

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	1	44

Federal Aid Proj. No. STP-C522(401)

GRADING
BRIDGE
SEEDING

STATE OF KANSAS
DEPARTMENT OF TRANSPORTATION



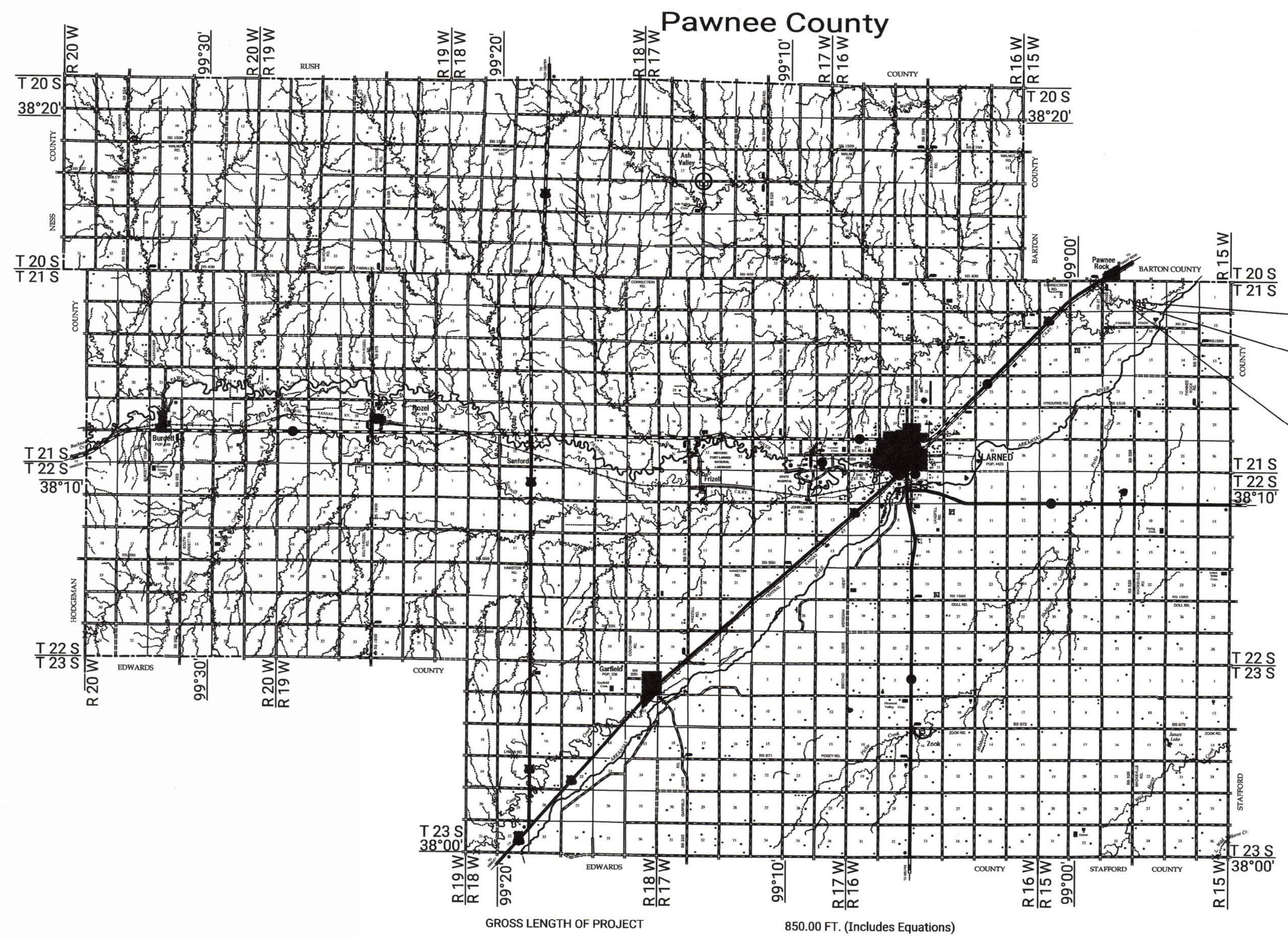
PLAN AND PROFILE OF PROPOSED
73 C-5224-01

FEDERAL AID PROJECT

INDEX OF SHEETS

- TITLE SHEET
- TYPICAL SECTIONS
- PLAN & PROFILE
- GUARDRAIL
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- SUMMARY OF PIPE CULVERTS
- BRIDGE DETAILS
- BRIDGE EXCAVATION
- STANDARD PILE DETAILS
- SUPPORTS & SPACERS FOR REINFORCING STEEL
- SUMMARY OF QUANTITIES
- TEMPORARY EROSION & POLLUTION CONTROL
- SEEDING
- TRAFFIC CONTROL
- CROSS SECTIONS

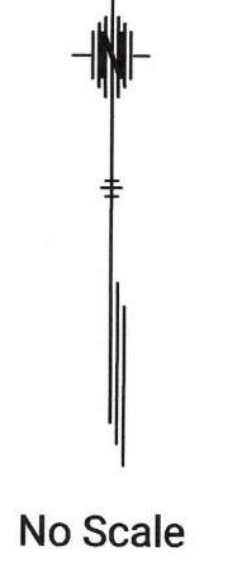
DATE	
BY	
SURVEY	
CADD TECHNICIAN	
DESIGNERS	
SQUAD	



STA. 97+50.00 BEGIN
KDOT Project 73 C-5224-01

Sta. 102+85.00
Br. No. 000730643005409 (OS 93)
30' - 40' - 30' R.C. Haunched
Slab Span Bridge (RCSH)
28' - 0" Roadway

