. All other structural steel shall meet ASTM A709 (Grade 36) Specifications. The rolled beams shall conform to the supplemental impact properties for Temperature Zone Designation 2.

SLOPE STABILITY: The site is to be monitored after substantial rain events for scour affecting the piling at the abutments. Contractor shall review the temporary bridge throughout it's service life for evidence of any bank degradation.

INDEX TO DETOUR BRIDGE DRAWINGS	
Sheet No.	Drawing
45	General Notes
46	Inventory
47	Construction Layout
48	Engineering Geology
49	Optional Abutment Details
50	Substructure Details
51	Bearing Device Details
52	Framing Plan and Diaphragm Details
53	K-Connection of Deck Panels
54	N-Connection of Deck Panels
55	Safety Barrier Curb

SLOPE PROTECTION (Riprap Stone): Place Slope Protection (Riprap Stone) to the limits and thicknesses shown on the plans or as directed by the Engineer. Use (Light 200 LB) as described in Division 1100 placed to the limits shown in the plans. Slope protection shall be removed prior to final channel grading, but Contractor may choose to re-use the rock in channel placements. For quantity of rock excavation for removal see Recapitulation of Road Quantities.

SUMMARY OF QUANTITIES	
Item	Quantity
Erection and Removal of Rolled Beam Detour Bridge	Lump Sum
Slope Protection (Riprap Stone)	273 Cu. Yd.

4	6-21-99	Numerous Revisions	GFK	GMC
3	12-9-98	Numerous Revisions		DRT/GMC
2	6-16-97	Numerous Revisions		DRT/GMC
1	10-1-96	K and N Panels		DRT/GMC
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
Detour Sta. 78+16.37

**GENERAL NOTES
(DETOUR BRIDGE)
WITH H-PILE**

Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED		DETAILED	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.
			CADD
			CADD CK.

Std. Base File: k:\bridge\standards\us\br415a.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\ventra\01\143371674\k4572801\br113-br415a.dgn
 Plot Date: 01-08-24

† The "Total Required" and "Preliminary Inventory" is based upon Abutment Backwall Option "B".

Recorded By: _____
 Completion Date of Post-Construction Inventory: _____

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	46	135

GENERAL NOTES

Pre-Construction Inventory: The Engineer and the Contractor shall inventory the parts which are to be supplied by KDOT and which are stockpiled at the receiving site(s) listed in the Storage Site Locations note. This inventory shall be recorded by the Engineer and verified by the Contractor. If there is a discrepancy between the actual number of parts stockpiled (Column 6) and the number listed in Column 3A and 3B, or if there are parts damaged beyond further use, the Contractor shall replace the missing and/or damaged parts (Column 7) and be reimbursed in accordance with KDOT Specifications, "Extra Work". The parts supplied by KDOT (column 6) and the replacement parts furnished by the Contractor (column 7) shall equal the total supplied in columns 3A and 3B.

Post-Construction Inventory: The parts to be returned to the KDOT storage site and to remain the property of KDOT shall be the sum of the total parts furnished by the Contractor and those supplied by KDOT. Exceptions are as noted by diagonal hatching in the Inventory Listing. The parts lost or damaged by the Contractor shall be replaced and transported to the storage site at the expense of the Contractor. This inventory shall be recorded by the Engineer and verified by the Contractor at the storage site at the time of delivery.

The Engineer shall send a copy of the Pre-Construction and Post-Construction Inventories to:
 Kevin Endsley
 13th Floor
 KDOT Bureau of Design
 Eisenhower State office Building
 700 SW Harrison St.
 Topeka, KS. 66603-35745

Storage Site Locations: All parts supplied by KDOT may be picked up from the following Receiving Site(s).

③A KDOT Mixing Strip at the US-50/K-177
Intersection (NE Quadrant), approximately 3
miles south of Project No. 177-9 KA-4430-01
Br 9(076)

③B _____

Upon completion of the project, all parts to be returned shall be stockpiled at the following Storage Site:

KDOT Mixing Strip to the east of the intersection
of K-31 and K-99 on the north side of K-31,
south of Project No. 99-99 KA-5728-01

7	07-09-10	Current Bid Item	JPJ	GMC
6	9-7-00	Stockpile 3A, 3B	GFK	DRT/GMC
5	6-21-99	Numerous Revisions	GFK	GMC
4	12-9-98	Numerous Revisions		DRT/GMC
3	8-25-97	Timber Quantities		DRT/GMC
2	6-16-97	Numerous Revisions		DRT/GMC
1	10-1-96	Removed K-Connector Note		DRT/GMC
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
 Detour Sta. 78+16.37

**INVENTORY
 (DETOUR BRIDGE)
 WITH H-PILE**

Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	DETAILED	QUANTITIES	CADD
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.

DETOUR BRIDGE INVENTORY SHEET

Col. #	Item Description	Units	† Total Req'd.	DESIGN ENGINEER				CONSTRUCTION ENGINEER											
				3A	3B	4	5	PRELIMINARY INVENTORY †		PRECONSTRUCTION INVENTORY		POSTCONSTRUCTION INVENTORY							
								Supplied by KDOT	Supplied by KDOT	CONTRACTOR FURNISHED		Supplied by KDOT from Stockpile	Replacements by Contractor (Col. 3A+3B minus Col. 6)	Returned to KDOT yard	Lost or damaged by Contractor	Damaged due to use and taken out of service			
										Refit Rolled Beam Detour Bridge	Furnish Rolled Beam Detour Bridge						3A	3B	
1	Predrilled Pile Holes	Lin. Ft.	367																
2	Support Bracing (Substructure)	(1)																	
3	Steel Pile (HP 10x42) (Substructure)	Lin. Ft.	*																
4	Abutment Cap C15x40 @ 40'-0"	(2 per Abutment)	Each	4															
5	Pier Cap C15x40 @ 29'-0"	(2 per Pier)	Each																
6	Brg. Plate (I 2x20x3'-9") w/4 - 1/4" Ø Anchor Studs (6 per support)	Each	12																
7	Rocker Plate (I 2x6x2'-3")	(12 per span) (8)	Each	12															
8	Timber 3"x12"	(120' per Abutment, Option 'A')	Lin. Ft.	-															
9	Timber 3"x12" (x5' Option 'A'; x7' Option 'B')	(40 timber per Abut.)	Each	80				80											
10	Timber 2x10x3'-0" (Nominal Size)	(8 per Abutment, Option 'A') (2)	Each	-				-											
11	Timber 4"x6"x7'-5"	(4 per Abutment)	Each	8				8											
12	Timber 4"x4"x4'-9/8"	(10 per Abutment)	Each	20				20											
13	Timber 3" Spacer	(40' per Abutment, Option 'A')	Lin. Ft.	-				-											
14	Timber 7 1/2" Spacer	(80' per Abutment, Option 'B')	Lin. Ft.	160				160											
15	Safety Barrier Curb, Precast (Interior Units)	(4)	Each	14															
16	Safety Barrier Curb, Precast (End Units)	(4)(5)	Each	4															
17	Anchor Plate (I 1/2 x6x1'-4")	(2 per Safety Barrier Unit)	Each	18															
18	Steel Grid Deck Panels	(9 per span) (6)	Each	9															
18a	"K" Panels	(6)	Each																
18b	"N" Panels	(6)	Each																
19	"N" - Connector Plate	(648 per N-Span)	Each	648															
20	Steel Spacer Plate (I 1/2 x4x0'-9 3/4")	(4 per abutment) (3)	Each	8															
21	Steel Girders (W30x17.3)	(6 per span)	Each	6															
22	Bent Plate Diaphragm BP (I 5/16 x27x4'-9 3/8")	(20 per span)	Each	20															
23	Bolts 7/8" Ø x2 1/4" (Beam E. to Rocker E. (Drill & Tap)	(48 per span) (8)	Each	48															
24	Bolts 3/4" Ø x17"	(4 per Barrier Unit)	Each	72															
25	Bolts 3/4" Ø x1 3/4" (Diaphragm to Intermediate Stiffener)	(120 per span)	Each	120															
26	Bolts 3/4" Ø x2 1/4" (Diaphragm to Brg. Stiffener)	(120 per span)	Each	120															
27	Bolts 3/4" Ø x2 1/4" (C15x40 to piling)	(120 per Pier; 160 per Abut.)	Each	320															
28	Bolts 3/4" Ø x3 1/2" (Floor panels to beams)	(648 per span) (4) (7)	Each	648															
29	Bolts 3/4" Ø x4" (bevel washer req'd.)	(12 per Bearing plate)	Each	144															
30	Lag Screws 1/2" Ø x5"	(4 per 3"x12" Timber)	Each	320				320											
31	Button Head Bolts 3/4" Ø x3 1/2" (Fasten 2x10 to Piling) (Option 'A' Only)		Each	-				-											
32	Button Head Bolts 3/4" Ø x6" (4"x6" & 4"x4" to stiffener)	(36 per Abutment)	Each	72				72											
33	Button Head Bolts 3/4" Ø x9" (4"x6" to wingwall piling)	(8 per Abutment)	Each	16				16											
34																			
35																			
36																			
37																			
38																			

- (1) 3/4" diameter bolts for bracing to piling connection as required. See Detail "B" of "Substructure Details" Sheet.
- (2) May be substituted for 1 1/2" steel spacer in Option 'B'.
- (3) A nominal 2x10 timber may be substituted.
- (4) Extras are to be supplied and stored at the site for immediate replacement of parts damage or lost during the life of the detour bridge.
- (5) Safety Barrier Unit, Precast (End Units): Select from the KDOT stockpile only those safety barrier end units to which three-beam guardrail can be attached. Select a minimum of 2 left-handed and 2 right-handed end units.
- (6) Steel Grid Deck Panels: All "N" type deck panels are identified by the painted RED end of a bearing bar. All "K" type deck panels do not have painted identification marks.
- (7) "K" Panels require 2 washers
 "N" Panels require 1 washer
- (8) Do not remove from rolled beams.

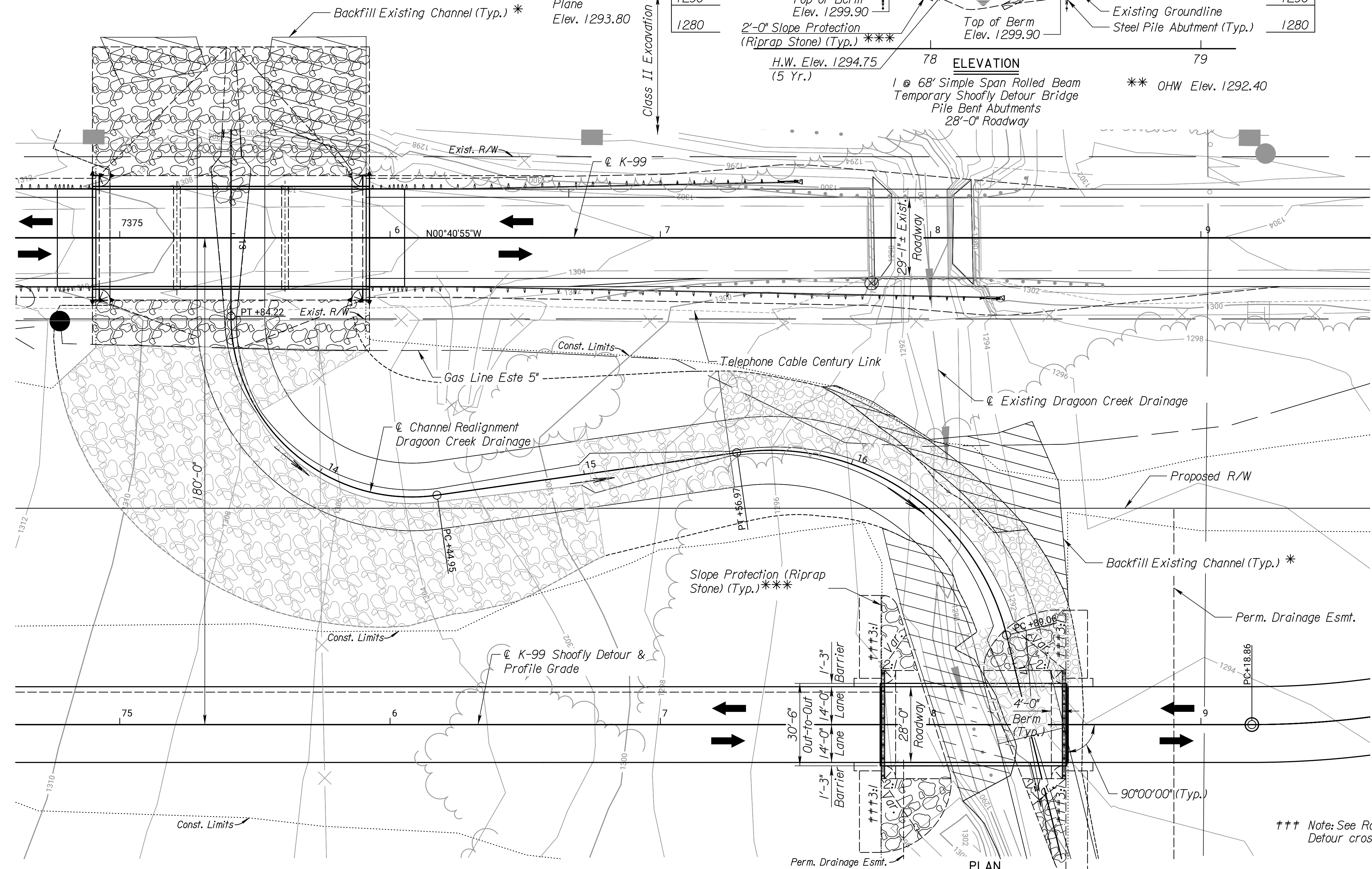
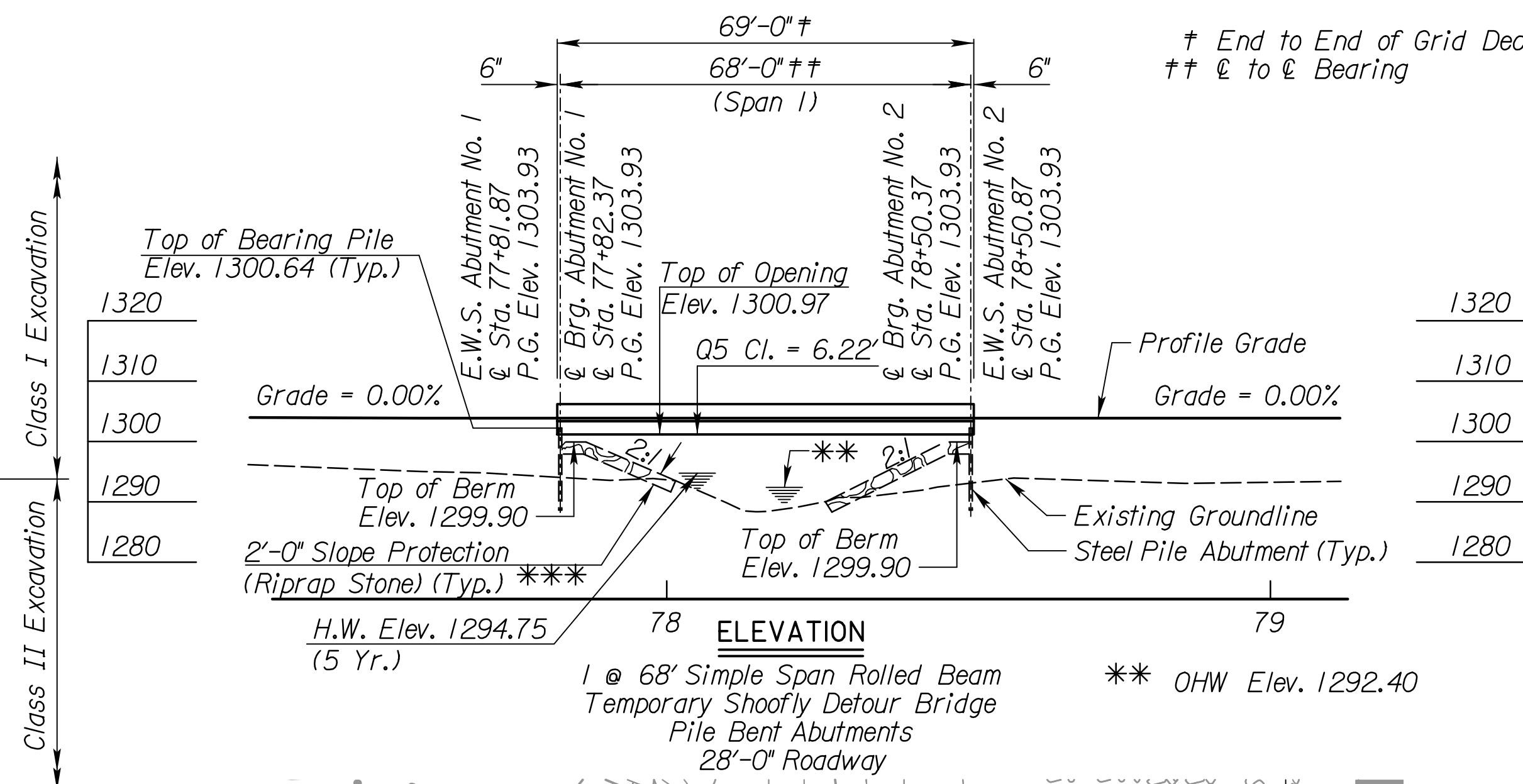
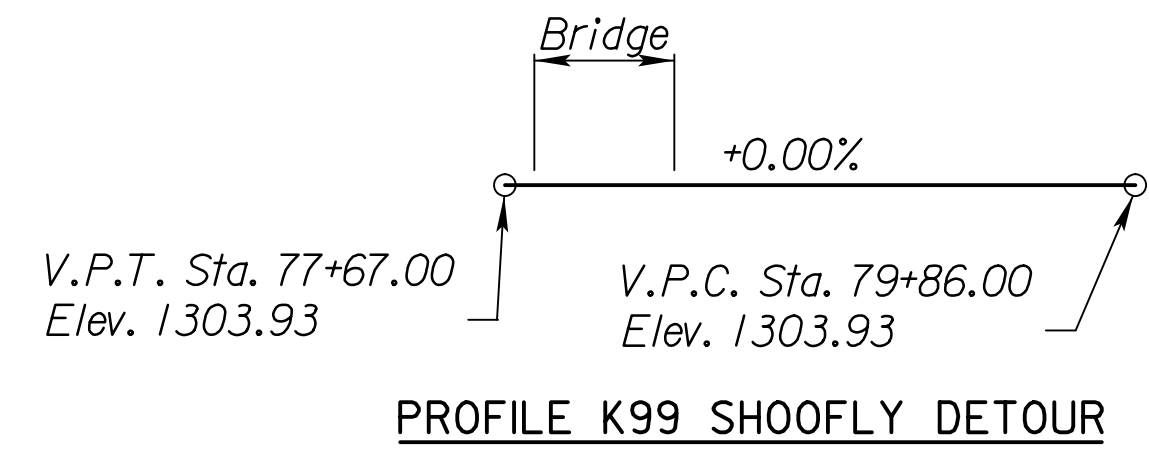
* 2 @ 31' Abut. No. 1
 6 @ 28' Abut. No. 1
 2 @ 31' Abut. No. 2
 6 @ 28' Abut. No. 2

Containers used to store parts at the Storage site must have holes drilled or punched in the bottom of these containers to provide free and total drainage. Steel parts must not be stored in containers that will hold water.

Std. Base File: k:\bridge\standards\br415b.dgn
 Plotted By: JHOVERSO
 File: c:\pwork\king\ventra\01\43371674\k4572801\br113-br415b.dgn
 Plot Location:
 Plot Date: 01-08-24

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	47	135

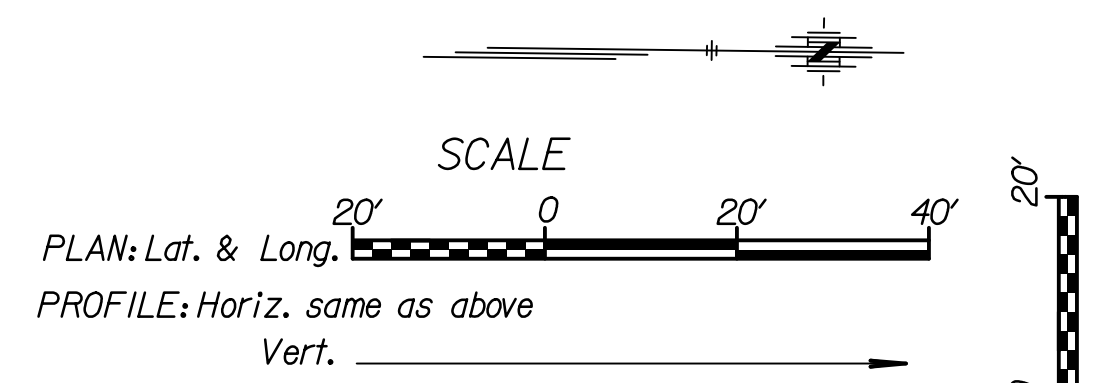


*** For additional information on slope protection and removal of slope protection see sheet "General Notes (Detour Bridge)".

SHOOFLY DRAINAGE DATA

Drainage Area	1.2	Sq. Mi.
Design Frequency	5	Years
Design Discharge (Q_{des})	780	cfs
Design High Water Elevation	1294.75	Ft.
Change in Design Backwater	0.48	Ft.
Design Backwater Elevation	1295.01	Ft.
Overtopping Elevation (Sta. 77+67)	1303.93	Ft.
Overtopping Discharge	5,700	cfs
Overtopping Frequency	>500	Years
Historic High Water Elevation (Exist. Bridge)	1301.00	Ft.
Ordinary High Water Elevation	1292.40	Ft.
Total Waterway Provided	473	Sq. Ft.
Design Waterway Provided	124	Sq. Ft.
Estimated Ordinary High Water Discharge	250	cfs

* For additional information see channel improvement sheets.



NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
Detour Sta. 78+6.37

**CONSTRUCTION LAYOUT (DETOUR BRIDGE)
WITH H-PILE**

Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO. OF SCALE	APP'D
DESIGNED ASF DETAILED JAH QUANTITIES ASF CADD JAH	
DESIGN CK. TKI DETAIL CK. TKI QUAN. CK. TKI CADD CK. TKI	

*** Note: See Road Plans for Detour cross sections.

BM#10 Set "T" shaped fc. po. S side of rock @ gate po.
22.29' Lt. & Sta. 7369+06.99 Elev. 1350.28

BM#11 Set mag. nail & KDOT wshr. top SE abut. of bridge
16.75' Rt. & Sta. 7377+78.17 Elev. 1302.52

BM#12 Set mag. nail & KDOT wshr. top E hdwl. RCB
15.22' Rt. & Sta. 7384+11.38 Elev. 1306.98

BM#13 Set RR spike in E face of pow. pole
34.18' Lt. & Sta. 7387+80.38 Elev. 1317.29

Vertical Datum: NAVD 88 (Geoid 18)

Plotted By: JHOVERSO
File: c:\pwworking\ventra\01\4337674\kds72801\br113-24.dgn
Plot Date: 01-08-24

TKI Graphics Certified

PILING: Due to shallow bedrock at this location, it is recommended that H-Piles be set within pre-drilled holes. Pre-drilling will encounter the Johnson Shale Formation, the Long Creek Limestone member, and the Hughes Creek Shale Member. Temporary casing should be used to ensure that the pre-drilled hole remains stable after being drilled. H-pile should be placed in the hole and driven to bearing after pre-drilled. Once sufficient bearing has been achieved, the annular space around the pile should be backfilled with concrete up to the top of bedrock. From the top of bedrock the annular space should be backfilled with sand or other loose material to within 2 to 3 feet of the surface according to standard Specification 704.4c.

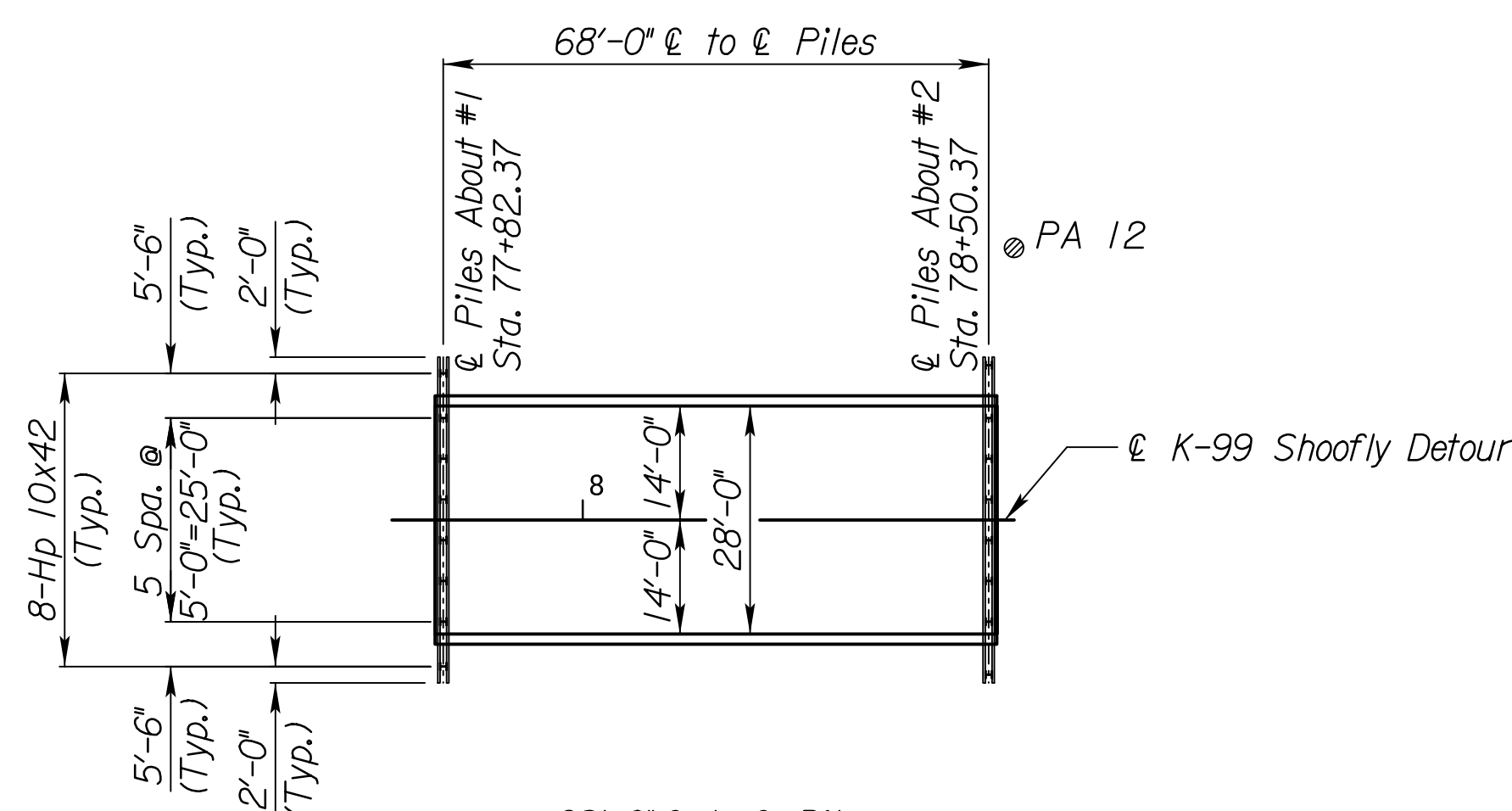
Where possible, these piling shall be driven to the minimum computed bearing value of 55 Tons.

In using the Pile driving formula of the KDOT Specifications, the Engineer shall not allow the Contractor to drive the pile MORE THAN 82.5 TONS.

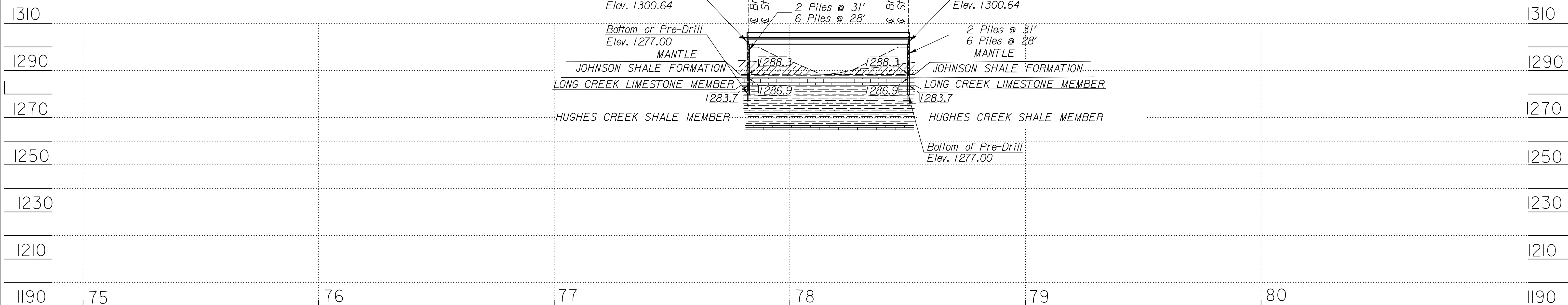
If additional piling is required due to change in pile tip elevation, this piling shall be provided by the Contractor without change in compensation.

PA 10

PA 16



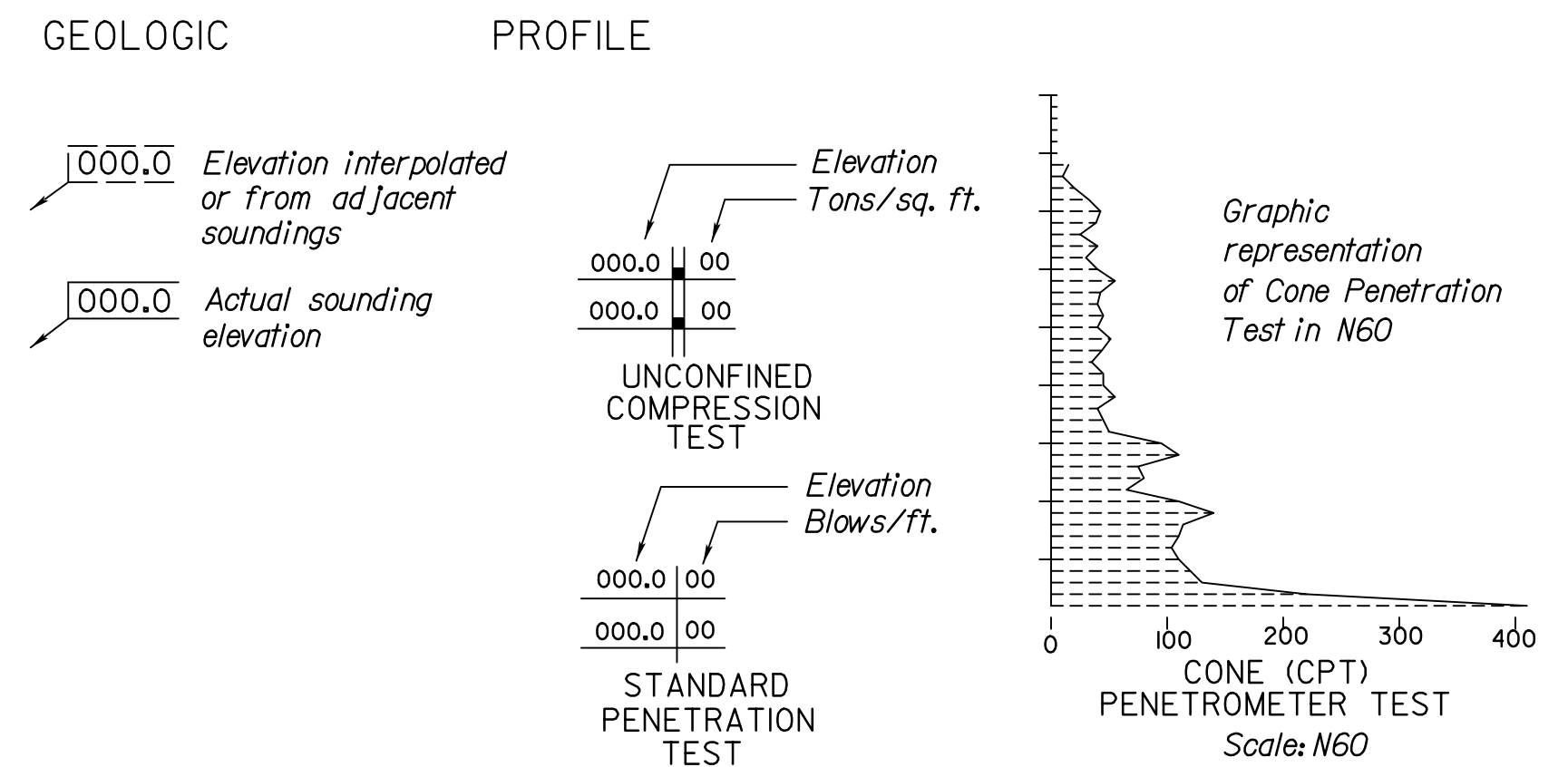
All piles shall be Gr. 50 Steel HP 10x42.



Plotted By: JHOVERSO
 File: c:\pwworking\ventra\01\43371674\k4572801\br113-24a.dgn
 Plot Date: 01-08-24

STANDARD		GEOLOGIC		SYMBOLS	
	Clay or Underclay		Caliche		Weathered Shale
	Silty Clay		Silty or Clayey Shale		Sandstone
	Silt		Limy Shale		Shaly Sandstone
	Sand		Black or Fissile Shale		Gypsum bed
	Gravel		Sandy Shale		Dolomite
	Boulders		Gypsiferous Shale		Bentonite
			Cross-bedded Sandstone		Loess
			Limestone		Mortarbed
			Cherty Limestone		Coal
			Shaly Limestone		Siltstone
			Sandy Limestone		Chalk
			Wavy limestone		Chalky limestone

- SOUNDINGS**
- Core drill
 - Power auger
 - Hand tools
 - Cone (CPT) penetrometer
 - Shelby tube
 - Water level



NOTE: Soundings shown on these plans are taken from notes obtained in the field and represent the best information available. Logs of these soundings are provided with the bid documents, or are available from the Kansas Department of Transportation in Topeka for inspection by interested and qualified bidders.

SCALE: 1" = 20' Horiz. 1" = 20' Vert.

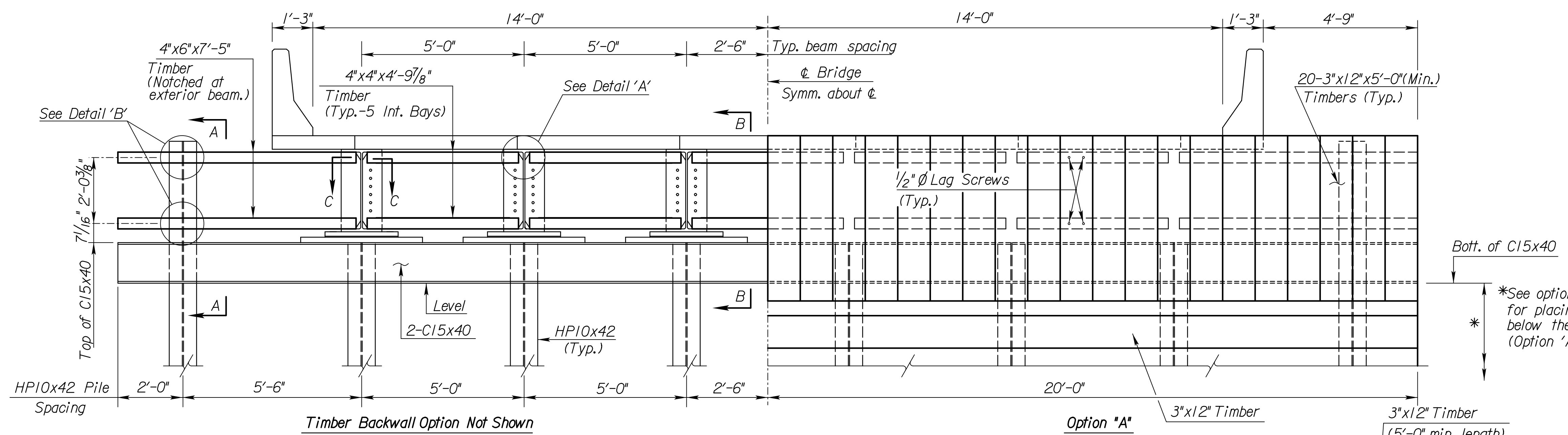
NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
 Detour Sta. 78+16.37
ENGINEERING GEOLOGY
 (DETOUR BRIDGE)
 WITH H-PILE

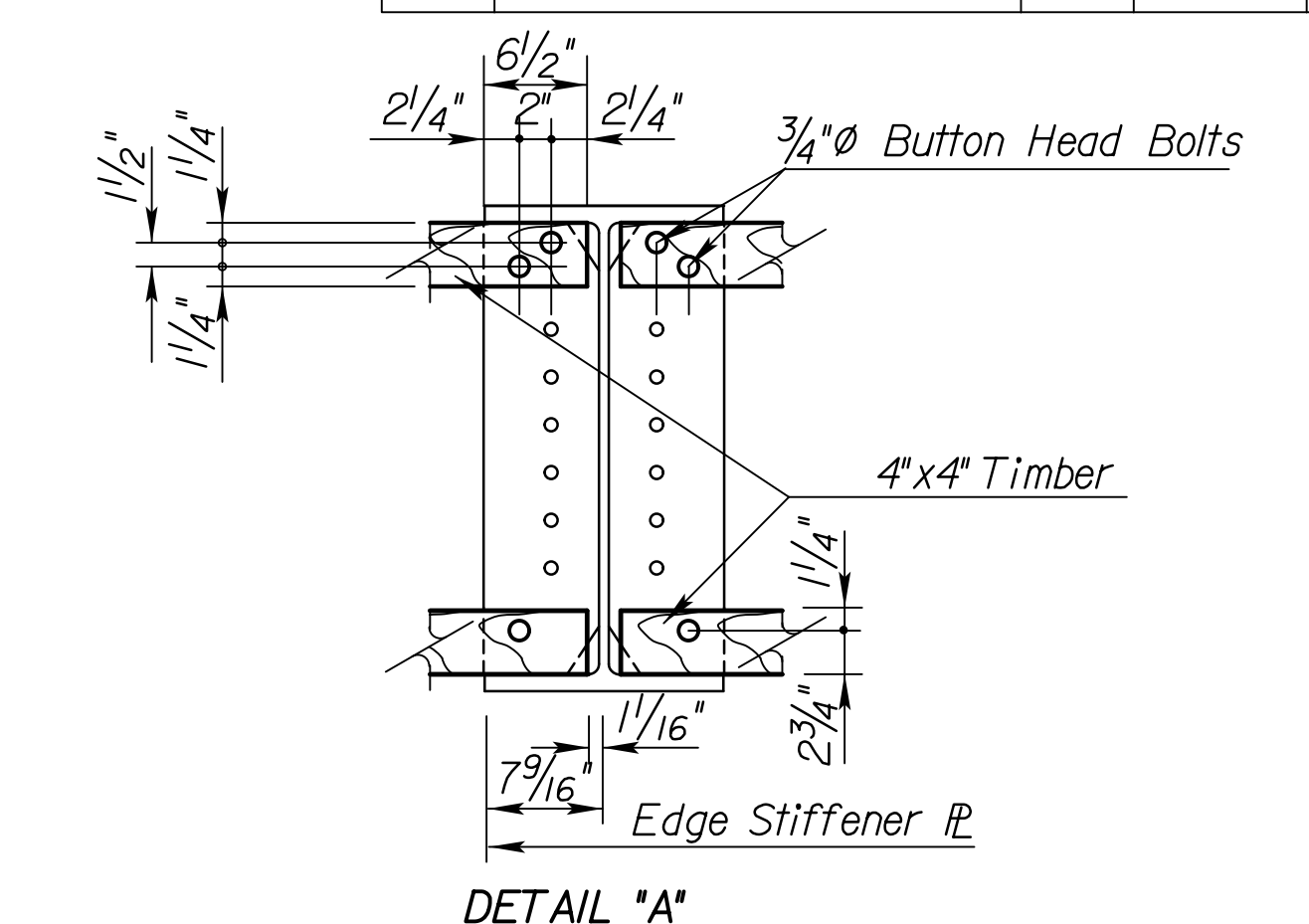
Proj. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO. OF	SCALE	APP'D
DESIGNED	DETAILED	GEOLOGY DRAWN BY:
DESIGN CK.	DETAIL CK.	GEOLOGY LOCATION:

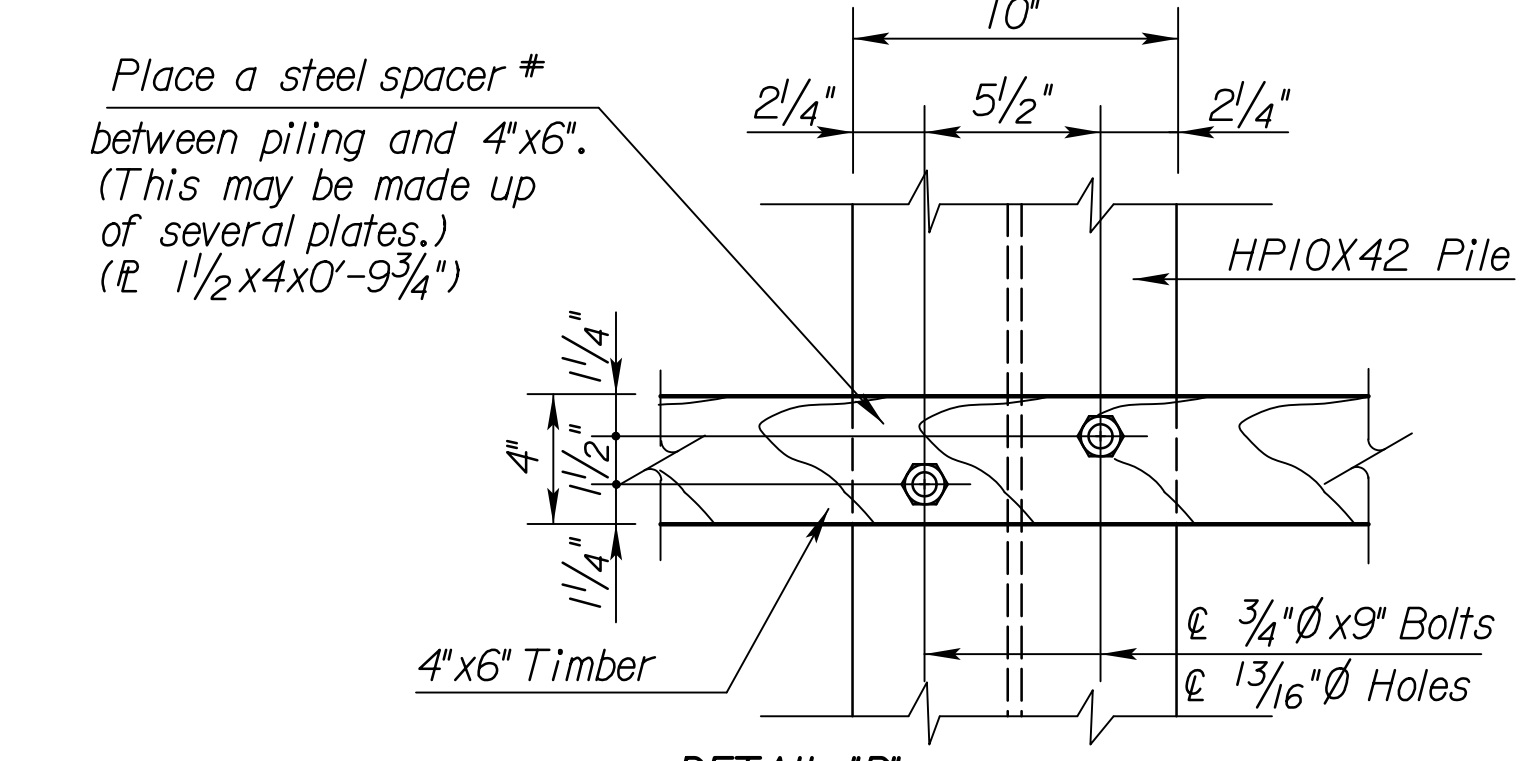
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	49	135



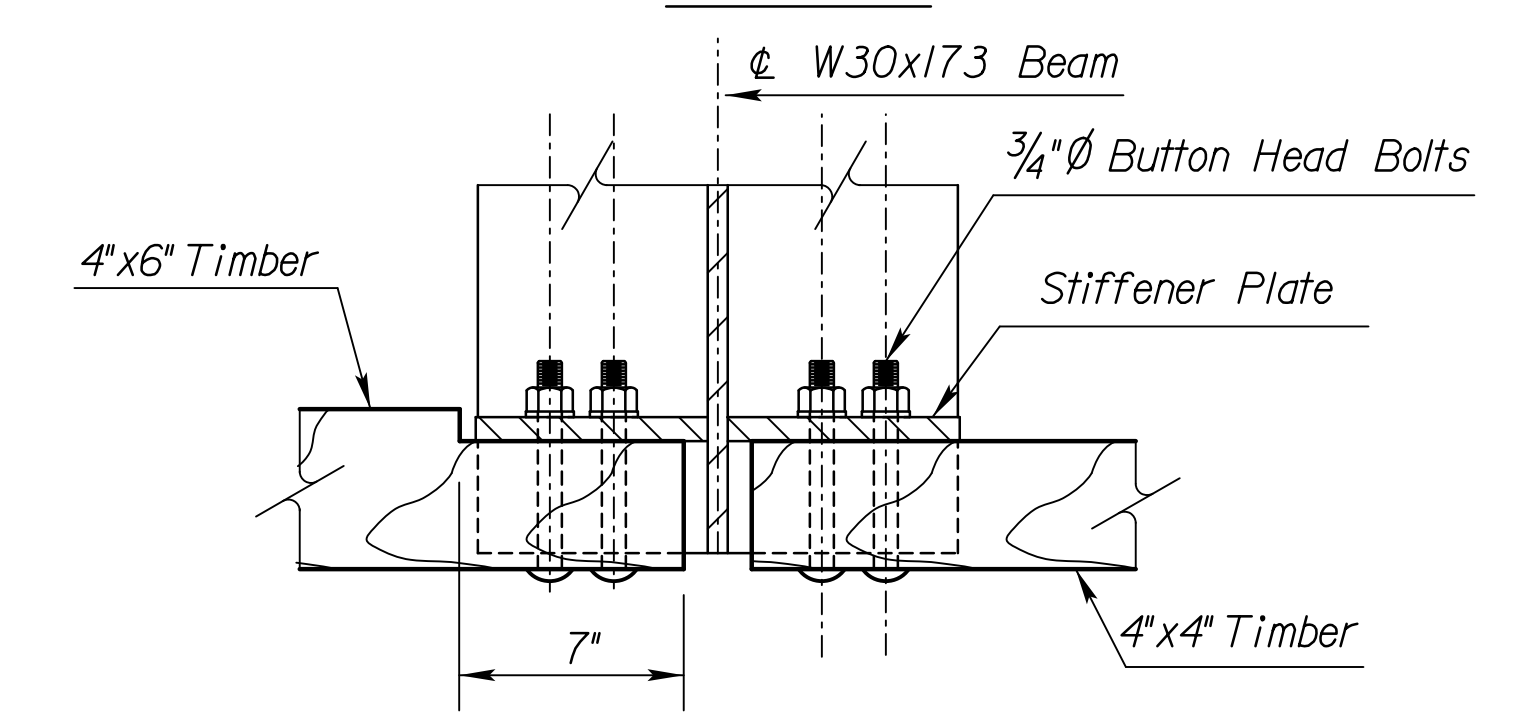
TYPICAL ELEVATION OF ABUTMENT BACKWALL



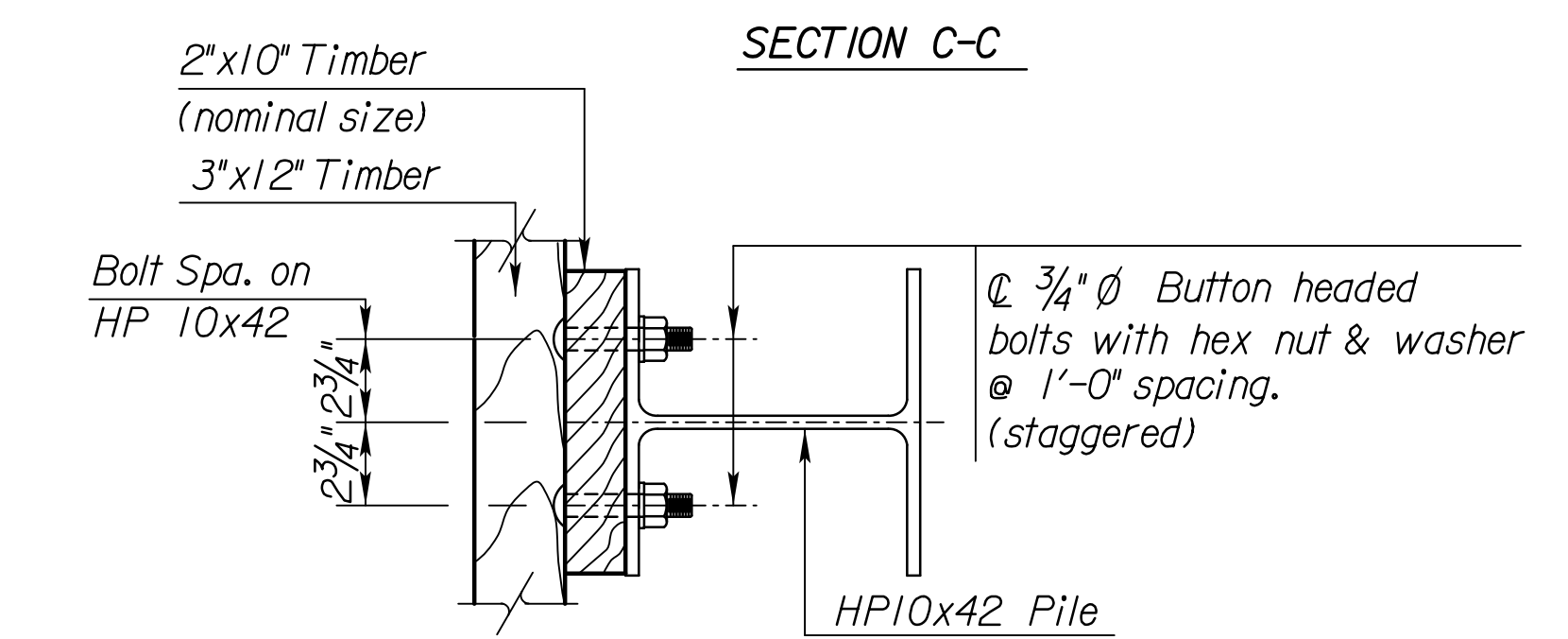
DETAIL 'A'



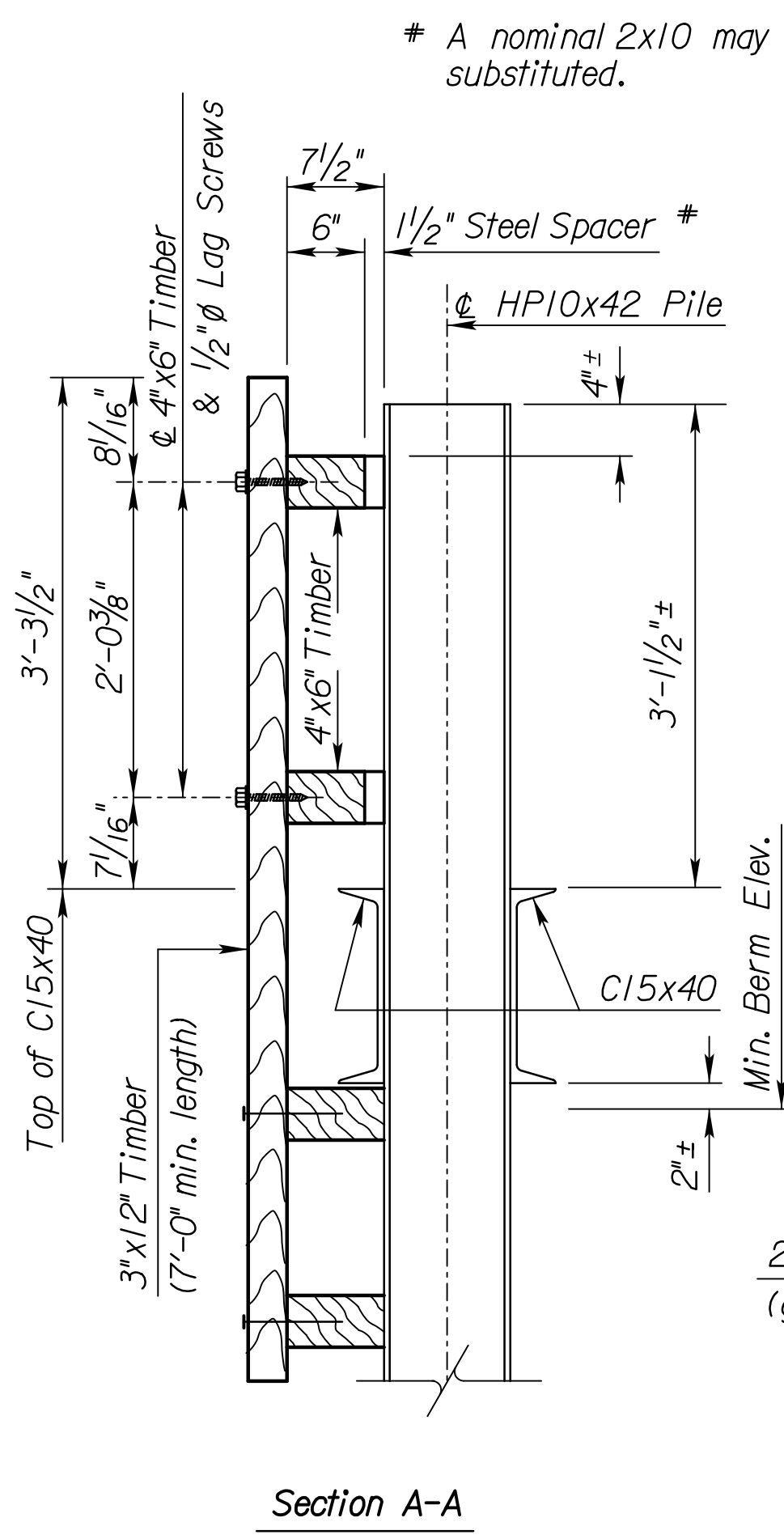
DETAIL 'B'



SECTION C-C

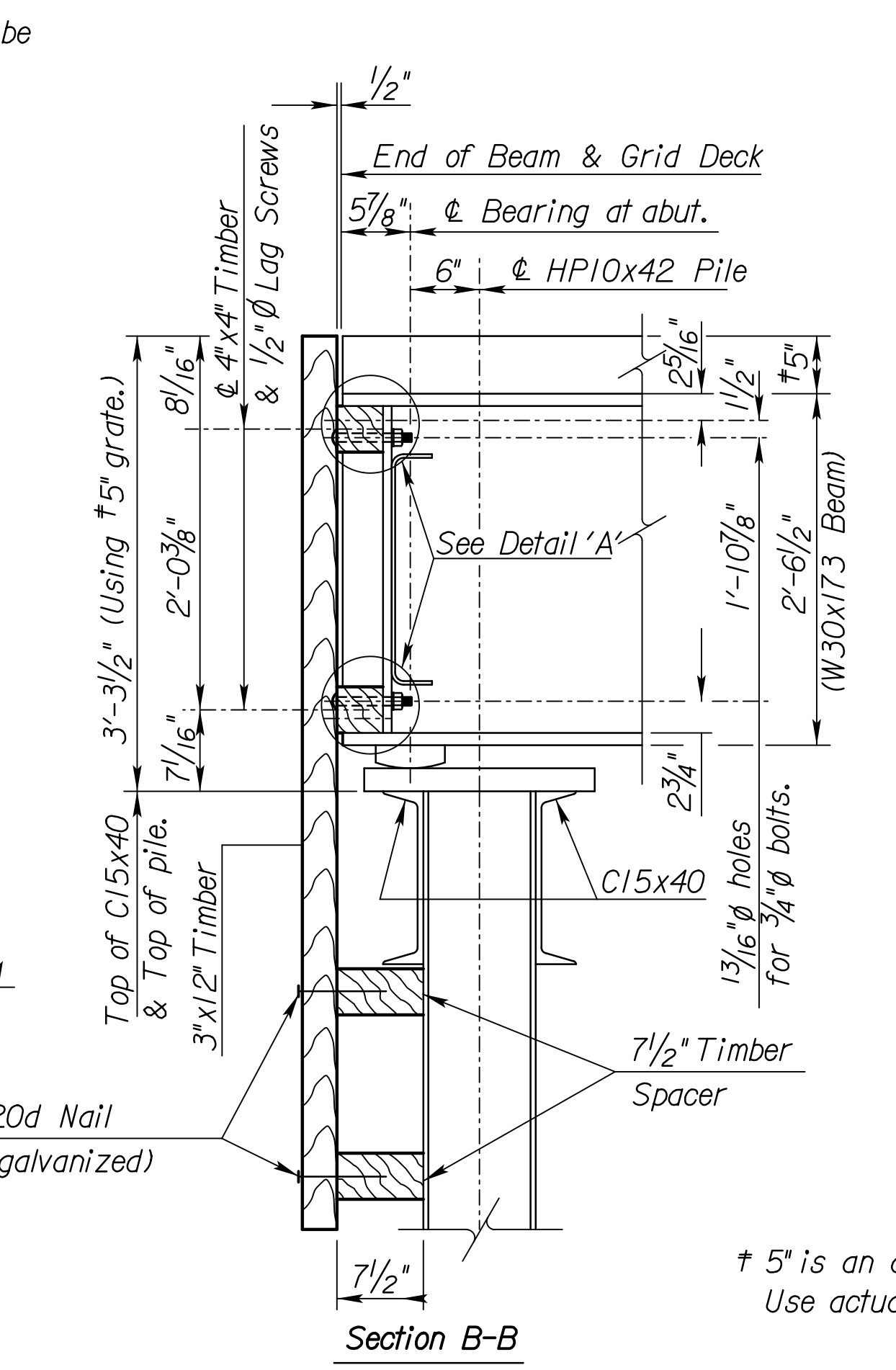


SECTION D-D



Section A-A

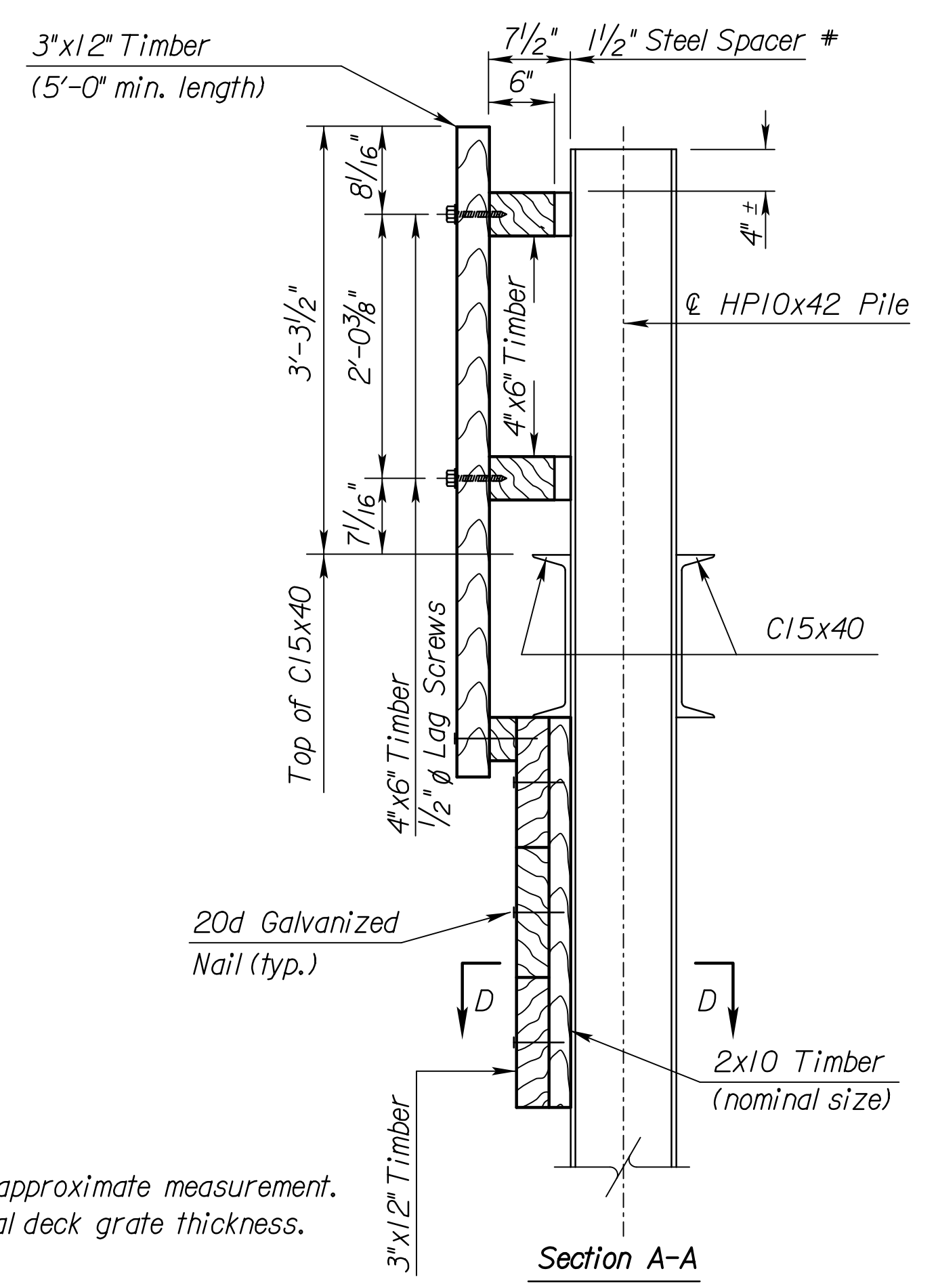
OPTION 'B'



Section B-B

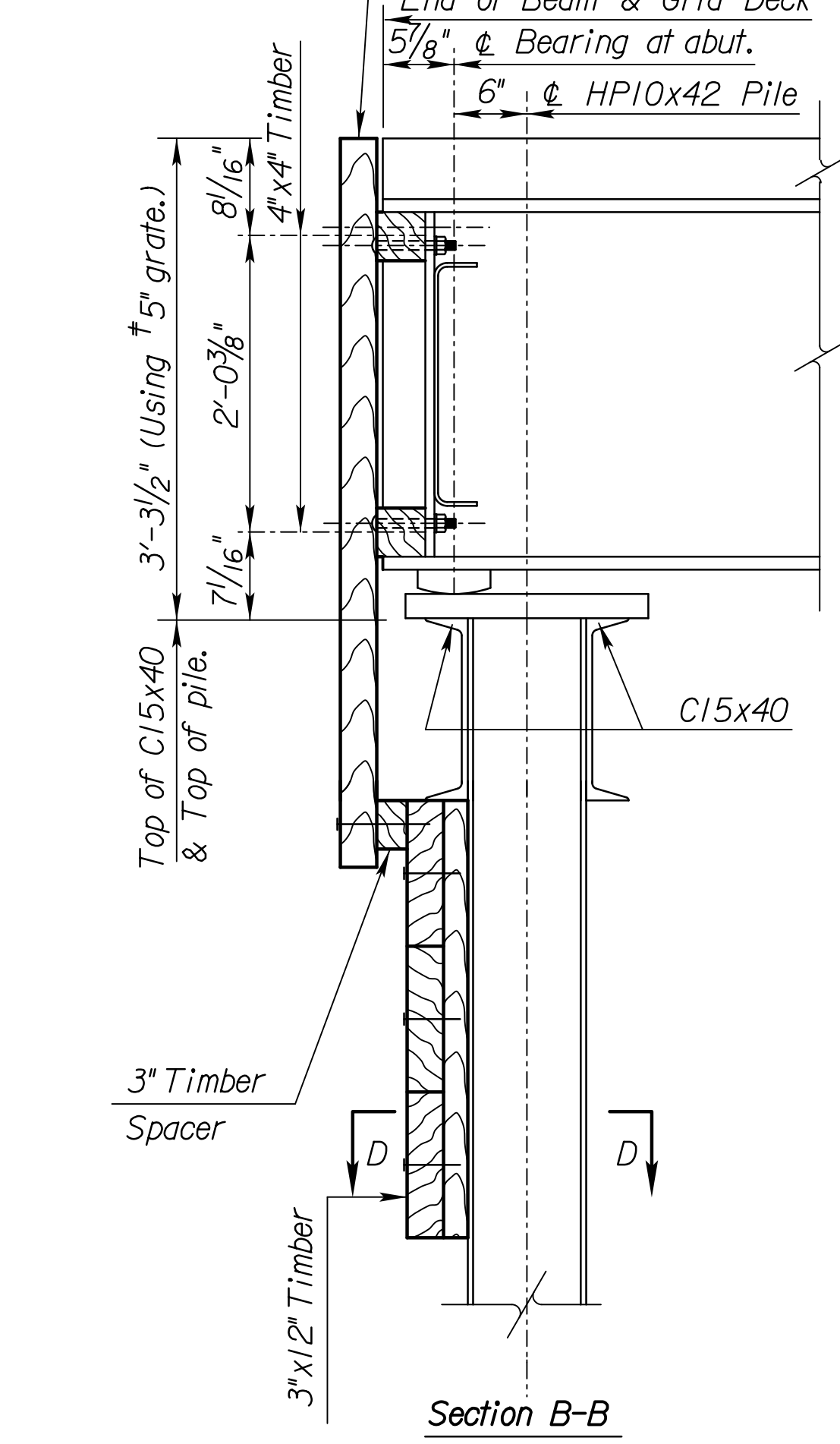
GENERAL NOTES

- All timber splices shall be made at points of support.
- Lag screws require prebored holes of the proper diameter. The lead hole diameter for the shank should equal that of the shank. The diameter of the lead hole for the threads should vary with the density of the wood.
- There shall be a minimum of 4- 1/2"Øx5" lag screws used to fasten each 3"x12" timber to the 4" timbers. There shall be a minimum of 1- 1/2"Øx5" lag screw at each point of support for the horizontal 3"x12" timbers. The 3" timber spacer may be nailed in place with 20d nails.
- Extend the 3"x12" timbers (option "A" or "B") approximately 2'-6" below the berm elevation.



Section A-A

OPTION 'A'



Section B-B

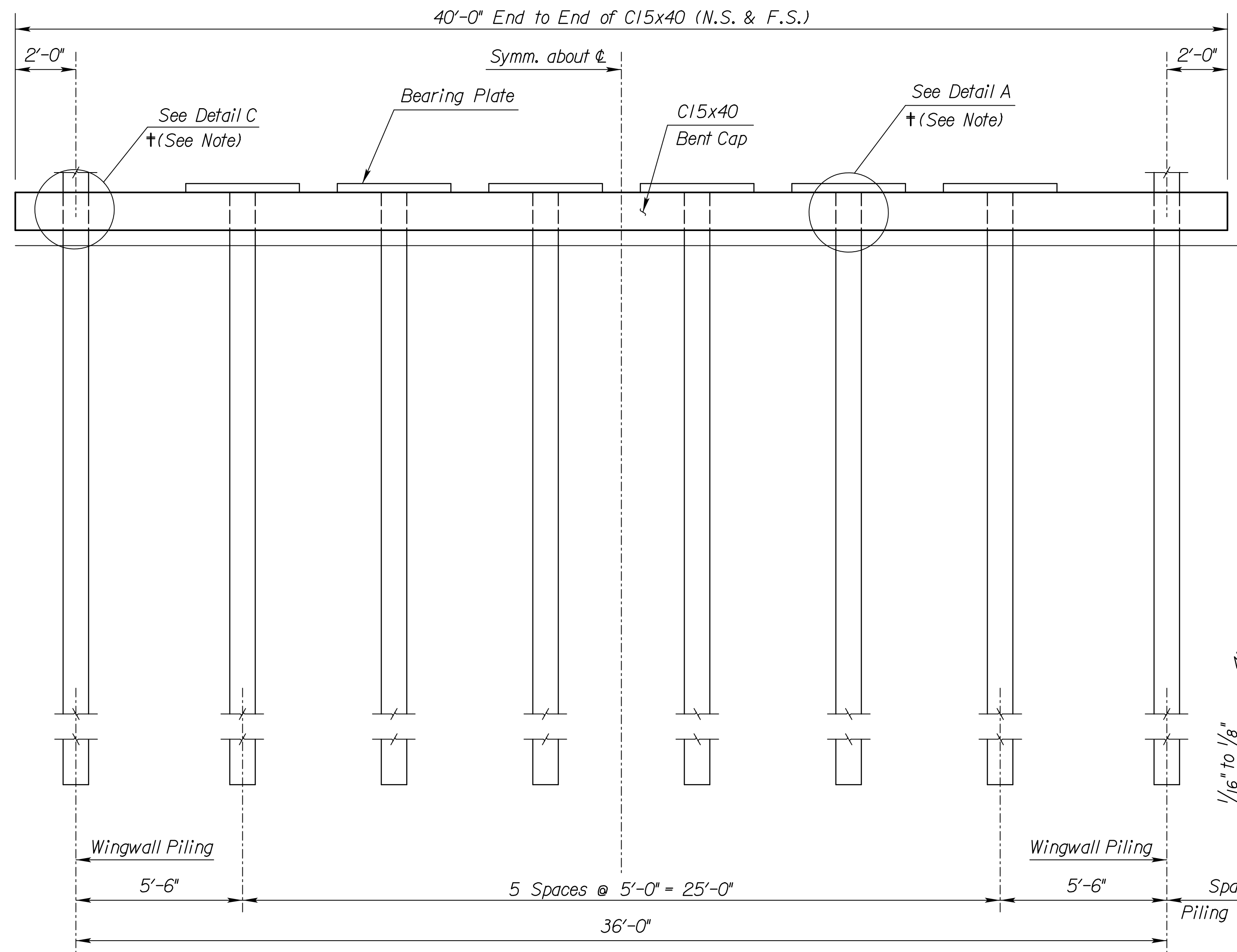
Std. Base File: k:\bridge\standards\br415c.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\ventral\01\43371674\k6572801\br113-br415c.dgn
 Plot Date: 01-08-24

3	6-21-99	Numerous Revisions	GFK	GMC
2	6-16-97	Change Treated Timber to Timber	DRT	GMC
1				
NO.	DATE	REVISIONS	BY	APP'D

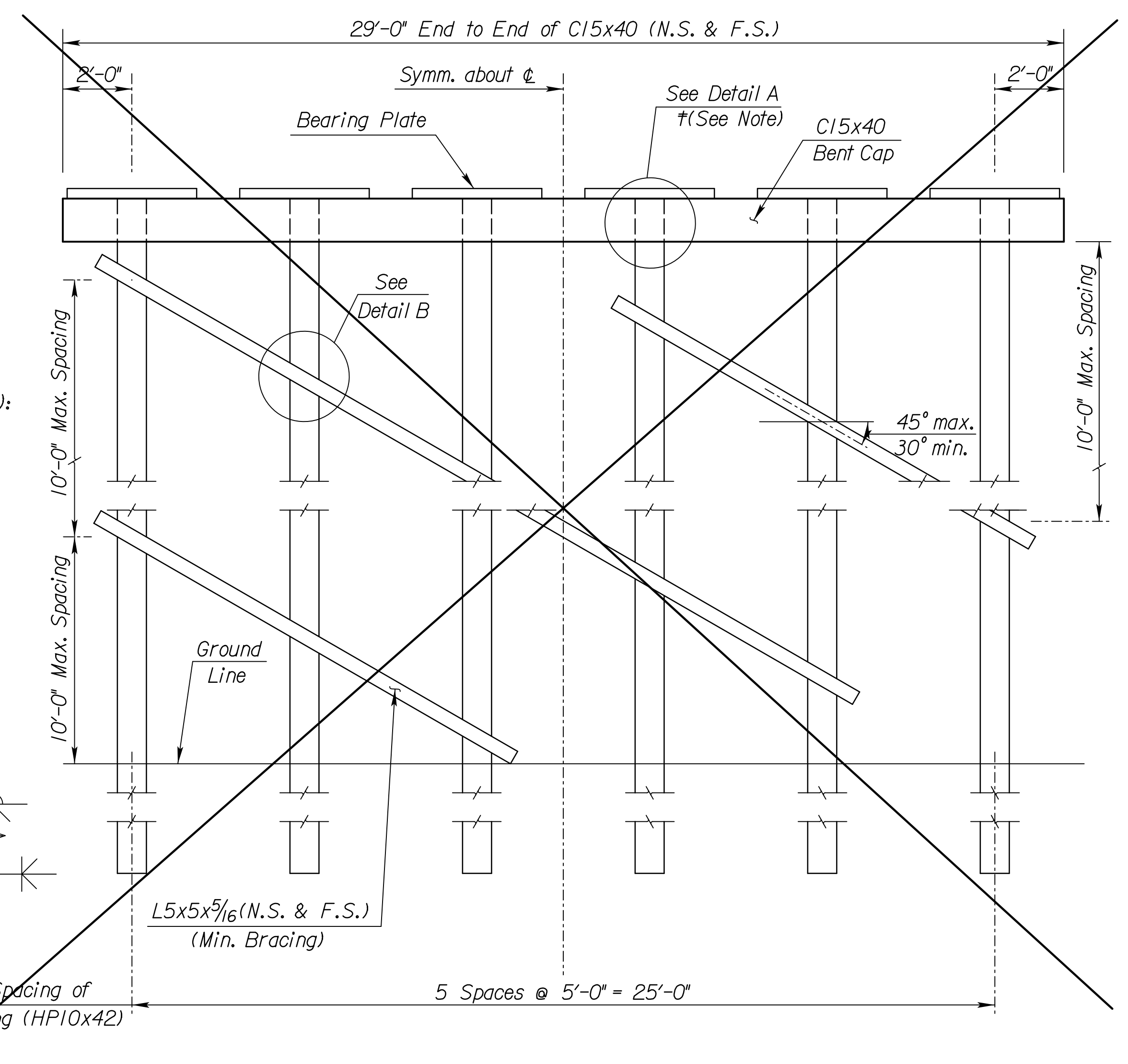
KANSAS DEPARTMENT OF TRANSPORTATION
 Detour Sta. 78+16.37
OPTIONAL ABUTMENT DETAILS
 (DETOUR BRIDGE)
 WITH H-PILE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	BY	QUANTITIES	CADD
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	50	135

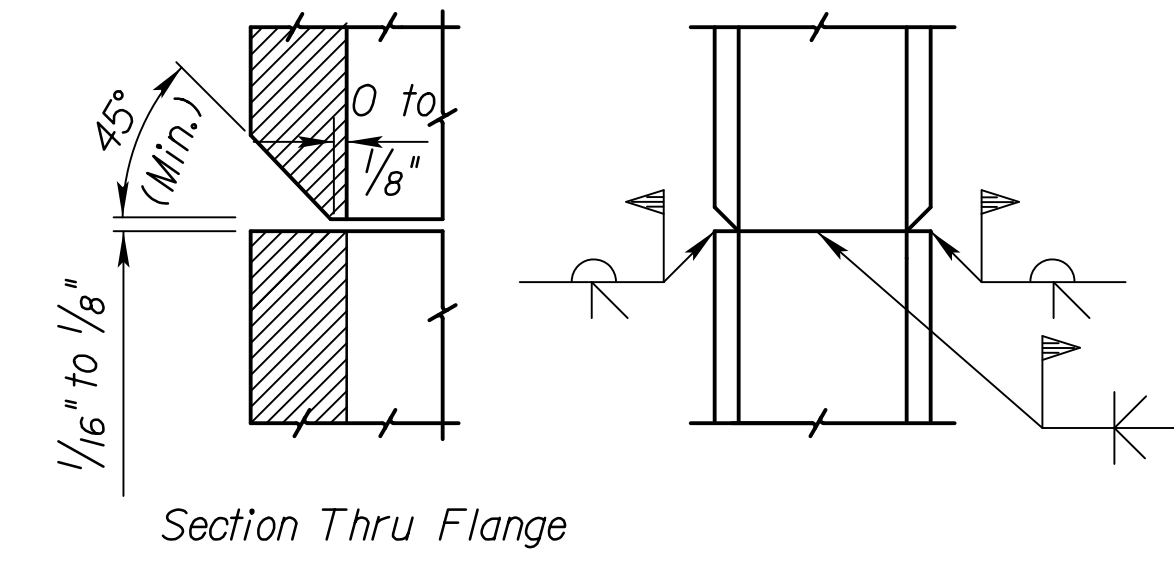


ABUTMENT SUPPORT

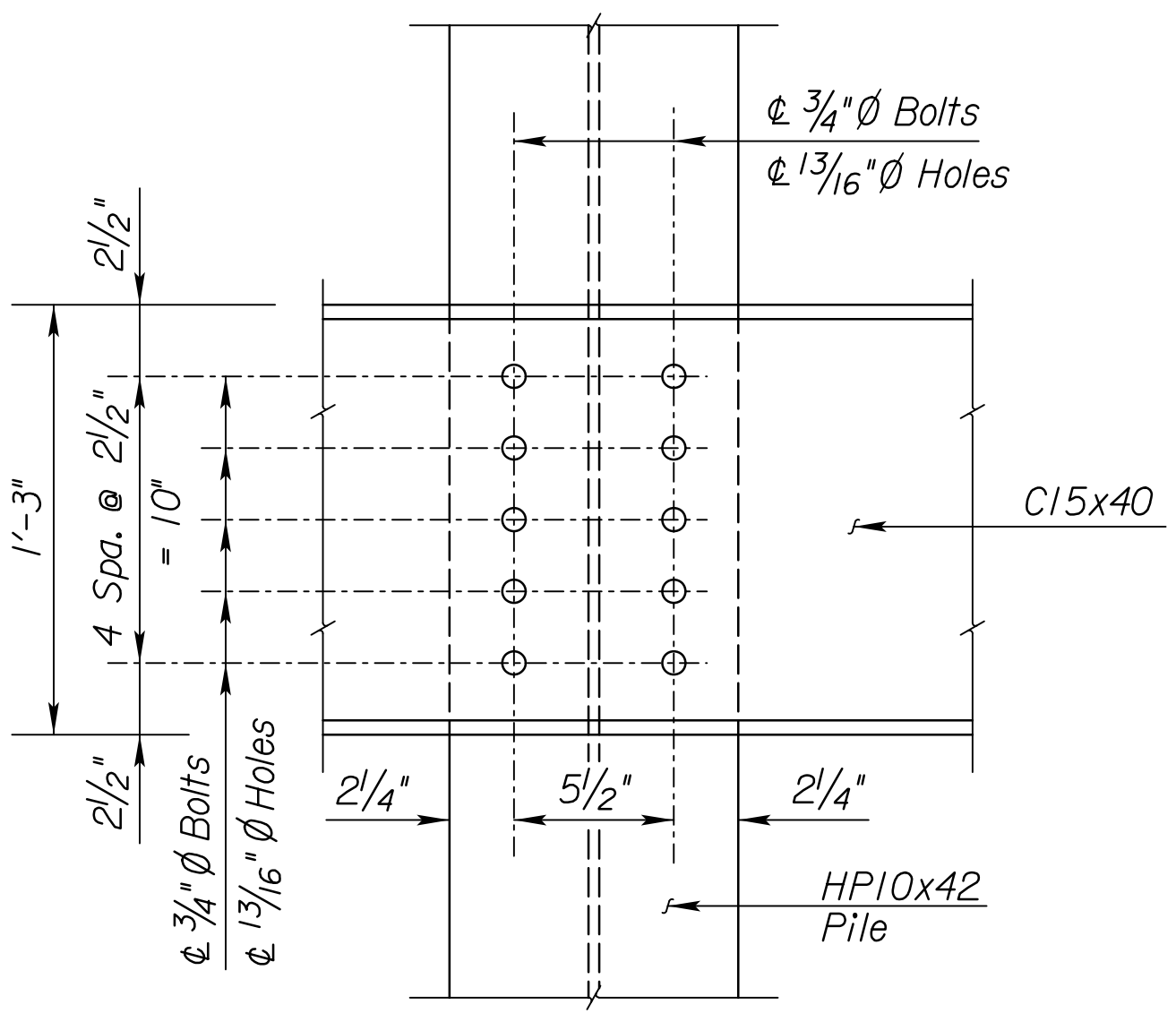


PIER SUPPORT

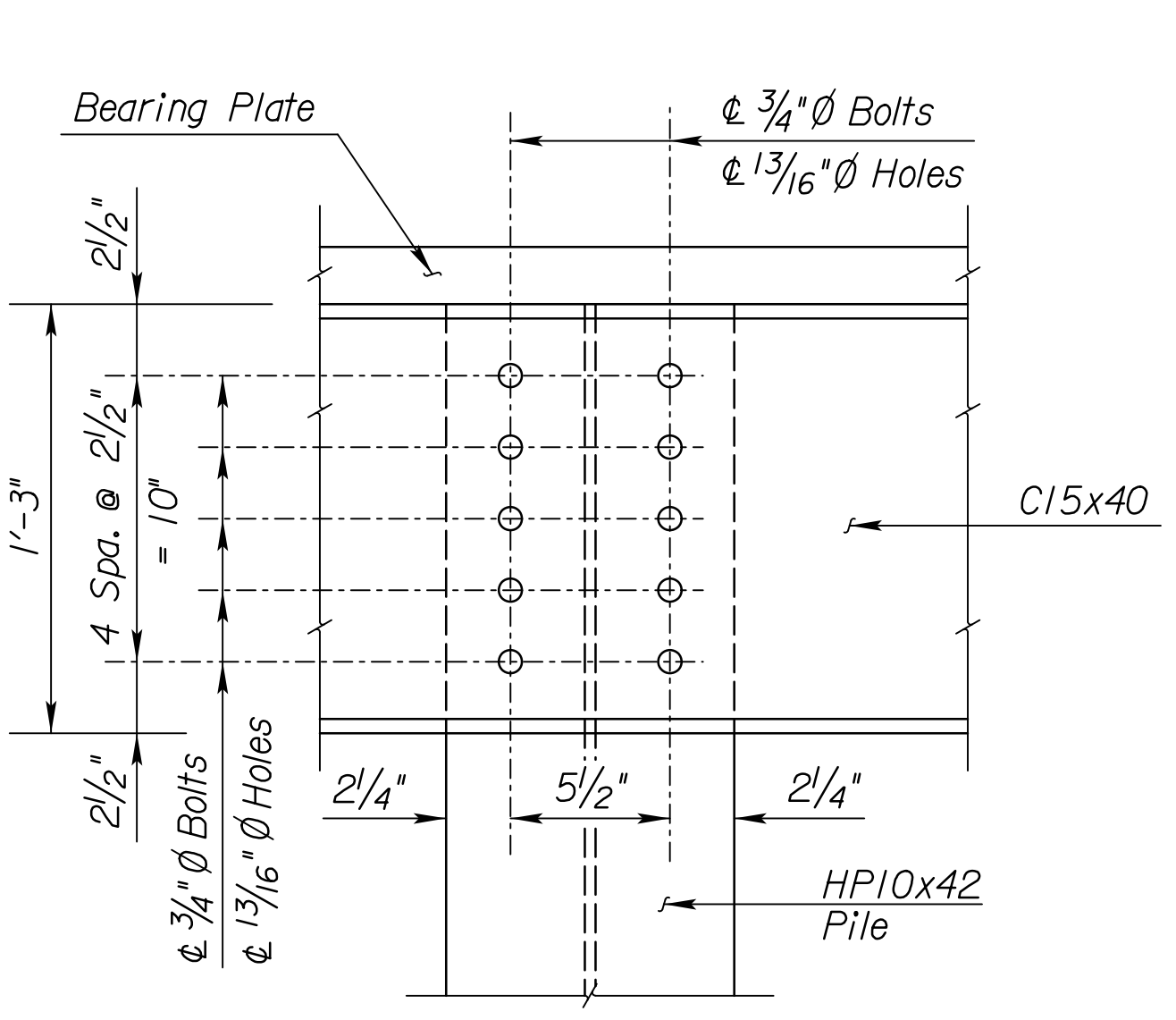
Top of Pile and Berm Elevations:
 Top of Abut. Wingwall Pile:
 Elev. = Cr. Gr. Elev. - 0.17 Ft.
 Top of Bearing Pile (Pier and Abut.):
 Elev. = Cr. Gr. Elev. - 3.29 Ft.
 Max. Berm Elev.:
 Elev. = Cr. Gr. Elev. - 3.96 Ft.



PILE SPLICE DETAILS

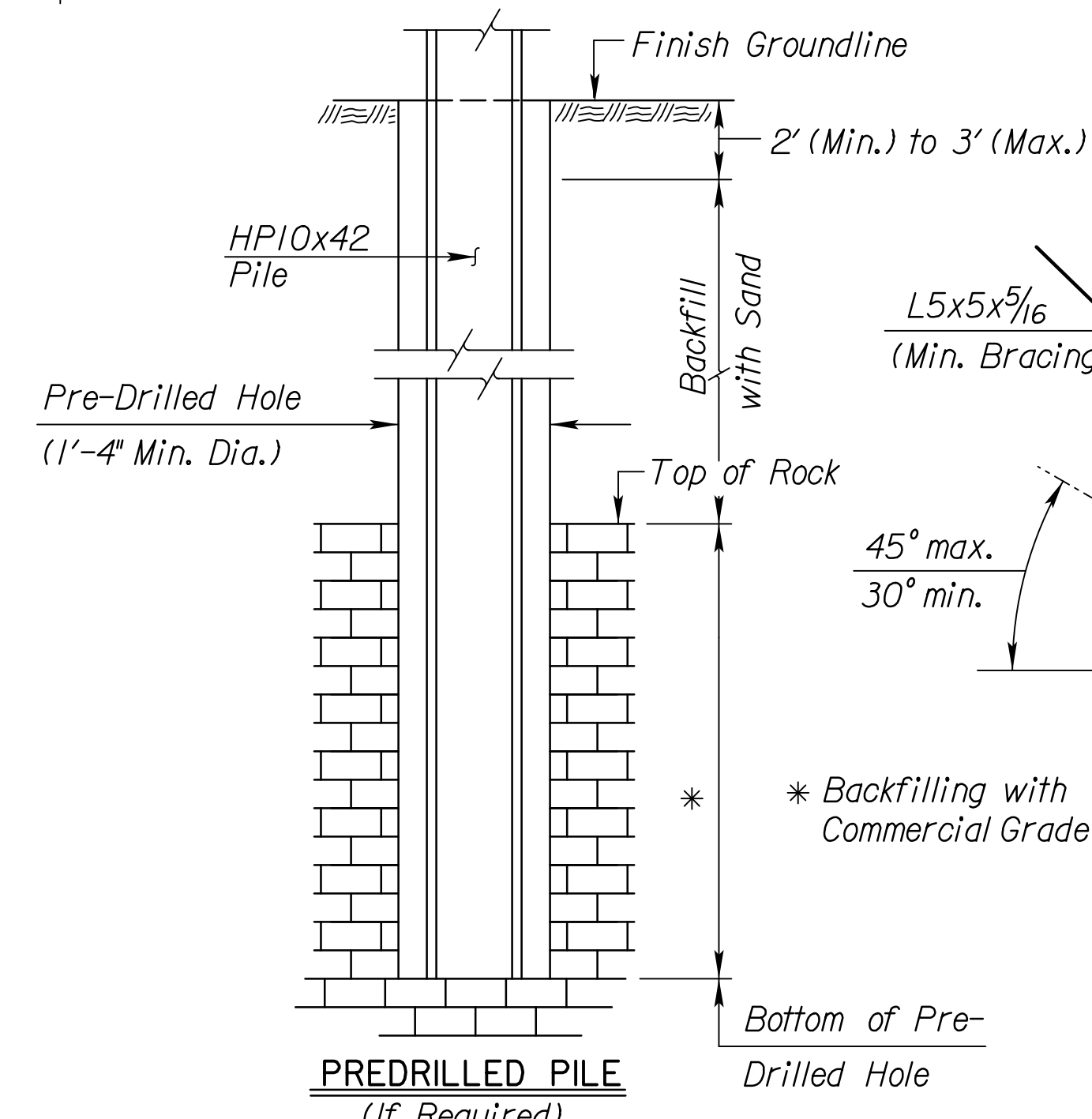


DETAIL C



DETAIL A

† NOTE: The 13/16" diameter bolt hole pattern in the C15x40 channels at the Piling Connections will be used as a template to field drill drill holes through the HPI0x42 flanges.



PREDRILLED PILE (If Required)

All piles shall be set in pre-drilled holes as shown above and when required on the Construction Layout or Geology sheets. Temporary casing should be used to ensure the pre-drilled hole remains stable after being drilled.

DESIGN PILE LOAD:

	Design Load (Tons per Piling)	Allowable Load
Abutment Loading		
Exterior - Group I (100%)	25.3	55.8
Interior - Group I (100%)	24.6	55.8
Pier Loading		
Exterior - Group I (100%)	38.3	55.8
Interior - Group I (100%)	32.9	55.8

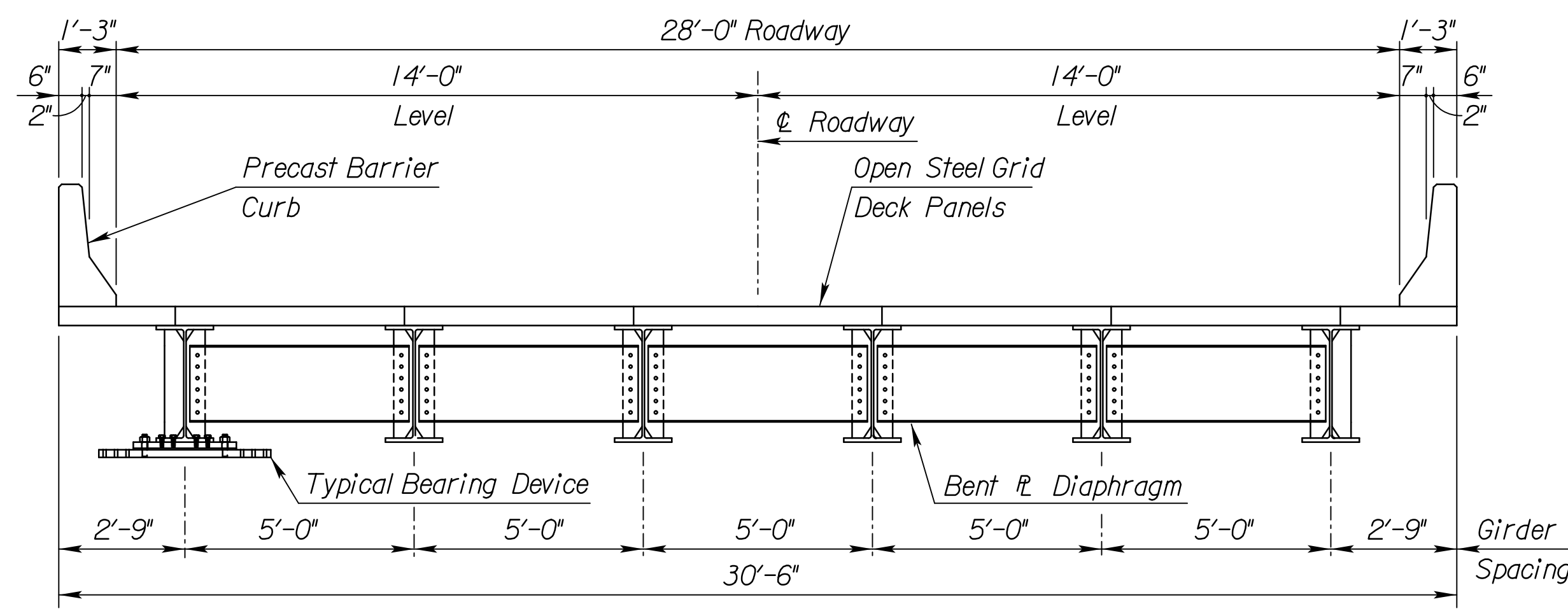
3				
2	6-21-99	Numerous Revisions	GFK	GMC
1	8-18-97	Add Detail 'C' & Revised Note		DRT/GMC
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
 Detour Sta. 78+16.37
SUBSTRUCTURE DETAILS (DETOUR BRIDGE) WITH H-PILE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

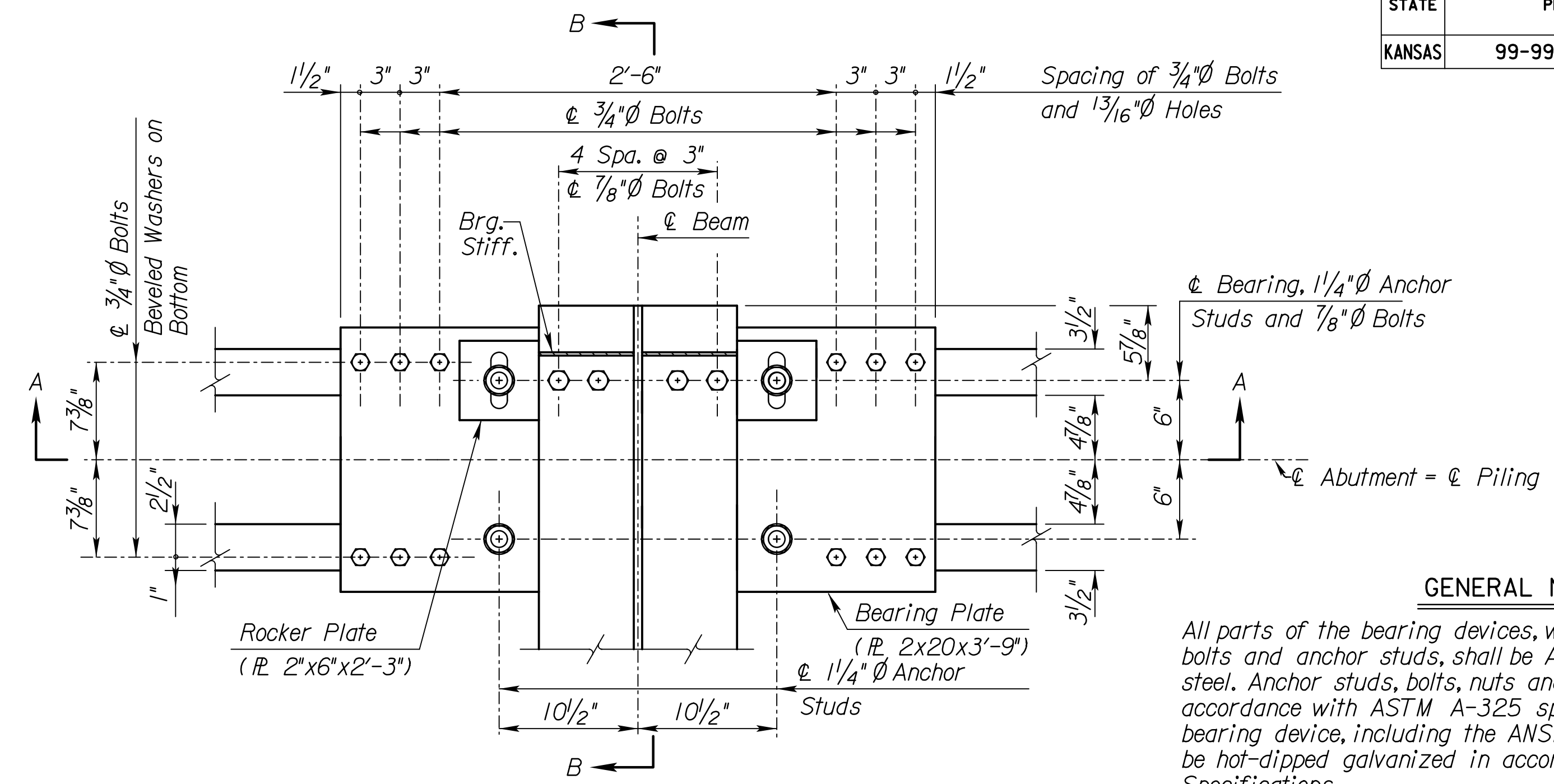
SHEET NO.	OF	SCALE	APP'D
DESIGNED	DETAILED	QUANTITIES	CADD
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.

Std. Base File: k:\bridge\standards\br415d.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\central\01\143371674\kds572801\br113-br415d.dgn
 Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	51	135



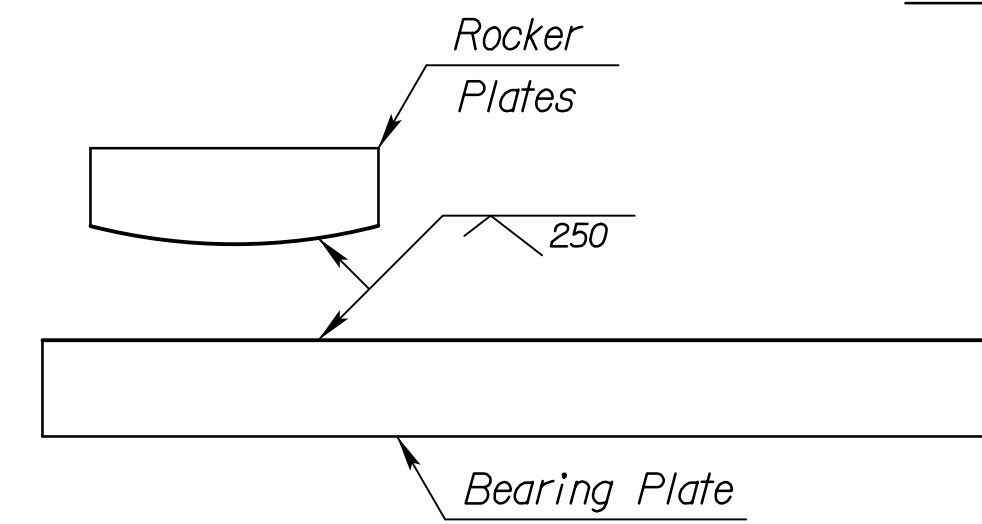
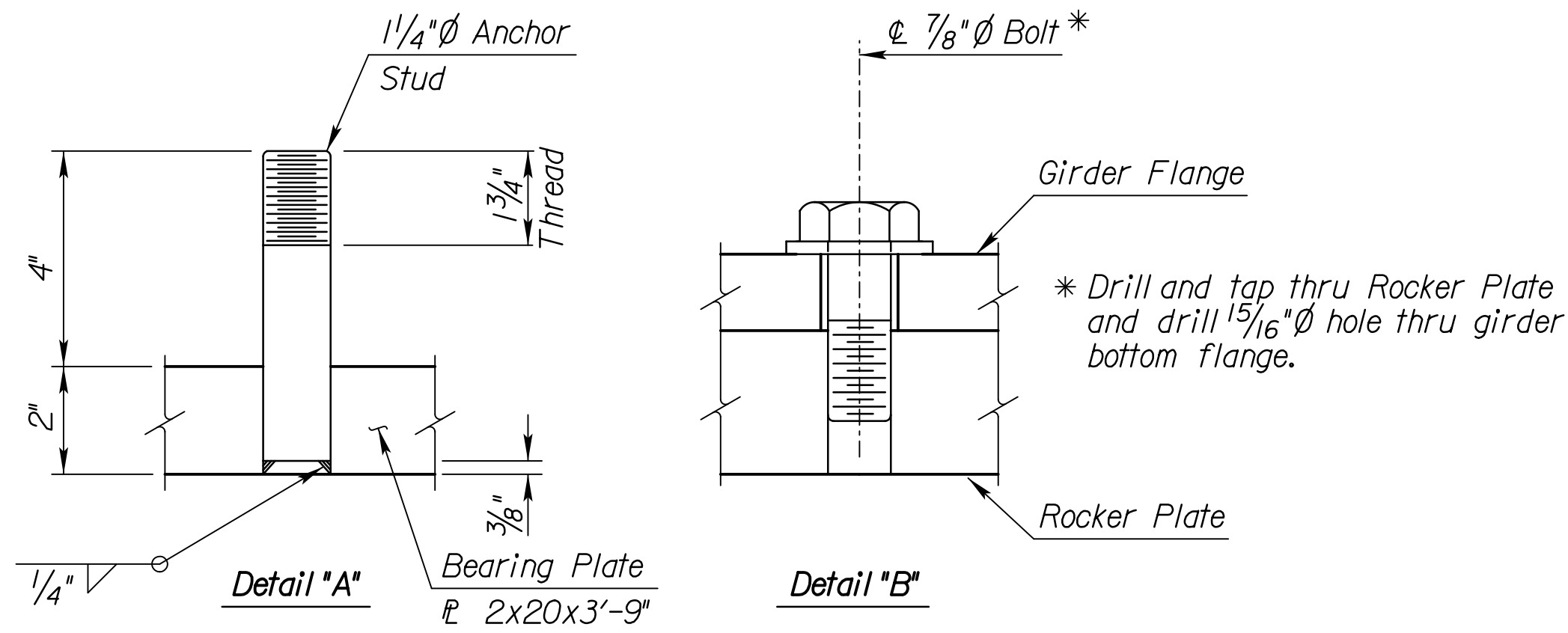
TYPICAL STRUCTURE SECTION



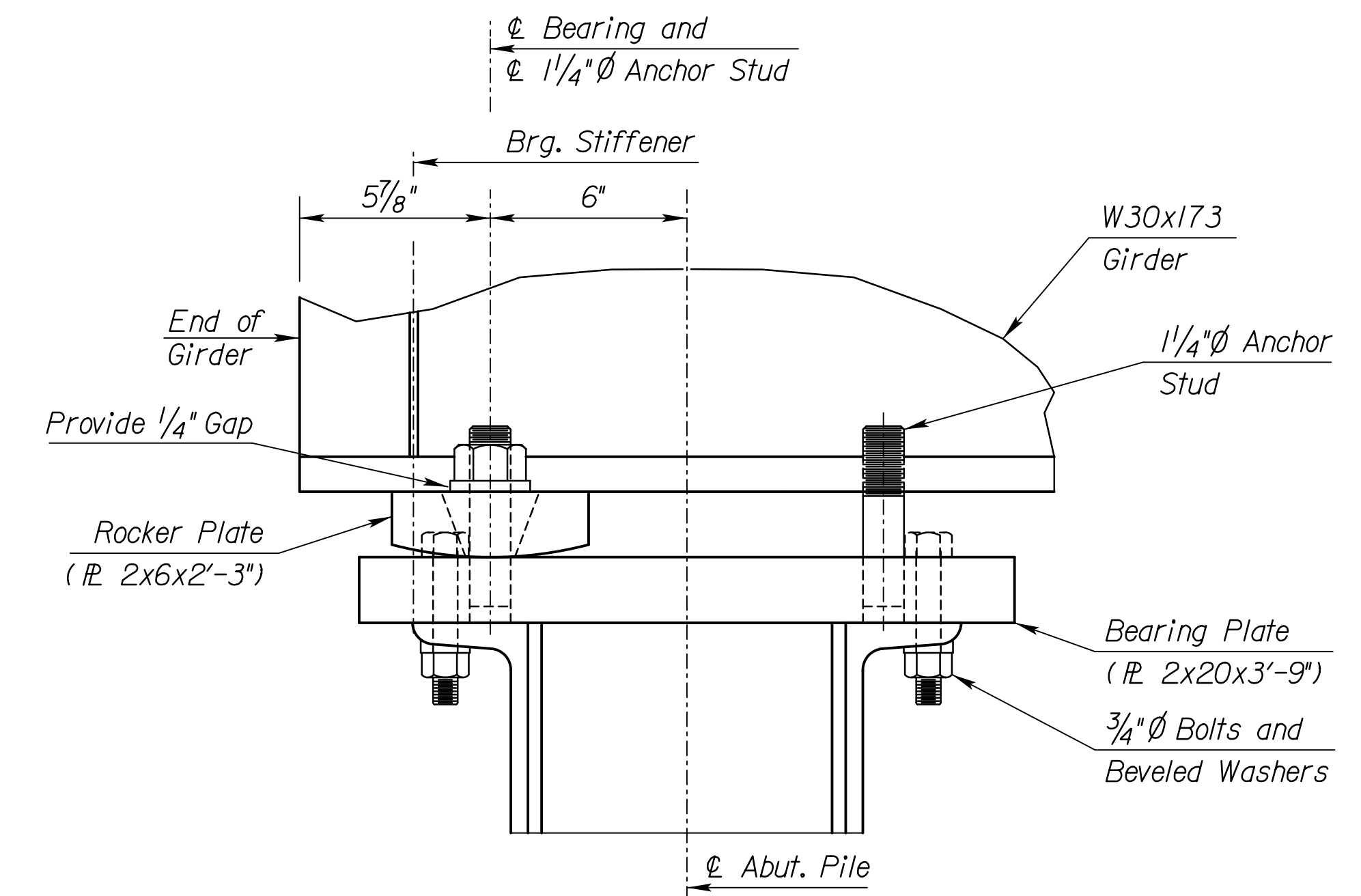
PLAN OF BEARING DEVICE AT ABUTMENT

GENERAL NOTES

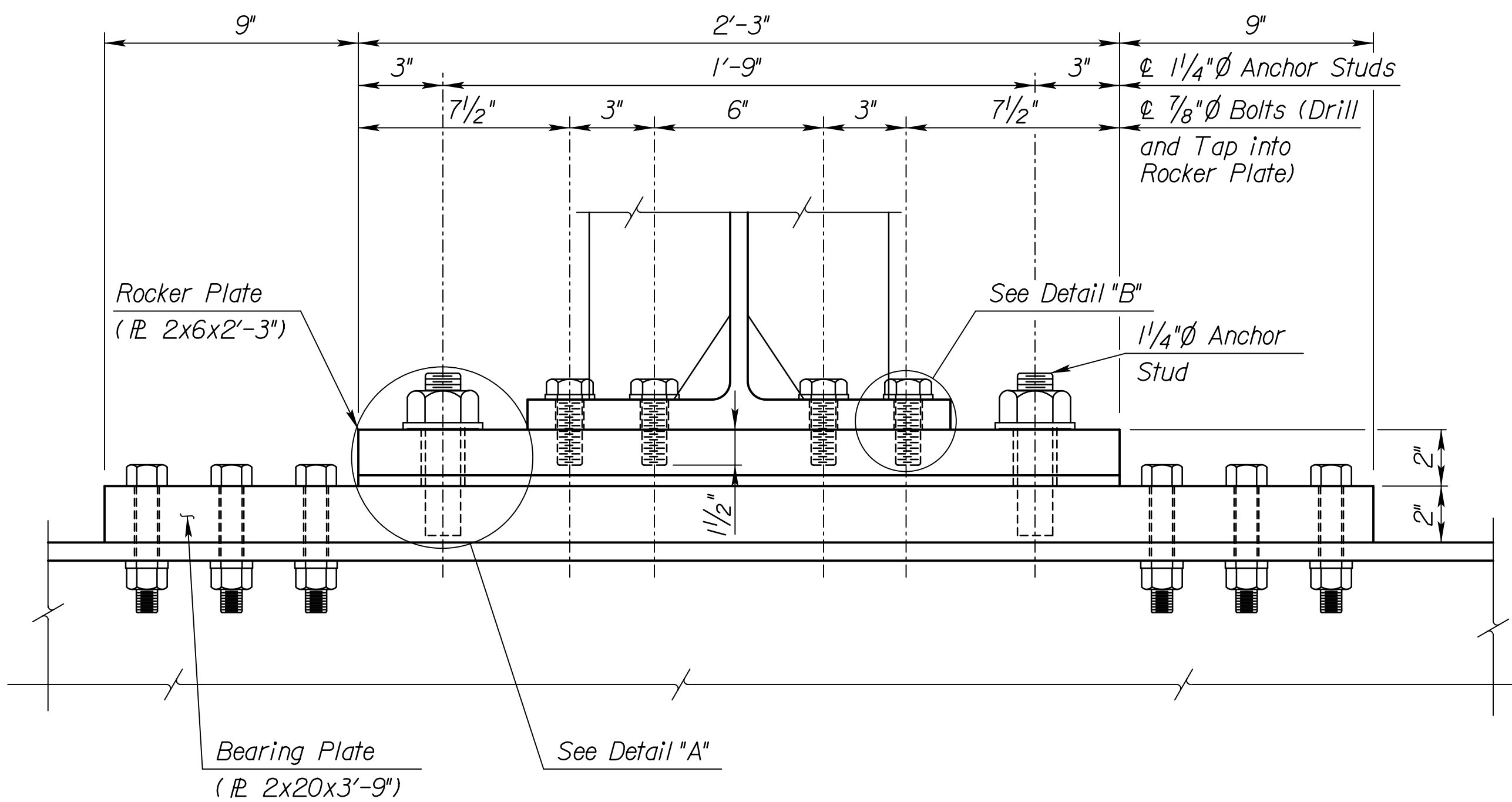
All parts of the bearing devices, with the exception of the bolts and anchor studs, shall be ASTM A709 (GR.36) structural steel. Anchor studs, bolts, nuts and washers shall be in accordance with ASTM A-325 specifications. All parts of the bearing device, including the ANSI 250 finish surfaces, shall be hot-dipped galvanized in accordance with ASTM A-123 Specifications.