STA. 106+00.00 END
KDOT Project 73 C-5224-01



DESIGN DESIGNATION

AADT (2023) 45
AADT (2043) 55
DHV
D
T
V 55 MPH
C of A
Clear Zone

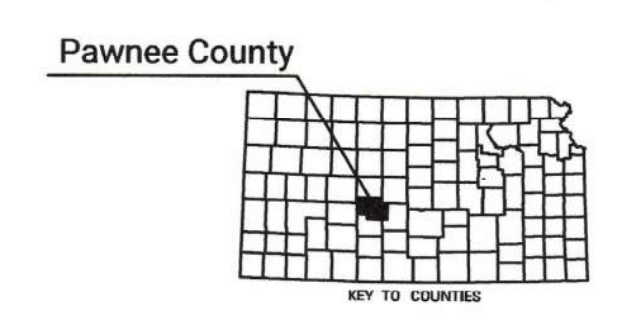
CONVENTIONAL SIGNS

COUNTY LINE	CENTER LINE OF PROJECT
CITY LIMITS	TERRACE
STATE OR NATIONAL LINE	CULVERTS
TOWNSHIP, SECTION or GRANT LINE	DROP INLET & STORM SEWER
PROPERTY LINE	ACCESS CONTROL
HIGHWAY FENCE	POWER POLE
EXISTING FENCE	TELEPHONE POLE
GUARDRAIL	MARSH
CONSTRUCTION LIMITS	HEDGE
RIGHT OF WAY LINE	TREES
TRAVELED WAY	PROFILE ELEVATION
RAILROADS	STREAM or CREEK

GROSS LENGTH OF PROJECT 850.00 FT. (Includes Equations)

EXCEPTIONS

NET LENGTH OF PROJECT	850.00 FT.	0.161 MILES
NET LENGTH OF BRIDGES	102.50 FT.	0.019 MILES
NET LENGTH OF ROAD	747.50 FT.	0.142 MILES



NOTE: This project will be closed to all traffic during construction.



217 N. Douglas, ELLSWORTH, KANSAS 67439
(785) 472-3163 FAX (785) 472-3817

Approved: Jul 16, 2024
Date

Dr. M. Shil
State Transportation Engineer

By: *Dawn Impfhuske*
Assistant Chief, Bureau of Local Projects

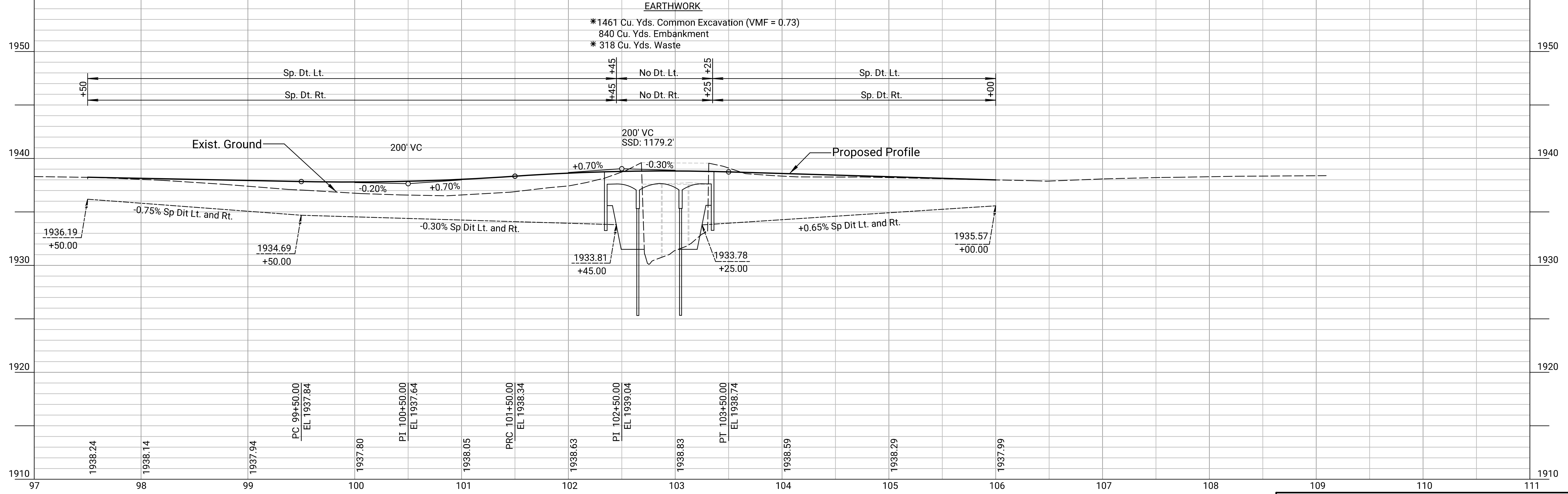
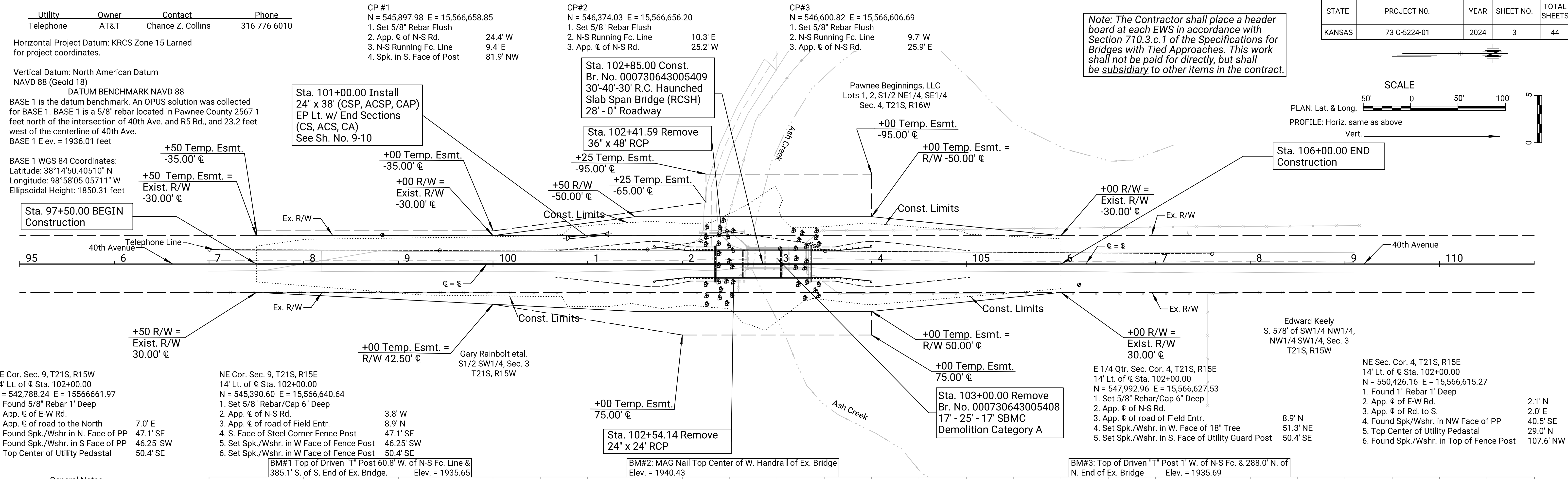
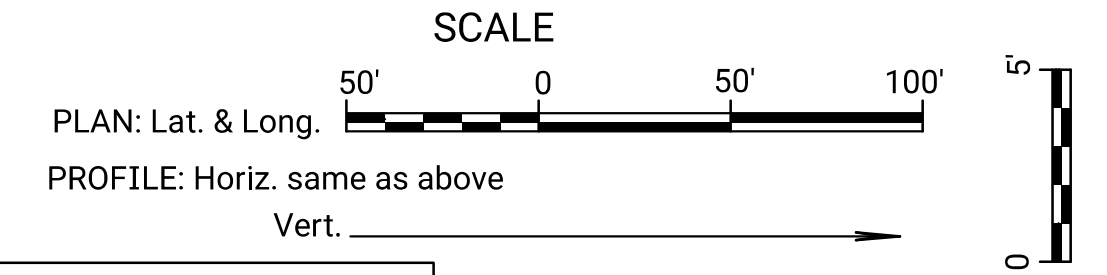
KANSAS DEPARTMENT OF TRANSPORTATION