TYPICAL FINISH DETAIL



SECTION B-B



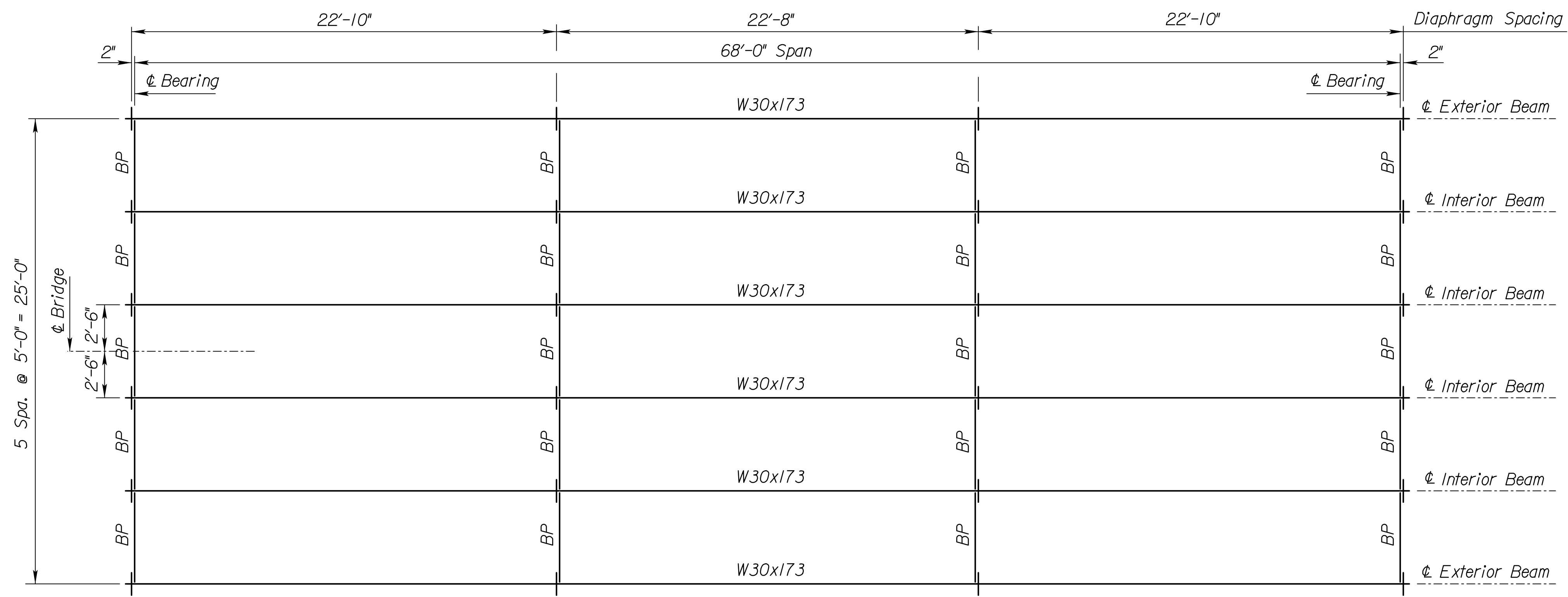
Section A-A

Std. Base File: k:\bridge\standards\br415e.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\central\01\43371674\kds572801\br113-br415e.dgn
 Plot Date: 01-08-24

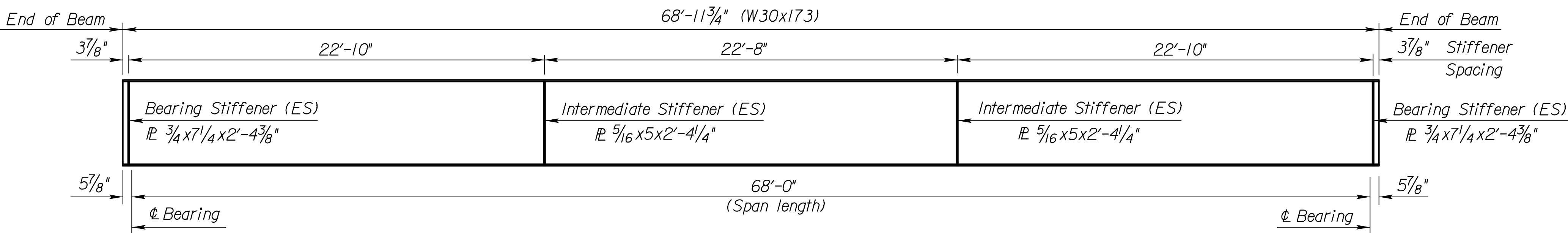
3				
2				
1	6-21-99	Numerous Revisions	GFK	GMC
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
 Detour Sta. 78+16.37
 BEARING DEVICE DETAILS
 (DETOUR BRIDGE)
 WITH H-PILE
 Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

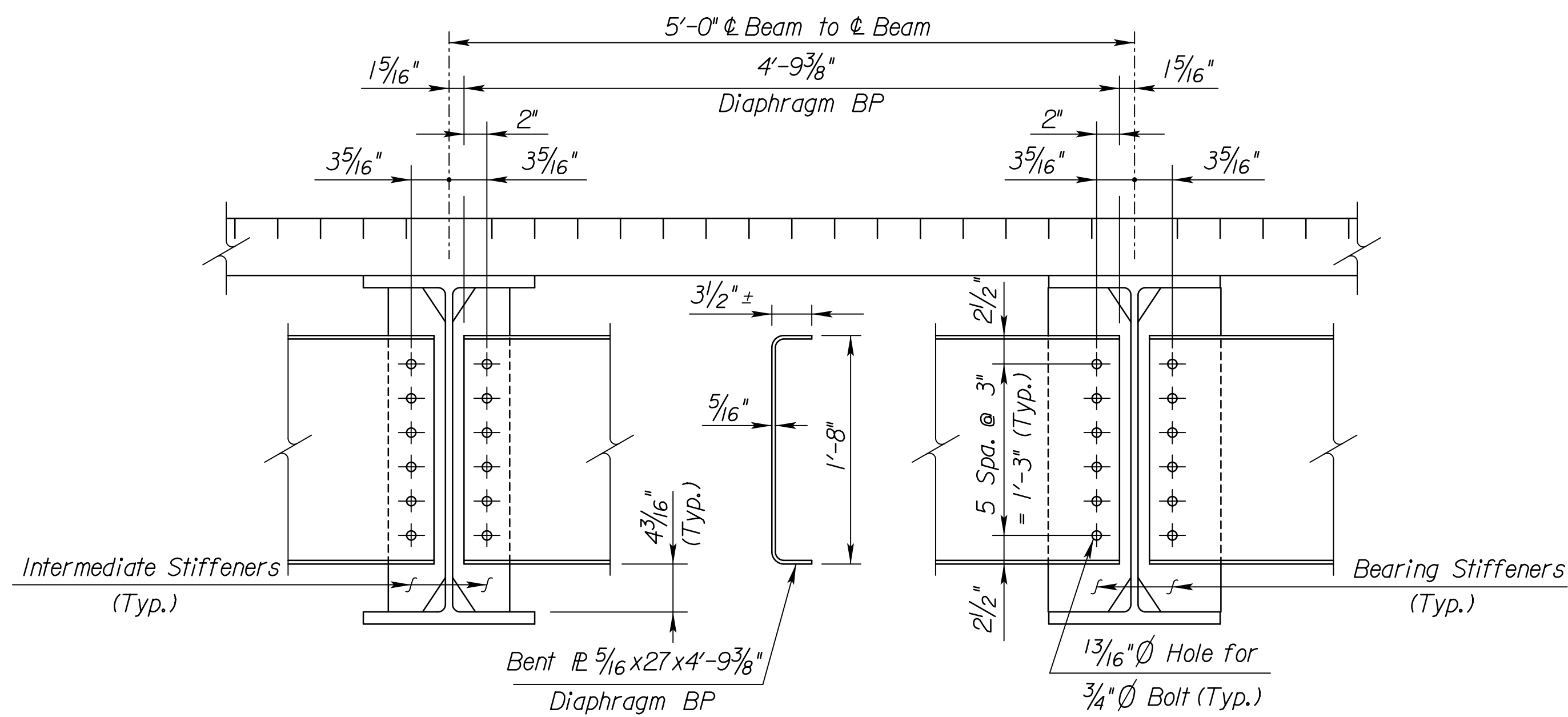
SHEET NO.	OF	SCALE	APP'D
DESIGNED	BY	DATE	QUANTITIES
DESIGN CK.	BY	DATE	CADD CK.



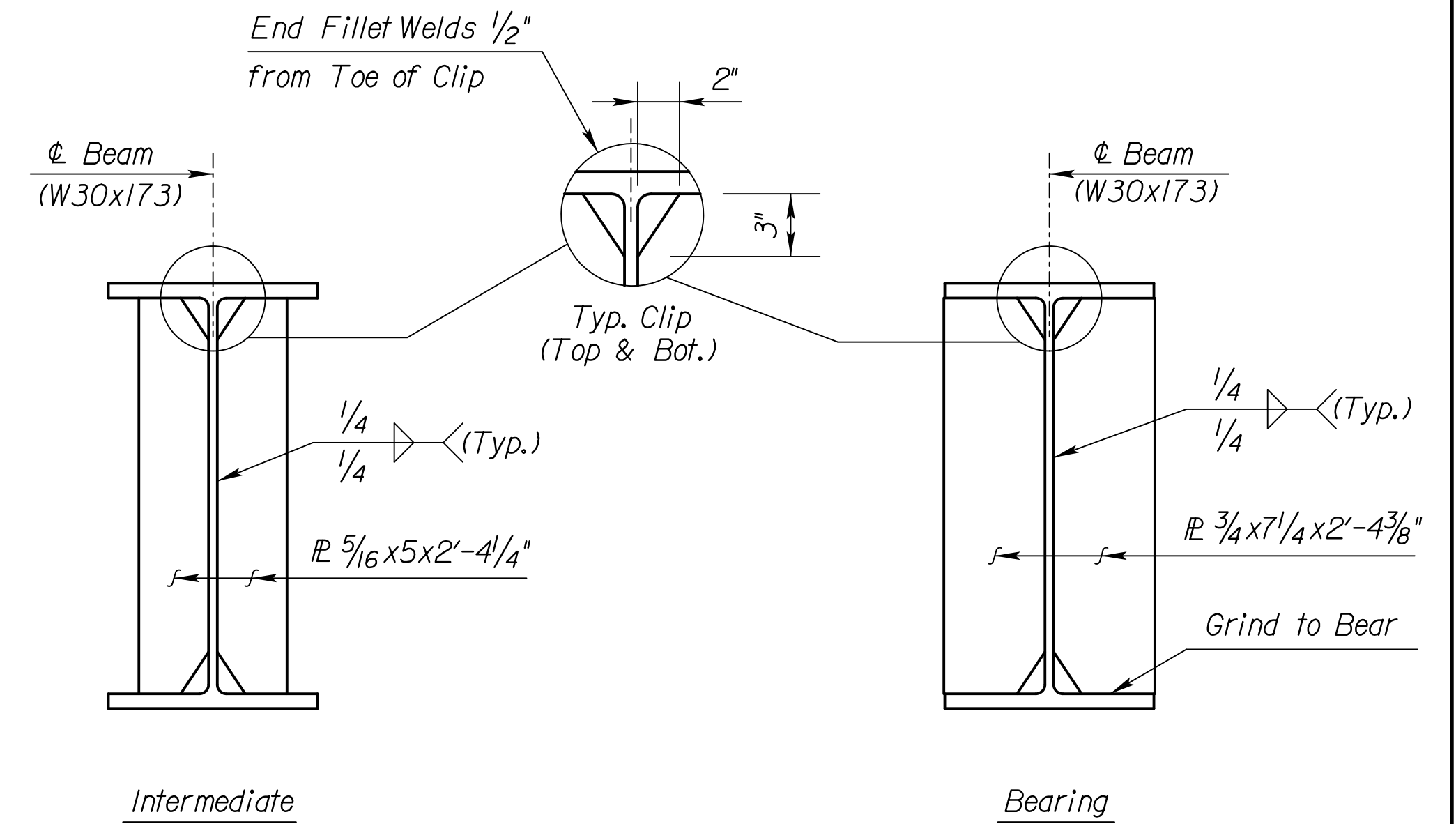
FRAMING PLAN



**TYPICAL ELEVATION OF BEAM
(W30x173)**



BENT PLATE DIAPHRAGM BP DETAILS



STIFFENER DETAILS

GENERAL NOTES

- 1) Any natural camber in the rolled beams shall be placed upward for fabrication.
- 2) All stiffeners shall be placed perpendicular to the bottom flange. Magnetic particle inspection shall be required at the bearing stiffeners.

**BILL OF MATERIAL-STRUCTURAL STEEL
(One span Only)**

No.	Item	Weight - Lbs.
6	W30x173 @ 68'-11 3/4"	71,600
24	PL 3/4 x 7 1/4 x 2'-4 3/8"	1,020
24	PL 5/16 x 5 x 2'-4 1/4"	290
20	Bent PL 5/16 x 27 x 4'-9 3/8"	2,740
12	Bearing Plates w/Anchor Studs	6,140
12	Rocker Plates	980

LEGEND

(ES) Each Side

NOTE: See "Abutment Details" sheet for details required to fasten 4"x4" and 4"x6" timbers to the abutment stiffeners. The bolt holes required in each abutment stiffener shall be drilled in the shop.

3				
2				
1	6-21-99	Numerous Revisions	GFK	GMC
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
Detour Sta. 78+16.37

FRAMING PLAN AND DIAPHRAGM DETAILS (DETOUR BRIDGE)

Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	BY	DATE	SCALE
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.

Std. Base File: k:\bridge\standards\br414a.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\central\01\43371674\kds72801\br113-br414a.dgn
 Plot Date: 01-08-24

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	53	135

GENERAL NOTES FOR FABRICATION

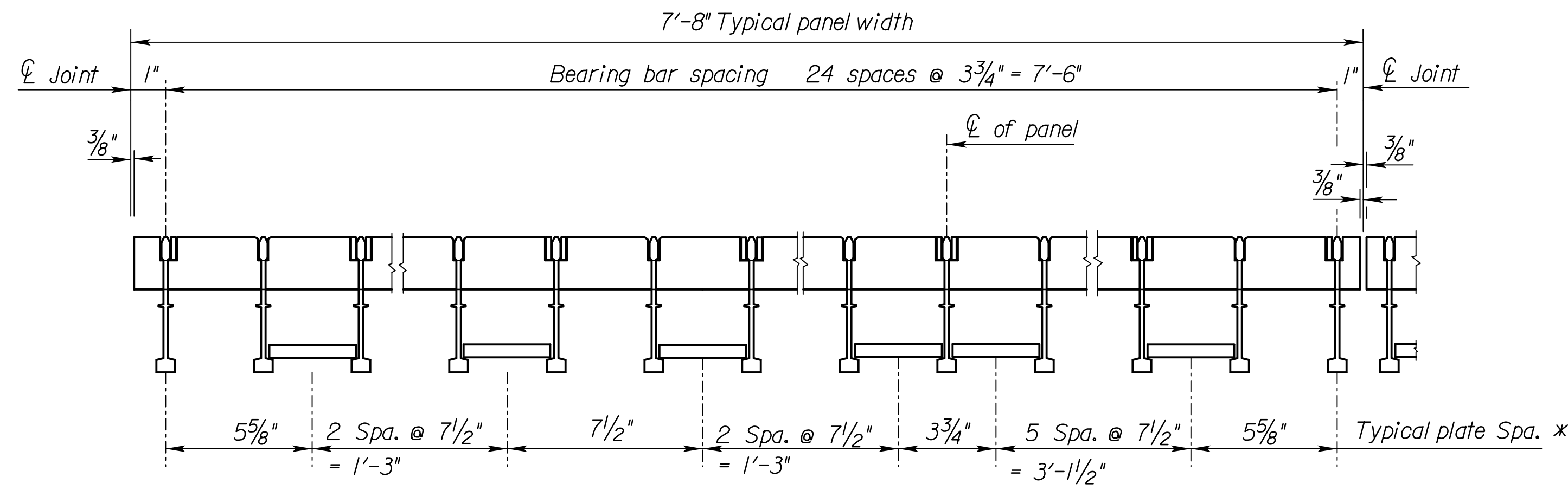
PANELS: Panels shall be 30'-6" in length with no splices parallel to traffic. If the centerline of a transverse joint is more than 1" from a bearing bar, end trim bars are required. The steel grid floor system shall consist of bars or beams perpendicular to traffic (bearing bars) and bars parallel to traffic (cross bars). Diagonal bars are required in all sections of the grid for stability. The system shall be designed to meet the current criteria of AASHTO Specifications for highway bridges. Live load shall be HS20-44. Dead weight of the curb is 240 lbs. per lin. ft.

After fabrication, the panels shall be hot-dipped galvanized in accordance with ASTM A-123 Specifications.

Details of the system and design calculations shall be submitted to the Engineer for review and approval. Details shown in the plans, representing the steel grid deck, are not meant to imply any given manufactured bridge floor system.

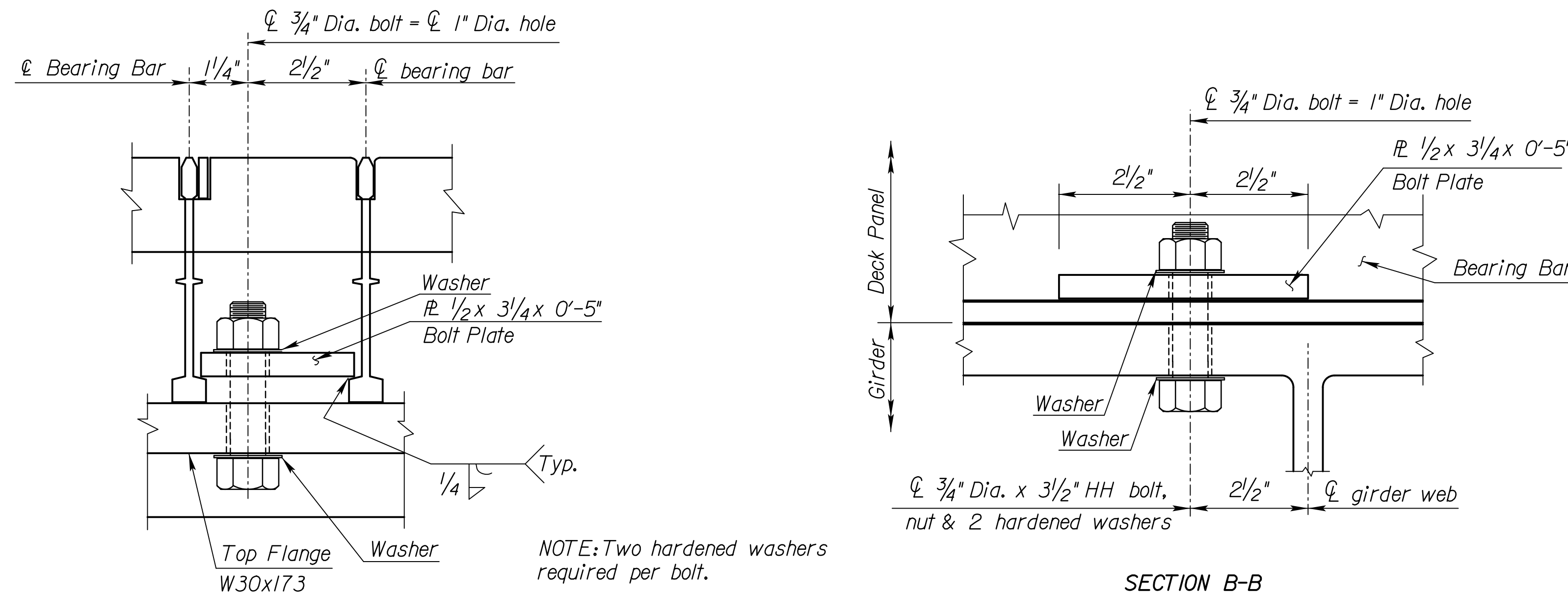
GENERAL NOTES FOR CONSTRUCTION

Panels are to be installed at right angles to the centerline of roadway.



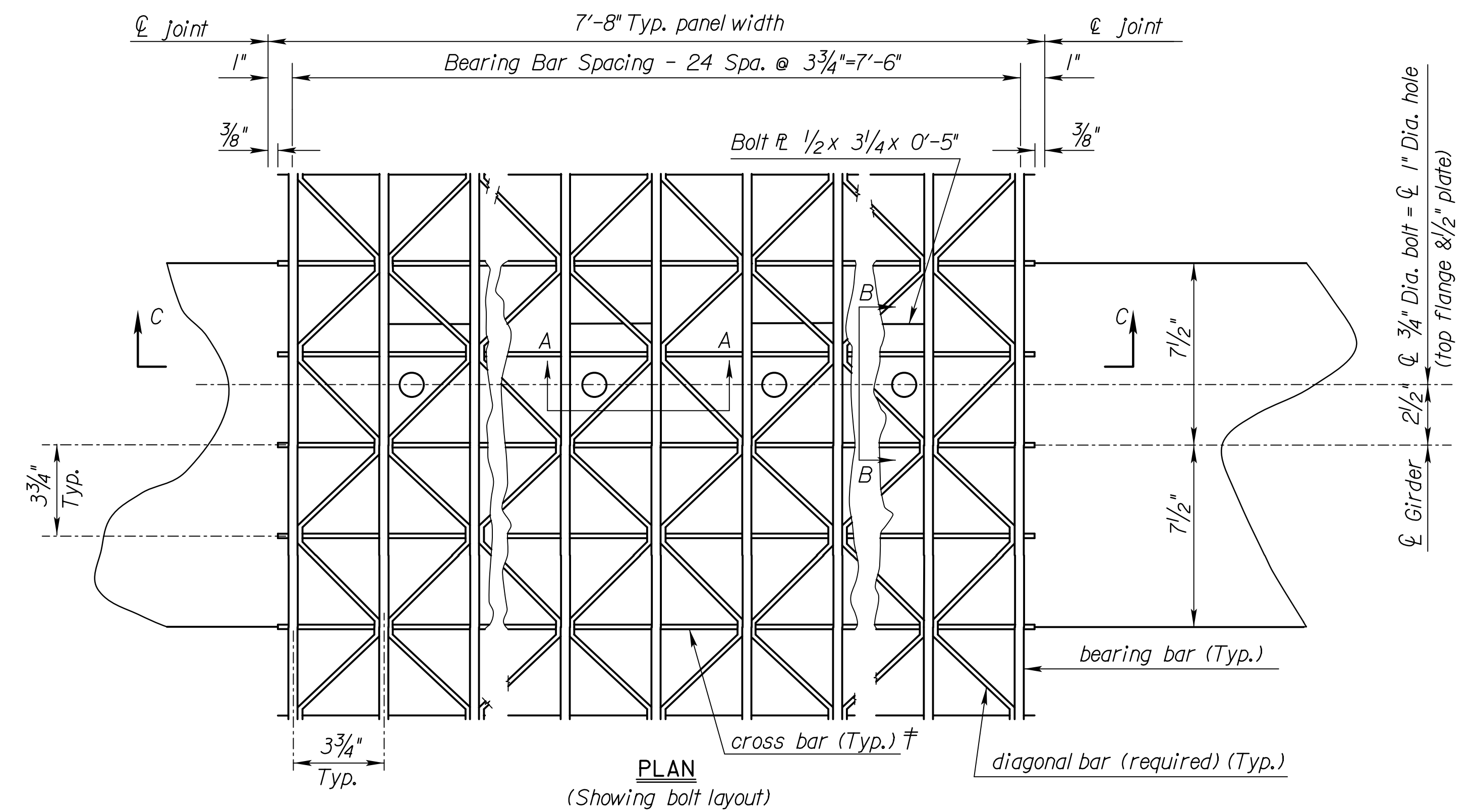
* All bearing bars shall be supported on one side or both sides by 1/2" Bolt PL's.

SECTION C-C
(Deck Panel Only)



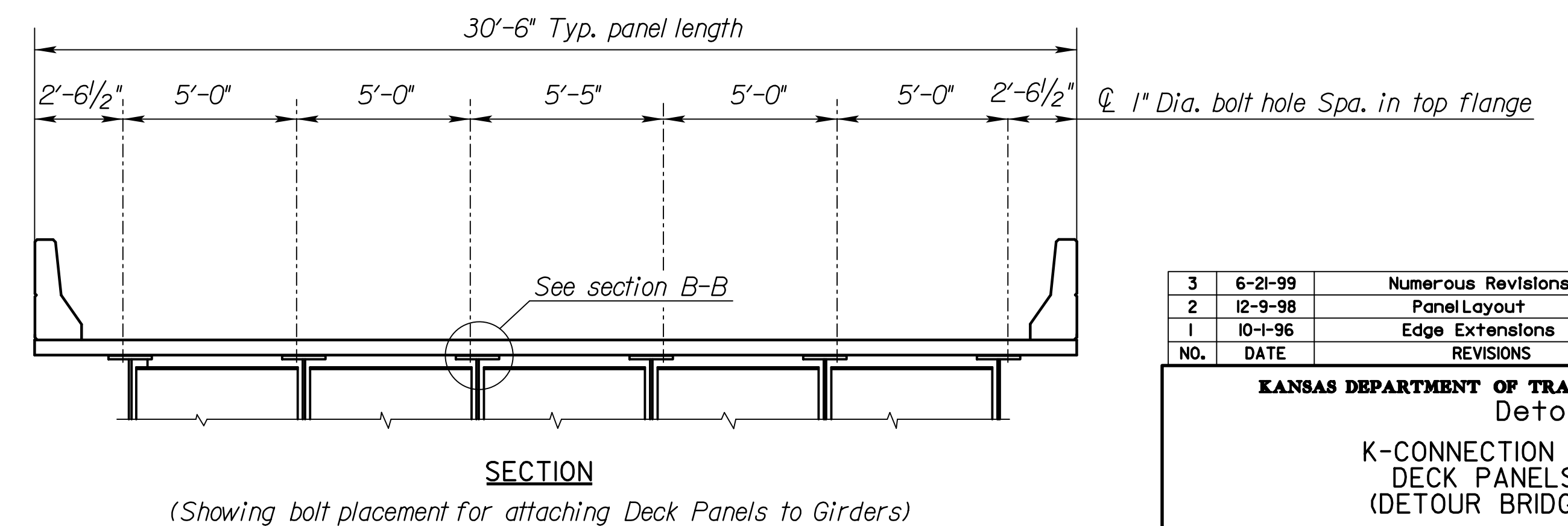
NOTE: Two hardened washers required per bolt.

SECTION B-B

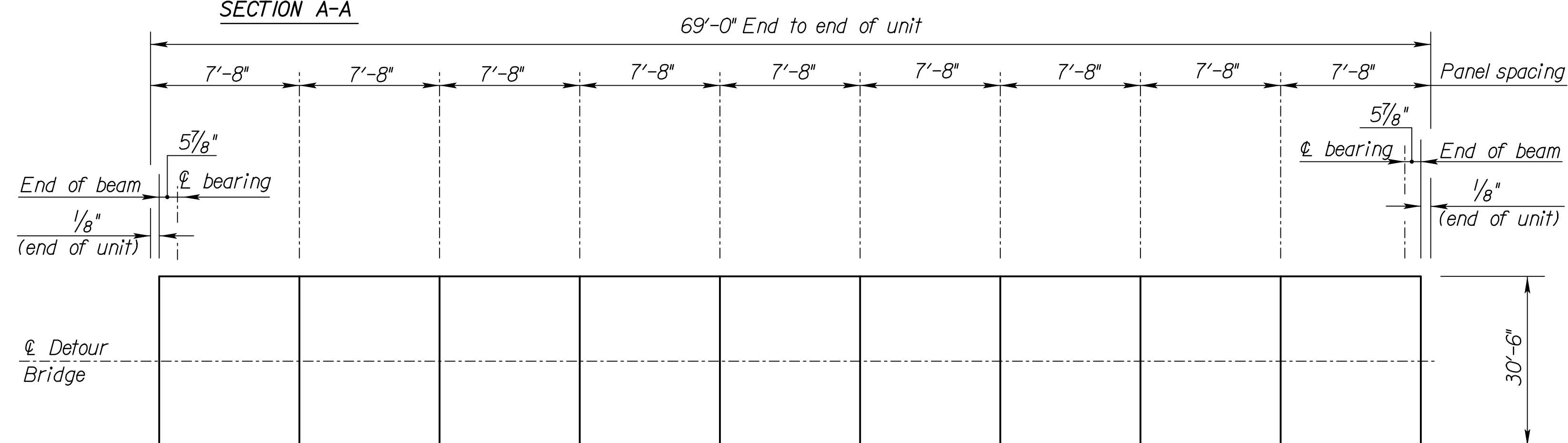


PLAN
(Showing bolt layout)

‡ Cross bars of grid to match edges of flange as shown.



SECTION
(Showing bolt placement for attaching Deck Panels to Girders)



PANEL LAYOUT
Showing bolt placement in top flange

Bolt hole spacing in top flange for each panel unit (1" Dia. holes for 3/4" Dia. bolts)
6" | 5 Spacing @ 7 1/2" | 5" | 5 Spacing @ 7 1/2" | 6"
= 3'-1 1/2" | = 3'-1 1/2"

Std. Base File: k:\bridge\standards\br414b.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\ventral\0\143371674\kds72801\br113-br414b.dgn
 Plot Date: 01-08-24

3	6-21-99	Numerous Revisions	GFK	GMC
2	12-9-98	Panel Layout		DRT/GMC
1	10-1-96	Edge Extensions		
NO.	DATE	REVISIONS	BY	APP'D

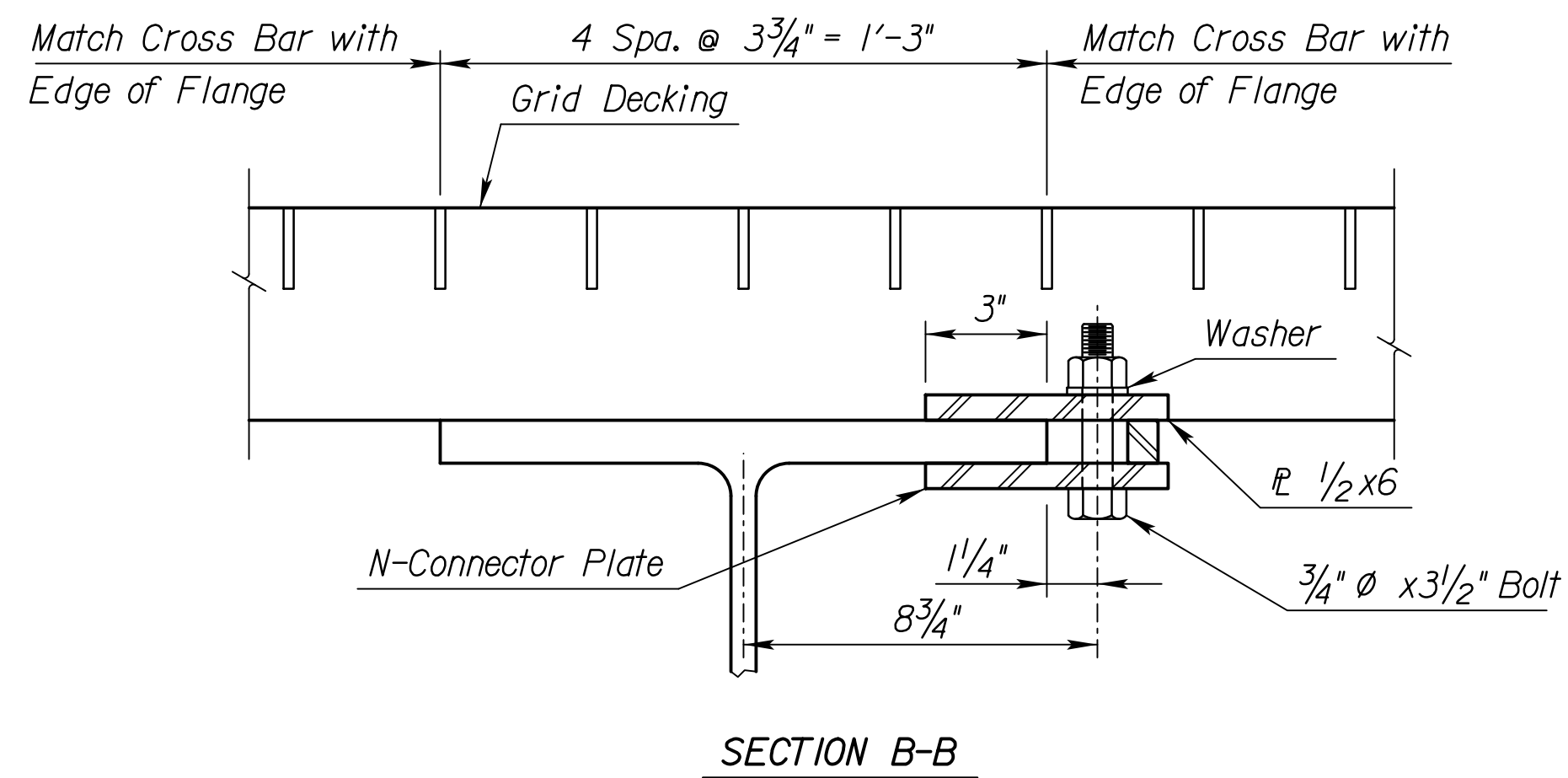
KANSAS DEPARTMENT OF TRANSPORTATION
Detour Sta. 78+16.37

K-CONNECTION OF DECK PANELS (DETOUR BRIDGE)

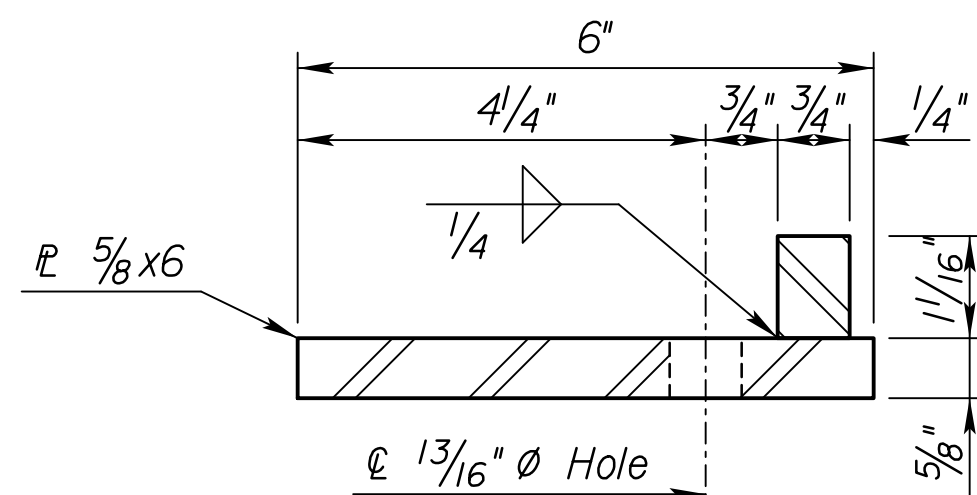
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	BY	DATE	QUANTITIES
DESIGN CK.	BY	DATE	CADD CK.

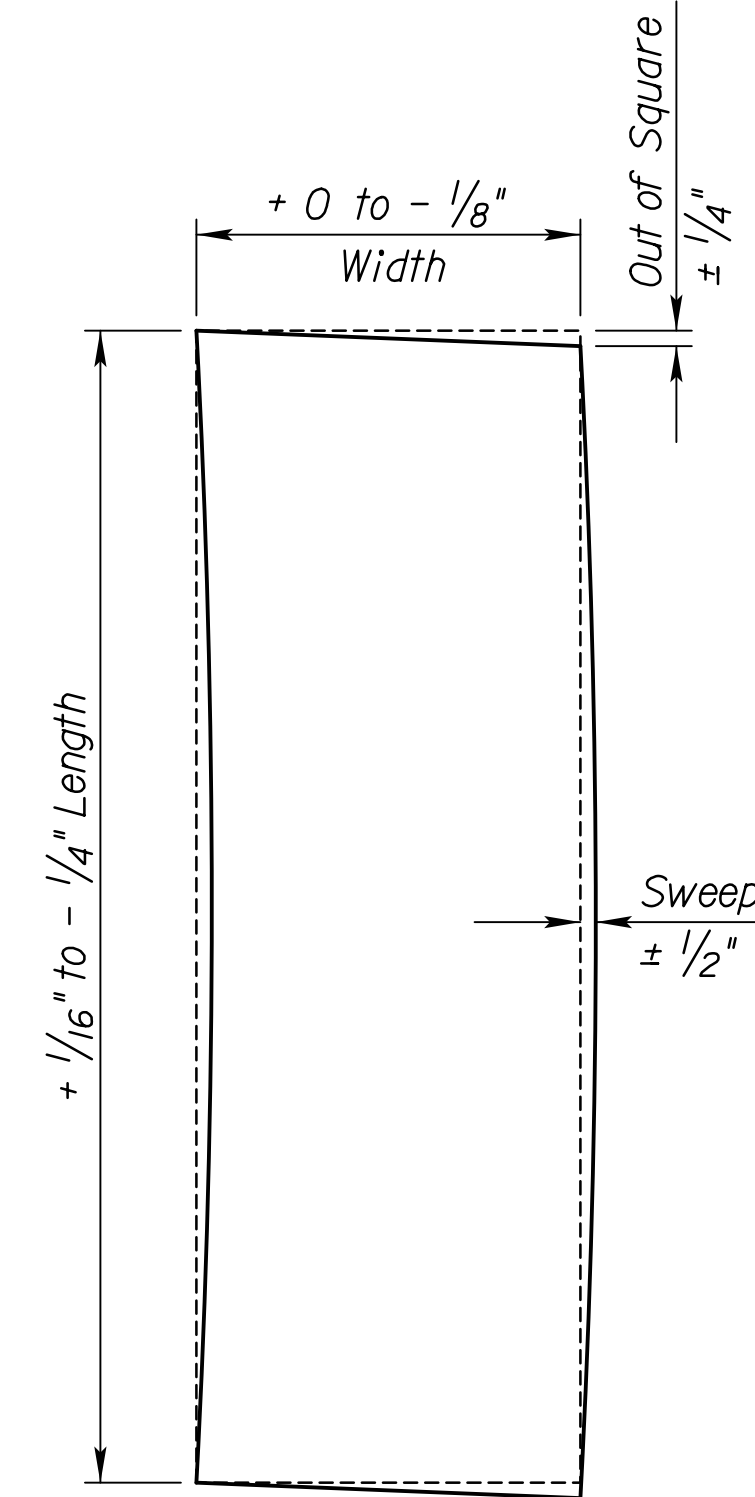
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	54	135



SECTION B-B



N-CONNECTOR PLATE



FLOOR PANEL TOLERANCES

GENERAL NOTES FOR FABRICATION

PANELS: Panels shall be 30'-6" in length with no splices parallel to traffic. If the centerline of a transverse joint is more than 1" from a bearing bar, end trim bars are required. The steel grid floor system shall consist of bars or beams perpendicular to traffic (bearing bars) and bars parallel to traffic (cross bars). Diagonal bars are required in all sections of the grid for stability. The system shall be designed to meet the current criteria of AASHTO Specifications for highway bridges. Live load shall be HS20-44. Dead weight of the curb is 240 lbs. per lin. ft.

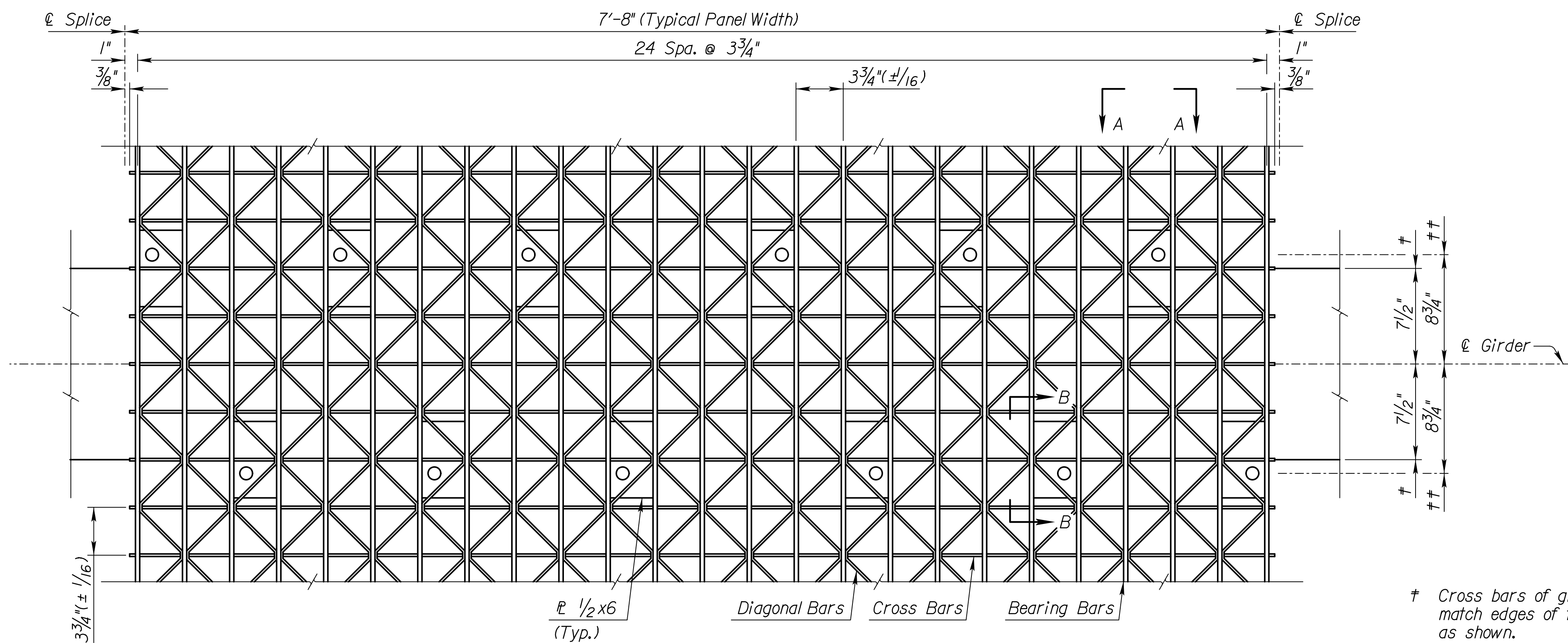
After fabrication, the panels shall be hot-dipped galvanized in accordance with ASTM A-123 Specifications.

Details of the system and design calculations shall be submitted to the Engineer for review and approval. Details shown in the plans, representing the steel grid deck, are not meant to imply any given manufactured bridge floor system.

The steel grid floor N-Panels shall be fabricated with both K-Connection and N-Connection bolt plates.

GENERAL NOTES FOR CONSTRUCTION

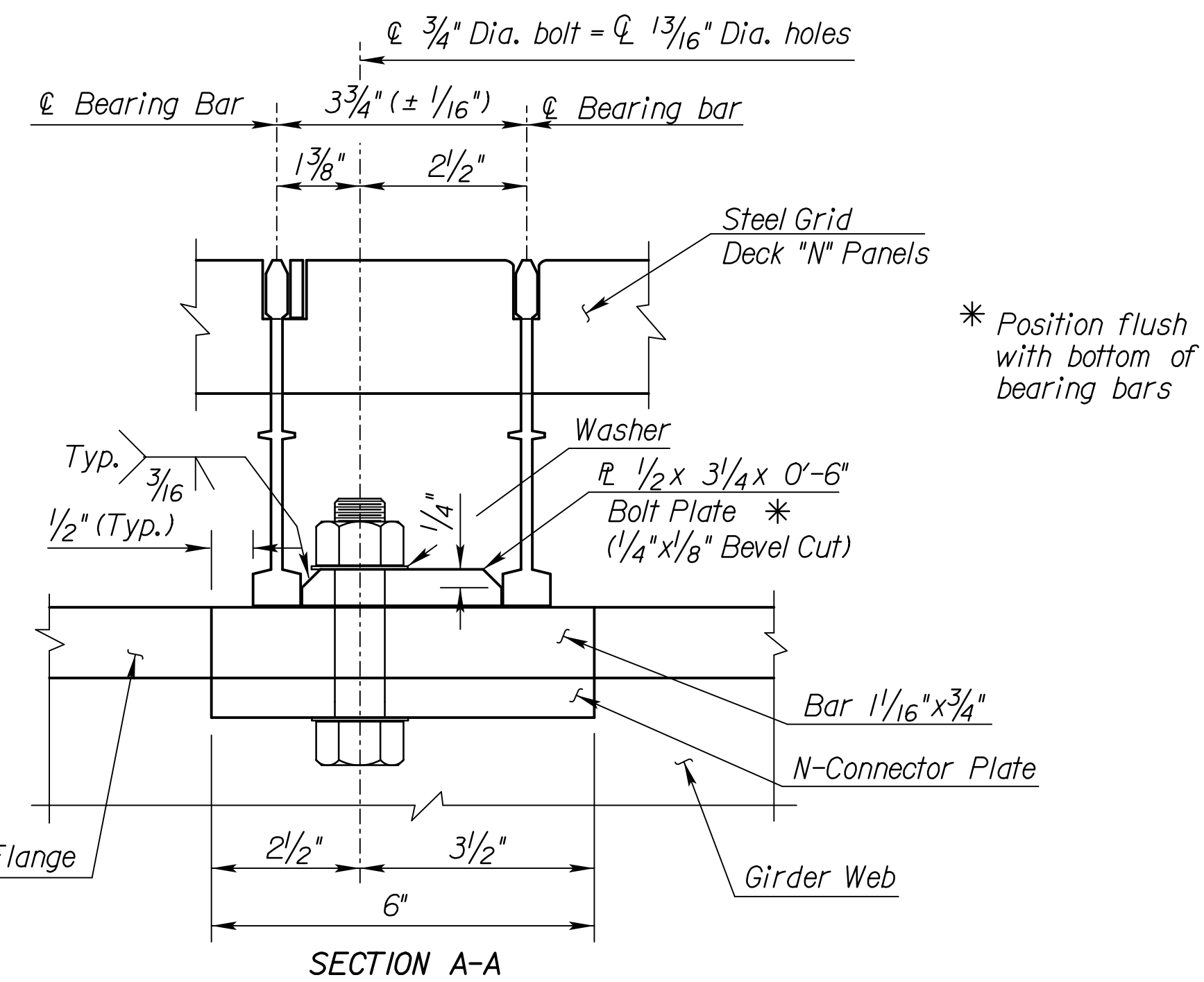
PANELS: Panels are to be installed at right angles to the centerline of the roadway.



PLAN VIEW OF PANEL WIDTH WITH TYPICAL BOLTING PATTERN

Cross bars of grid to match edges of flange as shown.

3/4" dia. bolt = 1 3/16" dia. bolt hole in 1/2 x 6.



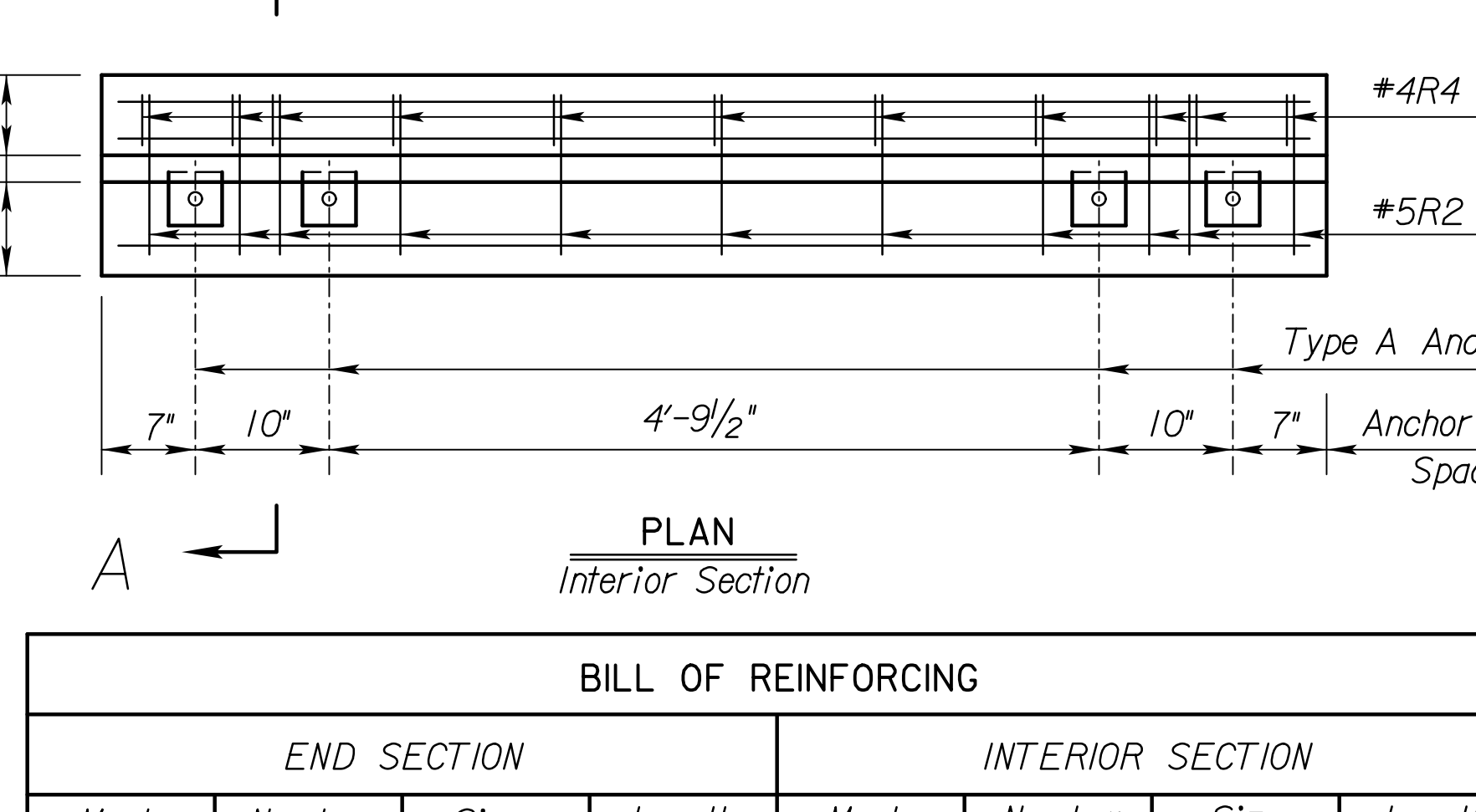
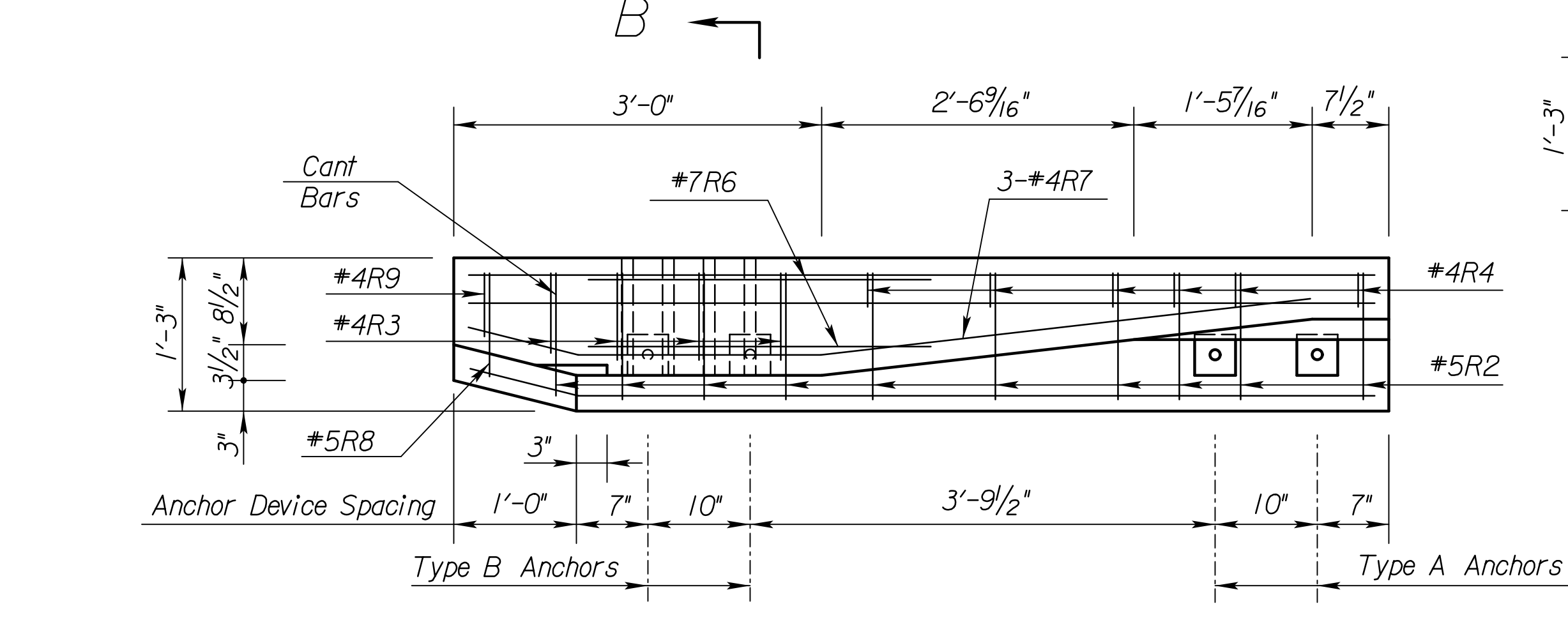
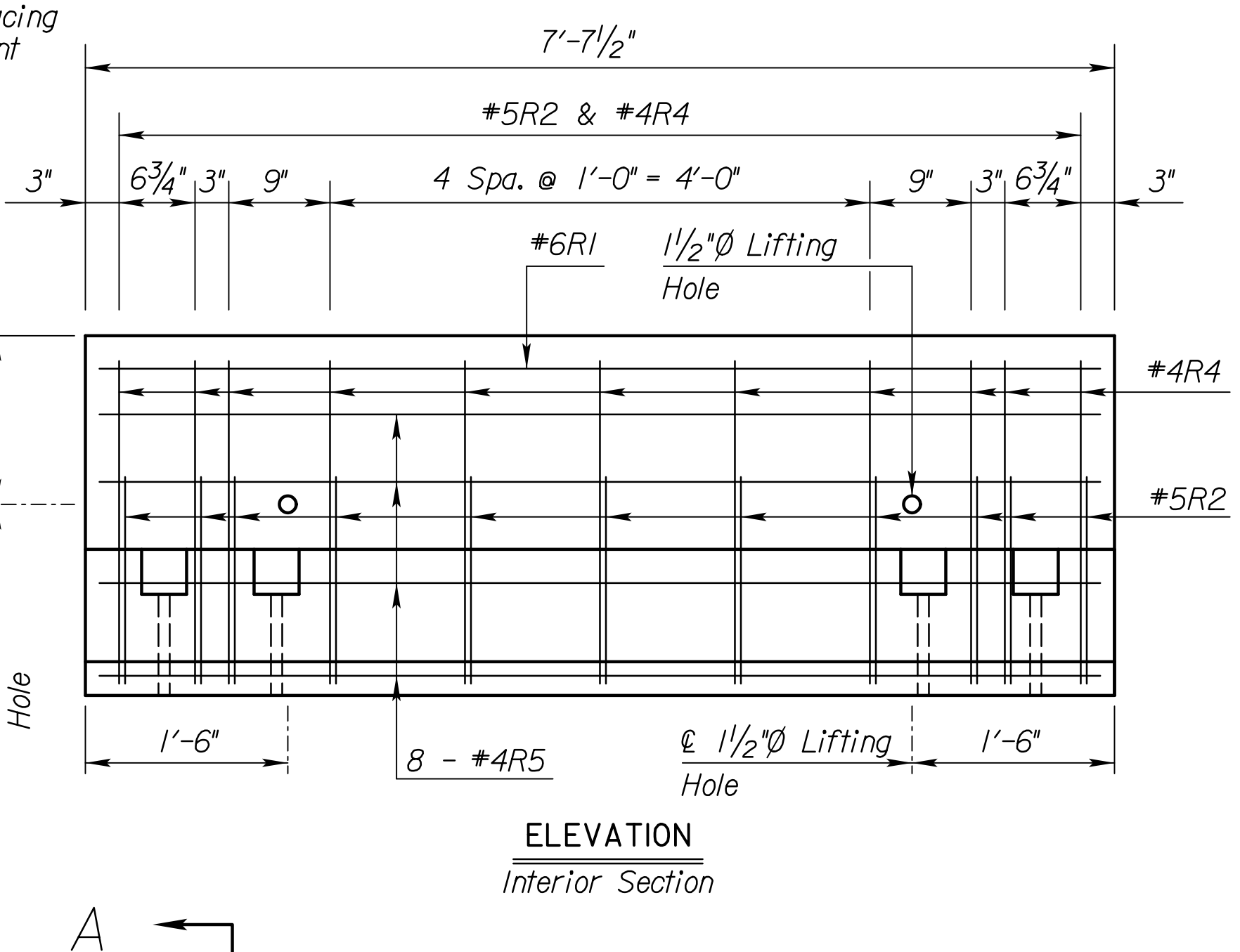
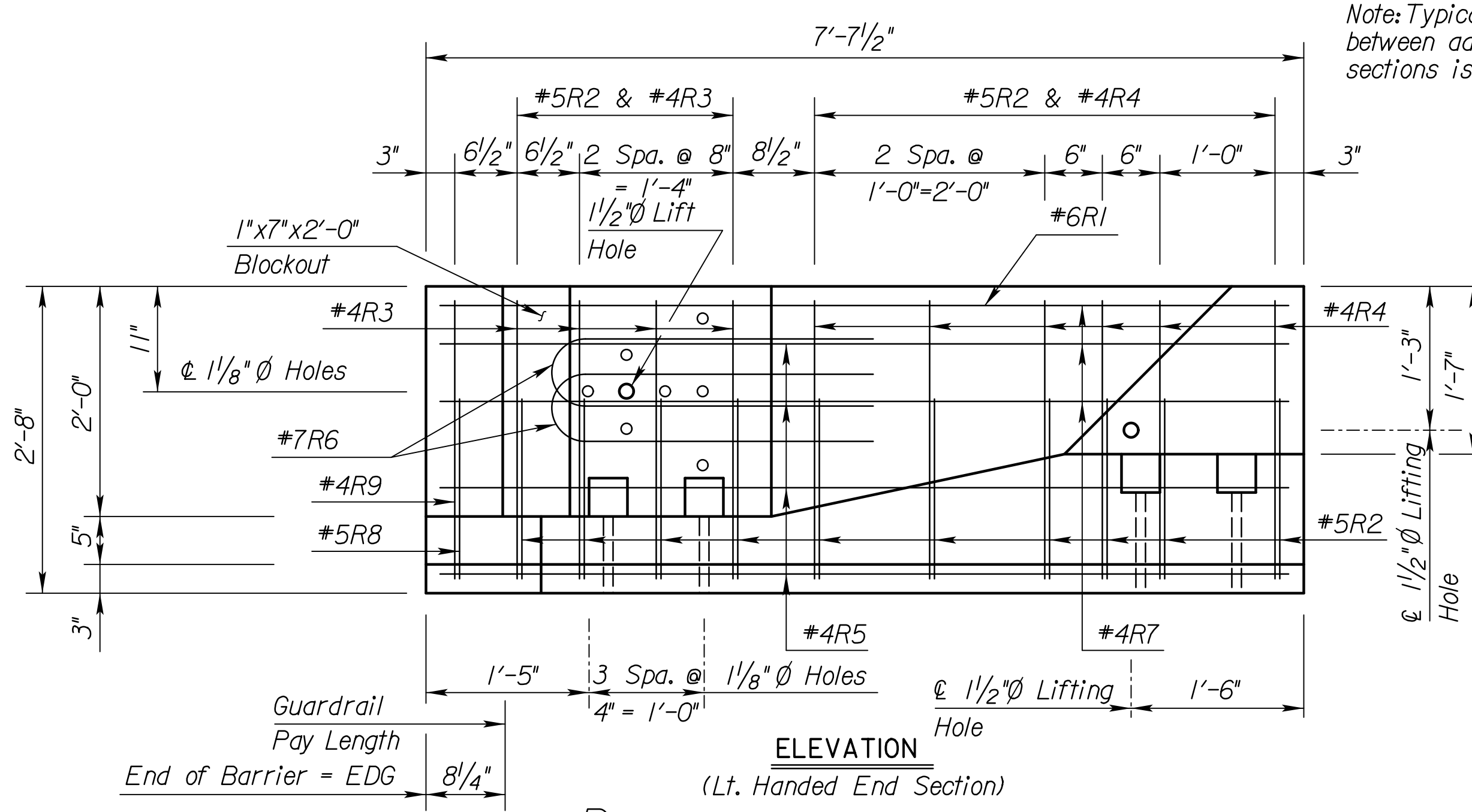
SECTION A-A

3				
2	6-21-99	Numerous Revisions	GFK	GMC
1	10-1-96	K & N Connection Bolt Plates		
NO.	DATE	REVISIONS	BY	APP'D

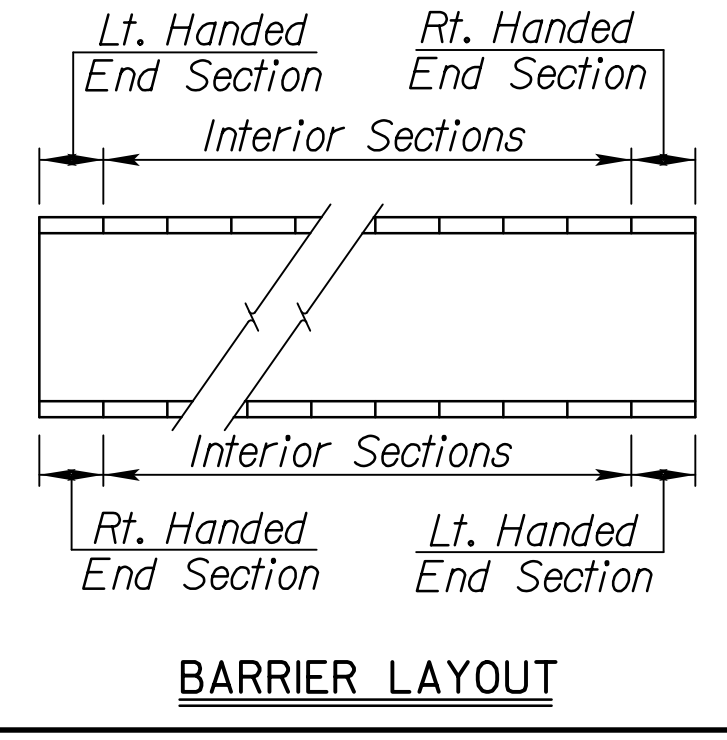
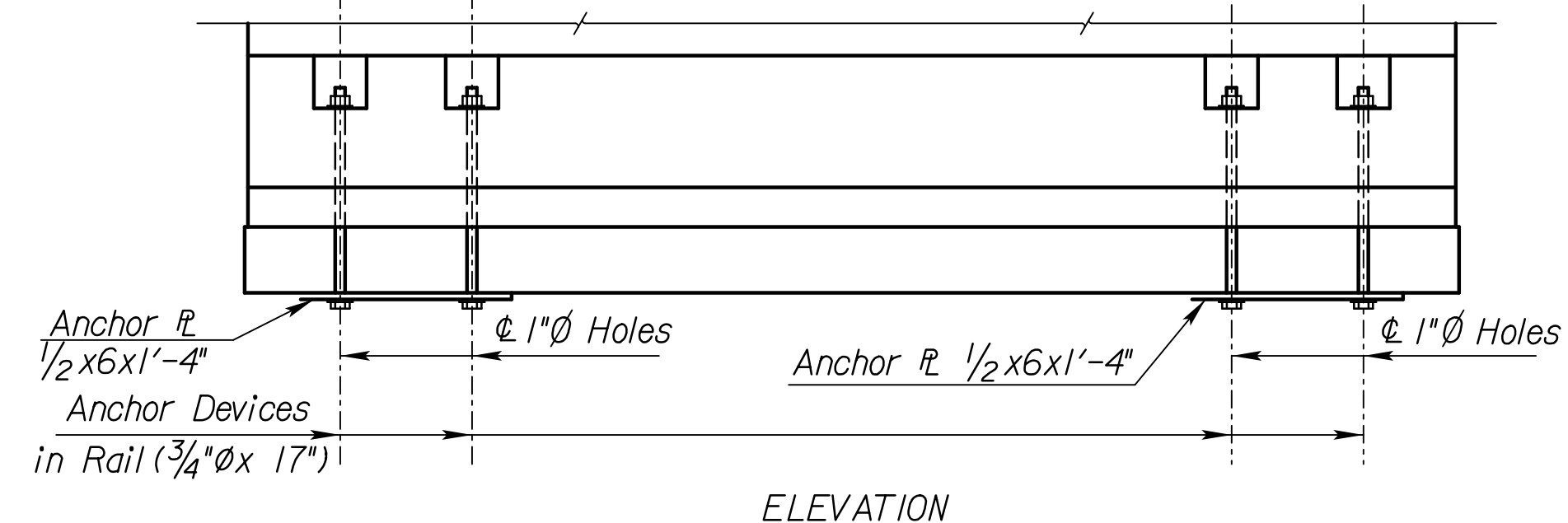
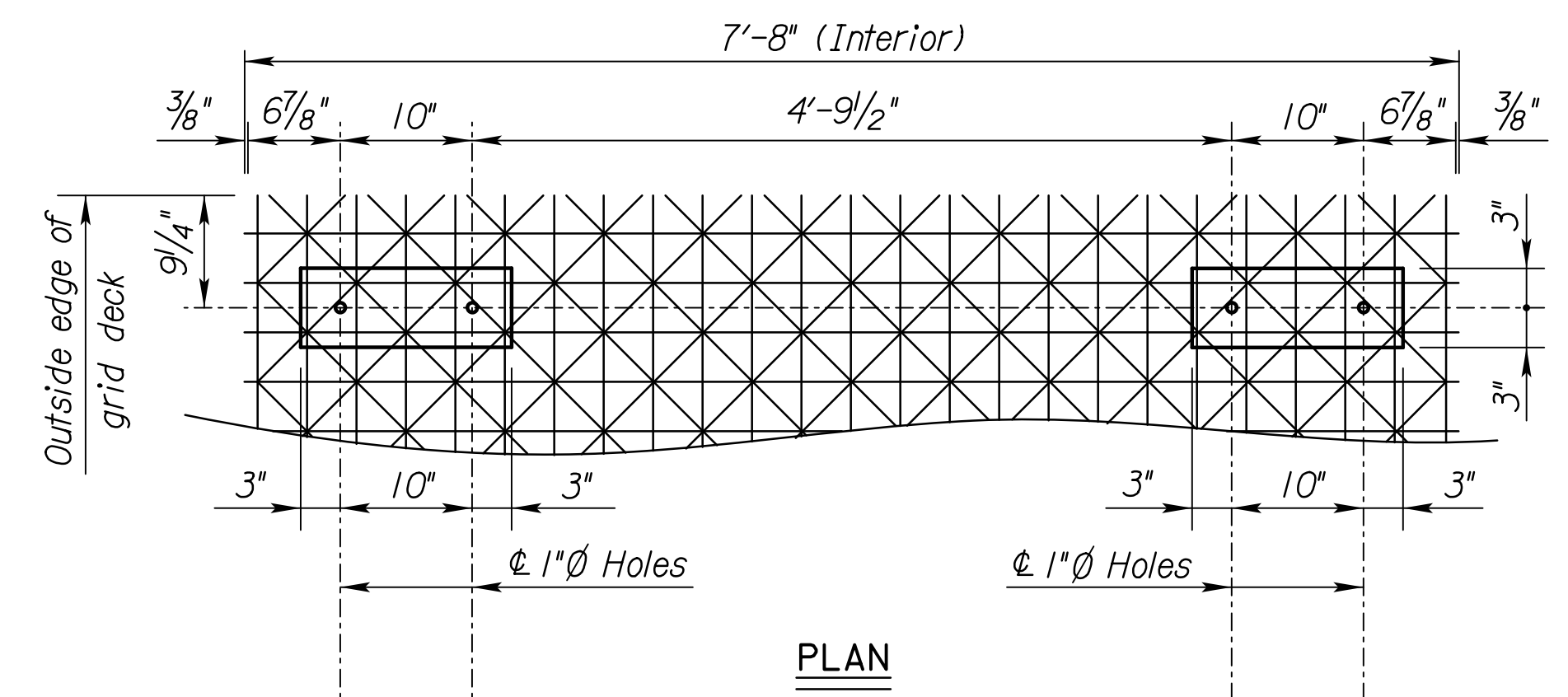
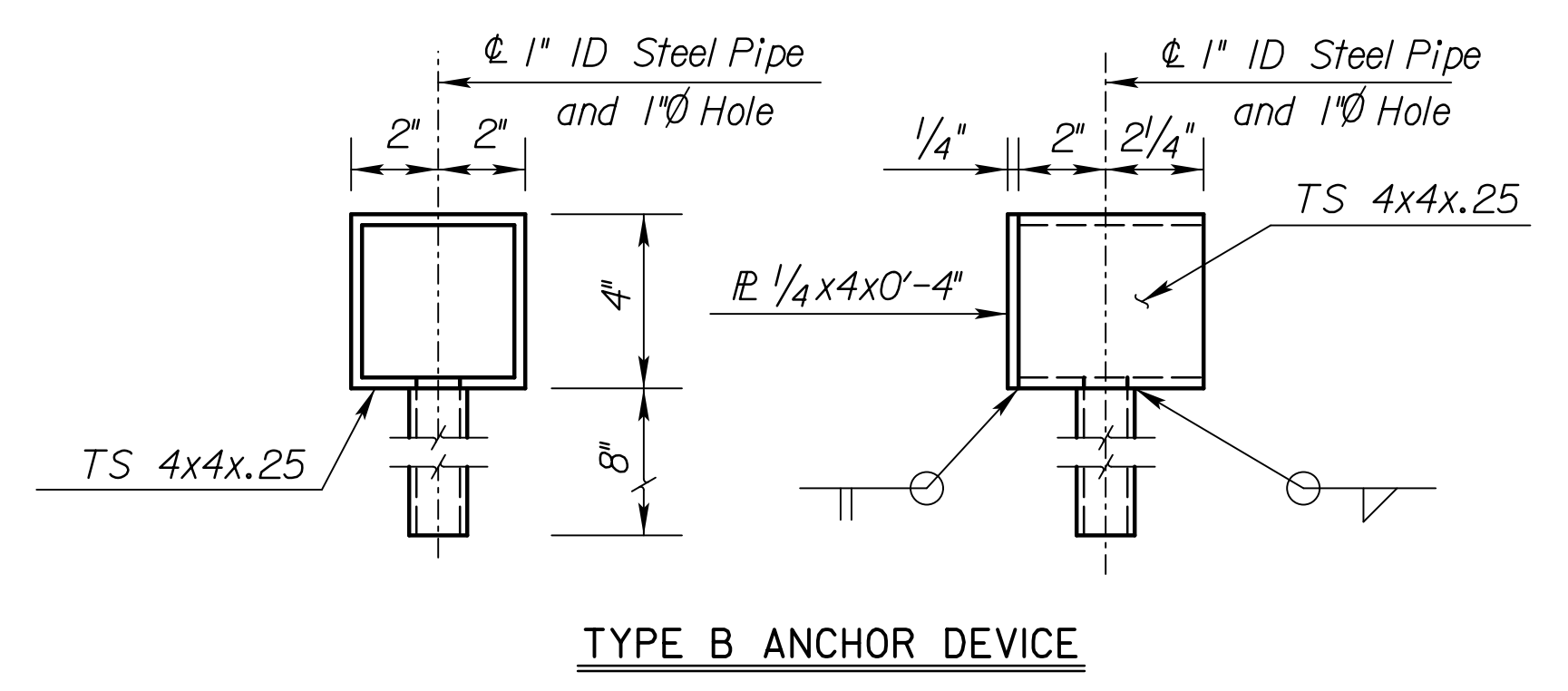
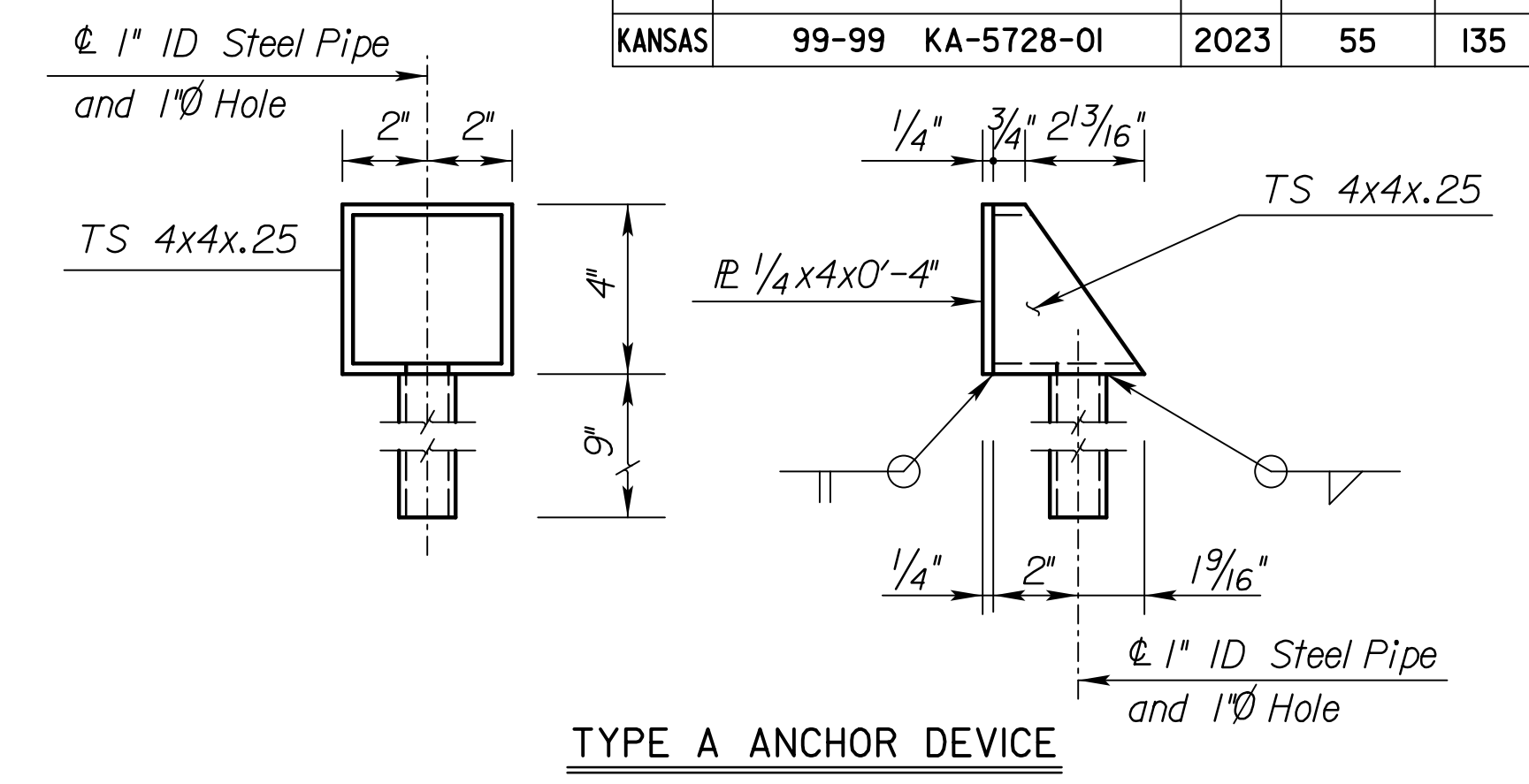
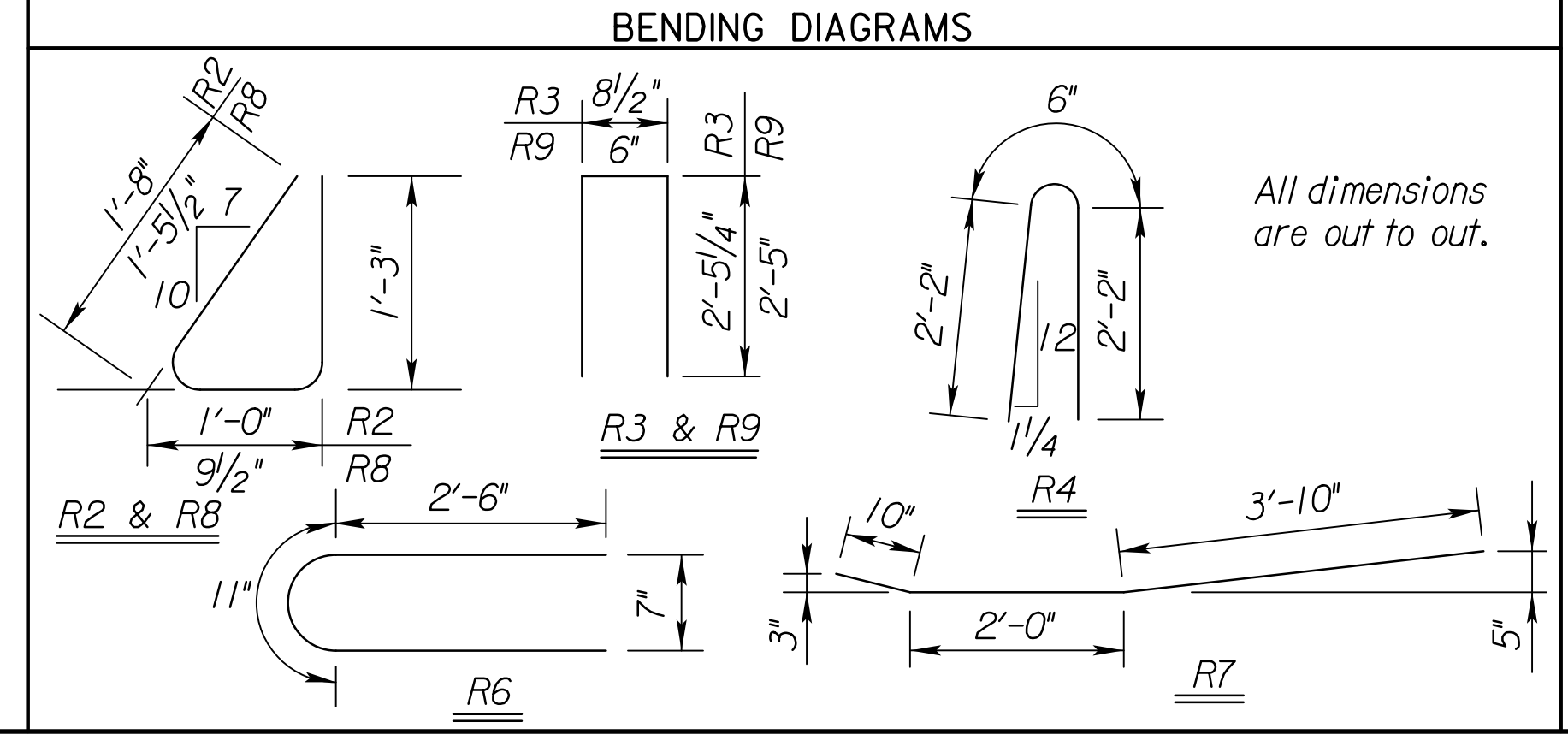
KANSAS DEPARTMENT OF TRANSPORTATION				
Detour Sta.78+16.37				
N-CONNECTION OF DECK PANELS (DETOUR BRIDGE)				
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.				
SHEET NO.	OF	SCALE	APP'D	
DESIGNED		DETAILED	QUANTITIES	CADD
DESIGN CK.		DETAIL CK.	QUAN. CK.	CADD CK.

Std. Base File: k:\bridge\standards\br414c.dgn
 Plotted By: JHOVERSO
 File: c:\pwworking\ventra\01\43371674\kds72801\br113-br414c.dgn
 Plot Date: 01-08-24

KDOT Graphics Certified



BILL OF REINFORCING							
END SECTION				INTERIOR SECTION			
Mark	Number	Size	Length	Mark	Number	Size	Length
R6	2	#7	5'-11"	R1	1	#6	7'-5"
R1	1	#6	7'-5"	R2	11	#5	3'-11"
R2	10	#5	3'-11"	R4	11	#4	4'-10"
R8	1	#5	3'-6"	R5	8	#4	7'-5"
R3	4	#4	5'-7"				
R4	6	#4	4'-10"				
R5	8	#4	7'-5"				
R7	3	#4	6'-8"				
R9	1	#4	5'-4"				



ANCHOR SYSTEM				
NO.	DATE	REVISIONS	QUANTITIES	CADD
3				
2				
1	07-14-04	Numerous Revisions	GFK	GMC
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
Detour Sta. 78+16.37
SAFETY BARRIER CURB
(DETOUR BRIDGE)

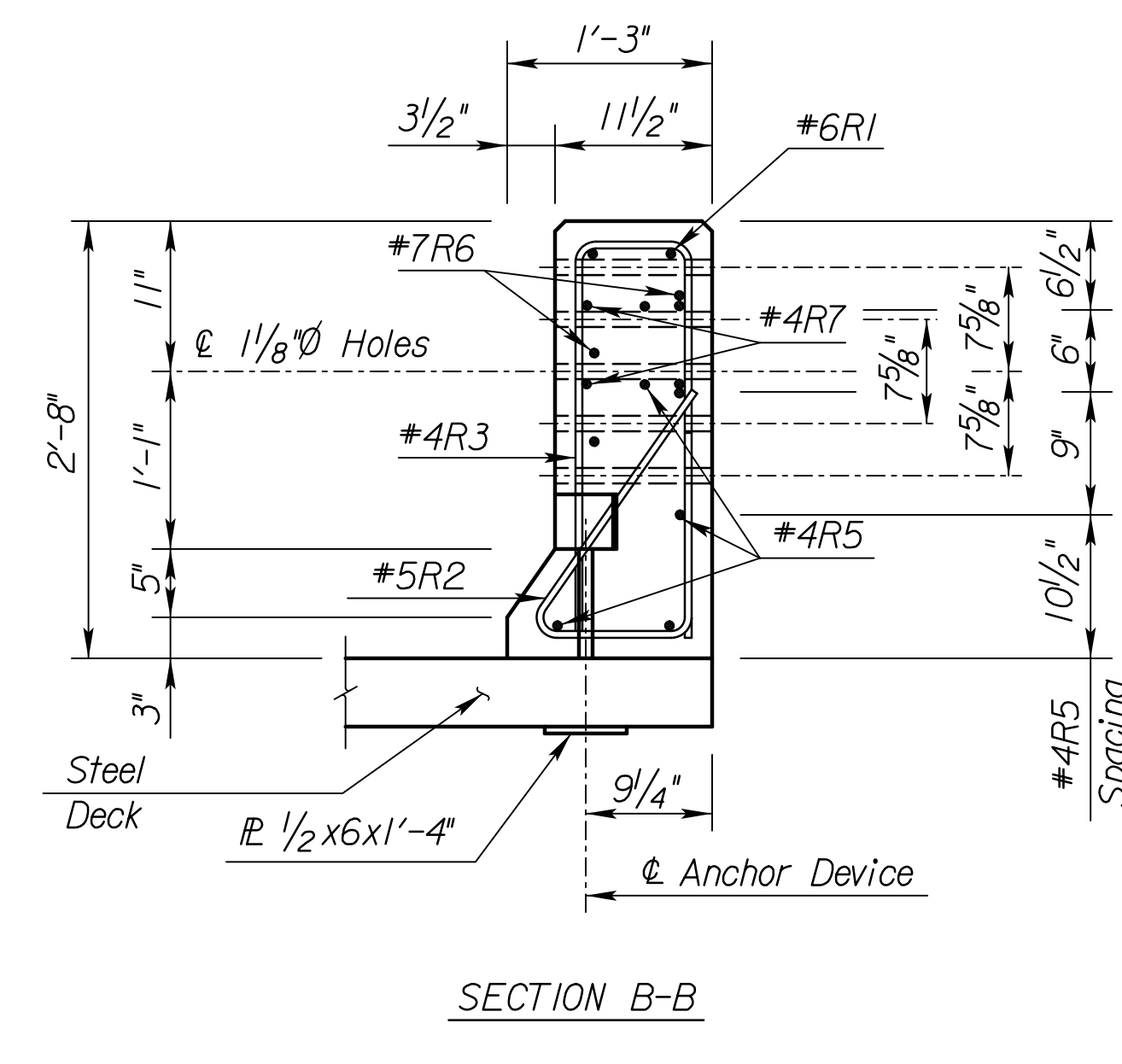
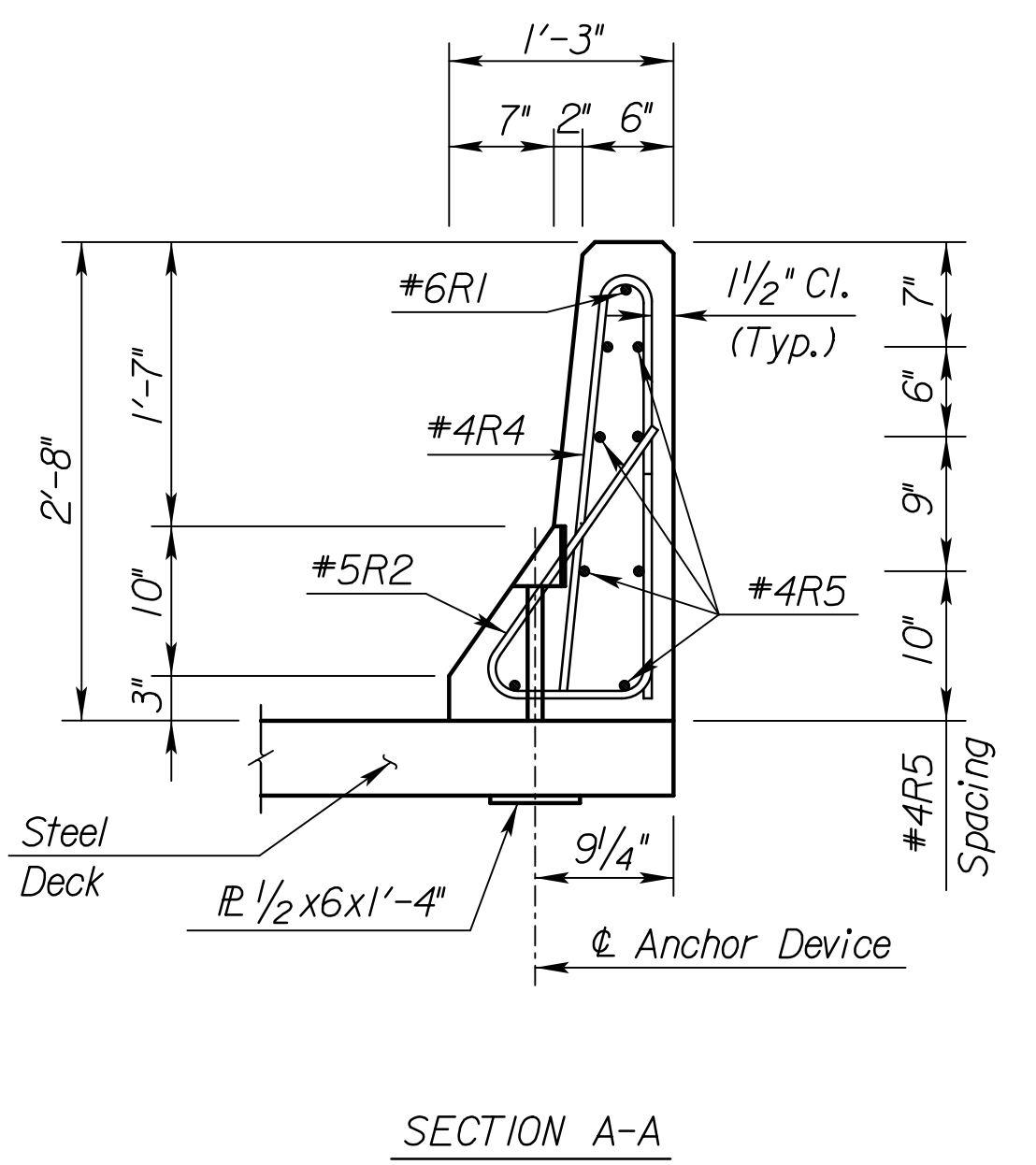
Proj. No. 99-99 KA-5728-01 Wabaunsee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	BY	DATE	QUANTITIES
DESIGN CK.	BY	DATE	CADD
			CADD CK.

KDOT Graphics Certified 01-08-2024 Sheet No. 55

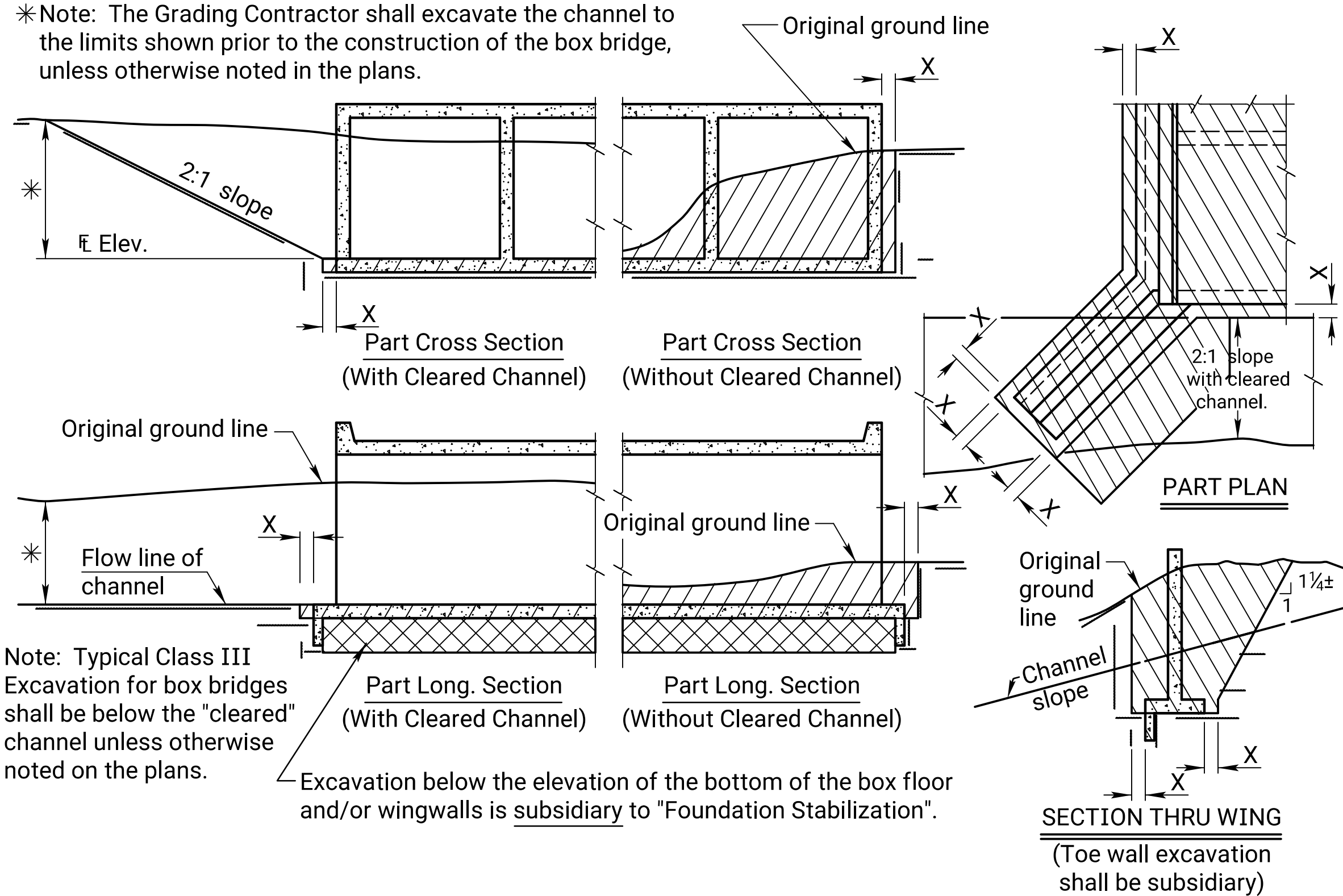
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 Plotted By: JHOVERSO
 File: c:\pwworking\ventra\01\3371674\k6572801\br113-br414d.dgn
 Plot Date: 01-08-24

NOTE: Diagonal bars may be removed from the grid if they interfere with Type A or Type B Anchors during construction of detour bridge.



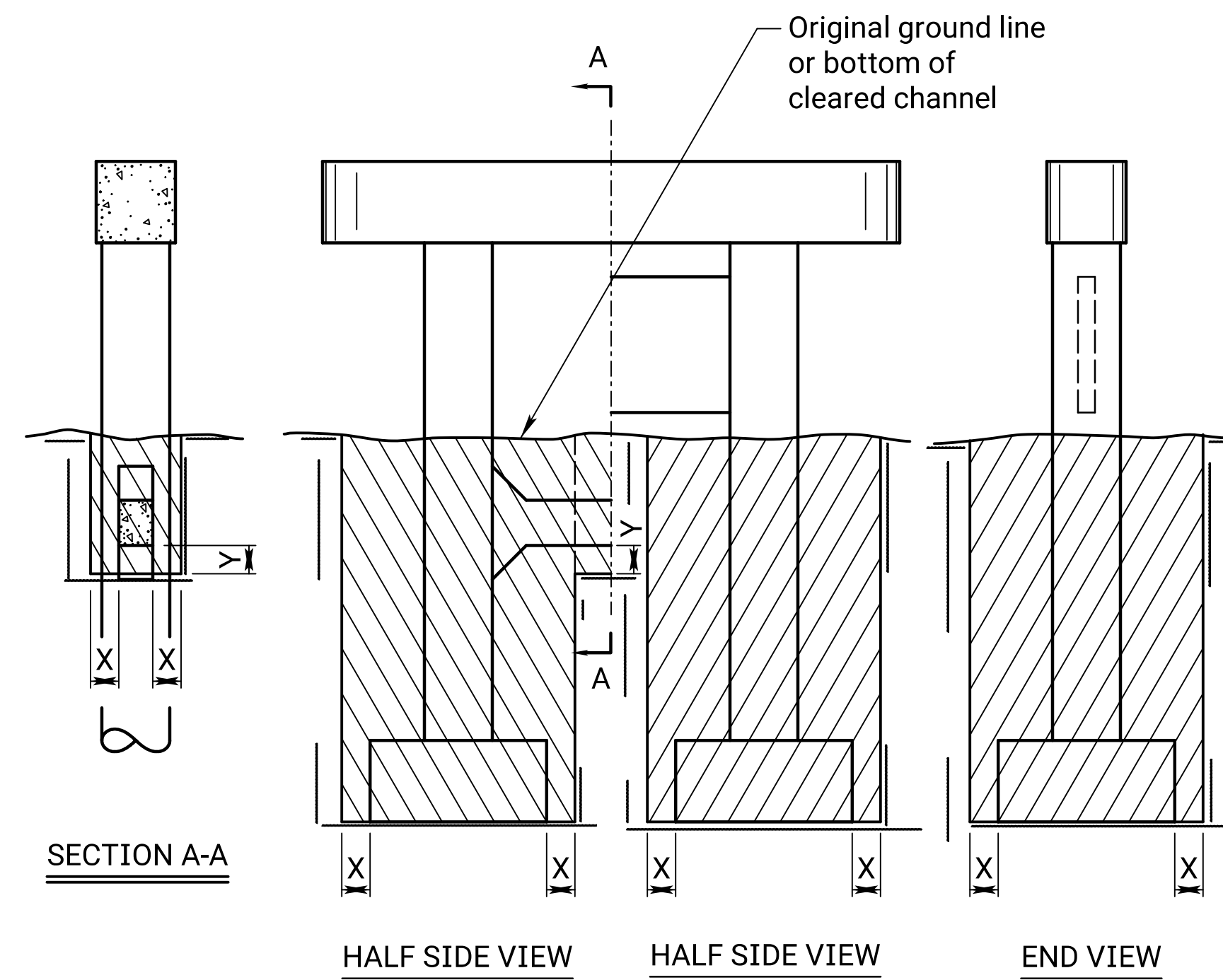
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	56	135

* Note: The Grading Contractor shall excavate the channel to the limits shown prior to the construction of the box bridge, unless otherwise noted in the plans.



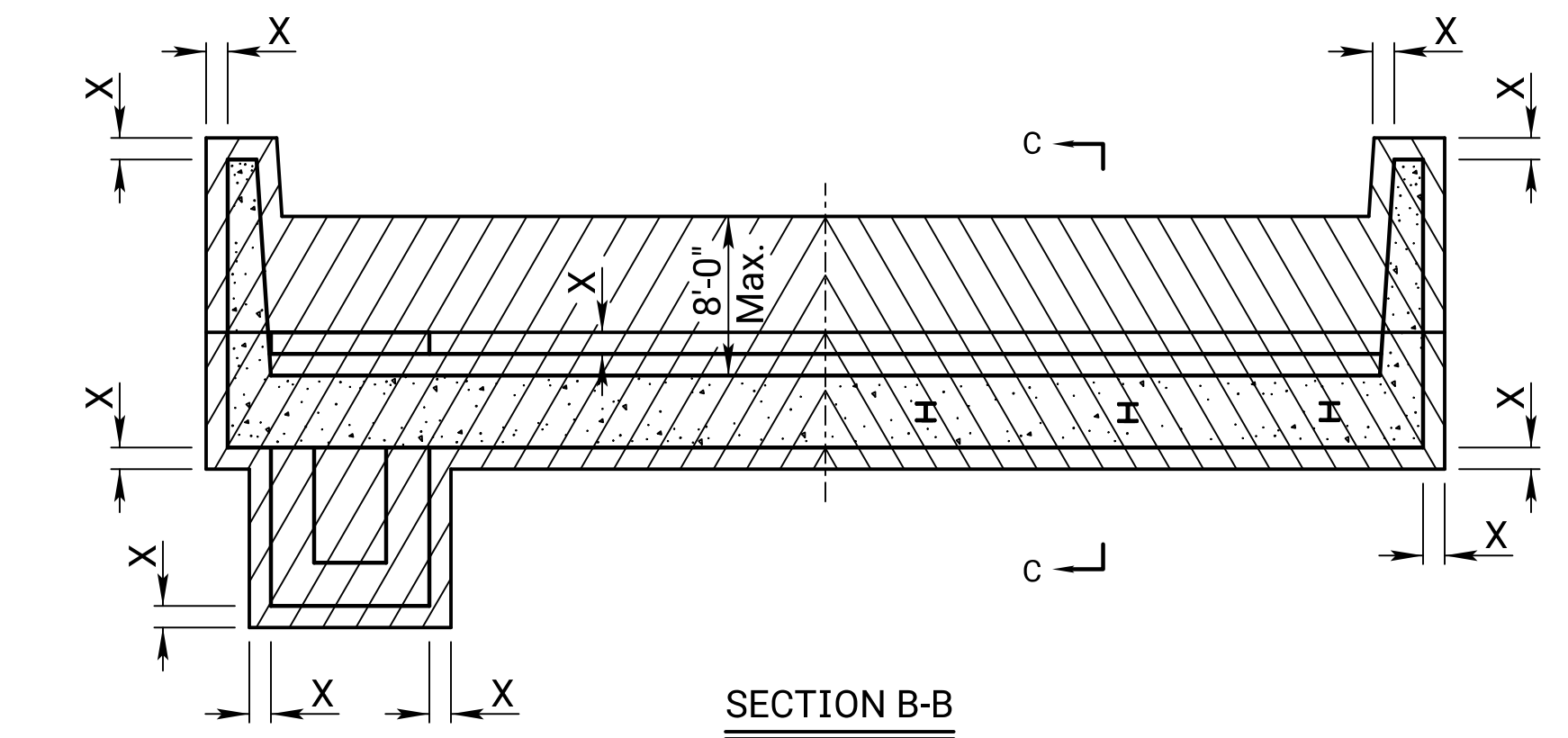
EXCAVATION DETAILS FOR REINFORCED CONCRETE BOX CULVERT

Note: Excavation for culverts less than bridge length and the additional excavation for "Embedded Structures" shall not be paid for as Class III Excavation, but shall be subsidiary to Grade 4.0 Concrete.

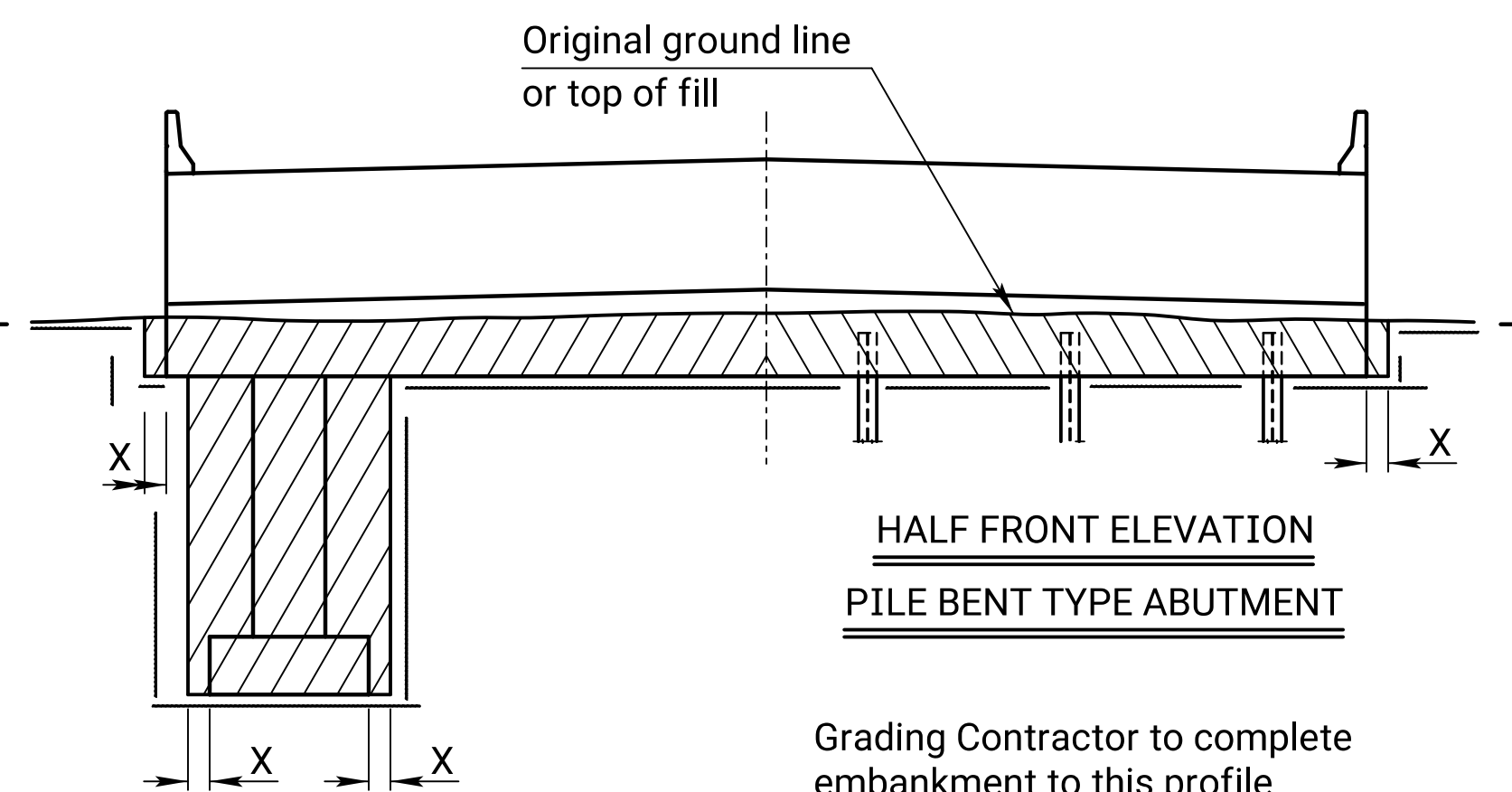


EXCAVATION DETAILS FOR TYPICAL PIERS

See detail when rock or shale (rock) is encountered. ⊗



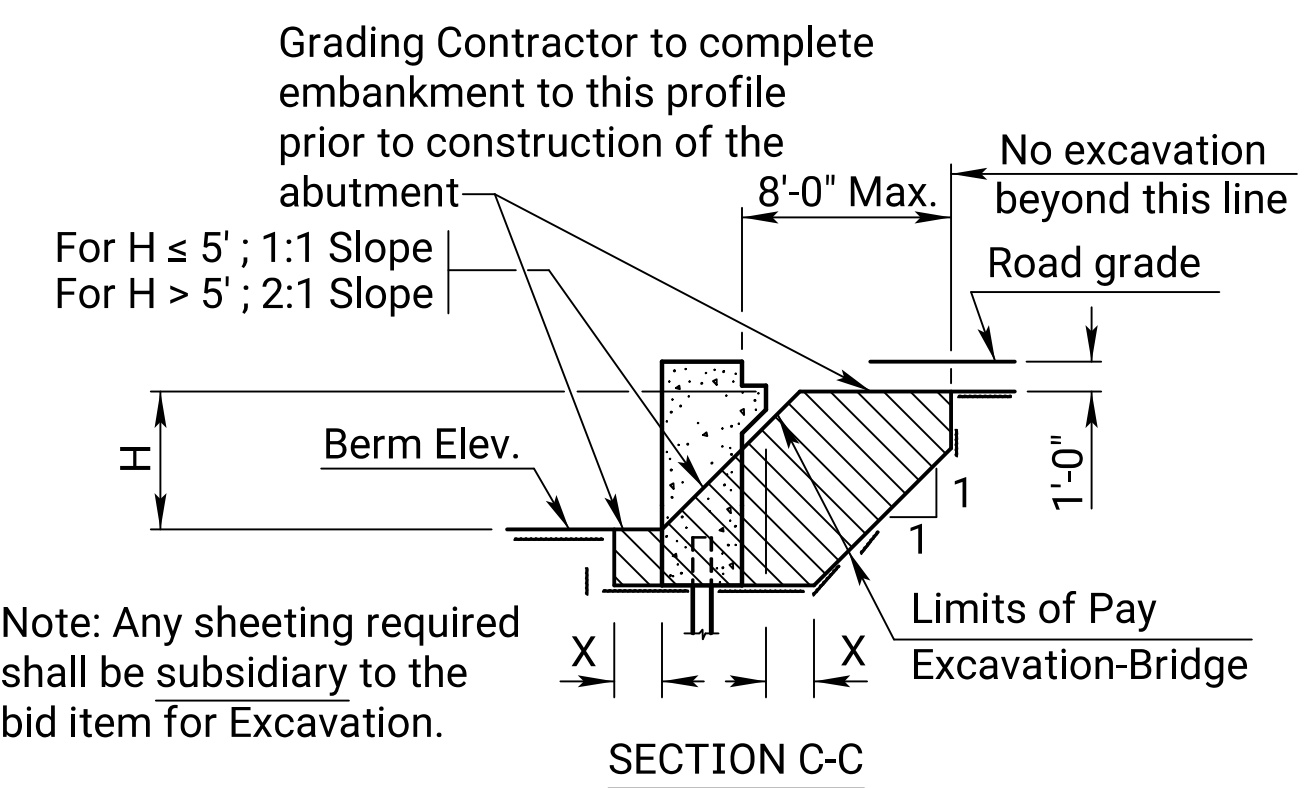
SECTION B-B



**HALF FRONT ELEVATION
PILE BENT TYPE ABUTMENT**

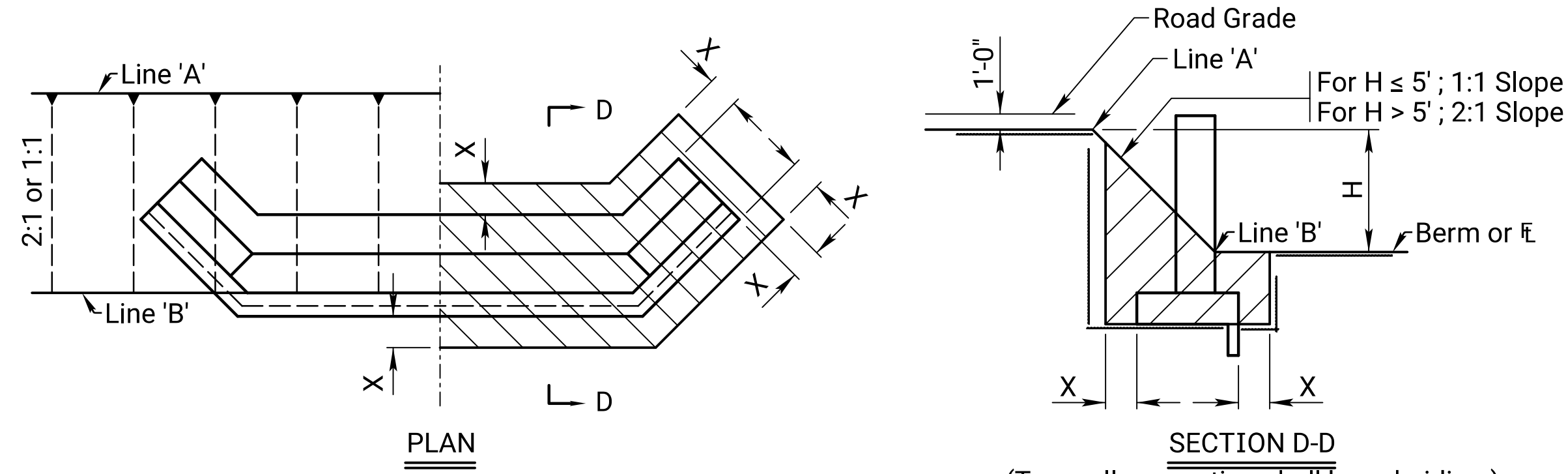
**HALF FRONT ELEVATION
PEDESTAL TYPE ABUTMENT**

Note: Bridge Contractor shall finish the embankment and berms after the construction of the abutment and dispose of any excess material as approved by the Engineer.



EXCAVATION DETAILS FOR TYPICAL ABUTMENTS

See detail when rock or shale (rock) is encountered. ⊗

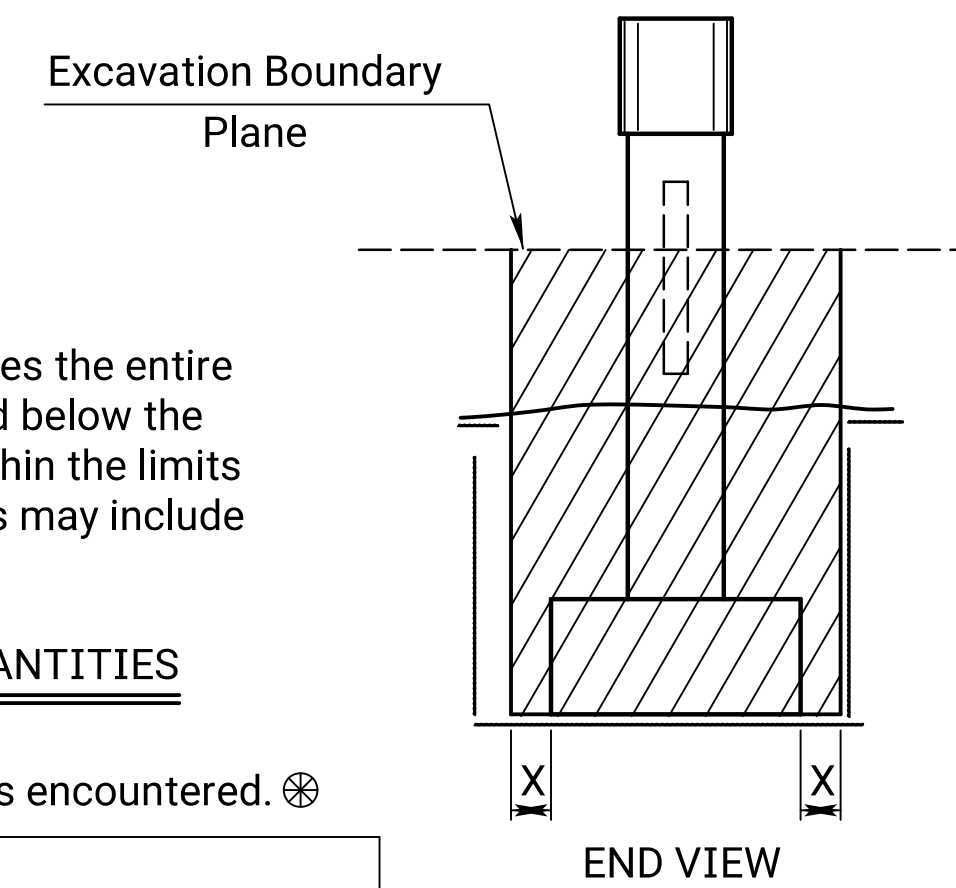


EXCAVATION DETAILS FOR ABUTMENTS WITH FLARED WINGWALLS

Note: Class II Excavation includes the entire volume of whatever nature found below the "Excavation Boundary Plane", within the limits specified for measurement. This may include water or air.

CLASS II EXCAVATION QUANTITIES

See detail when rock or shale (rock) is encountered. ⊗

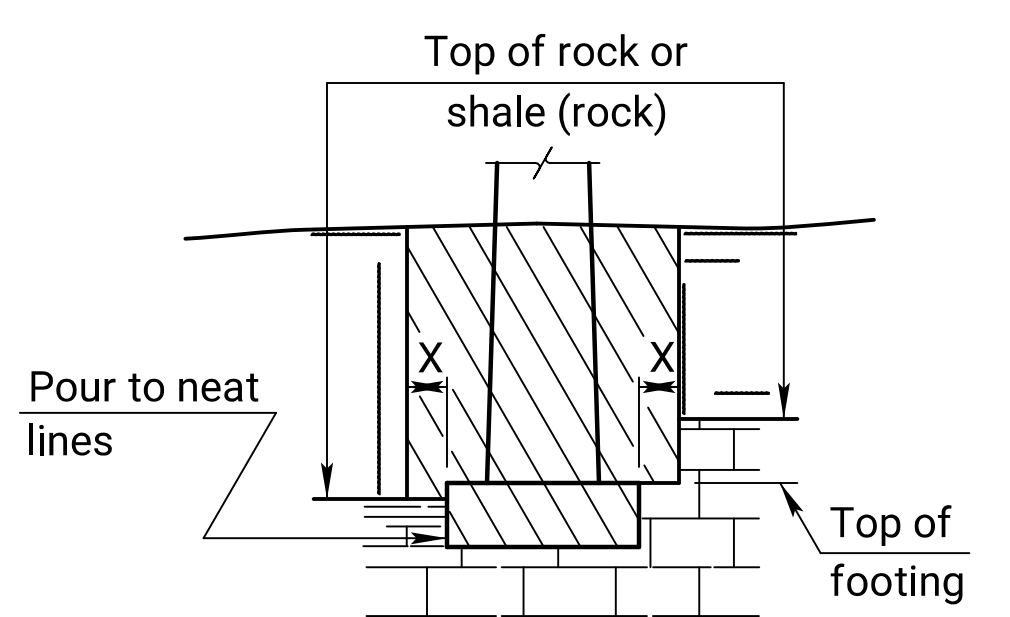


END VIEW

Note: All bridge excavation shall be computed on the basis of the cross-hatch areas and boundary lines indicated on this sheet and the Excavation Boundary Plane on the Construction Layout.

Sides of trenches in hard or compacted soil including embankments shall be shored, sheeted, braced or otherwise supported when the trench is more than 5 feet in depth and 8 feet or more in length. In lieu of the shoring, the sides of the trench above the 5 foot level may be sloped to preclude collapse. The slope for average soils shall be 1:1. If the angle of repose of the soil is less, flatter slopes shall be required.

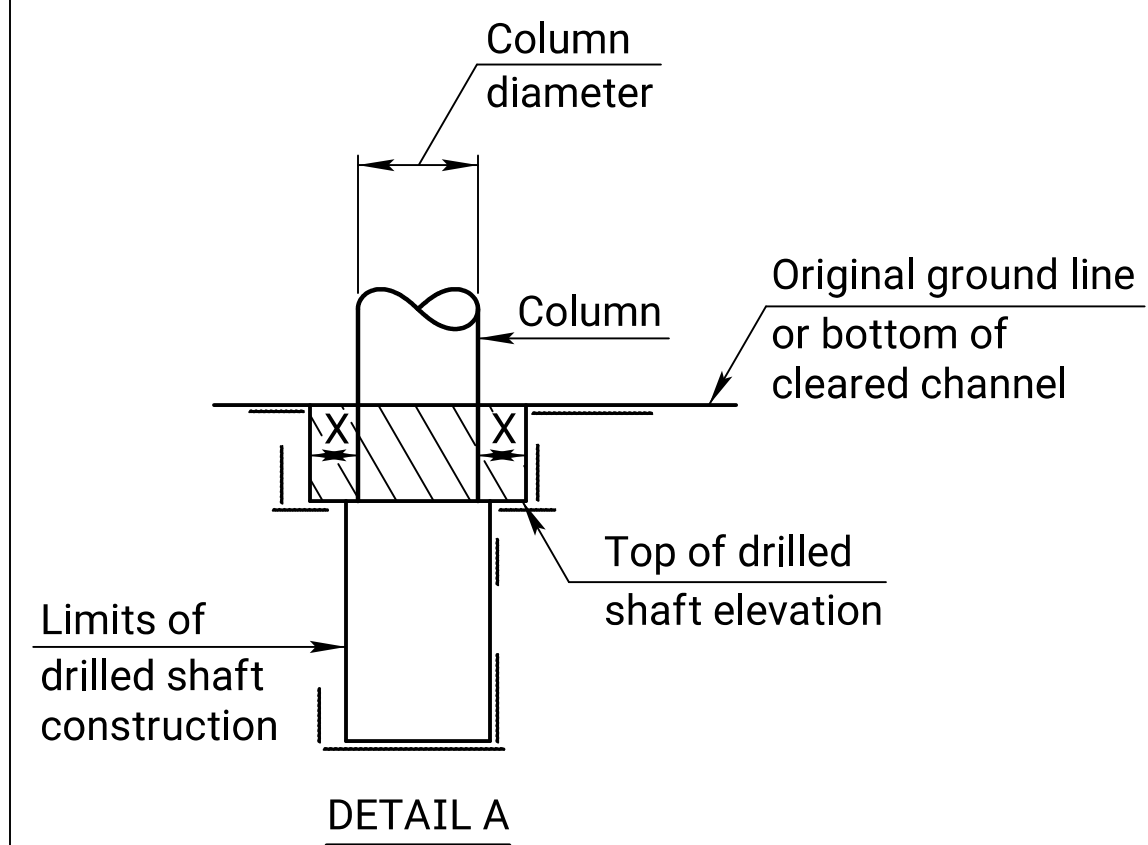
Dimension "X" shall be 2'-0" unless indicated otherwise on the general plans.
Dimension "Y" shall be 1'-6" unless indicated otherwise on the general plans.



EXCAVATION DETAIL FOR FOOTINGS IN ROCK OR SHALE (ROCK)

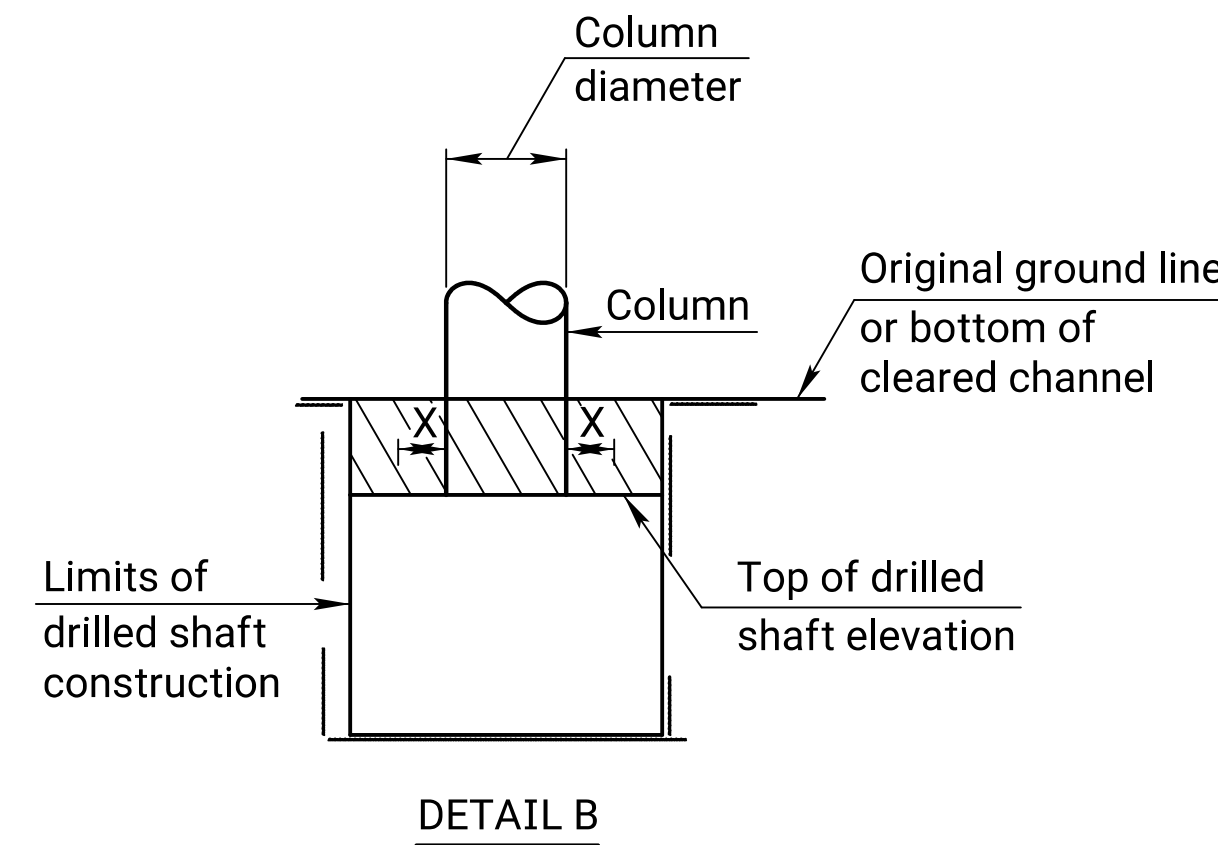
(Piers and Abutments)

Note: Excavation below top of rock, hard shale or below top of footing, whichever is lower, shall be to neat lines of the concrete construction.



DETAIL A

Note: Whenever the limits of the drilled shaft construction are greater than the Column Diameter + 2X, the limits of Class I, II or III Excavation shall be the limits of the drilled shaft construction. (See Detail B)



DETAIL B

DRILLED SHAFT DETAILS

Drawn By: JHOVERSO Plotted: 01-08-24
File: c:\pwworking\central01\vd3377674\ks572801\bs113-24.dgn

NO.	DATE	REVISIONS	BY	APPD
06	08-15-12	Embedment Excavation Subsidiary	J.P.J.	T.L.F.
05	05-15-12	Revised Wing Excavation	J.P.J.	T.L.F.
04	03-03-10	Revised Wing Excavation	J.P.J.	T.L.F.

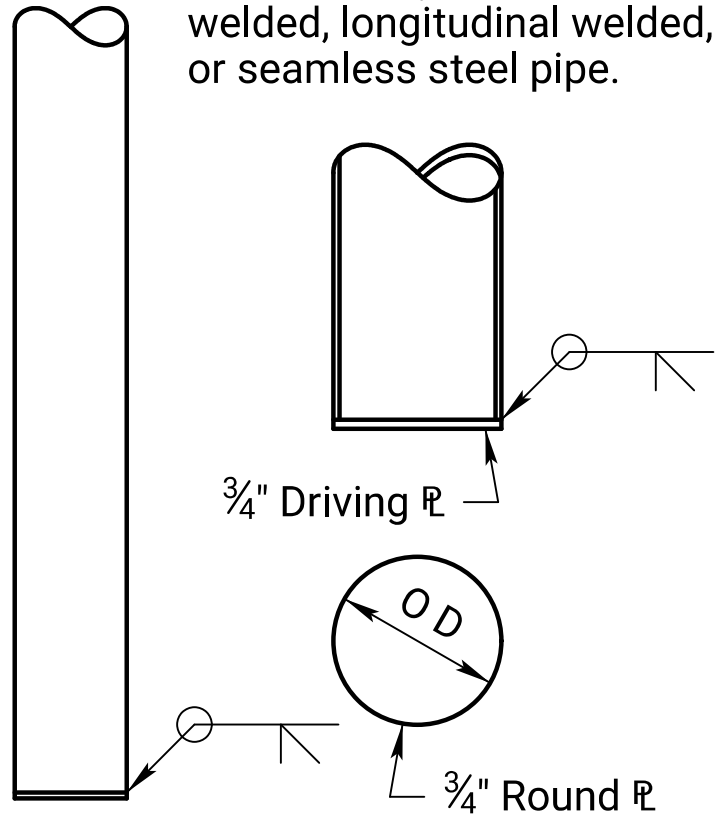
KANSAS DEPARTMENT OF TRANSPORTATION			
BRIDGE EXCAVATION (LRFD)			
BR100B			
FHWA APPROVAL	04-17-10	APPD.	Terry L. Fleck
DESIGNED	DETAILD	R.D.R. QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	L.R.R. QUAN. CK.	TRACE CK.

OD	10 3/4"	T. = ¶¶
OD	12 3/4"	T. = ¶¶
OD	14"	T. = ¶¶

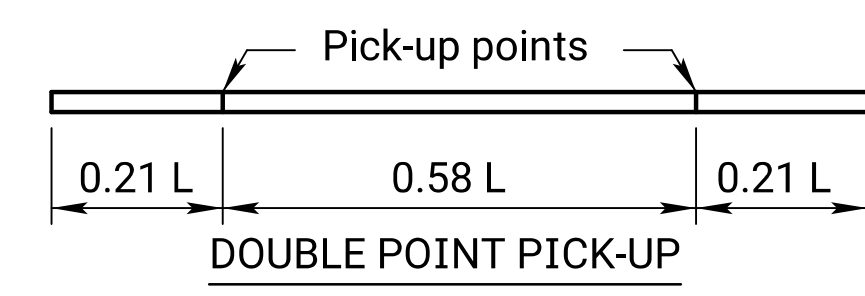
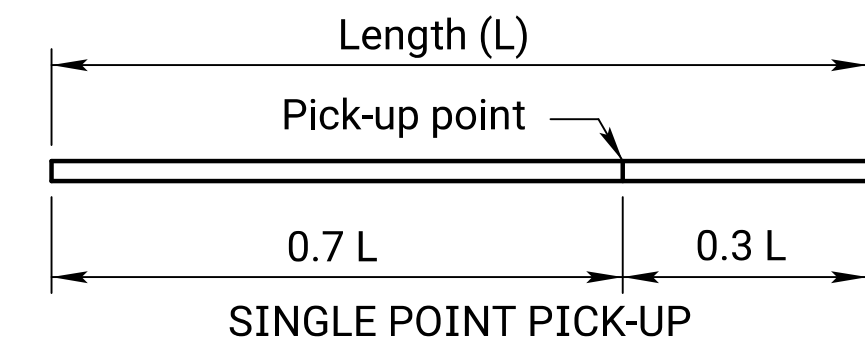
¶¶ See the Geology Report or "Summary of Quantities" for Pipe Pile wall thickness

Note: Pile shall be driven with a steel head having a projecting ring fitting inside the pipe. Clearance between ring and pipe should be 1/4".

Note: Pile pipe may be spiral welded, longitudinal welded, or seamless steel pipe.



PLAIN ROUND CAST-IN-PLACE CONCRETE PILES

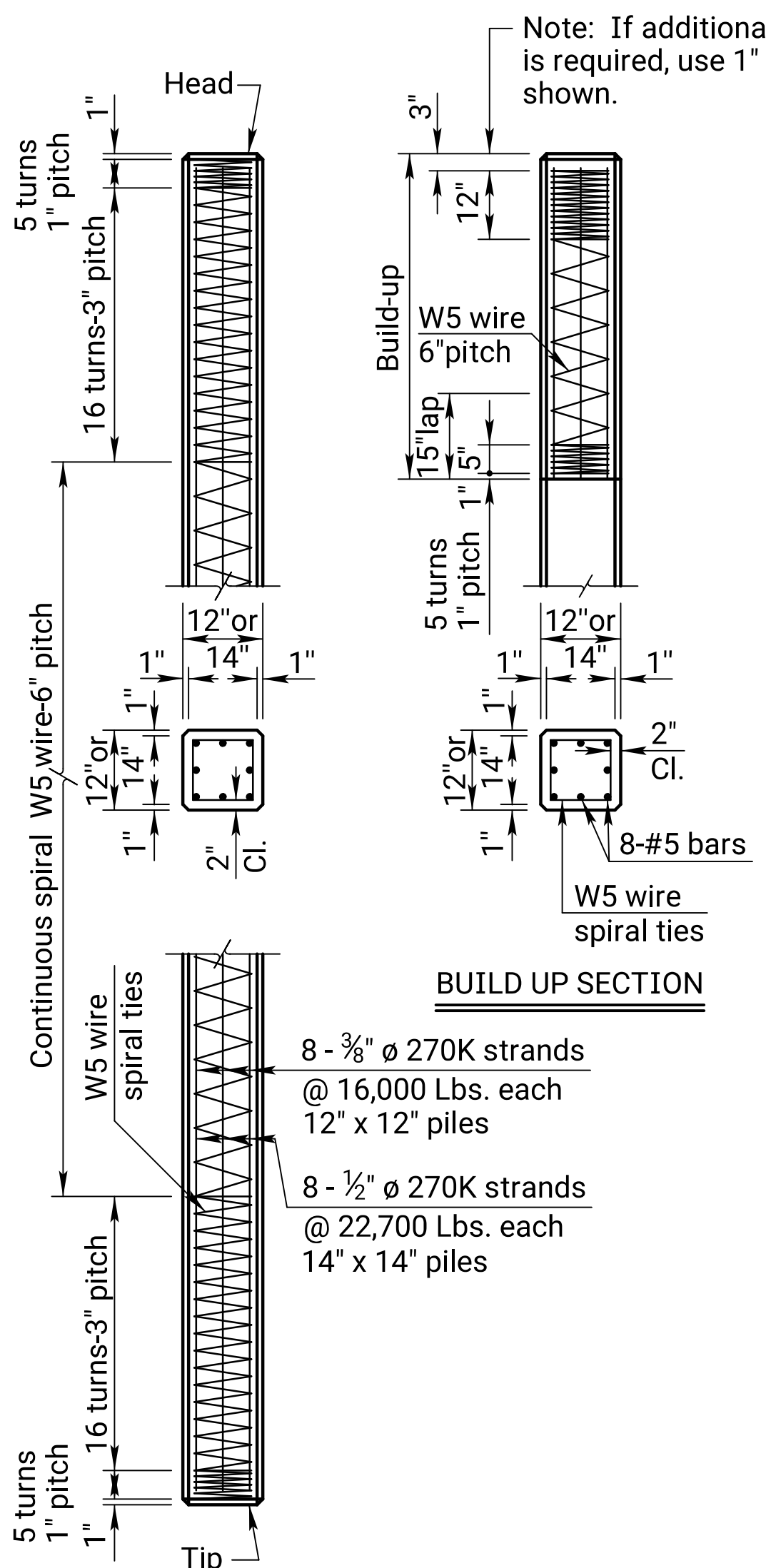


PICK-UP POINTS FOR PRESTRESSED PILING

Max. length - 55' single point pick-up
Max. length - 80' double point pick-up

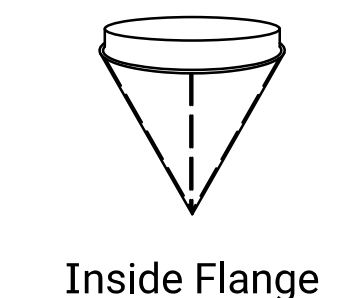
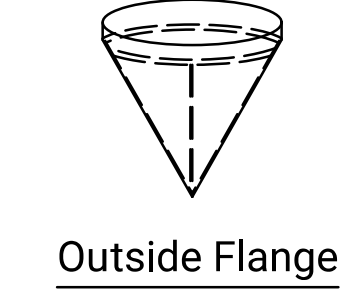
Note: Piles shall be marked at Pick-up points to indicate proper points for attaching handling lines.

12" OR 14" PRESTRESSED CONCRETE PILES

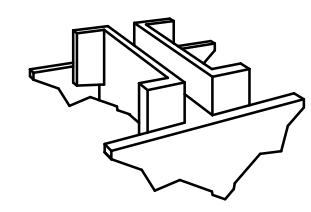


BUILD UP SECTION

8 - 3/8" ø 270K strands @ 16,000 Lbs. each
12" x 12" piles
8 - 1/2" ø 270K strands @ 22,700 Lbs. each
14" x 14" piles



SHELL PILE POINT



CAST STEEL PILE POINT

The pile point shall be a one-piece unit of cast steel. Weld pile points in accordance with manufacturer's recommendations to each steel pile before driving.

Weld Symbology Definition

Use grinder to bevel edges of splice as shown in weld symbology and drawing. In addition to bevels, produce clean, bare, and shiny surfaces at and around the splice welding location.

Lay full penetration root weld from beveled side of splice.

Back gouge root weld from side opposite of root welding application making sure to remove all foreign materials, porous steel, and inclusions from root weld. Finish welding the non beveled side of the splice.

Finish welding beveled side of the splice while removing slag, foreign materials, porous steel, and inclusions in between welding passes, use of a grinder may be needed.

Verify that enough filler metal has been correctly placed in all weld locations to obtain a flush or convex surface with no concavity produced upon completion of the final welds.

FOR INFORMATION ONLY EQUIVALENT POINT BEARING PILES

STEEL PILES	CONCRETE PILES	
	Pipe	Pre-stress
HP10x42	10 3/4"	
HP12x53	12 3/4"	
HP14x73	14	12
HP14x102		14
HP14x117		16

SPICES: Splices for steel piles and shell piling shall be in accordance with details shown on this sheet and the Standard Specifications.

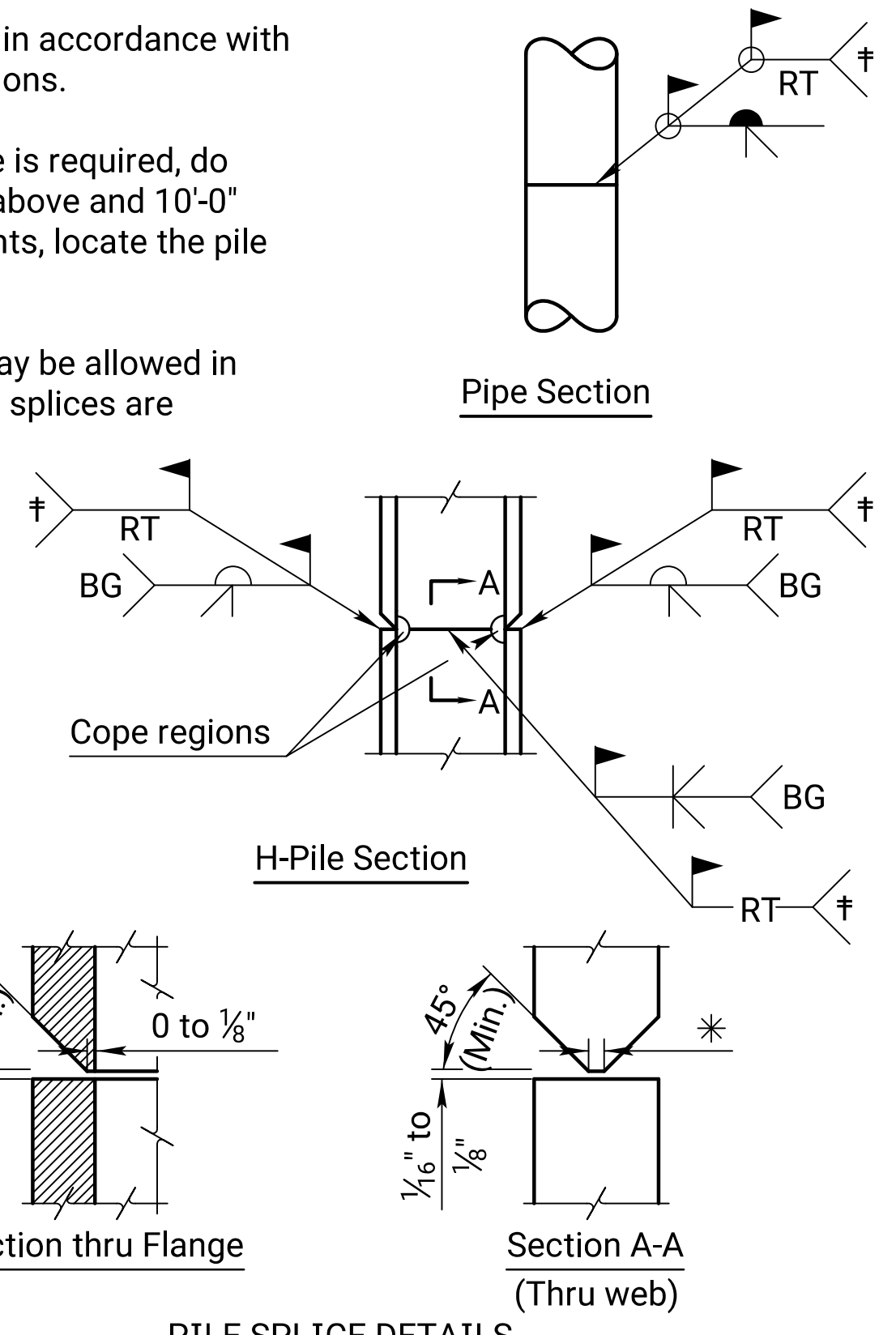
For integral pile bent abutments and piers, if a pile splice is required, do not locate the pile splice within a region extending 2'-0" above and 10'-0" below the bottom of the concrete web wall. For abutments, locate the pile splice at least 10'-0" below top of fill.

With the approval of the Engineer, one splice per bent may be allowed in the region described above without testing. If additional splices are anticipated, based on the geology, the Contractor prior to driving, will locate the splice so that the splice will not fall within the regions described above.

¶ For integral pile bent abutments and piers, if a splice is located within the regions described above, then the Contractor will test the welds by Radiograph (RT) test methods. Repair and retest any welds not passing the test(s). Each weld tested will have written confirmation of results. Report these results to the Engineer. This work is not paid for directly, but is subsidiary to "Piles".

* Minimum as required by welding process.

BG = Backgouge



PILE SPLICE DETAILS

GENERAL NOTES

PRESTRESSED PILES: Fabricate prestressed concrete pile splices in accordance with the Manufacturer's recommendations subject to the approval of the Engineer.

Method of attachment of pile to build-up may be by any of the methods given in the notes on "Alternate Methods." If mild reinforcing steel is used for attachment, the area shall be no less than that used in the build-up.

ALTERNATE METHODS: Method of attachment of a pile to build-up may be by any of the following methods:

1. Cut off at least 2'-0" of pile and expose a minimum of 2'-0" of strands.
2. Cast 8-#6, or 8-#5 bars (equally spaced) into pile head. All bars shall extend into pile head and project from pile head a minimum of 2'-0".
3. Drill 8 holes in pile head (equally spaced) for installation of 8 grouted dowel bars of same size and length as in 2.
4. Provide cored holes for bars as in 3.

No bars or strands are to extend from head of pile or build-up into footing or pile cap unless approved by the Engineer.

TEST PILES: Drive test piles where called for on the bridge plans. The test piles located within the limits of the substructure will become a part of the bridge pile system.

DRIVING FORMULA: Driving formula shall conform to the Standard Specifications.

MEASUREMENT AND PAYMENT: Measurement and payment for all piles shall comply with the Standard Specifications.

REINFORCEMENT: Use reinforcing steel conforming to ASTM A615, Grade 60. Hoops and spirals may be either plain or deformed bars.

PRESTRESSING STEEL: Use uncoated seven-wire low relaxation prestressing strand conforming to ASTM A416, Gr. 270.

STEEL PILE: Steel pile shall conform to the requirements of the Standard Specifications.

PILE POINTS: Pile points shall conform to the dimensions shown and to requirements of the Standard Specifications.

SPECIFICATIONS: Standard Specifications for State Road and Bridge Construction as currently used by the Kansas Department of Transportation.

CONCRETE: Concrete for cast-in-place shall be f'c = 3,500 PSI. Concrete for prestressed shall be f'c = 5,000 PSI.

WELDING: All field welding shall meet the requirements of the Standard Specifications.

Use only Shielded Metal Arch Welding SMAW (stick welding) for pile splices.

Use only low hydrogen E7018, 7016, or 7015 series welding rod (electrode) for all welding applications during pile splicing.

New electrodes are to be purchased for each KDOT project. The electrodes shall arrive on the project in factory hermetically sealed containers, opened and labeled with indelible ink in front of the engineer. The label shall include the current date and the project number. If the container seal is questionable or shows signs of damage the electrode is to be dried in an oven at least one hour at a temperature of 700°F to 800°F.

Upon removal from intact hermetically sealed factory packaging or the drying oven the electrode is to be placed in a storage oven with a minimum temperature of 250°F.

When electrodes are removed from the hermetically sealed container or storage oven and exposed to the atmosphere for less than 4 hours place into the storage oven for at least 4 hours before removing for use.

If electrode is exposed to the atmosphere for 4 hours or more (or 9 hours for moisture resistant electrodes designated with an R in their labeling) then electrode can be dried in a drying oven at a temperature of 450°F to 550°F.

If the electrode is exposed to the atmosphere for 4 hours or more a second time or the rod becomes wet discard rod.

CAST-IN-PLACE SHELLS: Steel shells for cast-in-place concrete piles shall conform to the requirements of the Standard Specifications.

All piles driven without a mandrel shall be of the minimum thicknesses shown. Piles driven with a mandrel shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion and/or buckling due to soil pressure after the mandrel is removed.

Remove, replace or correct to the satisfaction of the Engineer improperly driven, broken or otherwise defective pipe piles. Otherwise drive an additional pile at no extra cost.

The Contractor shall maintain a light suitable for visual inspection of the pile on the job at all times prior to and during the filling of the pipe.

PAIN: All paint shall comply with the Standard Specifications, or as specified on the plans.

MILL TEST REPORTS: Steel piles test reports and steel shell test reports shall comply with the Standard Specifications.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	57	135

NO.	DATE	REVISIONS	BY	APPD
04	08-16-18	Add splice web section, clarify note	M.L.L.	J.P.J.
03	09-15-15	Clarify Notes	J.P.J.	C.E.R.
02	06-18-12	Clarify f'c, rod type, use and weld	J.P.J.	T.L.F.

KANSAS DEPARTMENT OF TRANSPORTATION

STANDARD PILE DETAILS

BR110		10-04-12		APPD.	Terry L. Fleck
DESIGNED	J.P.J.	DETAILED	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		

GENERAL NOTES

Reference is made to the latest edition of the CRSI "Manual of Standard Practice" for recommended industry practices concerning reinforcing steel.

Use only the following types of bar supports:

- 1) Wire Bar Supports:
 - a) Epoxy coated reinforcing: Class 1 Protection
 - b) Non-epoxy coated reinforcing: Class 1, 2, or 3 Protection
- 2) Plastic Bar Supports
- 3) Supplementary bars

When securing epoxy coated reinforcement, use tie wires or metal clips that are epoxy or plastic coated.

Do not weld reinforcing steel to bar supports or to other reinforcing steel. Shop weld spacer frames for haunched slabs.

Tie bars at all intersections around the perimeter of each mat and at not less than 2'-0" centers or at every intersection, whichever is greater.

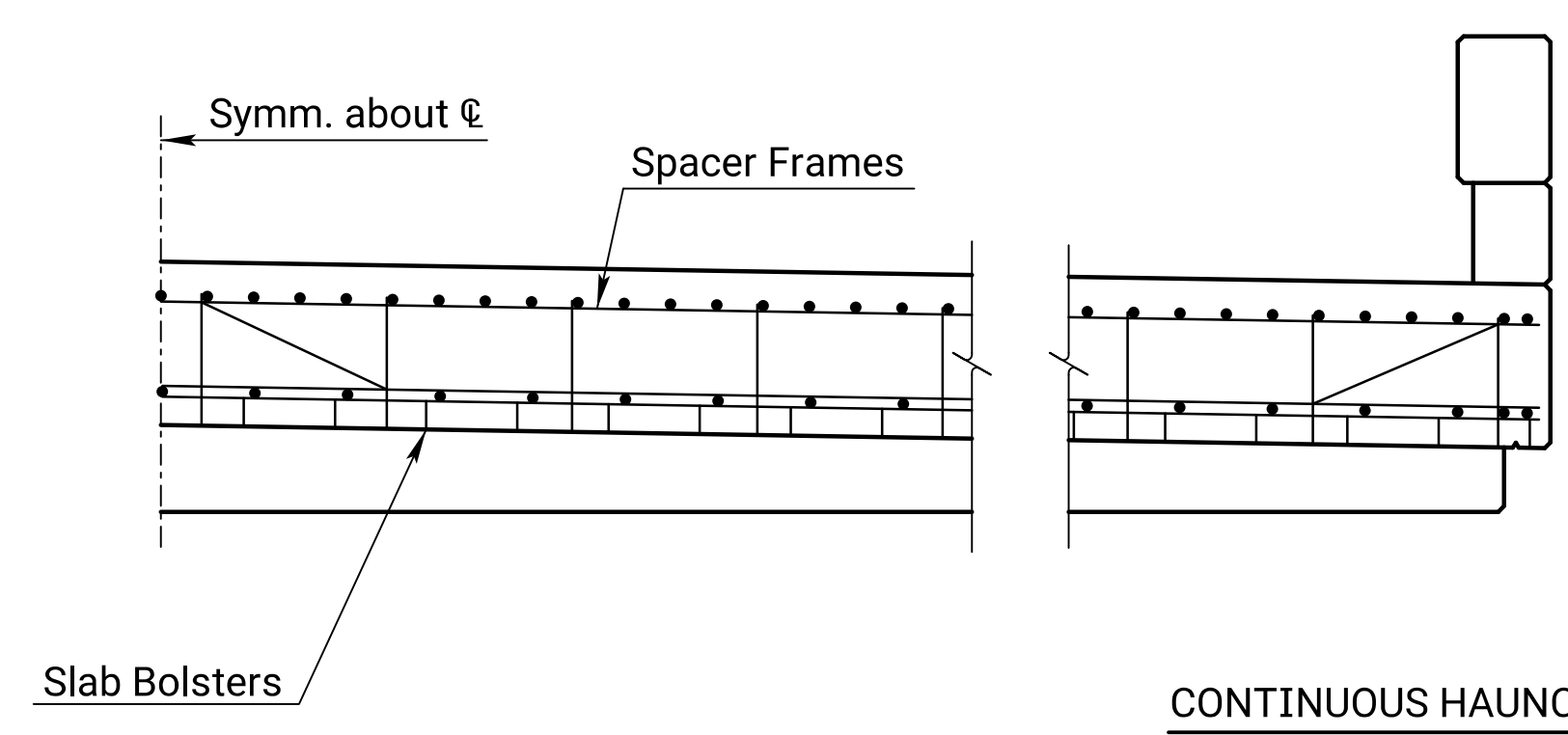
Where more than one length of bar support is required, lap the end legs so they are locked or tied together.

Use proper height supports to maintain the distance between the reinforcing and the formed surface or the top surface of deck slabs within 1/4" of that indicated on the plans.

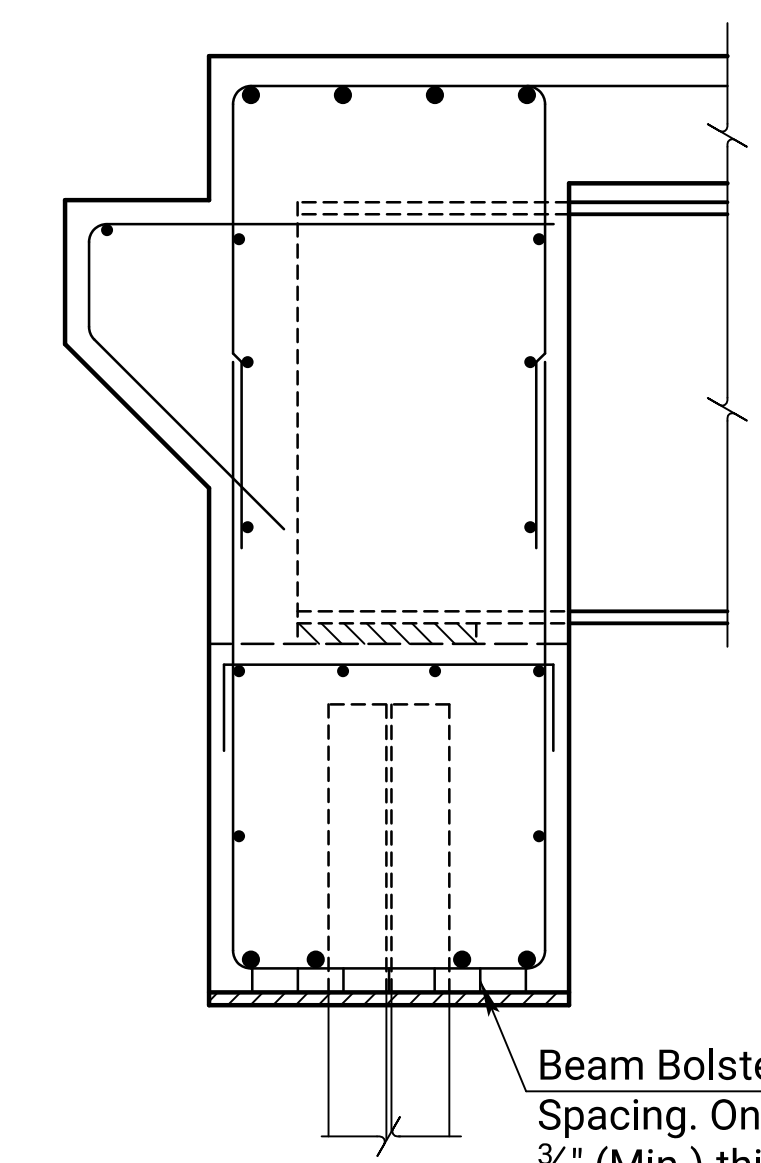
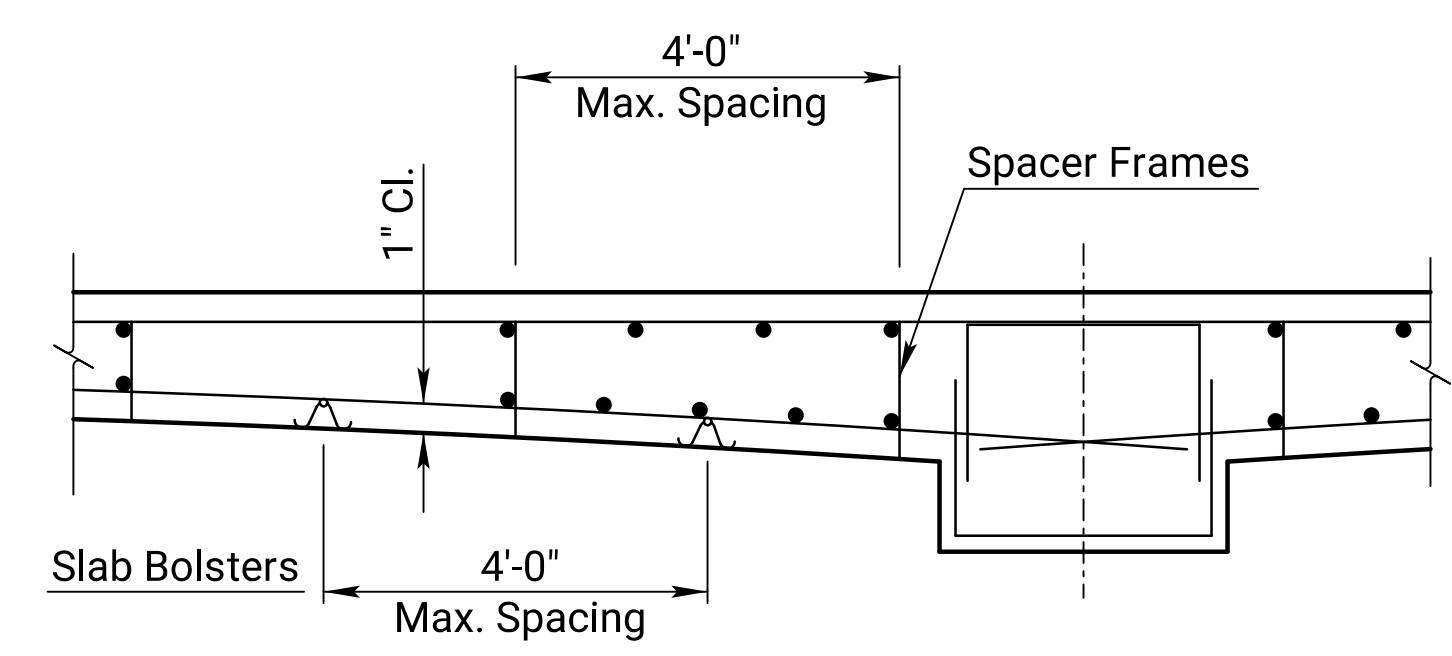
Spacings shown are maximums. Use sufficient supports, as determined by the Engineer, to retain the reinforcing steel in position.

Construct any platforms, required for the support of workers and/or equipment during concrete placement, directly on the forms and not on the reinforcing steel.

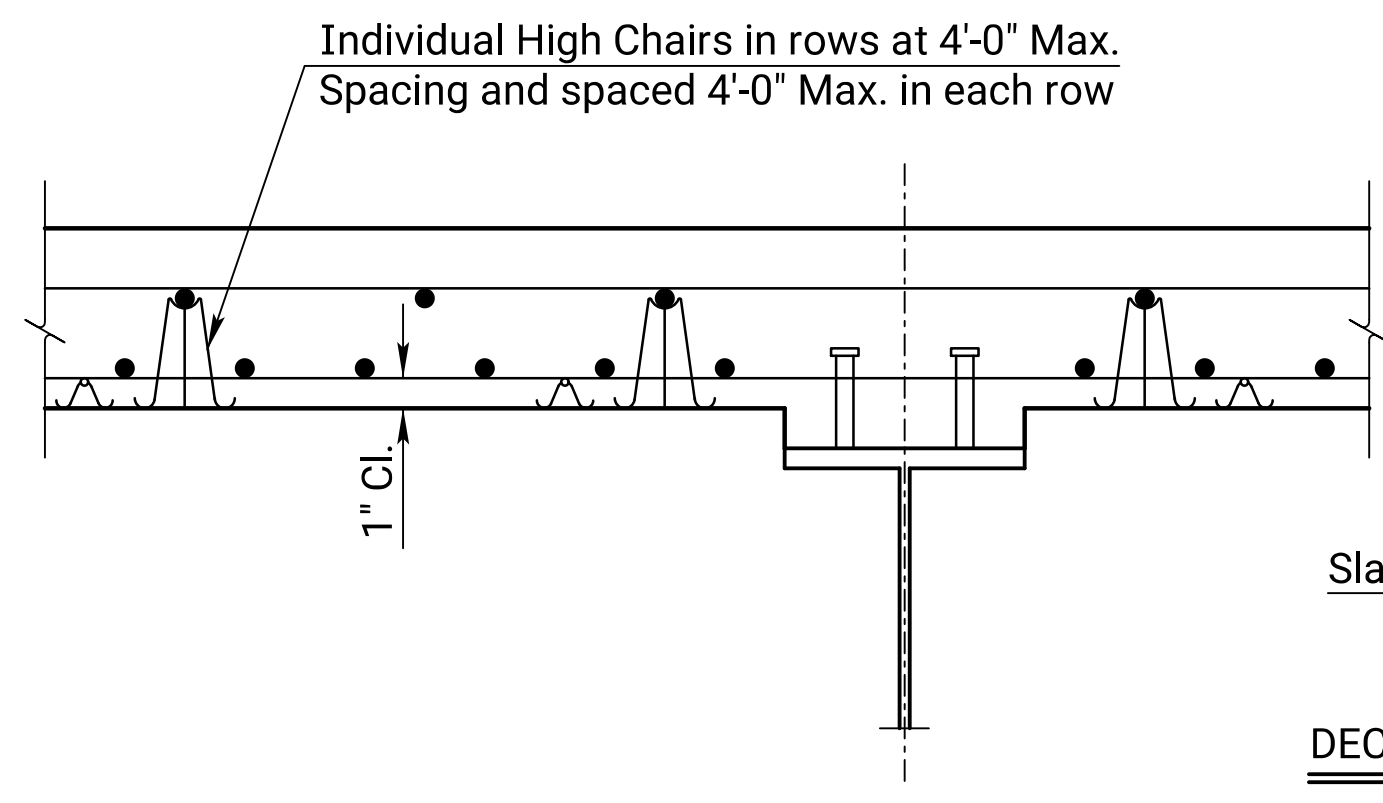
Designs and arrangements of Supports or Spacers other than as shown on this sheet, may be used with the permission of the Engineer.



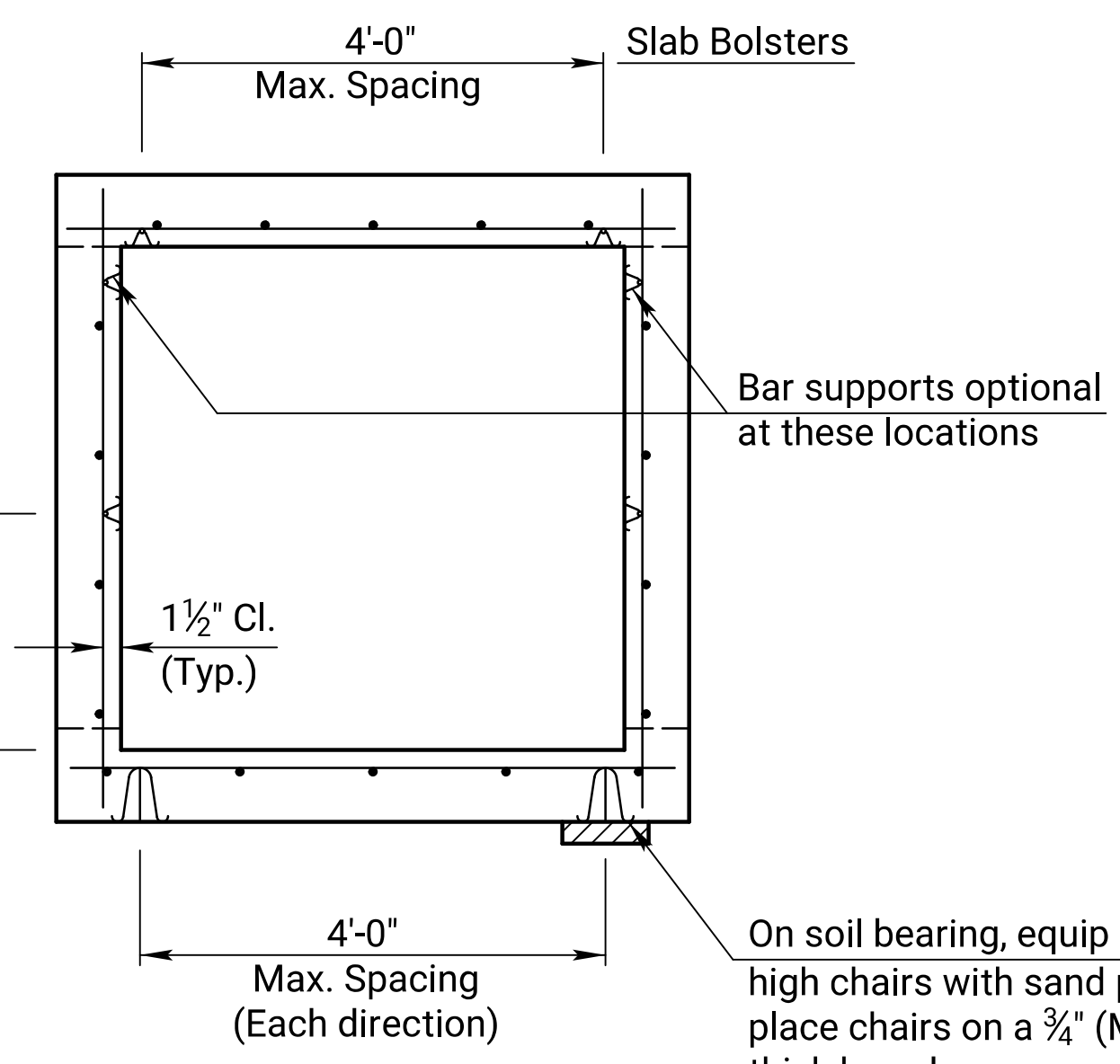
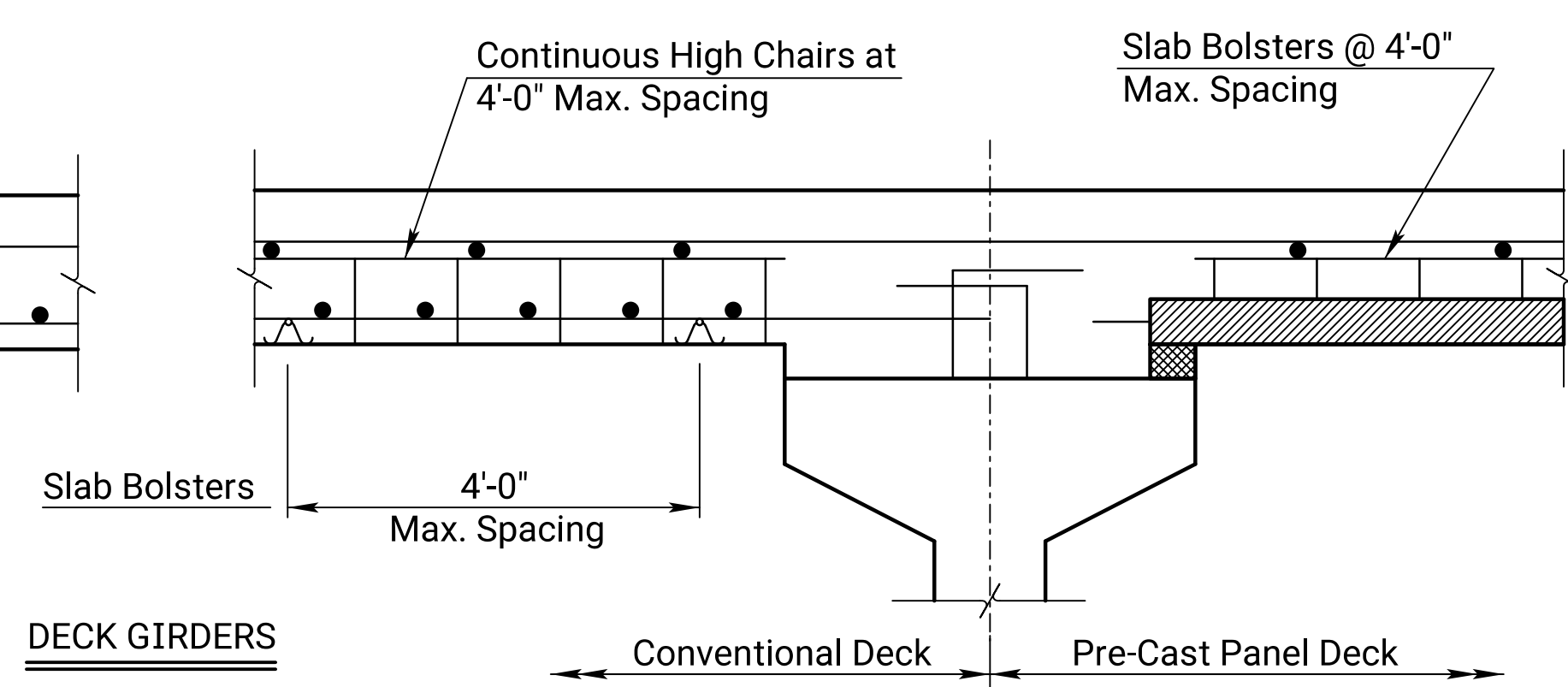
CONTINUOUS HAUNCHED SLAB



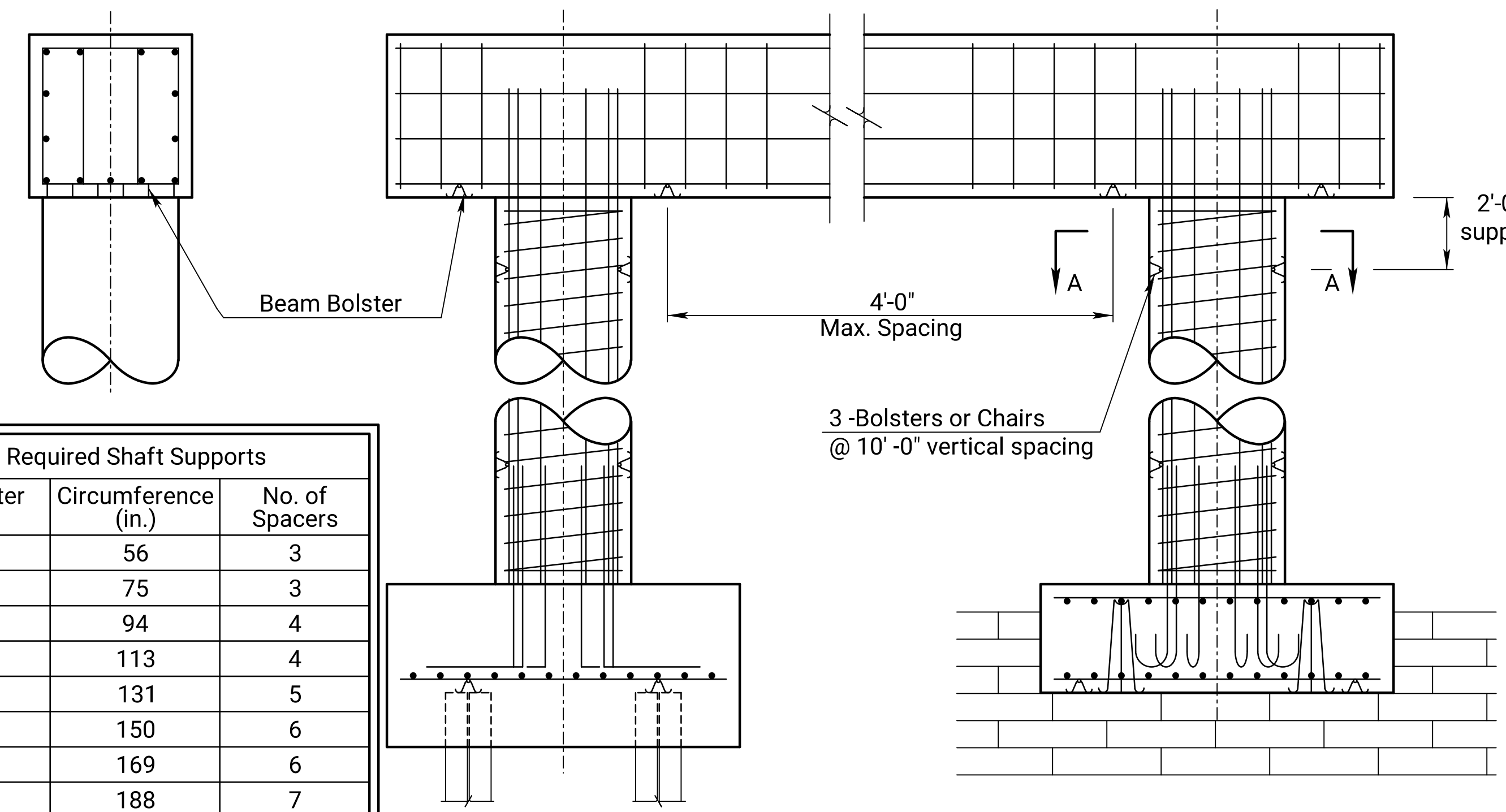
ABUTMENT



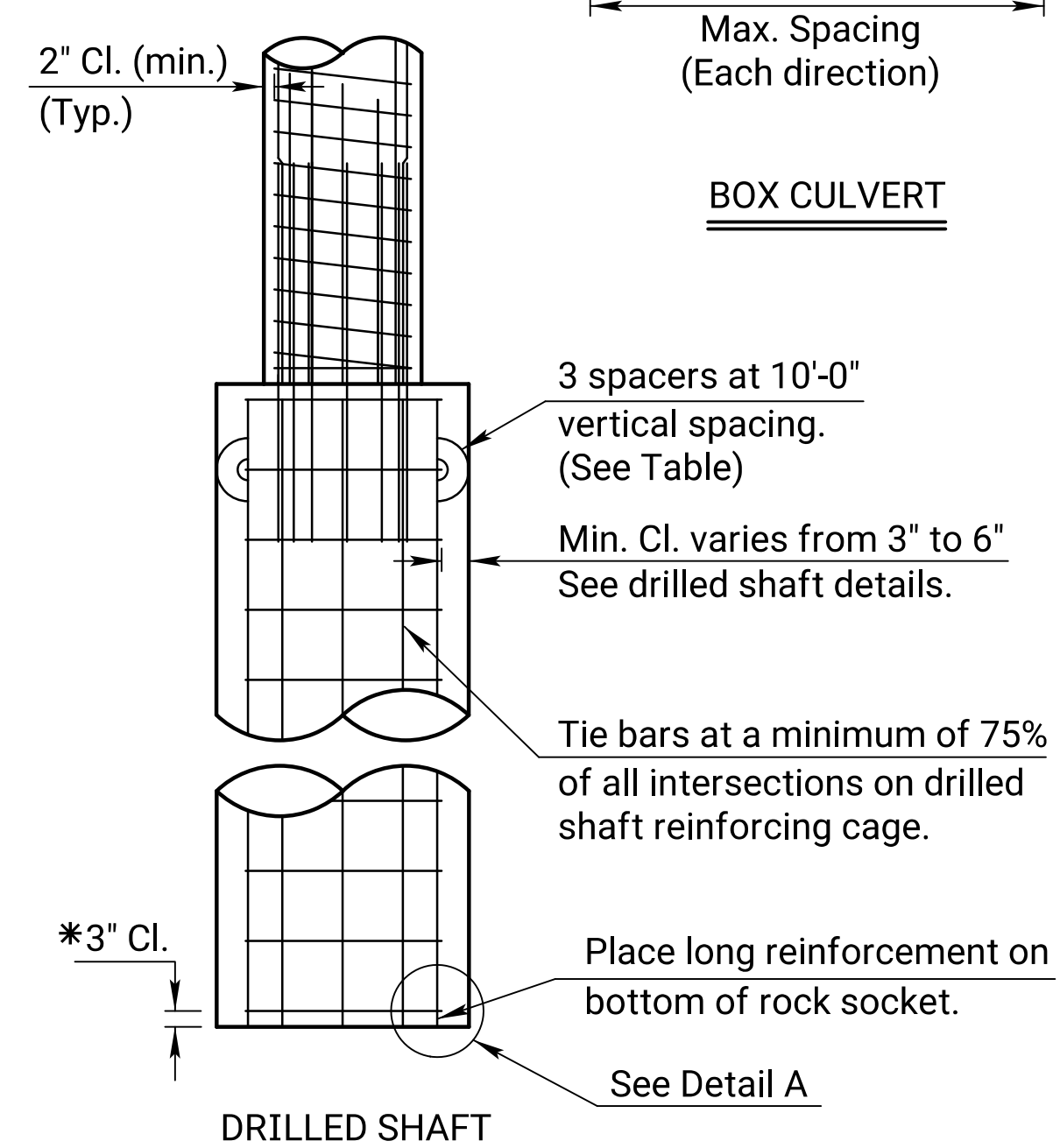
DECK GIRDERS



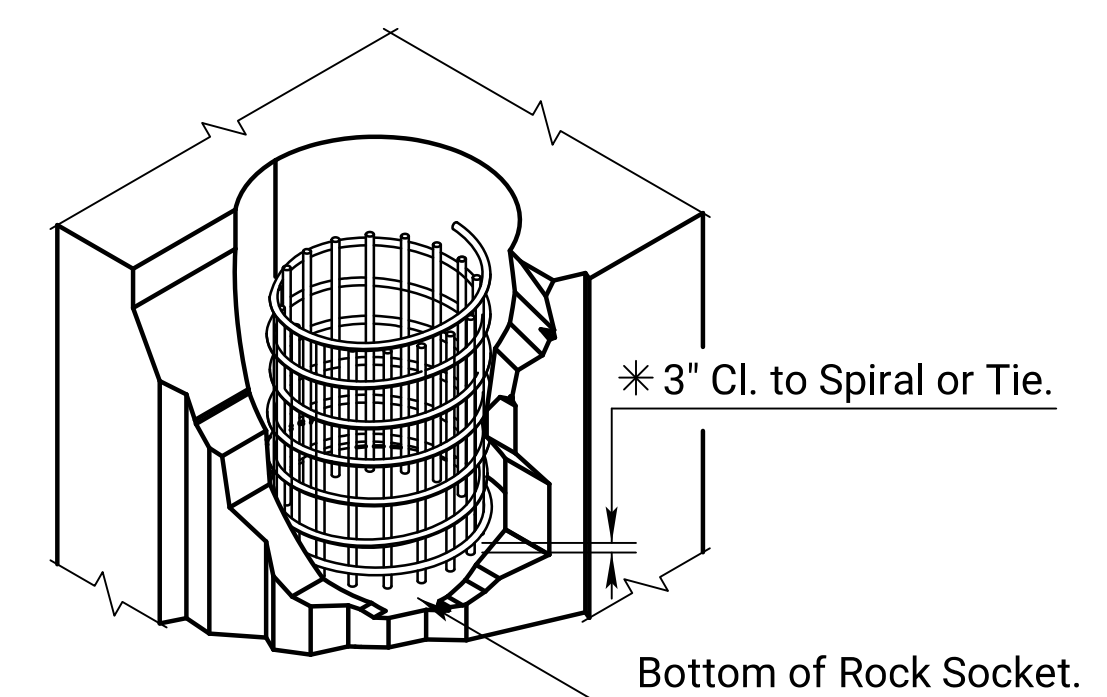
BOX CULVERT



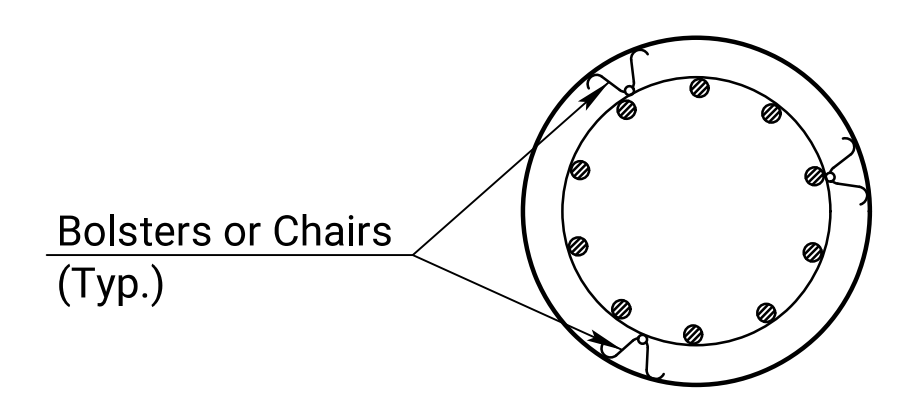
PIER



DRILLED SHAFT



* Note: Longitudinal reinforcing steel is placed on the bottom of the rock socket. Maintain 3" clearance from the bottom of rock socket to the first spiral or tie bar.



SECTION A-A

Required Shaft Supports		
Diameter (in.)	Circumference (in.)	No. of Spacers
18	56	3
24	75	3
30	94	4
36	113	4
42	131	5
48	150	6
54	169	6
60	188	7
66	207	7
72	226	8
78	244	9
84	263	9
90	282	10
96	301	11
102	320	11
108	339	12

NO.	DATE	REVISIONS	BY	APPD
05	11-10-10	Column Bar Supports Required	J.P.J.	T.L.F.
04	12-01-05	Drilled Shaft Spiral Steel Placement	J.P.J.	K.F.H.
03	08-21-00	Added Pre-Cast Panel Detail	R.A.M.	K.F.H.

KANSAS DEPARTMENT OF TRANSPORTATION

SUPPORTS AND SPACERS FOR REINFORCING STEEL

BR120

DESIGNED	R.A.M.	DETAILED	R.A.A.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	L.R.R.	DETAIL CK.	R.A.M.	QUAN. CK.	TRACE CK.	R.A.M.

11-17-10 APPD. Terry L. Fleck

KDOT Graphics Certified 07-27-2023 Sh. No. 58

Drawn By: JHOVERSO Plotted: 01-08-24
 File: c:\pwworking\central01\vd3377674\ks572801\bb113-26.dgn

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	59	135

P.I. Sta. 7339+07.78 = SW Cor. Sec. 16, T14S, R12E =
P.I. Sta. 165+66.60 on Proj. 99-99 K 516 Date 1928
N 476,313.797 E 8,615,481.392
1. Found 5/8" rebar (0.3' deep)
2. Found nail & wshr. NE side of pow. pole
3. Conc. nail & KDOT wshr. W side utility mrkr. po.
4. Conc. nail & KDOT wshr. E side of pow. pole
5. In TW of K-99 Hwy.

35.3' NW
52.2' NE
81.0' SSW

P.I. Sta. 7365+78.29 = W/4 Cor. Sec. 16, T14S, R12E =
Sta. 138+97.60 on Proj. 99-99 K 516 (Date 1928)
Δ=0°23'23" Lt. (No curve)
N 478,984.271 E 8,615,467.769
1. Set 1/2" x 24" rebar w/ orange KDOT cap
2. Top telephone pedestal
3. Line of N/S fence line
4. In line of fence line to E
5. In TW of K-99 Hwy.

26.5' E
30.5' W

P.O.T. Sta. 10+73.51
N 479896.45 E 8615245.27
P.O.T. Sta. 17+98.28
N 480235.77 E 8615599.81

P.I. Sta. 7392+27.09=NW Cor. Sec. 16, T14S, R12E =
Sta. 112+41.40 on Proj. 99-99 K 516 (Date 1928)
N 481,632,885 E 8,615,436.237
1. Found 5/8" rebar (0.3' deep)
2. Found nail & wshr. NW side cor. fc. po.
3. Found nail & wshr. top cor. fc. po.
4. Top steel cor. fc. po
5. TW of K-99 Highway

54.5' SSW
44.7' NW
44.7' NE
4.0' E

BID ITEM	QUANTITY	UNITS
Slope Protection (Riprap Stone)(Varies thick)(200#)	2,180	Cu.Yd.
Slope Protection (Aggregate)(12" thick)(D50=6")	107	Cu.Yd.
Bedding for Slope Protection (3" thick)(Type I)	33	Cu.Yd.
Bedding for Slope Protection (12" thick)(Type III)	547	Cu.Yd.
Temporary Erosion Control (Class 2)(Type F)	2,456	Sq. Yd.

W 1/4 Cor. Sec. 16, T14S, R12E
N 478,984.271 E 8,615,467.769
1. Set 1/2" x 24" KDOT plastic capped rebar (0.3' deep)
2. Cor. is on Sta. 7365+78.29
3. To the center of the top of telephone pedestal
4. To the line of a North-South Fence
5. In line with fence to E
6. In the traveled way of K-99 Highway

26.5' E
30.5' W

Ctr. Cor. Sec. 16, T14S, R12E
N 478,989.725 E 8,618,110.834
1. Found 1/2" x 12" (in rock) w/ KDOT alum. capped rebar (0.1' above ground)
2. Cor. is 2643.07' Rt. (East) of Sta. 7365+78.29 SE Δ=89°46'33"
3. Set conc. nail and KDOT wshr. W side fc. cor. po. 0.8' E
4. Set conc. nail and KDOT wshr. top fc. brace po.
5. Set cor. on W side fc. cor. po.
6. In line with fences North-South and East

Ctr. Cor. Sec. 17, T14S, R12E
N 478,942.533 E 8,612,817.273
1. Set 1/2" x 12" (in rock) w/ KDOT alum. capped rebar (0.3' above ground)
2. Cor. is 2650.82' Lt. (West) of Sta. 7365+78.29 SW Δ=89°23'24"
3. Set conc. nail and KDOT wshr. W side 30' cedar
4. To line of newer fence North
5. In line with fences North-West
6. At Northwest side fence corner post

NW Cor. Sec. 17, T14S, R12E
N 481,632,885 E 8,615,436.237
1. Found 5/8" rebar (0.3' deep)
2. Cor. is on Sta. 7392+27.09
3. Found nail and wshr. in top fc. cor. po.
4. To the center of the top steel fc. cor. po.
5. To the traveled way of K-99 Highway
6. In traveled way of East-West road

47.7' NW
44.7' NE
4.0' E

N 1/4 Cor. Sec. 17, T14S, R12E
N 481,648,799 E 8,618,080.040
1. Found 5/8" capped rebar LS (0.8' deep)
2. Cor. is 2643.85' Rt. (East) of Sta. 7392+27.09 SE Δ=89°39'46"
3. Found spike and wshr. in top fc. cor. po.
4. Set nail and KDOT wshr. in top strand steel fc. p.
5. To the traveled way of East-West road
6. In line with fence to South

20.4' S
25.9' NNE
3.0' S

N 1/4 Cor. Sec. 17, T14S, R12E
N 481,593.554 E 8,612,810.003
1. Set 1/2" x 24" KDOT alum. capped rebar (0.3' deep)
2. Cor. is 2626.53' Lt. (West) of Sta. 7392+27.09 SE Δ=89°49'27"
3. Set conc. nail and KDOT wshr. in side fc. cor. po.
4. Set conc. nail and KDOT wshr. in site fc. gate po.
5. To the traveled way of East-West road
6. In line with fence to North

19.7' N
28.7' NE

Remove Existing Br. No. 99-99-139.73(072), 2 ft. below New Construction

Exist. Channel

Begin Channel Realignment Sta. 10+73.51

Sta. 7375+41.16 Proposed Br. No. 99-99-139.72 (113)

Ch. Realignment Sta. 13+01.57

End Channel Realignment Sta. 17+98.28

CURVE NO.	START STATION	END STATION	RADIUS	Δ	TANGENT	LENGTH	CHORD DIRECTION	CHORD LENGTH
C-1	10+73.51	11+78.64	105.89'	56°52'59.98"	81.24'	105.13'	N60°53'09.86"E	100.86'
C-2	13+30.55	14+44.95	66.83'	98°04'36.91"	76.98'	114.40'	N40°17'21.39"E	100.93'
C-3	15+56.97	16+89.06	89.85'	84°14'10.32"	57.36'	132.09'	N33°22'08.09"E	120.51'

TANGENT NO.	START STATION	END STATION	LENGTH	DIRECTION	START POINT
L-1	11+78.64	13+30.55	151.91'	N89°19'40"E	N 479945.64 E 8615333.39
L-2	14+44.95	15+56.97	112.02'	N08°44'57"W	N 480024.41 E 8615550.56
L-3	16+89.06	17+98.28	109.22'	N75°29'13"E	N 480235.77 E 8615599.81

Utility Owners

Evergy
818 So. Kansas Ave
Topeka, KS 66612
Scott Schlageck
785-575-6014

Telephone Cable
Century Link
Kenny (Integrity Locate)
913-305-6679

Gas Line
City of Eskridge
785-449-2621



KANSAS DEPARTMENT OF TRANSPORTATION
GEOMETRY
CHANNEL REALIGNMENT
STA. 10+73.51 TO STA. 17+98.28

Plotted By: JHOVERSO
File: c:\pwworking\central\0142696672\k572801rpl-01.dgn
Plot Date: 01-08-24

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	60	135

For alignment and section corner reference, see Geometry Channel Realignment Sheet.

Channel realignment grading is to be done by the grading contractor in phases:

- Phase I - (Connect to existing channel between K-99 and Shoofly Detour while Shoofly Detour is in use).
- Phase II - (Complete remaining Channel Realignment during eradication of Shoofly Detour).

SLOPE PROTECTION QUANTITIES			
BID ITEM	QUANTITY	UNIT	
SLOPE PROTECTION (RIPRAP STONE)(Varies thick)(200*)	2,180	Cu. Yd.	
SLOPE PROTECTION (Aggregate) (12" thick)(D50 = 6")	107	Cu. Yd.	
BEDDING FOR SLOPE PROTECTION (3" thick)(Type I)	33	Cu. Yd.	
BEDDING FOR SLOPE PROTECTION (12" thick)(Type III)	547	Cu. Yd.	

DATE	8/26/21
BY	D. Wimmer
DATE	8/24/23
BY	E. Dove
REFERENCES NOTED	
REFERENCES CHECKED	

CHANNEL REALIGNMENT CURVE C-1 DATA
P.I. Sta. 11+30.87 Bk. = 10+73.51 Ahd.
N 479944.96 E 8615276.04
Δ=56°53'00" L=105.13'
R=105.89' E=14.54'
T=81.24'

CHANNEL REALIGNMENT CURVE C-2 DATA
P.I. Sta. 14+07.54 Bk. = 13+30.56 Ahd.
N 479948.32 E 8615562.27
Δ=98°04'37" L=114.40'
R=66.83' E=35.11'
T=76.98'

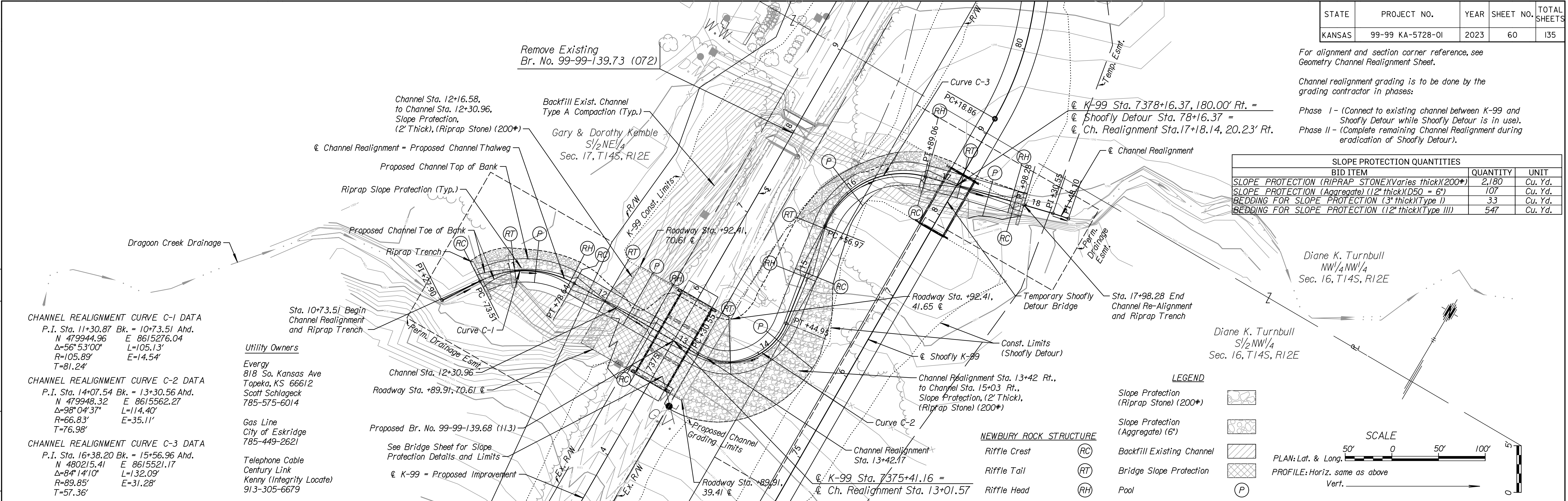
CHANNEL REALIGNMENT CURVE C-3 DATA
P.I. Sta. 16+38.20 Bk. = 15+56.96 Ahd.
N 480215.41 E 8615521.17
Δ=84°14'10" L=132.09'
R=89.85' E=31.28'
T=57.36'

Utility Owners

Evergy
818 So. Kansas Ave
Topeka, KS 66612
Scott Schlageck
785-575-6014

Gas Line
City of Eskridge
785-449-2621

Telephone Cable
Century Link
Kenny (Integrity Locate)
913-305-6679

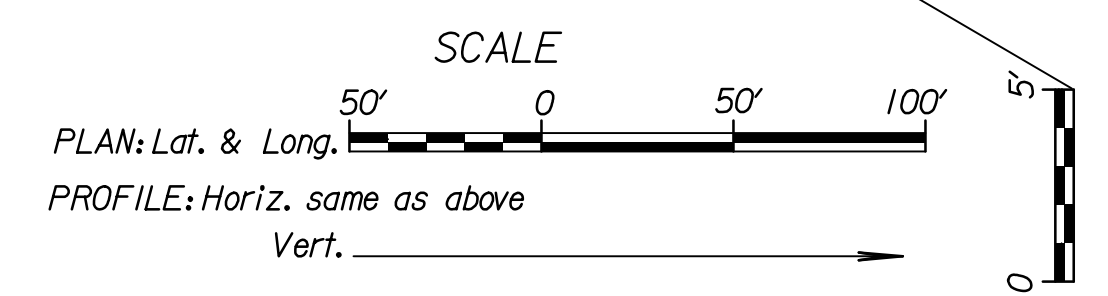


LEGEND

- Slope Protection (Riprap Stone) (200*)
- Slope Protection (Aggregate) (6")
- Backfill Existing Channel
- Bridge Slope Protection
- Pool

NEWBURY ROCK STRUCTURE

- Riffle Crest (RC)
- Riffle Tail (RT)
- Riffle Head (RH)



BM*10 Set "T" shaped fc. po. S side of rock @ gate po. 22.29' Lt. @ Sta. 7369+06.99 Elev. 1350.28

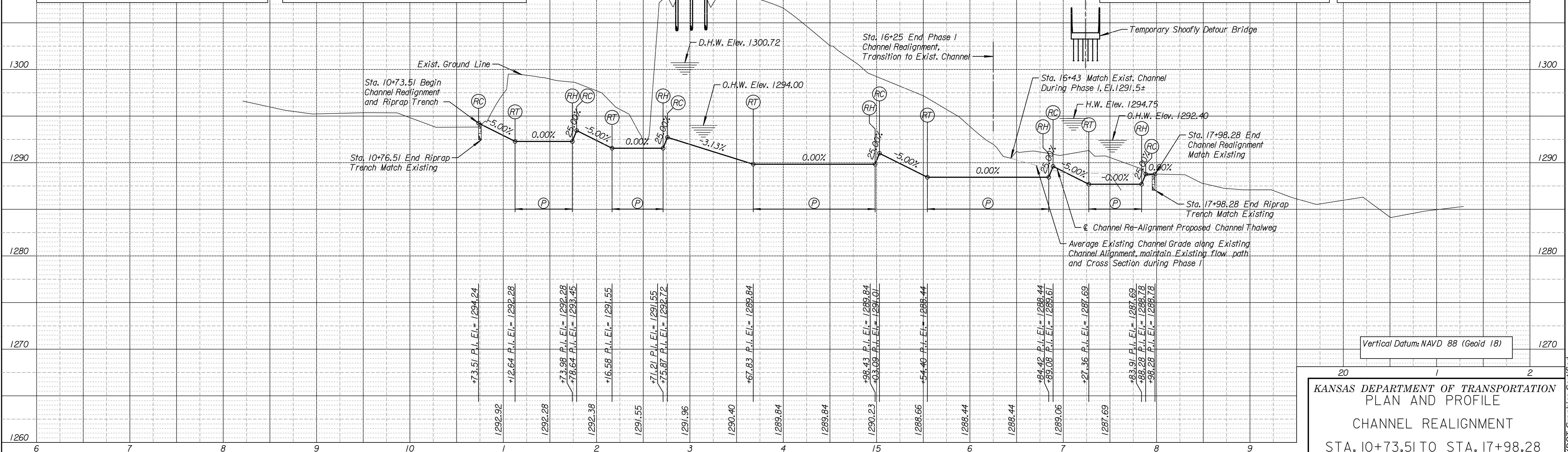
BM*11 Set mag. nail & KDOT wshr. top SE abut. of bridge 16.75' Rt. @ Sta. 7377+78.17 Elev. 1302.52

Proposed Br. No. 99-99-139.68 (113)

Sta. 17+18.14 @ Shoofly Detour Bridge

BM*12 Set mag. nail & KDOT wshr. top E hawl. RCB 15.22' Rt. @ Sta. 7384+11.38 Elev. 1306.98

BM*13 Set RR spike in E face of pow. pole 34.18' Lt. @ Sta. 7387+80.38 Elev. 1317.29



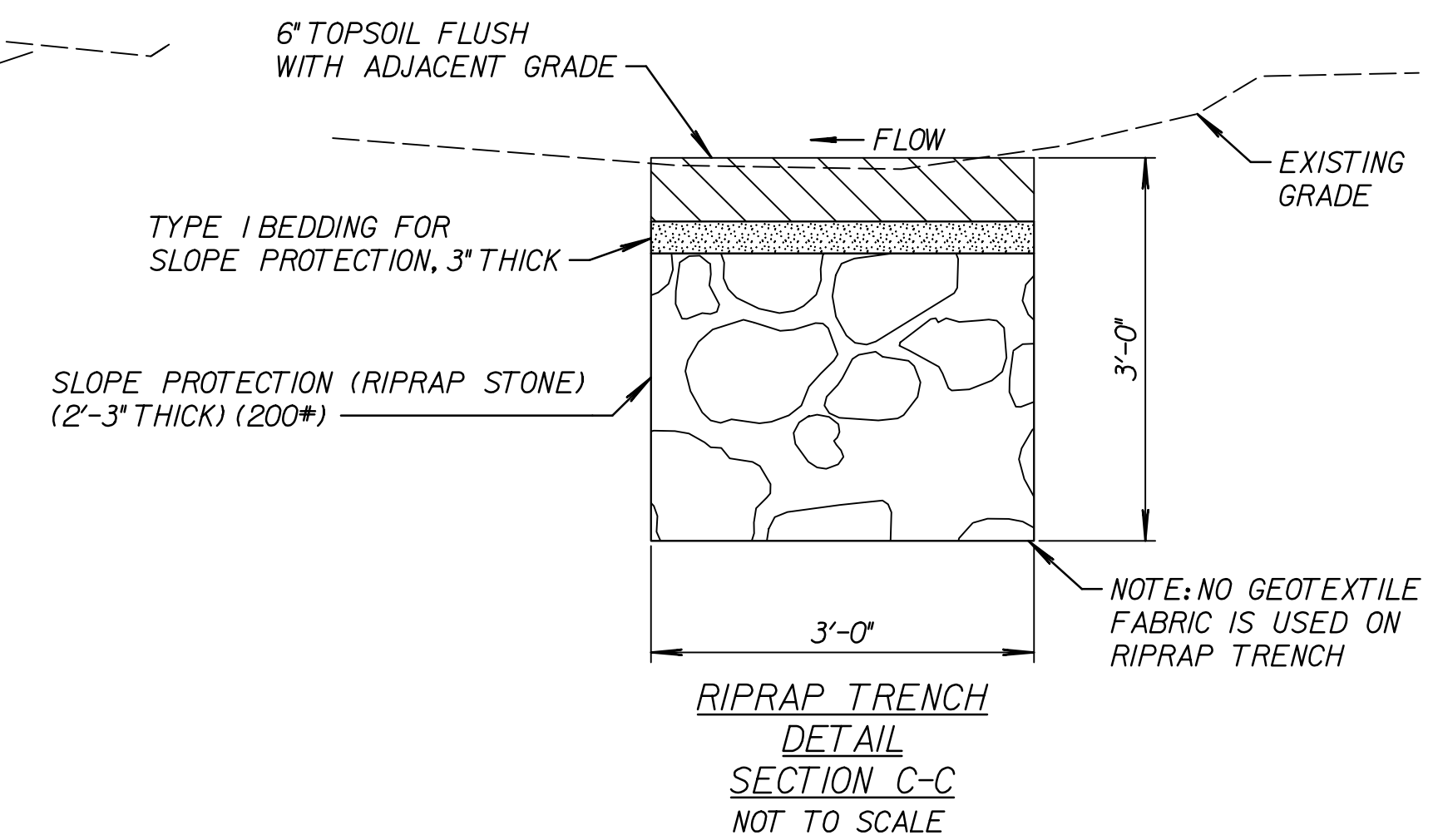
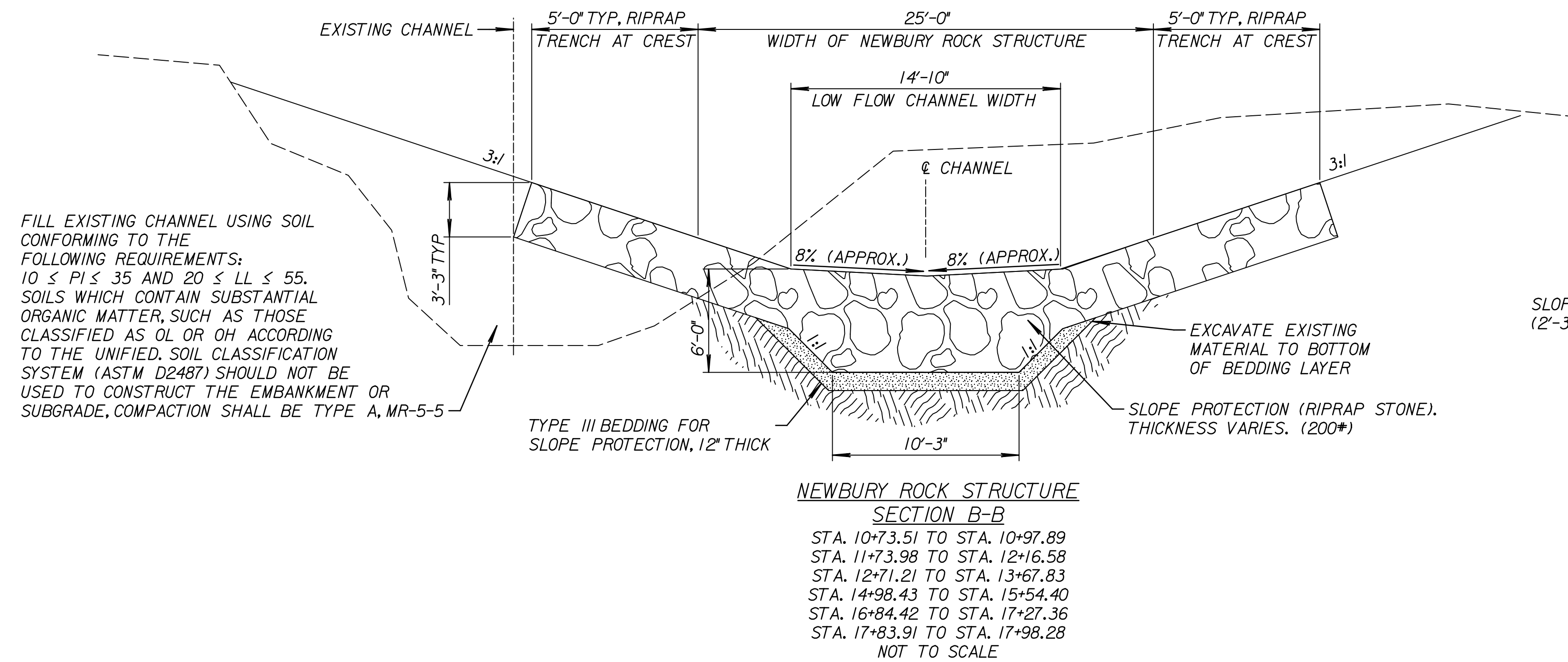
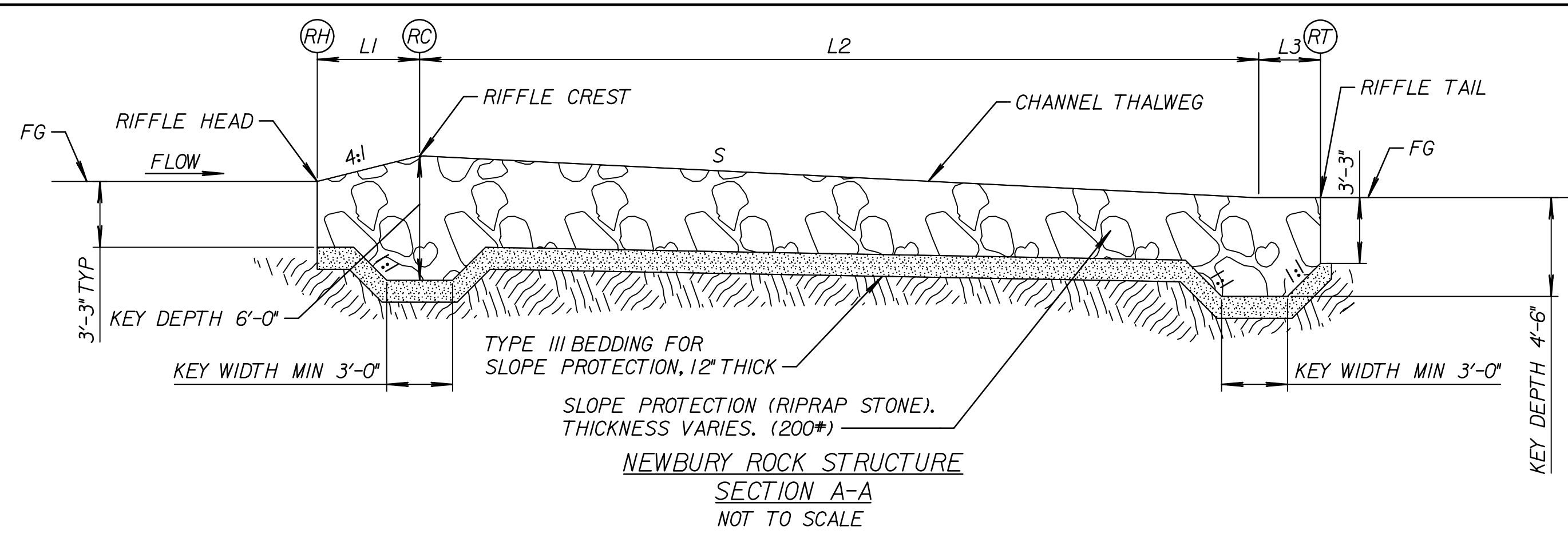
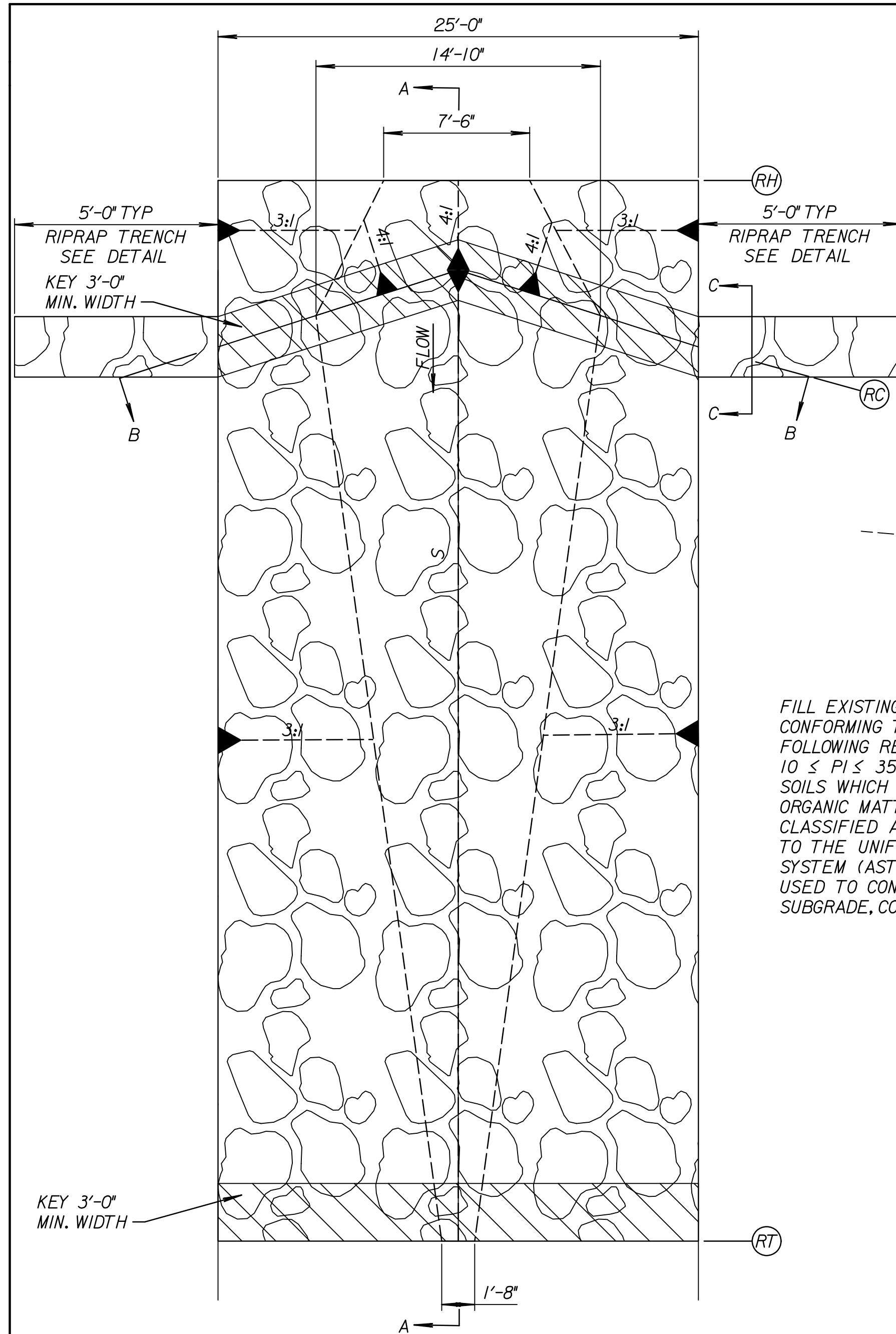
Vertical Datum: NAVD 88 (Geoid 18)

KANSAS DEPARTMENT OF TRANSPORTATION
PLAN AND PROFILE
CHANNEL REALIGNMENT
STA. 10+73.51 TO STA. 17+98.28
SDOT Graphics Certified 01-08-2024 Sh. No. 60

Drawn By: JHOVERSO Plotted: 01-08-24
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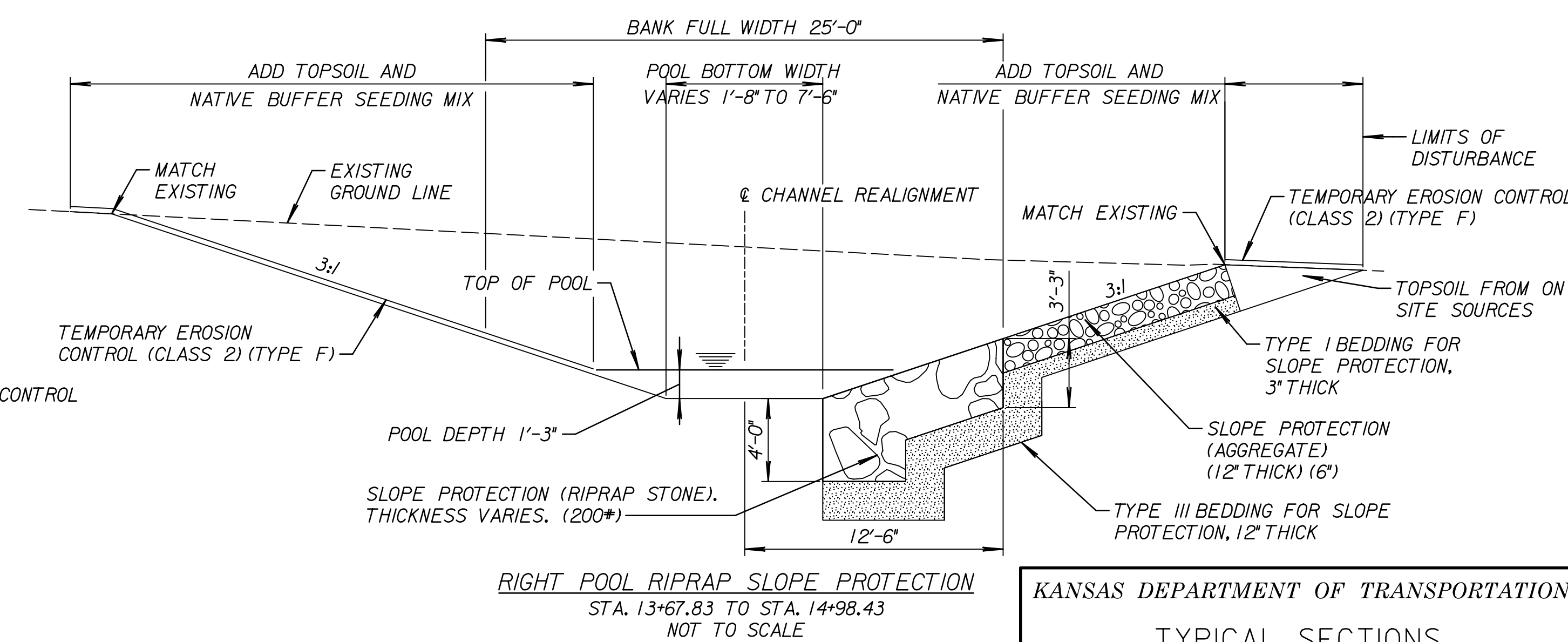
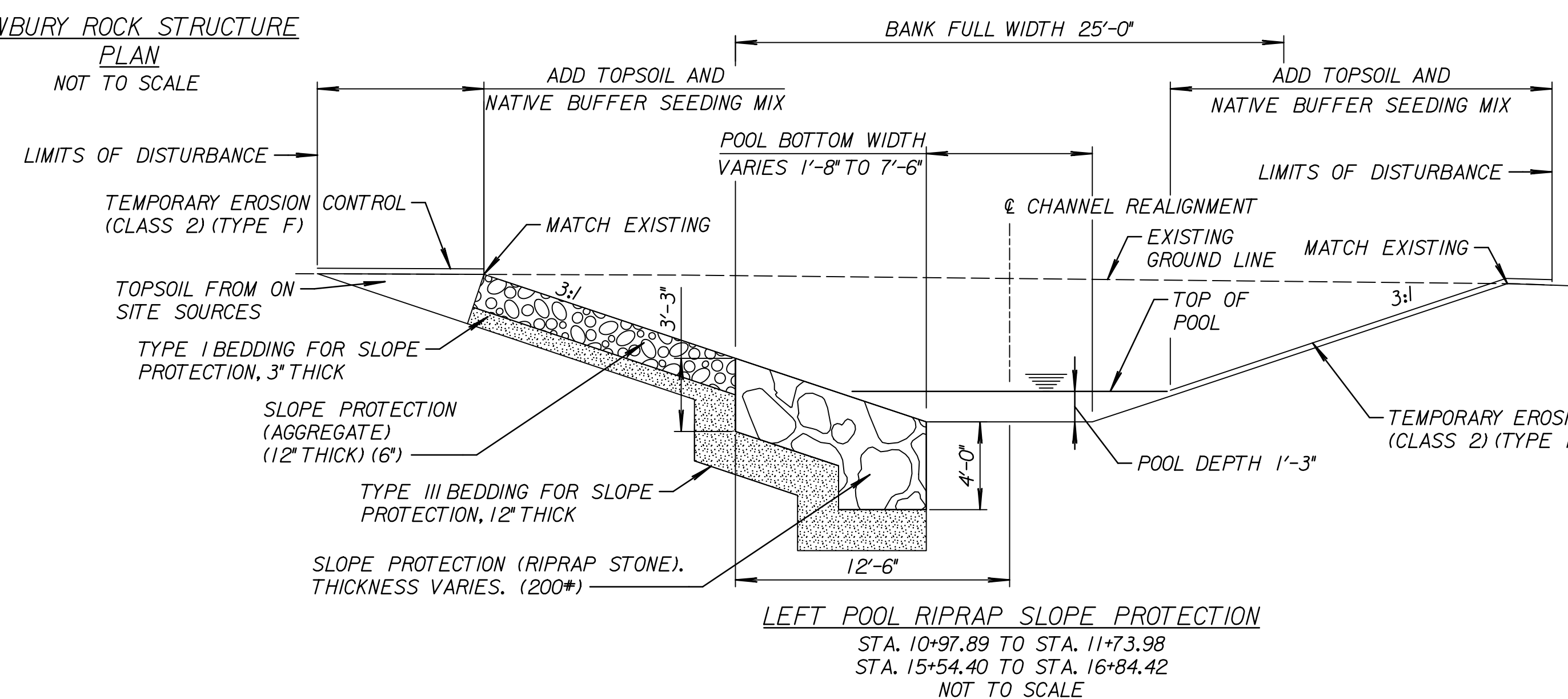
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	61	135

NEWBURY STRUCTURE LOCATIONS AND DIMENSION (Measured Along Channel Realignment @)				
RIFFLE CREST STATION	S	L1	L2	L3
16+89.08	5%	4'-8"	37'-11"	2'-10"
15+03.09	5%	4'-8"	51'-3"	2'-10"
12+75.89	3.13%	4'-6"	92'-0"	2'-10"
11+78.64	5%	4'-8"	37'-11"	2'-10"
10+73.51	5%	4'-8"	39'-2"	2'-10"



FILL EXISTING CHANNEL USING SOIL CONFORMING TO THE FOLLOWING REQUIREMENTS:
 $10 \leq PI \leq 35$ AND $20 \leq LL \leq 55$.
 SOILS WHICH CONTAIN SUBSTANTIAL ORGANIC MATTER, SUCH AS THOSE CLASSIFIED AS OL OR OH ACCORDING TO THE UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D2487) SHOULD NOT BE USED TO CONSTRUCT THE EMBANKMENT OR SUBGRADE. COMPACTION SHALL BE TYPE A, MR-5-5

NEWBURY ROCK STRUCTURE
 PLAN
 NOT TO SCALE



KANSAS DEPARTMENT OF TRANSPORTATION
 TYPICAL SECTIONS
 CHANNEL REALIGNMENT

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	62	135

DRAINAGE STRUCTURES

STATION	SIDE	SIZE	TYPE	Gr. 4.0 CONC. (Cu.Yds.)	Gr. 4.0 (AE) CONC. (Cu.Yds.)	BRIDGE BACKWALL PROT. SYSTEM (Sq. Yds.)	REINF. STEEL (GRADE 60 LBS.)	REINF. STEEL (GRADE60) (EPOXY COATED LBS.)	FOUND. STAB. (Cu. Yds.)	GRAN. BACKFILL Wingwalls (Cu. Yds.)	ENTRANCE PIPES						END SECTIONS ●											REMARKS													
											ENTRANCE PIPES						ENTRANCE PIPES						RCHE, CAMA, ACSMA						TYPE I		TYPE III		TYPE IV								
											RCPHE, CAMAC, ACSMAC						RCP, ACSP, CAP												LT.	RT.	LT.	RT.	LT.	RT.	Slope						
Mainline																																									
7380+70	Lt.	1.5 ^{0"}	EP (RCPHE, CAMAC, ACSMAC)								46																														
7384+11.04	Rt.	6' x 4'	RCB	10.4	2.2	11	680	750	6	12																															
Shoofly 81+50	Lt.	18"	EP (RCP, ACSP, CAP)																																						
TOTAL				10.4	2.2	11	680	750	6	12	46																														

Note: ● - See Pipe Culvert Summary Sheet for allowable End Section types.