RECOM. FOR APPROVAL-DATE
7-8-2024
Kurt Demel
LOCAL PUBLIC OFFICIAL

Plotted by: rsnw 3-JUL-2024 18:03
File: KA_2301810_rti.dgn

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Note: The Contractor shall place a header board at each EWS in accordance with Section 710.3.c.1 of the Specifications for Bridges with Tied Approaches. This work shall not be paid for directly, but shall be subsidiary to other items in the contract.



General Notes

The Contractor shall remove the existing 17'-25'-17' Steel Beam Continuous (SBMC) Bridge #000730643005408. All items of the existing structure shall become the property of the Contractor and shall be removed from the site.

The Contractor shall complete the Embankment as shown on the bridge excavation sheet prior to the driving of the abutment piling.

The Contractor shall excavate the channel at the bridge site to the limits shown prior to the construction of the bridge.

All trees, hedge rows, shelterbelts, and wood shrubs not shown to be removed and located between the construction limits and the right-of-way line or easement lines shall be spared unless directed by the Engineer to be removed.

Excavation shown to be wasted shall be wasted on sites provided by the Contractor. These sites shall be approved by the Engineer as to suitability, appearance and site location. Locations that in the opinion of the Engineer will leave an unsightly appearance will not be approved.

All disposal sites must be approved by the Kansas Department of Health and Environment. Material either stockpiled or disposed of in a flood plain would require a Kansas State Dept. of Agriculture permit. Any material dumped in waters of the United States or wetlands is subject to U.S. Corps of Engineers permitting regulations.

The County shall be responsible for all fencing.

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED

Plotted by: rsnw 3-JUL-2024 18:04
File: 2301810_rpp-01.dgn

PLAN AND PROFILE
STA. 95+00.00 TO STA. 111+00.00

Note to Designer - Design guardrail installations using guidance shown on KDOT's 'Guardrail Typical Alignments' Standard Drawings. 'Flared' guardrail installations are preferred over 'Parallel' or 'Zero Flare' installations. Where 'Flared' or 'Parallel' installations are used, the flare rate of the guardrail end terminal typically matches the flare rate of the remaining guardrail installation. For 'Zero Flare' installations, 'Parallel' guardrail end terminals should be designed using typical flare rates of 50:1 or flatter for the length of the end terminal. However, while 50:1 or flatter flare rates are typical for 'Parallel' guardrail end terminals, these end terminals may be flared as steep as 26:1 or flatter in order to offset the end terminal head as far from the edge of the through traveled lane as practicable.

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GENERAL NOTES

Install the guardrail end terminals according to the Manufacturer's Installation Manual. The Contractor will furnish a copy of the Manufacturer's Installation Manual to the Engineer prior to the start of the installation.

Use approved steel (preferred) or wood posts provided by the Manufacturer. The guardrail end terminal post type may be independent of the post type used in the remainder of the installation. However, no mixing of post types is permitted in the remaining w-beam and thrie-beam installation.

Use approved polymer (preferred) or wood blockouts provided by the Manufacturer. The guardrail end terminal blockout size and type may be independent of the blockout size and type used in the remainder of the installation. For blockout size and types for the remaining w-beam and thrie-beam portion of the installation see the details shown on KDOT's 'Guardrail Post Details' and 'Guardrail Thrie-Beam Transition Details' Standard Drawings.

Apply retroreflective sheeting to the end terminal impact head before installation.

Tighten all cable anchor assemblies as per the Manufacturer's Installation Manual.

Lap w-beam and thrie-beam guardrail splices, in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final configuration. Lap end terminal splices per the Manufacturer's Installation Manual in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final configuration.