Plotted by : August Zuno 18-JAN-2024 10:48
File : rss049.dgn

KANSAS DEPARTMENT OF TRANSPORTATION									
SUMMARY OF DRAINAGE STRUCTURES									
RD049									
FHWA APPROVAL		05-28-08				APPD.		James. O. Brewer	
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.					
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.	S.W.K.					

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	63	135

GUARDRAIL, STEEL PLATE (MGS)							
STATION	STATION	SIDE	STEEL PLATE GUARD RAIL (MGS) (ft)	END TERM. MGS-MSKT Alt. 1 (each)	END TERM. MGS-SOFTSTOP Alt. 2 (each)	END TERM. MGS-SRT Alt. 1 (each)	END TERM. MGS-FLEAT Alt. 2 (each)
7373+06.98	7374+90.60	Rt.	137.50	1	1		
7373+57.20	7374+90.60	Lt.	87.50	1	1		
7375+91.72	7378+25.11	Rt.	187.50	1	1		
7375+91.72	7377+75.35	Lt.	137.50	1	1		
Shoofly Detour							
76+80.33	77+82.56	Rt.	62.50			1	1
76+93.08	77+82.56	Lt.	50.00			1	1
78+50.18	79+38.37	Rt.	50.00			1	1
78+50.18	79+41.02	Lt.	50.00			1	1
TOTALS			762.50	4	4	4	4

SALVAGED TOPSOIL		
STATION TO STATION	SIDE	SQ. YDS.
7371+40 to 7381+90	Lt.	4,691.3
7367+50 to 7385+20	Rt.	15,735.9
TOTAL		20,427.2

RECAPITULATION OF BRIDGE QUANTITIES		
BRIDGE NUMBER	STATION	SEE SHEET NO.
99-99-139.68 (113)	7375+41.16	32
Detour Bridge	78+16.37	45

TEMPORARY FENCING				
STATION	STATION	SIDE	BARBED WIRE (ft)	COMMENTS
7366+05	7383+00	Rt.	1,898.3	
7374+20	7376+00	Lt.	494.2	
TOTAL			2,392.5	

SLOPE PROTECTION (RIPRAP STONE) (200 LB)						
LOCATION	STATION	SIDE	LENGTH FT.	DEPTH IN.	WIDTH FT.	VOLUME CU YDS
K-99	7384+00 7384+20	Rt.	10	18	25	375
TOTAL						375

TRANSPORTING SALVAGEABLE MATERIAL #			
ITEM	STATION TO STATION	CU YDS	REMARKS
COMMON EXCAVATION	66+10.67 85+59.36	1,000	
TOTAL		1,000	

#Federally Non-Participating - See General Note Sh. No. 7

CONCRETE PAVEMENT (10" UNIFORM) (AE) (BR. APPROACH)			
STATION	STATION	Sq. Yard	REMARKS
7374+78.16	7374+91.16	53.40	South Bridge Approach
7375+91.16	7376+04.16	53.40	North Bridge Approach
TOTAL		106.80	

SLOPE DRAIN (STONE)			
LOCATION	STATION	SIDE	LIN. FT.
K-99	7376+05.41	Rt.	22
K-99	7376+05.41		54
TOTAL			76

MOWING		
LOCATION	STATION	PMPS
K-99	7367+50 to 7385+20	0.3

REMOVAL OF EXISTING STRUCTURES * (FOR INFORMATION ONLY)		
STATION	SIDE	DESCRIPTION
7376+75.26 to 7378+64.03	Rt.	(189') K-99 Guardrail
7377+19.47 to 7378+36.37	Lt.	(120') K-99 Guardrail
7377+97.47	€	Br. No. 99-99-139.73 (072)
7376+29	Rt.	18" x 21.60' CMP
7378+72	Lt.	15" x 36.40' CMP
7380+70	Lt.	15" x 19.80' CMP

*The listing shown may not be complete. Payments for structures or obstruction not listed but whose removal is required by the construction, as determined by the Engineer, should not be paid for directly, but should be included in the bid item "Removal of Existing Structures."

EARTHWORK														
LOCATION	STATION TO STATION	EXCAVATION				COMPACTION		NOT SUBGRADED THROUGH CUTS				* EMBANKMENT (CU.YDS.)		▲ PLACE SELECT SOIL CU.YDS.
		COMMON		Ø ROCK		Ø CONTR. FURN. CU.YDS.	TYPE AA MR-5-5 CU.YDS.	TYPE A MR-5-5 CU.YDS.	COMM. CU.YDS.	TYPE AA CU.YDS.	INITIAL CONSOL.	SETTLE-MENT		
		CU.YDS.	VMF	CU.YDS.	VMF								CU.YDS.	
Phase I	66+10.67 to 85+59.36	4,117	0.78			35	1.10	7,803	1,535	7,762	695	695		
Phase II	7367+50 to 7385+20	14,272	0.78	451	1.00	108	1.10		664	7,994	352	352		
Phase III	66+10.67 to 85+59.36	9,748	0.78	≈ 237		18	1.10							
TOTALS		28,137		688		161		7,803	2,199	15,756	1,047	1,047		

Ø Existing Pavement and Non-Durable Shale to be wasted.
 Ø Assumed VMF for Contractor furnished excavation is 0.80
 ≈ Includes shoofly detour bridge rock riprap removal

* Subsidiary (see General Note).
 ▲ See General note.

RECAPITULATION OF ROAD QUANTITIES		
ITEM	QUANTITY	UNIT
CONTRACTOR CONSTRUCTION STAKING	Lump Sum	LSUM
FIELD OFFICE AND LABORATORY (TYPE A)	1	EACH
FOUNDATION STABILIZATION (SET PRICE)	1	CUYD
FOUNDATION STABILIZATION	6	CUYD
MOBILIZATION	Lump Sum	LSUM
MOBILIZATION (DBE)	Lump Sum	LSUM
REMOVAL OF EXISTING STRUCTURE	Lump Sum	LSUM
MAINTENANCE AND RESTORATION OF HAUL ROAD (SET PRICE)	Lump Sum	LSUM
CONCRETE FOR SEAL COURSE (SET PRICE)	1	CUYD
TRANSPORTING SALVAGABLE MATERIAL	1,000	CUYD
CLEARING AND GRUBBING	Lump Sum	LSUM
COMMON EXCAVATION (RURAL SMALL)	29,184	CUYD
COMMON EXCAVATION (CONTRACTOR FURNISHED)	7,803	CUYD
ROCK EXCAVATION	688	CUYD
ROCK EXCAVATION (NON-DURABLE SHALE)	161	CUYD
COMPACTION OF EARTHWORK (TYPE A) (MR-5-5)	15,756	CUYD
COMPACTION OF EARTHWORK (TYPE AA) (MR-5-5)	3,246	CUYD
WATER (GRADING) (SET PRICE)	1	MGAL
SALVAGED TOPSOIL	20,427	SOYD
END SECTION (1.5 SQ.FT.)	2	EACH
ENTRANCE PIPE (1.5 SQ.FT.)	46	LNFT
ENTRANCE PIPE (18")	32	LNFT
CONCRETE (GRADE 4.0) (RCB)	10.4	CUYD
CONCRETE (GRADE 4.0) (AE) (RCB)	2.2	CUYD
BRIDGE BACKWALL PROTECTION SYSTEM	11	SOYD
REINFORCING STEEL (GRADE 60)	680	LBS
REINFORCING STEEL (GRADE 60)(EPOXY COATED)	750	LBS
GRANULAR BACKFILL (WINGWALLS)	12	CUYD
GUARDRAIL, STEEL PLATE (MGS)	762.50	LNFT
GUARDRAIL, END TERMINAL (MGS-SRT) Alt #1	4	EACH
GUARDRAIL, END TERMINAL (MGS-FLEAT) Alt #2	4	EACH
GUARDRAIL, END TERMINAL (MGS-MSKT) Alt #1	4	EACH
GUARDRAIL, END TERMINAL (MGS-SOFTSTOP) Alt #2	4	EACH
GUARDRAIL, REMOVAL OF STEEL PLATE	375.00	LNFT
SLOPE PROTECTION (AGGREGATE)	107	CUYD
SLOPE PROTECTION (RIPRAP STONE)	2,555	CUYD
BEDDING FOR SLOPE PROTECTION	580	CUYD
SLOPE DRAIN (STONE)	76	LNFT
TEMPORARY SURFACING MATERIAL (AGGREGATE) (SET PRICE)	1	CUYD
MAILBOX INSTALLATION (SET PRICE)	1	EACH
MOWING	0.3	PMPS
RIGHT-OF-WAY SURVEY MONUMENT	11	EACH
CONCRETE PAVEMENT (10" UNIFORM) (AE) (BR APP)	107	CUYD
FENCE (BARBED WIRE) (TEMPORARY)	2,393	LNFT
CURING ENVIRONMENT	Lump Sum	LSUM

Federally Non-Participating
 For Channel Realignment Quantities, See Sh. No. 60
 For Surfacing Quantities, See Sh. No. 64
 For Temporary Project Pollution Control Quantities, See Sh. No. 65
 For Seeding Quantities, See Sh. No. 76
 For Signing Quantities, See Sh. No. 82
 For Traffic Control Quantities, See Sh. No. 95

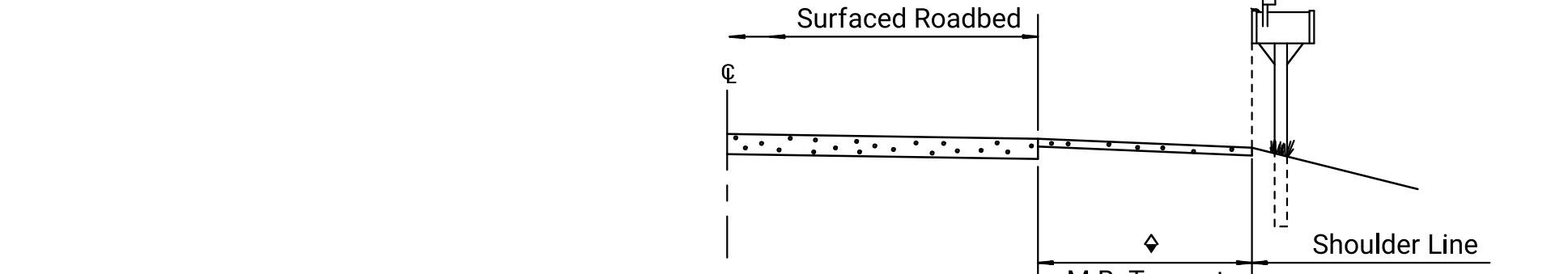
KANSAS DEPARTMENT OF TRANSPORTATION					
SUMMARY OF QUANTITIES					
RD050					
NO.	DATE	REVISIONS	BY	APPD	
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01	01-09-91	Detailed on CADD	R.J.S.	J.O.B.	
FHWA APPROVAL 05-28-08 APPD. James O. Brewer					
DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.	
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	S.W.K.	

Table with 5 columns: STATE, PROJECT NO., YEAR, SHEET NO., TOTAL SHEETS. Row 1: KANSAS, 99-99 KA-5728-01, 2023, 64, 135

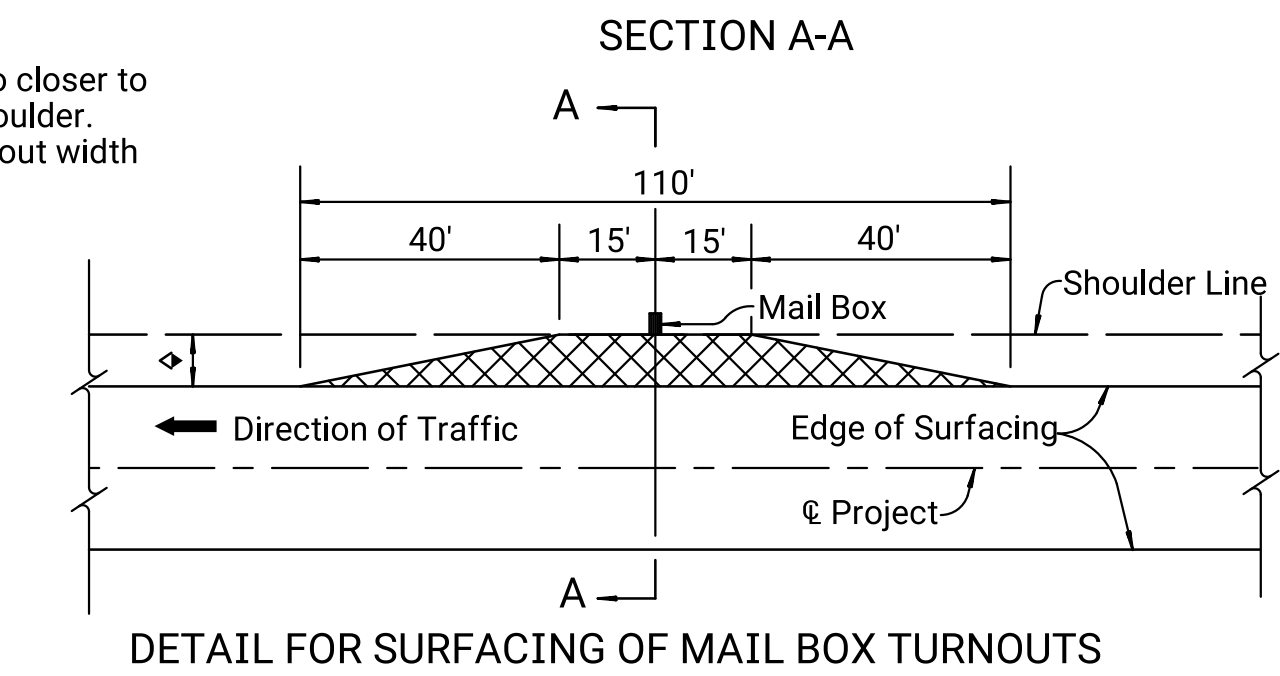
GENERAL NOTE: On surfacing projects, the 6" of Compaction Type AA, shown for the center portion on the roadbed, is for the purpose of restoring the original Compaction Type AA which may have been lost since grading operations.

SUMMARY OF QUANTITIES table with 7 columns: ITEM, MAINLINE, SHOOFLY, ENTRANCE, # GUARDRAIL, TOTAL, UNIT. Includes rows for HMA Commercial Grade, Aggregate Base, Surfacing Material, and Aggregate Shoulder.

- Quantities are based on the following rates:
Computed at the rate of 145 lbs per cu. ft.
Computed at the rate of 156 lbs per cu. ft.
Computed at the rate of 4" thickness for guardrail widening pads
Includes 20.3 Tons & 489.5 Sq Yds. for mailbox turnout.

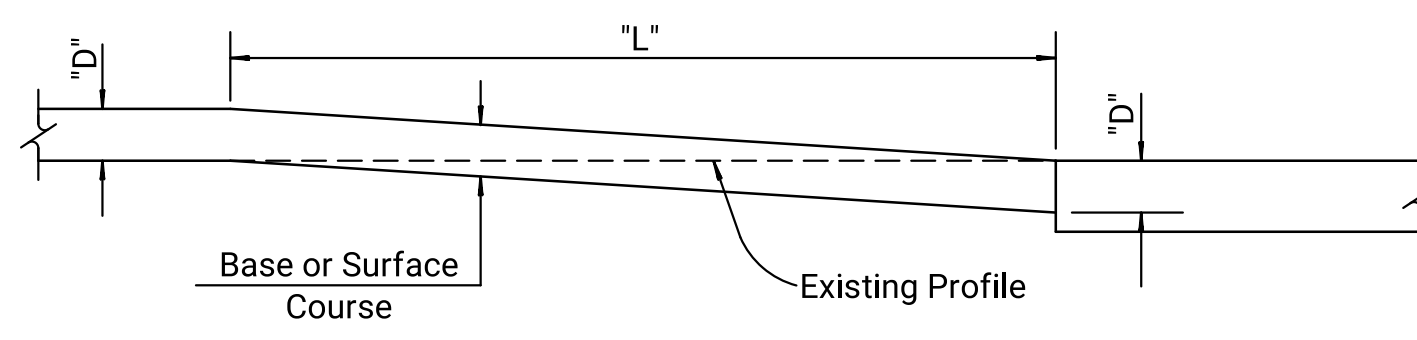
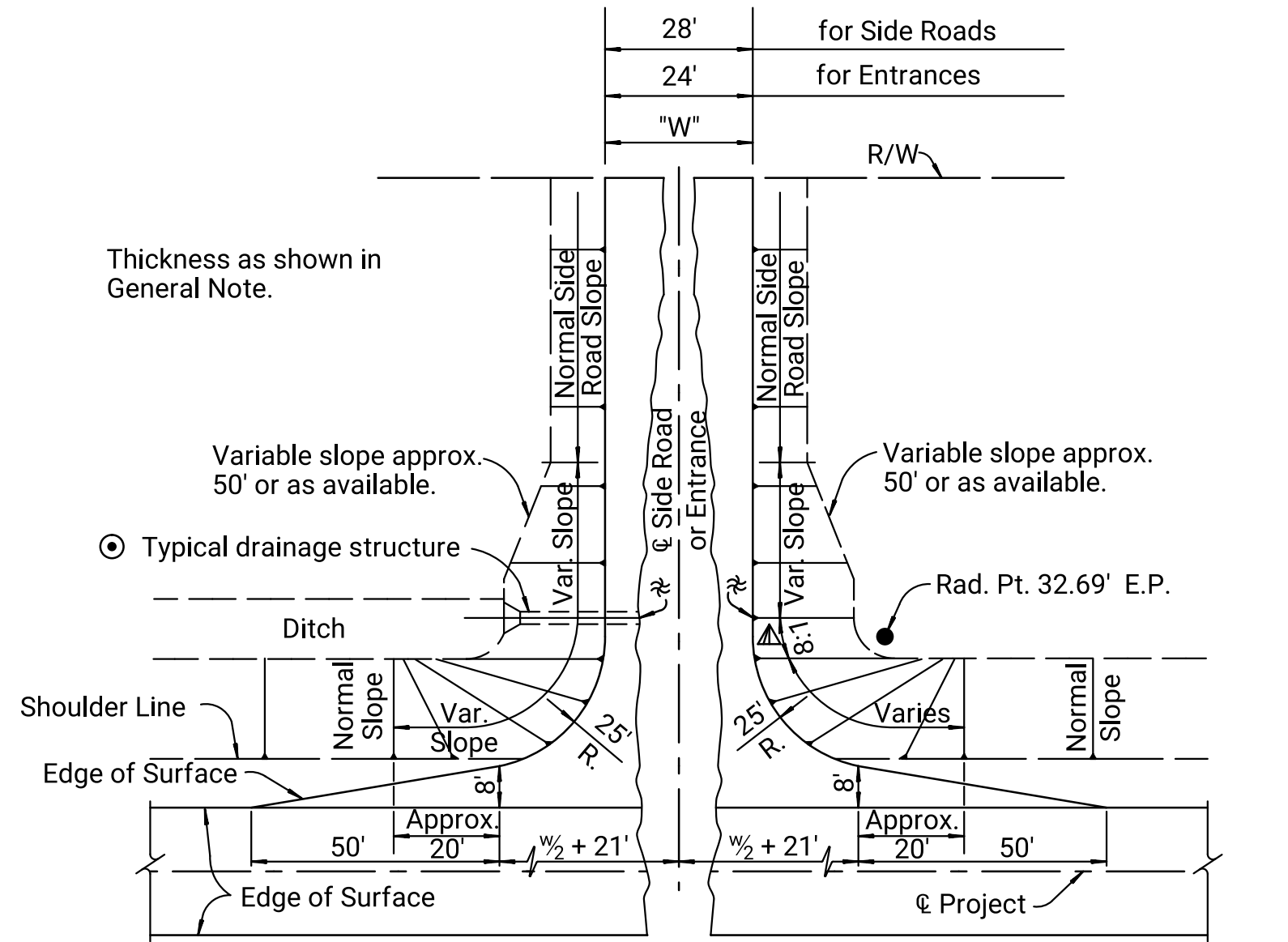


Note: The face of Mail Box should be no closer to the roadway than the edge of the shoulder. Align with edge of turnout when turnout width is greater than shoulder width.



MILLINGS table with 3 columns: LOCATION, STATION, TON. Includes rows for MILLINGS and SHOOFLY.

TRANSPORTING SALVAGEABLE MATERIAL table with 4 columns: ITEM, STATION TO STATION, TON, REMARKS.



The Contractor shall cut the subgrade in accordance with this profile at all grade control points, i.e.; existing pavements, grade bridges and R.R. crossings, also at changes in thickness of base or surface courses.

TABLE OF DIMENSIONS table with 12 columns for D and L values ranging from 1" to 300'.

RATES OF APPLICATION table with 5 columns: RATE, UNIT, ITEM, and empty columns for additional data.

RECAPITULATION OF QUANTITIES table with 6 columns: ITEM, TOTAL, UNIT. Summarizes items like HMA Commercial Grade, Aggregate Base, etc.

- 8:1 Slope at the appropriate clear zone shall apply to all mound entrances and mound side roads to 10' fill height.
Normal Slope (but not steeper than 6:1) for over 10' fill height.

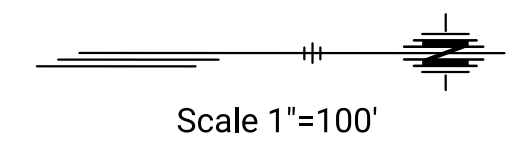
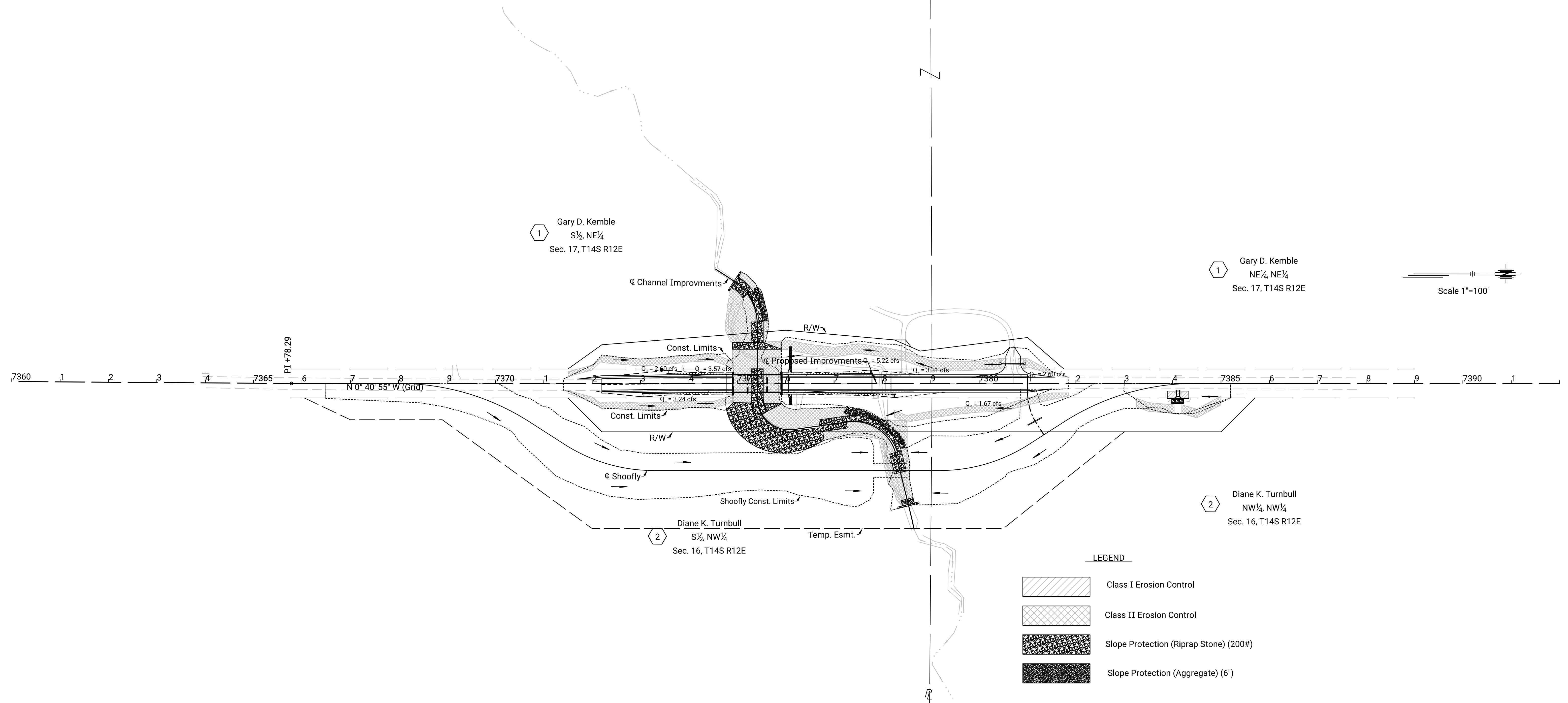
Summary of Quantities (Surfacing) block containing project details, revision table, and design approval information.

Plotted by: August Zuno
File: rrs051.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	65	135

BY	DATE
C. McCain	6/2023

REFERENCES NOTED	REFERENCES CHECKED



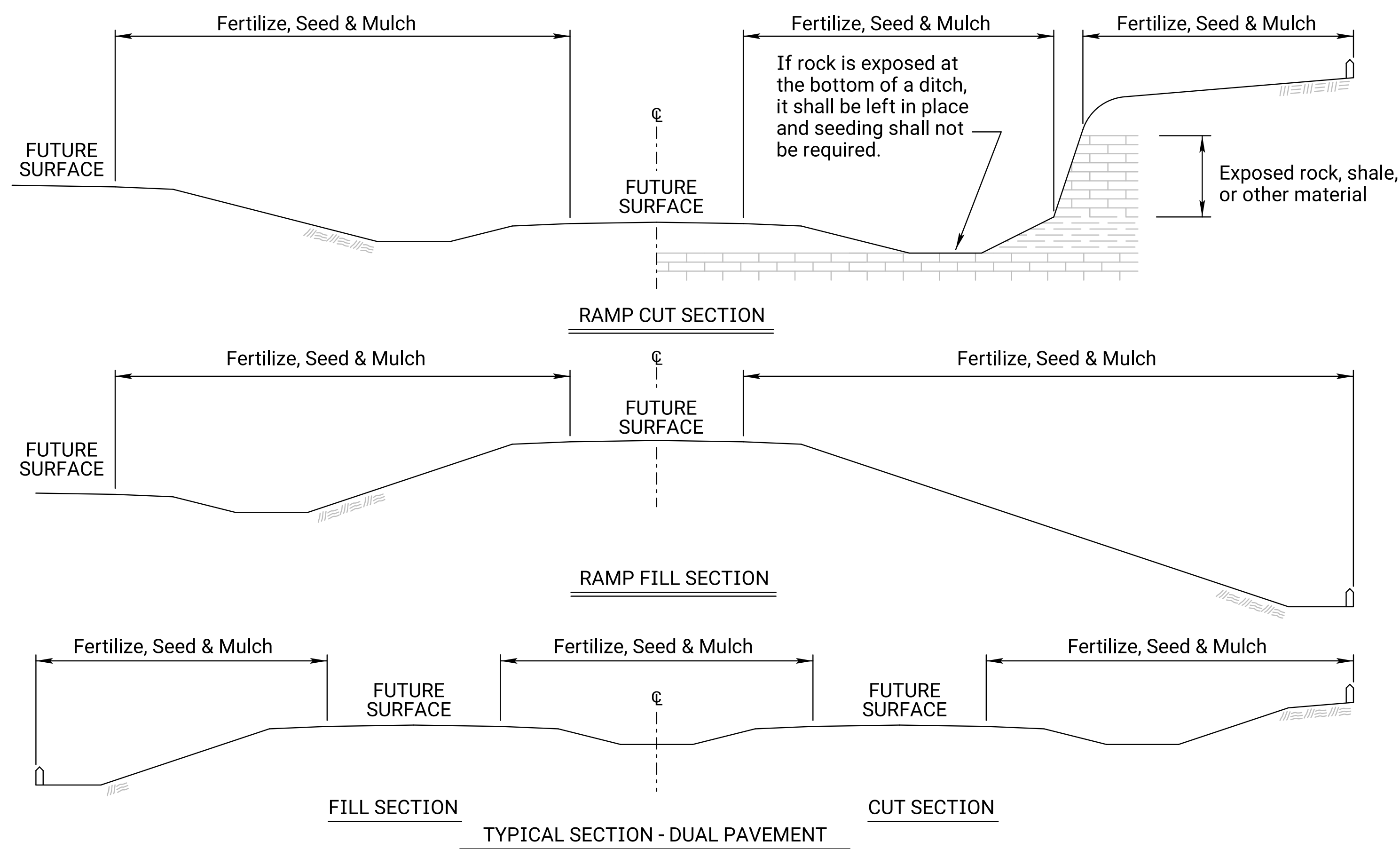
LEGEND

	Class I Erosion Control
	Class II Erosion Control
	Slope Protection (Riprap Stone) (200#)
	Slope Protection (Aggregate) (6")

Plotted by : August Zuno 08-JAN-2024 09:31
File : ka572801rec-01.dgn

KANSAS DEPARTMENT OF TRANSPORTATION
**PROPOSED FINAL
EROSION CONTROL PLAN**
STA. 7370+00 TO STA. 7388+00

KDOT Graphics Certified



SUMMARY OF SEEDING / EROSION CONTROL QUANTITIES						
P.L.S. RATE/ ACRE		ACRES		BID ITEM	QUANTITY	UNIT
CLT	SL/CH	CLT	SL/CH			
150		5.71		Temporary Fertilizer (15 - 30 - 15)	856.5	LB
20		3.81		Temporary Seed (Canada Wildrye)	76.2	LB
45		3.81		Temporary Seed (Grain Oats)	171.5	LB
45		3.81		Temporary Seed (Sterile Wheatgrass)	171.5	LB
	169.9		1.9	Soil Erosion Mix	322.8	LB
				Erosion Control (Class 1, Type C)	139	SQ YD
				Erosion Control (Class 2, Type E)	6510	SQ YD
				Erosion Control (Class 2, Type F)	2456	SQ YD
				Sediment Removal (Set Price)	1	CU YD
				Temporary Berm (Set Price)	1	LF
				Temporary Ditch Check (Rock)	965.6	CU YD
				Temporary Inlet Sediment Barrier		EACH
				Temporary Sediment Basin		CU YD
				Temporary Slope Drain		LF
				Temporary Stream Crossing		EACH
				Biodegradable Log (9")		LF
				Biodegradable Log (12")		LF
				Biodegradable Log (20")	590	LF
				Filter Sock (18")	530	LF
				Geotextile (Erosion Control)		SQ YD
				Silt Fence	180	LF
				SWPPP Design †	Lump Sum	LS
				SWPPP Inspection †	55	EACH
				Water Pollution Control Manager †	55	EACH
900 lbs / acre		3.81		Mulch Tacking Slurry	3429	LB
2 tons / acre		3.81		Mulching	11.4	TON
				Water (Erosion Control) (Set Price)	1	MGAL

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. See Permanent Seeding Summary of Seeding Quantities sheet LA850 for further details.

Geotextile (Erosion Control) shall be removed prior to placement of permanent slope protection.

Regreen and Quick Guard are the approved sterile wheatgrass products.

† If the total disturbed area of the project, not just the seeding area, is 1 acre or more, then these bid items must be included.

**** List size of material.

The amount of mulch and mulch tacking slurry in the bid quantities is estimated. (Acres of Seeding X 1.5 X 2 Tons/Acre). The estimated quantity includes mulching associated with both temporary and permanent seeding operations. The total mulch and mulch tacking slurry required shall be determined in the field. The bid item for mulching and mulch tacking slurry shall be paid for according to the Standard Specifications.

Quantities for all erosion control items are estimated to give full flexibility for compliance with the NPDES permit. Final quantities will be determined in the field.

SOIL EROSION MIX		
PLS RATE	NAME	QTY (lb)
20	Canada Wildrye	38.0
20	Grain Oats	38.0
20	Sterile Wheatgrass	38.0
0.5	Blue Grama (Lovington)	1.0
4.5	Buffalograss (Treated)	8.6
45	Perennial Ryegrass	85.5
2.6	Prairie Junegrass	4.9
6.3	Side Oats Grama (El Reno)	12.0
45	Tall Fescue (Endophyte Free)	85.5
6	Western Wheat (Barton)	11.4
169.9	Total (lb)	322.8

Fertilizer for Soil Erosion Mix is included and shown on the Summary of Seeding / Erosion Control chart above.

The Soil Erosion Mix is to be placed under the Class 1 and/or Class 2 erosion control material.

The Soil Erosion Mix consists of the Shoulder Area of the Permanent Seed Mix used on the project.

FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P₂O₅, K₂O listed in Summary of Quantities will be acceptable.

- * - N = Nitrogen Rate of Application
- ** - P₂O₅ = Phosphorous Rate of Application
- *** - K₂O = Potassium Rate of Application

The Contractor will be required to finish areas of excavation, borrow and embankment in accordance with the specifications. Areas that require installation or construction of temporary water pollution control items will be finished in reasonable close conformity to the alignment, grade and cross section shown on the plans or as established by the Engineer.

CLT = Construction Limit Tract. This area is defined by the entire disturbed area of the project that requires seeding and erosion control measures to be placed. Any impervious areas (i.e. pavement, gravel, riprap, etc.) shall not be included in this measurement.

Slope = Defined by the area of the project that requires Class 1 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

Channel = Defined by the area of the project that requires Class 2 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

GENERAL NOTES

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded, and mulched. Soil preparation shall conform to the Standard Specifications.

Temporary seeding shall be done during any time of the year that the soil can be cultivated. After the temporary seeding has been completed on the entire project, permanent seeding shall be done during the normal seeding season.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching materials is generally as follows:

1¼ - 2¼ Tons per Acre = 1½" loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

NO.	DATE	REVISIONS	BY	APPD
03	08-03-20	Added Note	M.R.D.	M.L.
02	12-01-17	Revised Standard	M.R.D.	S.H.S.
01	06-01-17	Revised Standard	M.R.D.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY EROSION AND POLLUTION CONTROL

LA852A

DESIGNED	M.R.D.	DETAILLED	M.R.D.	QUANTITIES	TRACED
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.	TRACE CK.

Scott H. Shields

DOT Graphics Certified 07-10-2023 Sh. No. 66

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	67	135

Class 1, Type C

Type	SIDE	SLOPE H:V	ALIGN-MENT	STATION TO STATION		LENGTH (FT)	AVG. WIDTH SLOPE (FT)	EROSION CONTROL CLASS I (SQUARE YARDS)	
								TYPE C	TYPE D
C	LT	4:1	K-99	7380+46.00	7380+55.00	9.00	40.00	40	
C	LT	4:1	K-99	7380+85.00	7380+92.00	7.00	20.00	16	
C	RT	4:1	K-99	7383+85.00	7384+35.00	50.00	15.00	83	
TOTAL (ALL SHEETS)								139	0

Class 2, Type E and F

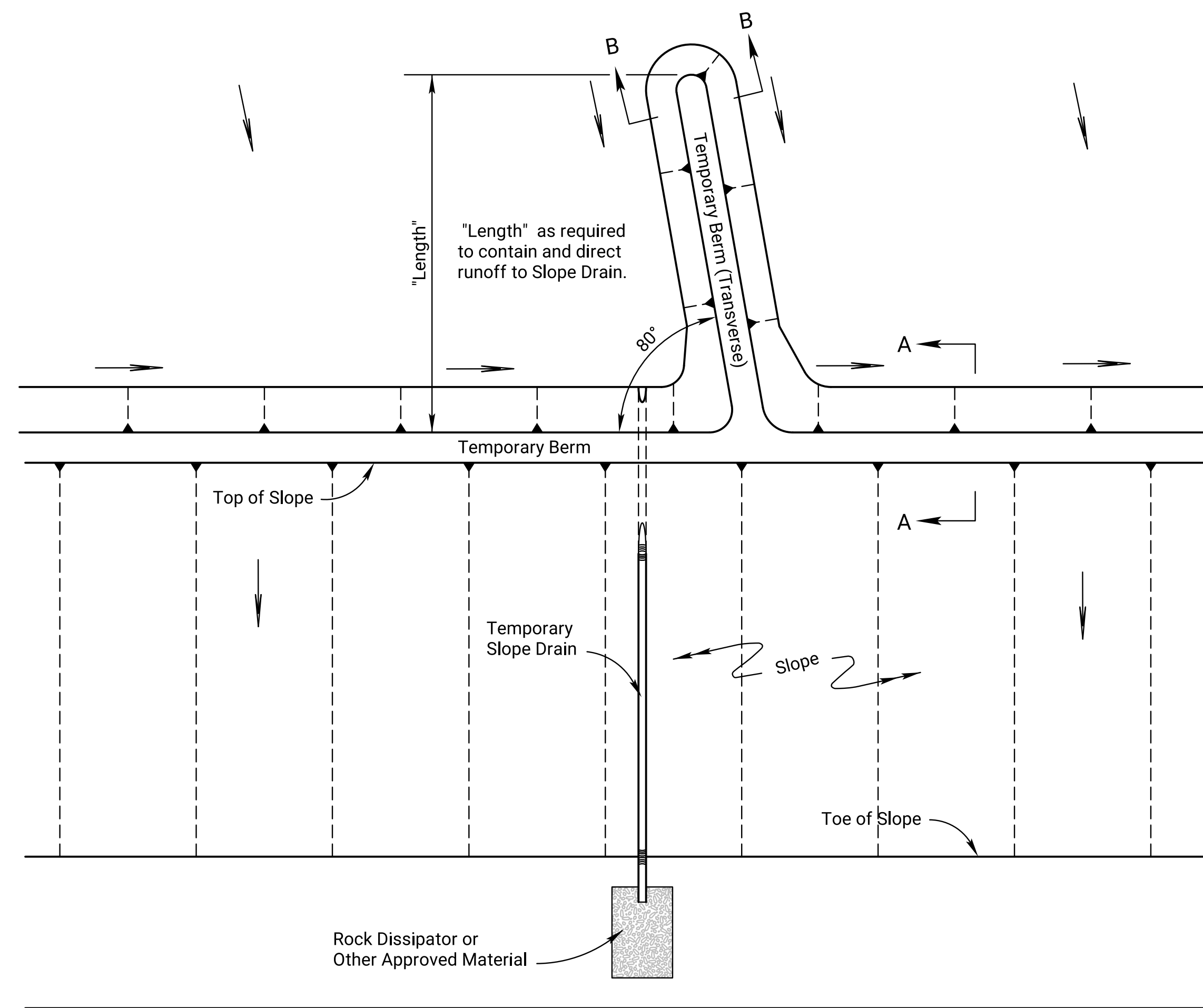
Type	SIDE	ALIGN-MENT	STATION TO STATION		LENGTH (FT)	AVG. WIDTH (FT)	EROSION CONTROL CLASS II (SQUARE YARDS)			
							TYPE E	TYPE F	TYPE G	TYPE H
E	LT	K-99	7371+50.00	7372+20.00	70.00	10.00	78			
E	RT	K-99	7371+50.00	7372+20.00	70.00	10.00	78			
E	LT	K-99	7372+20.00	7374+90.00	270.00	14.00	420			
E	RT	K-99	7372+20.00	7374+90.00	270.00	14.00	420			
E	LT	K-99	7375+90.00	7380+46.00	456.00	20.00	1013			
E	RT	K-99	7378+25.00	7381+00.00	275.00	14.00	428			
E	LT	K-99	7380+92.00	7381+50.00	58.00	14.00	90			
E	RT	K-99	7381+00.00	7381+85.00	85.00	14.00	132			
E	RT	K-99	7383+25.00	7384+03.00	78.00	18.00	156			
E	RT	K-99	7384+19.00	7385+00.00	81.00	18.00	162			
E	RT	Shoofly	67+00.00	77+80.00	1080.00	12.00	1440			
E	LT	Shoofly	70+00.00	77+75.00	775.00	12.00	1033			
E	LT	Shoofly	78+50.00	83+00.00	450.00	12.00	600			
E	RT	Shoofly	79+70.00	82+05.00	235.00	12.00	313			
E	RT	Shoofly	83+50.00	84+60.00	110.00	12.00	147			
F	LT	Channel	10+73.50	11+12.64	39.14	15.01		65		
F	LT	Channel	11+78.64	12+75.87	97.23	25.42		275		
F	RT	Channel	10+73.50	12+30.83	157.33	37.80		661		
F	LT	Channel	13+67.83	15+54.40	186.57	28.26		586		
F	RT	Channel	14+98.43	17+98.28	299.85	18.09		603		
F	LT	Channel	16+04.42	17+98.28	193.86	12.33		266		
TOTAL (ALL SHEETS)							6510	2456	0	0

Plotted by : Melissa.Davidson@ks.gov 10-JUL-2023 18:07
File : LA852A-EC.dgn

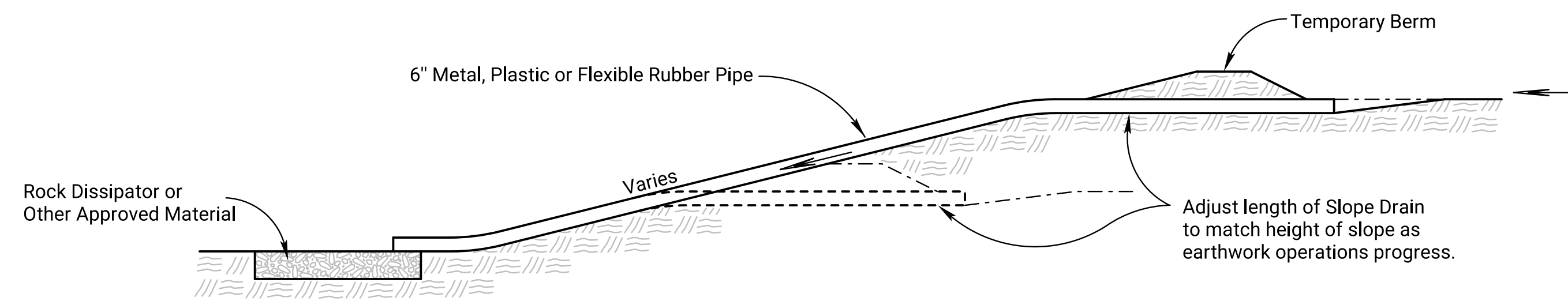
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
EROSION CONTROL SEEDING-SODDING				
LA852A-EC				
DESIGNED M.R.M.		APPD. Scott H. Shields		
DETAIL CK. S.H.S.		QUANTITIES		TRACED M.R.M.
DESIGN CK. S.H.S.		TRACE CK. S.H.S.		
KDOT Graphics Certified 07-10-2023 Sh. No. 67				

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	68	135

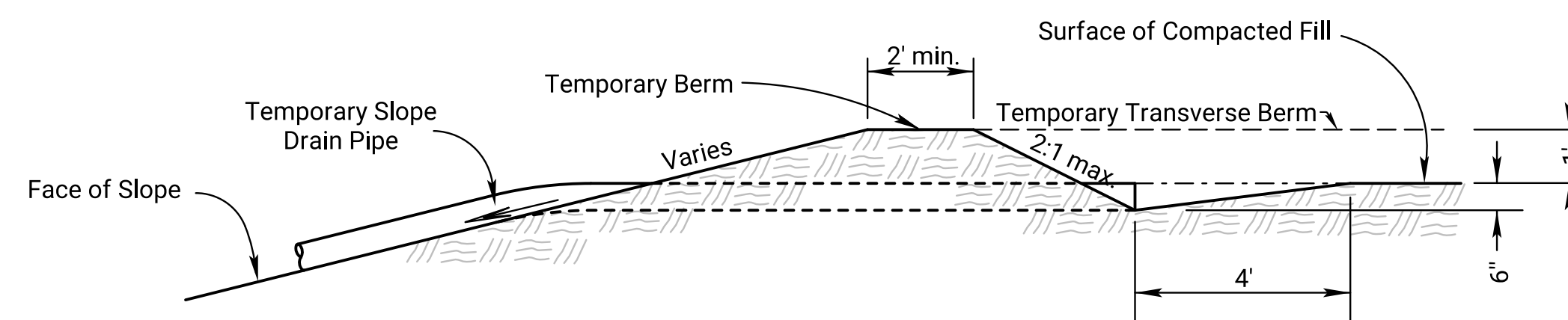


TYPICAL PLAN VIEW OF TEMPORARY BERM AND TEMPORARY SLOPE DRAIN
NO SCALE

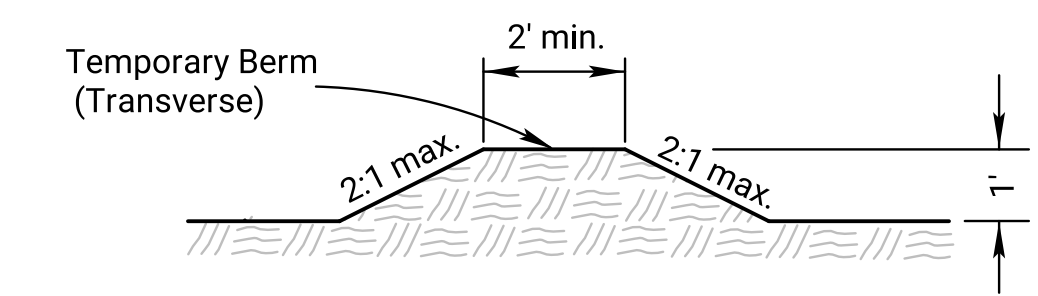


TYPICAL PROFILE OF TEMPORARY SLOPE DRAIN
NO SCALE

- NOTES:
- 1) Temporary Slope Drain and Temporary Berm may be used on either project foreslopes or project backslopes.
 - 2) Discharge of Slope Drains shall be into stabilized ditch or area, or into Sediment Basin.
 - 3) Pipe shall be secured in place as approved by Engineer.
 - 4) Temporary Berms under 2,000 feet shall be bid by Set Price.

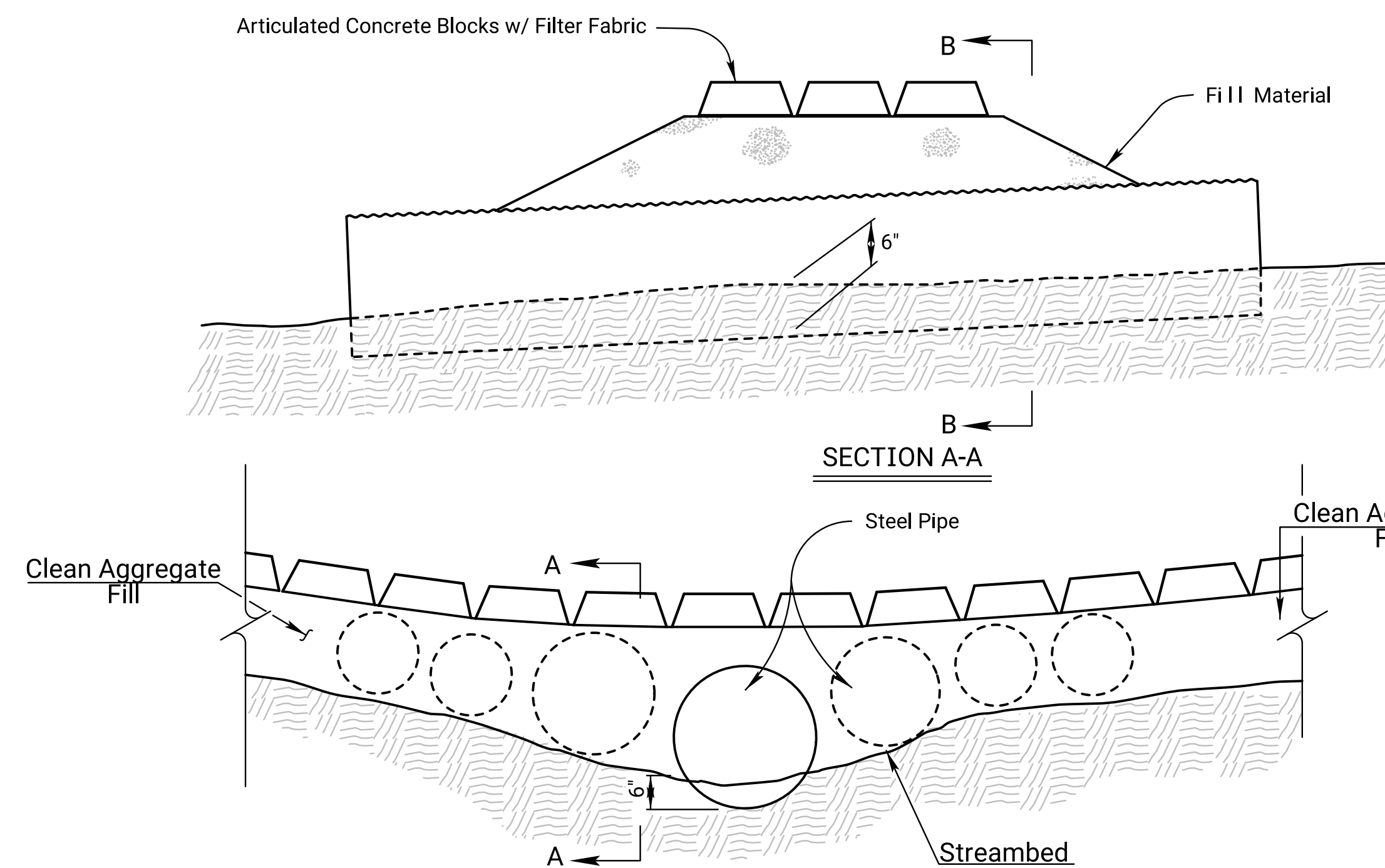


SECTION A-A
NO SCALE



SECTION B-B
NO SCALE

TYPICAL PROFILE OF TEMPORARY BERM
NO SCALE

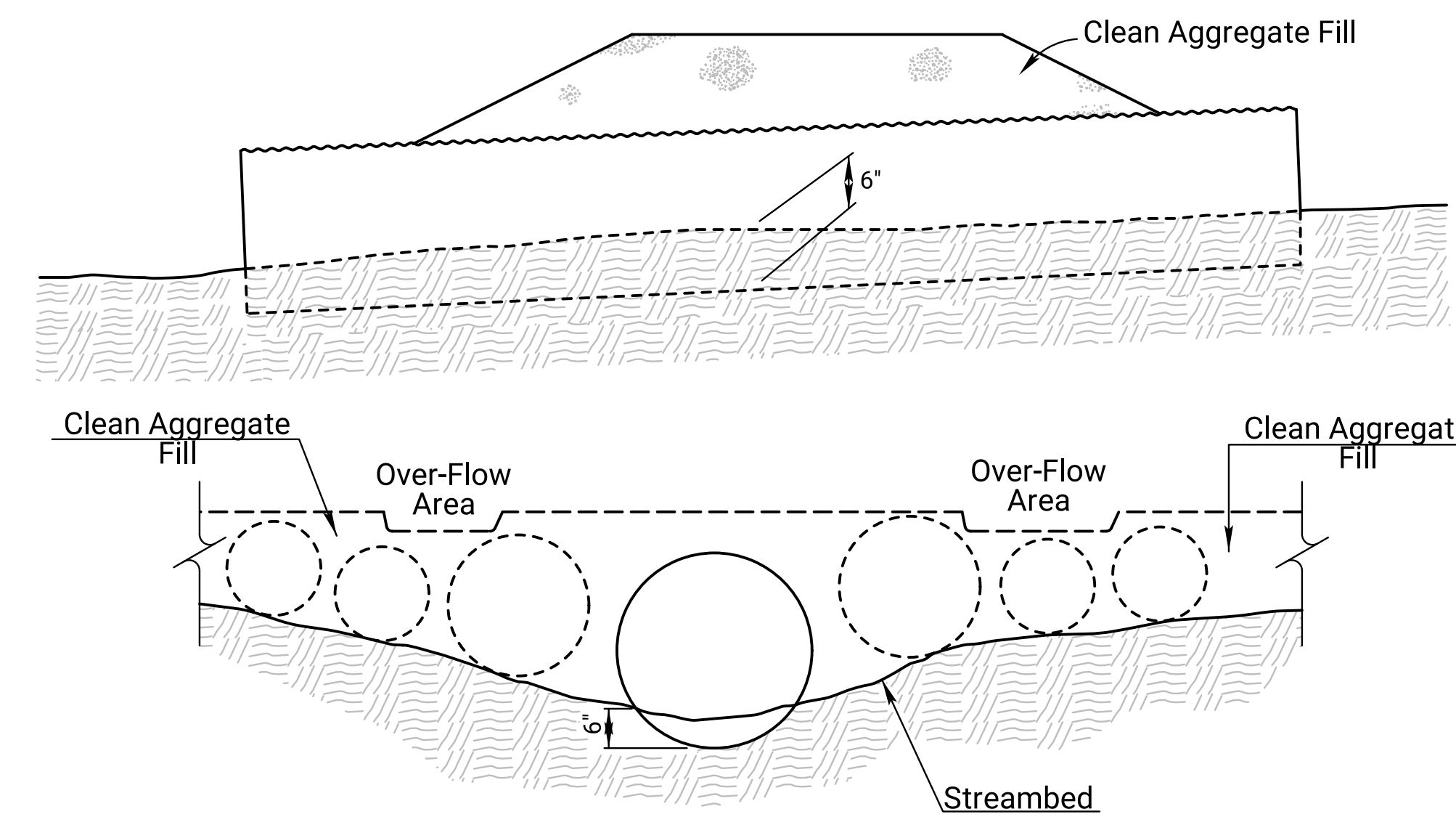


TEMPORARY STREAM CROSSING (ARTICULATED CONCRETE BLOCKS)
NO SCALE

Pipe size may vary.
Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

Clean aggregate fill will extend a minimum of 50' on the entrance and exit side of the crossing to prevent tracking. The aggregate shall be clean aggregate and a minimum of 6" thick and will be maintained through the use of the crossing.

See KDOT Specifications for more information.



TEMPORARY STREAM CROSSING (AGGREGATE)
NO SCALE

Pipe size may vary.
Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

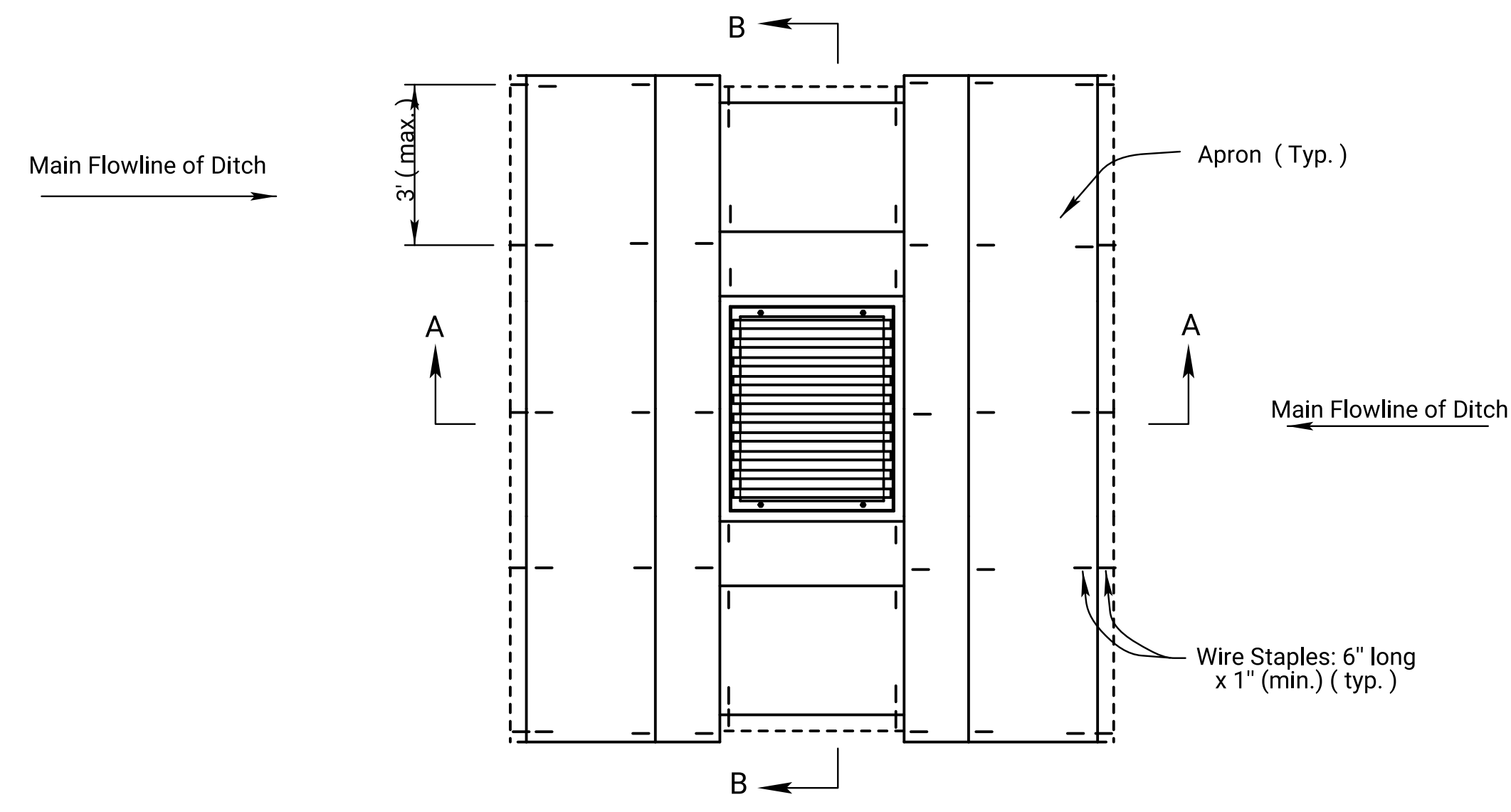
Clean aggregate fill will extend a minimum of 50' on the entrance and exit side of the crossing to prevent tracking. The aggregate shall be clean aggregate and a minimum of 6" thick and will be maintained through the use of the crossing.

See KDOT Specifications for more information.

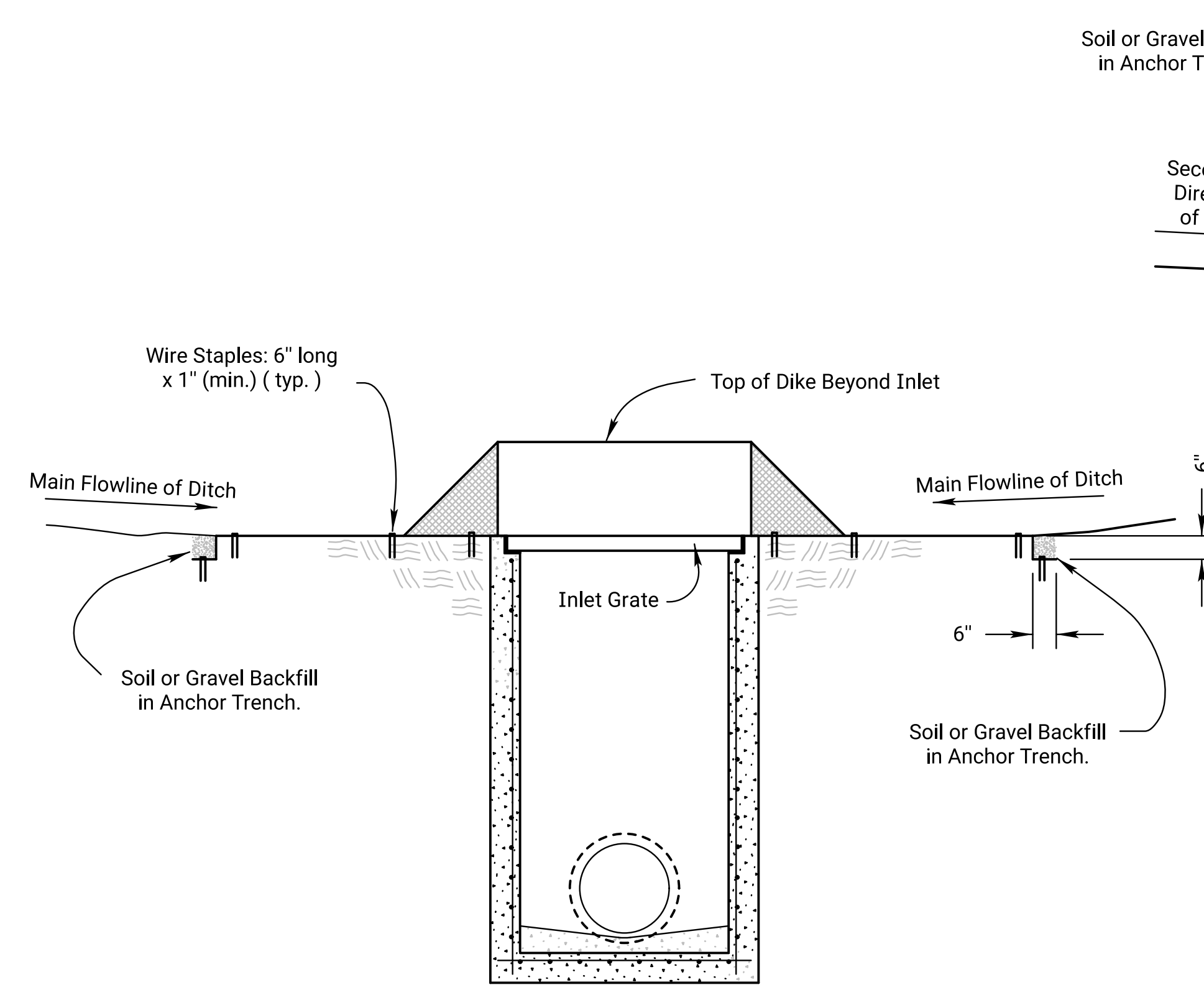
NO.	DATE	REVISIONS	BY	APPD
03	01-21-22	Temp Stream Crossing - Clean Aggregate Fill Note Added	M.R.D.	M.L.
02	08-24-21	Temp Stream Crossing - Clean Aggregate Fill Note Added	M.R.D.	M.L.
01	06-11-13	Revised Standard	M.R.M.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION			
TEMPORARY EROSION AND POLLUTION CONTROL			
TEMPORARY SLOPE DRAIN, TEMPORARY STREAM CROSSING (AGGREGATE)			
LA852B			
FHWA APPROVAL	01-21-22	APPD.	Mervin Lare
DESIGNED	DETAILD	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

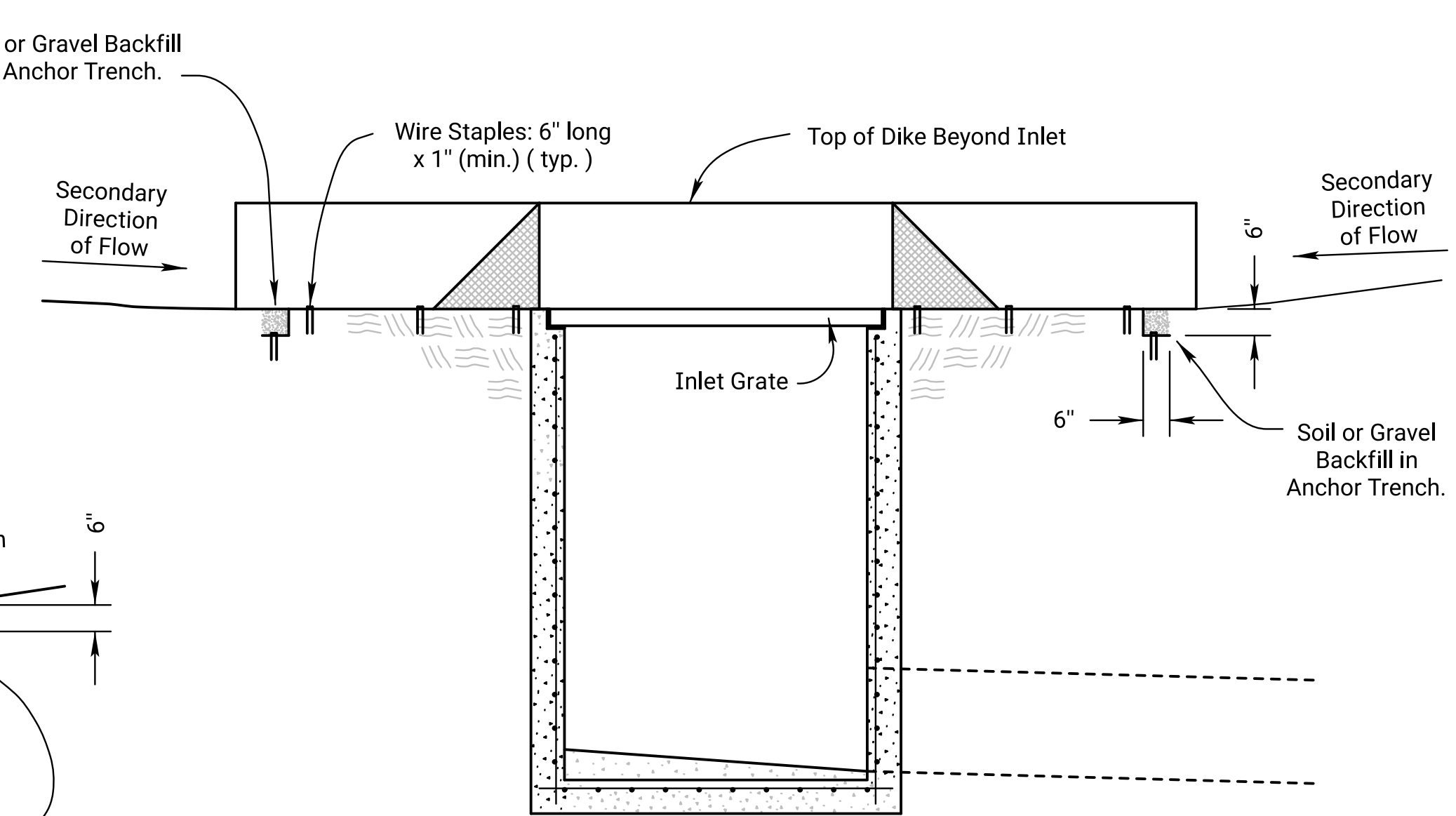
Plotted by: Melissa.Davidson@ks.gov 10-JUL-2023 18:07
File: LA852B.dgn



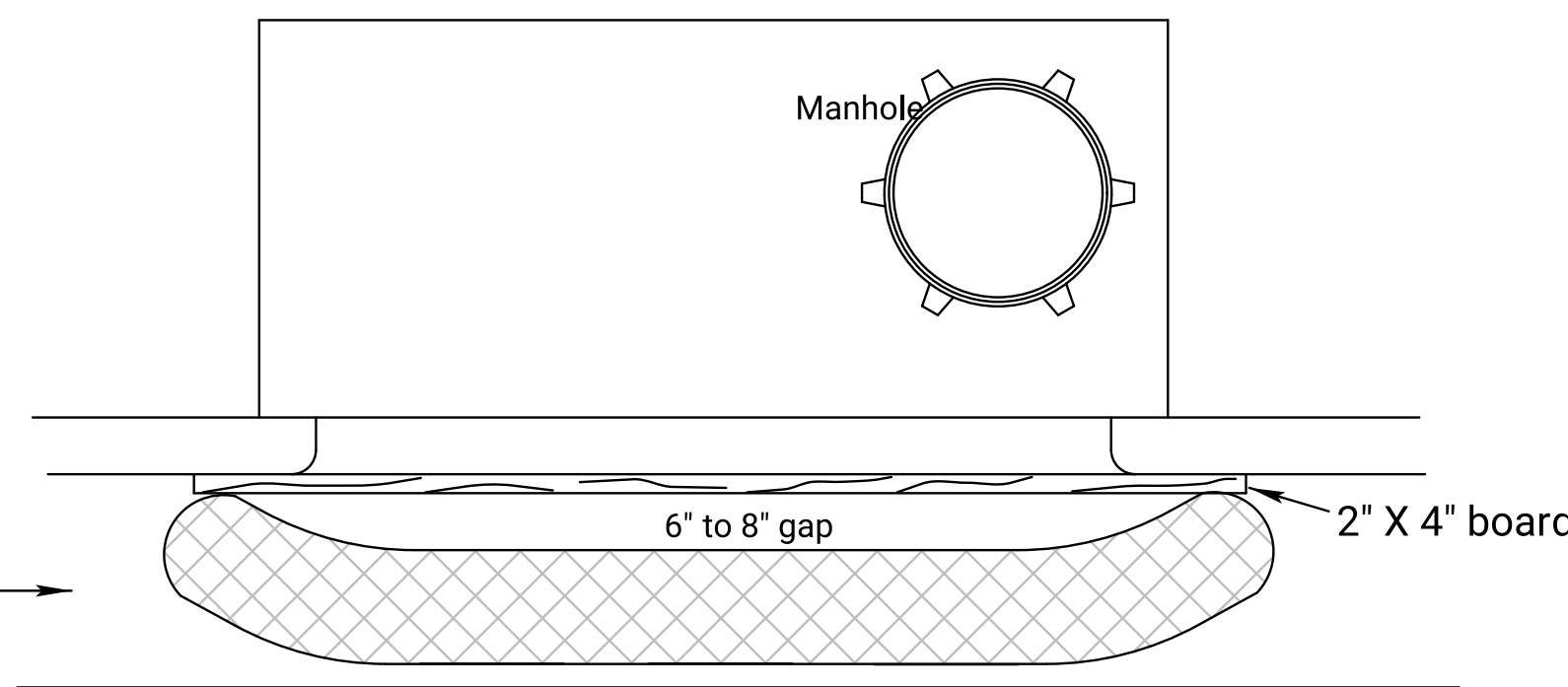
PLAN
TEMPORARY INLET SEDIMENT BARRIER
(TRIANGULAR SILT DIKE METHOD)
 NO SCALE



SECTION A - A

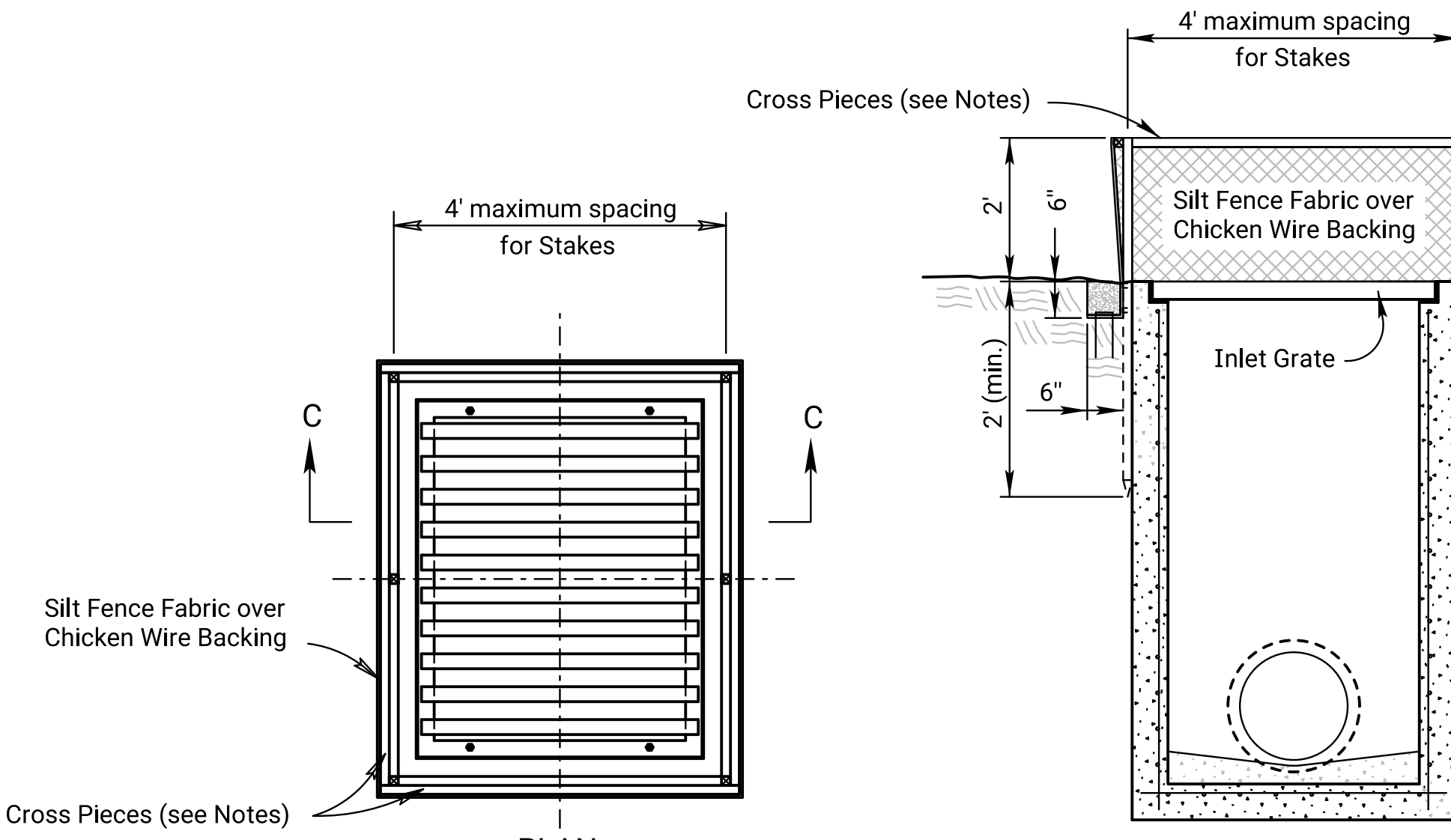


SECTION B - B



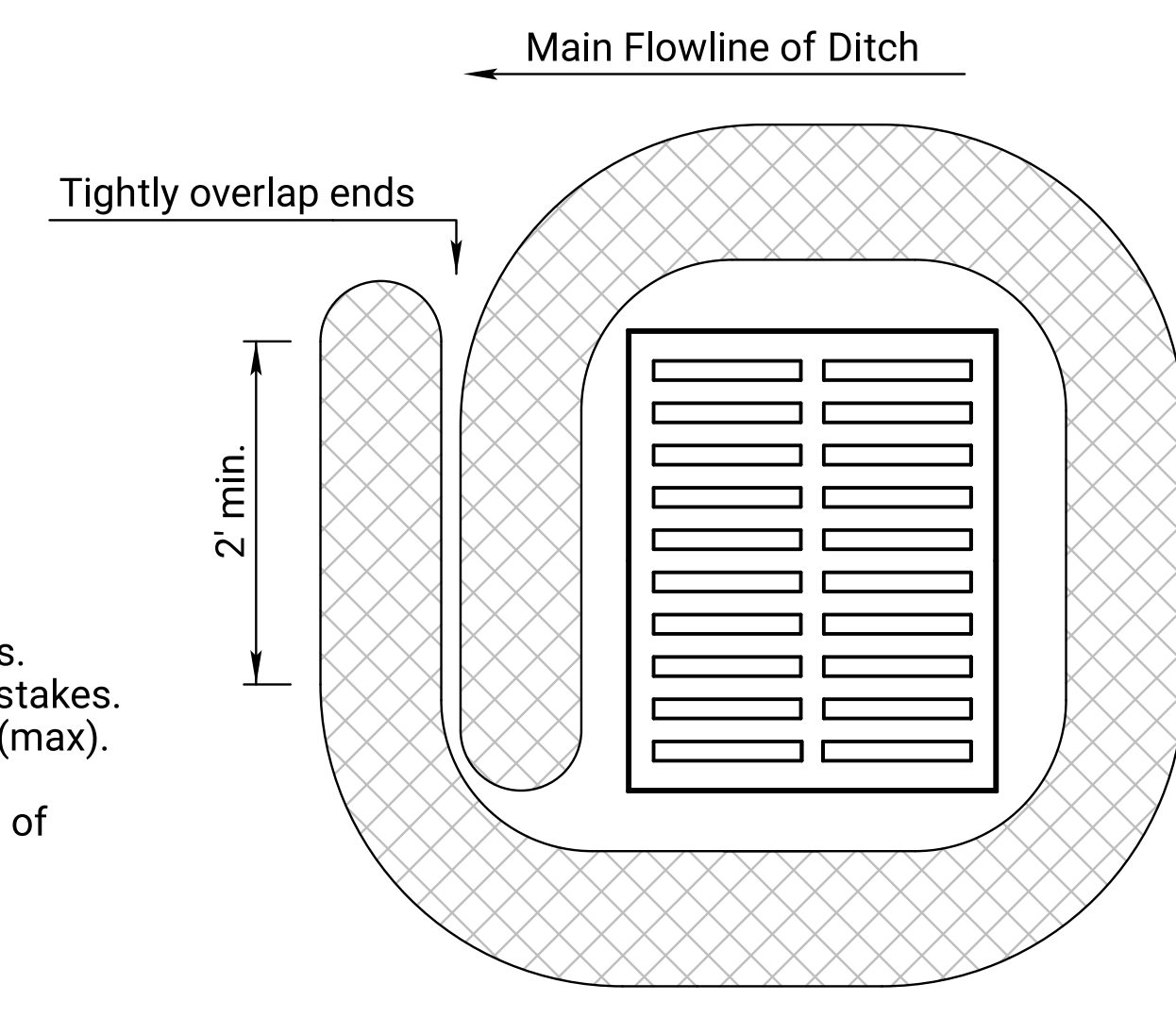
CURB INLET PROTECTION

1. If multiple gravel bags are required, place them in such a way that no gaps are evident.
2. Height of bags (8" minimum diameter) must not be above top of curb.
3. Alternative products may be used other than gravel bags such as the "Gutter Buddy". Products must be approved by the Engineer.
4. Curb inlet protection will be measured and paid for as Filter Sock.



PLAN
TEMPORARY INLET SEDIMENT BARRIER
(SILT FENCE METHOD)
 NO SCALE

- SILT FENCE:**
1. Stakes shall be 4' (min.) long and of one of the following materials:
 - a. Hardwood - 1 3/4" x 1 3/4";
 - b. Southern Pine (No. 2) - 2 5/8" x 2 5/8";
 - c. Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
 - d. Synthetic - same strength as wood stakes.
 2. Cross pieces shall be of same material as stakes.
 3. Attach fence fabric securely on 6" centers (max).
 4. Use of high flow material is acceptable.
 5. Refer to plan sheets to estimate the length of silt fence required.



Drop inlet use
1'-6" TO 1'-8" diameter log
BIODEGRADABLE LOG/FILTER SOCK
DROP INLET PROTECTION

Note: 25% of log shall be keyed into ground during installation.
 Stake every 4'

Material Requirements	
Use 100% shredded mulch or other non-compost biodegradable material as fill for logs.	
No compost or fines.	
No hay or straw.	
Do not use material which prohibits water infiltration.	
Log Mesh:	
Use mesh with 1/4" openings or larger. Mesh must allow water infiltration but also hold fill material in place.	

NO.	DATE	REVISIONS	BY	APPD.
03	09-26-19	Changed Direction of Main Flowline of Ditch Arrow	M.R.D.	S.H.S.
02	03-10-15	Revised Standard	R.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION
TEMPORARY EROSION AND POLLUTION CONTROL, TEMPORARY INLET SEDIMENT BARRIER (SILT FENCE)
TEMP. INLET SEDIMENT BARRIER (T.S.D.)
LA852C

DESIGNED	R.A.	DETAILED	R.A.	QUANTITIES	TRACED
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.	TRACE CK.

Scott H. Shields

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	70	135

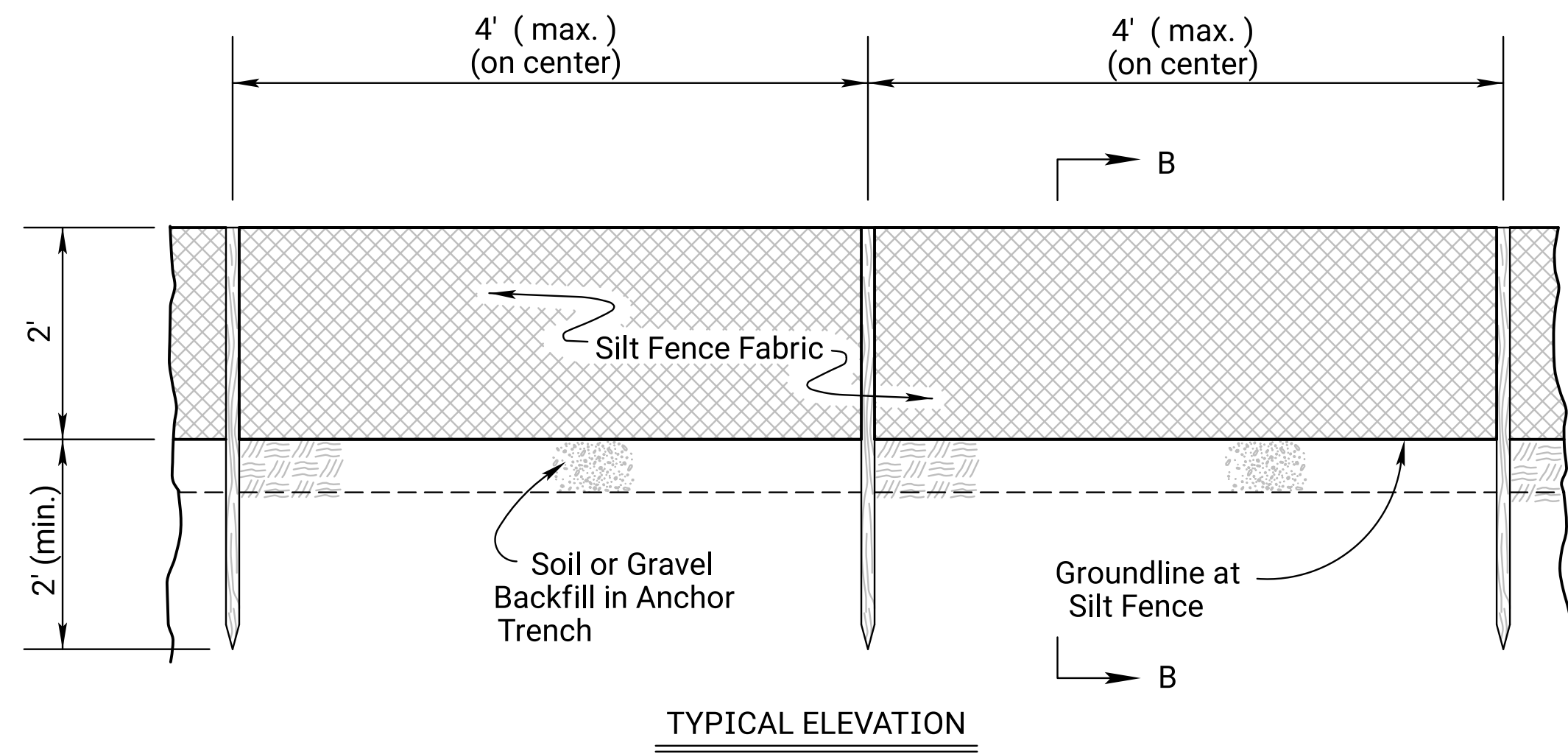
INSTALLATION NOTES

SILT FENCE:

- Stakes shall be 4' (min.) long and of one of the following materials:
 - Hardwood - 1 3/16" x 1 3/16";
 - Southern Pine (No. 2) - 2 5/8" x 2 5/8";
 - Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
 - Synthetic - same strength as wood stakes.
- Attach fence fabric with 3 zip ties within the top 8" of the fence. Alternate attachment methods may be approved by the Engineer on a performance basis.
- Use of high flow material is acceptable.
- Refer to plan sheets to estimate the length of silt fence required.

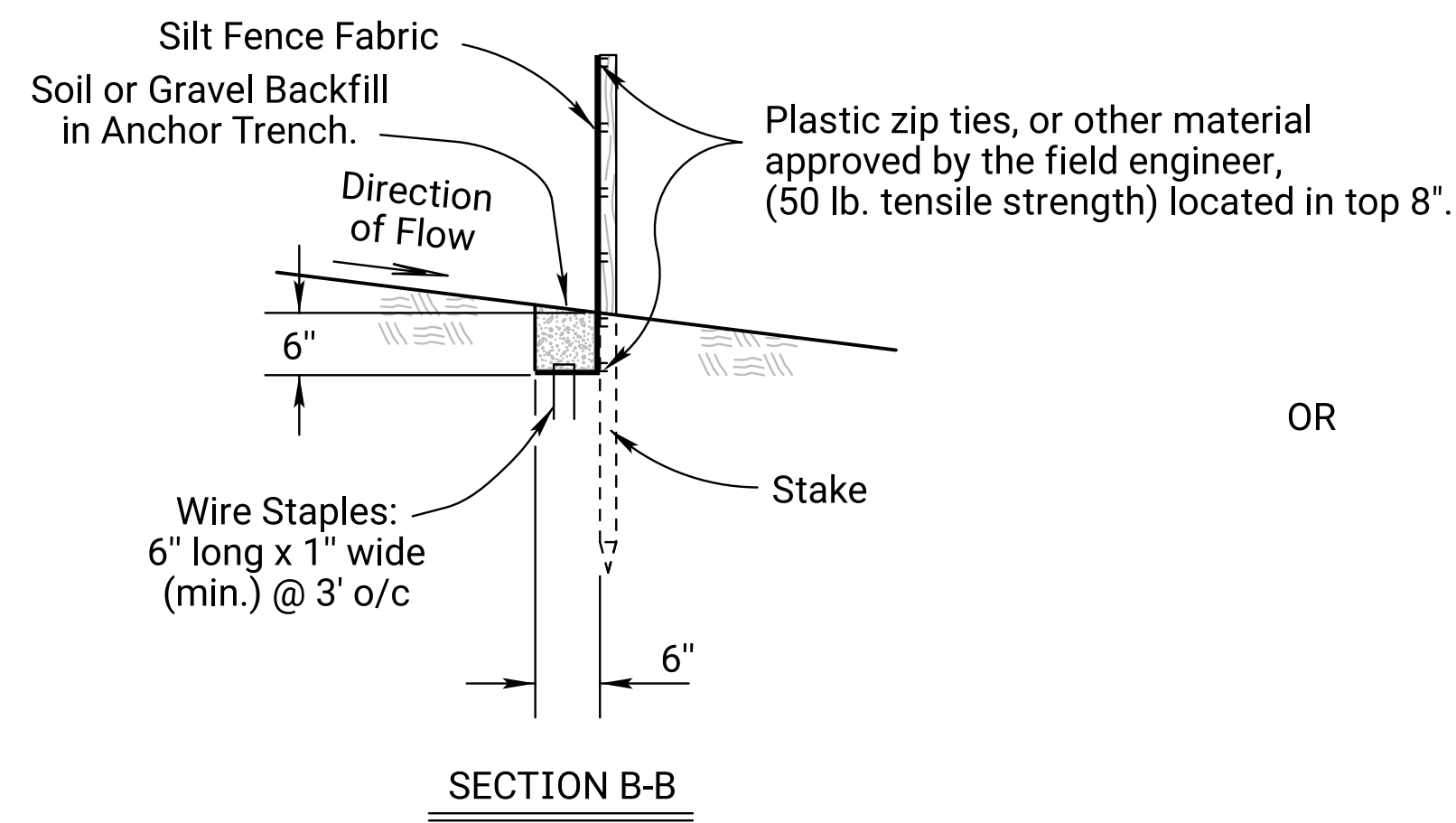
BIODEGRADABLE LOG OR FILTER SOCK

- Place biodegradable logs or filter sock tightly together minimum overlap of 18".
- Wood stakes shall be 2" x 2" (nom.).
- Refer to plan sheets to estimate length of biodegradable log and filter sock required.
- Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.
- Length of stakes should be 2 times the height of the log at a minimum with minimum ground embedment equal to the height of the log / sock.

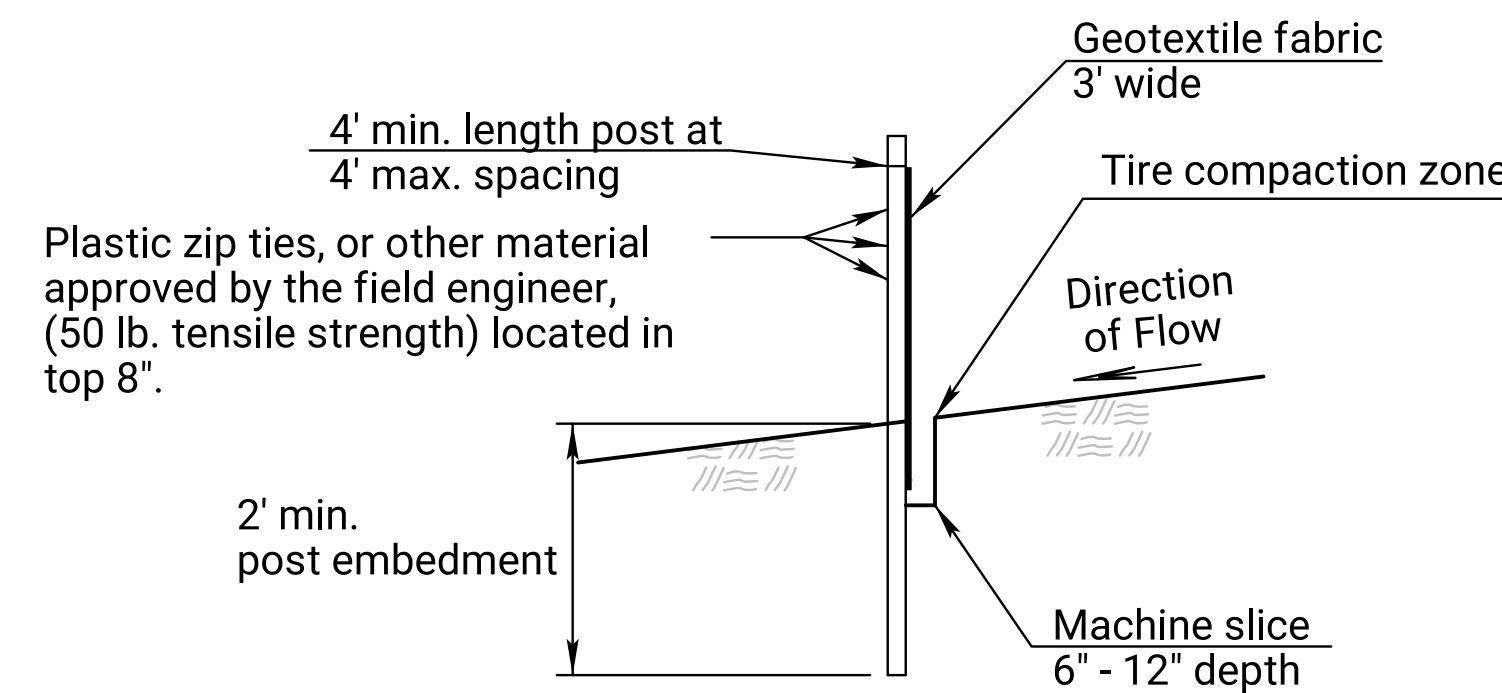


TYPICAL ELEVATION

SILT FENCE BARRIER
NO SCALE



SECTION B-B



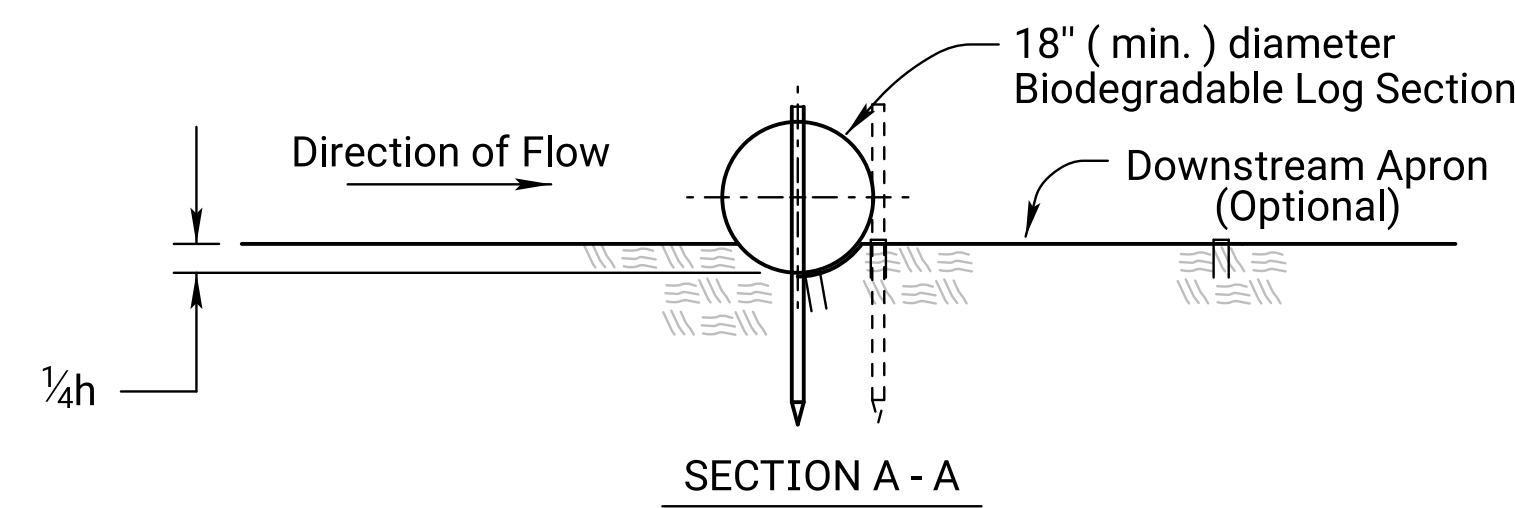
SECTION B-B

Biodegradable Log or Filter Sock Slope Interruptions

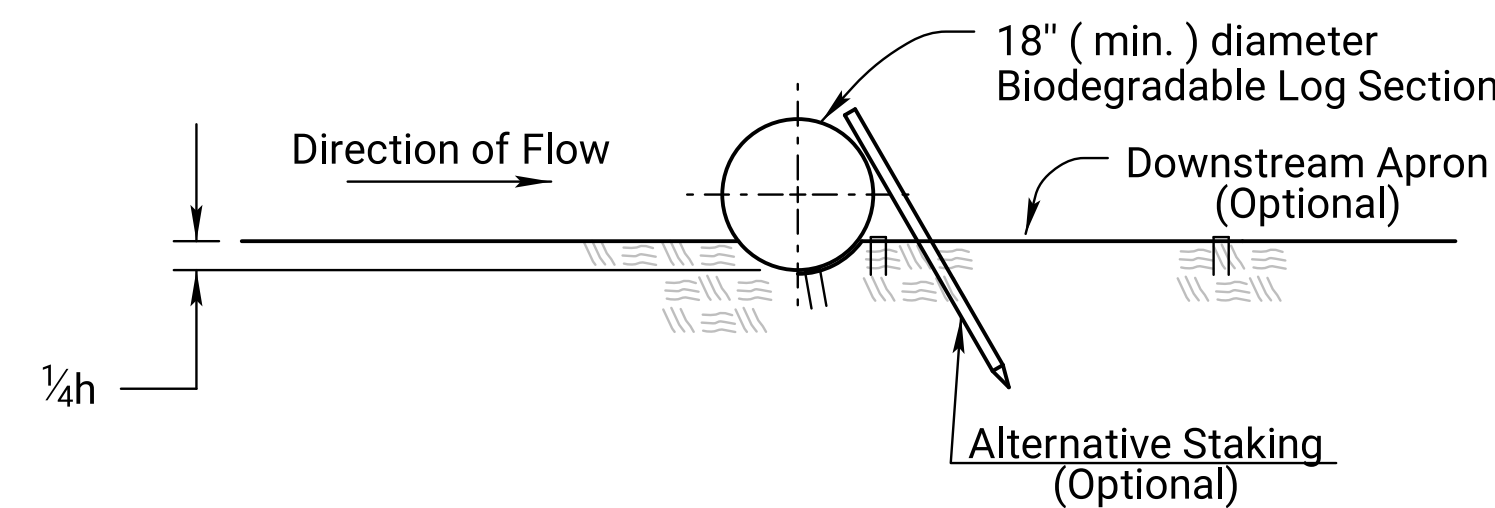
		PRODUCT		
		9" Sediment Log or 8" Filter Sock (ft)	12" Sediment Log or 12" Filter Sock (ft)	20" Sediment Log or 18" Filter Sock (ft)
Slope Gradient	≤4H:1V	40	60	80
	3H:1V	30	45	60

BIODEGRADABLE LOG MATERIAL		
	LOW FLOW	HIGH FLOW
9"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
12"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
18"-20"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber

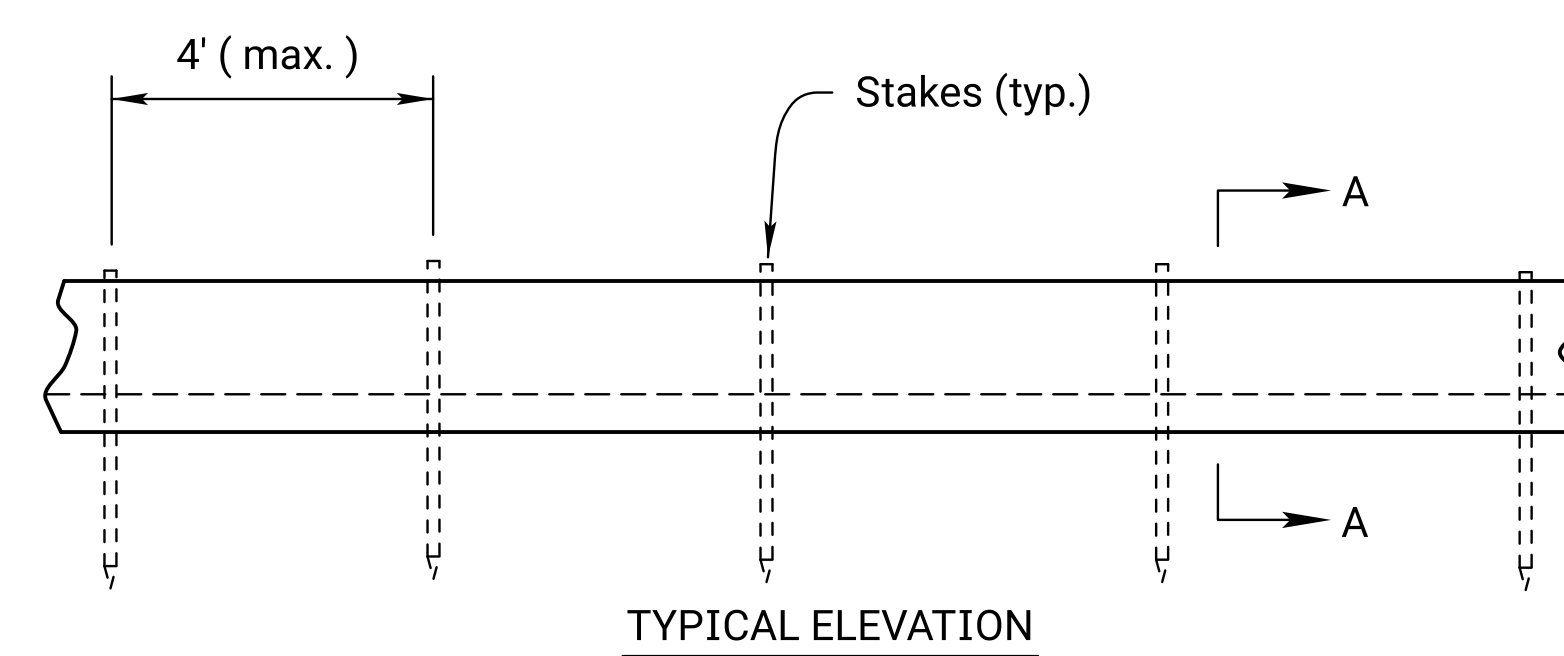
Deviations should be approved by the Field Engineer.



SECTION A - A



ALT. DETAIL
OPTIONAL



TYPICAL ELEVATION

BIODEGRADABLE LOG SLOPE INTERRUPTIONS
OR Filter Sock

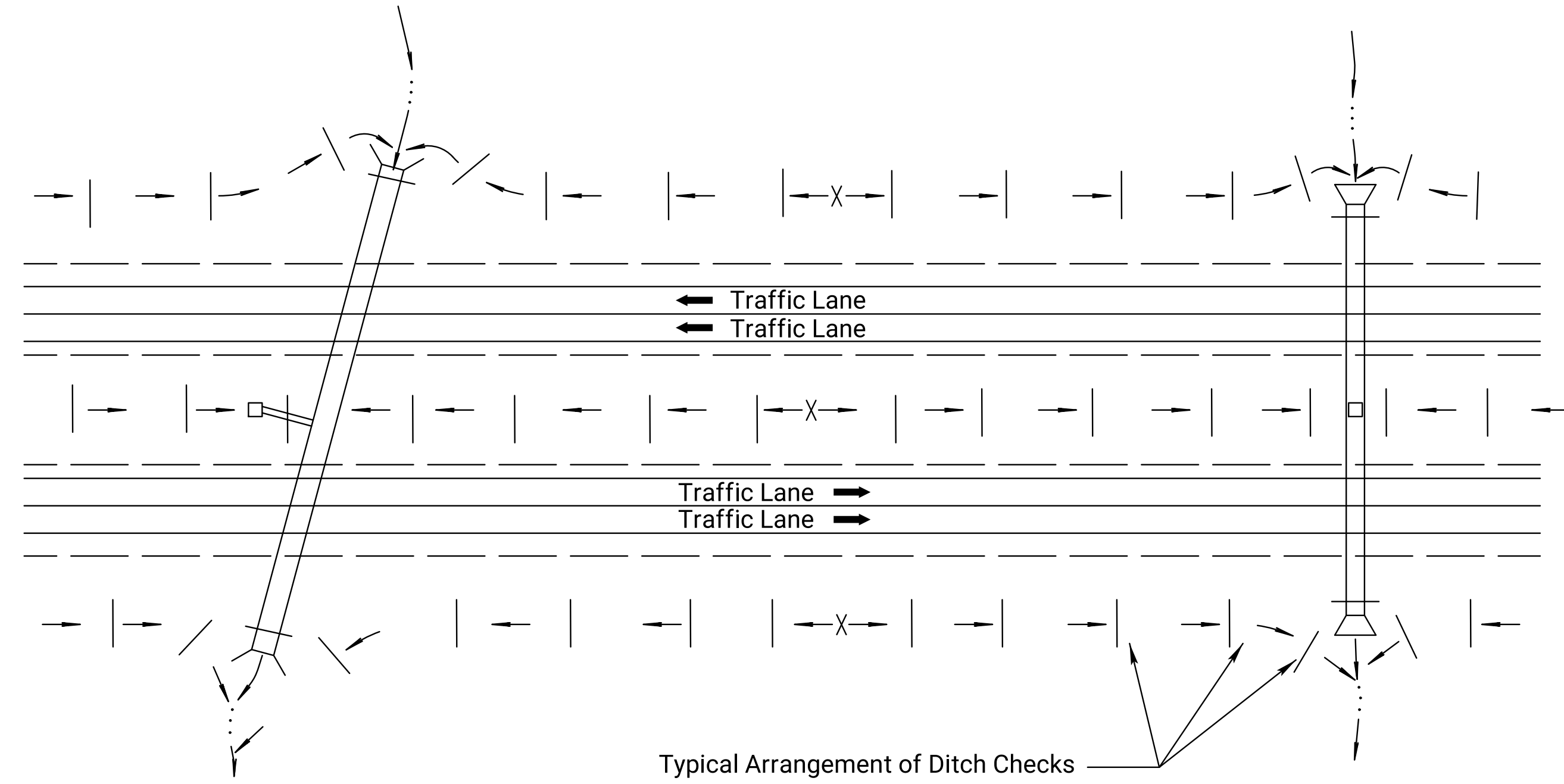
GENERAL NOTES

- Slope interruptions shall be placed along contour lines, with a short section turned upgrade at each end of the barrier.
- The maximum length of the slope interruptions shall not exceed 250 feet, and the barrier ends need to be staggered.
- Interruptions damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired immediately by Contractor at no additional cost to KDOT.
- Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

03	06-28-16	Revised Standard	R.A.	S.H.S.
02	03-01-15	Revised Standard	R.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL SLOPE INTERRUPTIONS BIODEGRADABLE LOG / SILT FENCE				
LA852D				
DESIGNED	S.H.S.	APPD.	Scott H. Shields	
DETAIL CK.	S.H.S.	QUANTITIES	TRACED	
DESIGN CK.	S.H.S.	QUAN. CK.	TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	71	135



TYPICAL DITCH CHECK LAYOUT PLAN
NO SCALE

20" BIOLOG CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
1.0	125
2.0	60
3.0	40
4.0	30
5.0	25

NOTE: Use this spacing for all except Rock Ditch Checks.

18" FILTER SOCK CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
1.0	110
2.0	55
3.0	35
4.0	25
5.0	20

NOTE: Use this spacing for all except Rock Ditch Checks.

GENERAL NOTES

- 1) The choice of ditch check methods is at the option of the Contractor.
- 2) Use only rock checks in situations where the ditch slope is 6 percent or greater.
- 2) Ditch checks damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired by Contractor at no extra cost to KDOT.

Plotted by : Melissa.Davidson@ks.gov 10-JUL-2023 18:08
File : la852e.dgn

NO.	DATE	REVISIONS	BY	APPD
03	08-10-16	Revised Standard	R.A.A.	S.H.S.
02	06-28-16	Revised Standard	R.A.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.

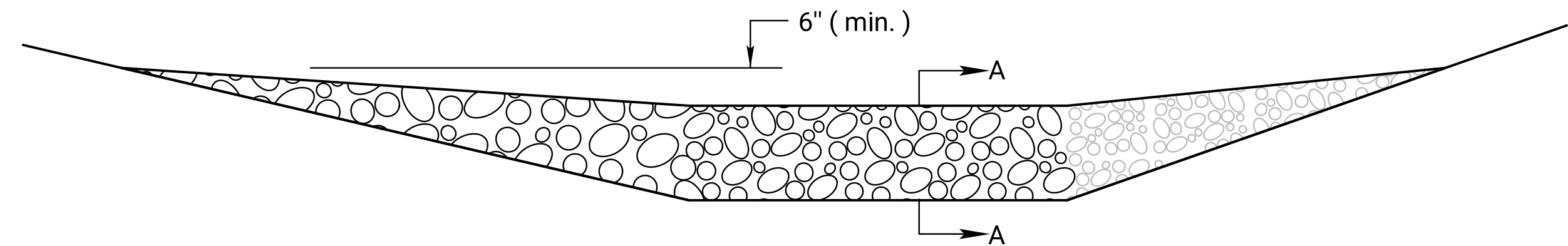
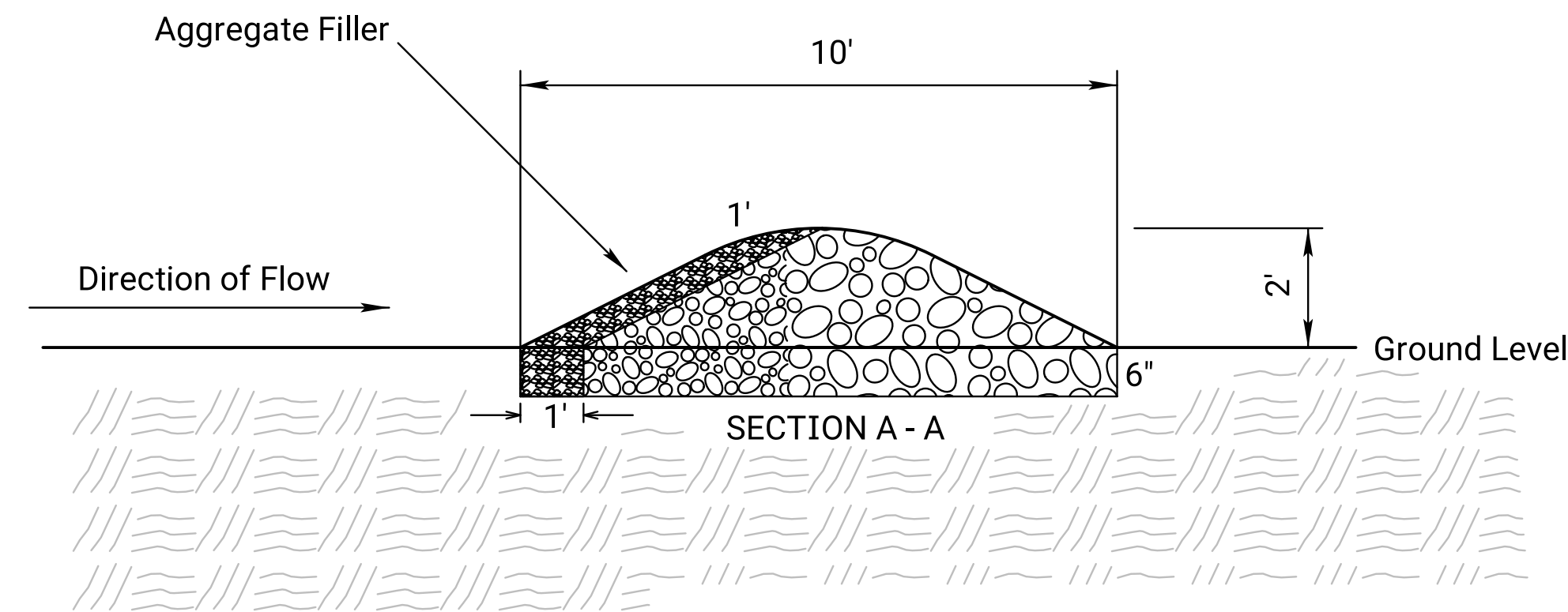
KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY EROSION AND POLLUTION CONTROL DITCH CHECKS

LA852E

DESIGNED	S.H.S.	DETAILED	R.A.A.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN.CK.	TRACE CK.	S.H.S.

FHWA APPROVAL 09-14-16 | APPD. Scott H. Shields



TYPICAL ELEVATION

ROCK DITCH CHECK

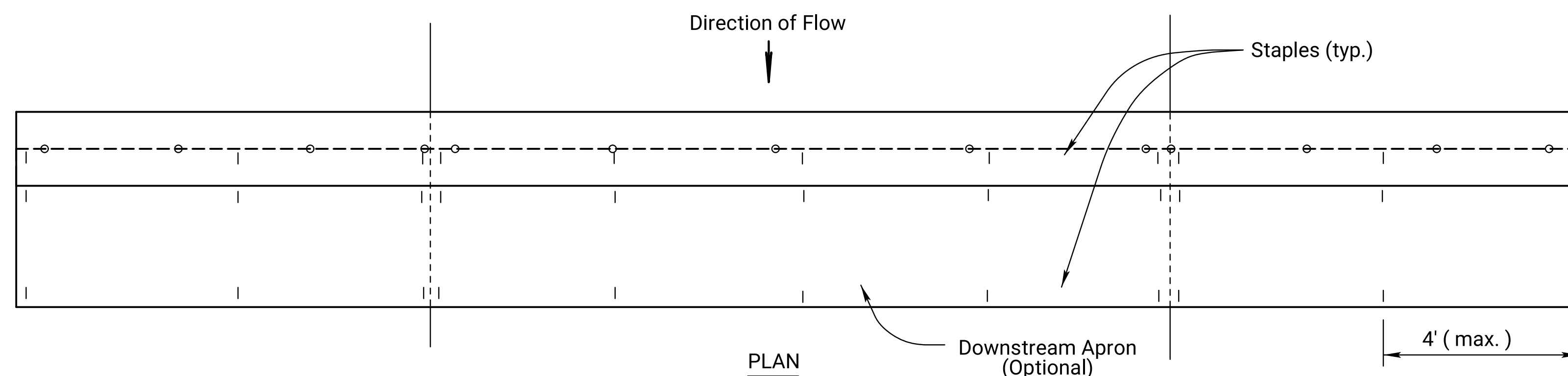
NO SCALE

TEMPORARY ROCK DITCH CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
5.0	60
6.0	50
7.0	43
8.0	36
9.0	33
10.0	29

NOTE: Use this spacing for Rock Ditch Checks only.

ROCK DITCH CHECK NOTES

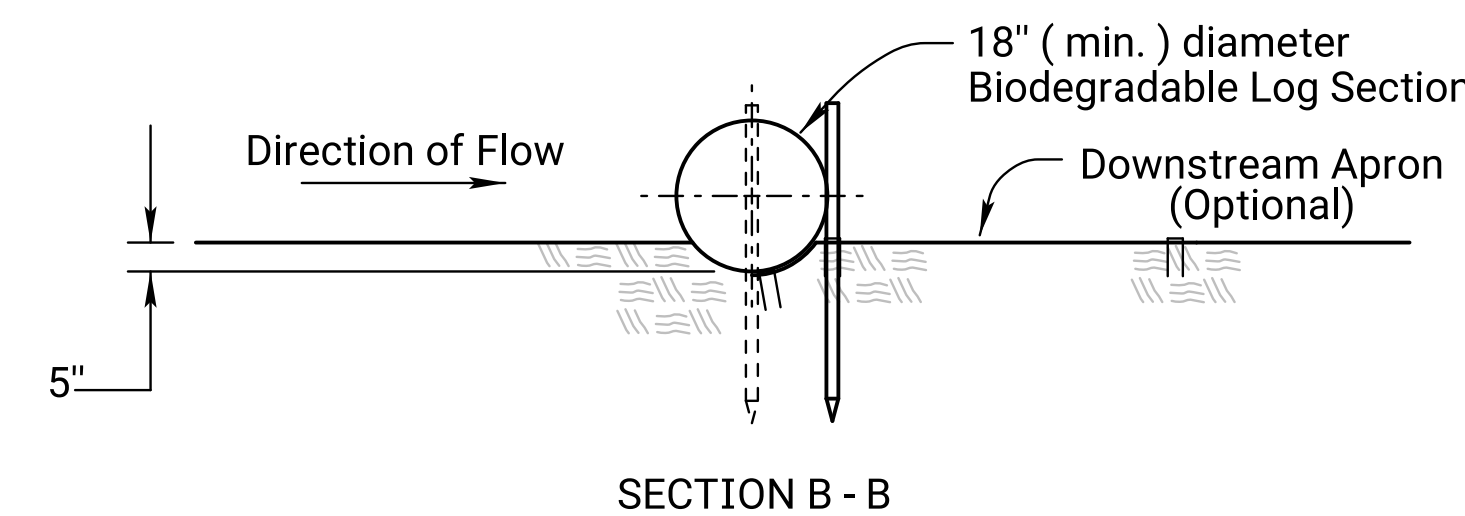
1. Rock shall be clean aggregate, D50-6" and aggregate filler.
2. Place rock in such manner that water will flow over, not around ditch check.
3. Do not use rock ditch checks in clear zone.
4. Excavation: The ditch area shall be reshaped to fill any eroded areas. Prior to placement of the rock, the ditch shall be excavated to the dimensions of the Rock Ditch Check and to a minimum depth of 6" (150mm). After placement of the rock, backfill and compact any over-excavated soil to ditch grade. This work shall be subsidiary to the bid item Temporary Ditch Check (Rock).
5. Aggregate excavated on site may be used as an alternate to the 6" rock, if approved by the Engineer.
6. The Engineer may approve the use of larger aggregates for the downstream portion of the check when conditions warrant their use.
7. When the use of larger rock is approved, D50-6" rock will be placed between the larger aggregate and the aggregate filler.
8. Aggregate filler will be placed on the upstream face of the ditch check. Aggregate filler will comply with Filter Course Type I, Division 1114.



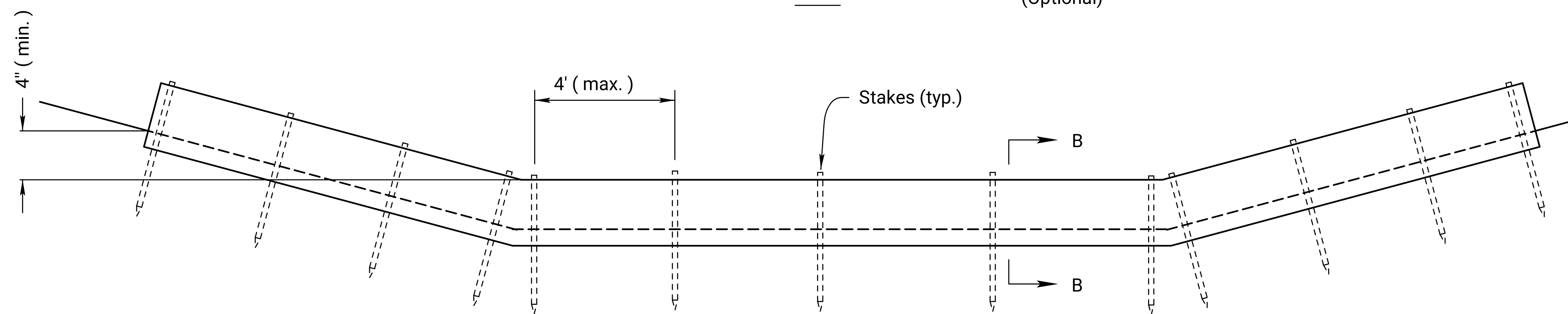
PLAN

Downstream Apron (Optional)

4' (max.)



SECTION B - B



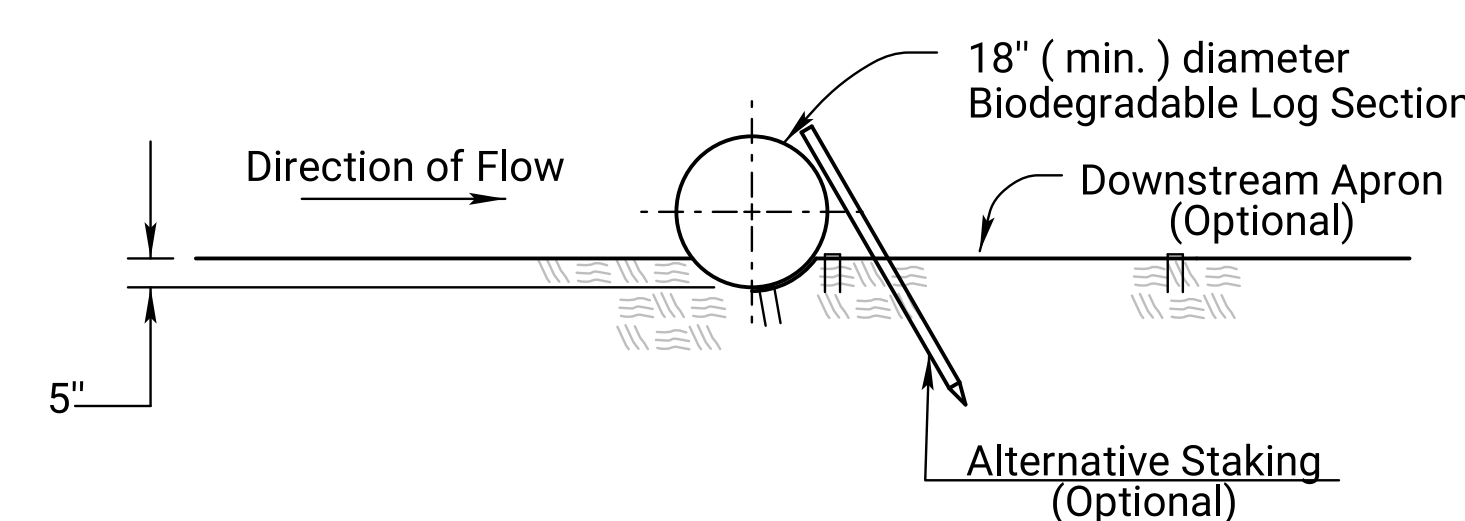
TYPICAL ELEVATION

BIODEGRADABLE LOG DITCH CHECK

OR Filter Sock Ditch Check
NO SCALE

BIODEGRADABLE LOG DITCH CHECK NOTES

1. Use as many biodegradable log sections as necessary to ensure water does not flow around end of ditch check.
2. Overlap sections a minimum of 18".
3. Stakes shall be wood or steel according to Section 2114 of the Standard Specifications. Length of stakes shall be a minimum of 2 x the diameter of the log.
4. Use Erosion Control (Class 1) (Type C) as the downstream apron when required.
5. A downstream apron is required when directed by the Engineer. Apron material will be paid at the contract unit price.
6. Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.

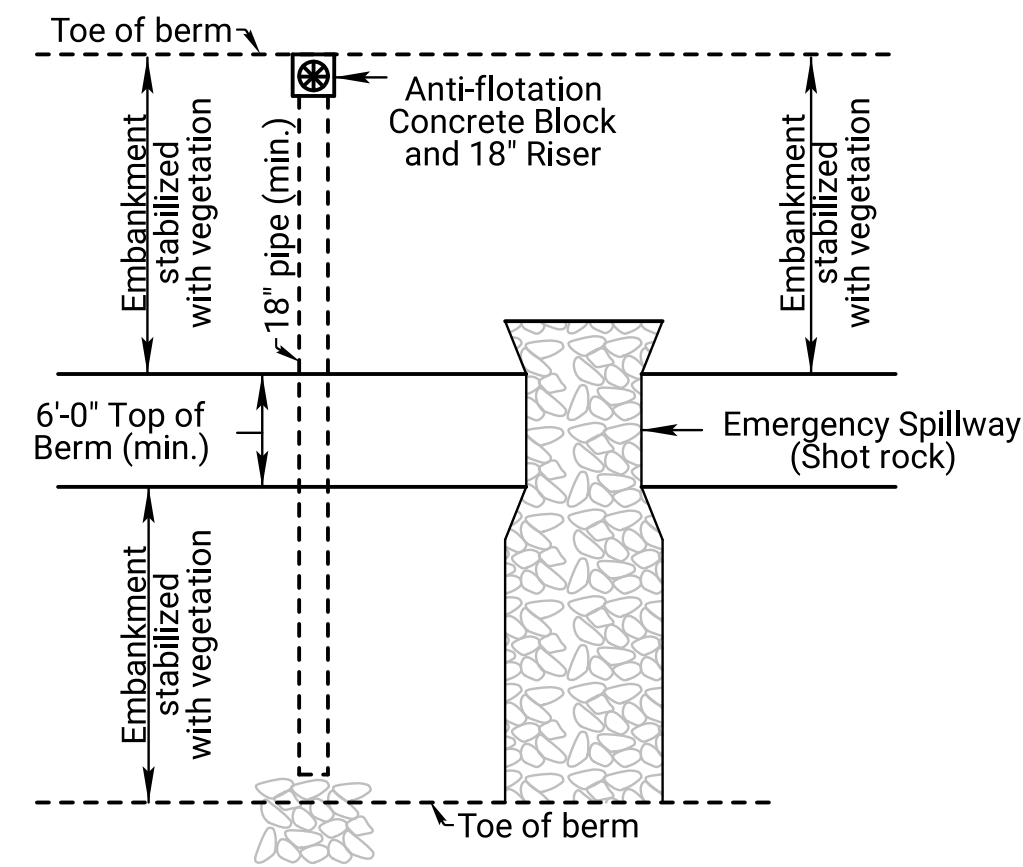


ALT. DETAIL OPTIONAL

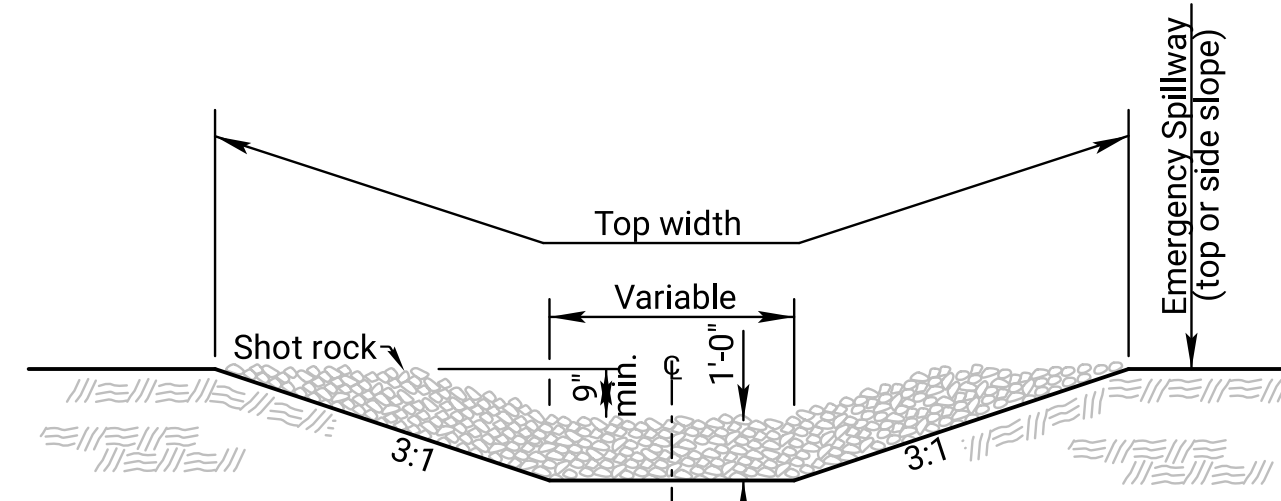
NO.	DATE	REVISIONS	BY	APPD
03	11-19-20	Revised Standard	M.R.D.	M.L.
02	08-10-16	Revised Standard	R.A.A.	S.H.S.
01	10-21-15	Revised Standard	R.A.A.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION					
TEMPORARY EROSION AND POLLUTION CONTROL					
ROCK DITCH CHECKS					
BIODEGRADABLE LOG DITCH CHECKS					
LA852G					
FHWA APPROVAL		11-19-20		APPD	
DESIGNED	M.L.	DETAILED	D.K.	QUANTITIES	TRACED
DESIGN CK.	M.L.	DETAIL CK.	M.L.	QUAN. CK.	TRACE CK.
		Mervin Lare			

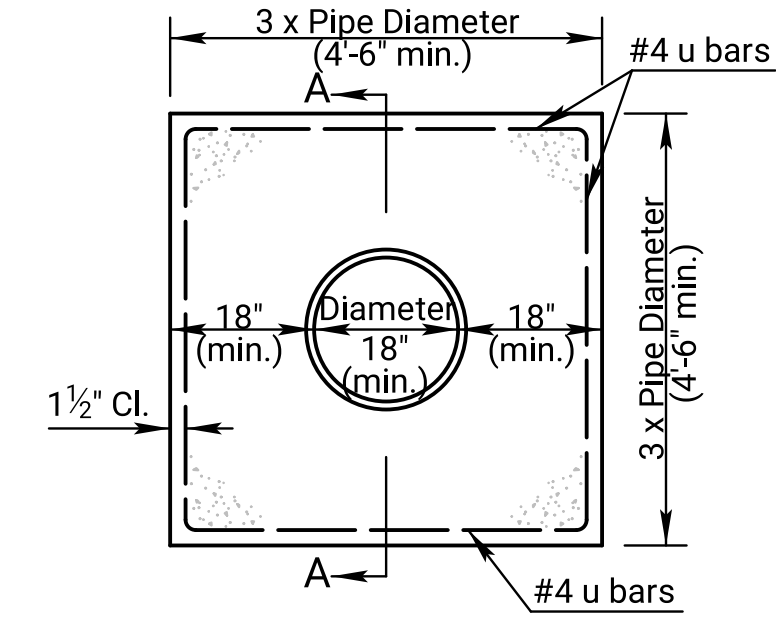
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	73	135



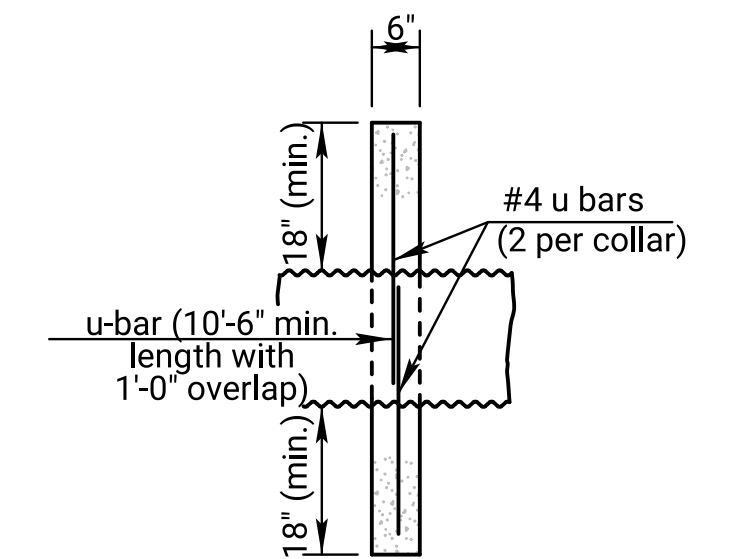
SEDIMENT STORAGE BASIN (PLAN)



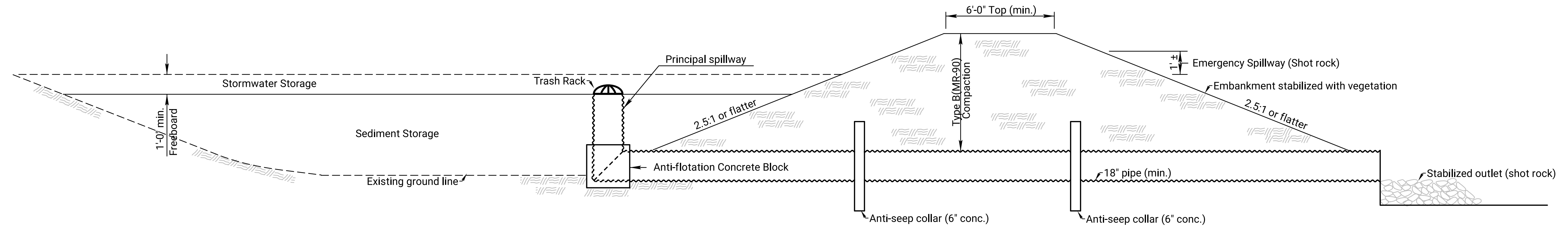
CROSS SECTION (EMERGENCY SPILLWAY)



CONCRETE ANTI-SEEP COLLAR



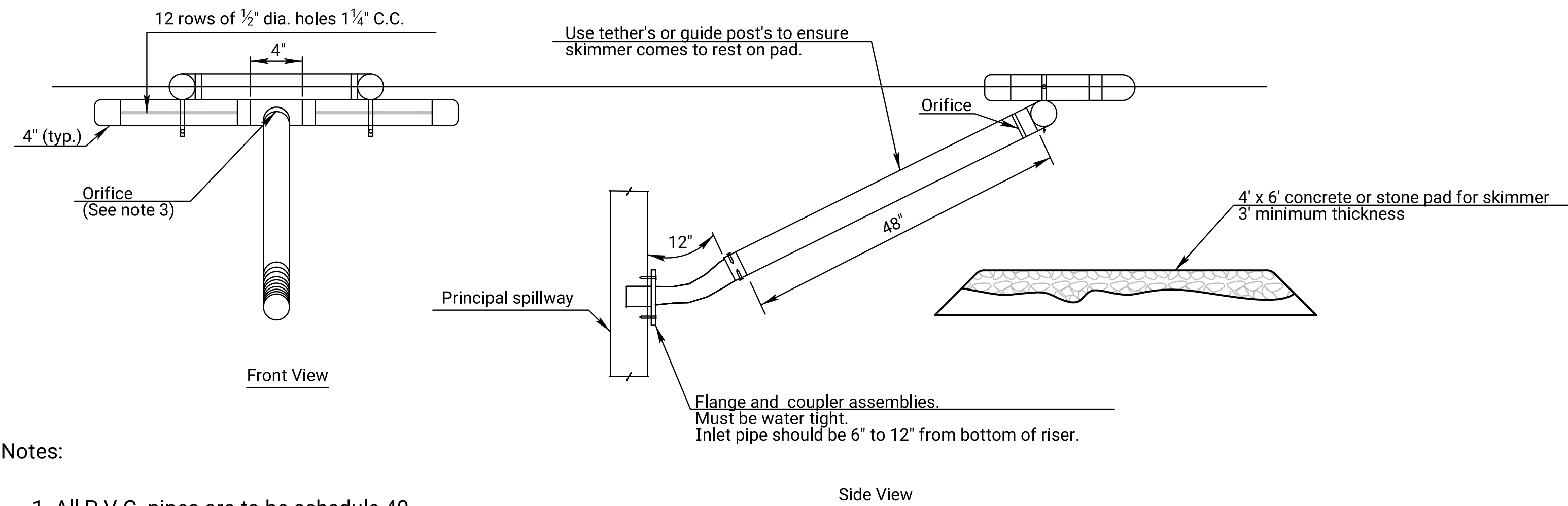
SECTION A-A



SEDIMENT STORAGE BASIN (ELEVATION)

NOTES:

- 1) Temporary Sediment Basins shall be constructed at locations as directed by the Engineer or as approved in the SWPPP Schedule. All work and materials necessary, including but not limited to, the fill material, compaction, drainage pipes, aggregates and all other incidentals necessary to construct the basin, shall be paid as "Temporary Sediment Basin".
- 2) Lengths and top dimensions shall be determined in the field by the Engineer.
- 3) Skimmer dewatering device required and must be used regardless the size of the drainage area.



SKIMMER DEWATERING DEVICE

- Notes:**
1. All P.V.C. pipes are to be schedule 40.
 2. HDPE flexible drain pipes is to be attached to the pond outlet structure with water-tight connections.
 3. The orifice shall be sized of to provide drawdown time to 2 to 5 days and approved by the engineer.
 4. Other skimmer designs maybe used that dewateres from the surface at a controlled rate. The design must be approved by the engineer.

SEDIMENT STORAGE BASIN LOCATIONS		
STATION TO STATION	SIDE	REQUIRED STORAGE CAPACITY

02	09-03-13	Added Skimmer Dewatering Device	M.R.M.	S.H.S.
01	07-17-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

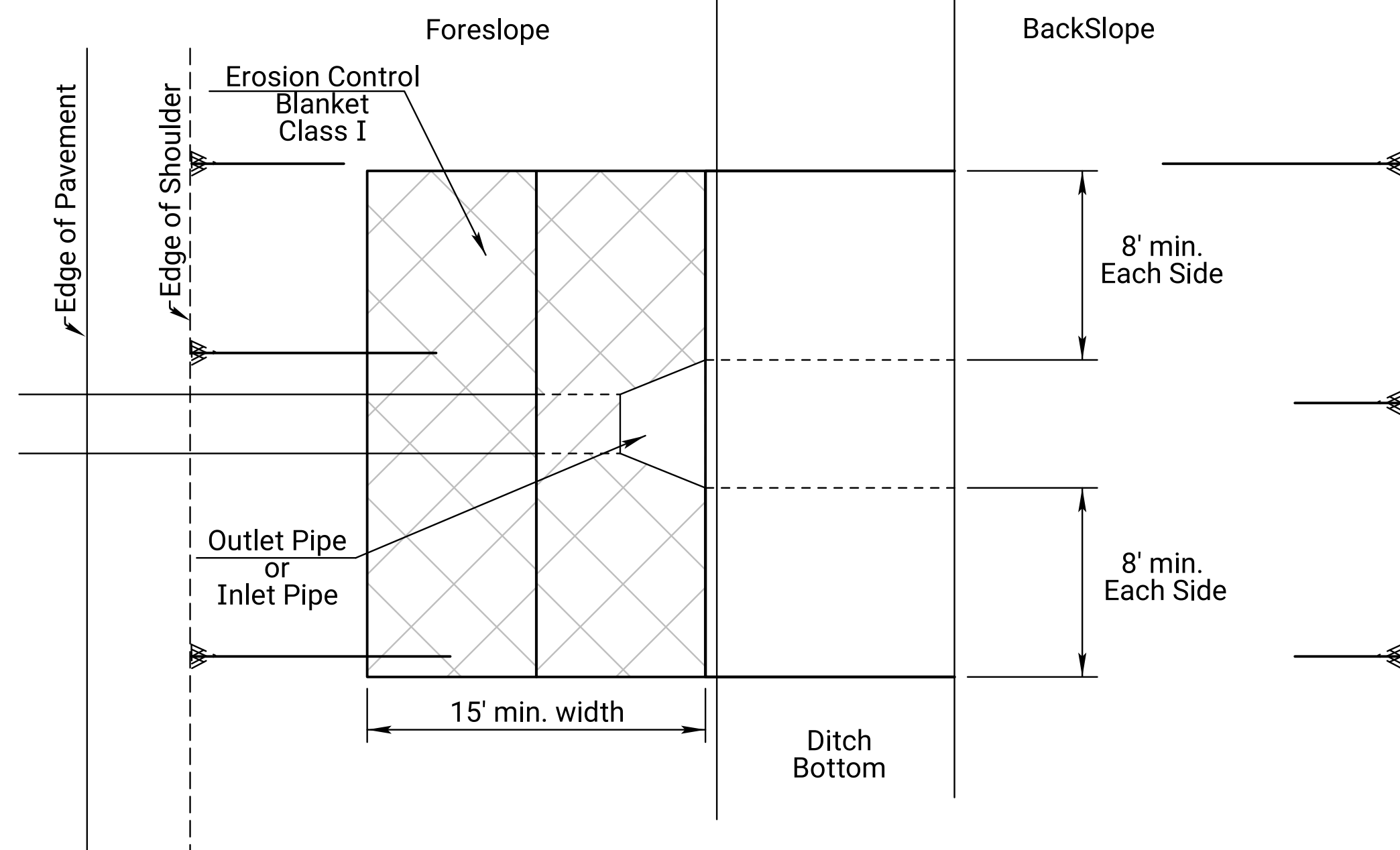
TEMPORARY EROSION AND POLLUTION CONTROL SEDIMENT STORAGE BASIN

LA852H

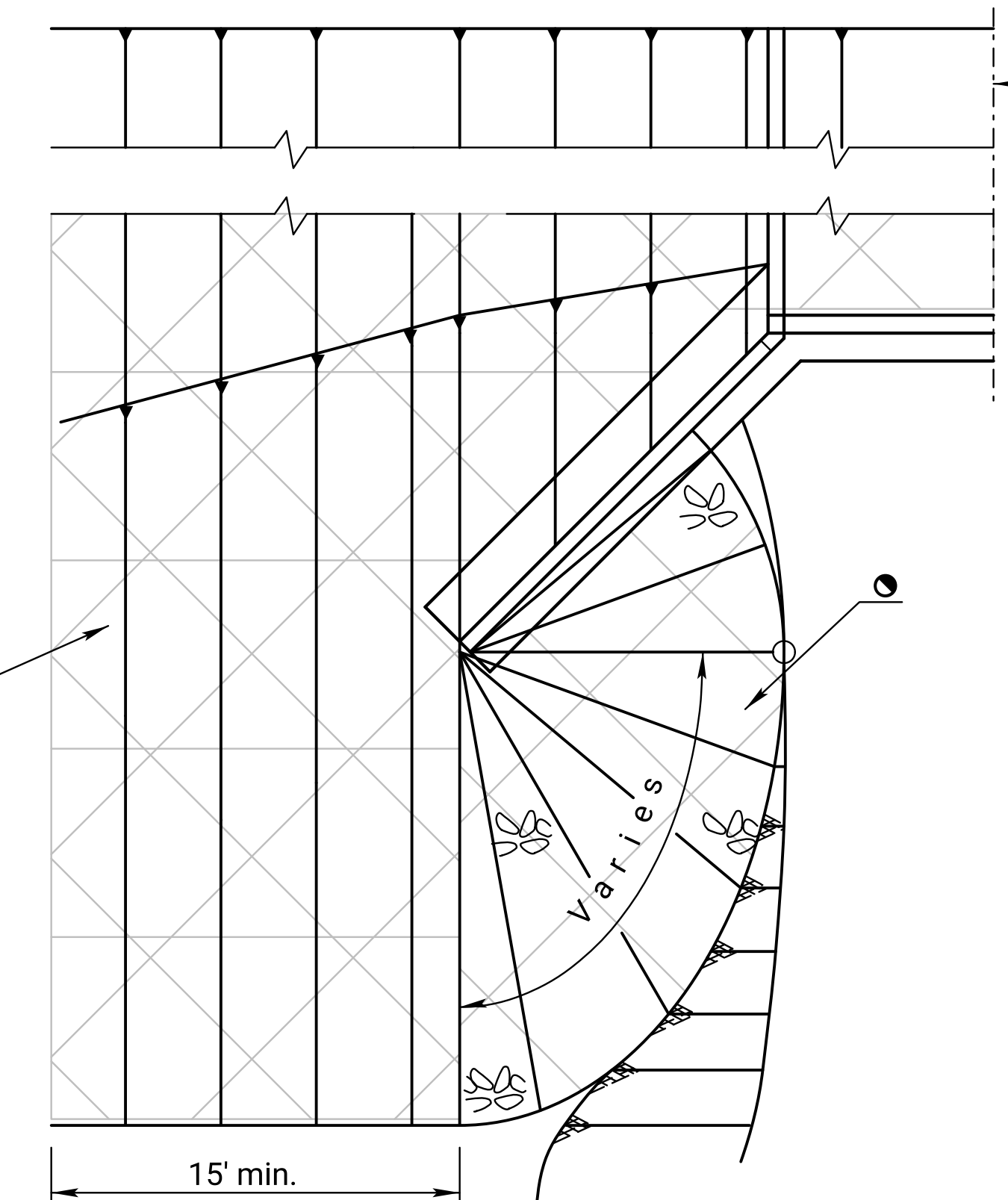
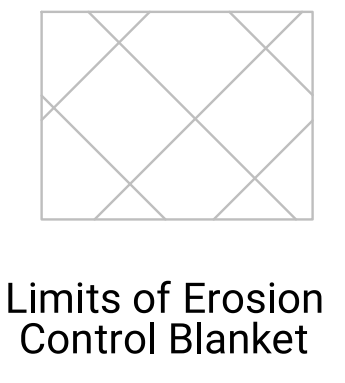
DESIGNED	B.B.	QUANTITIES	TRACED	B.B.
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.

DOT Graphics Certified 06-20-2022 Sh. No. 73

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	74	135



PARTIAL PLAN PIPE

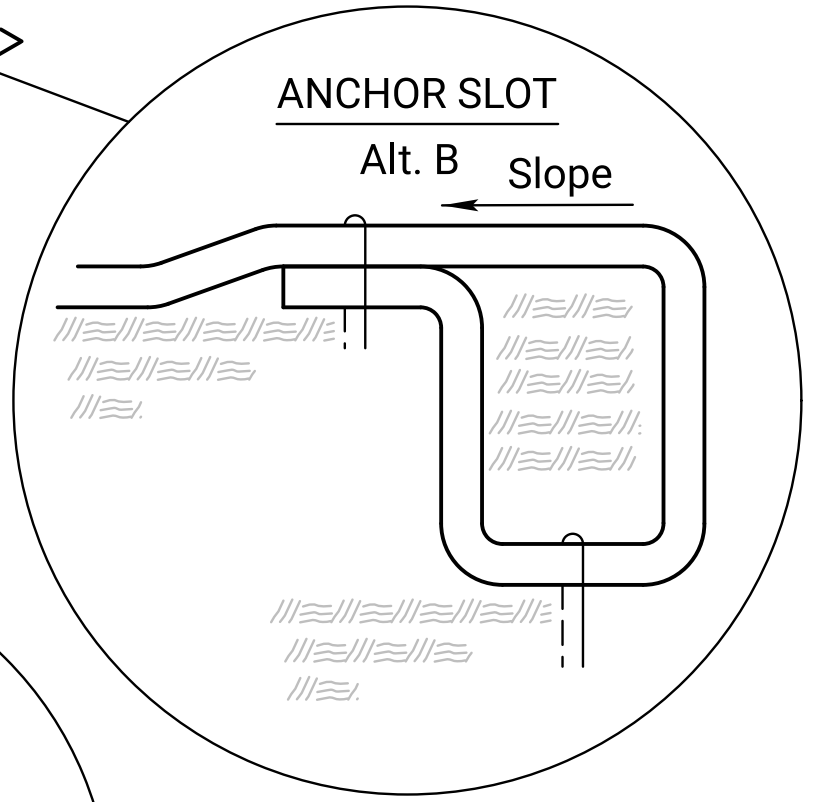
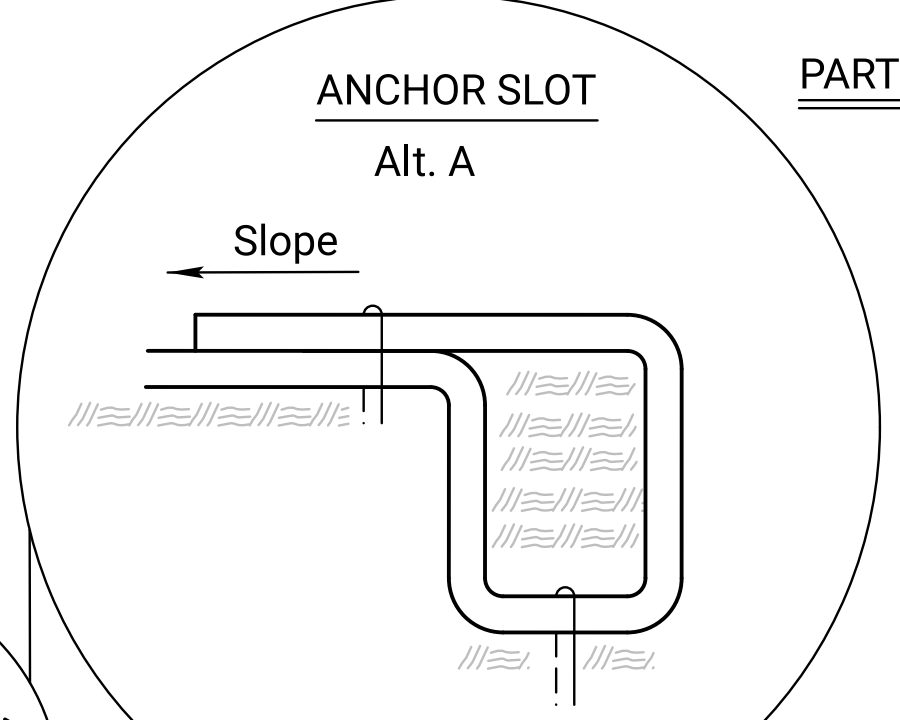
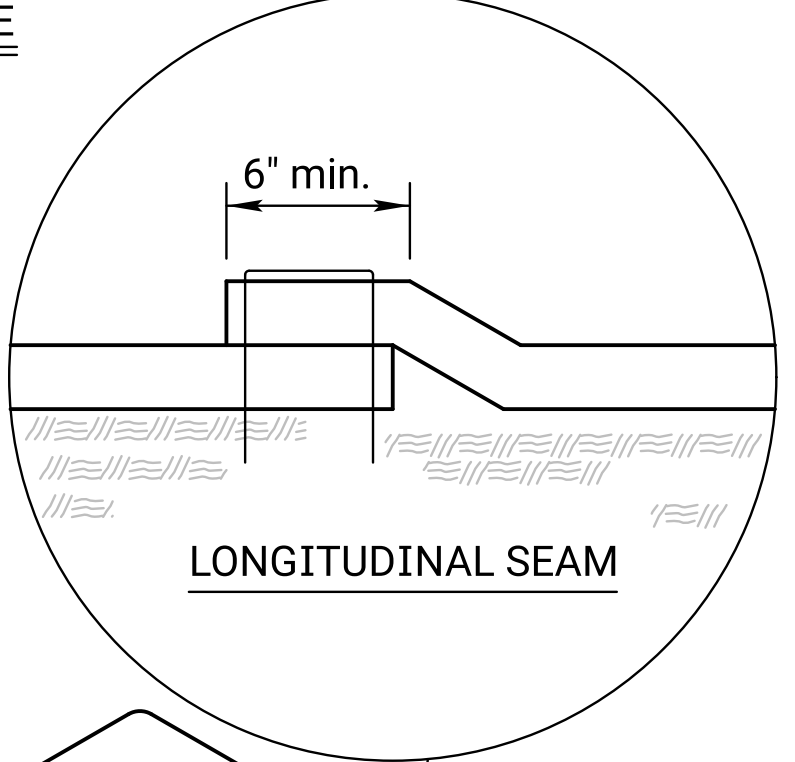
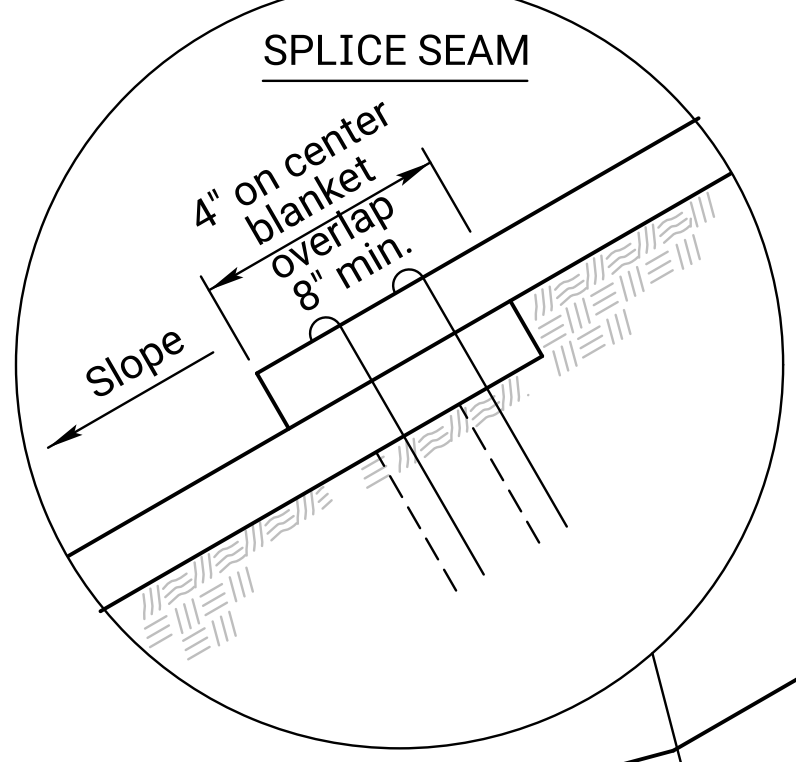


PARTIAL PLAN BOX CULVERT

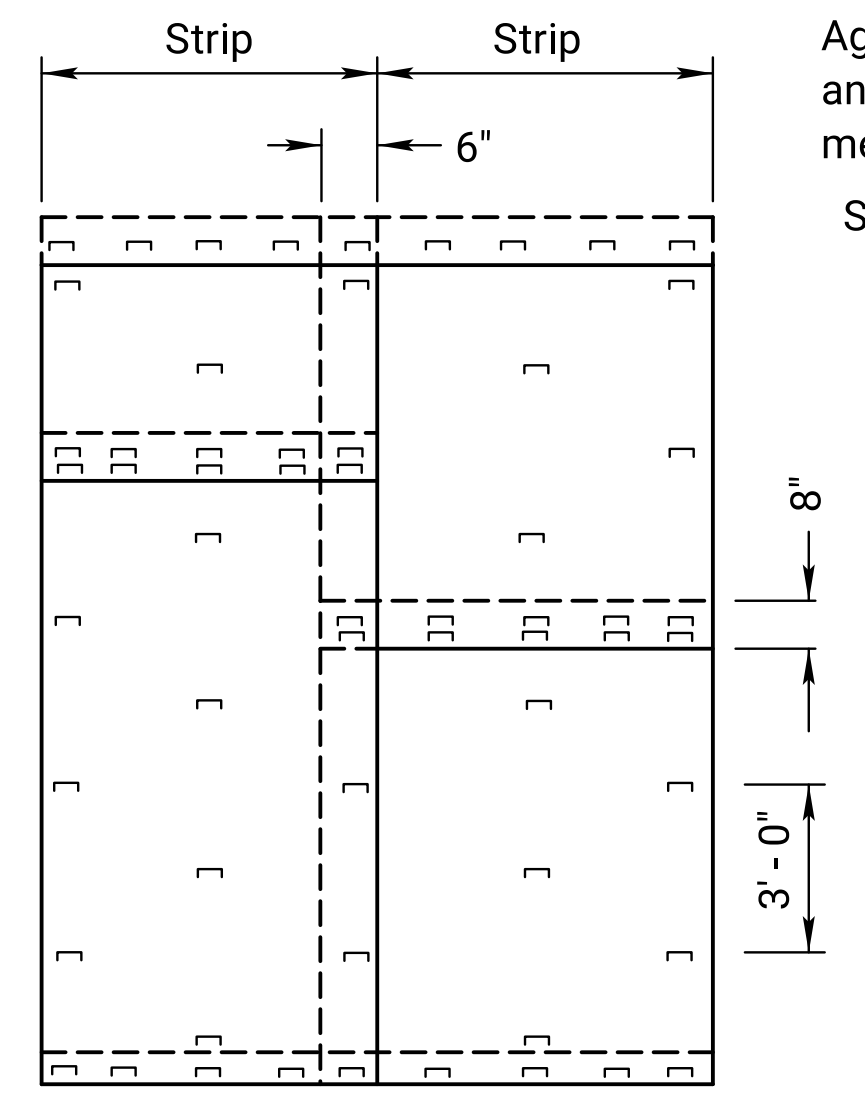
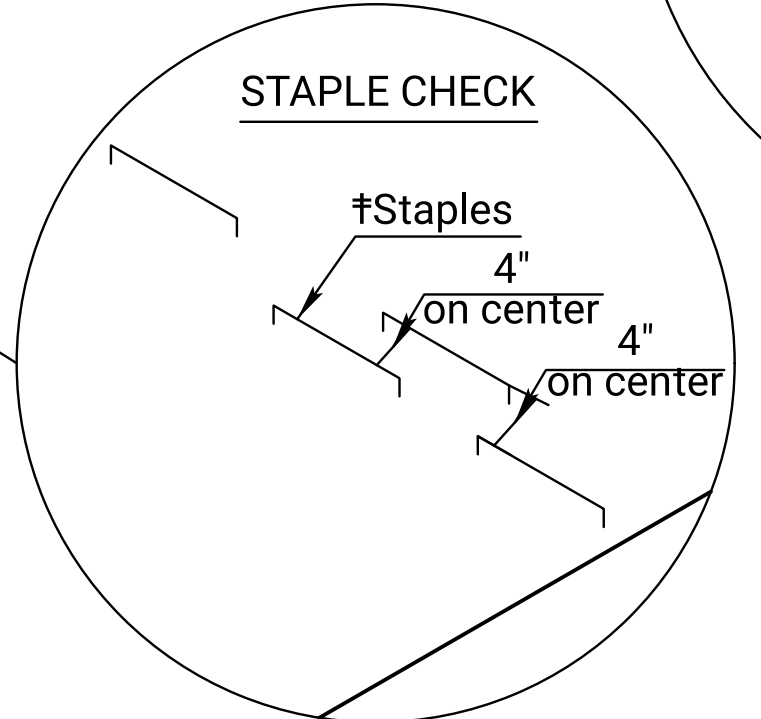
INSTALLATION DETAILS FOR EROSION CONTROL CLASS 1

Erosion Control Blankets shall be laid loosely in the direction of the slope, beginning at the bottom of the slope. In order for blanket to be in contact with the soil, lay blanket loosely, avoiding stretching.

- ANCHOR SLOTS:** The top of the blanket should be "slotted in" at the top of the slope and anchored in place with anchors 6 inches apart. The slots should be 6 inches wide x 6 inches deep with the blanket anchored in the bottom of the slot, then backfilled, tamped and seeded.
- LONGITUDINAL SEAMS:** The edges of the blanket should overlap each other a minimum of 6 inches, with anchors catching the edges of both blankets.
- SPLICE SEAM:** When splices are necessary, overlap end a minimum of 8 inches in direction of water flow. Stagger splice seams.
- TERMINAL FOLD:** The bottom edge of the blanket shall be turned under a minimum of 4 inches, then anchored in place with anchors 9 inches apart.
- TYPICAL ANCHORS:** Anchor design shall be as recommended by the manufacturer.
- STAPLE CHECK:** Establish Staples in 2 rows 4" on center apart. Staple Checks - shall be 30' apart.

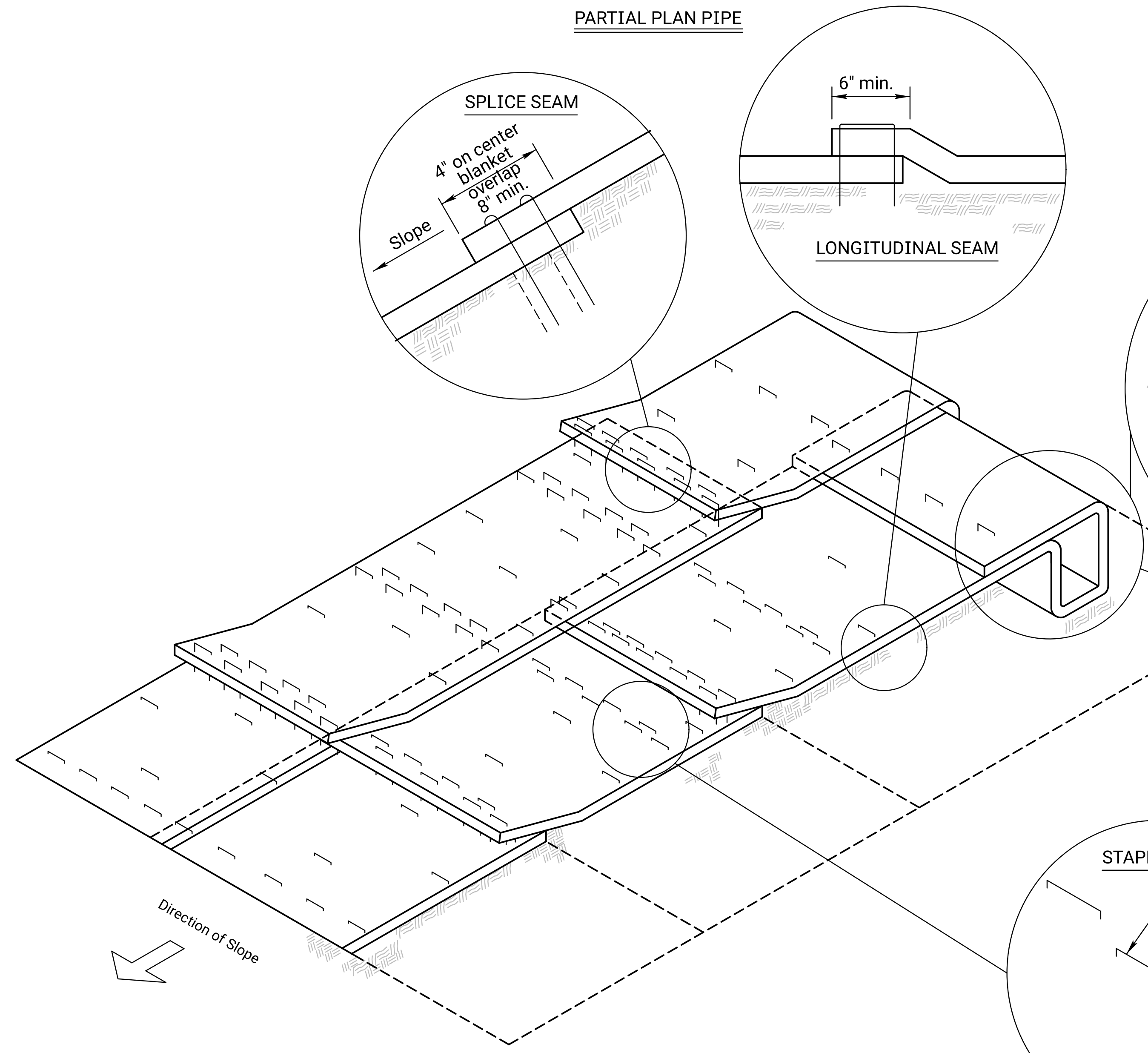


● Erosion Control Class I may be omitted if the area is immediately covered by permanent slope protection (where directed by the plans).



PLAN VIEW - ANCHORING DIAGRAM

NOTE: Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards. Single post ring and shank staple is acceptable.



NO.	DATE	REVISIONS	BY	APPD
04	03-01-15	Revised Standard	R.A.A.	S.H.S.
03	02-23-15	Revised Standard	R.A.A.	S.H.S.
02	09-15-14	Revised Standard	M.R.M.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION				
INSTALLATION DETAIL EROSION CONTROL CLASS 1 SLOPE PROTECTION				
LA855				
DESIGNED	R.A.A.	DETAILED	R.A.A.	APPD.
QUANTITIES	R.A.A.	TRACED	R.A.A.	SCOTT H. SHIELDS
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	R.A.A.

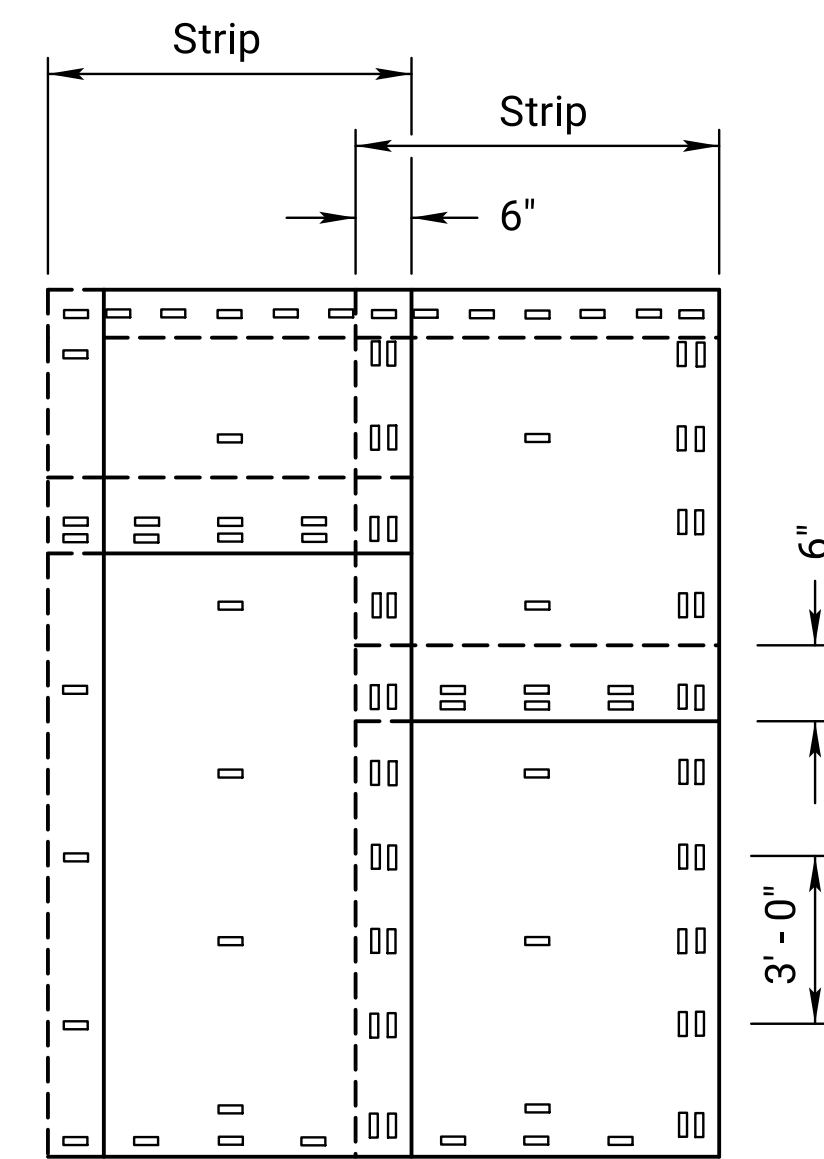
Plotted by: Melissa.Davidson@ks.gov 10-JUL-2023 18:08
File: LA855.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	75	135

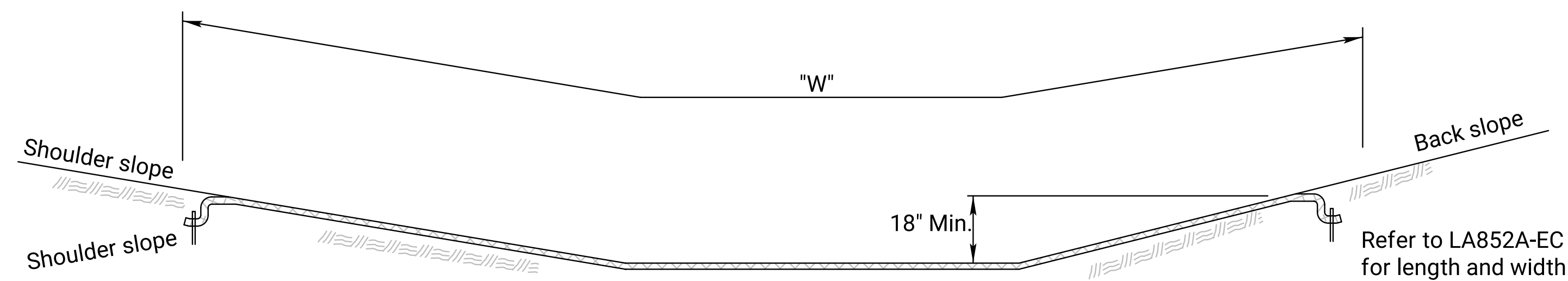
INSTALLATION DETAILS FOR EROSION CONTROL CLASS 2

Erosion Control Mats shall be laid loosely in the direction of the flow, with the first course at the centerline of channel, where applicable. In order for the mat to be in contact with the soil, lay the mat loosely, avoiding stretching.

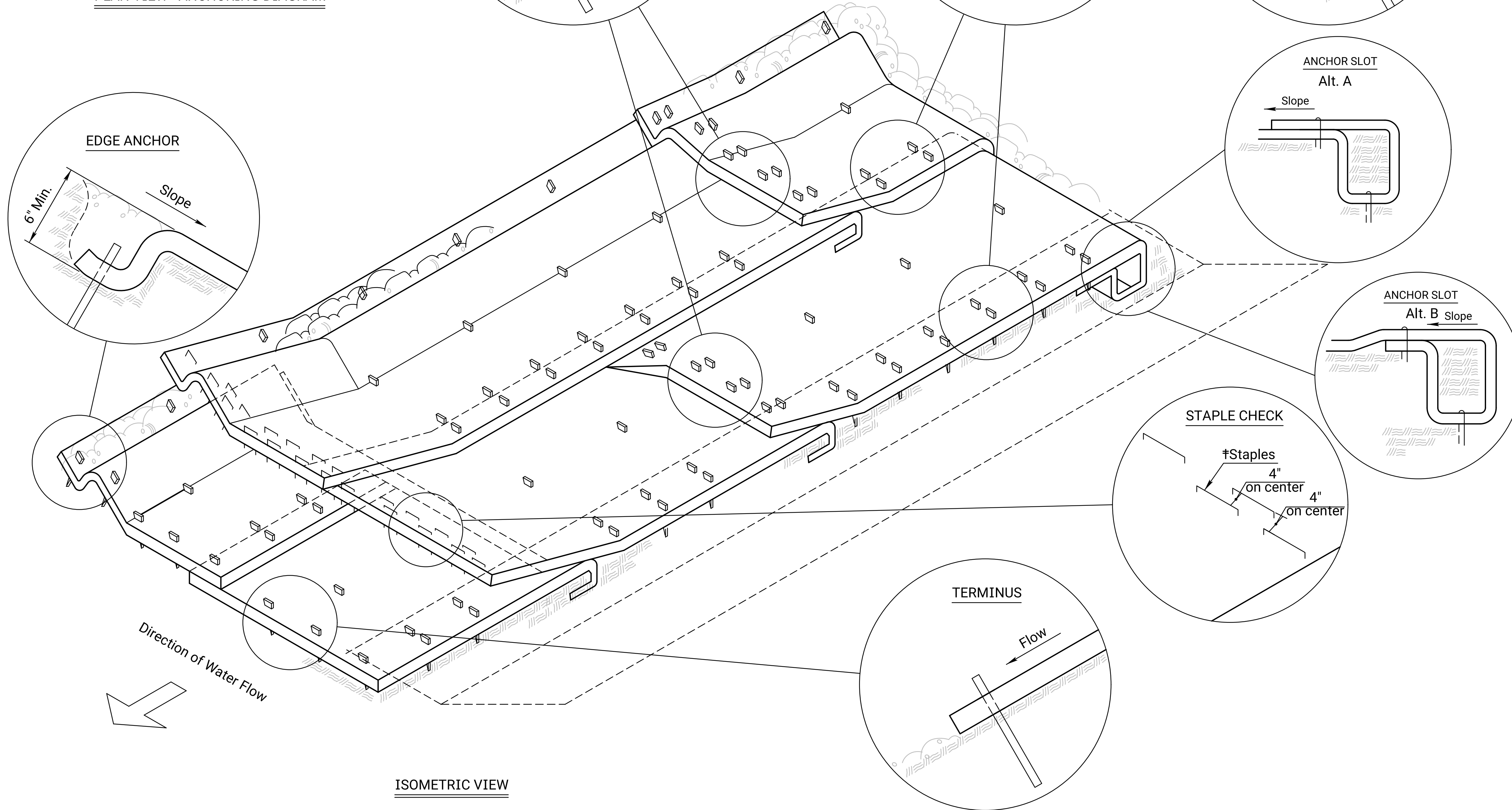
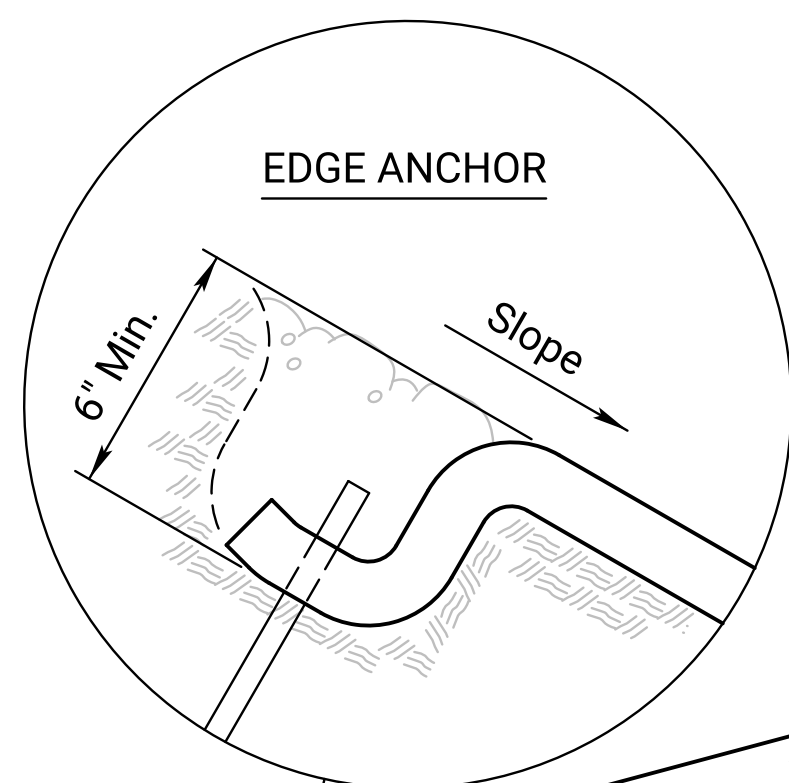
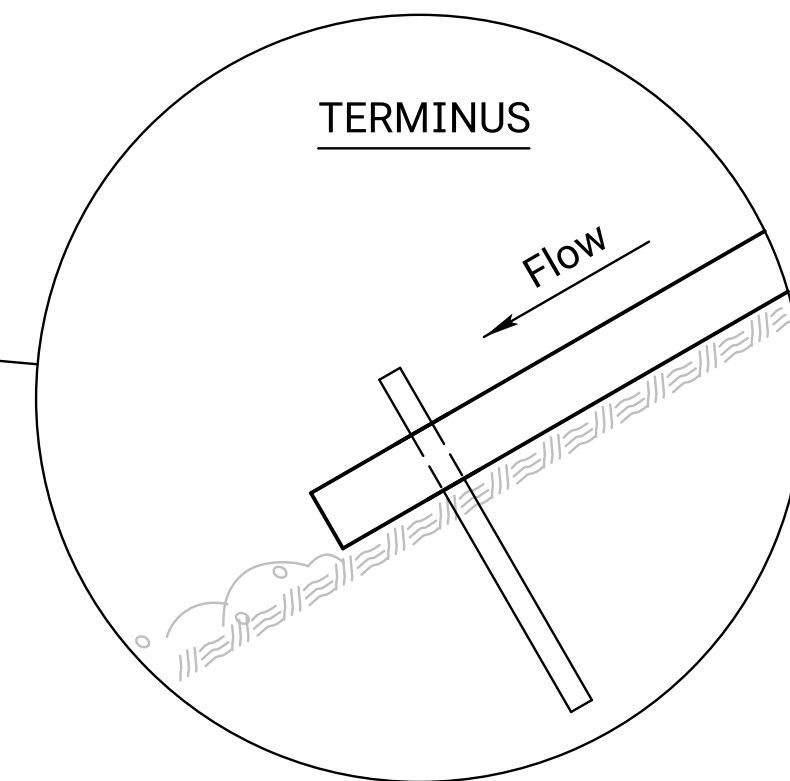
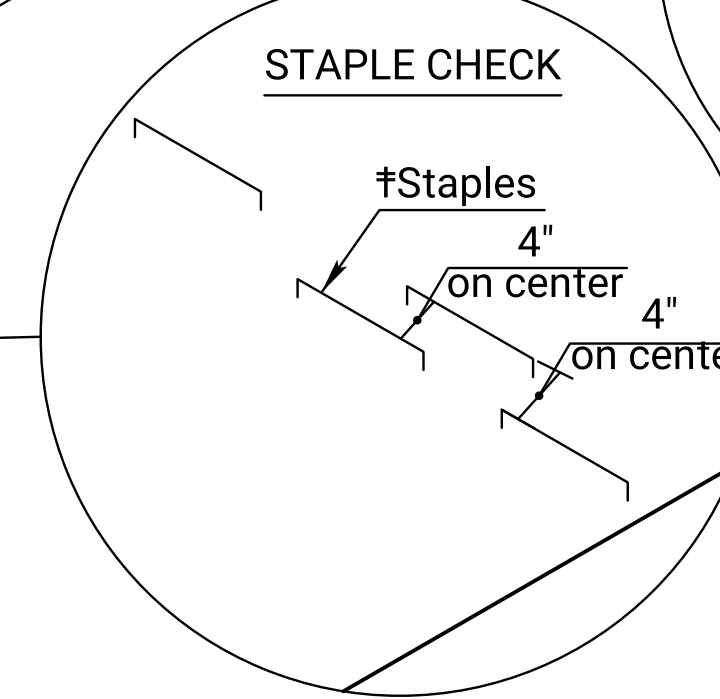
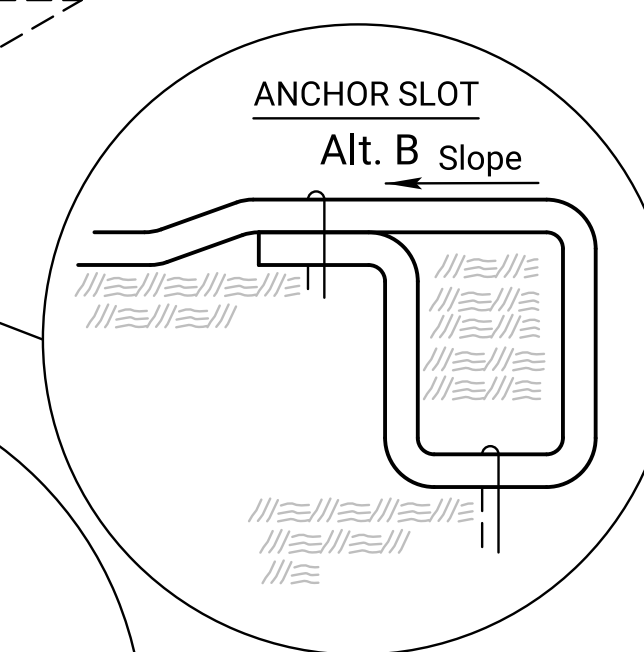
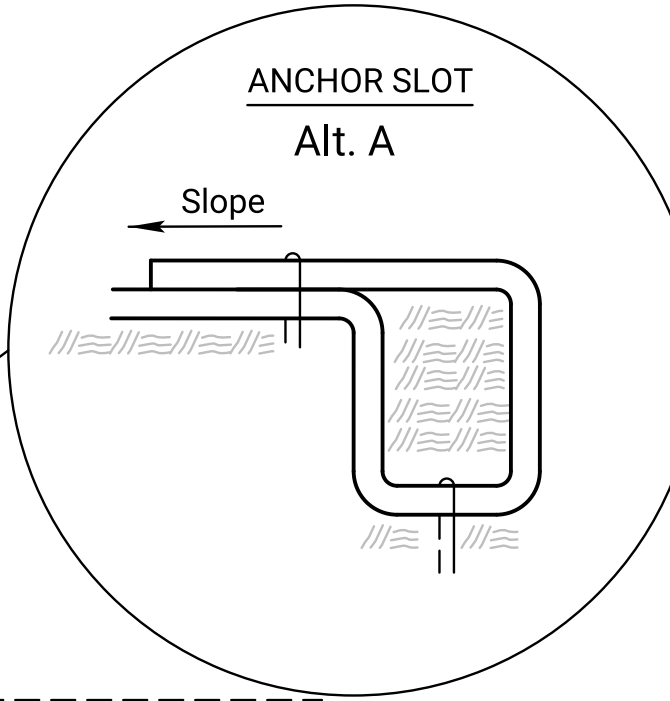
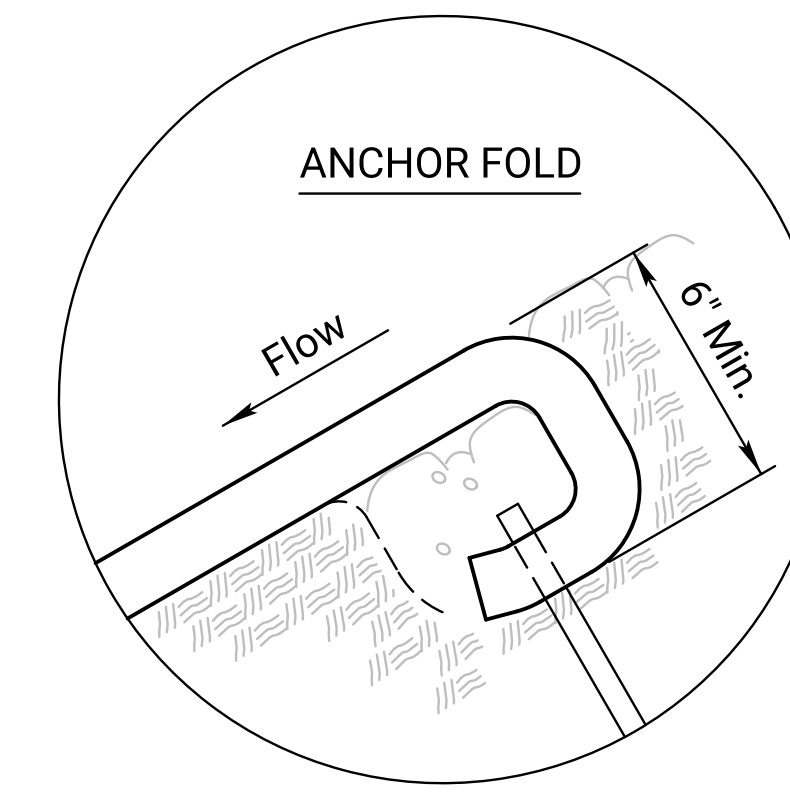
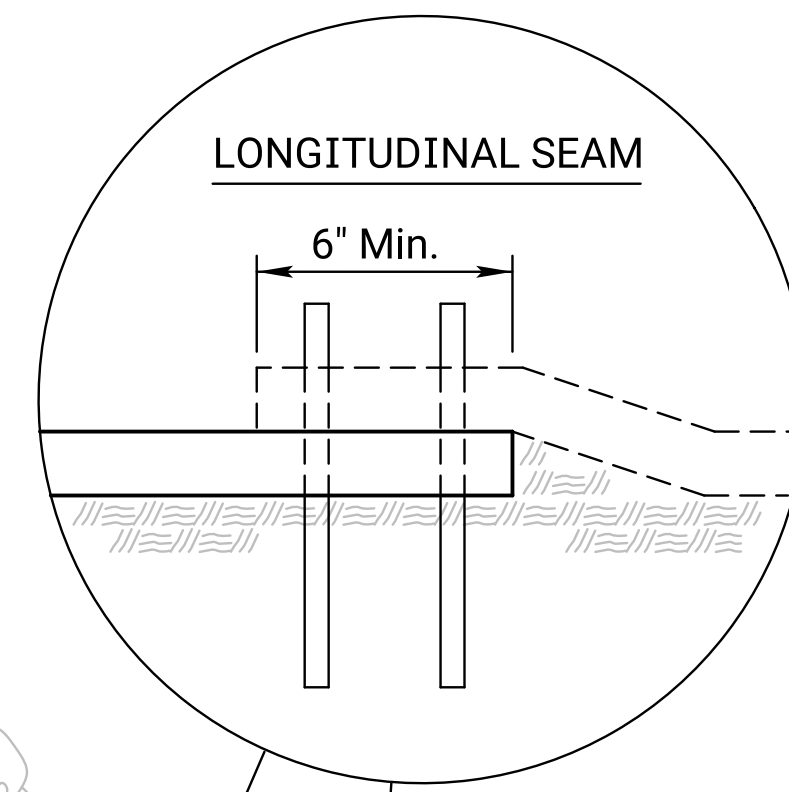
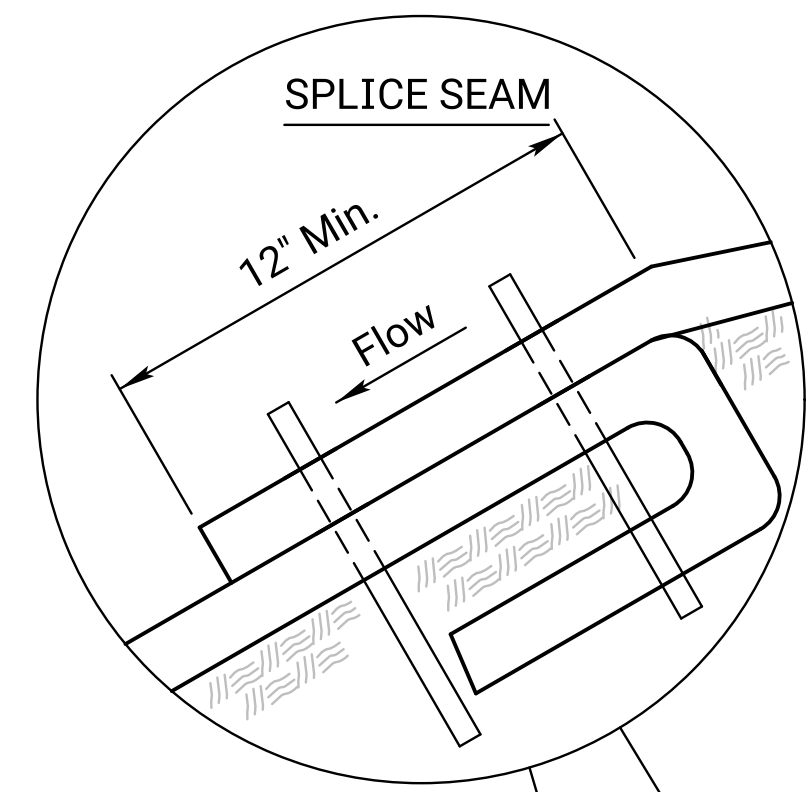
- ANCHOR FOLD:** The top of the mat should be folded under, buried and secured with approved anchors placed 6 inches apart. The top edge of the mat should be buried in a slot, 6 inches wide x 6 inches deep; anchored in the bottom of the slot, backfilled, and the mat folded over the top as shown in detail.
- LONGITUDINAL SEAMS:** The adjacent edges of the mat should overlap a minimum of 6 inches, with anchors catching the edges of both mats.
- SPLICE SEAM:** When splices are necessary, overlap end a minimum of 12 inches in direction of water flow. Stagger splice seams.
- STAPLE CHECK:** *Establish Staples in 2 rows 4" on center apart. Staple Checks - shall be 30' apart.
- EDGE ANCHOR:** Lay outside edge of mat into trench at top of side slope. Anchor at 3 foot intervals along trench.
- TERMINUS:** The bottom edge of the mat shall be anchored in place with anchors spaced at 9 inch intervals along the terminating edge.
- TYPICAL ANCHORS:** Anchor design shall be as recommended by the manufacturer.



PLAN VIEW - ANCHORING DIAGRAM



CROSS SECTION (Ditch Lining)



ISOMETRIC VIEW

04	09-25-15	Modified Staple Check	R.A.A.	S.H.S.
03	09-15-14	Revised Standard	R.A.A.	S.H.S.
02	03-01-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION				
INSTALLATION DETAIL EROSION CONTROL CLASS 2 FLEXIBLE CHANNEL LINER				
LA856				
DESIGNED	R.A.A.	11-02-15	APPD.	Scott H. Shields
DETAIL CD.	S.H.S.	DETAIL CD.	S.H.S.	QUAN. CK.
DESIGN CK.	S.H.S.	DETAIL CD.	S.H.S.	QUAN. CK.
TRACED	R.A.A.	TRACED	R.A.A.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	76	135

GRASS & WILDFLOWER SEEDING SEASONS

COOL SEASON GRASSES February 15 thru April 20 August 15 thru September 30	WARM SEASON GRASSES & WILDFLOWERS November 15 thru June 1
SPECIES	SPECIES
Bluegrasses	Bermuda Grass
Brome Grasses	Big Bluestem
Canada Wildrye	Blue Grama
Fescues	Buffalo Grass
Prairie Junegrass	Indiangrass
Ryegrasses	Little Bluestem
Sterile Wheatgrass	Sand Bluestem
Tall Dropseed	Sand Dropseed
Western Wheatgrass	Sand Lovegrass
	Side Oats Grama
	Switchgrass
	Wildflower Mixes

When the area to be seeded is 1 acre or more, if Cool Season grasses are mixed with Warm Season grasses, seed the area during the Warm Season.

When the area to be seeded is less than 1 acre, seed the area any time of the year.

GENERAL NOTES

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded and mulched. Soil preparation shall conform to the Standard Specifications except as noted below.

All borrow areas shown on the plans are to be fertilized, seeded, and mulched. However, operation in borrow areas where crops are growing may be omitted when requested by the owner.

If temporary cover has provided stable slopes with no erosion, seed the permanent grasses into the existing cover. If there has been erosion that requires repair prior to seeding, then it may be necessary to regrade the area, resulting in bare ground.

FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P₂O₅, K₂O listed in Summary of Seeding Quantities will be acceptable.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching material is generally as follows:

1 3/4 - 2 1/4 Tons per Acre = 1 1/2" loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

When seeding is less than 1 acre, temporary and permanent seeding shall be combined and seeded at the same time.

There is no seasonal restriction when seeding projects less than one acre.

SODDING SEASONS

COOL SEASON GRASSES March 1 thru April 15 September 1 thru November 15	WARM SEASON GRASSES May 15 thru September 1
SPECIES	SPECIES
Bluegrass Sod	Buffalo Grass Sod
Fescue Sod	

If the soil is workable, the Engineer may allow placement of sod between November 15 and March 1. If sod is placed during this time, maintain the sod until 20 days after the beginning of the spring sodding season.

SUMMARY OF SEEDING QUANTITIES

P.L.S. RATE/ACRE		ACRES		BID ITEM	QUANTITY	UNIT
SHLDR	OTHER	SHLDR	OTHER			
	80		3.81	Fertilizer (15 - 30 - 15)	304.8	Lbs
	2		3.81	Seed (Big Bluestem (Kaw))	7.6	Lbs
	10		3.81	Seed (Canada Wildrye)	38.1	Lbs
	2		3.81	Seed (Indiangrass (Osage))	7.6	Lbs
	2		3.81	Seed (Little Bluestem (Aldous))	7.6	Lbs
	6.3		3.81	Seed (Side Oats Grama (El Reno))	24	Lbs
	10		3.81	Seed (Sterile Wheatgrass)	38.1	Lbs
	0.7		3.81	Seed (Switchgrass (Blackwell))	2.7	Lbs
	0.5		3.81	Seed (Tall Dropseed)	1.9	Lbs
	4		3.81	Seed (Western Wheat (Barton))	15.2	Lbs
	10.3		3.81	Seed (Native Wildflower Mix 1)	39.2	Lbs
				Mulching *		

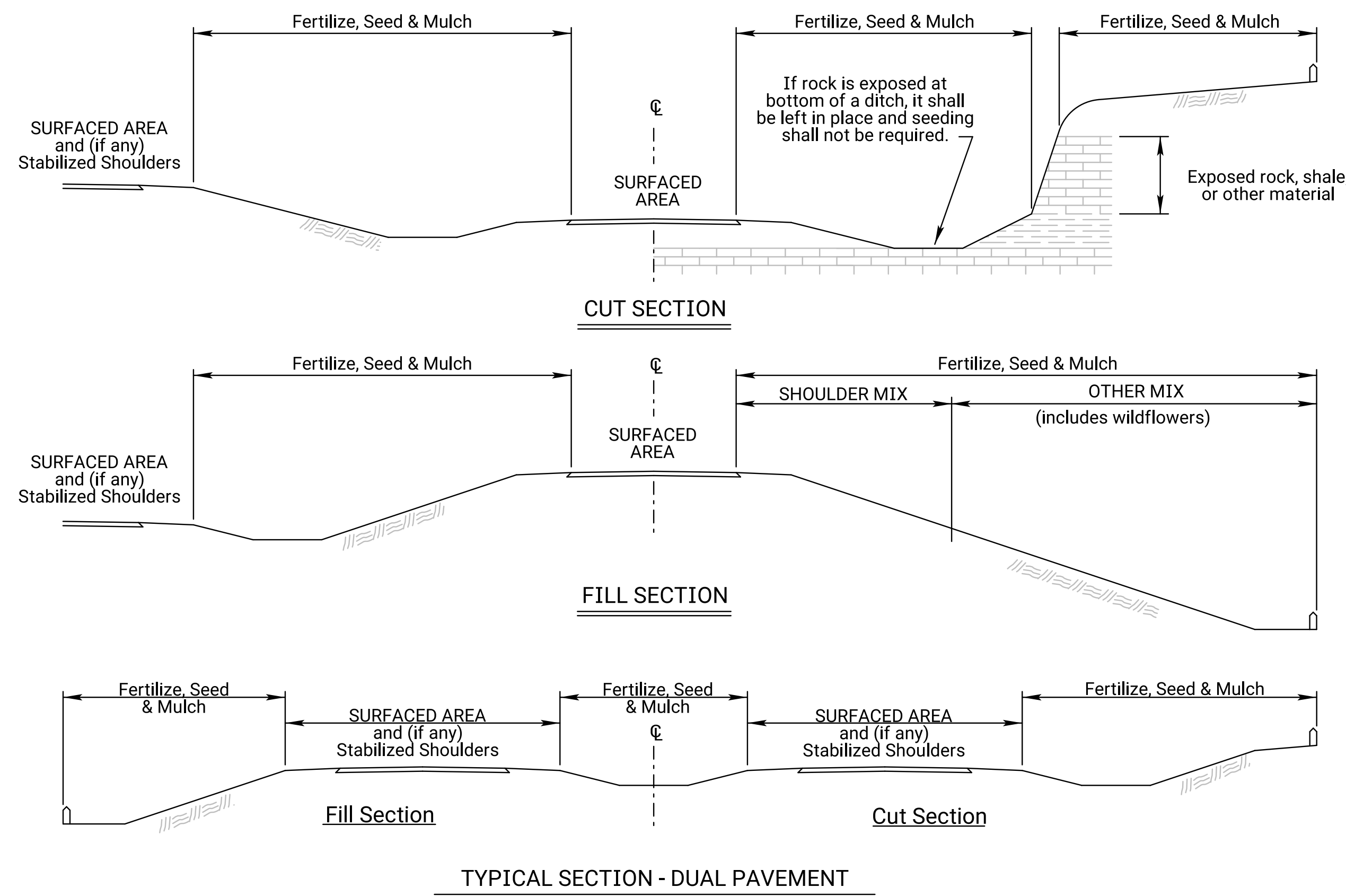
SHLDR = Seeded with the Shoulder Mix. Typically 15 feet for 2-lane roads and 30 feet for 4-lane roads. Includes outside roadsides, turfed portions of shoulders, and turfed portion of the median.

OTHER = Seeded with the "Other" Mix. Designated as all other turf areas, except the Shoulder. Usually includes a Native Wildflower Mix.

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. All disturbed areas shall be seeded, fertilized and mulched at the listed rate per acre. The acres are estimated.

Refer to the Standard Specifications, Division 900, Section 904 'Seeding', and Section 907 'Sodding', for the seeding and sodding seasons.

* See LA852A for mulching quantity. The quantity of mulch is estimated (Acres of Seeding X 1.5 X 2 Tons/Acre). The total mulch required shall be determined in the field. The bid item for mulching shall be paid for according to the Standard Specifications.



NATIVE WILDFLOWER MIX 1

PLS RATE	NAME	QTY (lb)
0.3	Butterfly Milkweed	1.14
0.3	Common Milkweed	1.14
0.3	Black Eyed Susan	1.14
0.5	Blanket Flower	1.91
0.5	False Sunflower	1.91
0.5	Lance-Leaf Coreopsis	1.91
0.2	Maximilian Sunflower	0.76
0.1	New England Aster	0.38
0.2	Pinnate Prairie Coneflower	0.76
0.2	Plains Coreopsis	0.76
0.3	Purple Coneflower	1.14
0.3	Upright Prairie Coneflower	1.14
0.3	Dames Rocket	1.14
0.3	Lemon Mint	1.14
0.2	Pitcher Sage	0.76
0.2	Wild Bergamot	0.76
1.0	Illinois Bundleflower	3.81
0.2	Common Evening Primrose	0.76
0.1	Hoary Verbena	0.38
0.8	Purple Prairie Clover	3.05
0.3	Roundhead Lespedeza	1.14
3.0	Showy Partridge Pea	11.43
0.2	White Prairie Clover	0.76
10.3	Total (lb)	39.2

NATIVE WILDFLOWER MIX 2

PLS RATE	NAME	QTY (lb)
0.3	Butterfly Milkweed	
0.3	Black Eyed Susan	
0.5	Black Sampson Coneflower	
1.0	Blanket Flower	
0.2	Maximilian Sunflower	
0.2	Plains Coreopsis	
0.2	Upright Prairie Coneflower	
0.2	Western Yarrow	
0.3	Lemon Mint	
0.4	Pitcher Sage	
1.5	Illinois Bundleflower	
0.2	Common Evening Primrose	
1.0	Blue Wild Indigo	
0.4	Leadplant	
0.4	Purple Prairie Clover	
0.3	White Prairie Clover	
7.4	Total (lb)	

Package and deliver the wildflower seed separately from the grass seed mix. Package and deliver the Tall Drop Seed separately from the grass seed and the wildflower mix. Place the grass seed (except Tall Drop Seed) in the large seed box and drill (cover) seed 1/8" - 1/4". Place the wildflower seed in a separate seed box and drill (cover) seed 1/16" maximum. Place the Tall Drop Seed in a separate (third) seed box and place the seed (using the seed drill) on the soil surface.

OPTION: Broadcast Tall Drop Seed on the soil surface.

NO.	DATE	REVISIONS	BY	APPD
02	11-25-20	Updated Seeding / Sodding Periods Charts	M.R.D.	M.L.
01	08-03-20	Revised Standard	M.R.D.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION

PERMANENT SEEDING SUMMARY OF SEEDING QUANTITIES





LA850

DESIGNED	05-06-19	APPD.	Mervin Lare
DESIGN CK.	DETAIL CK.	QUANTITIES	TRACED
		QUAN.CK.	TRACE CK.