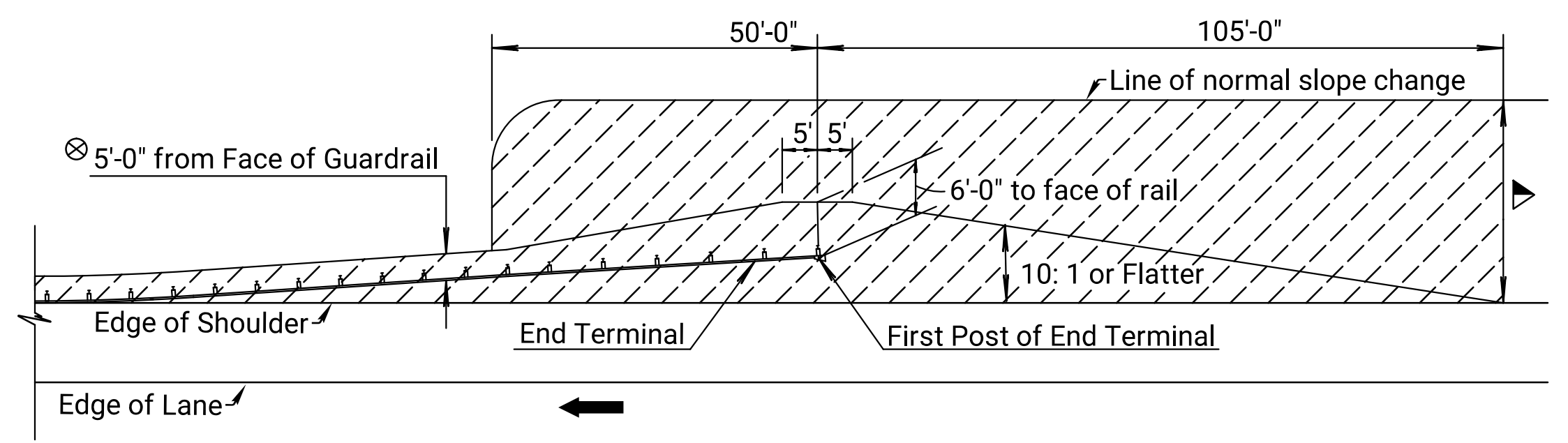
The minimum length of w-beam guardrail required between the thrie-beam transition and the guardrail end terminal is 12'-6" for all installations; unless otherwise stated in the Manufacturer's Installation Manual.

Where pavement with a thickness less than or equal to 8" is encountered during installation, use the details shown on KDOT's 'Guardrail Post Details' Standard Drawings to provide openings in the pavement for the guardrail posts. Where pavement with a thickness greater than 8" or geologic rock is encountered during installation, follow the Manufacturer's Installation Manual for guidance. Where the Manufacturer's Installation Manual does not address pavement with a thickness greater than 8" or geologic rock, contact the manufacturer for instructions or install the guardrail posts as directed by the Engineer.

All work and materials required for w-beam and thrie-beam guardrail installations are paid for under the appropriate bid items for either CGS or MGS guardrail depending on the type of installation.

All work and materials required for guardrail end terminal installations are paid for under the bid item for the selected guardrail end terminal. See the table on this sheet for the appropriate end terminal bid item information.

GUARDRAIL CLEAR AREA
 Applies to all guardrail installations unless otherwise shown in the plans.

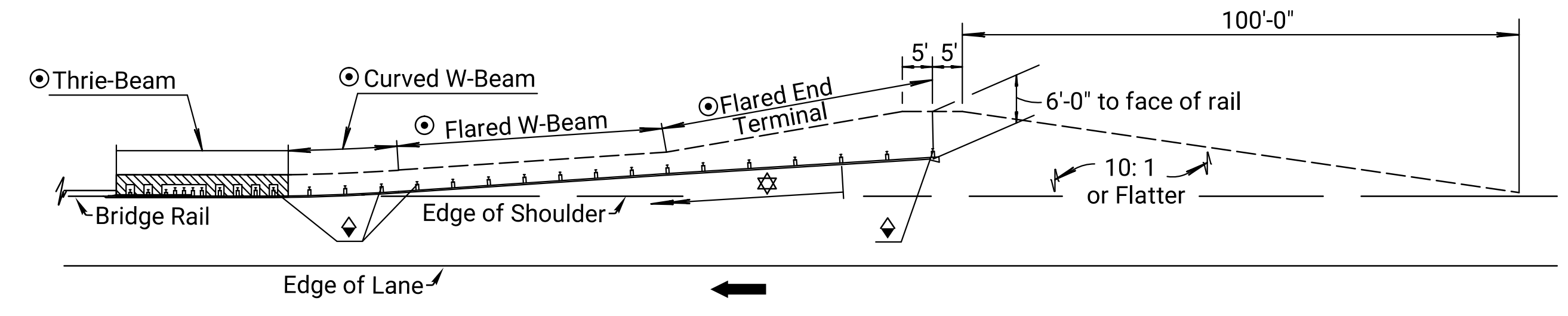


Keep Area Free of Stockpiled Material, Equipment, or Other Obstacles, Such as Temporary Signs, Regardless of Crash Worthiness. This Clear Area Extends 105 Feet in Advance of and 50 Feet behind the First Post of the Guardrail End Terminal and Then, in Order to Maintain Full Post Spacing, Continues 5 Feet behind the Face of the Guardrail through the W-Beam Portion of the Installation as Shown in the 'Guardrail Clear Area' Detail on this Sheet.

Normal Project Side Slope.