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STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
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SYMBOL KEY

-  REMOVE SIGN
-  REMOVE POST
-  REMOVE FOOTING
-  REMOVE SIGN & POST
-  REMOVE POST & FOOTING
-  REMOVE SIGN, POST, & FOOTING
-  MOUNT ON WOOD POST IN CONCRETE FOOTING
-  MOUNT ON WOOD POST IN SOIL
-  MOUNT ON STEEL BEAM BREAKAWAY POST
-  MOUNT ON STEEL U-POST
-  MOUNT ON PSST POST
-  MOUNT ON EXISTING POST
-  MOUNT ON VERTICAL SUPPORT
-  SHOULDER MOUNTED INSTALLATION
-  OFFSET MOUNTED INSTALLATION
-  EXISTING SIGN
-  EXISTING SIGN TO BE OVERLAID
-  SIGN IS NOT PART OF PROJECT
-  TYPE 'A' DELINEATOR (RIGID)
-  TYPE 'A' DELINEATOR (RIGID) (BK-BK)
-  TYPE 'B' DELINEATOR (RIGID)
-  TYPE 'A' DELINEATOR (FLEXIBLE)
-  TYPE 'A' DELINEATOR (FLEXIBLE) (BK-BK)
-  TYPE 'B' DELINEATOR (FLEXIBLE)
-  TYPE 2 OBJECT MARKER
-  TYPE 3 OBJECT MARKER
-  TYPE 3 OBJECT MARKER (BK-BK)

GENERAL NOTES

In order to expedite the completion of the project for traffic service, the signing and delineator work shall be sequenced with any other contract work such that the phases of construction may proceed and be completed at the same time.

New signs erected on the project which are in conflict with existing signing are to be completely covered until the existing signs are removed or the new signing is applicable. The existing signs that are being replaced, removed, or do not follow the current MUTCD signing standards are to be removed when the project is completed or as determined by the Engineer.

The Contractor shall exercise caution at all times when installing sign supports in and around areas where utilities exist, either underground or overhead, and will be held responsible for any damage incurred to the system. The installation of sign supports shall include the excavation, drilling, or driving the support footing and the erection of the sign support. The contractor shall exercise caution when working around any existing signs that are to remain and will be held responsible for any damage to the signs, supports, or footings. The Contractor shall exercise care when working around shrubbery while removing or installing signs or sign supports.

An existing sign post installation shall be plumb and the compaction of the backfill soil shall comply with the specifications after the removal and resetting of a sign, the removal and replacement of a sign, or the installation of a new sign.

The Contractor shall provide mounting bolts that are of a length that does not extend more than a nominal 1 inch beyond the sign post. The Contractor shall not make any field modifications to the mounting bolt prior to or after the sign is installed.

Specific service (LOGO) signs that are to be removed shall have the business logo plaques removed and transported to location determined by KDOT, at which time the plaques become the property of KDOT. The Contractor will be assessed a replacement cost for any damage to a business logo plaque prior to the plaque becoming the property of KDOT.

The materials and fabrication for signing and delineation work shall conform to the Standard Specifications for State Road and Bridge Construction (2015 edition) and Special Provisions.

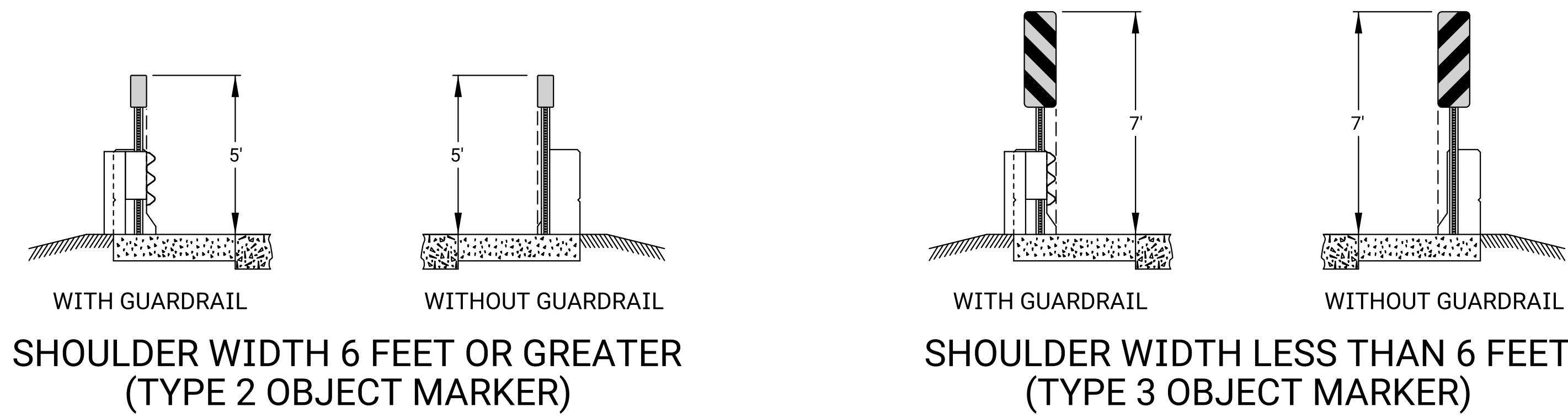
INDEX OF SHEETS

- 77 SIGNING INDEX, SYMBOLS, & GENERAL NOTES
POST SPACING & SIGN ANGLE DETAILS
HEIGHT & LATERAL DISTANCE FOR ERECTION
POSITIONING, DESIGN, & MOUNTING OF DELINEATORS
- 78-79 POSITIONING, DESIGN, & MOUNTING FOR OBJECT MARKERS (TYPE 2 & 3)
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PLAN SHEETS (INSTALLATIONS)
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QUANTITIES SHEETS (INSTALLATIONS)
- 80 QUANTITIES SHEET (DELINEATORS & OBJECT MARKERS)
- 81 SUMMARY SHEET (INSTALLATIONS & REMOVALS)
SUMMARY SHEET (REMOVAL & RESET)
- 82 RECAPITULATION SHEET
STANDARD STRUCTURAL SIGN SUPPORTS (WOOD & STEEL POSTS)
MOUNTING OF SIGNS ON WOOD POSTS
MOUNTING OF FLAT SHEET SIGNS ON STEEL I-BEAM POSTS
MOUNTING OF REINFORCED PANEL SIGNS ON I-BEAM POSTS
- 83 DETAILS FOR FLAT SHEET SIGN BLANKS
DETAILS FOR PROCESSED SIGNS
DETAILS FOR REINFORCED PANELS
DETAILS FOR GUIDE SIGN LEGEND
DETAILS FOR GUIDE SIGNS
- 84 DETAILED SIGN SPECIFICATIONS

Plotted by : xkwaters 5-JAN-2024 12:32
File : ka572801pss402-01.dgn

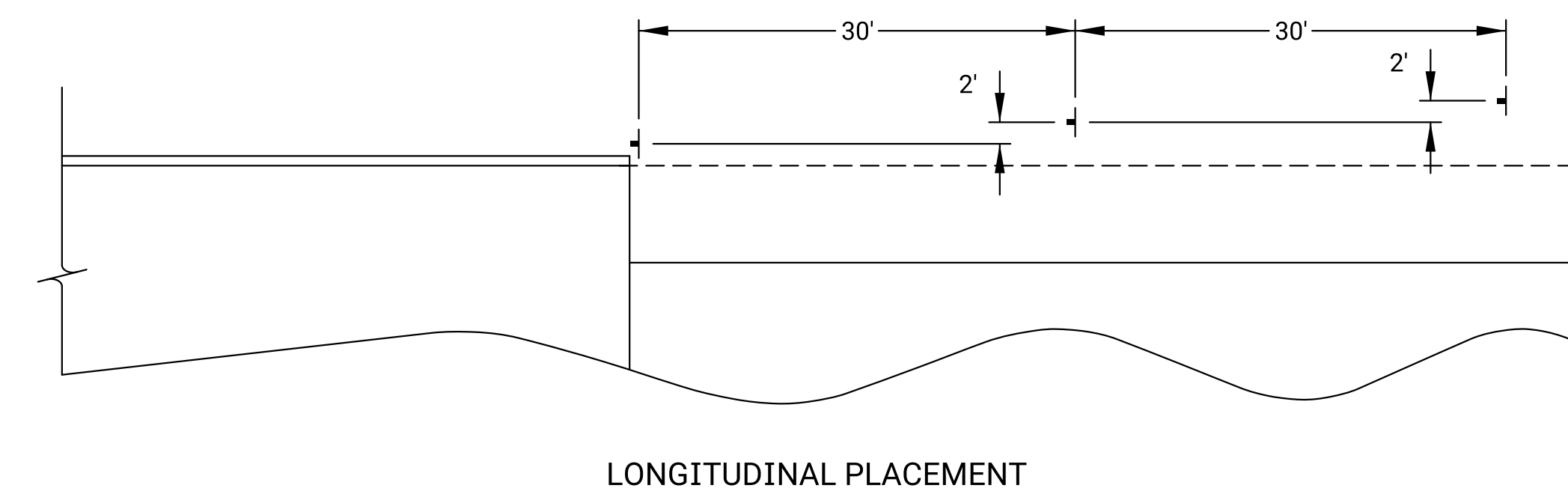
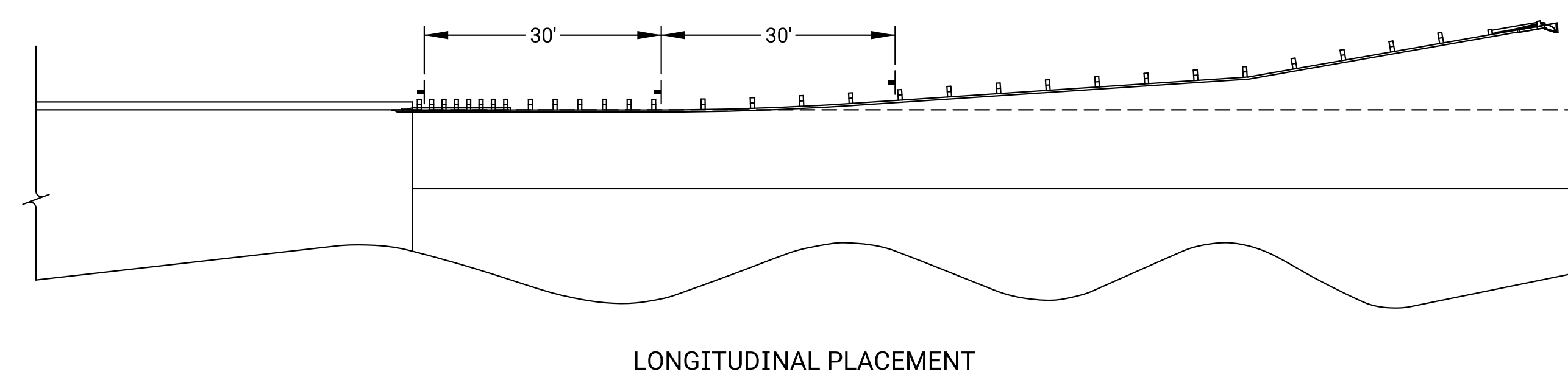
02	10-01-19	Changed symbol, notes, & index		D.D.G.	E.W.N.
01	07-23-10	Changed General Notes and Spec Book Date		D.D.G.	D.B.
NO.	DATE	REVISIONS		BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION					
SIGNING SYMBOL KEY GENERAL NOTES AND INDEX					
TE402				07-01-03	
FHWA APPROVAL		10-01-19		APPD	
DESIGNED	D.D.G.	DETAILED	W.S.B.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN. CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	78	135

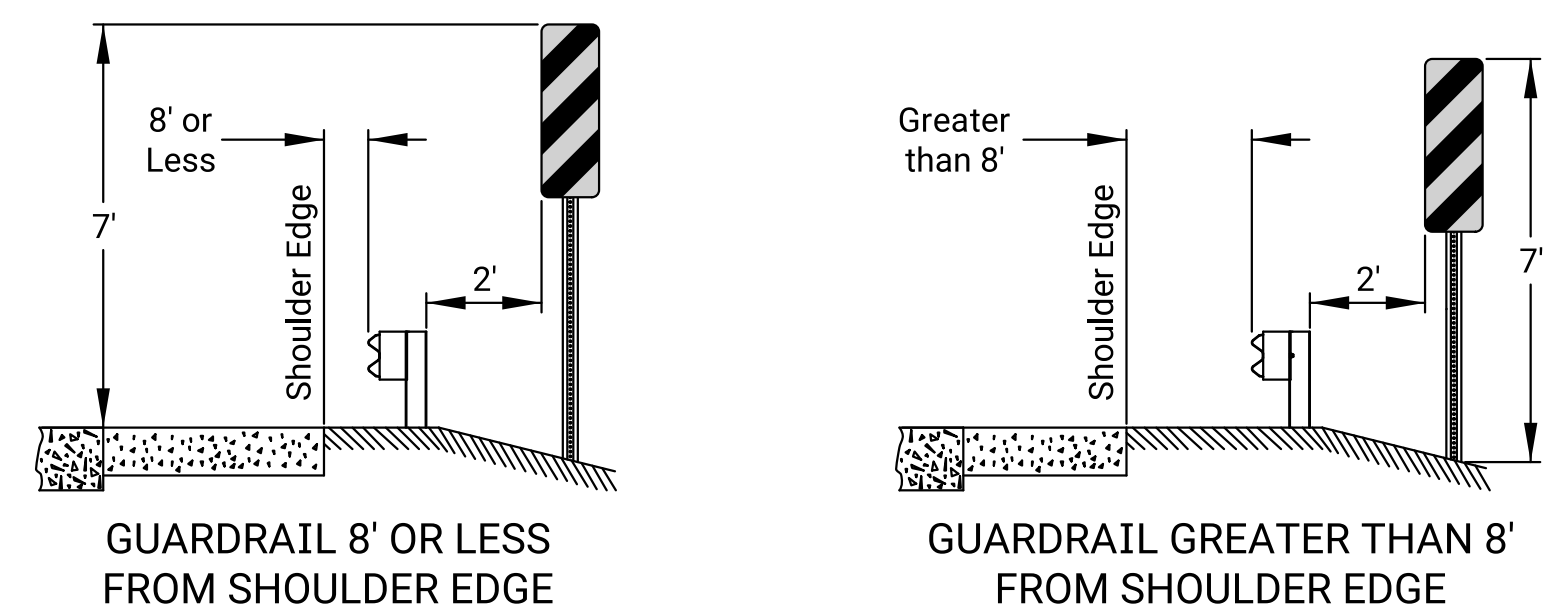


NOTE:
The longitudinal location of the object markers from the structure end shall be a maximum spacing of 42".

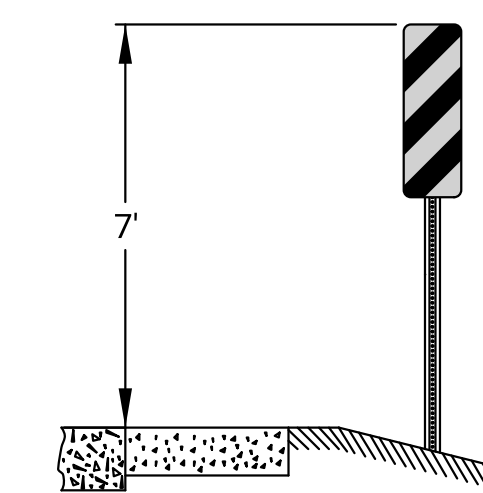
END OF STRUCTURE



NOTE:
The lateral offset is measured from the centerline of the object markers.



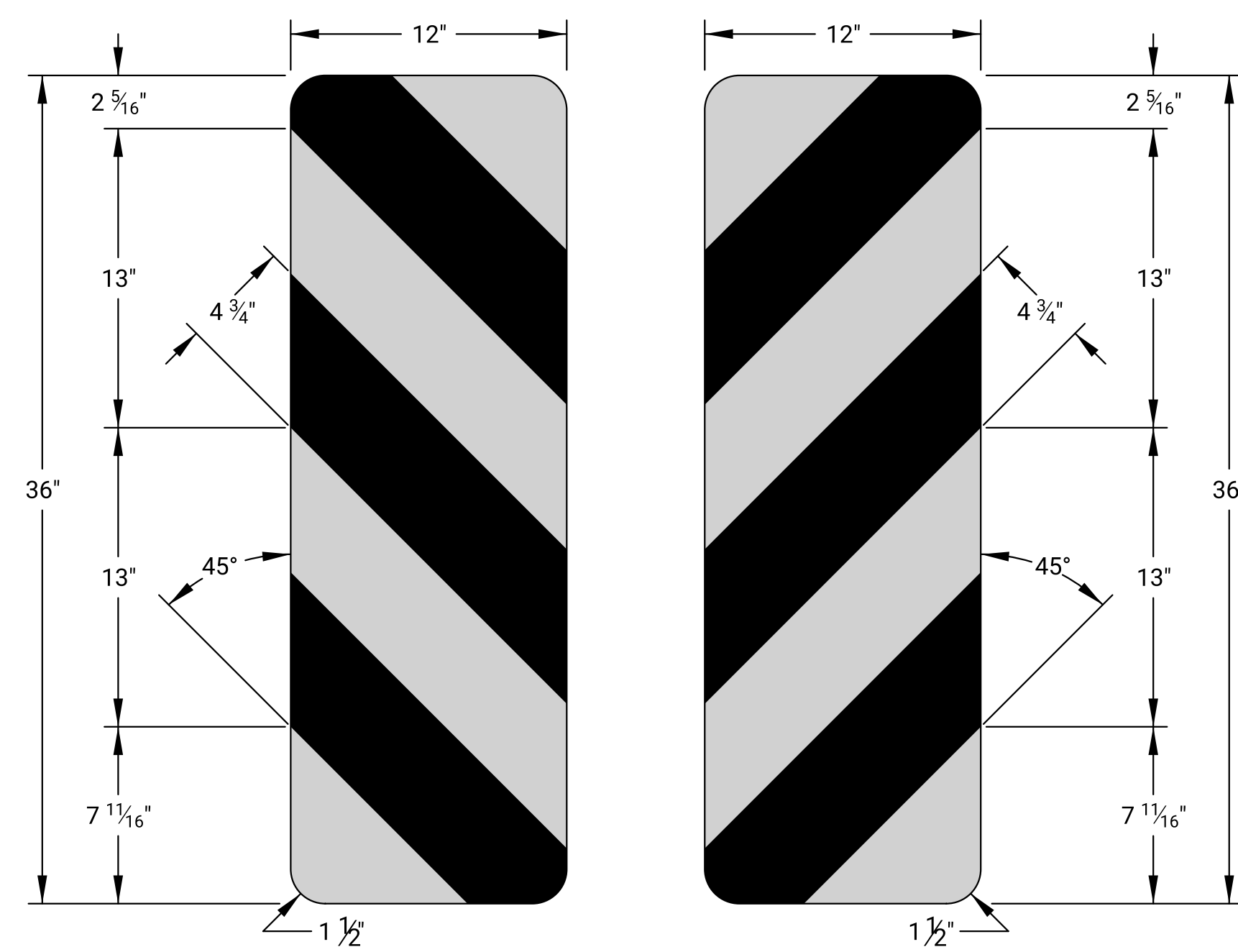
STRUCTURE APPROACH
GUARDRAIL WITHOUT MARKERS



STRUCTURE APPROACH
WITHOUT GUARDRAIL

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
DESIGN DETAILS FOR OBJECT MARKERS (TYPE 2 & 3) FOR STRUCTURES WITH PARAPETS				
TE415				10-01-19
FHWA APPROVAL	10-01-19	APPD.	Eric W. Nichol	
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN. CK.
				TRACED
				TRACE CK.

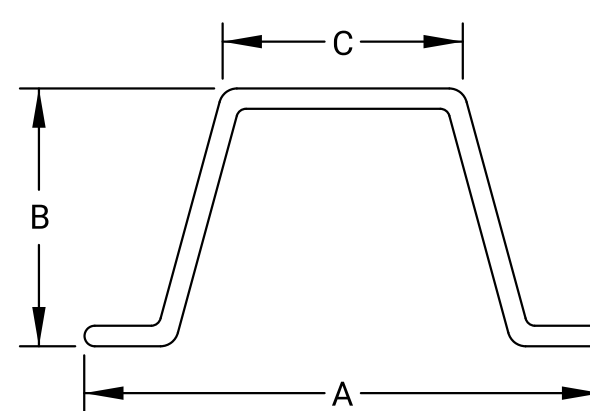
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	79	135



OM3-L

OM3-R

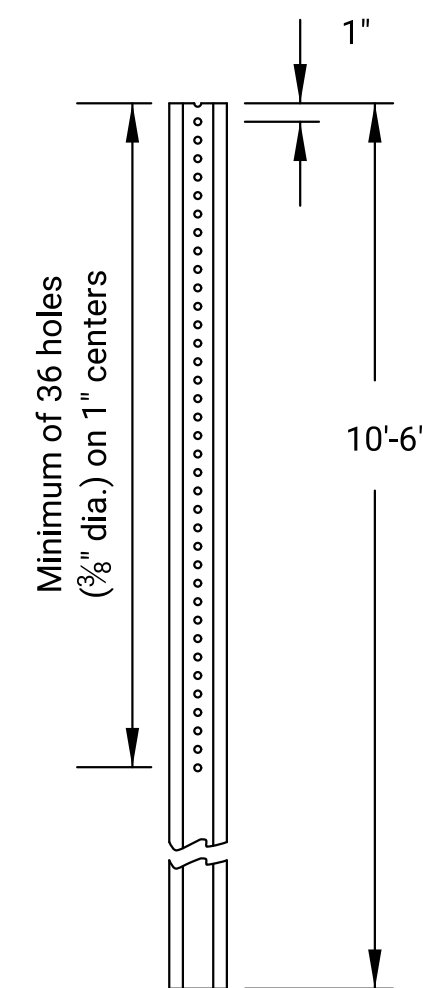
COLORS:
Yellow Background (Reflective)
Black Stripes (Non-reflective)



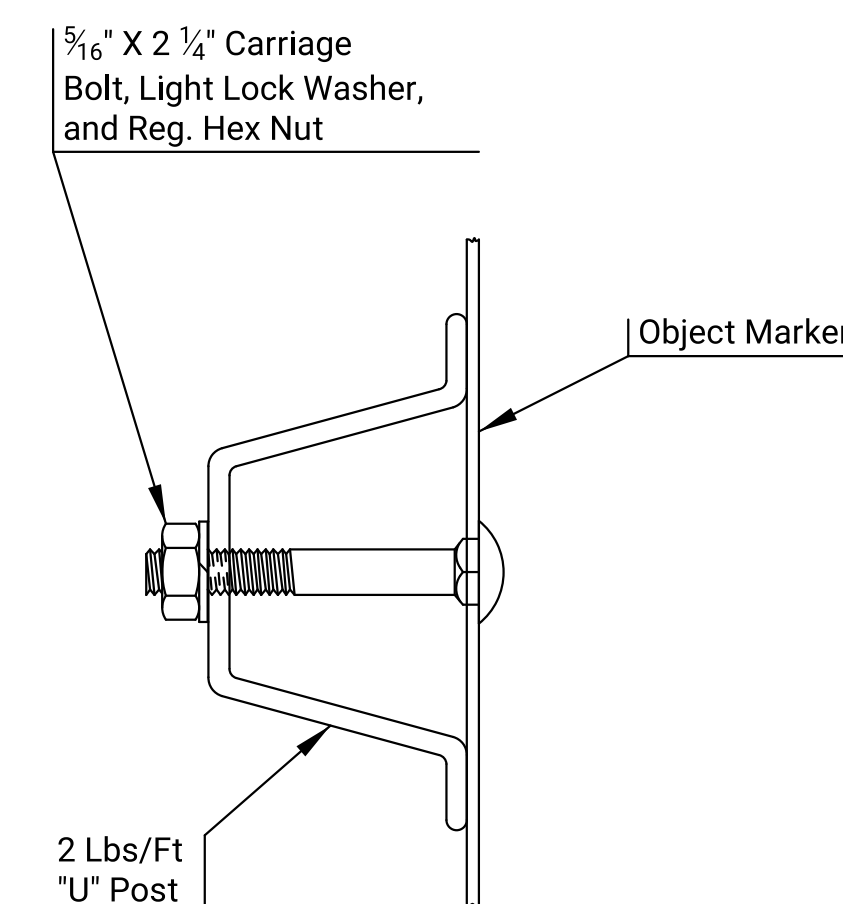
DIMENSIONS	
A	3 1/8
B	1 17/32
C	1 1/4

(Dimensions are nominal)

2 lb/ft "U" POST



PUNCHING DETAILS

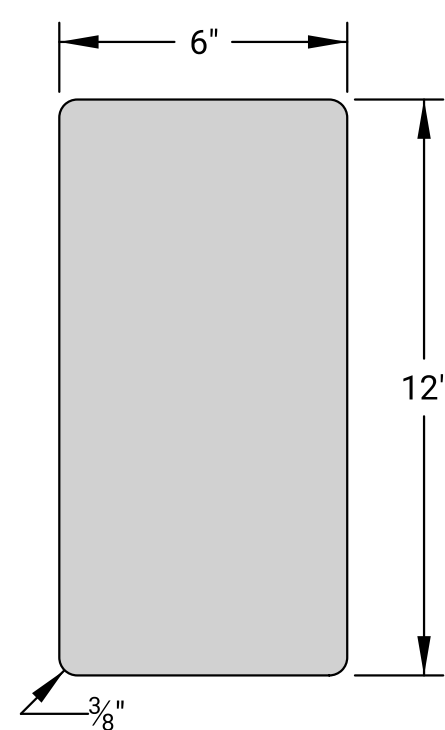


MOUNTING DETAILS

TYPE 3 OBJECT MARKER

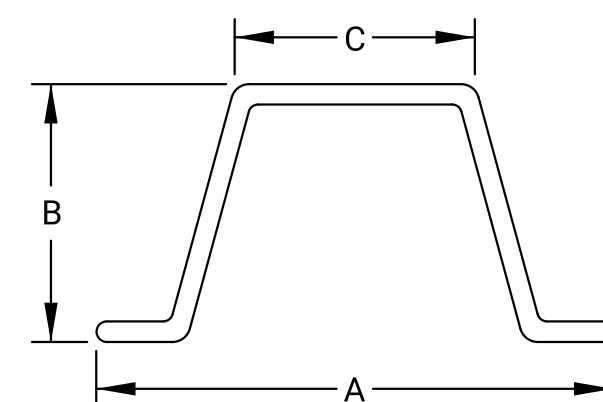
GENERAL NOTE:
See flat sheet sign blank standard sheets for the 6" x 12" and 12" x 36" sign blank details.

The object markers shall be covered with Type XI High Intensity yellow retroreflective sheeting.



OM2

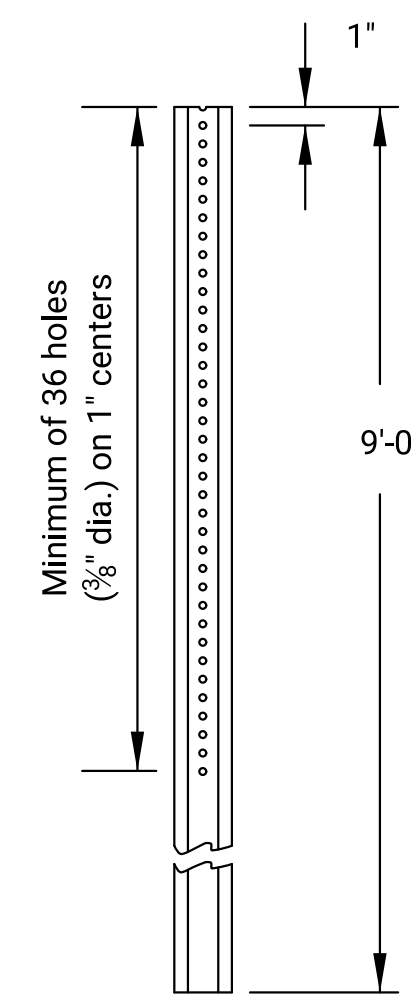
COLOR:
Yellow Background (Reflective)



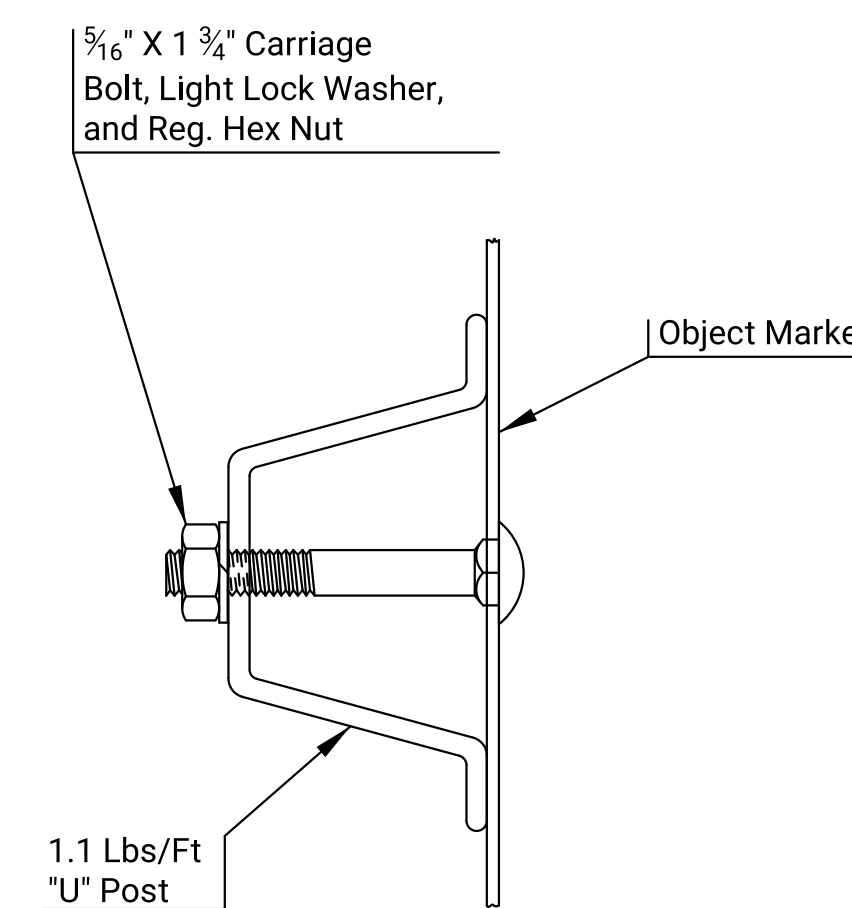
DIMENSIONS	
A	2 1/16
B	7/8
C	1 3/16

(Dimensions are nominal)

DELINEATOR POST
(1.1 lb/ft "U" Post)



PUNCHING DETAILS



MOUNTING DETAILS

TYPE 2 OBJECT MARKER

All dimensions are in inches unless otherwise noted.
See standard plan sheet TE590 for detailed specifications.

NO.	DATE	REVISIONS	BY	APPD.
KANSAS DEPARTMENT OF TRANSPORTATION				
DESIGN DETAILS FOR OBJECT MARKERS TYPE 2 AND TYPE 3				
TE416		10-01-19		
FHWA APPROVAL		10-01-19		APPD. Eric W. Nichol
DESIGNED	D.D.G.	DETAILED	D.D.G.	QUANTITIES
DESIGN CK.	E.W.N.	DETAIL CK.	E.W.N.	QUAN. CK.
				TRACED
				TRACE CK.

QUANTITIES SHEET
DELINEATORS AND OBJECT MARKERS

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	80	135

BEGINNING STATION	ENDING STATION	LOCATION DESCRIPTION	RIGID DELINEATORS												FLEXIBLE DELINEATORS												OBJECT MARKERS				
			TYPE 'A'								TYPE 'B'				TYPE 'A'								TYPE 'B'				TYPE 2	TYPE 3			
			WHITE		WHITE (BACK TO BACK)		YELLOW		YELLOW (BACK TO BACK)		WHITE		YELLOW		WHITE		WHITE (BACK TO BACK)		YELLOW		YELLOW (BACK TO BACK)		WHITE		YELLOW			LEFT	RIGHT	CENTER	BACK TO BACK
'U' POST	BRACKET MOUNT	'U' POST	BRACKET MOUNT	'U' POST	BRACKET MOUNT	'U' POST	BRACKET MOUNT	'U' POST	BRACKET MOUNT	'U' POST	BRACKET MOUNT	'U' POST	BRACKET MOUNT	TYPE I ANCHOR	TYPE III ANCHOR	TYPE I ANCHOR	TYPE III ANCHOR	TYPE I ANCHOR	TYPE III ANCHOR	TYPE I ANCHOR	TYPE III ANCHOR	TYPE I ANCHOR	TYPE III ANCHOR	TYPE I ANCHOR	TYPE III ANCHOR	'U' POST	'U' POST	'U' POST	'U' POST	'U' POST	
7374+00	7377+00	K-99 Mainline																										2	2		

Plotted by : xkwaters 5-JAN-2024 12:33
File : ka572801pss436-01.dgn

01	10-09-21	Added delineator & object marker types				D.D.G.	E.W.N.
NO.	DATE	REVISIONS				BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION							
QUANTITIES SHEET							
DELINEATORS & OBJECT MARKERS							
TE436				07-01-03			
FHWA APPROVAL		10-01-19		APPD.		Steven A. Buckley	
DESIGNED	D.D.G.	DETAILED	K.D.S.	QUANTITIES	TRACED		
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN.CK.	TRACE CK.		

SUMMARY OF QUANTITIES

SIGNS		
TYPE	NUMBER	SQUARE FEET
FLAT SHEET		
REINFORCED PANEL		
OVERLAY		

POSTS AND ALUMINUM BEAMS																
	4" x 6" POST			312.25 ALUMINUM BEAM	"U" POST		GALVANIZED STEEL BEAM POST				PERFORATED SQUARE STEEL TUBE (PSST)					
	WOOD		STEEL		2 LBS/FT	3 LBS/FT	W6x9		W10x12		W10x22		1-3/4"	2"	2-1/4"	2-1/2"
	FLAT SHEET SIGN	REINFORCED PANEL SIGN	STRUCTURAL TUBING				A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)				
NUMBER																
FEET																

DELINEATORS				
TYPE	FLEXIBLE DELINEATOR		RIGID DELINEATOR	
	TYPE I ANCHOR	TYPE III ANCHOR	"U" POST	BRACKET MOUNT
TYPE 'A' WHITE				
TYPE 'A' YELLOW				
TYPE 'B' WHITE				
TYPE 'B' YELLOW				
TYPE 'A' WHITE (BACK TO BACK)				
TYPE 'A' YELLOW (BACK TO BACK)				

OBJECT MARKERS			
TYPE	NUMBER		
TYPE 2 ("U" POST)			
TYPE 3 ("U" POST)			4
INFORMATION ONLY	OM3-L	2	X
	OM3-R	2	
	OM3-C		
TYPE 3 ("U" POST) (BACK TO BACK)			

POST FOOTINGS AND BRACKETS													
	CONCRETE FOOTING (DIA.)					PERFORATED SQUARE STEEL							
	WOOD	A36 STEEL		A572 STEEL (ALT)		TUBE FOOTING			BRACKET				
		18"	24"	30"	24"	30"	1-3/4"	2"	2-1/4"	2-1/2"	1-3/4"	2"	
NUMBER													
FEET													

NUMBER & LENGTHS OF POSTS & ALUMINUM BEAMS (INFORMATION ONLY)																
LENGTH OF POST OR BEAM	4" x 6" POST				312.25 ALUMINUM BEAM	"U" POST		GALVANIZED STEEL BEAM POST					PERFORATED SQUARE STEEL TUBE (PSST)			
	WOOD		STEEL	2 LBS/FT		3 LBS/FT	W6x9		W10x12		W10x22		1-3/4"	2"	2-1/4"	2-1/2"
	FLAT SHEET SIGN	REINFORCED PANEL SIGN	STRUCTURAL TUBING				A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)				
2.1' - 4'																
4.1' - 6'																
6.1' - 8'																
8.1' - 10'																
10.1' - 12'																
12.1' - 14'																
14.1' - 16'																
16.1' - 18'																
18.1' - 20'																
20.1' - 22'																
22.1' - 24'																
24.1' - 26'																
26.1' - 28'																
28.1' - 30'																
30.1' - 32'																

BASE PLATES AND STUB POSTS						
	W6x9		W10x12		W10x22	
	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)	A36 STEEL	A572 STEEL (ALT)
BREAKAWAY BASES						
BASE PLATE (TOP)						
STUB POST WITH BASE PLATE						
NON-BREAKAWAY BASES						
BASE PLATE						

REMOVALS	
TYPE	NUMBER
SIGNS	
POSTS	
FOOTINGS	
SIGN STRUCTURES	

SIGN STRUCTURES				
TYPE	NEW	MODIFIED	REMOVE AND RESET	RESET
OVERHEAD STRUCTURE				
CANTILEVER STRUCTURE				
BUTTERFLY STRUCTURE				
BRIDGE MOUNT ATTACHMENT				
MAST ARM SIGN SUPPORT				
SINGLE TAPERED TUBE SIGN SUPPORT				

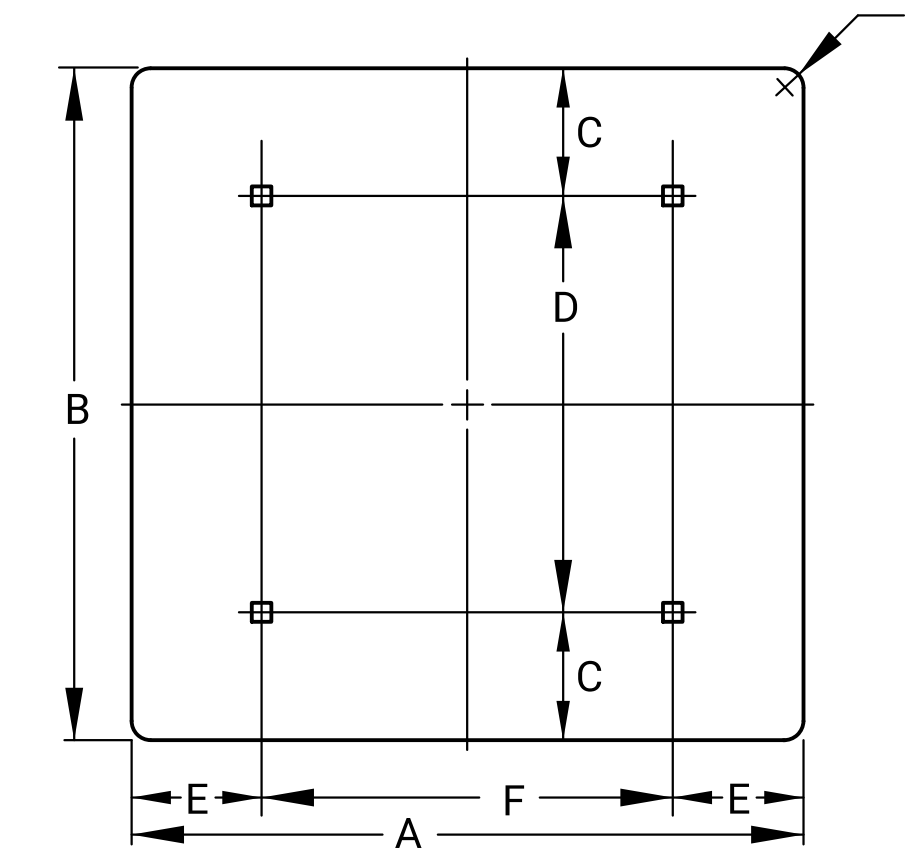
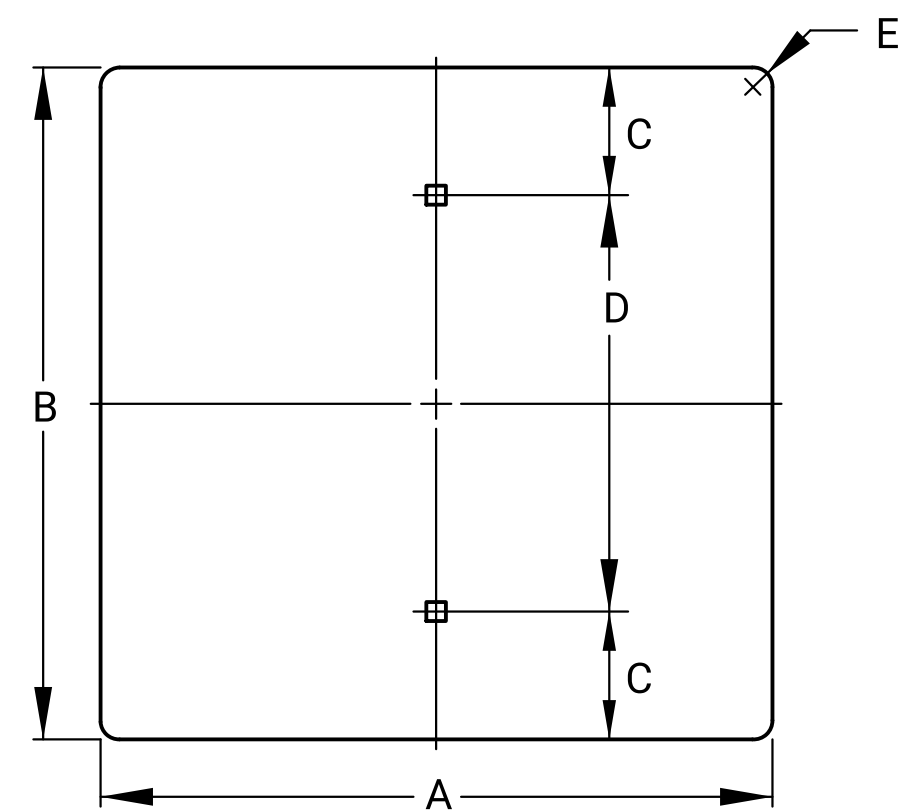
02	10-01-19	Revised Tables	D.D.G.	E.W.N.
01	07-23-10	Revised Tables	D.D.G.	D.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES FOR INSTALLATIONS AND REMOVALS

TE439 07-01-03

DESIGNED	D.D.G.	DETAILED	K.D.S.	QUANTITIES	TRACED
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN. CK.	TRACE CK.



	SIGN SIZE	A	B	C	D	E	T	AREA
①	3 X 8	3	8	1	6	3/8	0.040	0.17
①	6 X 12	6	12	3	6	3/8	0.063	0.50
	12 X 6	12	6	1 1/2	3	3/4	0.063	0.50
	12 X 9	12	9	1 1/2	6	1 1/2	0.063	0.75
	12 X 18	12	18	3	12	1 1/2	0.063	1.50
	12 X 24	12	24	3	18	1 1/2	0.080	2.00
	12 X 36	12	36	6	24	1 1/2	0.080	3.00
	12 X 48	12	48	6	36	1 1/2	0.080	4.00
	18 X 6	18	6	1 1/2	3	1 1/2	0.063	0.75
	18 X 18	18	18	3	12	1 1/2	0.063	2.25
	18 X 30	18	24	3	24	1 1/2	0.080	3.75
	18 X 36	18	24	6	24	1 1/2	0.080	4.50
	18 X 42	18	24	6	30	1 1/2	0.080	5.25
	18 X 48	18	24	6	36	1 1/2	0.080	6.00
	21 X 15	21	15	1 1/2	12	1 1/2	0.080	2.19
	24 X 6	24	6	1 1/2	3	1 1/2	0.080	1.00
	24 X 12	24	12	3	6	1 1/2	0.080	2.00
	24 X 18	24	18	3	12	1 1/2	0.080	3.00
	24 X 24	24	24	3	18	1 1/2	0.080	4.00
	24 X 30	24	30	3	24	1 1/2	0.080	5.00
	24 X 36	24	36	6	24	1 1/2	0.080	6.00
	30 X 12	30	12	3	6	1 7/8	0.080	2.50
	30 X 15	30	15	1 1/2	12	1 7/8	0.080	3.13
	30 X 18	30	18	3	12	1 7/8	0.080	3.75
	30 X 21	30	21	1 1/2	18	1 1/2	0.080	4.38
	30 X 24	30	24	3	18	1 7/8	0.080	5.00
	30 X 30	30	30	3	24	1 7/8	0.080	6.25
	30 X 36	30	36	6	24	1 7/8	0.080	7.50
	36 X 12	36	12	3	6	1 1/2	0.080	3.00
	36 X 18	36	18	3	12	1 1/2	0.080	4.50
	36 X 24	36	24	3	18	1 1/2	0.080	6.00
	36 X 30	36	30	3	24	2 1/4	0.080	7.50
	36 X 36	36	36	6	24	2 1/4	0.080	9.00
③	45 X 36	45	36	3	30	2 1/4	0.100	11.25

	SIGN SIZE	A	B	C	D	E	F	G	T	AREA
	36 X 12	36	12	3	6	3	30	1 1/2	0.080	3.00
	36 X 30	36	30	3	24	3	30	2 1/4	0.080	7.50
	36 X 48	36	48	9	30	6	24	0	0.100	12.00
	36 X 60	36	60	12	36	6	24	0	0.100	15.00
②	36 X 72	36	72	6	60	6	24	0	0.100	18.00
	42 X 12	48	12	3	6	6	30	1 1/2	0.080	3.50
	42 X 18	48	18	3	12	6	30	1 1/2	0.080	5.25
	42 X 24	48	24	6	12	6	30	1 7/8	0.080	7.00
	42 X 36	48	36	6	24	6	30	0	0.100	10.50
	48 X 12	48	12	3	6	9	30	1 1/2	0.080	4.00
	48 X 18	48	18	3	12	9	30	1 1/2	0.080	6.00
	48 X 24	48	24	6	12	9	30	1 7/8	0.080	8.00
	48 X 30	48	30	6	18	9	30	0	0.100	10.00
	48 X 36	48	36	6	24	9	30	0	0.100	12.00
	48 X 42	48	42	6	30	9	30	0	0.100	14.00
	48 X 48	48	48	9	30	9	30	0	0.100	16.00
	48 X 60	48	60	12	36	9	30	0	0.100	20.00
②	48 X 72	48	72	6	60	9	30	0	0.100	24.00
②	48 X 96	48	96	12	72	9	30	0	0.100	32.00
	60 X 12	60	12	3	6	12	36	0	0.100	5.00

	SIGN SIZE	A	B	C	D	E	F	G	T	AREA
	60 X 18	60	18	3	12	12	36	0	0.100	7.50
	60 X 24	60	24	6	12	12	36	0	0.100	10.00
	60 X 30	60	30	6	18	12	36	0	0.100	12.50
	60 X 36	60	36	6	24	12	36	0	0.100	15.00
	60 X 42	60	42	6	30	12	36	0	0.100	17.50
	60 X 48	60	48	9	30	12	36	0	0.100	20.00
	72 X 12	72	12	3	6	15	42	0	0.100	6.00
	72 X 18	72	18	3	12	15	42	0	0.100	9.00
	72 X 24	72	24	6	12	15	42	0	0.100	12.00
	72 X 30	72	30	6	18	15	36	0	0.100	15.00
	72 X 36	72	36	6	24	15	42	0	0.100	18.00
	72 X 42	72	42	6	30	15	42	0	0.100	21.00
	72 X 48	72	48	9	30	15	42	0	0.100	24.00
	84 X 12	84	18	3	6	18	48	0	0.100	7.00
	84 X 18	84	18	3	12	18	48	0	0.100	10.50
	84 X 24	84	24	6	12	18	48	0	0.100	14.00
	84 X 30	84	30	6	18	18	48	0	0.100	17.50
	84 X 36	84	36	6	24	18	48	0	0.100	21.00
	84 X 42	84	42	6	30	18	48	0	0.100	24.50
	84 X 48	84	48	9	30	18	48	0	0.100	28.00

NOTE:

All holes are 3/8" square, unless otherwise noted.

The dimension "T" is the thickness of the aluminum blank.

- ① Holes shall be 5/16" diameter.
- ② Dimension "D" requires a center hole.
- ③ Additional hole 12" below top hole.

All dimensions are in inches.

01		10-01-19	Updated sign blank details and dimensions				D.D.G.	E.W.N.
NO.	DATE	REVISIONS				BY	APPD.	
KANSAS DEPARTMENT OF TRANSPORTATION								
SIGN BLANK DETAILS FOR FLAT SHEET SIGNS								
TE506						07-01-03		
DESIGNED		D.D.G.		10-01-19		APPD. Steven A. Buckley		
DETAILER		D.D.G.		A.A.D.		QUANTITIES TRACED		
DESIGN CK.		S.A.B.		D.D.G.		QUAN. CK. TRACE CK.		

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	84	135

DETAILED SPECIFICATIONS FOR FLAT SHEET SIGNS AND OVERLAY PANELS

All new flat sheet sign blanks shall be of the fabrication and thickness shown on the flat sheet blank detail sheets, unless other details are shown in the plans.

Flat sheet blanks shall be used for signs that are less than or equal to 7'-0" in length and/or less than or equal to 4'-0" in height, unless other details are shown in the plans. Flat sheet blanks shall also be used for signs that are 4'-0" in length and less than or equal to 8'-0" in height, unless other details are shown in the plans.

The design details for signs (color, letter height, and letter series) shall be as shown in the FHWA Standard Highway Signs and Markings book (2004 edition and supplements), unless other details are shown in the plans.

All sign faces shall be covered with Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The sheeting used for the direct applied legend and borders shall be Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The school warning signs, the "SCHOOL" portion of the S5-1 sign, S4-3p plaque, and any supplemental plaques used with these warning signs shall have a fluorescent yellow-green background, unless otherwise noted in the plans.

The type of adhesive used for retroreflective sheeting or lettering film shall be heat activated or pressure sensitive.

DETAILED SPECIFICATIONS FOR REINFORCED PANEL SIGNS

All new reinforced sign panels shall be of the fabrication and thickness shown on the reinforced panel detail sheets. If extrusheet fabricated sign panels are used, they shall be of the length, width and in the position shown. If extrusheet fabricated panel dimensions are not shown, a line of legend should be placed entirely on one panel. If extruded fabricated sign panels are used, either 1'-0" or 6" panels shall be used. The 6" panels shall be used only at the top or bottom of signs.

Reinforced panels shall be used for signs that are greater than 7'-0" in length or greater than 4'-0" in height, unless other details are shown in the plans.

All sign faces shall be covered with Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The sheeting used for the direct applied legend and borders shall be Type IV high intensity retroreflective sheeting, unless otherwise noted in the plans.

The type of adhesive used for retroreflective sheeting or lettering film shall be heat activated or pressure sensitive.

Letters and numbers on reinforced panel signs are modified Series "E" unless otherwise shown.

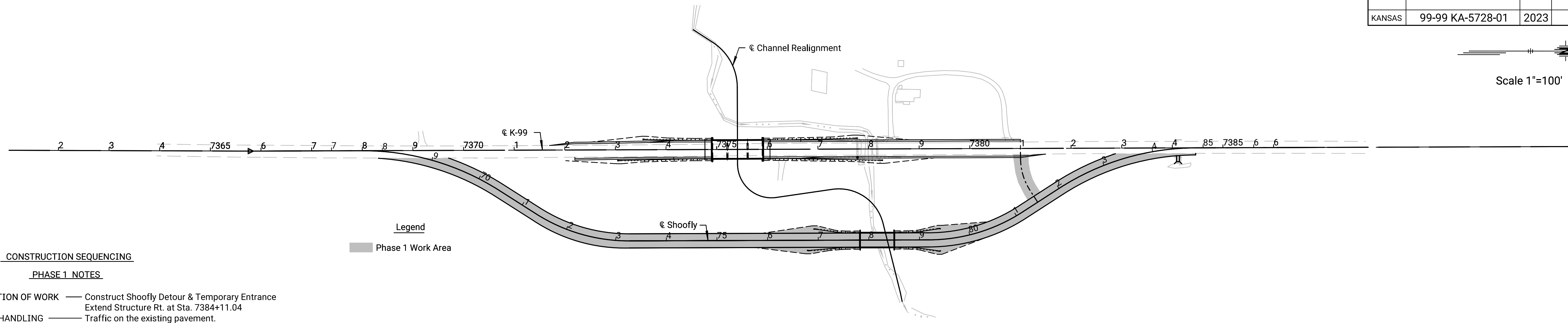
Spacing table dimensions are in inches.

Plotted by : xkwaters 5-JAN-2024 12:33
File : ka572801pss590-01.dgn

02	10-01-19	Changed notes	D.D.G.	E.W.N.
01	07-23-10	Changed Notes and Sheeting Type	D.D.G.	D.B.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
DETAILS SPECIFICATIONS FOR REINFORCED SIGN PANELS AND FLAT SHEET SIGNS				
TE590			07-01-03	
FHWA APPROVAL		10-01-19	APPD. Steven A. Buckley	
DESIGNED	D.D.G.	DETAILED	K.D.S.	QUANTITIES
DESIGN CK.	S.A.B.	DETAIL CK.	D.D.G.	QUAN. CK.
			TRACED	
			TRACE CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	85	135

Scale 1"=100'



CONSTRUCTION SEQUENCING

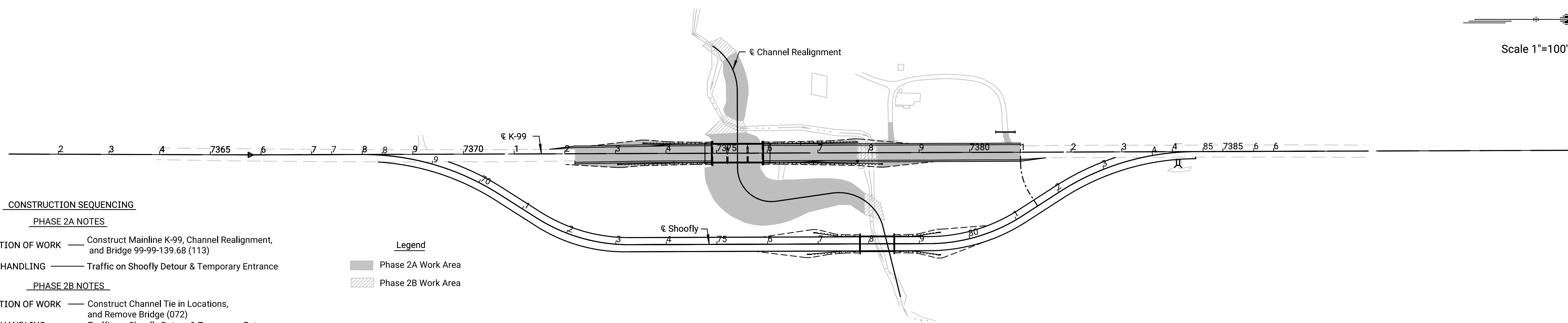
PHASE 1 NOTES

DESCRIPTION OF WORK — Construct Shoofly Detour & Temporary Entrance
Extend Structure Rt. at Sta. 7384+11.04
TRAFFIC HANDLING — Traffic on the existing pavement.

Legend
■ Phase 1 Work Area

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED

Scale 1"=100'



CONSTRUCTION SEQUENCING

PHASE 2A NOTES

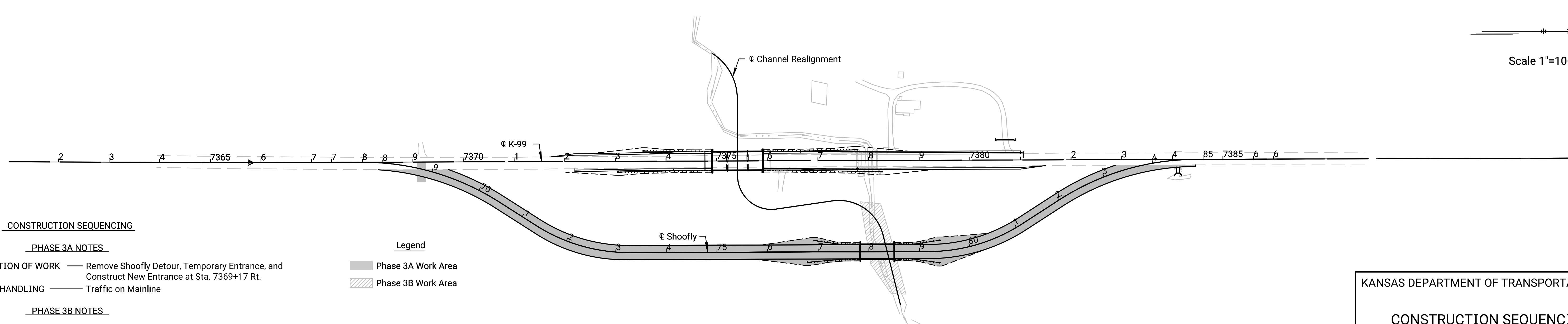
DESCRIPTION OF WORK — Construct Mainline K-99, Channel Realignment,
and Bridge 99-99-139.68 (113)
TRAFFIC HANDLING — Traffic on Shoofly Detour & Temporary Entrance

PHASE 2B NOTES

DESCRIPTION OF WORK — Construct Channel Tie in Locations,
and Remove Bridge (072)
TRAFFIC HANDLING — Traffic on Shoofly Detour & Temporary Entrance

Legend
■ Phase 2A Work Area
▨ Phase 2B Work Area

Scale 1"=100'



CONSTRUCTION SEQUENCING

PHASE 3A NOTES

DESCRIPTION OF WORK — Remove Shoofly Detour, Temporary Entrance, and
Construct New Entrance at Sta. 7369+17 Rt.
TRAFFIC HANDLING — Traffic on Mainline

PHASE 3B NOTES

DESCRIPTION OF WORK — Construct Channel Tie in.
TRAFFIC HANDLING — Traffic on Mainline

Legend
■ Phase 3A Work Area
▨ Phase 3B Work Area

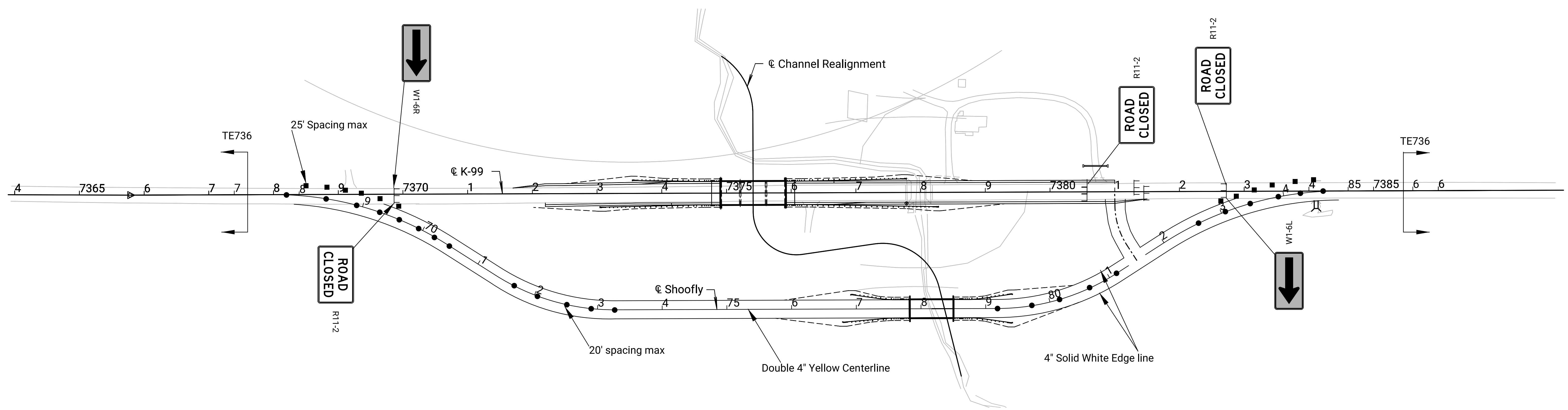
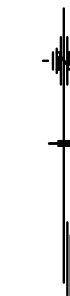
KANSAS DEPARTMENT OF TRANSPORTATION
CONSTRUCTION SEQUENCING

Plotted by : August Zuno 05-JAN-2024 14:31
File : ka572801trcs-01.dgn

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	86	135

Notes: Any signs not shown, refer to TE736



NOT TO SCALE

- Bi-Directional Temp. Raise Pavement Markers (Type I)
- ▬▬ Type III Barricades
- Channelizing Device

KANSAS DEPARTMENT OF TRANSPORTATION			
Traffic Control Phase II Shoofly			
APPD DESIGNED	DETAILED	DESIGN CK.	DETAIL CK.

Plotted by: Robert.Barrtron@ks.gov 4-JAN-2024 18:01
File: cpi-01.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	87	135

1) Design Speed: Those items delegated to temporary traffic control should be designed and installed using the posted/legal speed of the roadway prior to work starting.

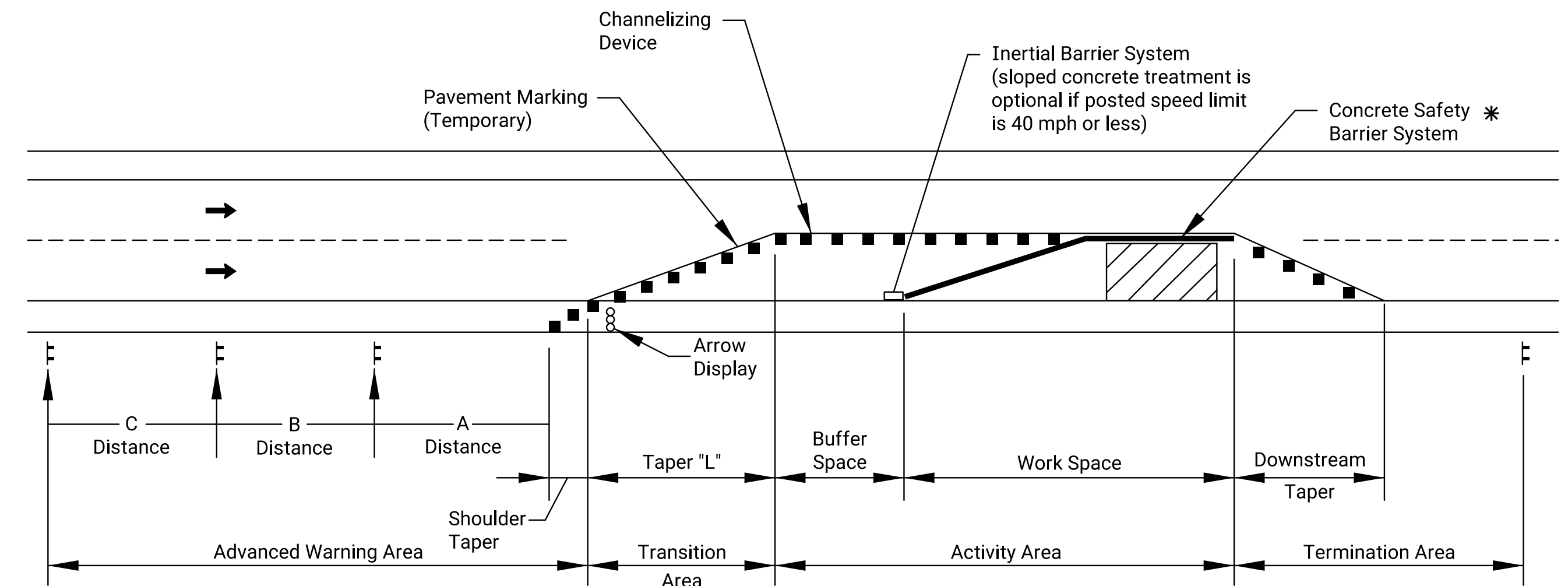
2) Minimum Lane Width: Lane widths shall be a minimum of 11' (measured between centerlines of pavement markings) or as shown on the plans, or as directed by the engineer. A lane width less than 11' may require restricted roadway width signing.

3) Consideration should be made to separate pedestrian and, if needed, bicycle movements from both work site activity and vehicular traffic. Unless a reasonable safe route that does not involve crossing the roadway can be provided, pedestrians should be appropriately directed with advance signing that encourages them to cross to the opposite side of the roadway. In urban and suburban areas with high vehicular traffic volumes, these signs should be placed at intersections (rather than midblock locations) so that pedestrians are not confronted with midblock work sites that will induce them to attempt skirting the work site or making a midblock crossing.

4) When existing pedestrian facilities are disrupted, closed, or relocated, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility.

5) When the driving surface open to traffic is milled or is a temporary surface made of loose material, or when directed by the engineer a W8-15 (Grooved Pavement) or W8-7 (Loose Gravel) sign shall be used on mainline approaches. This sign should be placed a "C" distance after the W20-1 (Road Work Ahead) sign. A W8-15p motorcycle plaque shall be used to supplement the W8-15 or W8-7 signs. All signs shall be displayed as long as the condition is present.

6) Alternative temporary rumble strip options may be available. Please contact the Temporary Traffic Control Unit for more information at 785-296-1179 or 785-296-1183.



TYPICAL WORK ZONE COMPONENTS

* When concrete barrier system is used, portable channelizing devices are not needed along the tangent barrier section.

Minimum advance warning sign spacing (in feet):

SPEED (MPH) *	A	B	C
URBAN (40 MPH OR LOWER)	100	100	100
URBAN (45 MPH OR HIGHER)	350	350	350
RURAL (55 MPH OR LOWER)	500	500	500
RURAL (60 MPH OR HIGHER)	750	750	750
EXPRESSWAY/FREEWAY	1000	1500	2640

* Posted speed prior to work starting
The minimum spacing between signs shall be no less than 100', unless directed by the engineer.
The spacing between any signs may be increased beyond the minimum values in the table above as approved by the engineer in order to maximize visibility.

Taper Formulas:

$L = WS$ for speeds of 45 MPH or more
 $L = WS^2/60$ for speeds of 40 MPH or less

Where: L = Minimum length of taper in feet
 S = Numerical value of posted speed prior to work starting in MPH
 W = Width in offset feet

Shifting Taper = 1/2 L
Shoulder Taper = 1/3 L

Channelizer Placement:

- The spacing between devices in transition area (taper) should not exceed a distance in feet equal to 1/2 the posted speed limit in mph prior to work starting.
- The spacing between devices in the advanced warning area and the activity area should not exceed a distance in feet equal to two times the posted speed limit in mph prior to work starting.
- Channelizing devices shall be placed for optimum visibility, normally at right angles to the traffic flow.
- Place directional indicator barricades in series to direct traffic onto the new path. The arrow sign should not be visible to opposing traffic.
- Alternating diagonal orange and white striping must slope downward in the direction traffic is expected to pass.

Buffer Space

SPEED (MPH) *	20	25	30	35	40	45	50	55	60	65	70	75
LENGTH (ft)	115	155	200	250	305	360	425	495	570	645	730	820

* Posted speed prior to work starting

Neither work activity nor storage of equipment, vehicles, or material should occur in the buffer space. When a protection vehicle is placed in advance of the work space, only the space upstream of the vehicle constitutes the buffer space.

If temporary concrete safety barrier system is used to separate approaching traffic from the work space, the barrier system shall be considered part of the activity area. A full lane width should be available throughout the length of the buffer space. See typical work zone components above.

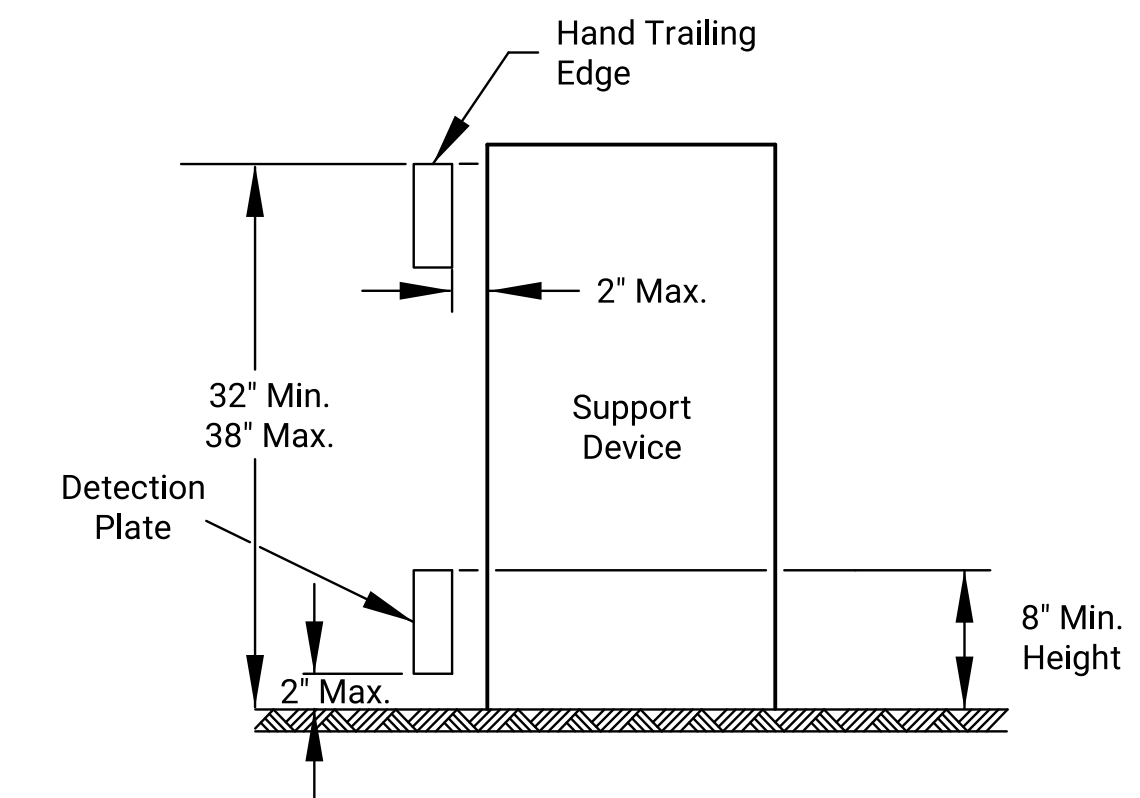
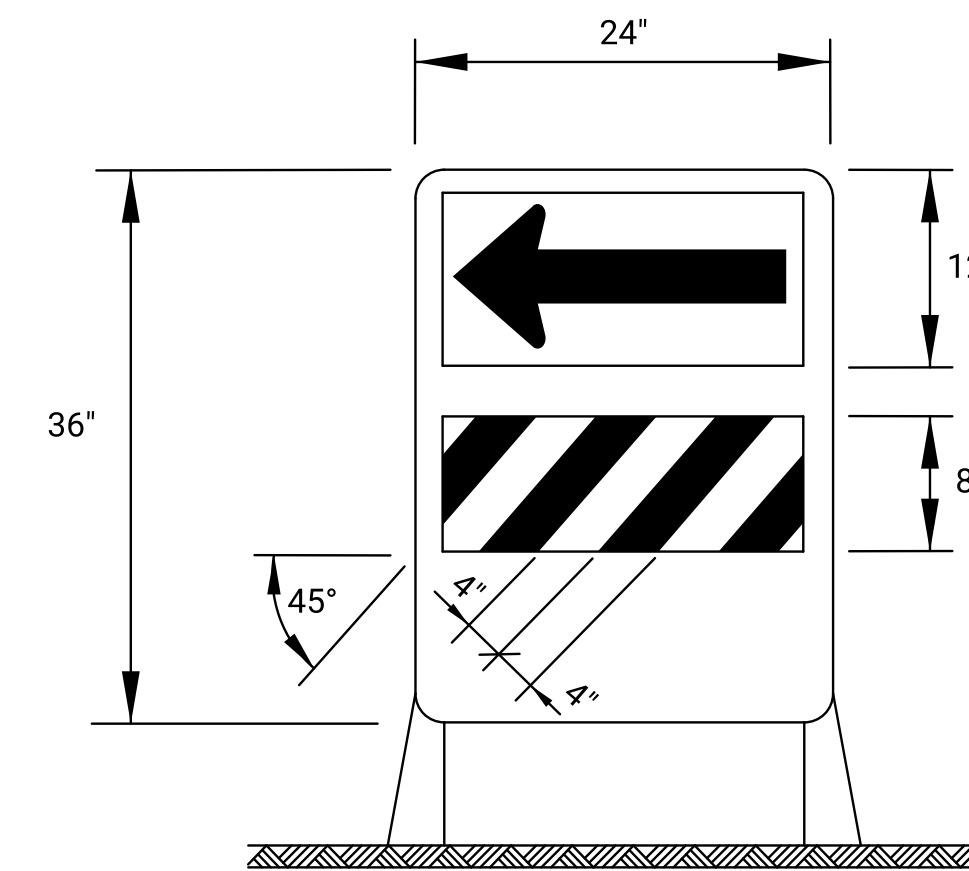
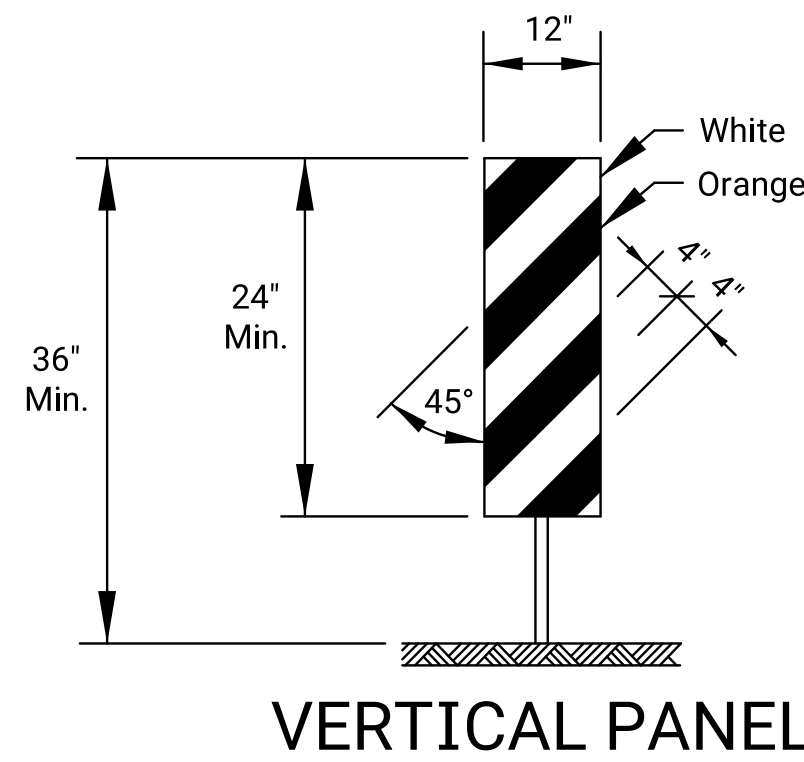
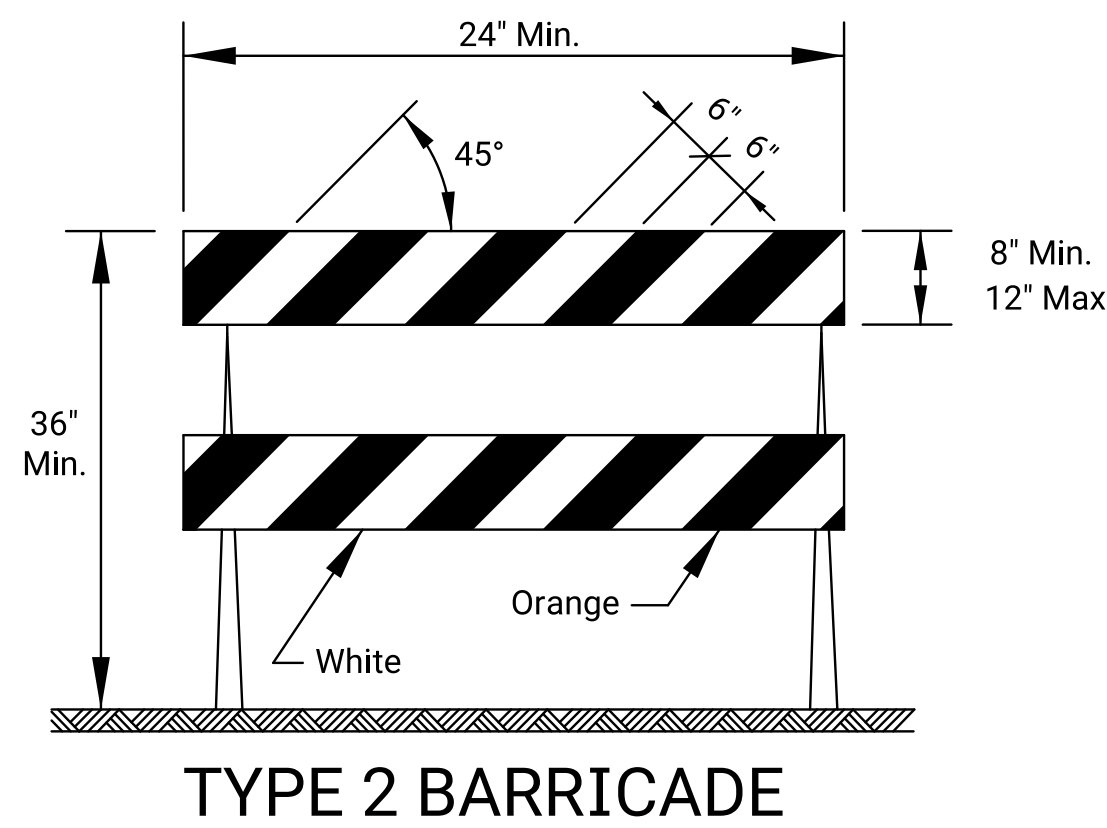
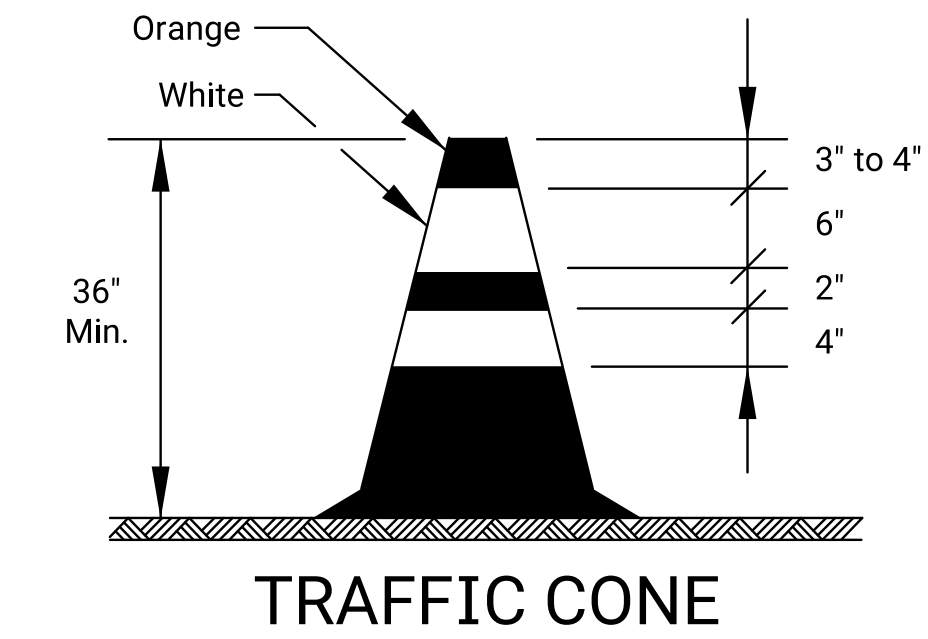
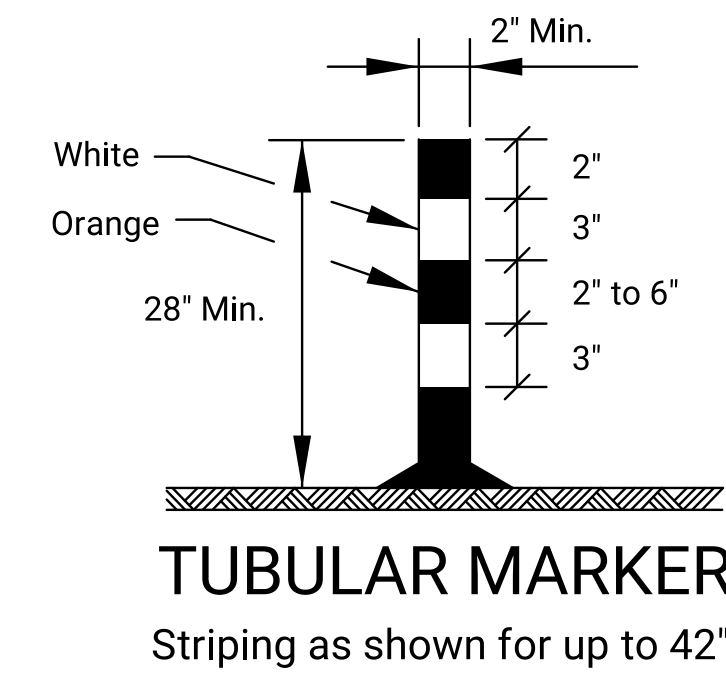
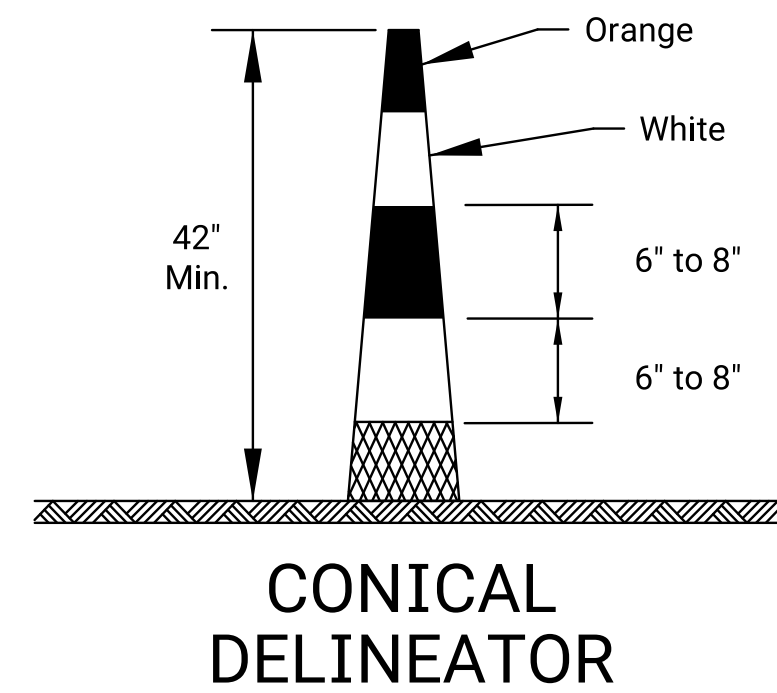
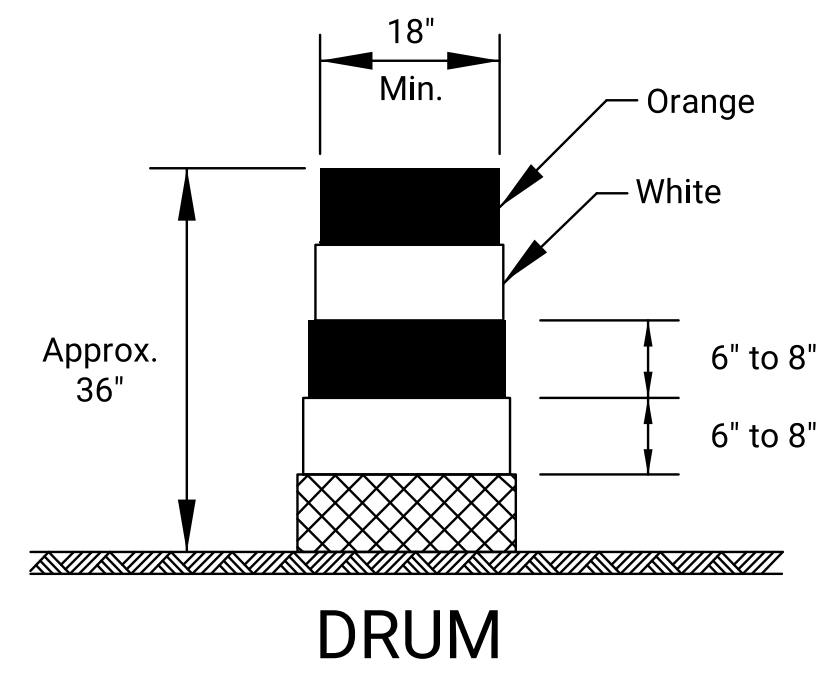
NO.	DATE	REVISIONS	BY	APPROD.
02	03-13-18	W8-15p usage changed to Shall	R.W.B.	E.K.G.
01	08-18-15	Channelizer spacing info	R.W.B.	K.E.

KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL GENERAL NOTES

TE700

DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		



For rails less than 36" long, 4" wide stripes may be used. All stripes shall slope downward to the traffic side for channelization.

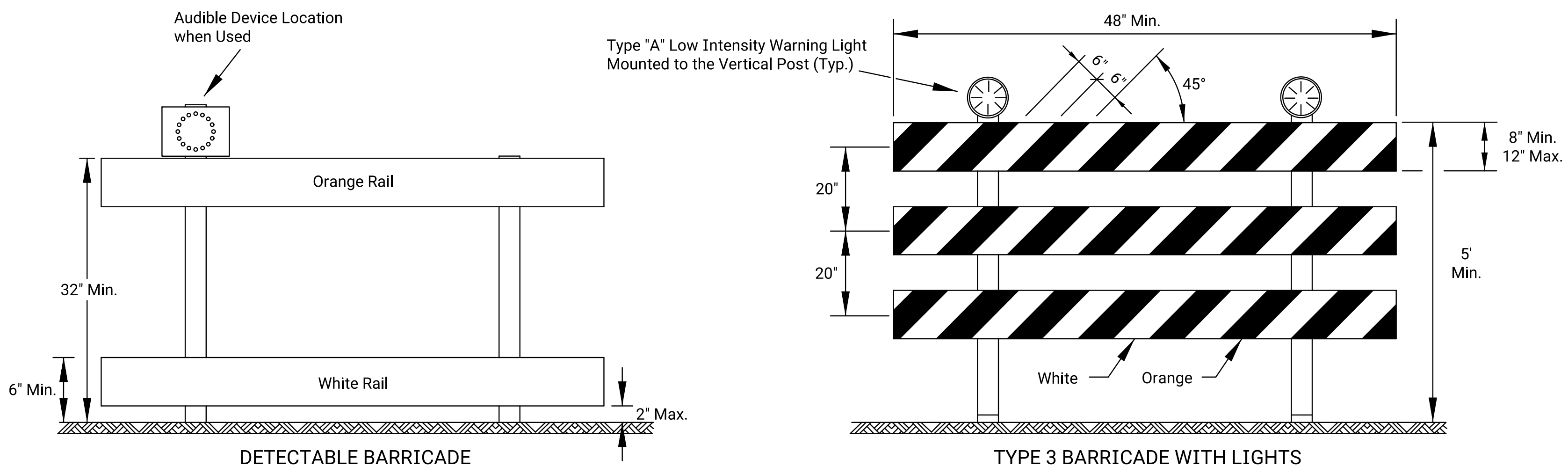
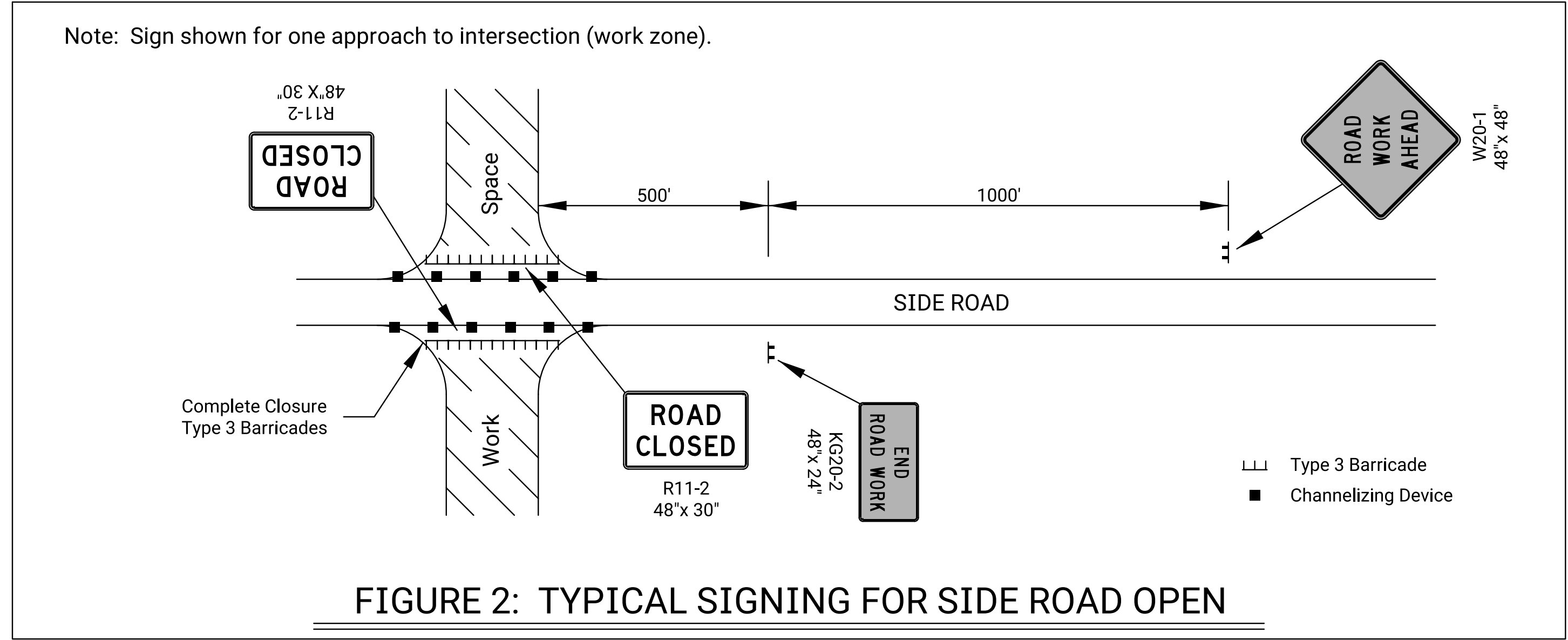
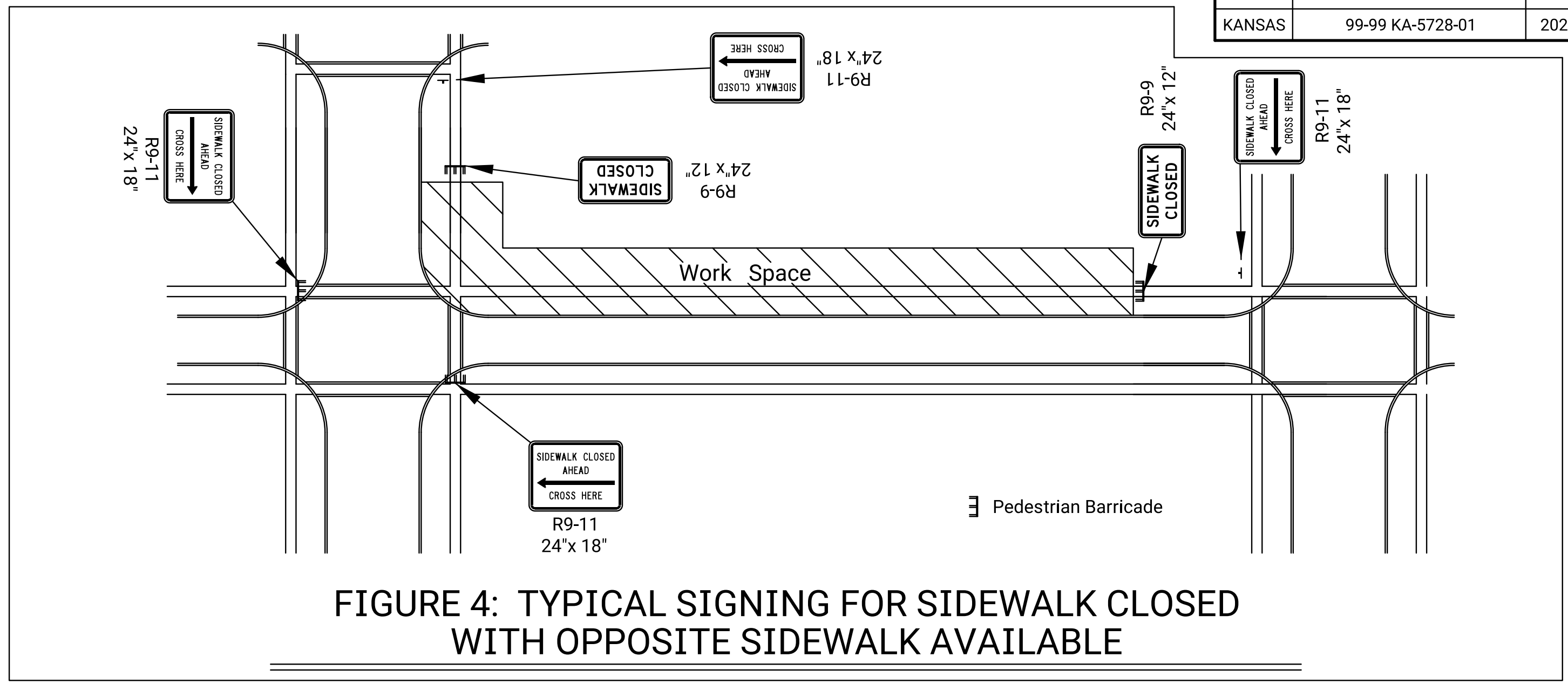
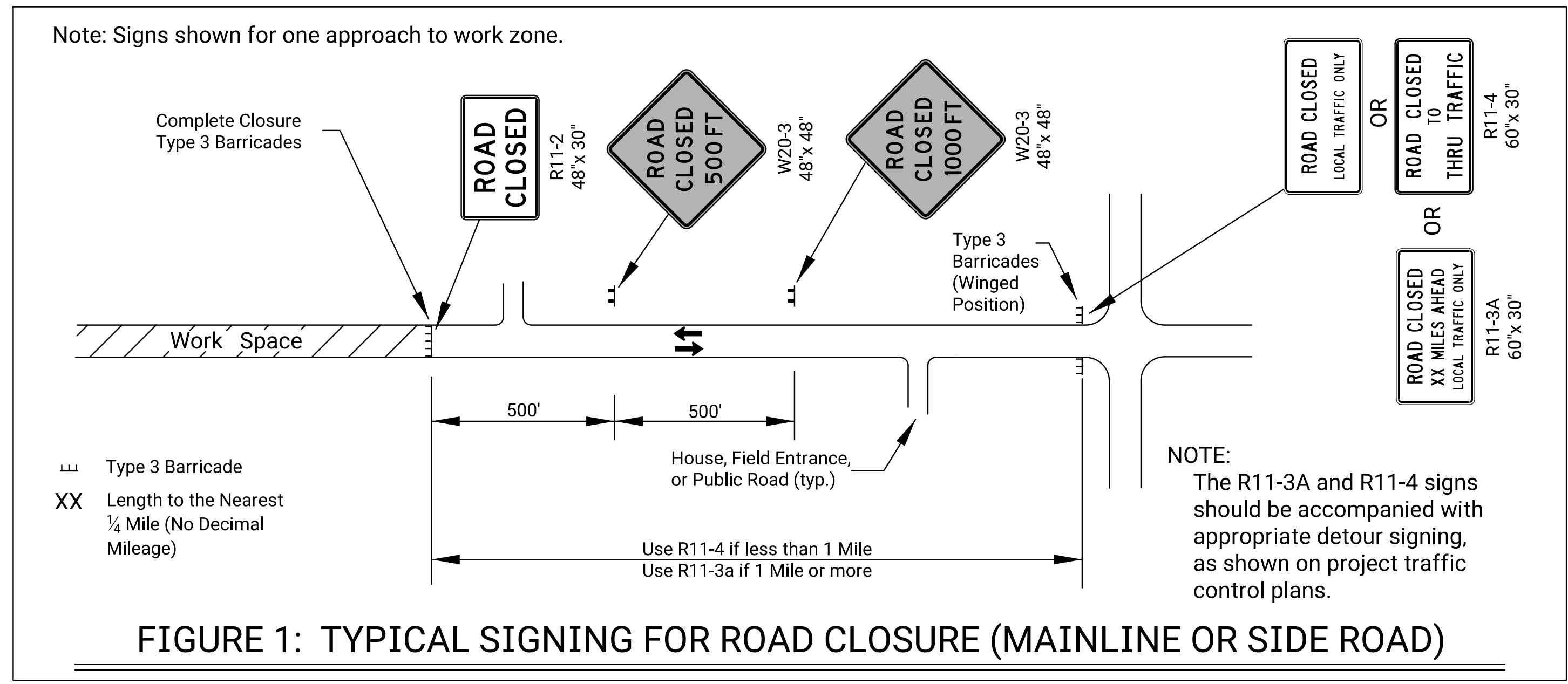
The stripes shall slope downward to the traffic side for channelization.

The stripes shall slope downward in the direction traffic is to pass. The direction indicator barricade shall be used in series to direct the motorist into the intended lane of travel.

- Support device shall not project beyond the detection plate into the pathway.
- Hand trailing edges and detection plates are optional for continuous walls.
- Interconnect pedestrian channelizers to prevent displacement and to provide continuous guidance through or around work.
- Alternate pathways shall be firm, stable, and slip resistant.
- Treat height differentials > 1/2" in the surfaces of alternate paths with a firm, stable, and slip resistant temporary ramp having a slope of 12:1 or flatter and having a width equal to the alternate path.
- Use alternating orange/white on interconnected devices.

Item	Location									
		Cross-overs	Shoofly Divisions	Tangents	Tapers	Ramps	Head to Head	Object Identifier	Lead-in Devices	Gores
Portable	Drums	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes
	Conical Delineators	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes
	Vertical Panels	(2)	(2)	(2)	(2)	(2)	(1,2)	Yes	(2)	(2)
	Direction Indicator Barricade	No	No	No	Yes	No	No	No	No	No
	Type 2 Barricade	(2)	(2)	(2)	(2)	No	No	Yes	No	No
Fixed	Traffic Cones	No	No	(4)	(4)	(4)	No	(4)	(4)	(4)
	Tubular Markers	(3)	(3)	(3)	No	(3)	Yes	No	Yes	Yes
	Vertical Panels	(3)	(3)	(3)	(3)	(3)	(3)	Yes	(2,3)	(2)

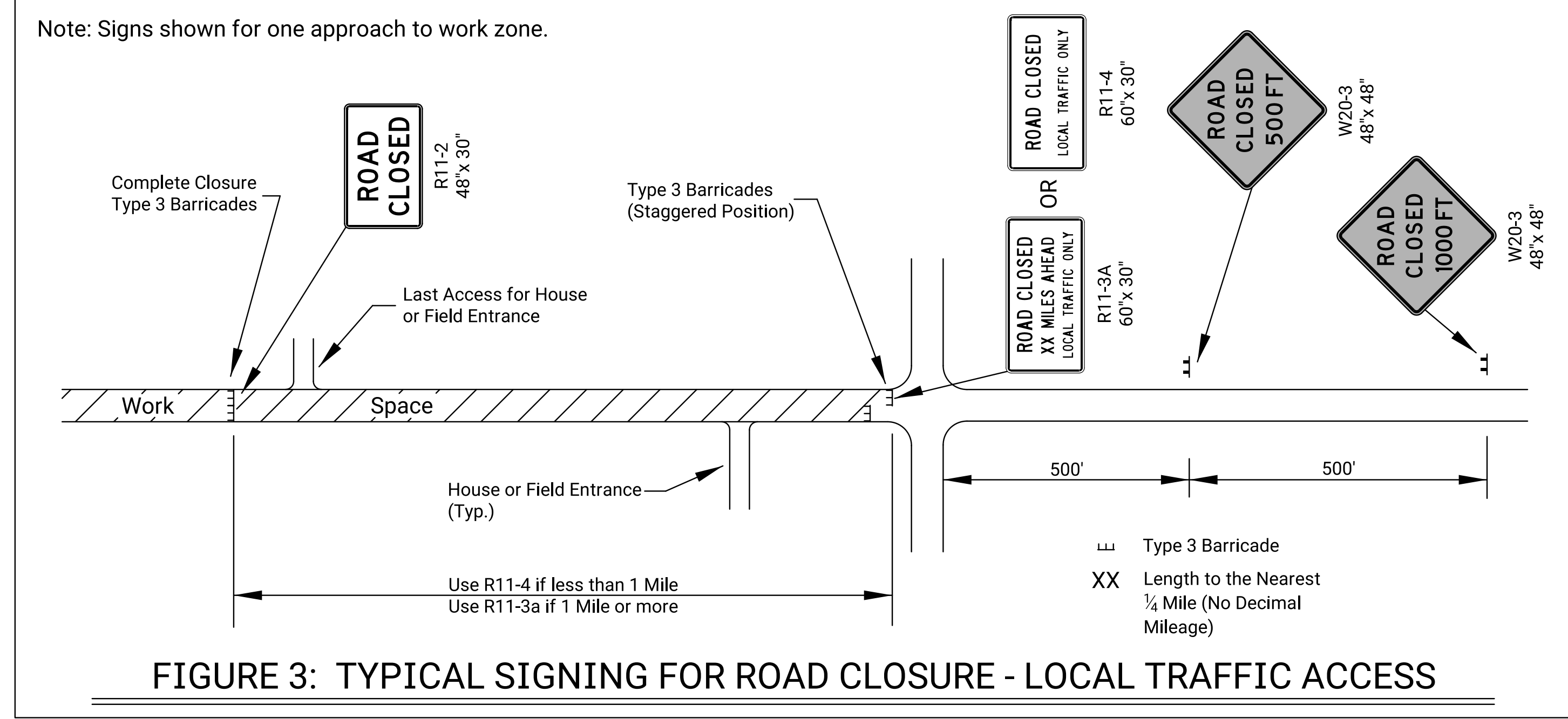
- Not allowed on centerline delineation along freeways or expressways.
- The stripes shall slope downward to the traffic side for channelization.
- May be used upon the approval of the engineer.
- Daytime operations only.



1. Support device shall not project beyond the detection plate into the pathway.
2. Barricades shall be used to close the entire width of the pathway.
3. Do not use warning lights on pedestrian barricades.
4. Do not use warning lights on audible devices.

Approved signs mounted on Type 3 barricades should not cover more than 50% of the top two rails or 33% of the total area of the three rails.

When barricades are placed end-to-end or staggered, a Type "A" low intensity warning light shall be mounted to the vertical post near each outside corner of the end barricades.



ROAD CLOSED GENERAL NOTES

As shown in Figure 1, at the point where thru traffic must detour and local traffic can proceed to the location where the roadway is completely closed, the R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) or R11-4 (ROAD CLOSED LOCAL TRAFFIC ONLY or ROAD CLOSED TO THRU TRAFFIC) sign shall be used with Type 3 barricades (winged position), placed on the shoulders of roadway.

As shown in Figure 3, when local traffic must be allowed access into the work zone, Type 3 barricades shall be longitudinally staggered to maintain the appearance of a closed roadway. A second line of end-to-end Type 3 barricades shall be placed just beyond the last access point in the work zone, to completely close the roadway.

The R11-4 (ROAD CLOSED TO THRU TRAFFIC or ROAD CLOSED LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is less than 1 mile.

The R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is 1 mile or greater.

The words "BRIDGE OUT" (or BRIDGE CLOSED) may be substituted for the words "ROAD CLOSED" on the R11-3a or R11-4 sign where applicable.