Deflection Distance for Normal Post Spacing

FLARED GUARDRAIL DETAIL
 Applies to CGS AND MGS (MGS Shown)

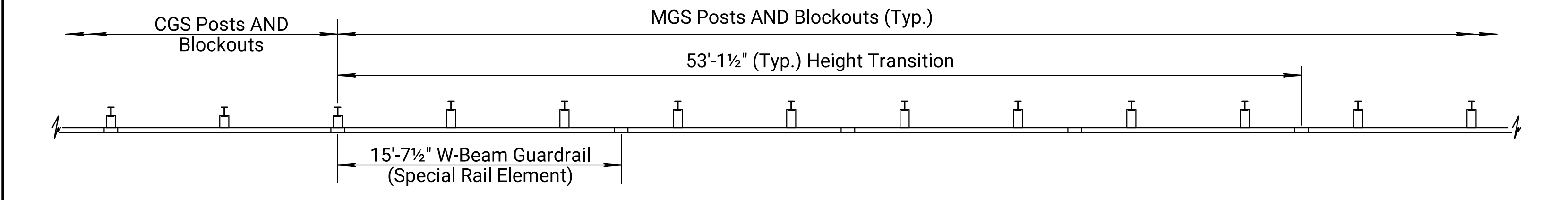
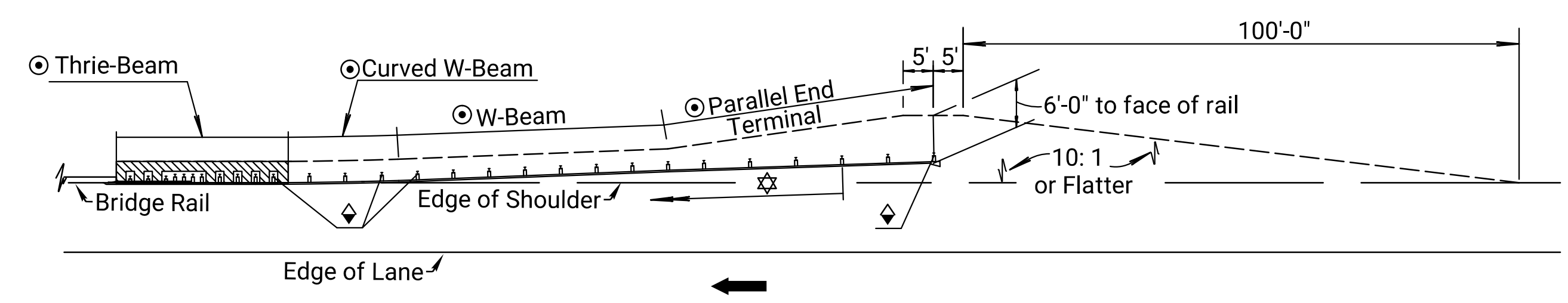


See Guardrail Layout Sheets for Details

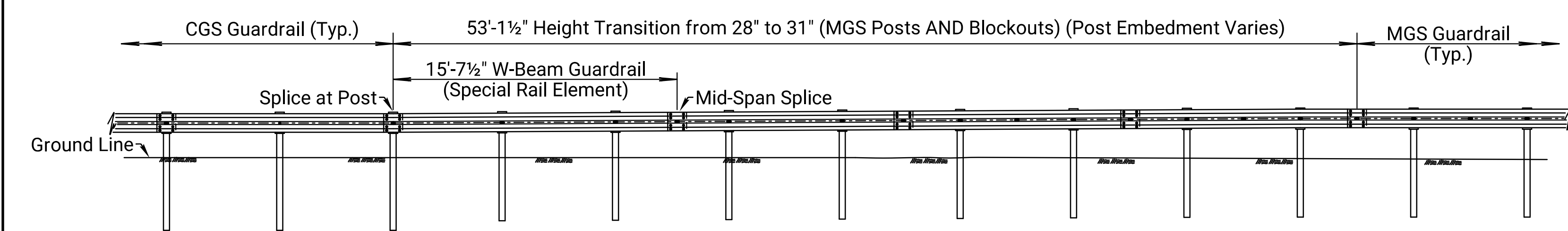
On Guardrail Layout Sheets, Show Station AND Offset from the Roadway Alignment to the Face of Post at these Locations.

Length of Need (Begins at Post 3)

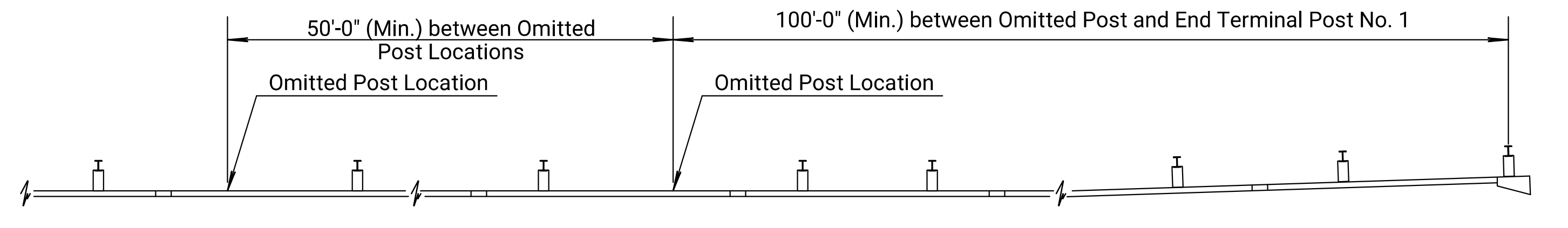
PARALLEL GUARDRAIL DETAIL
 Applies to CGS AND MGS (MGS Shown)