Plotted by : Robert.Barrton@ks.gov 4-JAN-2024 18:01
File : TE704.dgn

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL CLOSURES				
TE704				
DESIGNED	B.A.H.	06-01-15	APPD.	Kristina Erickson
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACED	TRACE CK.
DOT Graphics Certified 07-18-2022 Sh. No. 89				

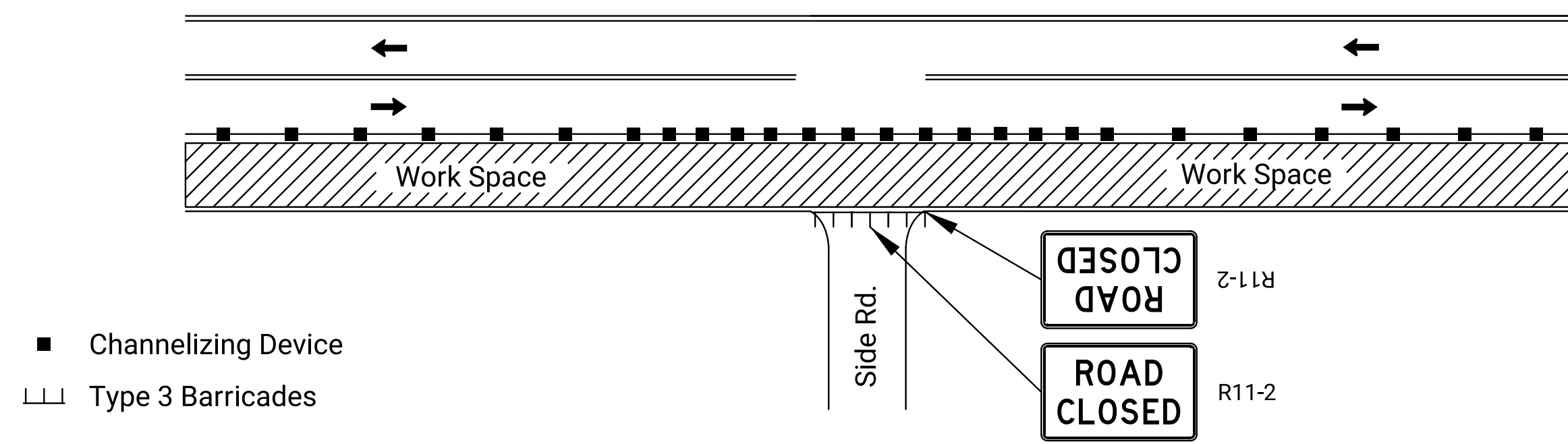
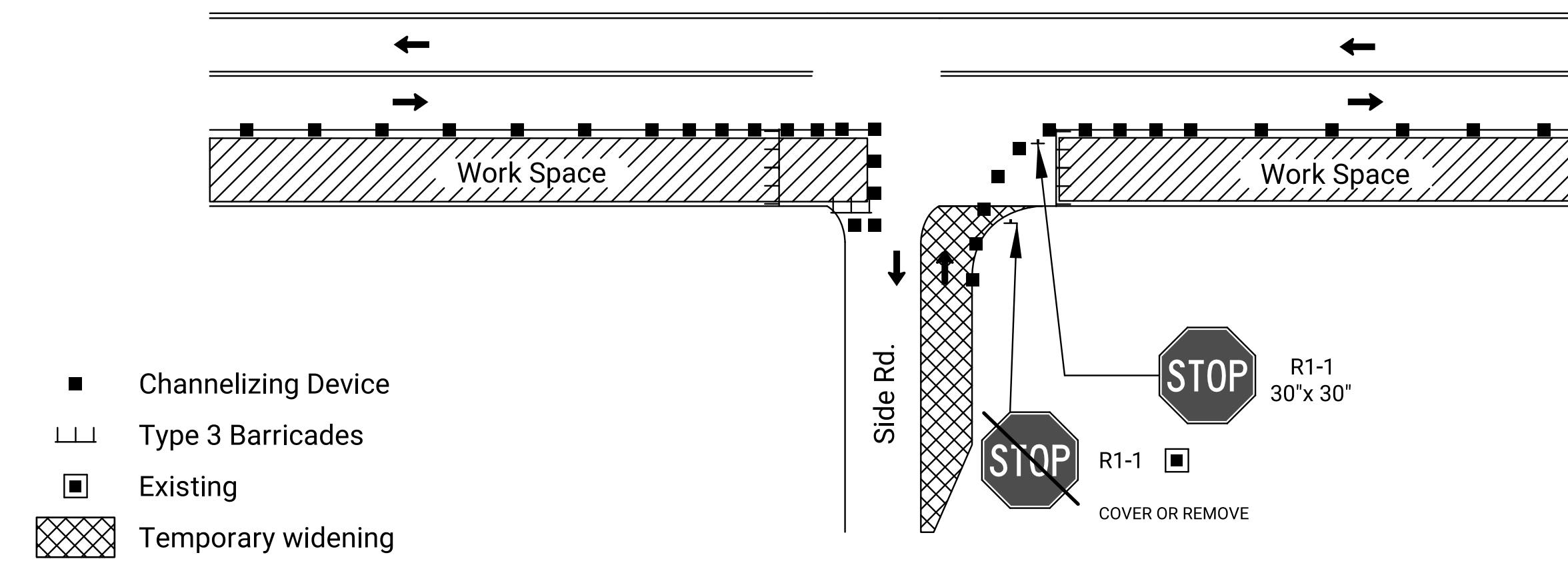


FIGURE 1: SIDE ROAD OR ENTRANCE CLOSED THROUGH WORK AREA



**FIGURE 4: SIDE ROAD OR ENTRANCE CONSTRUCTED HALF AT A TIME:
TWO WAY TRAFFIC REQUIRED**

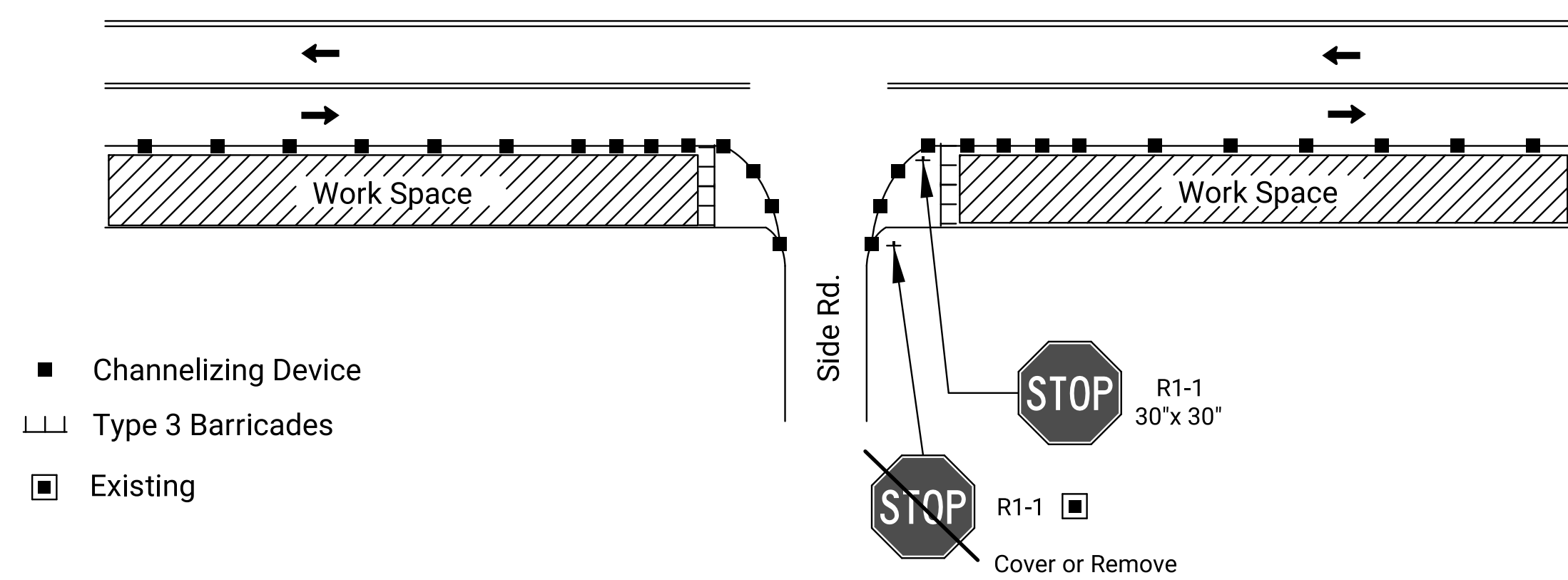


FIGURE 2: SIDE ROAD OR ENTRANCE OPEN THROUGH WORK AREA

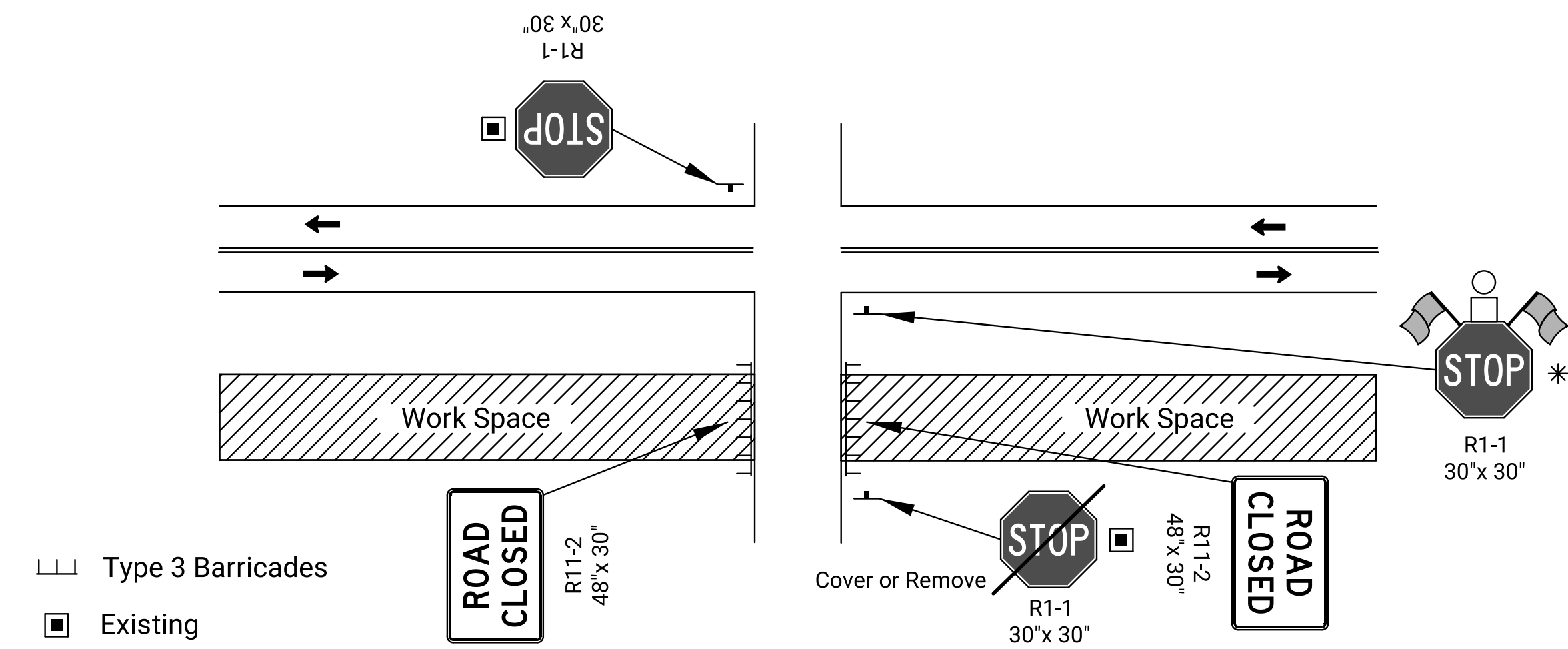


FIGURE 5: SIDE ROAD OPEN THROUGH WORK AREA ON DIVIDED ROADWAY

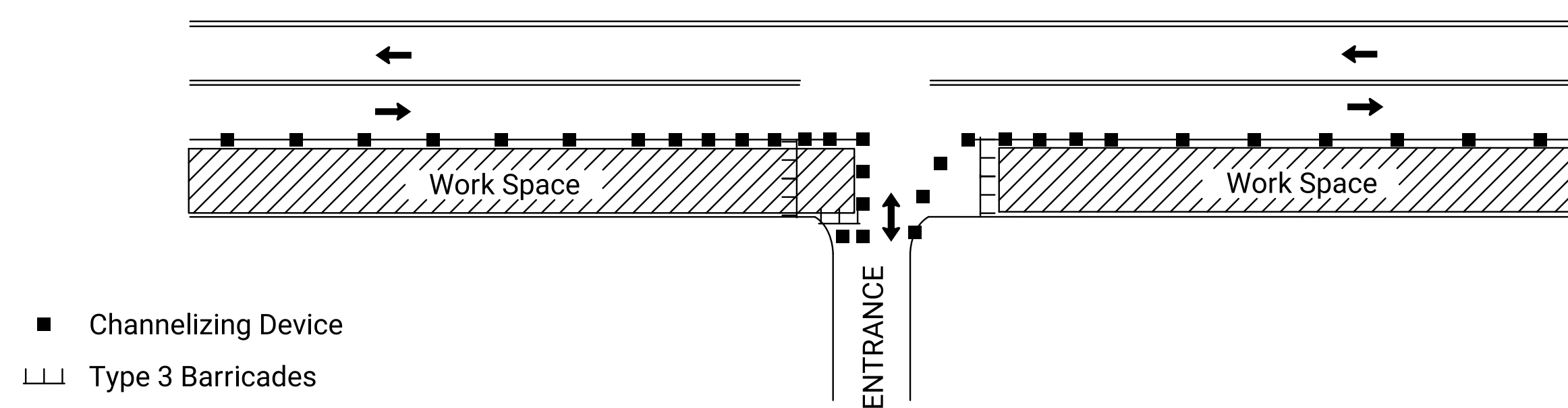


FIGURE 3: LOW VOLUME ENTRANCE CONSTRUCTED HALF AT A TIME

Note: Consider large vehicles making right turns into and out of entrance and use figure 4 as needed

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL ACCESS THROUGH THE WORK AREA				
TE705				
DESIGNED	R.W.B.	06-01-15	APPD.	Kristina Erickson
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACED	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	91	135

SIGN LAYOUT INFORMATION



Std. Size
Expwy/Freeway
6" C
48"x 24"



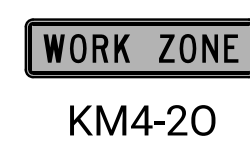
Std. Size
Expwy/Freeway
8" D
48"x 48"



Std. Size
Expwy/Freeway
6" C
48"x 24"

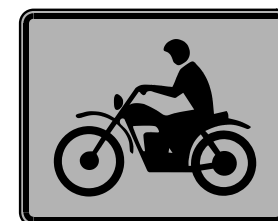


Std. Size
Expwy/Freeway
8" D
48"x 48"



Std. Size
3" C
24"x 6"

Expwy/Freeway
6" C
48"x 12"



Std. Size
Expwy/Freeway
30"x 24"



Mileage to be Determined
by the Engineer.



Std. Size
Expwy/Freeway
8" D
48"x 48"



Std. Size
Expwy/Freeway
48"x 48"



Std. Size
Expwy/Freeway
30"x 24"



Std. Size
6" C

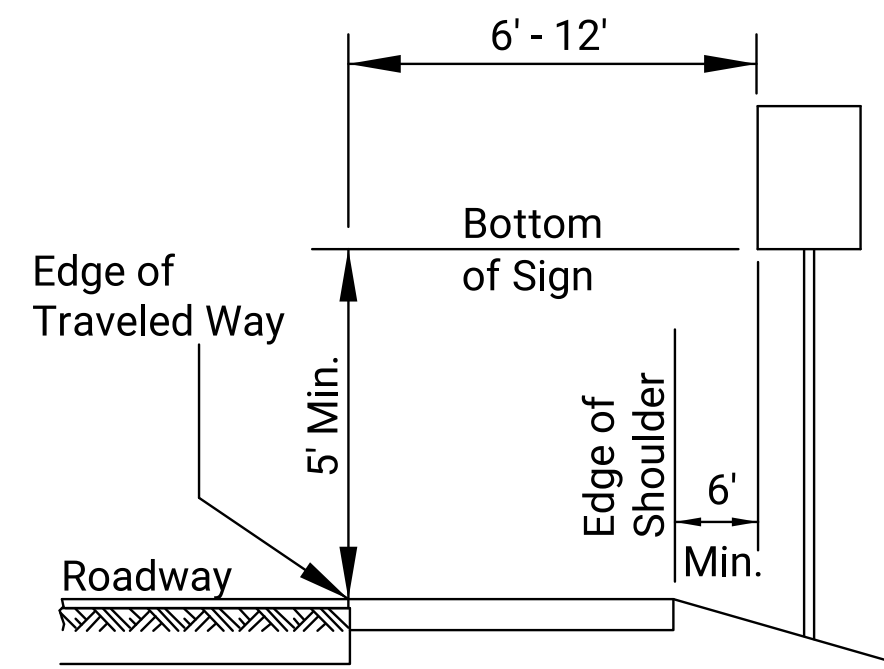
Expwy/Freeway
10" D



Std. Size
Uppercase: 6" C
Lowercase: 4.5" C

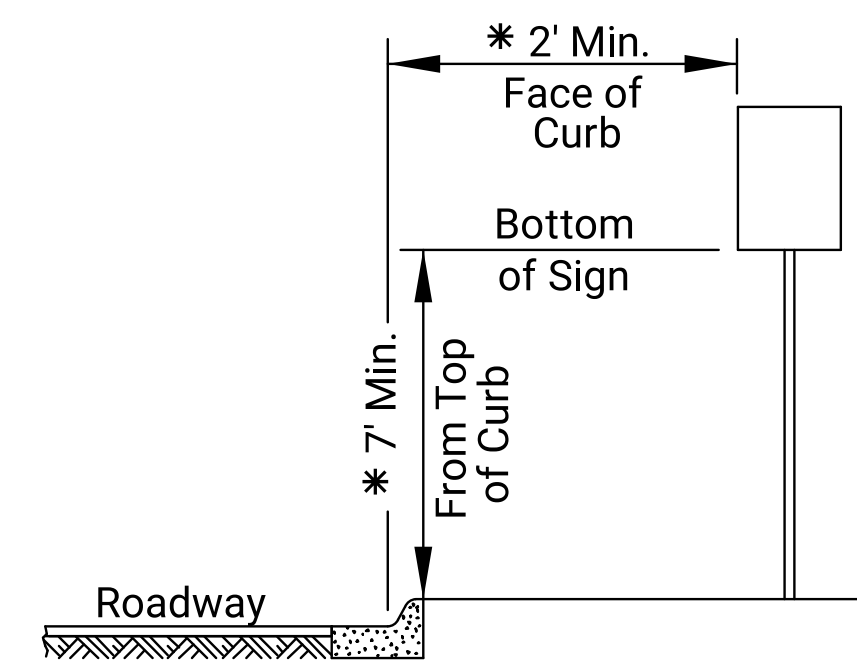
Expwy/Freeway
Uppercase: 10" D
Lowercase: 8" D

All city names and street names on special signs and destination signs must have upper and lower case letters.



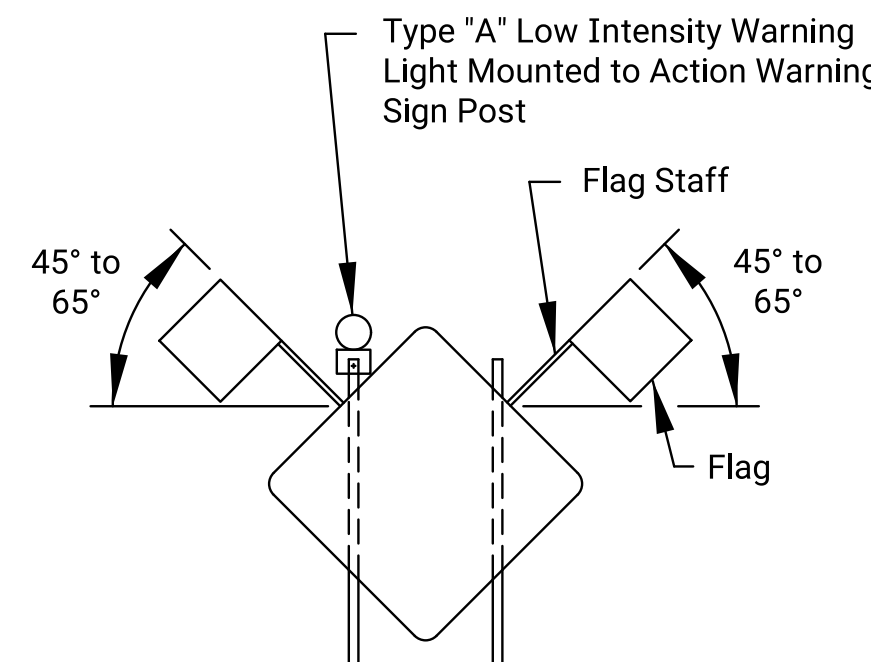
RURAL

- 1) Ground-mounted signs shall be mounted at a minimum height of 5' measured from the bottom of sign to the near edge of the pavement.
- 2) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- 3) The height of the secondary sign mounted below another sign may be 4' measured from the bottom of the sign to the near edge of the pavement. Signs shall not overlap each other.



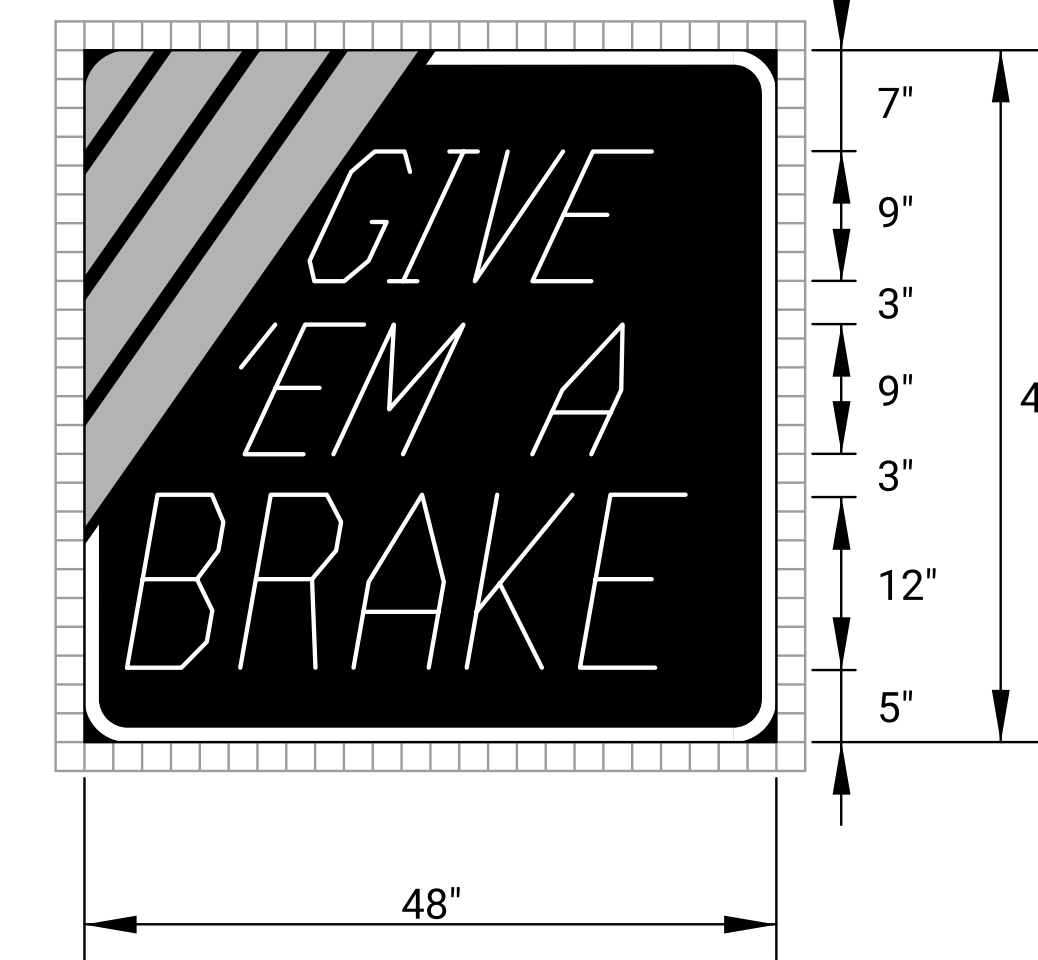
URBAN

- 1) Signs shall be mounted at a minimum height of 7' measured from the bottom of sign to the near edge of the pavement.
- 2) Neither portable nor permanent sign supports should be located on sidewalks or areas designated for pedestrian or bicycle traffic.
- 3) Signs mounted lower than 7' should not project more than 4" into pedestrian facilities.
- 4) The height from of the secondary sign mounted below another sign may be 6' measured from the bottom of sign to the near edge of the pavement. Signs shall not overlap each other.
- 5) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- * 6) Pedestrian detour signing shall be a minimum of 2' measured from the top of the pedestrian pathway to the bottom of the sign and shall not protrude into the walkway nor shall it project beyond the back of curb.

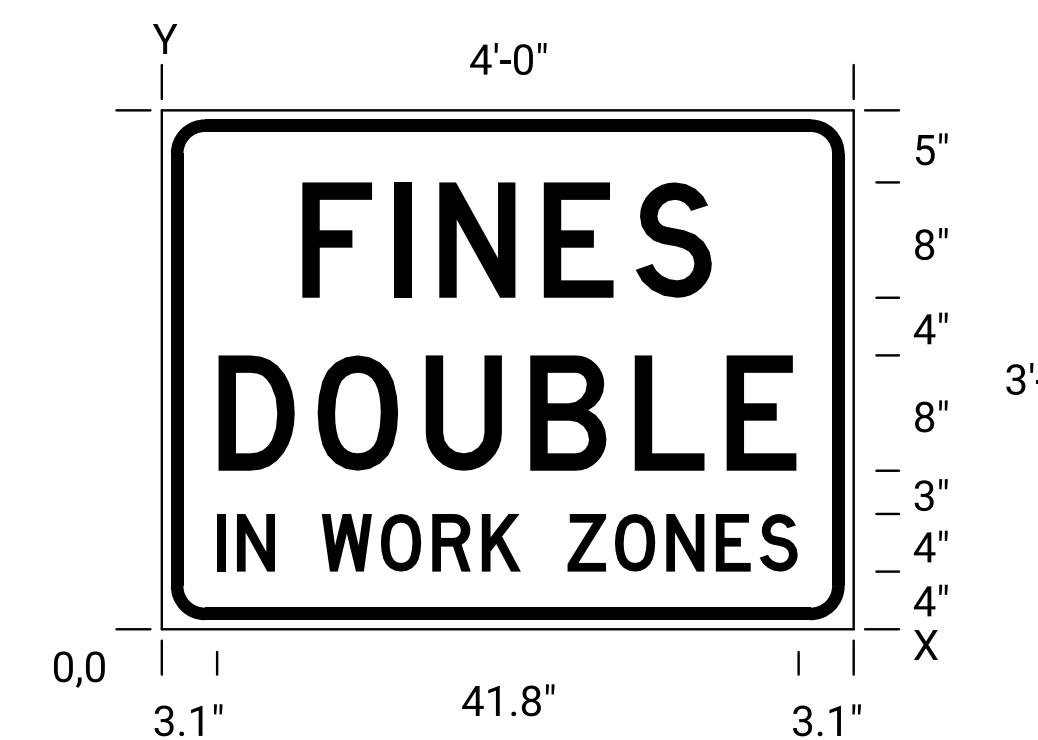


When the sign width is equal to or greater than 9', three or more wood posts may be used with a minimum of 4' between the centerline of each post. All signs less than 9' in width shall use a maximum of two wood posts.

- In the case of hitting rock when driving posts
1. Shift the sign location. Do not violate minimum sign spacing.
 2. With the engineer's approval, use acceptable alternative sign stands.



KI-104a



KI-105a

Sign Number	GIVE EM A BRAKE
Width x Height	4'-0" x 4'-0"
Border Width	1.0"
Corner Radius	4.0"
Stripe Width	3.0"
Mounting	Ground
Background	Type: Non-Reflective Color: Black
Legend/Border	Type: Reflective Color: White
Legend Font	Dutch 801 Roman SWC 25 Degree Slant
Stripes	Type: Reflective Color: Orange

Sign Number	FINES DOUBLE
Width x Height	4'-0" x 3'-0"
Border Width	0.9"
Corner Radius	3.0"
Mounting	Ground
Background	Type: Reflective Color: White
Legend/Border	Type: Non-Reflective Color: Black

Dimensions in inches Spacings are to start of next letter

Y FONT	LETTER SPACINGS													HT LEN	
23.0 D	F	I	N	E	S									8.0	
	9.7	6.4	3.2	7.3	6.4	5.4	9.7							28.6	
11.0 D	D	O	U	B	L	E								8.0	
	3.9	6.9	7.5	7.3	6.4	4.9	3.9							40.3	
4.0 D	I	N	W	O	R	K	Z	O	N	E	S			4.0	
	3.1	1.6	2.7	3.2	4.3	3.8	3.6	2.8	3.4	3.8	3.6	3.2	2.7	3.1	41.8

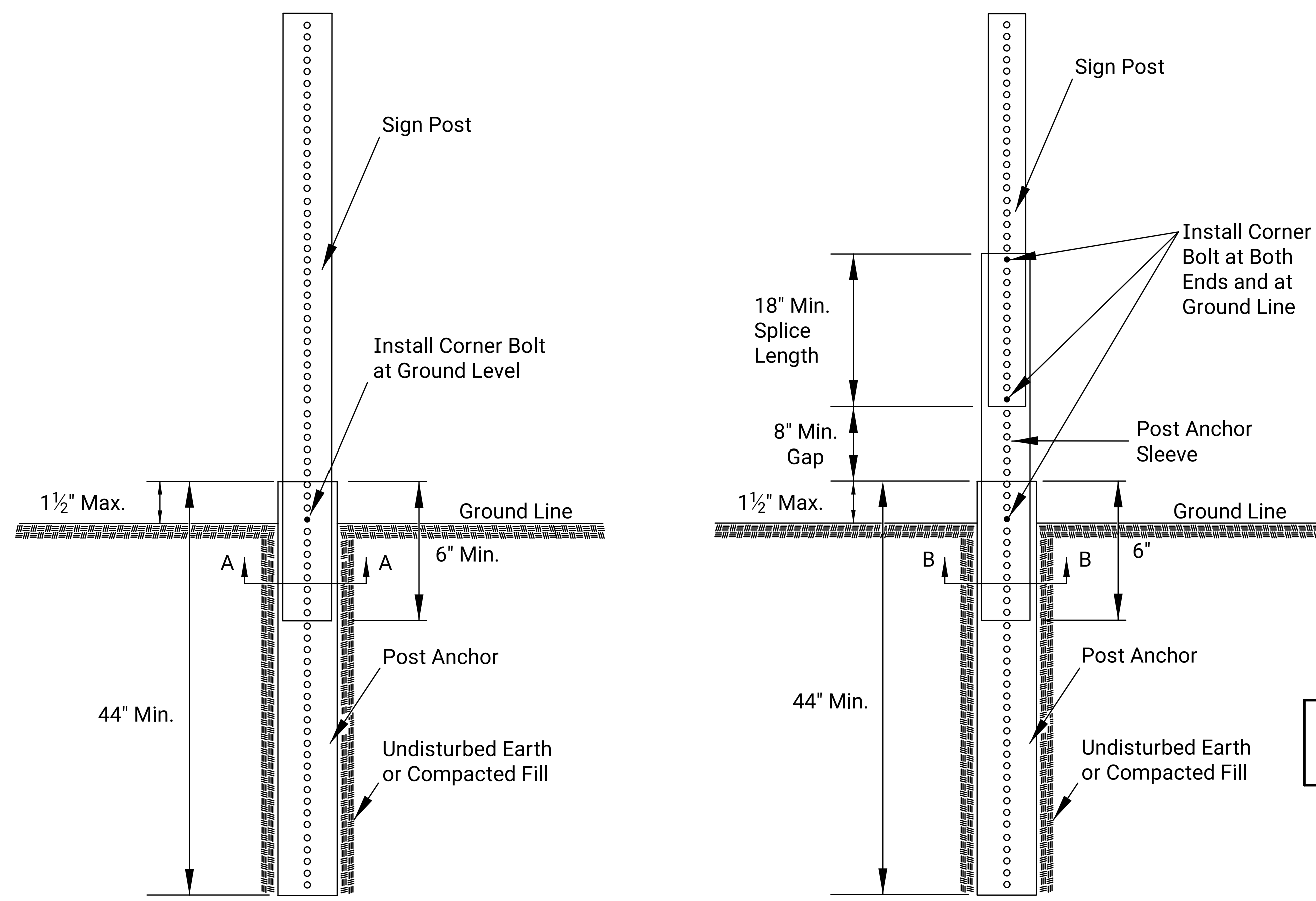
Notes:

- Typically, there are two sets of informational signs installed per project: one for each direction of traffic.
- Install signs a minimum of 500' in advance of the road work ahead sign. The engineer may designate a more appropriate location if conditions dictate.
- The informational signs are not to interfere with the traffic control signs for the project.

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL SIGN INFORMATION				
TE710				
FHWA APPROVAL		06-01-15		APPD. Kristina Erickson
DESIGNED	R.W.B.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	TRACE CK.

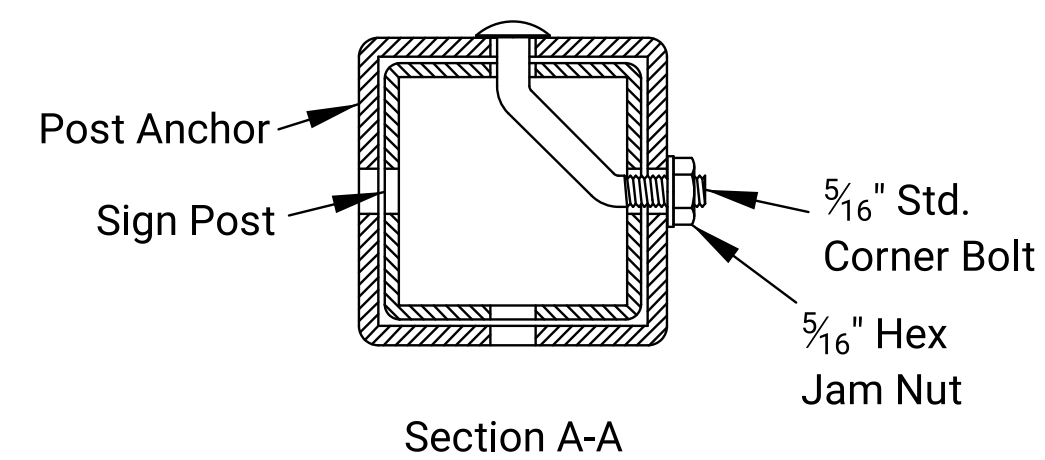
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	92	135

PERFORATED SQUARE STEEL TUBE (P.S.S.T.) POST SETUP

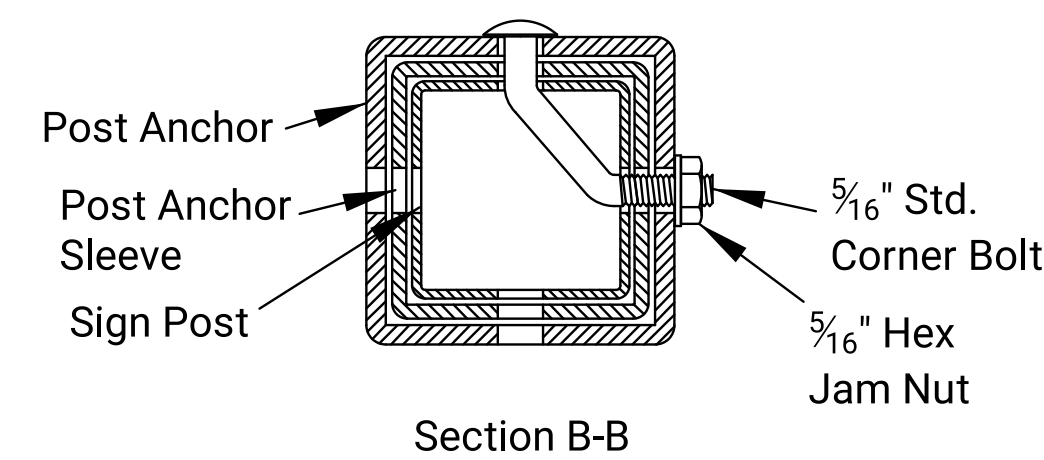


P.S.S.T. Detail

Telescoping P.S.S.T. Detail



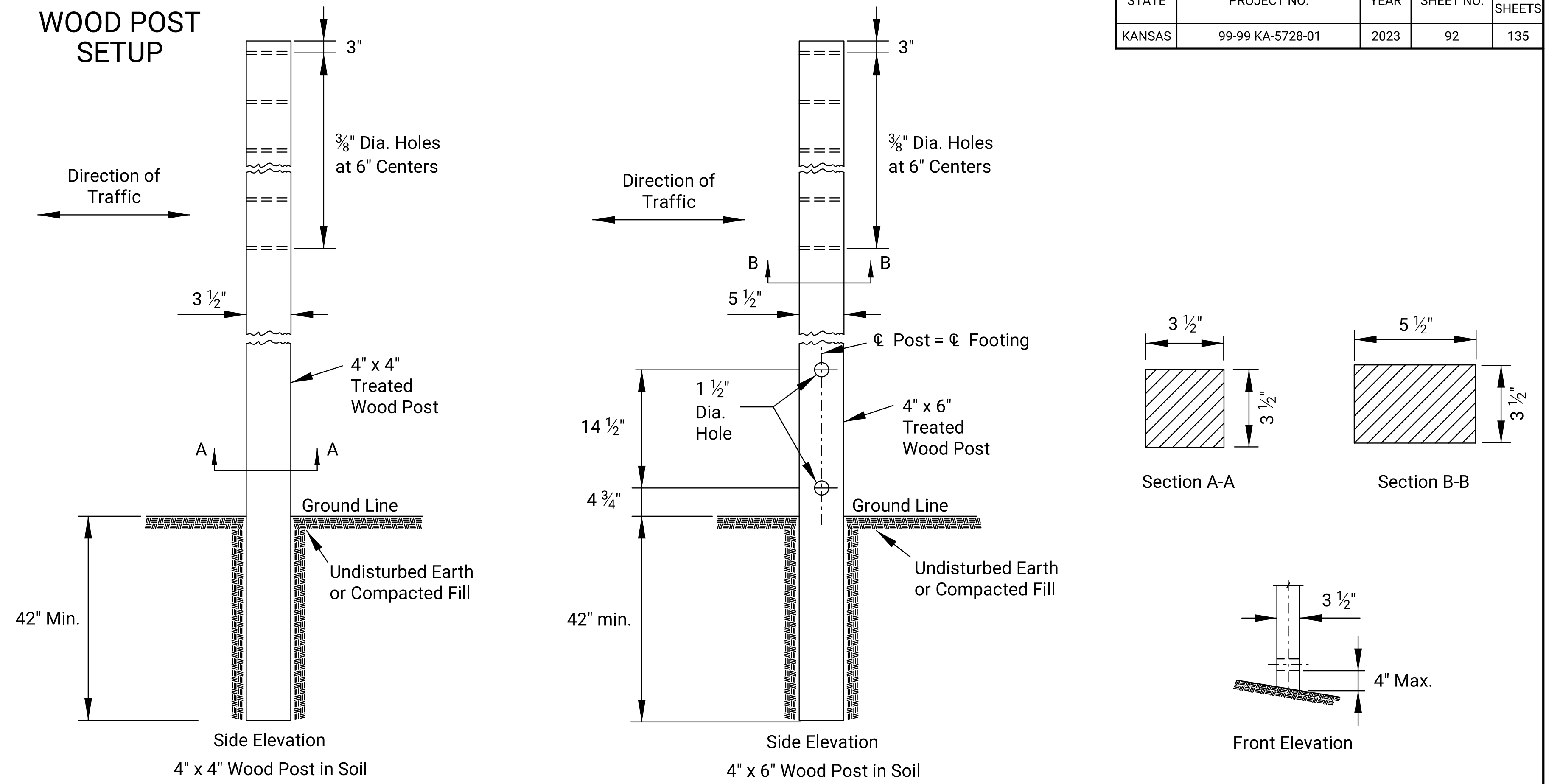
Section A-A



Section B-B

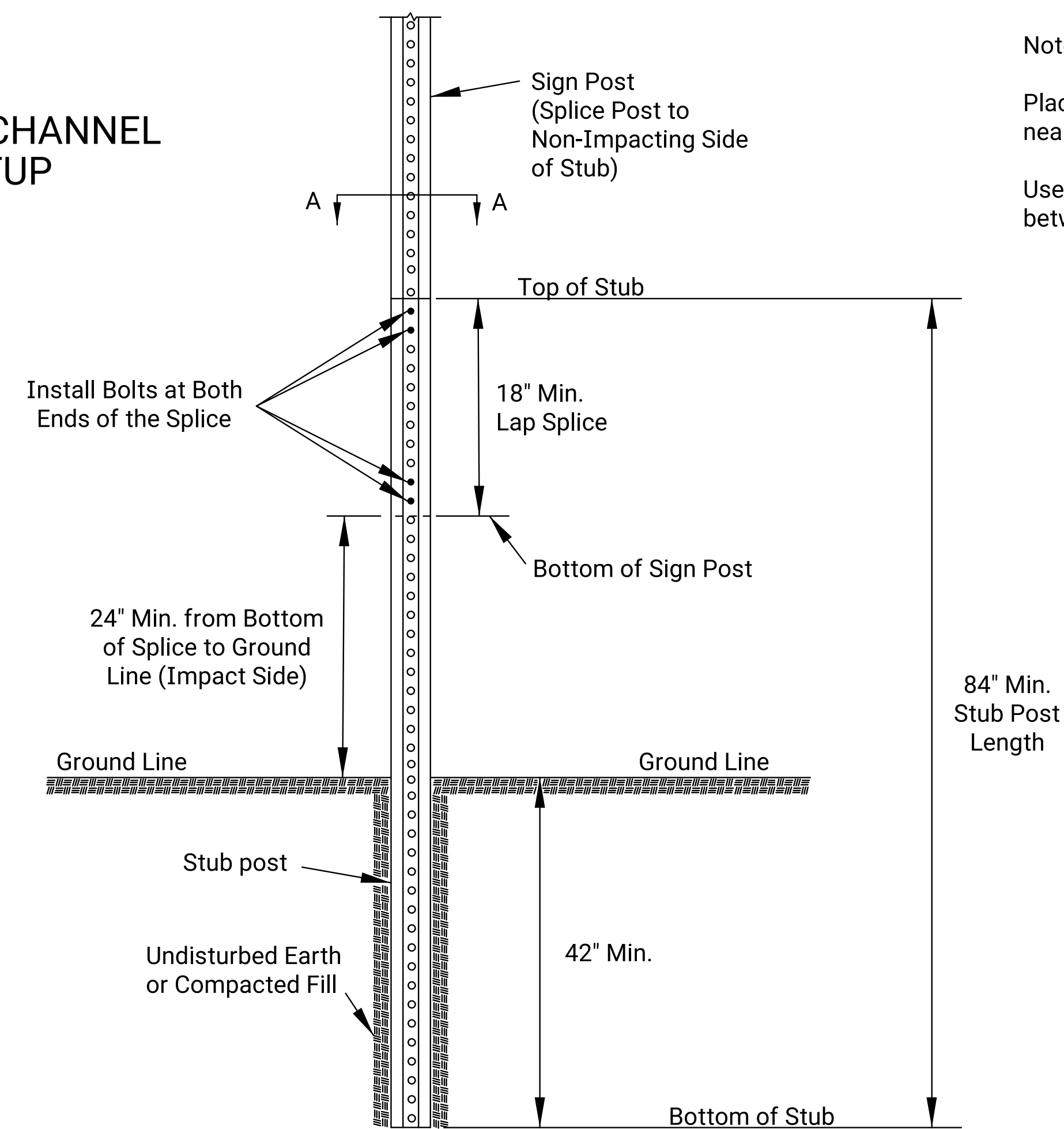
Details for 2", 2 1/4", or 2 1/2" sign posts
Place bolts in the same corner along each sign post.

WOOD POST SETUP



See TE710 for Additional Details and Requirements

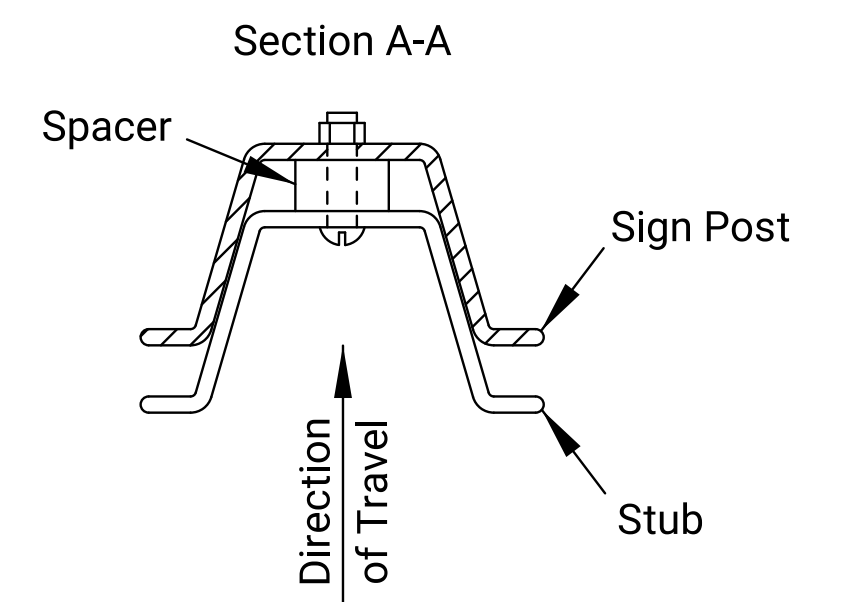
3 LB/F U-CHANNEL SETUP



Notes:

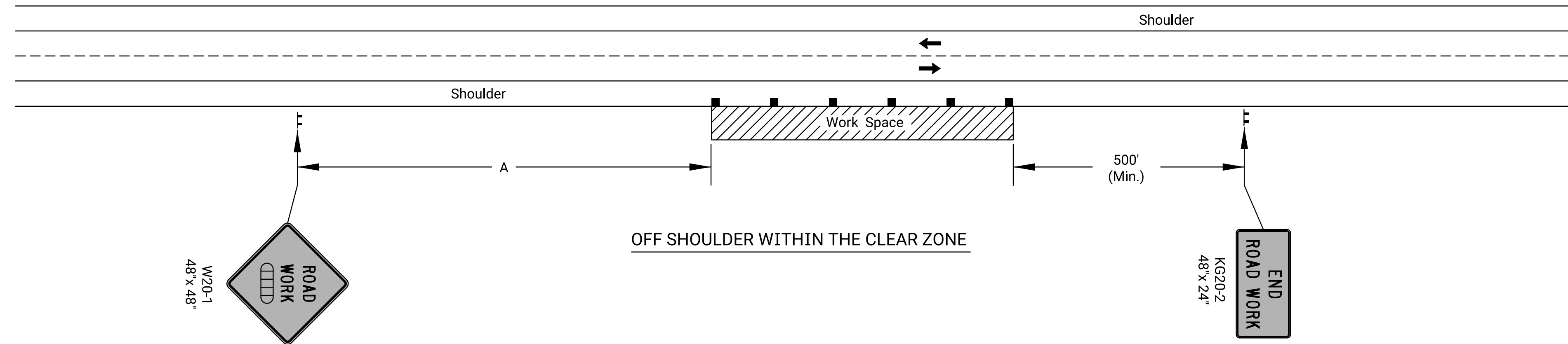
Place two bolts at both ends of the splice through the holes nearest the ends of the splice.

Use manufacturer recommended spacers over the bolts between the spliced pieces of U-Channel.



NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL SIGN POSTS				
TE712				
DESIGNED	B.A.H.	APPD.	06-01-15	Kristina Erickson
DESIGN CK.	DETAIL CK.	R.W.B.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	QUAN. CK.	TRACE CK.

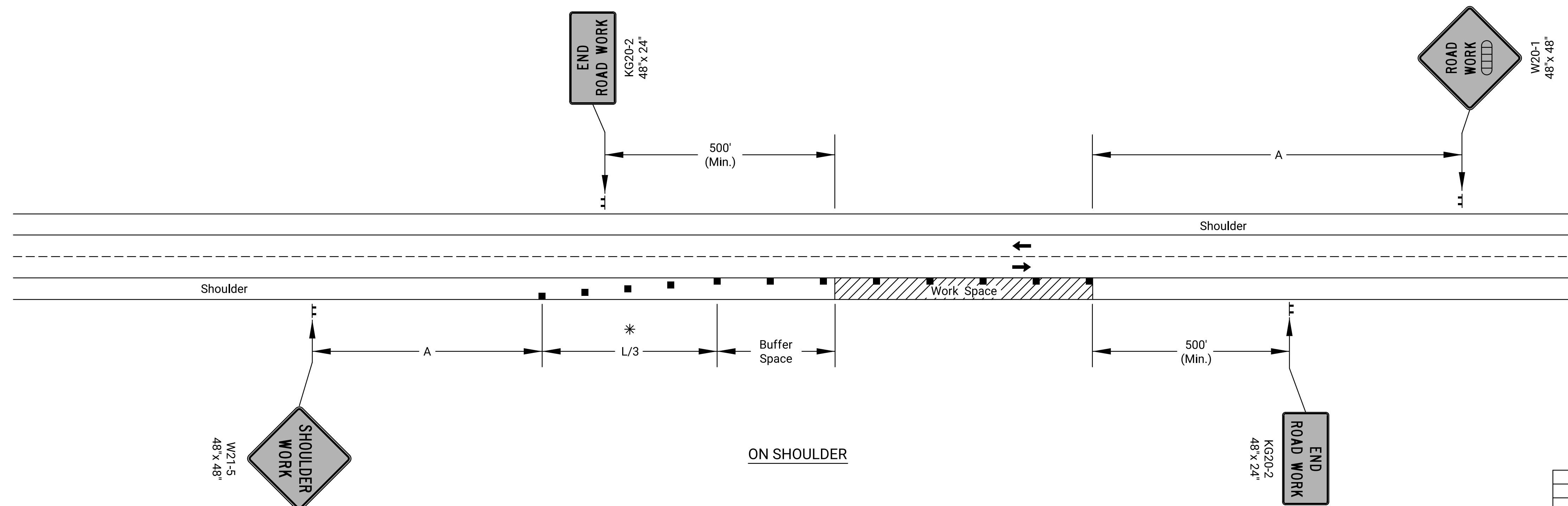
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	93	135



Notes:

No traffic control is required if the Work Space is located outside of the clear zone.

For operations of 60 minutes or less, all signs and channelizing devices may be eliminated if a vehicle with high-intensity rotating, flashing, oscillating, or strobe lights is used.

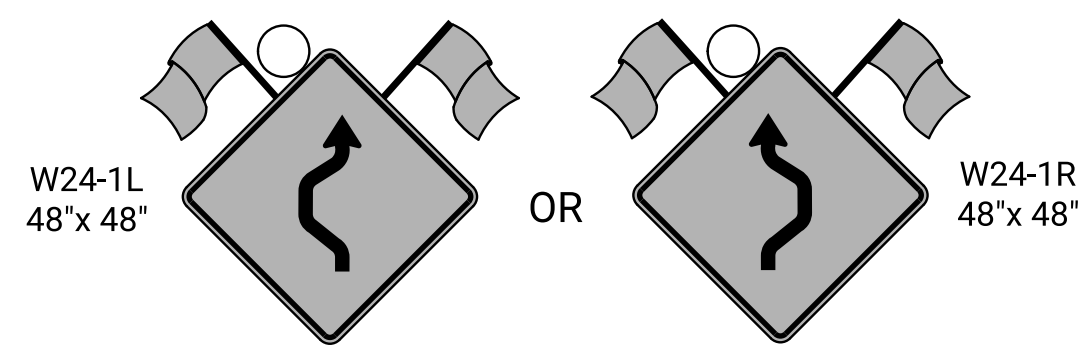


* Omit taper if paved shoulder is less than 8' wide.

- Channelizing Device
- ◻◻◻◻ Ahead, 1500 ft, or 1 Mile

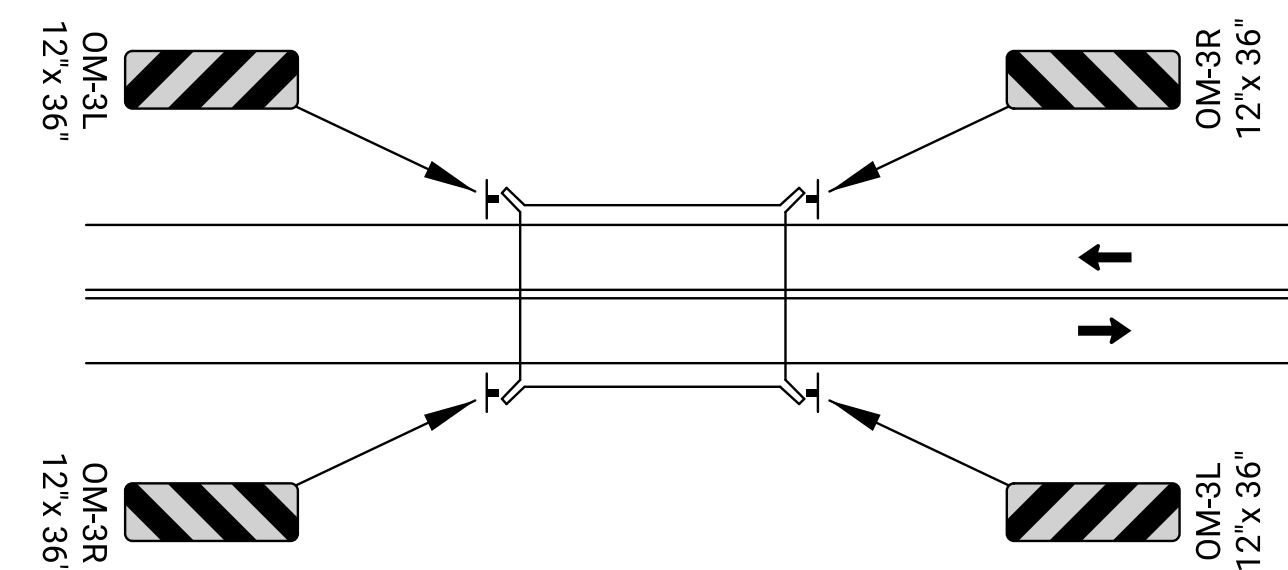
Plotted by : Robert.Barron@ks.gov 4-JAN-2024 18:02
File : TE720.dgn

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL SHOULDER WORK UNDIVIDED ROADWAY				
TE720				
FHWA APPROVAL		06-01-15		APPD. Kristina Erickson
DESIGNED	L.E.R.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	TRACE CK.



One W24-1 should be used per approach where the tangent distance between two reverse curves is less than 600 ft. If used, use in place of the first W1-4 and eliminate the second.

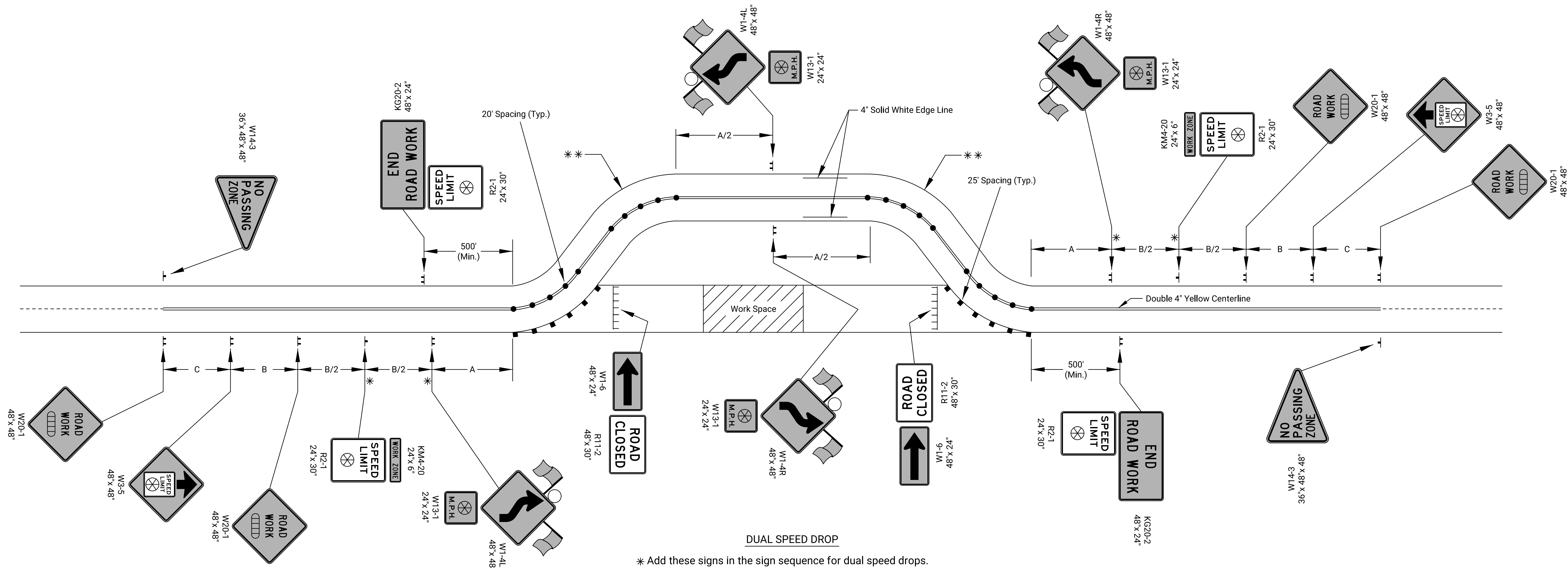
TYPICAL TEMPORARY STRUCTURE END TREATMENT ON SHOOFLY



The entire area of object marker shall have ASTM Type III sheeting. The stripes shall slope downward to the traffic side for channelizing.

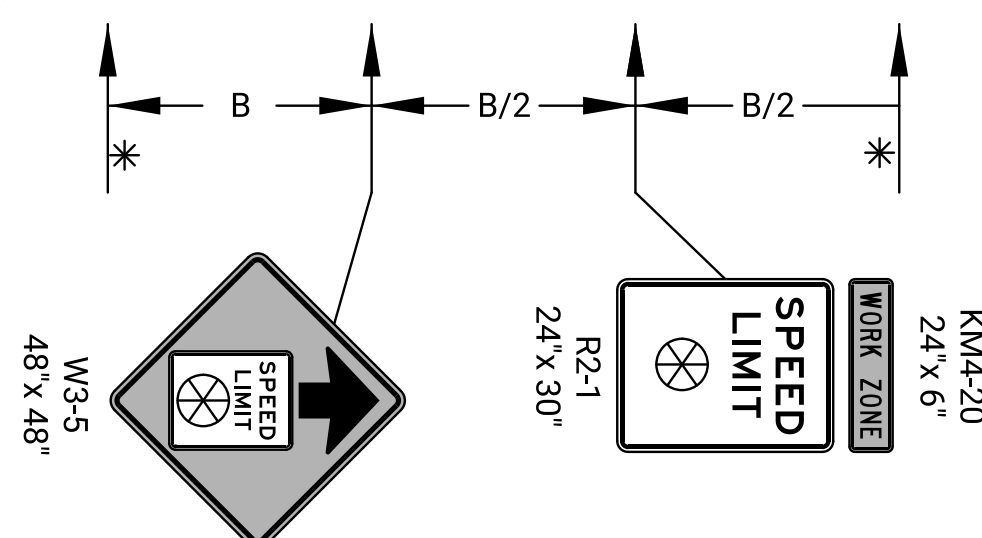
** Black on orange 24" x 30" chevron signs (W1-8) shall be mounted back to back on the outside edge of shoofly curves with a radius of 1000' or less at the spacing shown below. A minimum of 3 chevrons should be installed per curve.

Suggested Chevron Spacing	
Curve Radius	Max. Spacing
1000' - 800'	100'
800' - 450'	80'
Less than 450'	60'



DUAL SPEED DROP

* Add these signs in the sign sequence for dual speed drops. Adjust all other signs and pavement marking accordingly.



- Channelizing Device
- ⊥ Type 3 Barricades
- Bi-Directional Temp. Raised Pavement Marker (Type I)
- ⊞ Ahead, 1500 ft, or 1 mile
- ⊗ Speed to be determined by the Engineer
- Type "A" Low Intensity Warning Light

01	03-13-18	W24-1 usage changed to Should	R.W.B.	E.G.K.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL SHOOFLY DIVERSION

TE736

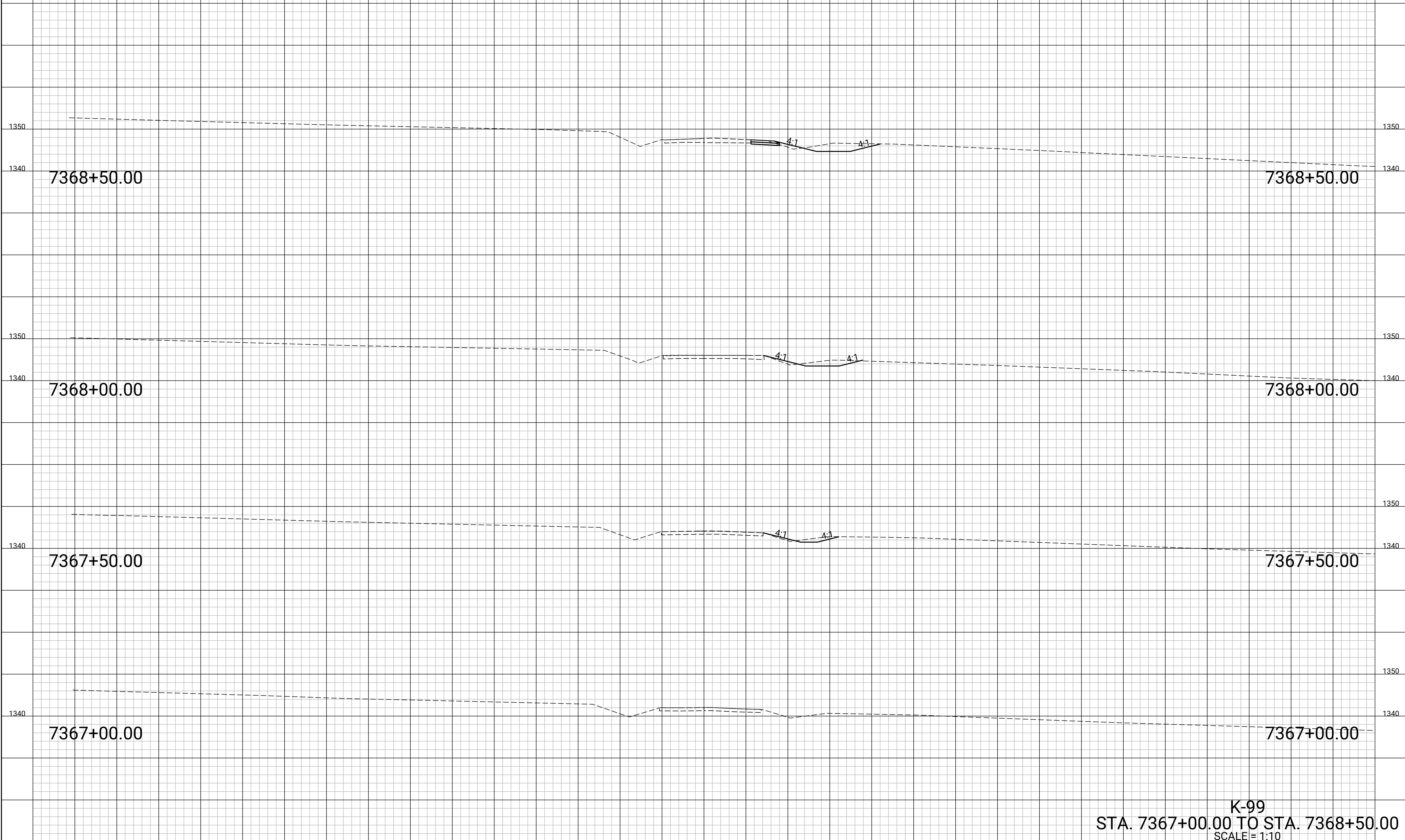
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		

Eric Koehler

KDOT Graphics Certified 07-18-2022 Sh. No. 94

160 150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160

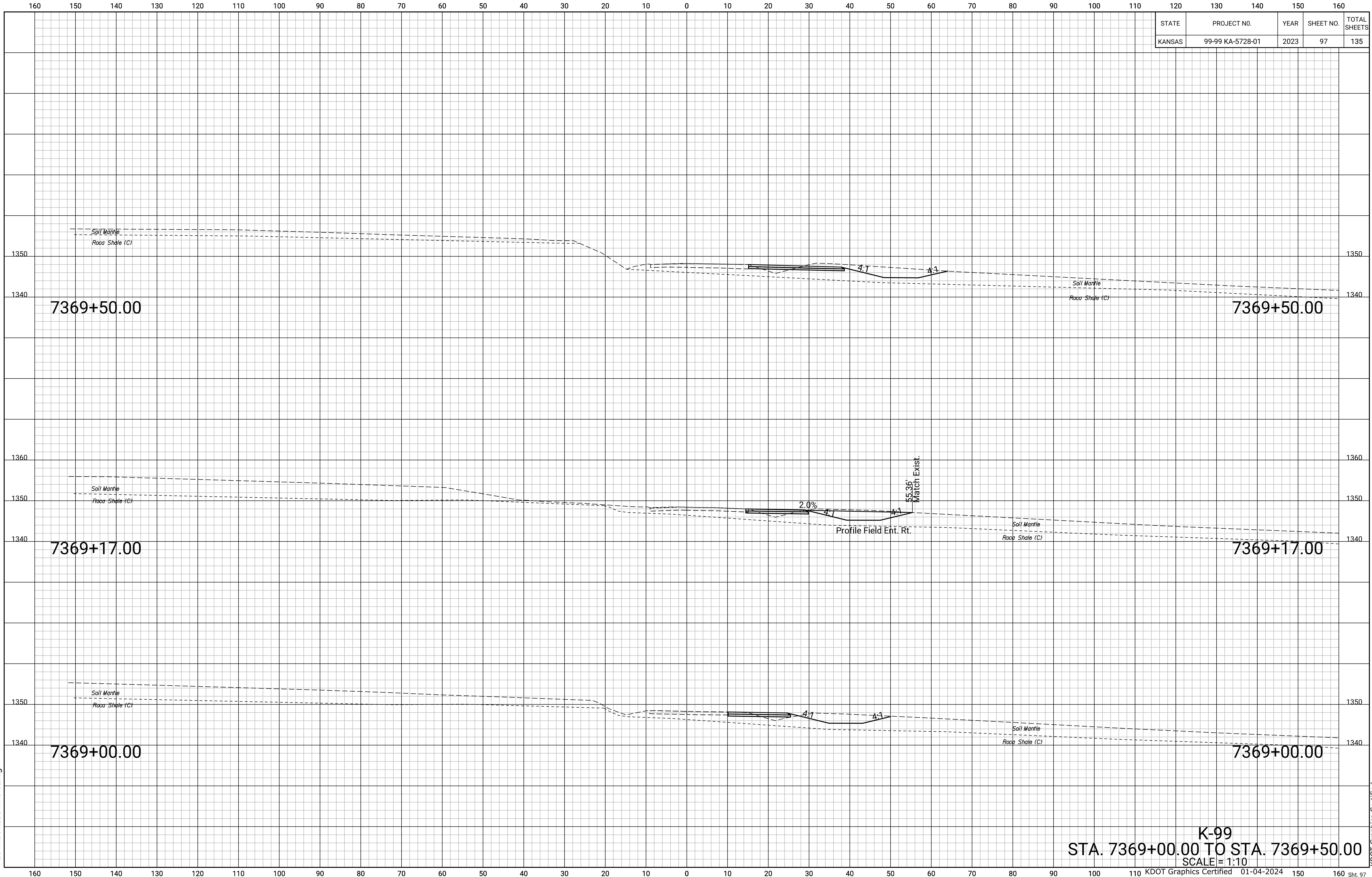
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	96	135



Plotted by: August Zuno 8-JAN-2024 16:34
File: ka572801Trxs-02.dgn

K-99
STA. 7367+00.00 TO STA. 7368+50.00
SCALE=1:10
DOT Graphics Certified 01-04-2024 Sht. 96

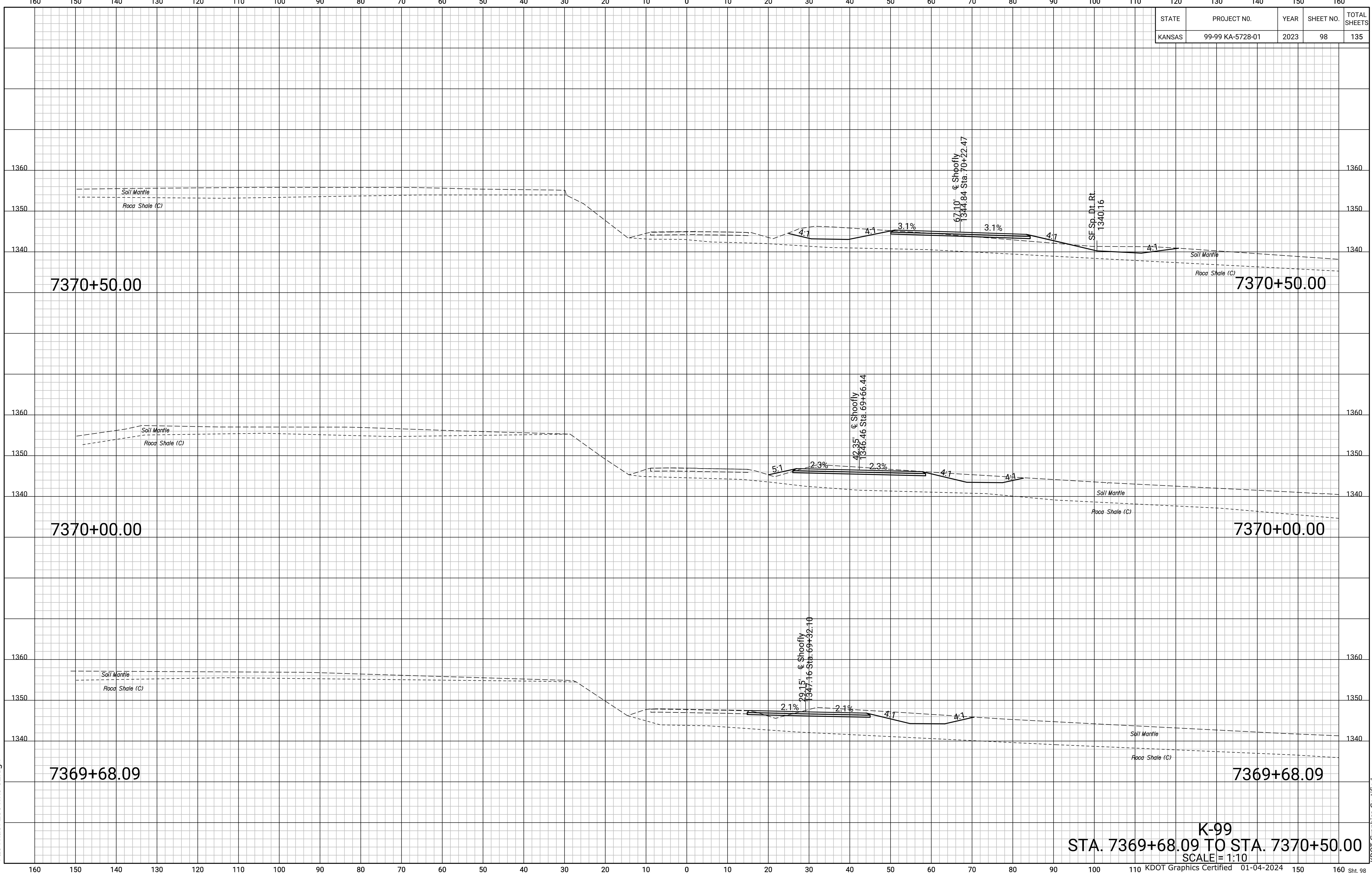
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	97	135



Plotted by: August Zuno 8-JAN-2024 16:34
 File: ka572801rxs-02.dgn

K-99
 STA. 7369+00.00 TO STA. 7369+50.00
 SCALE = 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	98	135

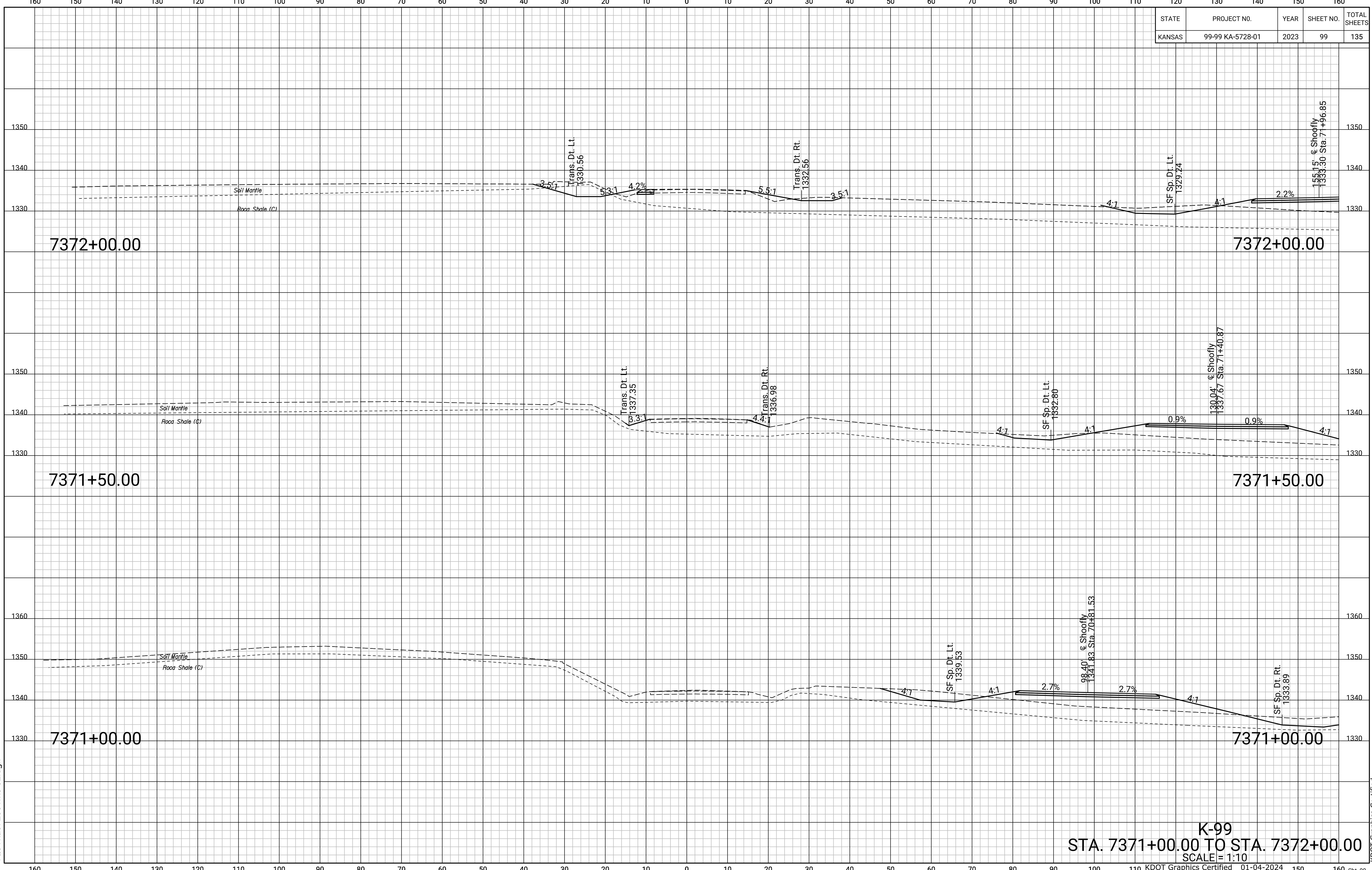


Plotted by: August Zuno
 File: ka572801Trxs-02.dgn
 8-JAN-2024 16:34

K-99
 STA. 7369+68.09 TO STA. 7370+50.00
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	99	135

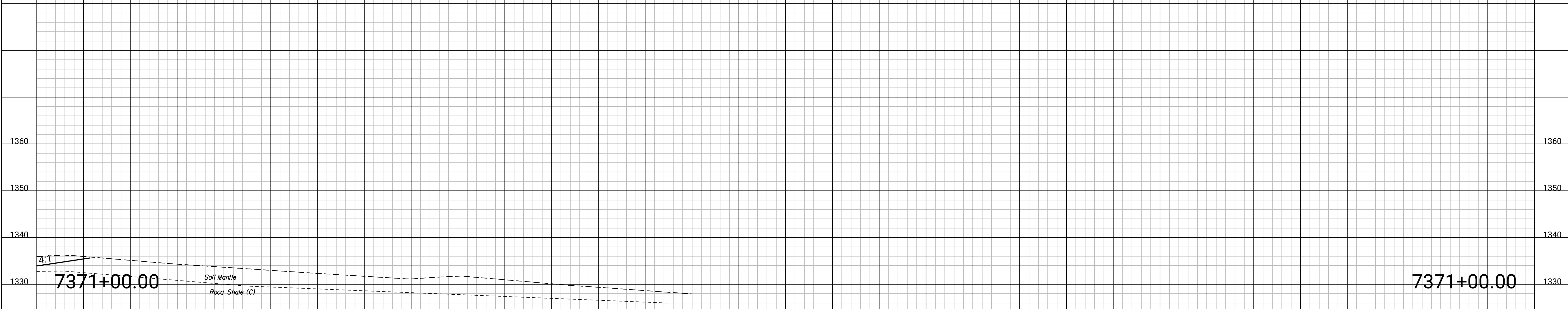
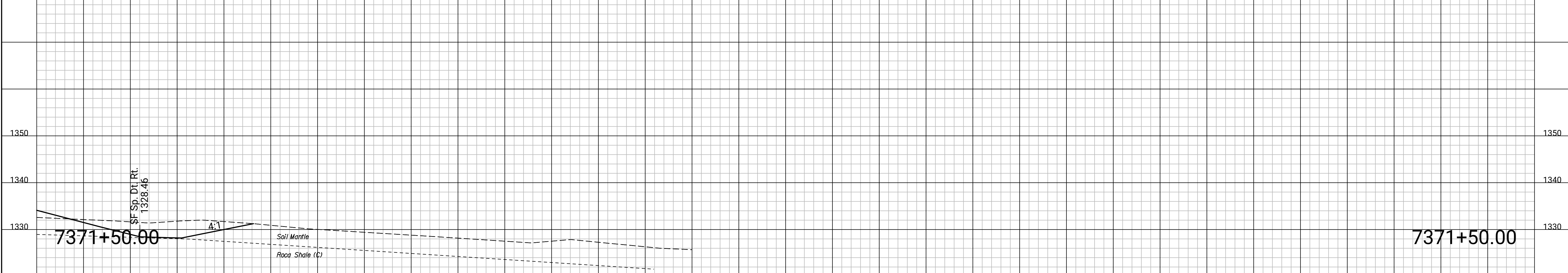
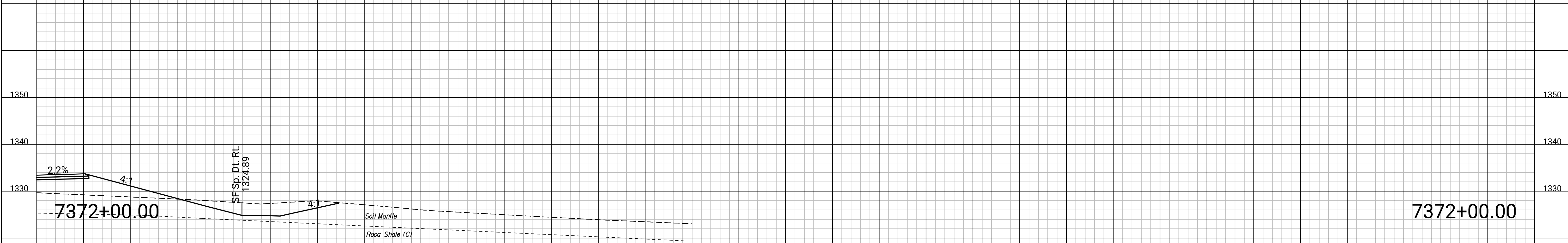


Plotted by : August Zuno 8-JAN-2024 16:34
File : ka572801Trxs-02.dgn

K-99
STA. 7371+00.00 TO STA. 7372+00.00
SCALE = 1:10
KDOT Graphics Certified 01-04-2024 Sht. 99

KDOT Graphics Certified

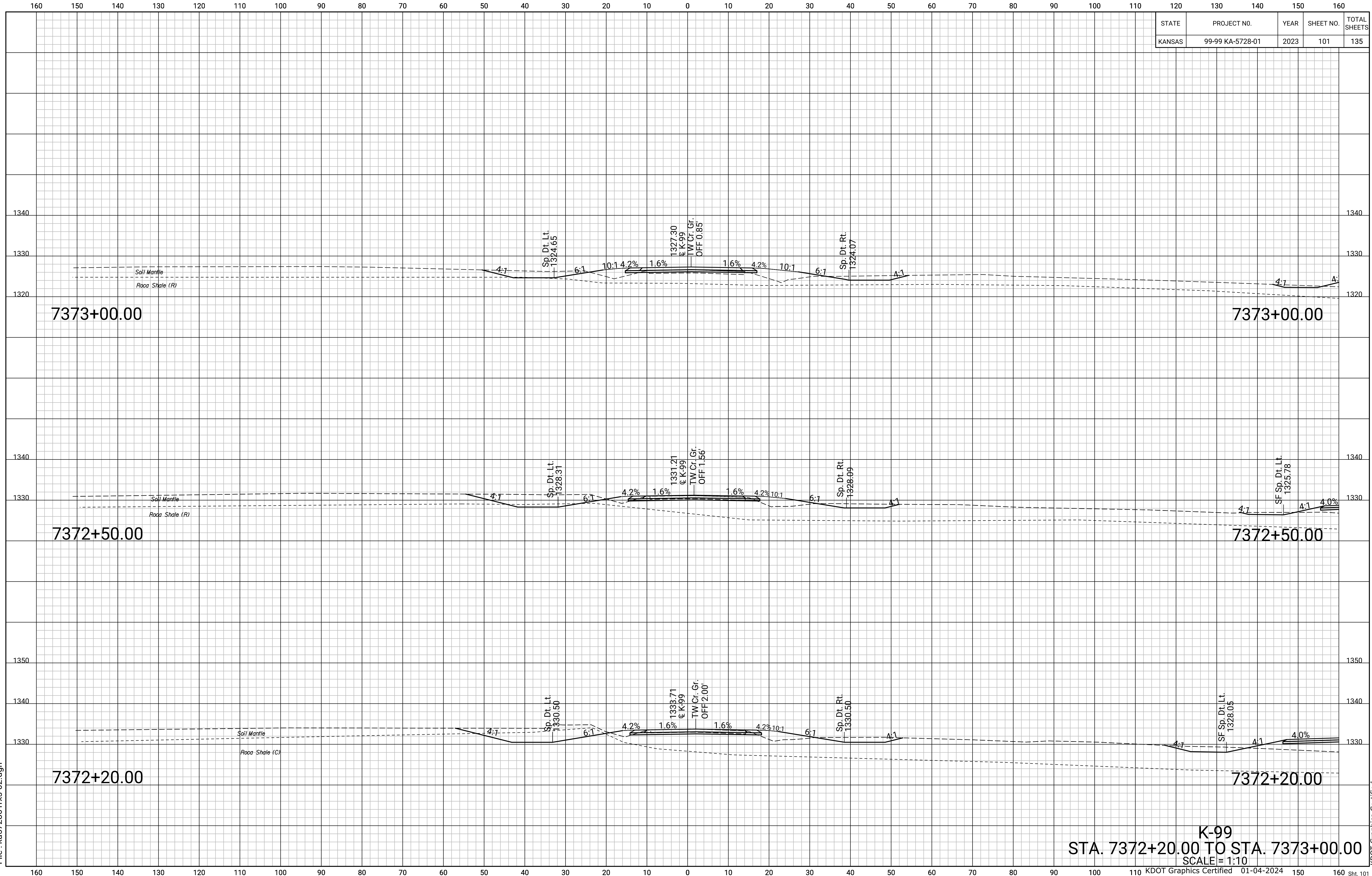
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	100	135



K-99 - Ext. Rt.
 STA. 7371+00.00 TO STA. 7372+00.00
 SCALE = 1:10

Plotted by: August Zuno
 File: ka572801Trxs-02.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	101	135

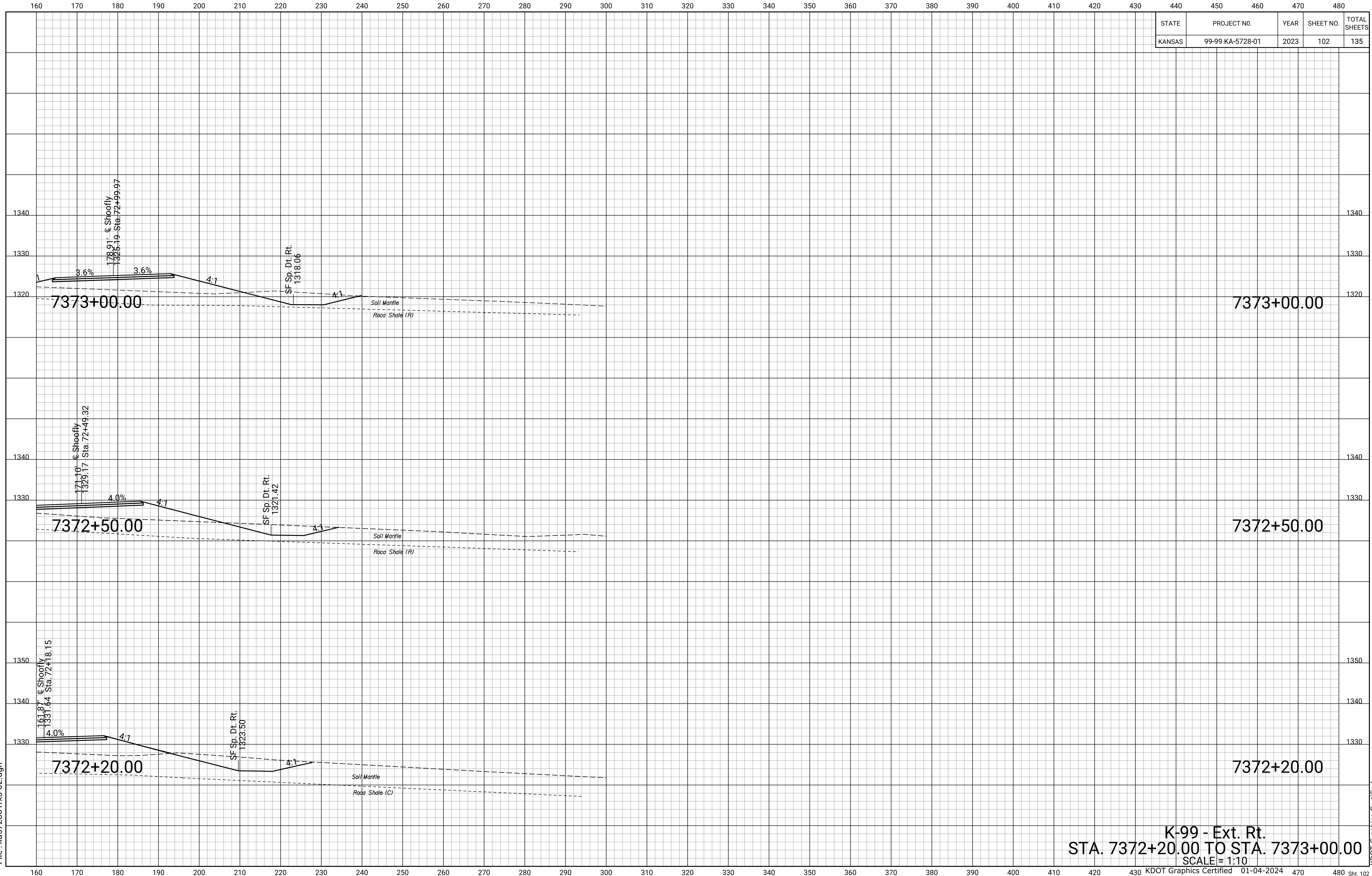


Plotted by: August Zuno
 File: ka572801Trxs-02.dgn
 8-JAN-2024 16:35

K-99
 STA. 7372+20.00 TO STA. 7373+00.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	102	135

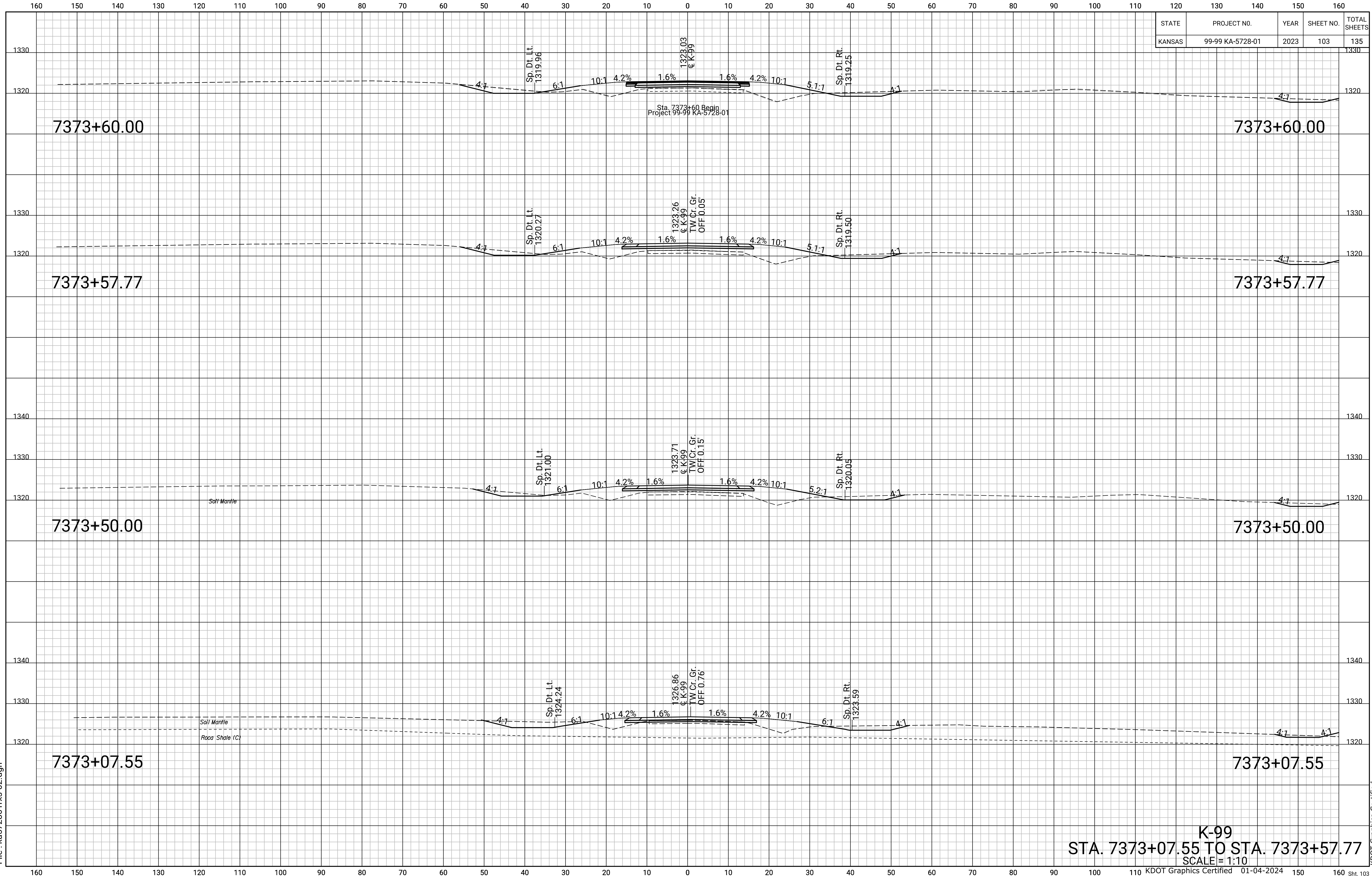


Plotted by: August Zuno
 File: ka572801rxs-02.dgn
 8-JAN-2024 16:32

K-99 - Ext. Rt.
 STA. 7372+20.00 TO STA. 7373+00.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	103	135

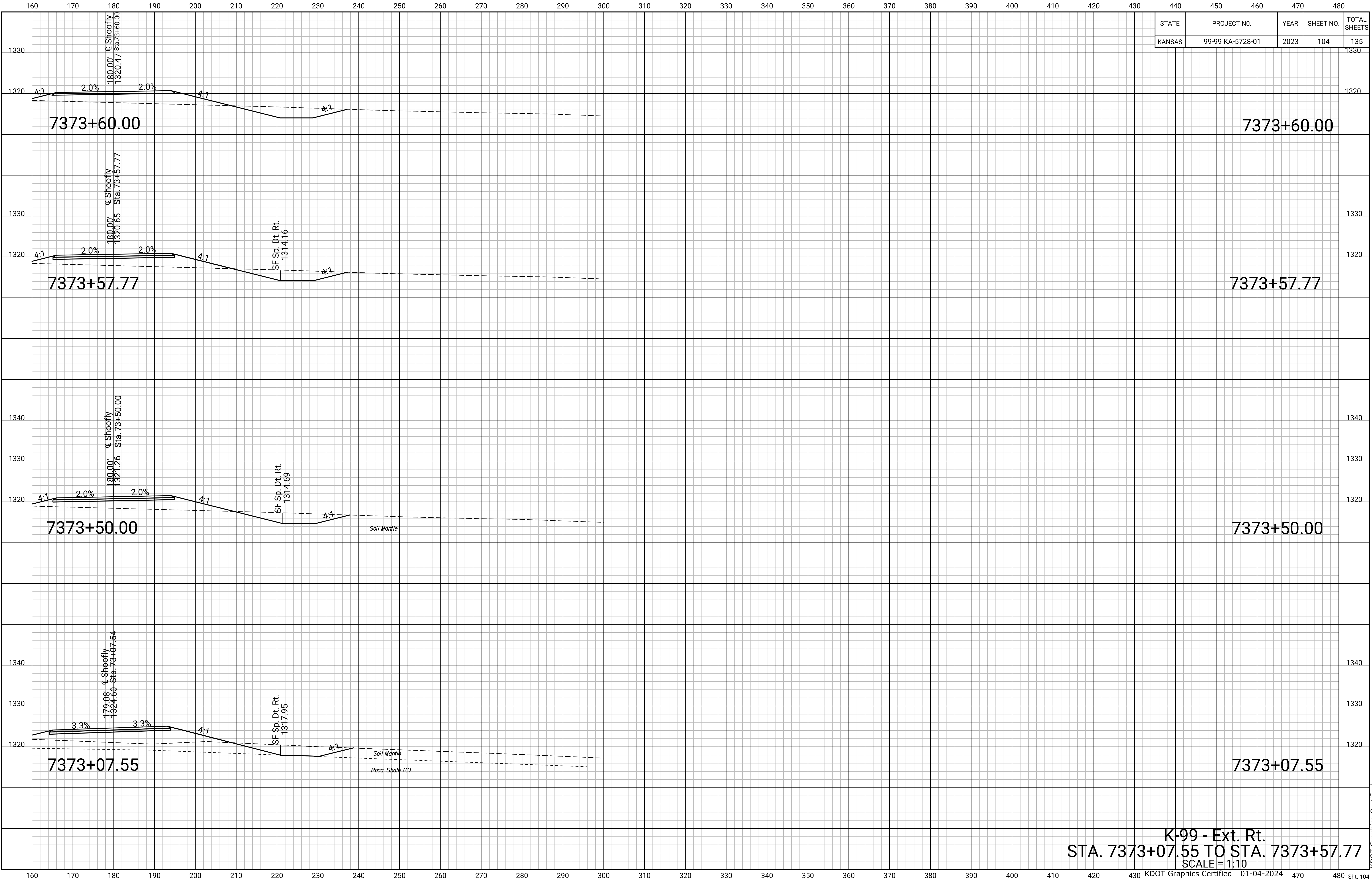


Plotted by: August Zuno 8-JAN-2024 16:35
 File: ka572801Trxs-02.dgn

K-99
 STA. 7373+07.55 TO STA. 7373+57.77
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	104	135

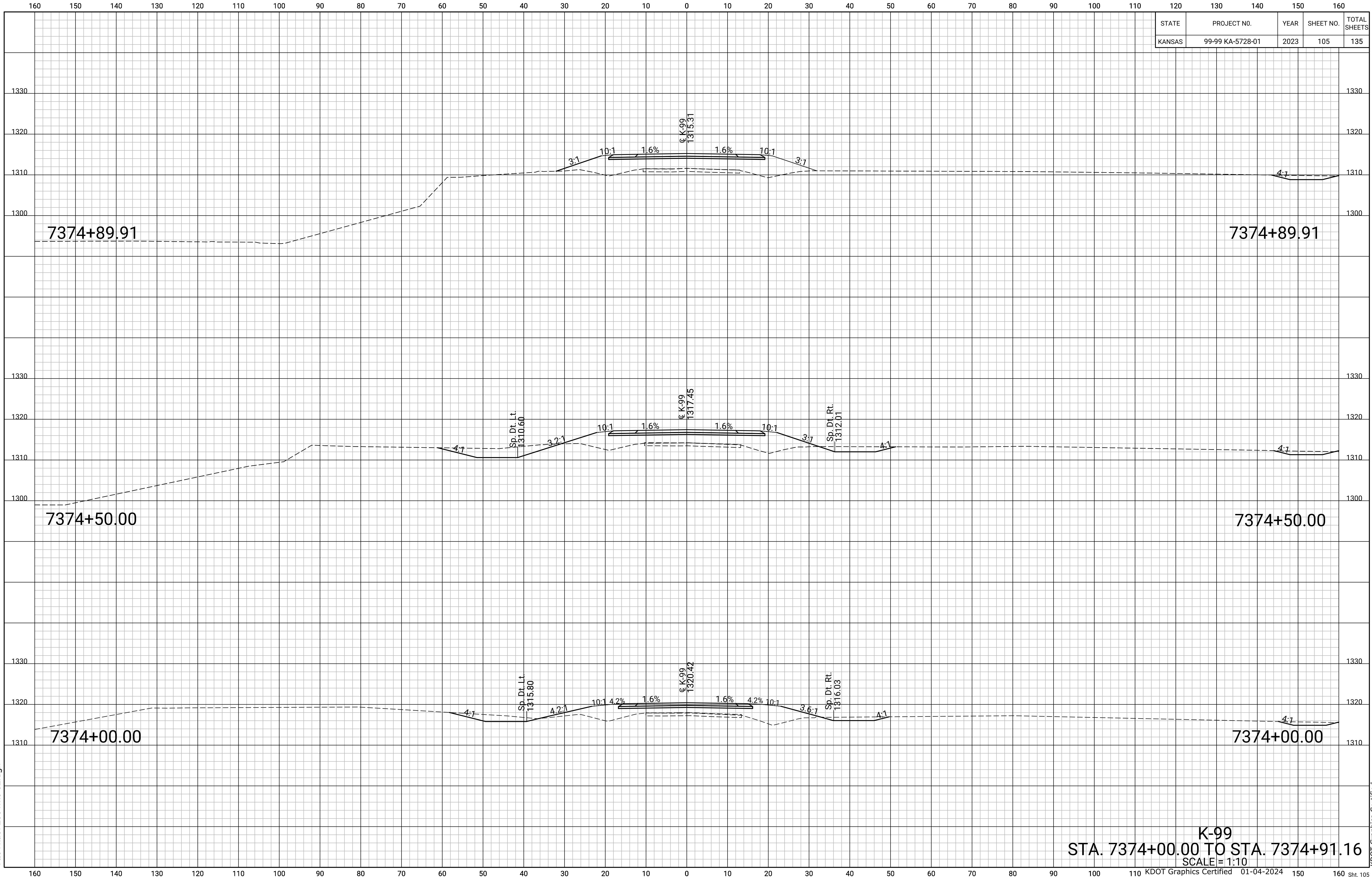


Plotted by: August Zuno 8-JAN-2024 16:32
 File: ka572801rxs-02.dgn

K-99 - Ext. Rt.
 STA. 7373+07.55 TO STA. 7373+57.77
 SCALE = 1:10

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	105	135

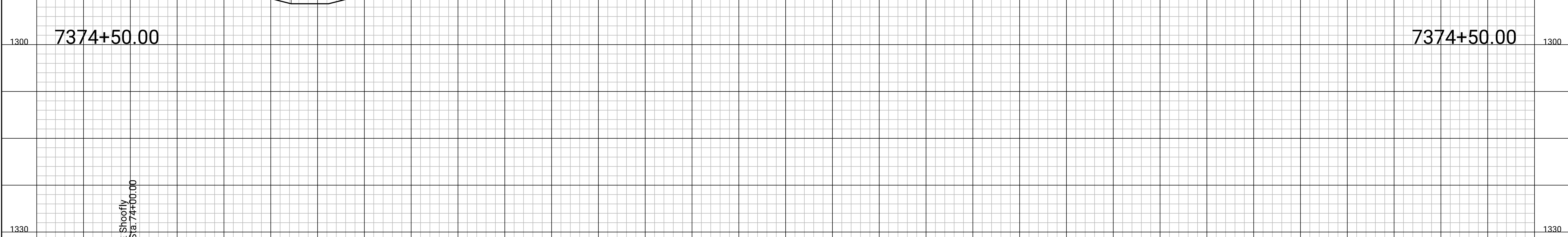
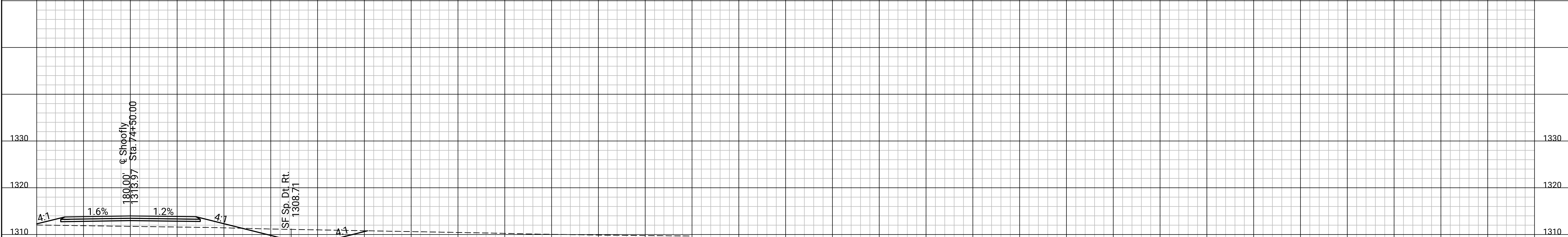
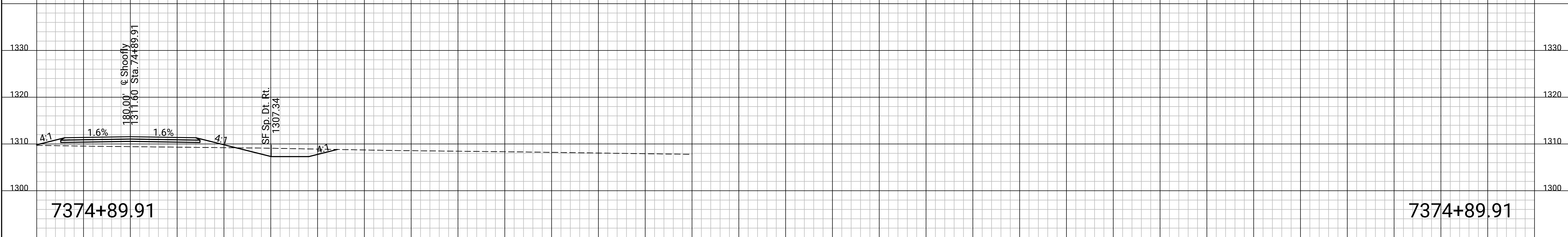