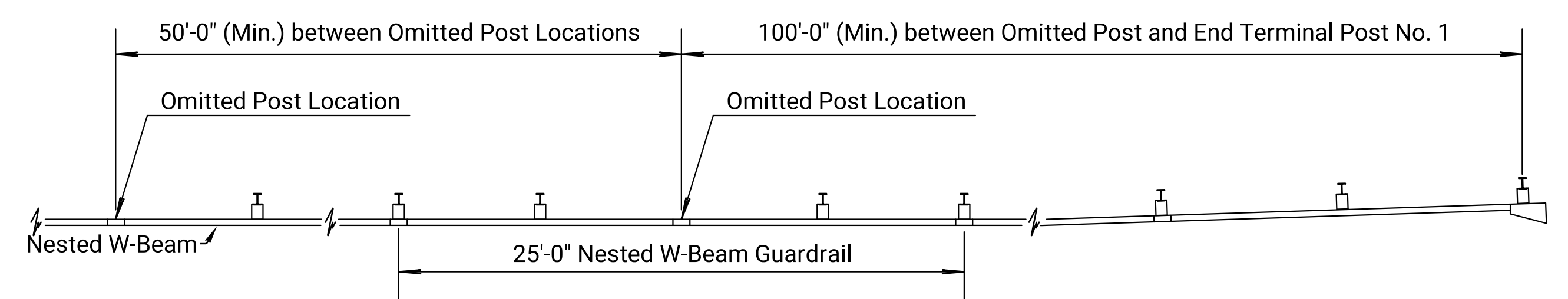
CGS TO MGS TRANSITION DETAILS (PLAN)



CGS TO MGS TRANSITION DETAILS (ELEVATION)



MGS OMITTED POST DETAIL



CGS OMITTED POST DETAIL

MIDWEST GUARDRAIL SYSTEM (MGS) END TERMINALS

END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (MGS-FLEAT)	Flared	31"	NCHRP 350	Yes	Yes	Yes	Road Systems	40'-7 1/2"	37'-6"
Guardrail End Terminal (MGS-SRT)	Flared	31"	NCHRP 350	Yes	Yes	No	Valtir	40'-7 1/2"	37'-6"
Guardrail End Terminal (MGS-MSKT)	Parallel	31"	MASH	Yes	No	Yes	Road Systems	46'-10 1/2"	46'-10 1/2"
Guardrail End Terminal (MGS-SOFTSTOP)	Parallel	31"	MASH	Yes	No	Yes	Valtir	46'-10 1/2"	50'-9 1/2"

CONVENTIONAL GUARDRAIL SYSTEM (CGS) END TERMINALS

END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (FLEAT)	Flared	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	37'-6"	37'-6"
Guardrail End Terminal (SRT)	Flared	28"	NCHRP 350	Yes	Yes	No	Valtir	37'-6"	37'-6"
Guardrail End Terminal (SKT)	Parallel	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	50'-0"	50'-0"

02	09-05-18	ADD. OMITTED POST AND TRANS. DETAILS	A.L.R.	T.T.R.
01	06-05-18	INITIAL RELEASE	A.L.R.	T.T.R.
NO.	DATE	REVISIONS	BY	APPD

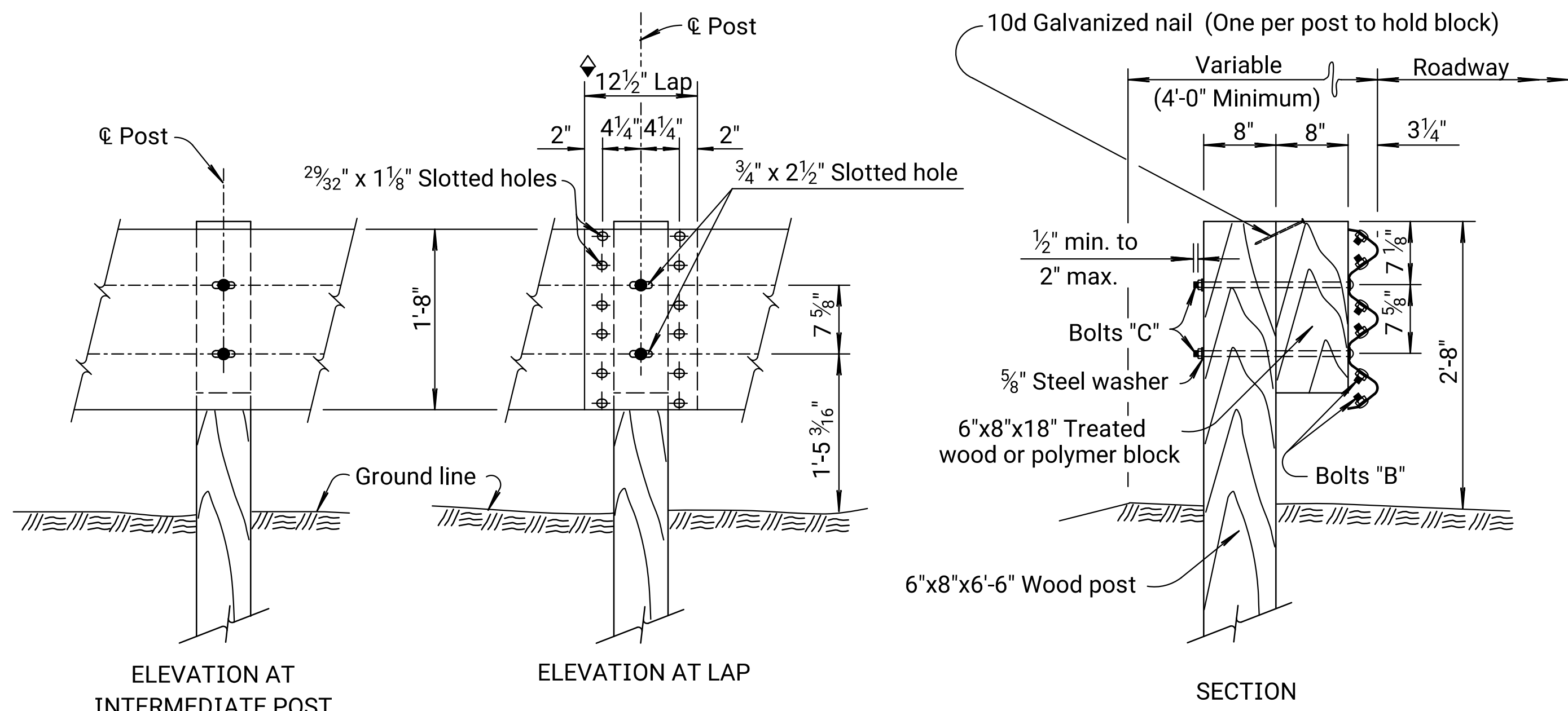
GUARDRAIL AUXILIARY DETAILS

RD606

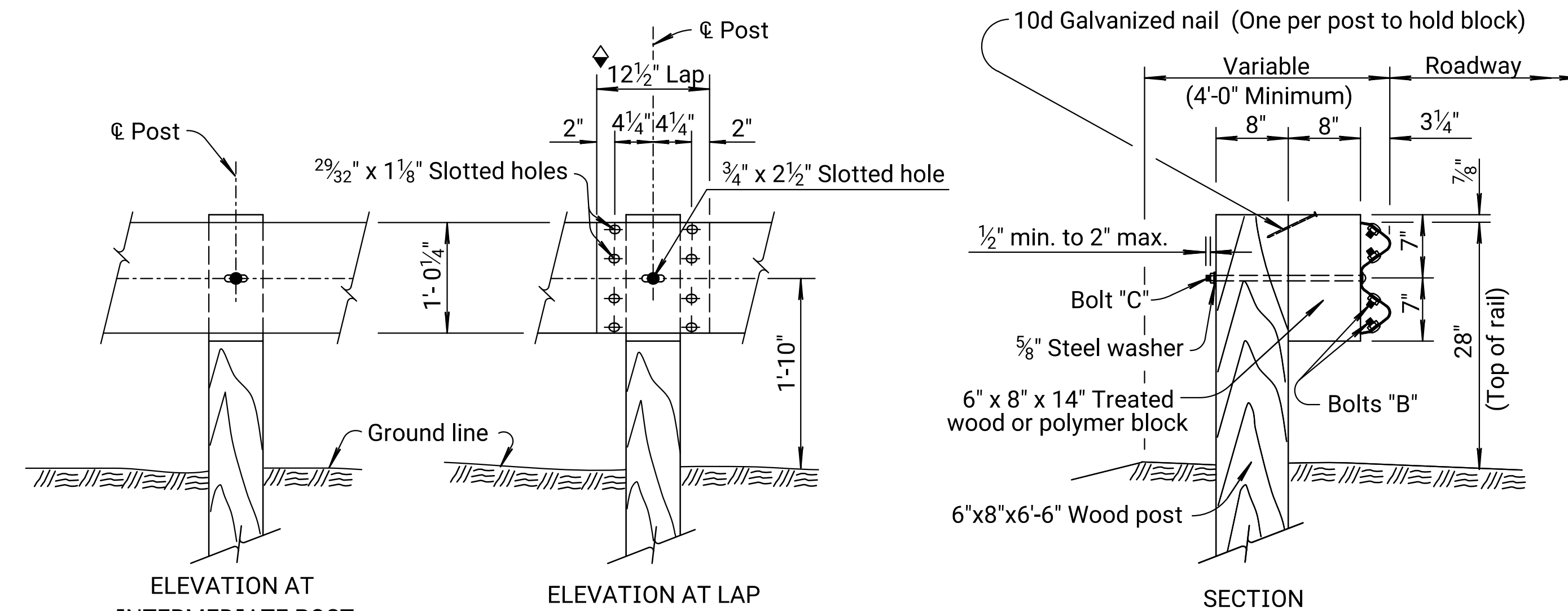
DESIGNED	09-25-18	APPD.	Scott W. King
DETAIL CK.		QUANTITIES	
DESIGN CK.		QUAN. CK.	
		TRACED	
		TRACE CK.	

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Notes to Designer: For posts installed in pavement thicker than 8" or posts installed in rock formations refer to AASHTO's Roadside Design Guide for details then revise this drawing and all supporting drawings appropriately.