Plotted by: August Zuno 8-JAN-2024 16:35
 File: ka572801Trxs-02.dgn

K-99
 STA. 7374+00.00 TO STA. 7374+91.16
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	106	135

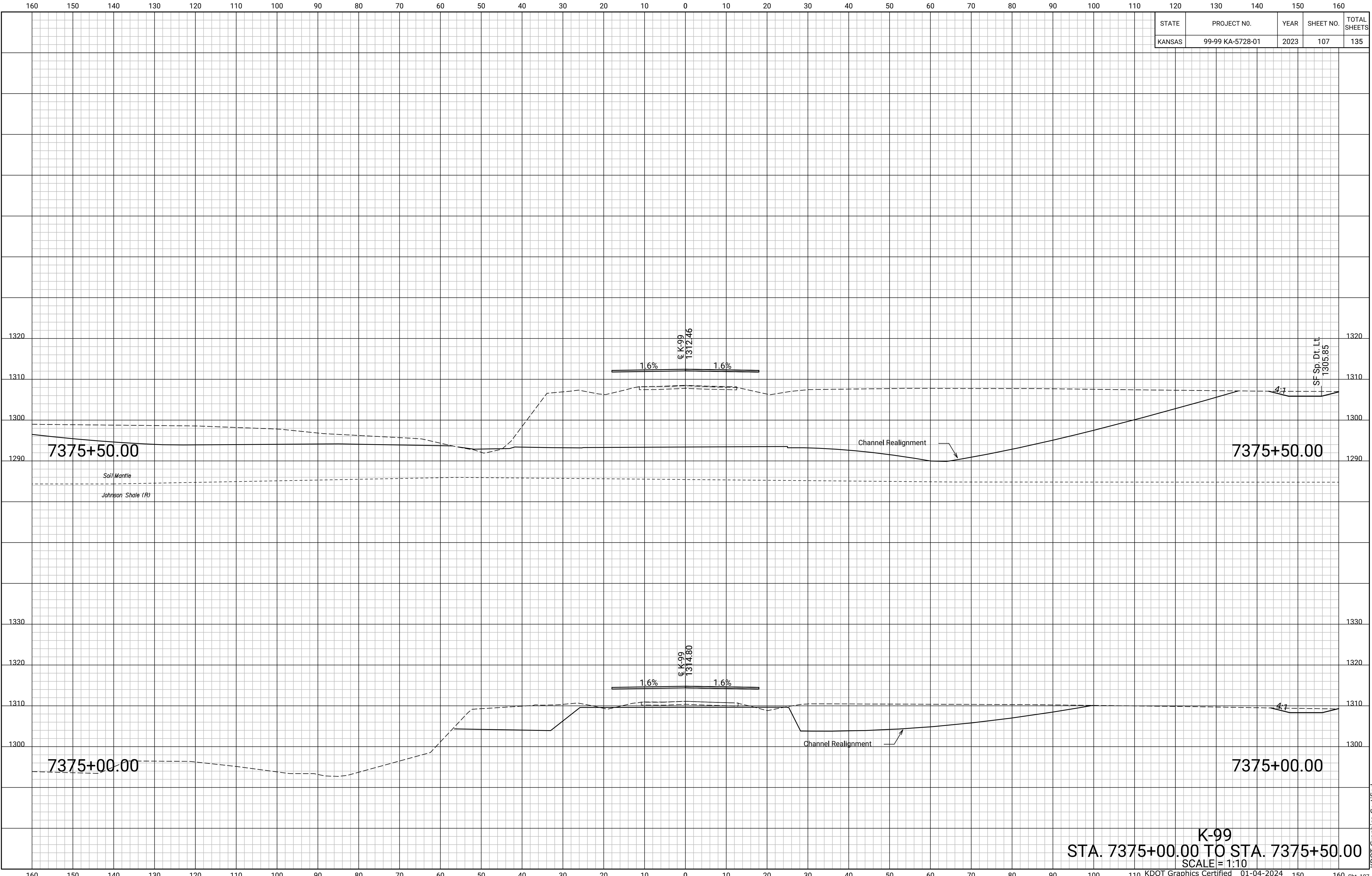


K-99 - Ext. Rt.
 STA. 7374+00.00 TO STA. 7374+91.16
 SCALE = 1:10

Plotted by: August Zuno 8-JAN-2024 16:33
 File: ka572801rxs-02.dgn

KDOT Graphics Certified

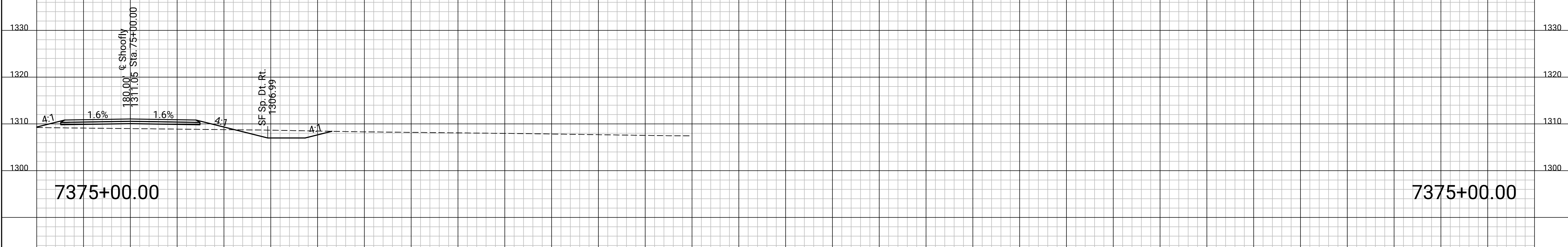
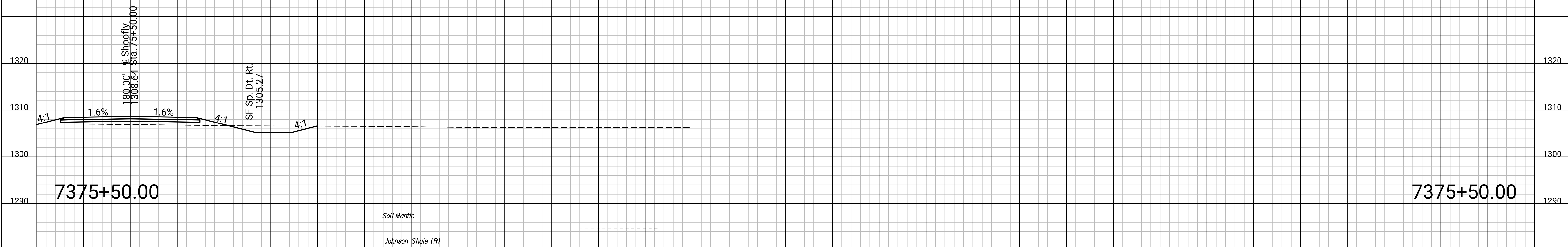
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	107	135



Plotted by: August Zuno 8-JAN-2024 16:35
 File: ka572801Trxs-02.dgn

K-99
 STA. 7375+00.00 TO STA. 7375+50.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024 Sht. 107

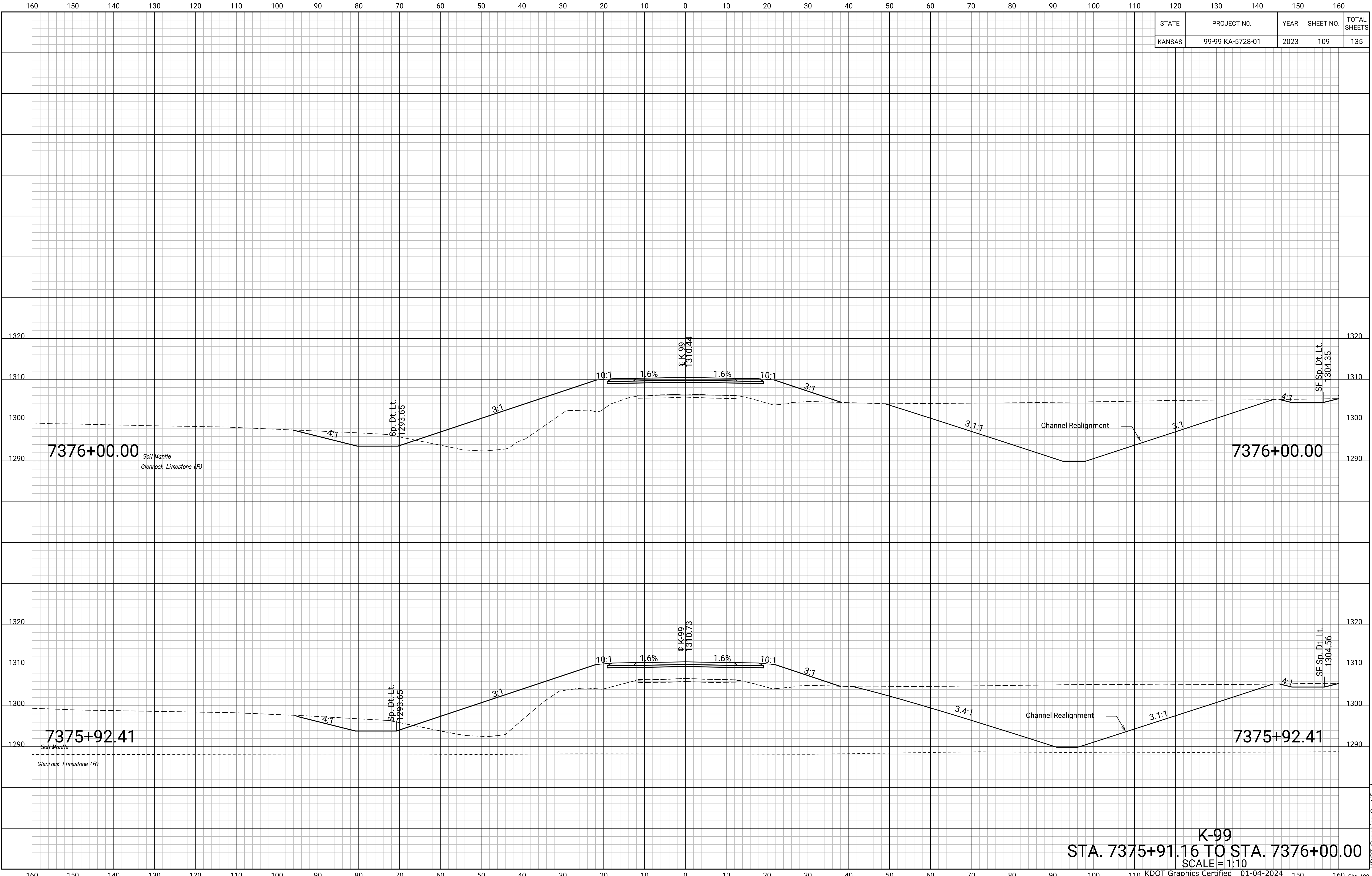
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	108	135



Plotted by : August Zuno 8-JAN-2024 16:33
File : ka572801Trxs-02.dgn

K-99 - Ext. Rt.
STA. 7375+00.00 TO STA. 7375+50.00
SCALE = 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	109	135

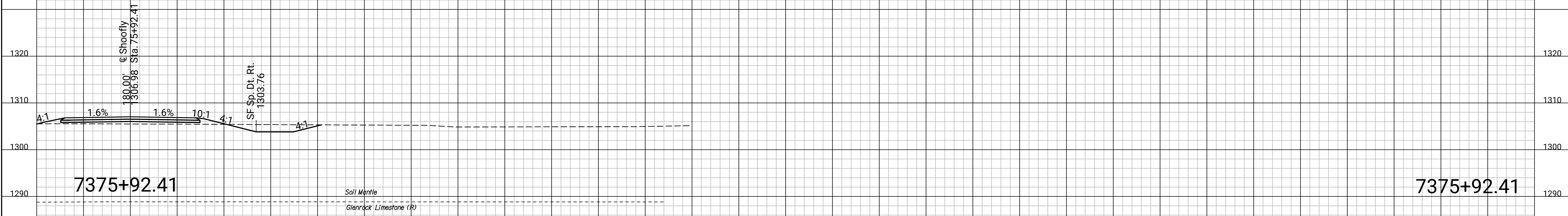
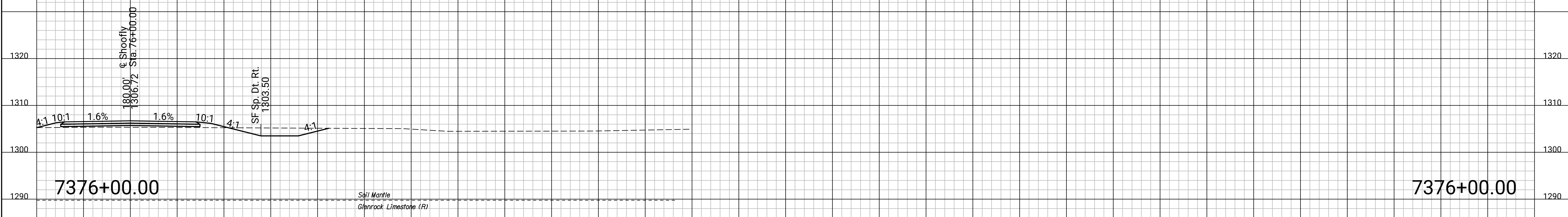


Plotted by : August Zuno
 File : ka572801Trxs-02.dgn
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K-99
 STA. 7375+91.16 TO STA. 7376+00.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

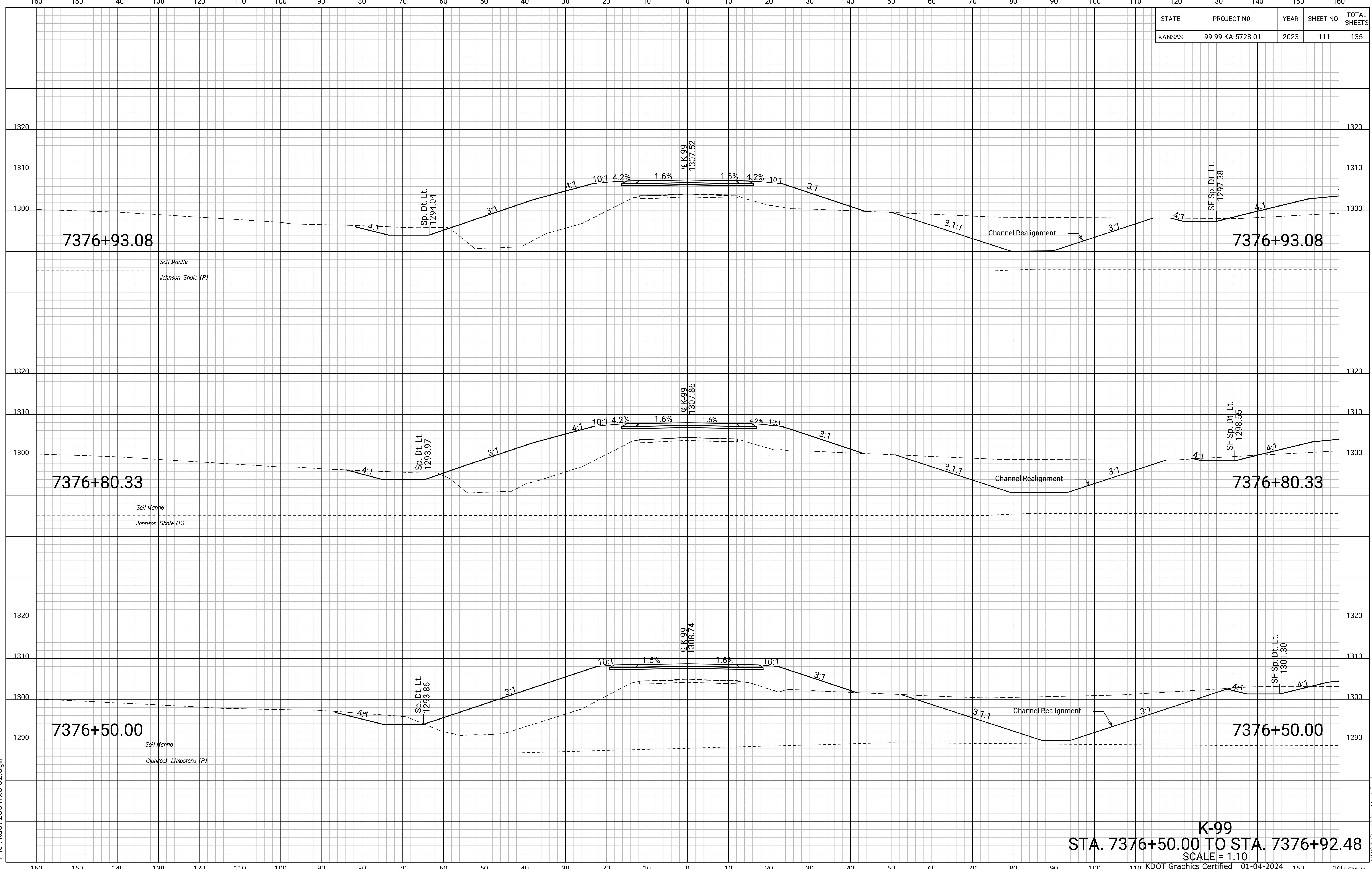
DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	110	135



K-99 - Ext. Rt.
 STA. 7375+91.16 TO STA. 7376+00.00
 SCALE = 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	111	135

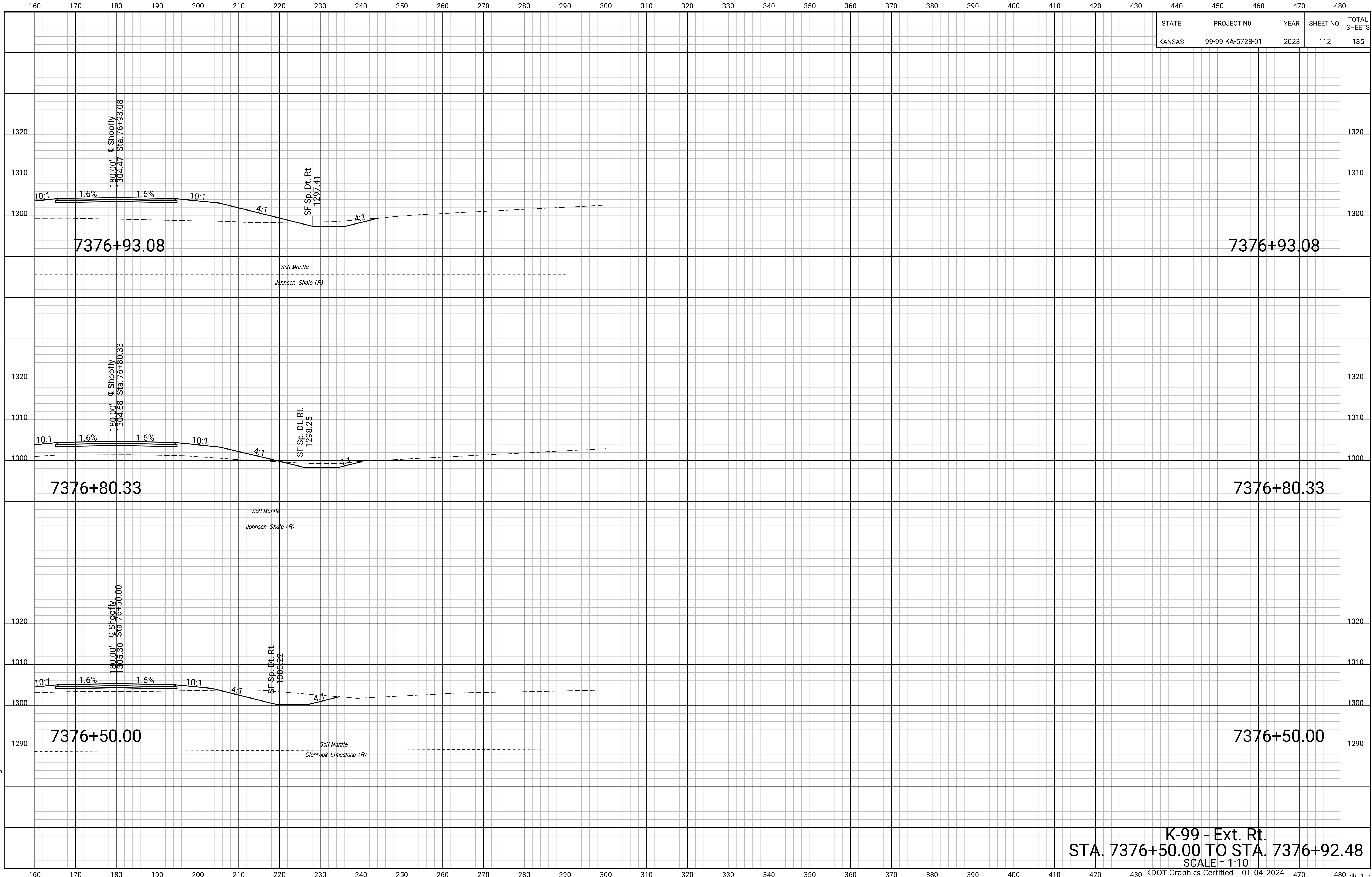


Plotted by: August Zuno
 File: ka572801rxs-02.dgn
 8-JAN-2024 16:35

K-99
 STA. 7376+50.00 TO STA. 7376+92.48
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	112	135

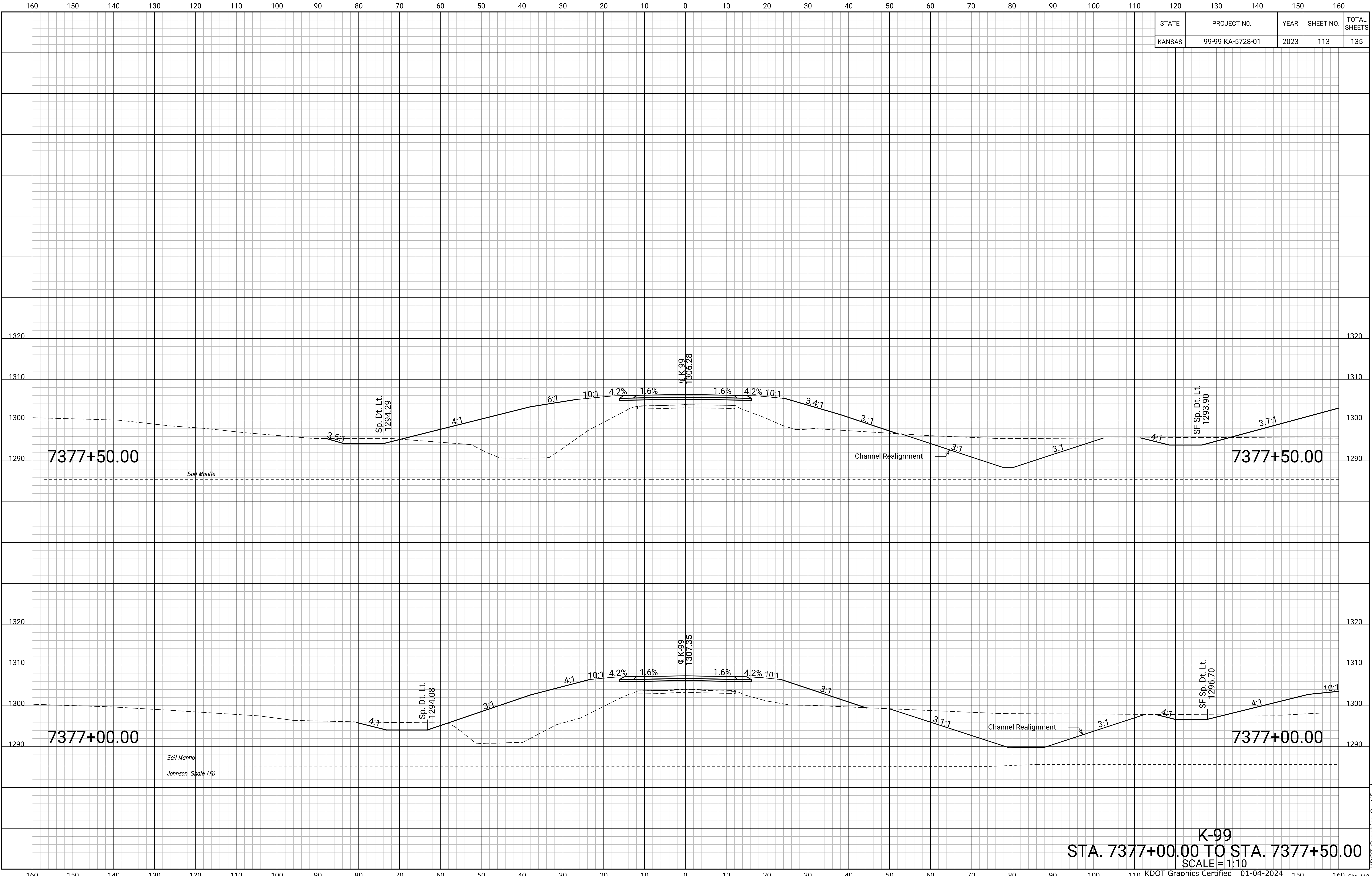


Plotted by : August Zuno
 File : ka572801Trxs-02.dgn
 8-JAN-2024 16:33

K-99 - Ext. Rt.
 STA. 7376+50.00 TO STA. 7376+92.48
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	113	135

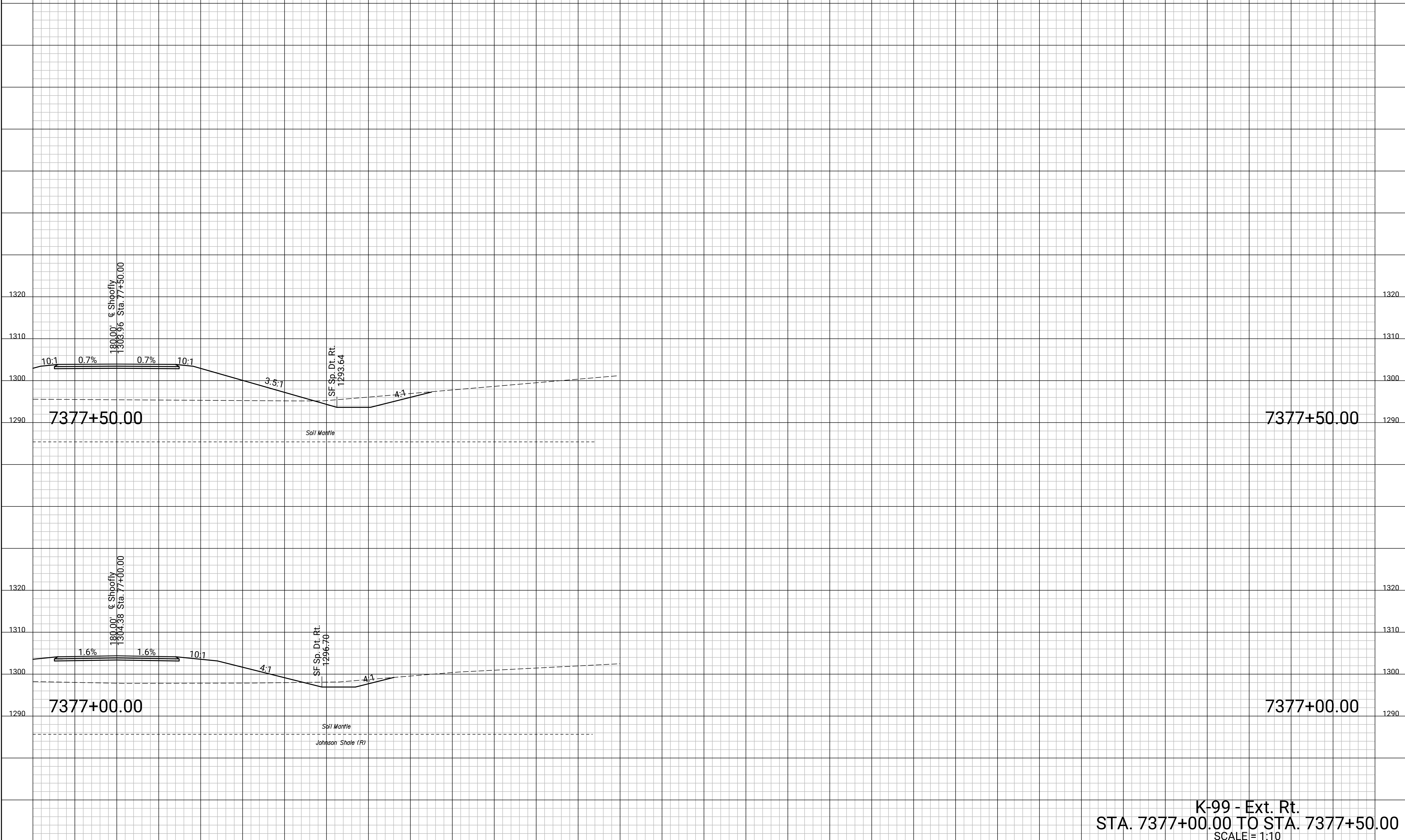


Plotted by: August Zuno 8-JAN-2024 16:35
 File: ka572801Trxs-02.dgn

K-99
 STA. 7377+00.00 TO STA. 7377+50.00
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	114	135

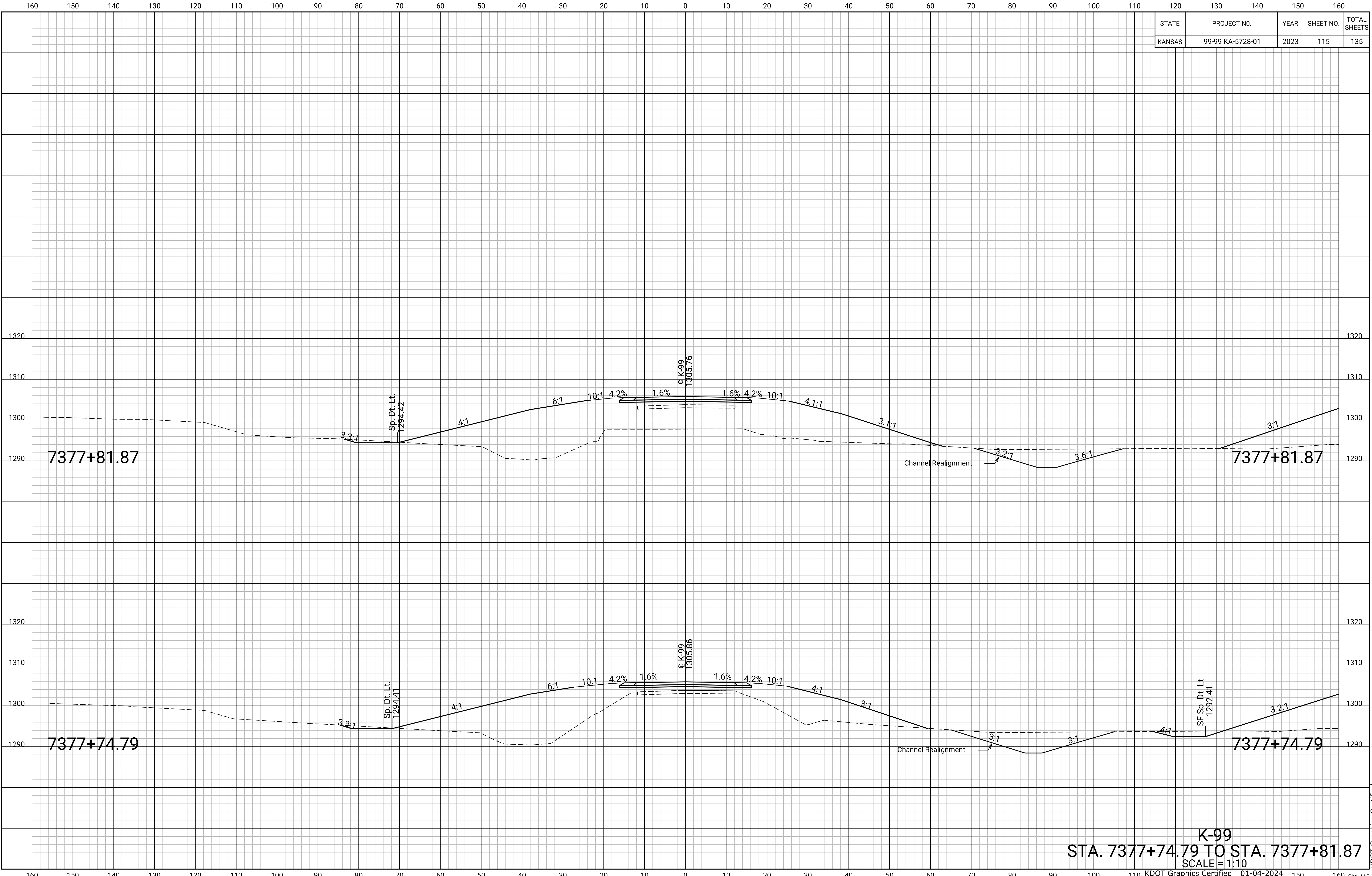


Plotted by: August Zuno
 File: ka572801Trxs-02.dgn
 8-JAN-2024 16:33

K-99 - Ext. Rt.
 STA. 7377+00.00 TO STA. 7377+50.00
 SCALE = 1:10
 8-DOT Graphics Certified 01-04-2024

8-DOT Graphics Certified

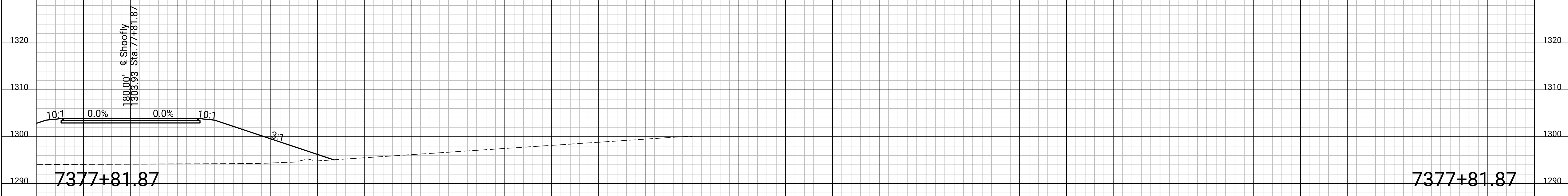
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	115	135



Plotted by: August Zuno 8-JAN-2024 16:36
 File: ka572801Trxs-02.dgn

K-99
 STA. 7377+74.79 TO STA. 7377+81.87
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024 Sht. 115

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	116	135

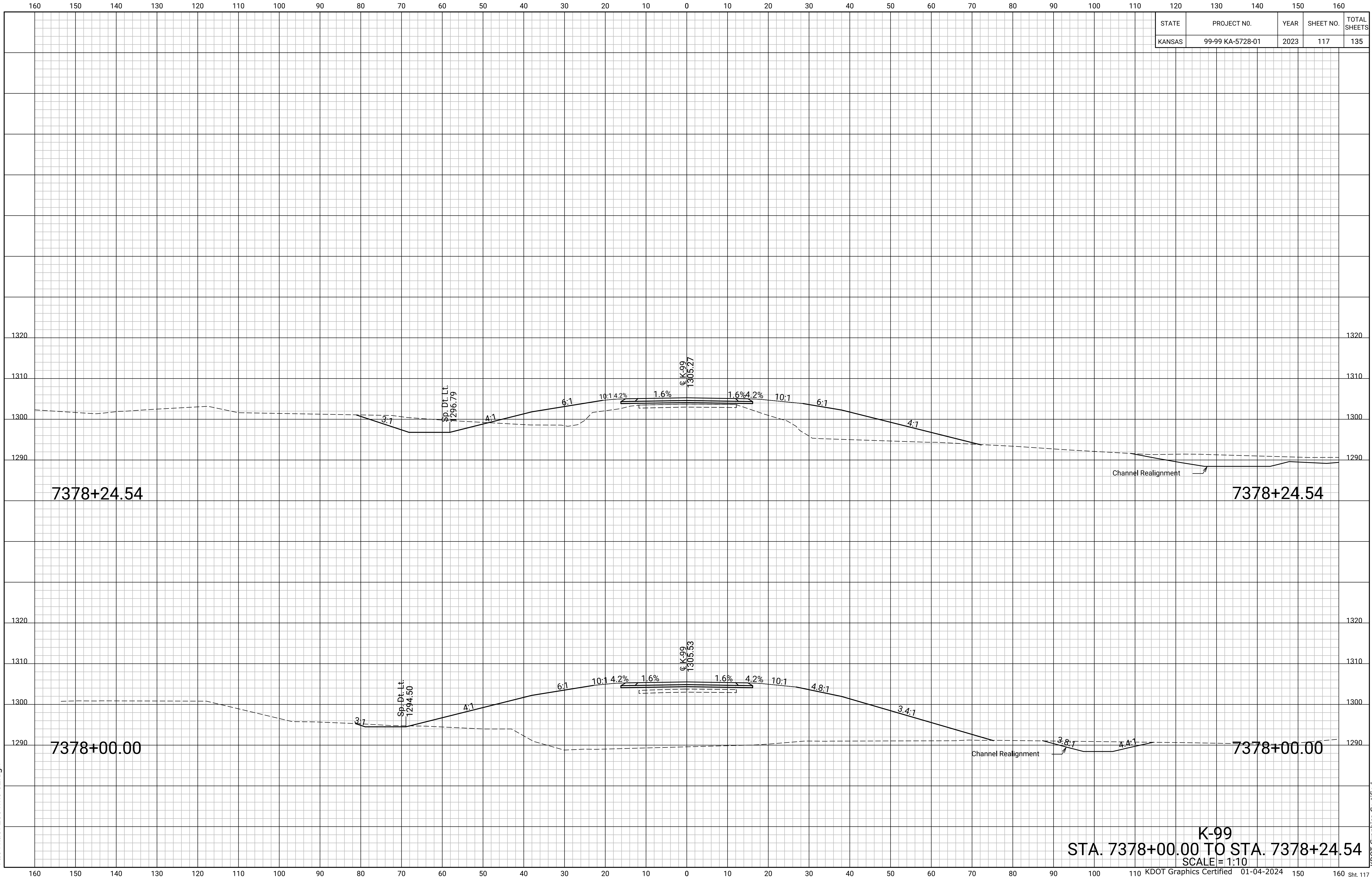


Plotted by: August Zuno 8-JAN-2024 16:33
File: ka572801Trxs-02.dgn

K-99 - Ext. Rt.
STA. 7377+74.79 TO STA. 7377+81.87
SCALE = 1:10
DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	117	135

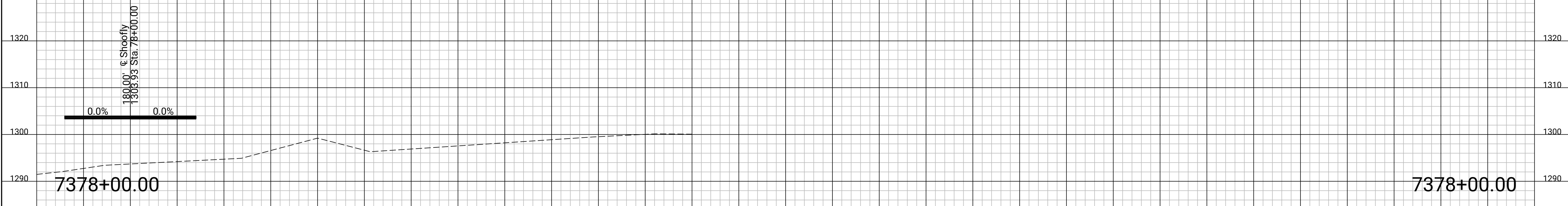
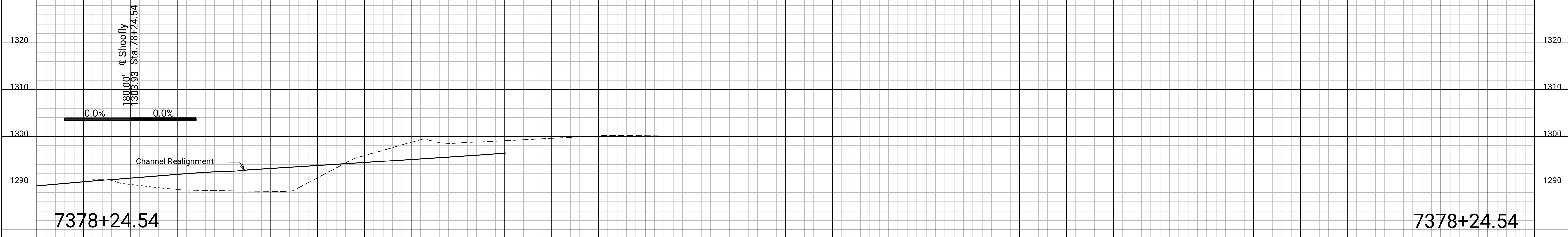


Plotted by: August Zuno 8-JAN-2024 16:36
 File: ka572801Trxs-02.dgn

K-99
 STA. 7378+00.00 TO STA. 7378+24.54
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	118	135

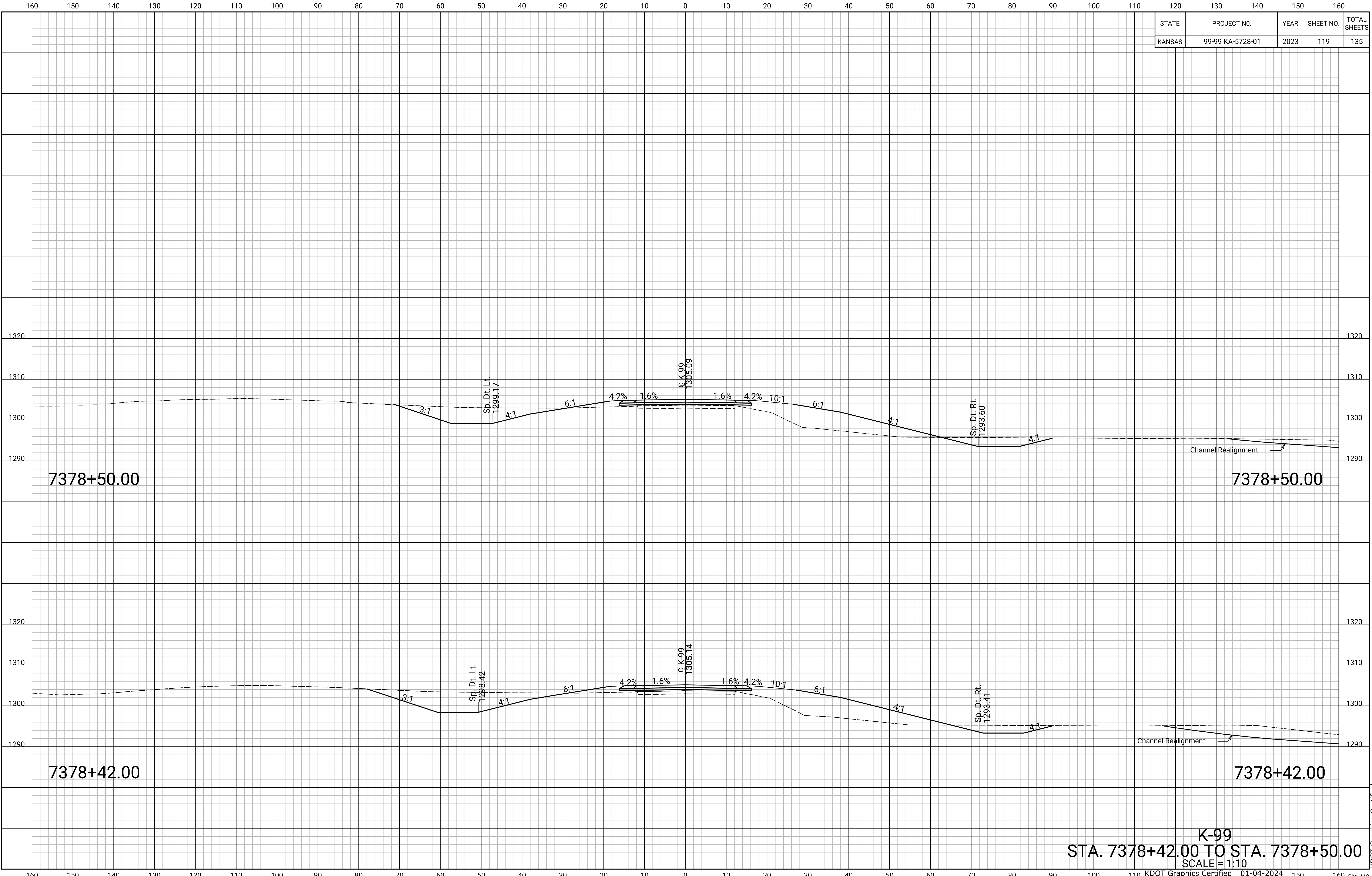


Plotted by: August Zuno
8-JAN-2024 16:33
File: ka572801Trxs-02.dgn

K-99 - Ext. Rt.
STA. 7378+00.00 TO STA. 7378+24.54
SCALE = 1:10
DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	119	135

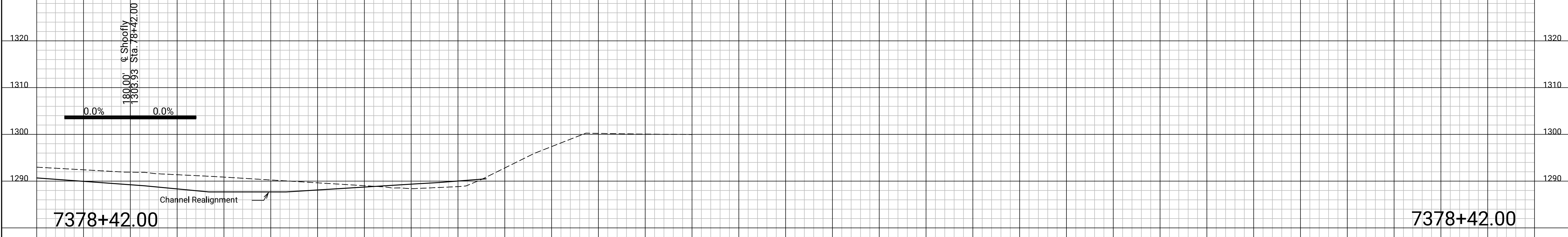
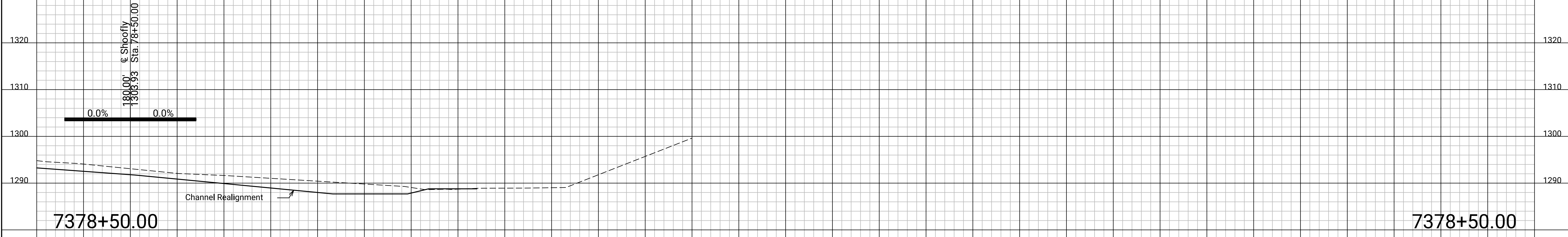


Plotted by: August Zuno
 File: ka572801Trxs-02.dgn
 8-JAN-2024 16:36

K-99
 STA. 7378+42.00 TO STA. 7378+50.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

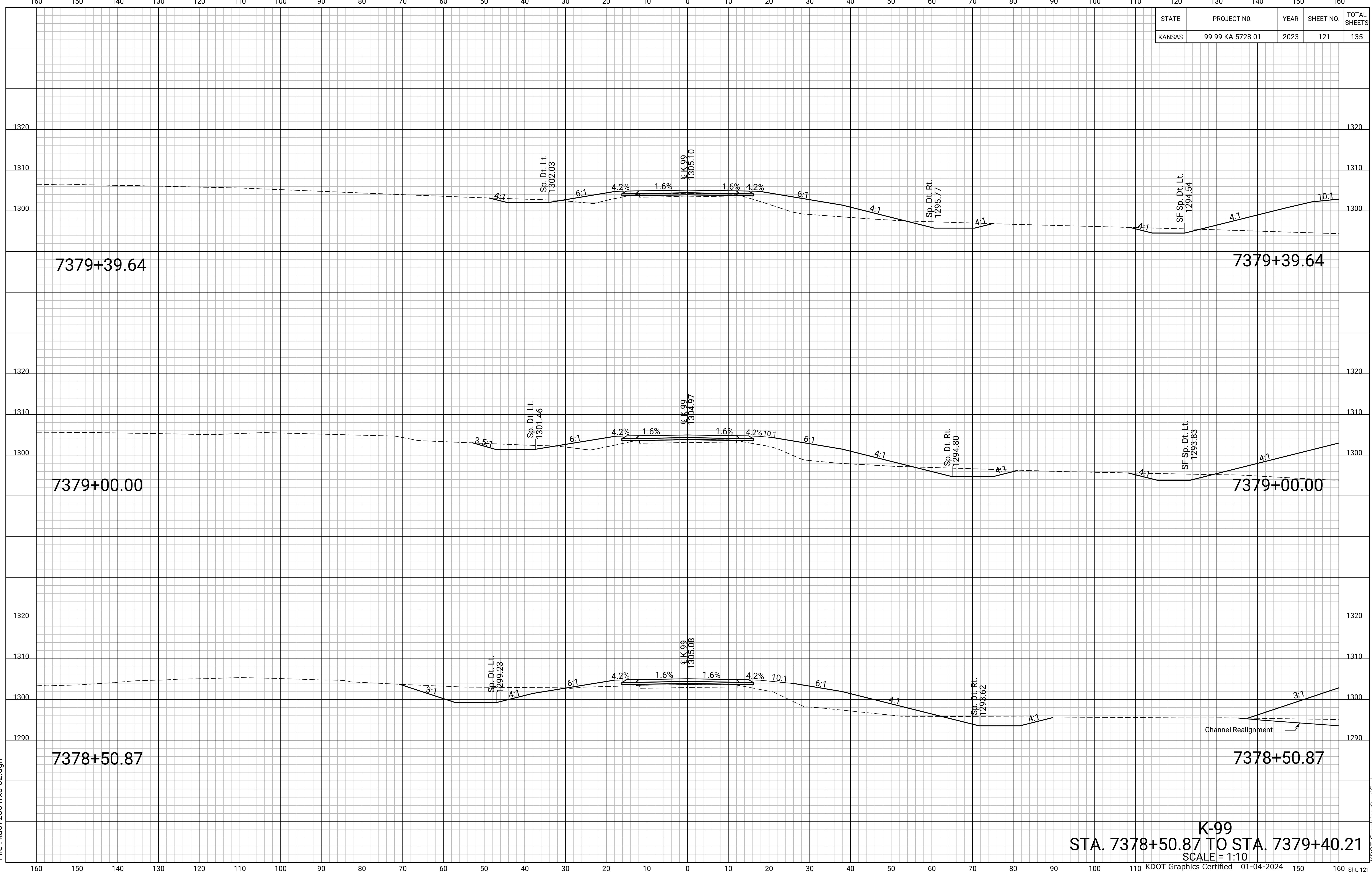
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	120	135



K-99 - Ext. Rt.
 STA. 7378+42.00 TO STA. 7378+50.00
 SCALE = 1:10

Plotted by: August Zuno
 File: ka572801Trxs-02.dgn
 8-JAN-2024 16:34

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	121	135

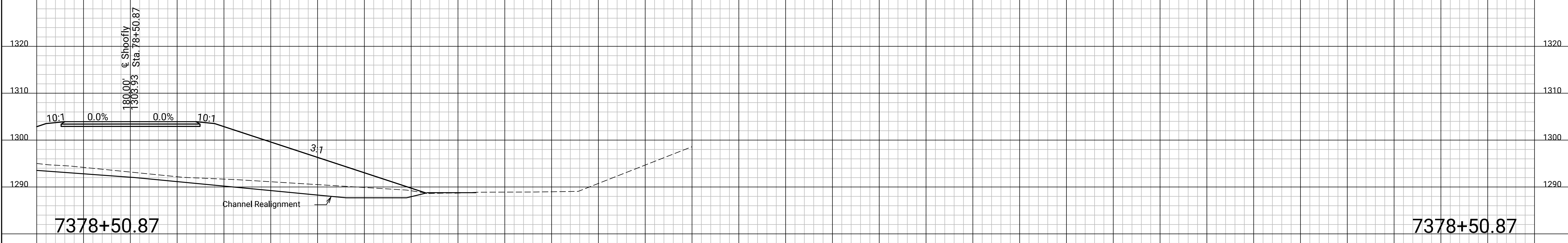
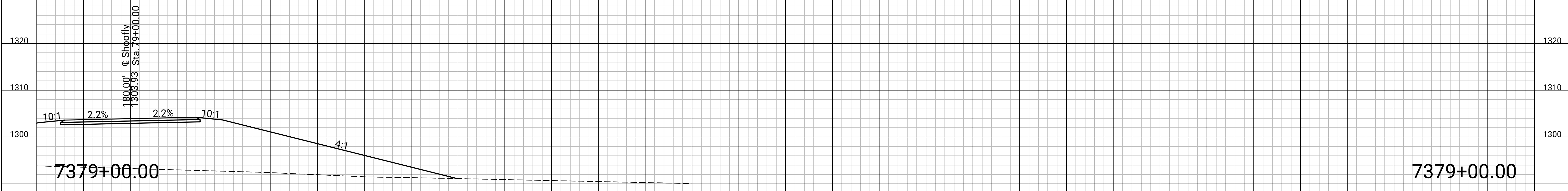
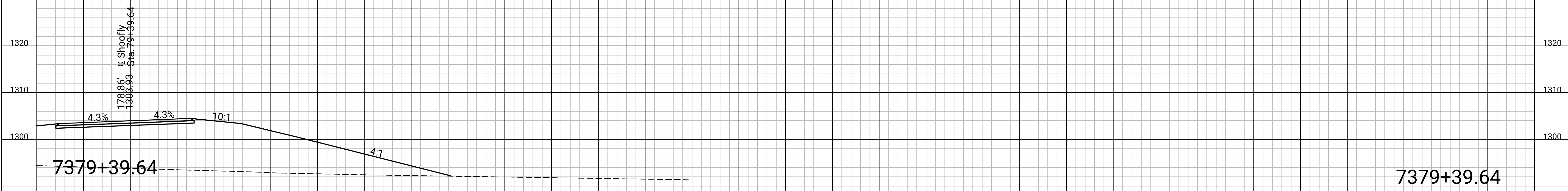


Plotted by : August Zuno 8-JAN-2024 16:36
 File : ka572801rxs-02.dgn

K-99
STA. 7378+50.87 TO STA. 7379+40.21
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	122	135

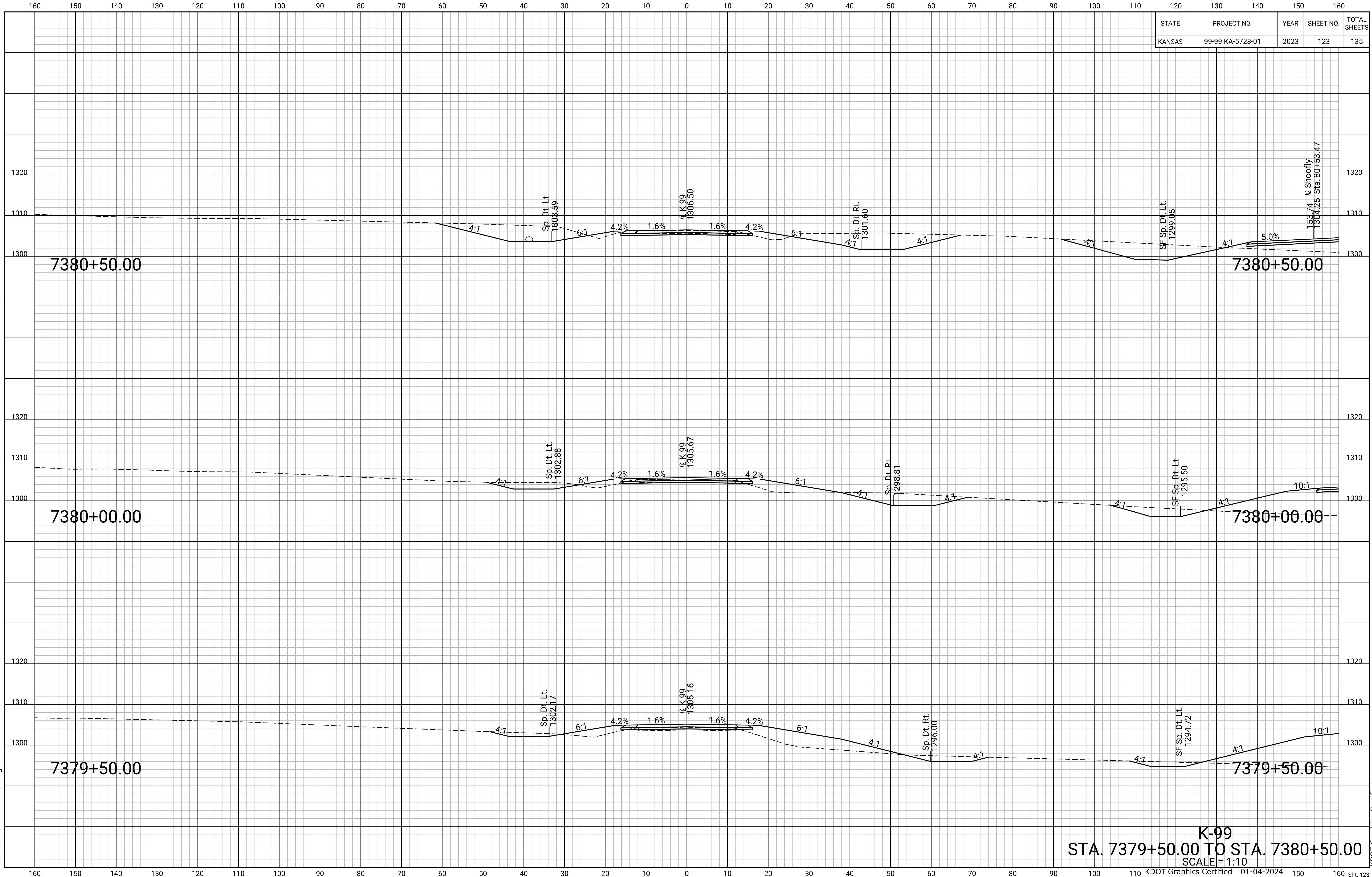


Plotted by : August Zuno
 File : ka572801Trxs-02.dgn
 8-JAN-2024 16:34

K-99 - Ext. Rt.
 STA. 7378+50.87 TO STA. 7379+40.21
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

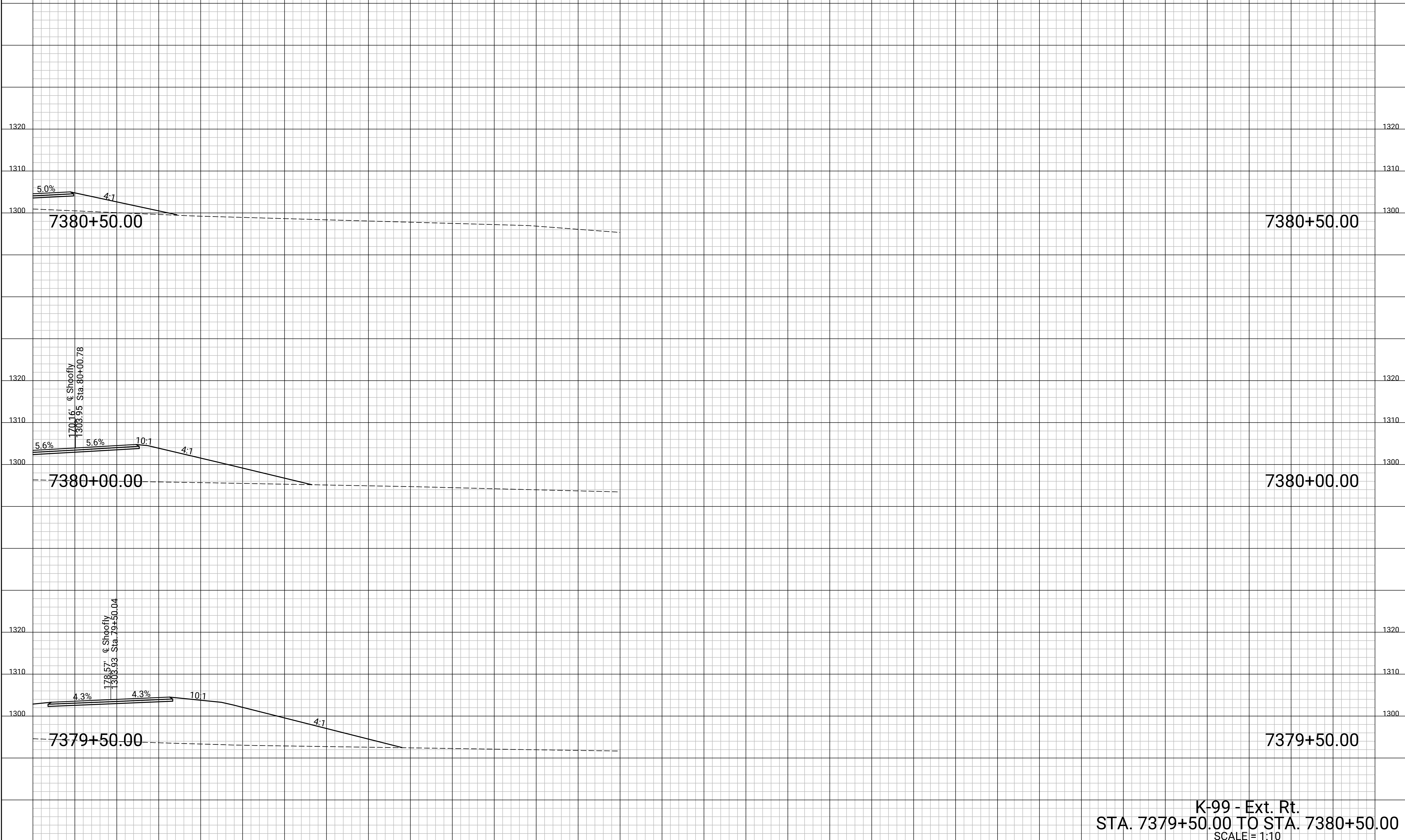
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	123	135



Plotted by : August Zuno 8-JAN-2024 16:36
 File : ka572801Trxs-02.dgn

K-99
 STA. 7379+50.00 TO STA. 7380+50.00
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024 Sht. 123

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	124	135

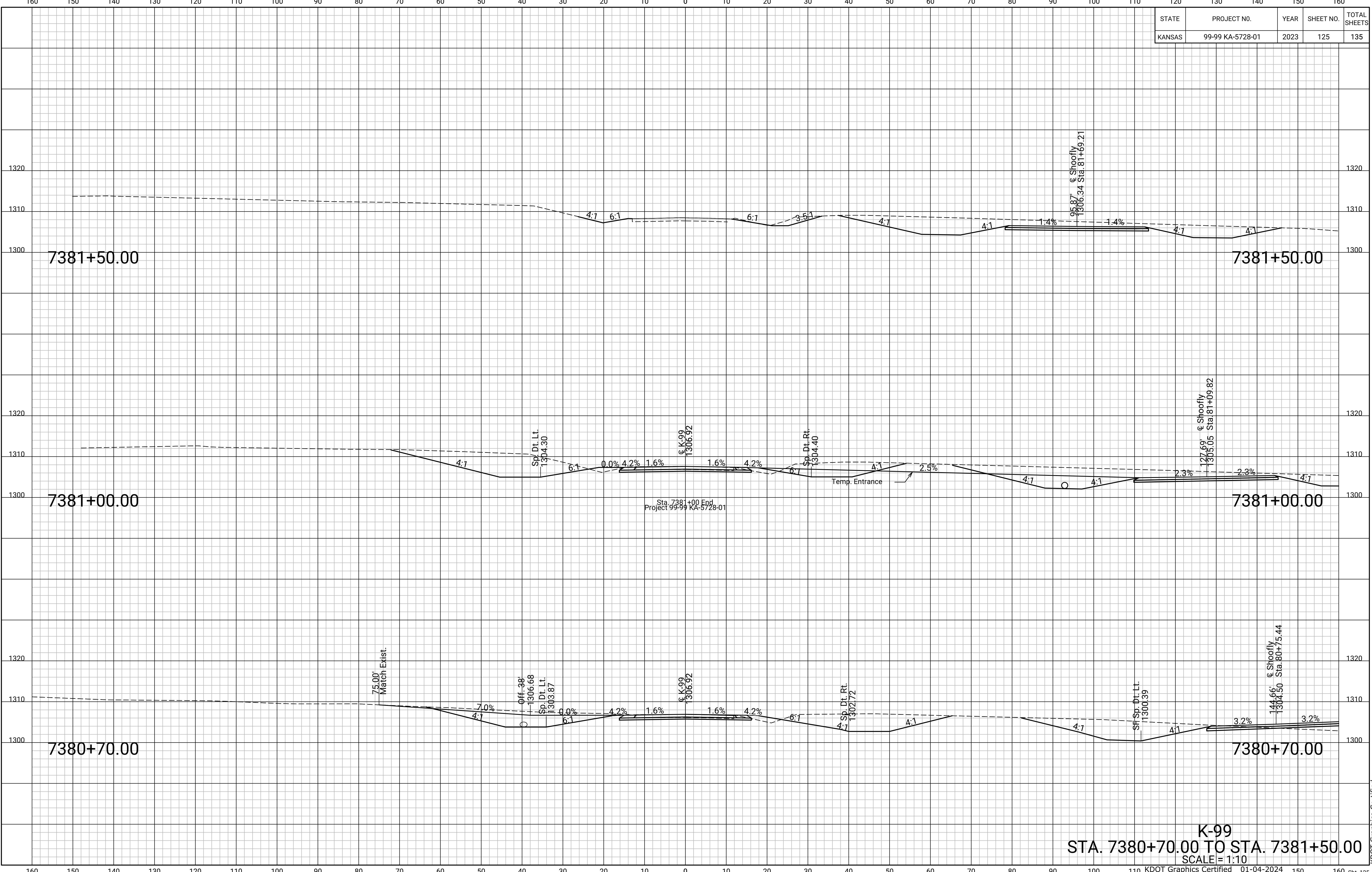


Plotted by: August Zuno 8-JAN-2024 16:34
File: ka572801Trxs-02.dgn

K-99 - Ext. Rt.
STA. 7379+50.00 TO STA. 7380+50.00
SCALE = 1:10
DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	125	135



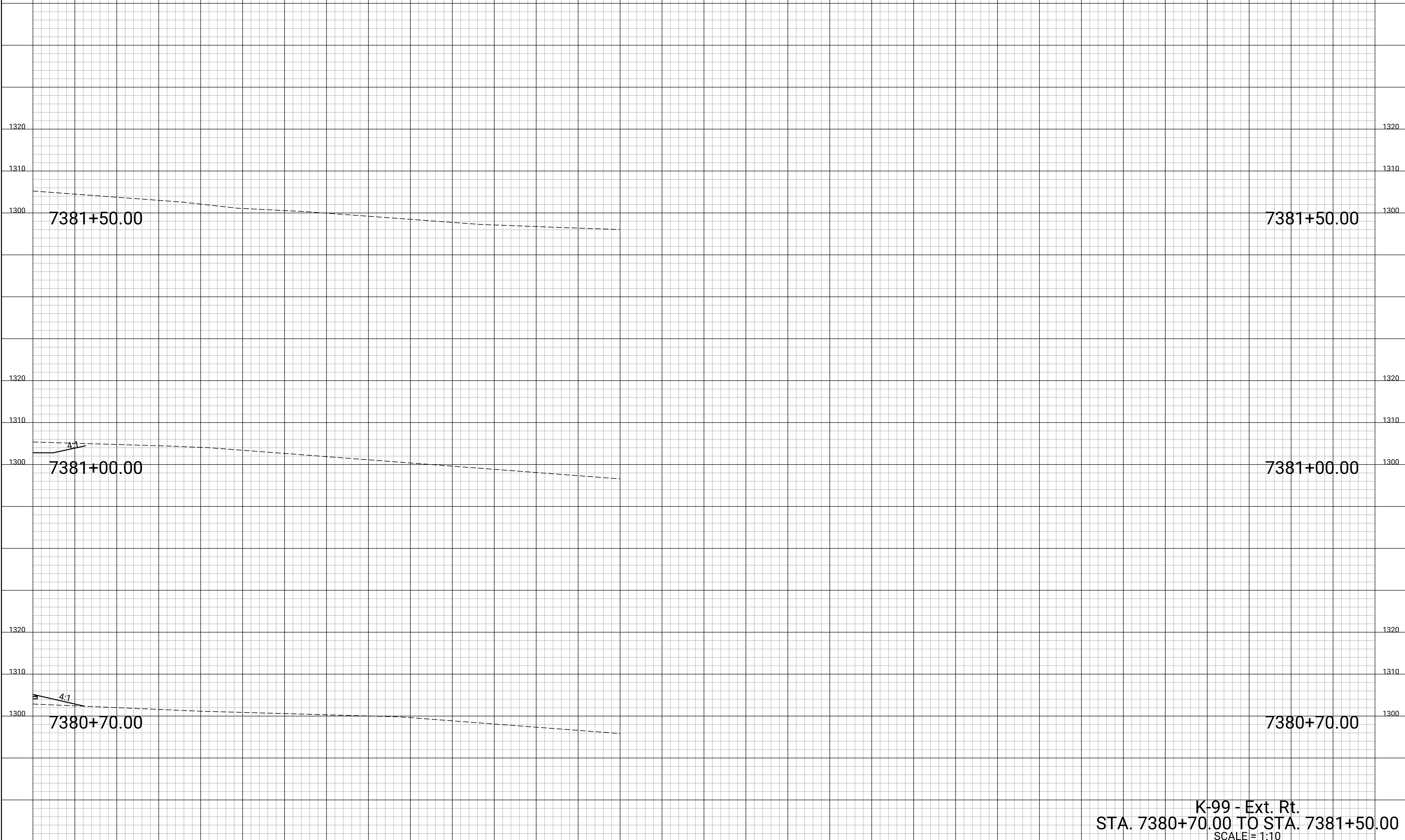
Plotted by: August Zuno 8-JAN-2024 16:36
 File: ka572801Trxs-02.dgn

K-99
 STA. 7380+70.00 TO STA. 7381+50.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

160 170 180 190 200 210 220 230 240 250 260 270 280 290 300 310 320 330 340 350 360 370 380 390 400 410 420 430 440 450 460 470 480

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	126	135

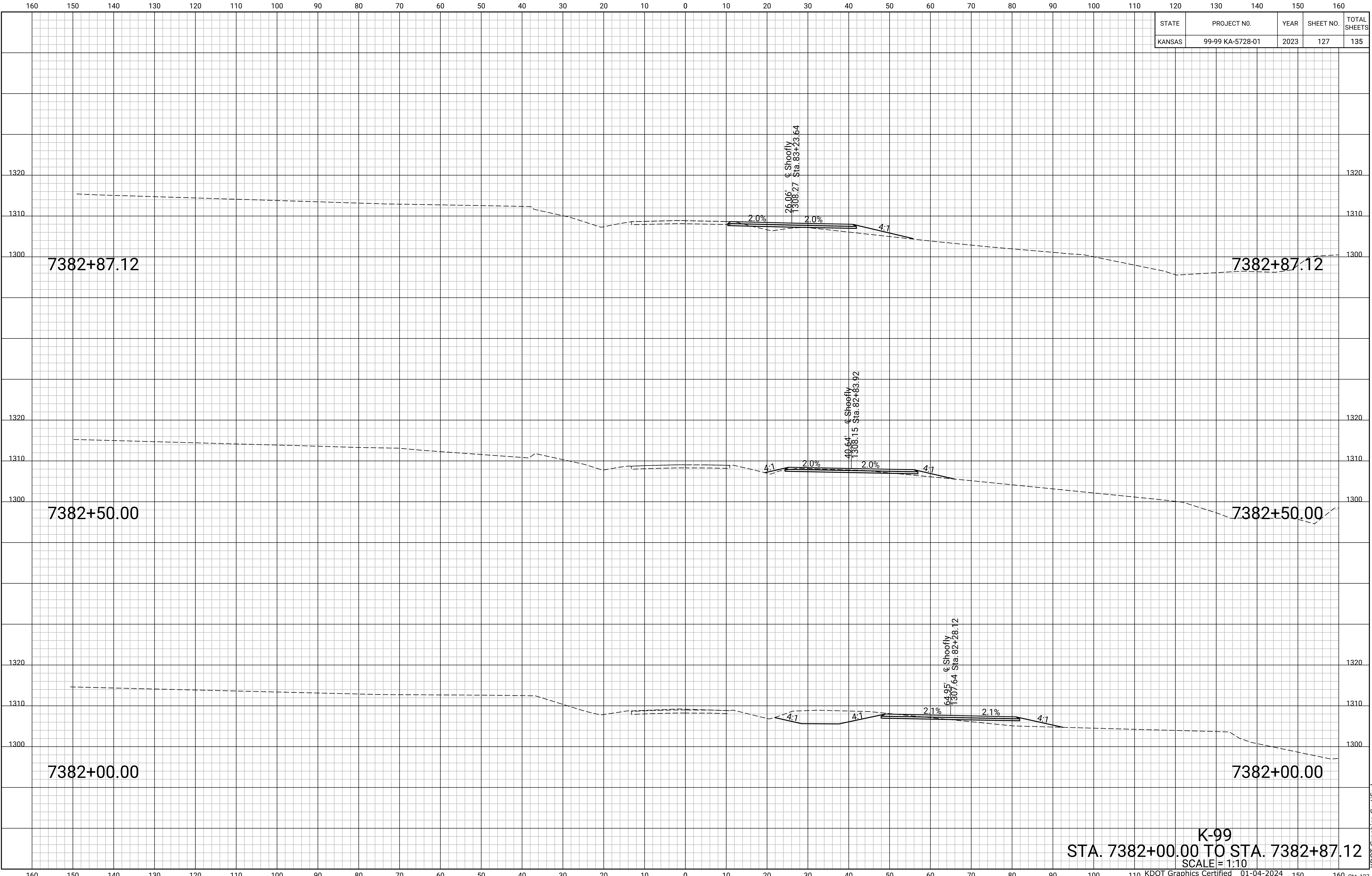


Plotted by : August Zuno 8-JAN-2024 16:34
 File : ka572801Trxs-02.dgn

K-99 - Ext. Rt.
 STA. 7380+70.00 TO STA. 7381+50.00
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	127	135

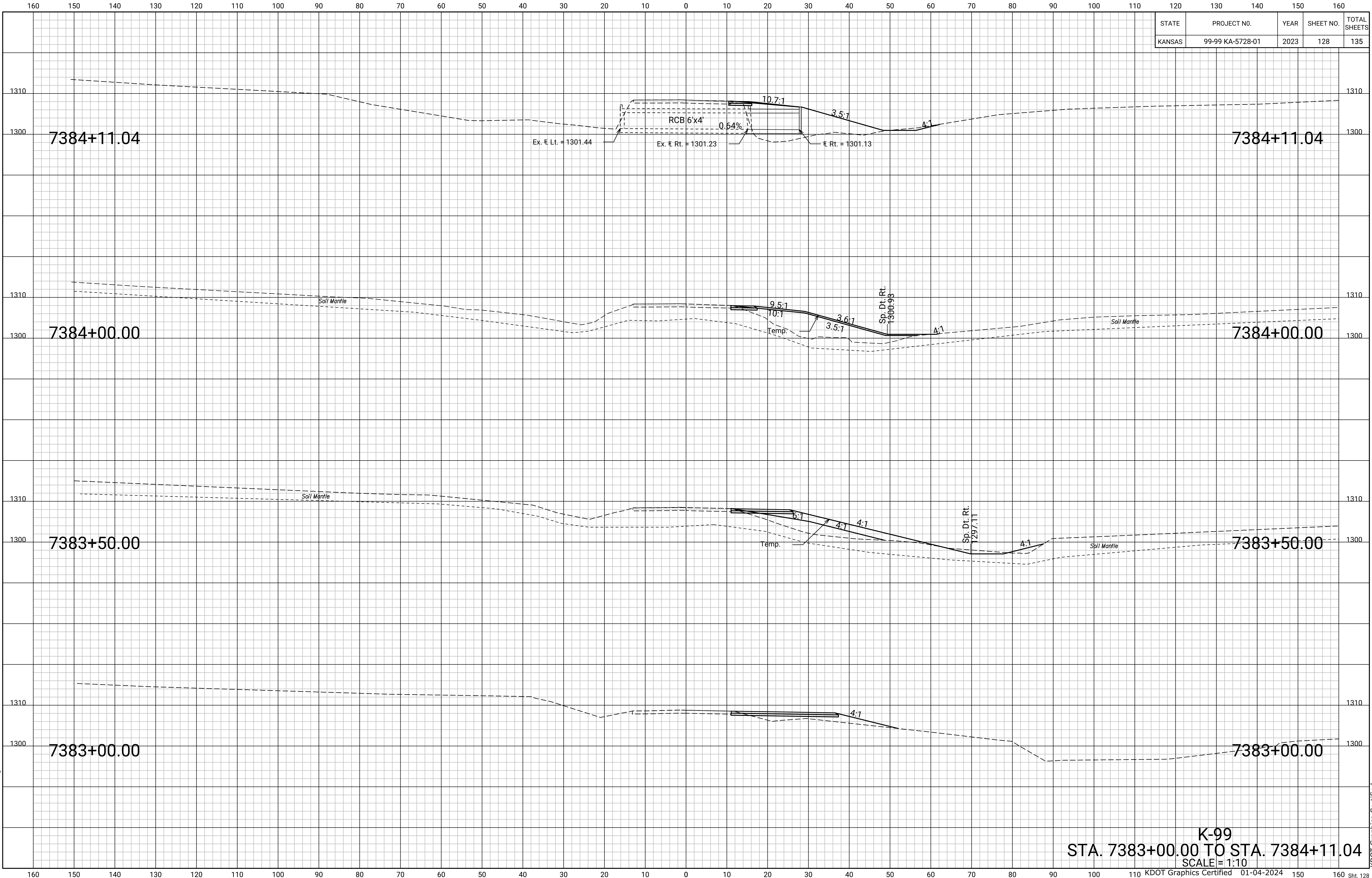


Plotted by: August Zuno
 File: ka572801Trxs-02.dgn
 8-JAN-2024 16:36

K-99
 STA. 7382+00.00 TO STA. 7382+87.12
 SCALE = 1:10
 DOT Graphics Certified 01-04-2024

DOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	128	135



Plotted by: August Zuno 8-JAN-2024 16:36
 File: ka572801Trxs-02.dgn

K-99
 STA. 7383+00.00 TO STA. 7384+11.04
 SCALE = 1:10
 KDOT Graphics Certified 01-04-2024

KDOT Graphics Certified

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	129	135

1310 7385+50.00 7385+50.00 1310

1310 7385+20.00 7385+20.00 1310

1310 7385+00.00 7385+00.00 1310

1310 7384+50.00 7384+50.00 1310

Sp. Dt. Lt.
1307.39

Sp. Dt. Rt.
1306.09

Sp. Dt. Rt.
1303.04

Soil Mantle
Bennett Shale (C)

Soil Mantle
Bennett Shale (C)

Soil Mantle
Bennett Shale (R)

Soil Mantle
Bennett Shale (R)

4:1

4:1

4:1

6.5:1

4:1

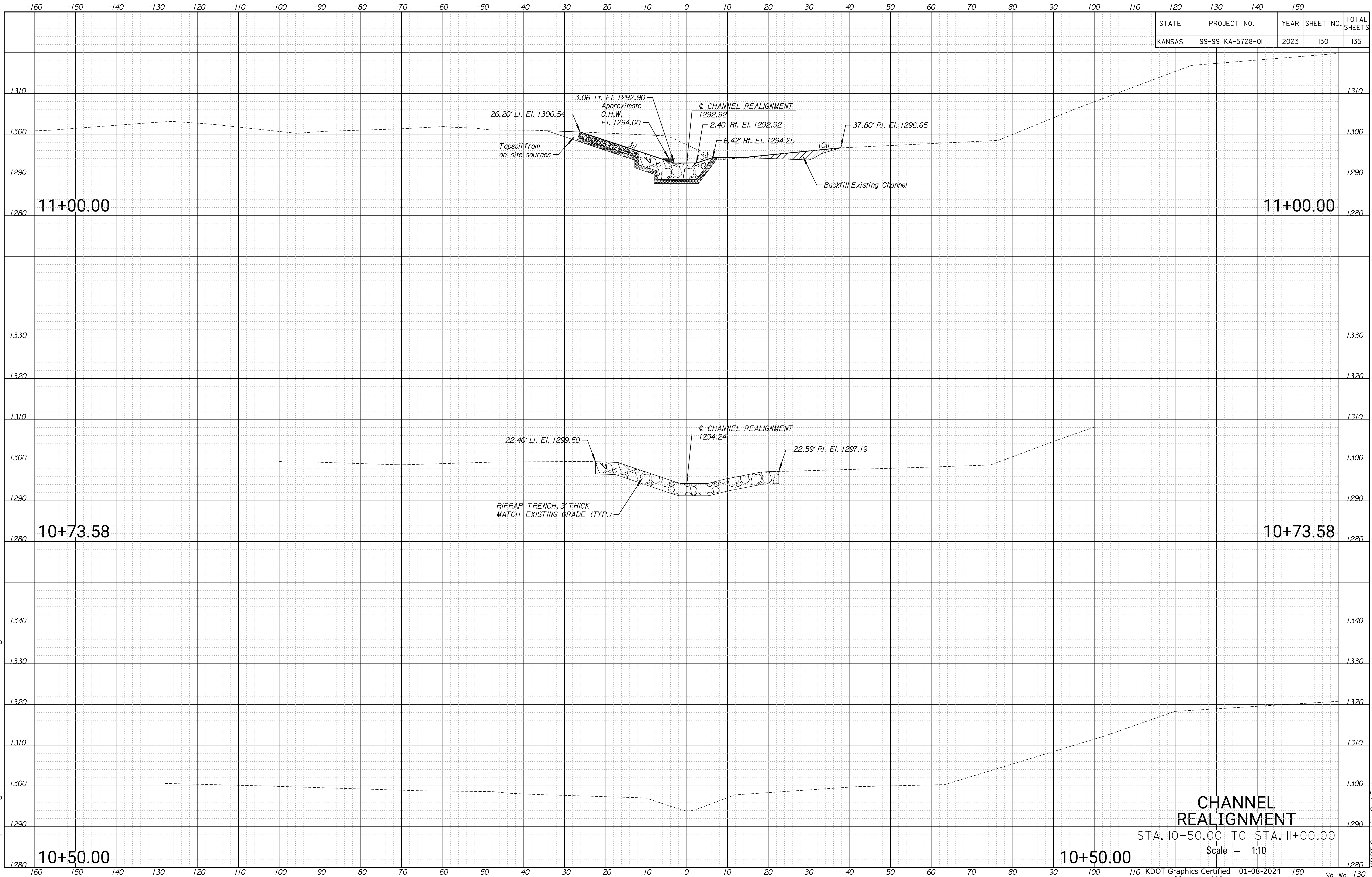
4:1

Temp.

K-99
STA. 7384+50.00 TO STA. 7385+50.00
SCALE=1:10

Plotted by : August Zuno 8-JAN-2024 16:37
File : ka572801Trxs-02.dgn

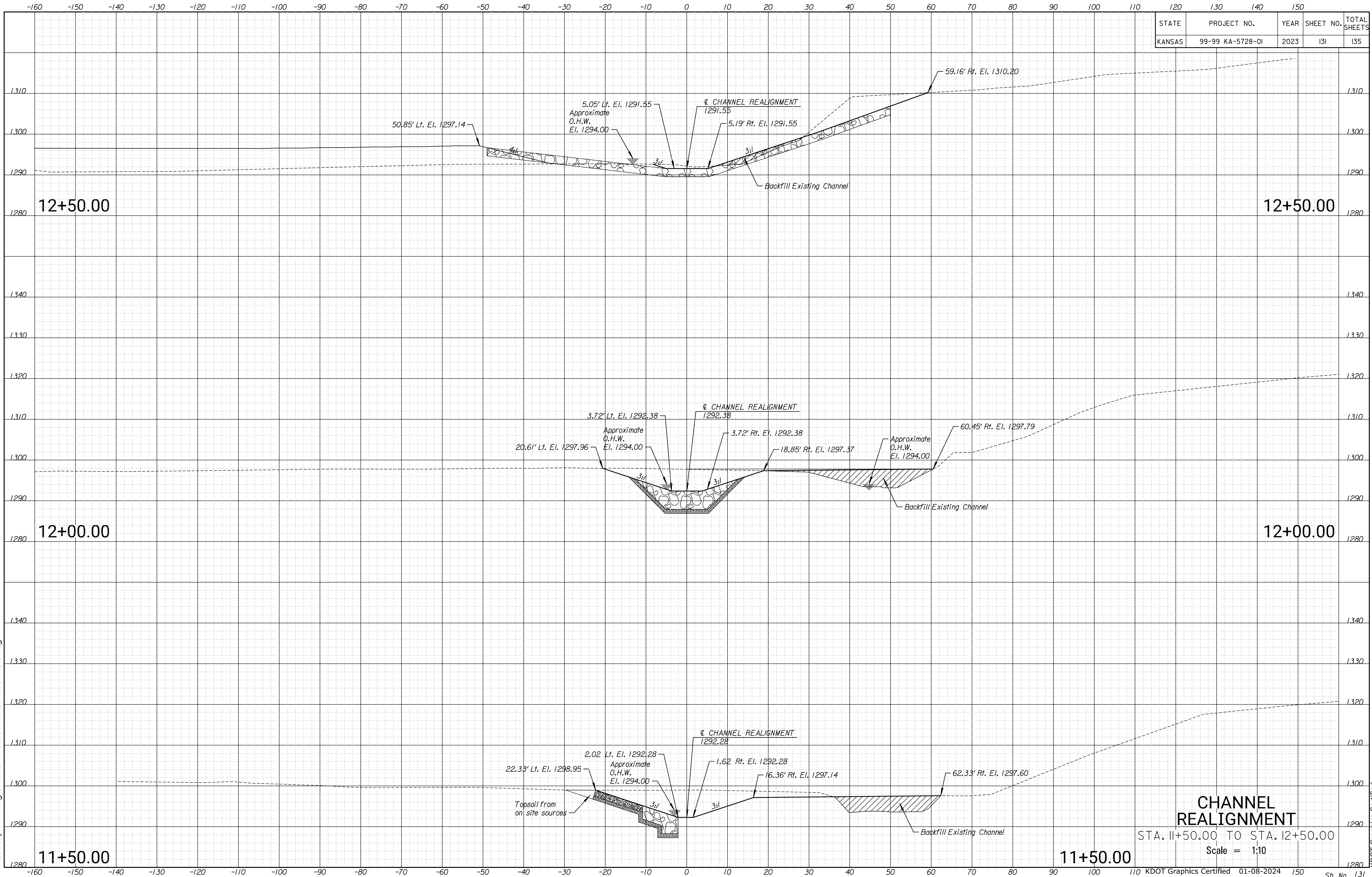
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	130	135



Drawn By : JHOVERSO Plotted : 01-08-24
 File : c:\pwworking\central01\2696672\ka572801\rs-01.dgn

CHANNEL REALIGNMENT
 STA. 10+50.00 TO STA. 11+00.00
 Scale = 1:10

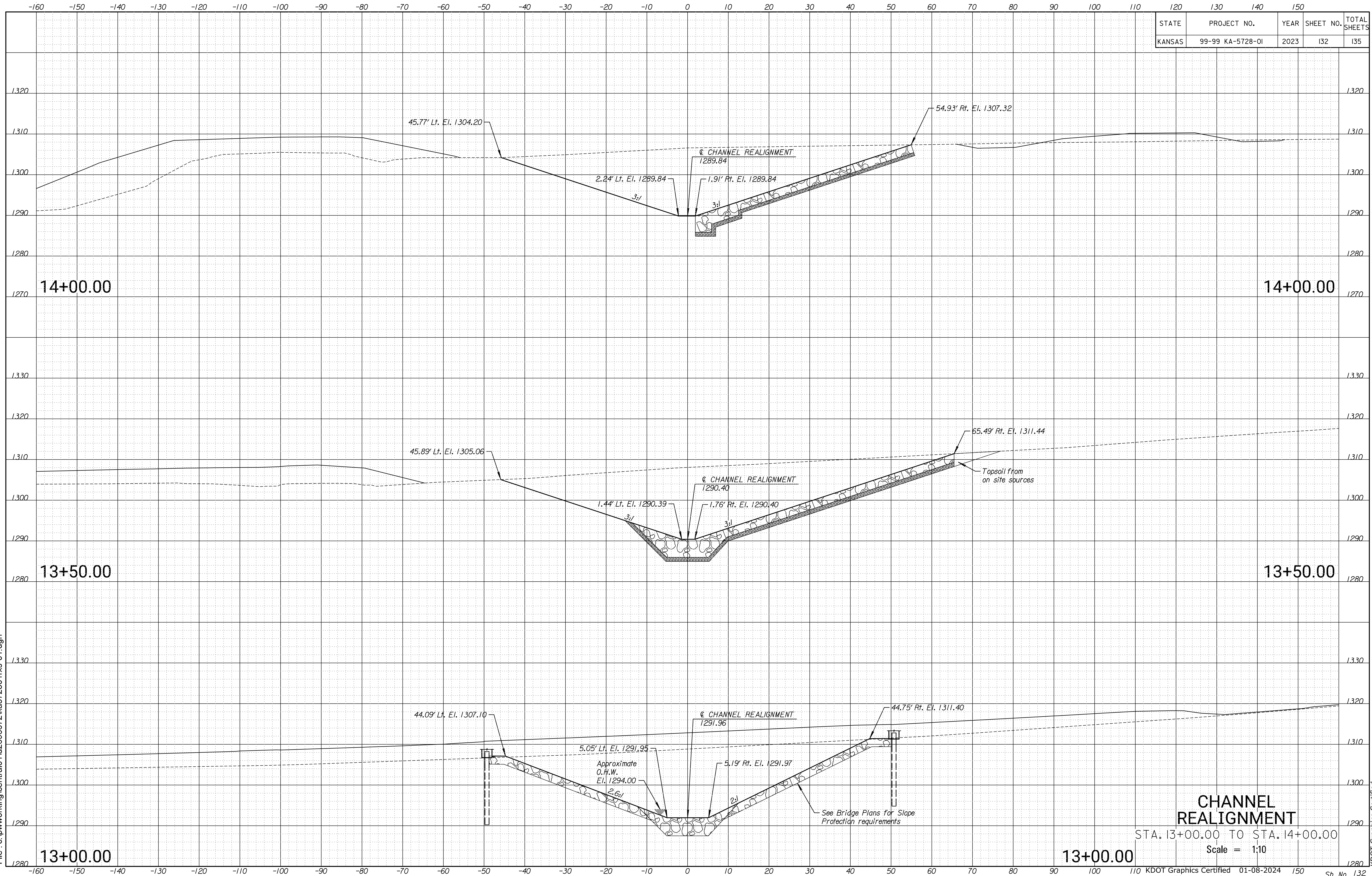
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	131	135



Drawn By : JHOVERSO Plotted : 01-08-24
 File : c:\pwworking\central\01\2696672\ka572801rxs-01.dgn

CHANNEL REALIGNMENT
 STA. 11+50.00 TO STA. 12+50.00
 Scale = 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	132	135



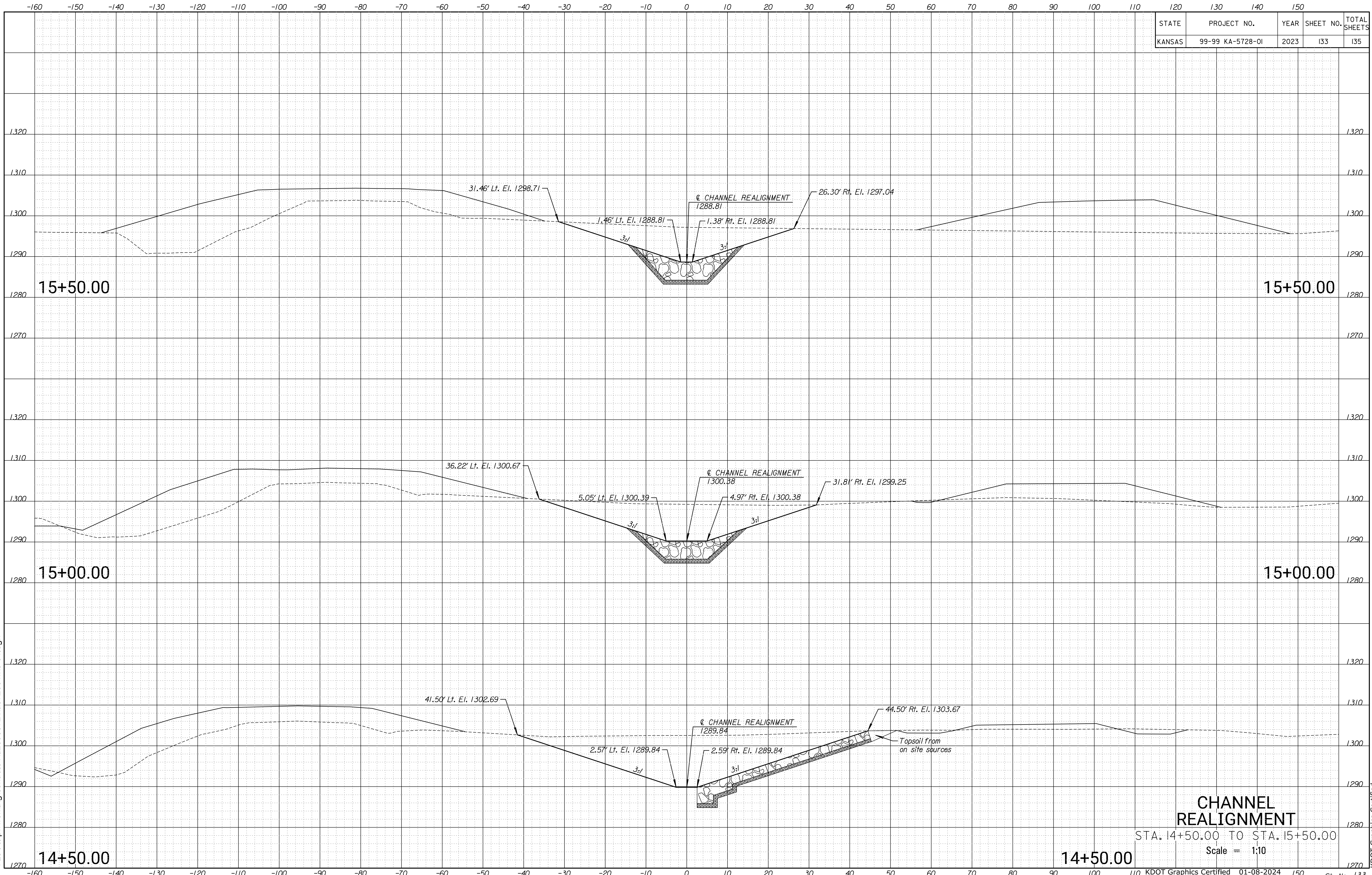
Drawn By : JHOVERSO Plotted : 01-08-24
 File : c:\pwworking\central01\2696672\ka572801\rs-01.dgn

CHANNEL REALIGNMENT

STA. 13+00.00 TO STA. 14+00.00

Scale = 1:10

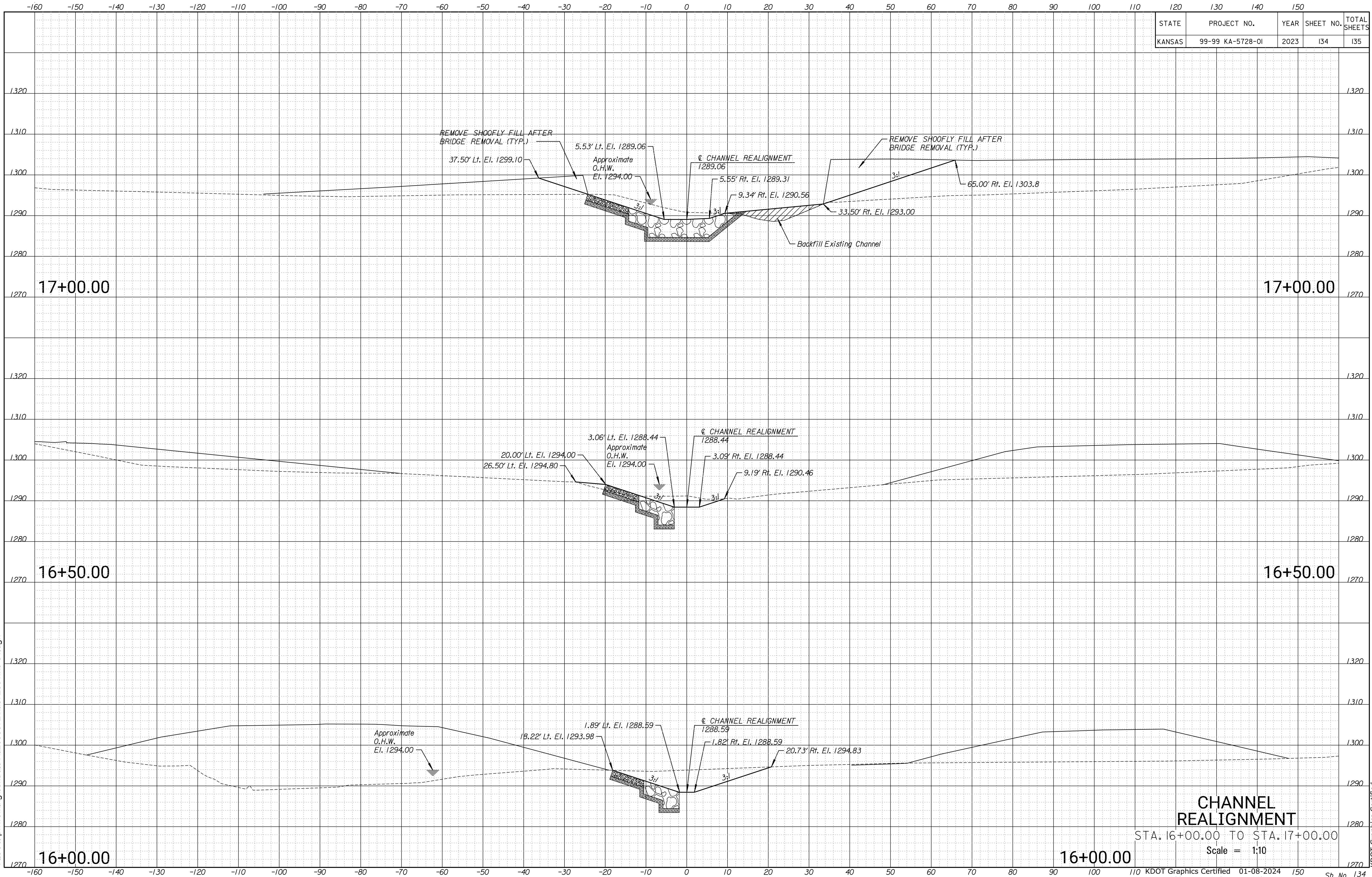
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	133	135



Drawn By : JHOVERSO Plotted : 01-08-24
 File : c:\pwworking\central01\2696672\ka572801\rs-01.dgn

CHANNEL REALIGNMENT
 STA. 14+50.00 TO STA. 15+50.00
 Scale = 1:10

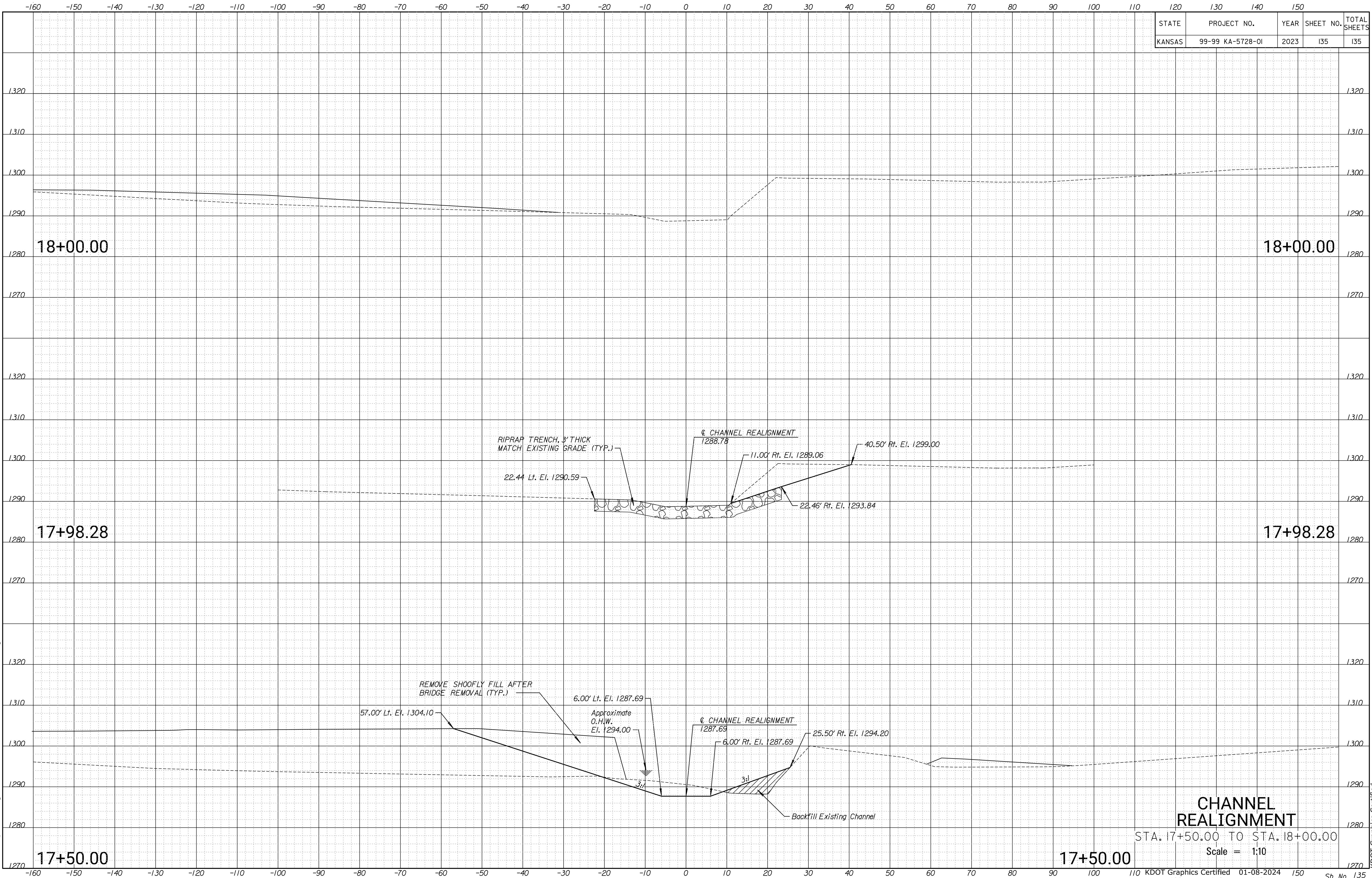
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	134	135



Drawn By : JHOVERSO Plotted : 01-08-24
 File : c:\pwworking\central01\2696672\ka572801\rs-01.dgn

CHANNEL REALIGNMENT
 STA. 16+00.00 TO STA. 17+00.00
 Scale = 1:10

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	99-99 KA-5728-01	2023	135	135



Drawn By : JHOVERSO Plotted : 01-08-24
 File : c:\pwworking\central01\2696672\ka572801rxs-01.dgn

CHANNEL REALIGNMENT
 STA. 17+50.00 TO STA. 18+00.00
 Scale = 1:10

KDOT Graphics Certified