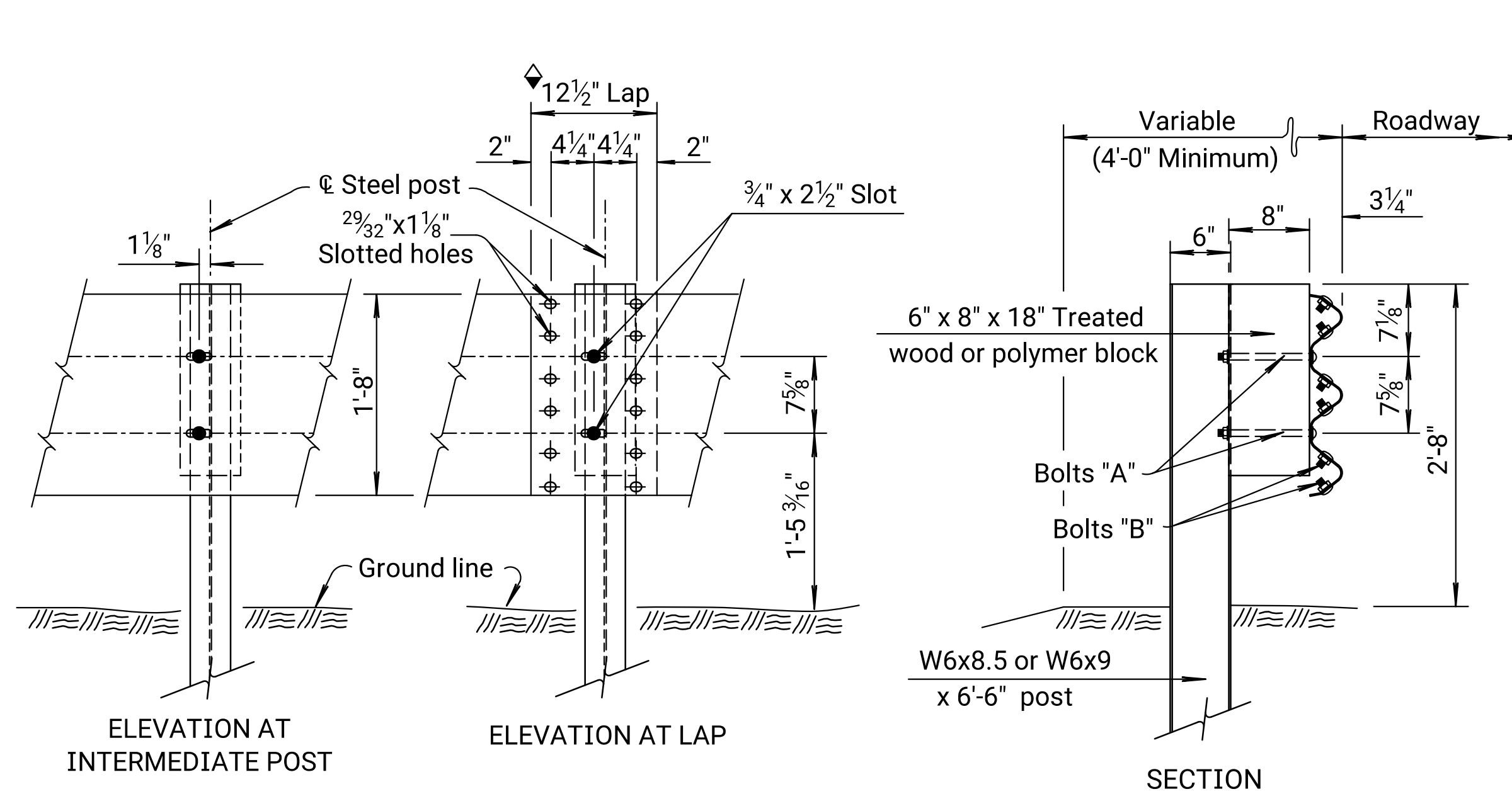


THRIE BEAM POST DETAILS



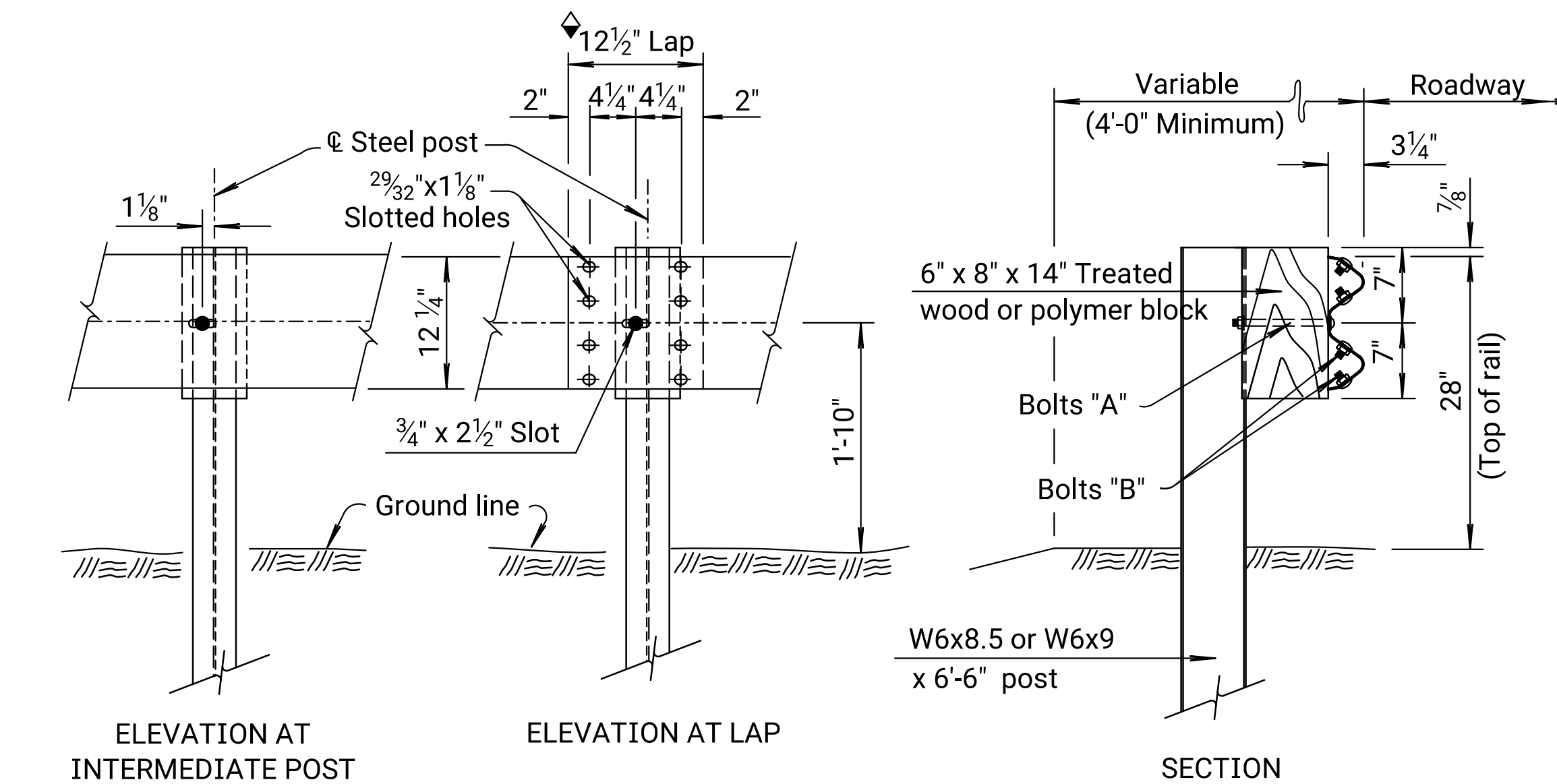
WOOD POSTS

◆ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.



THRIE BEAM POST DETAILS

◆ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.



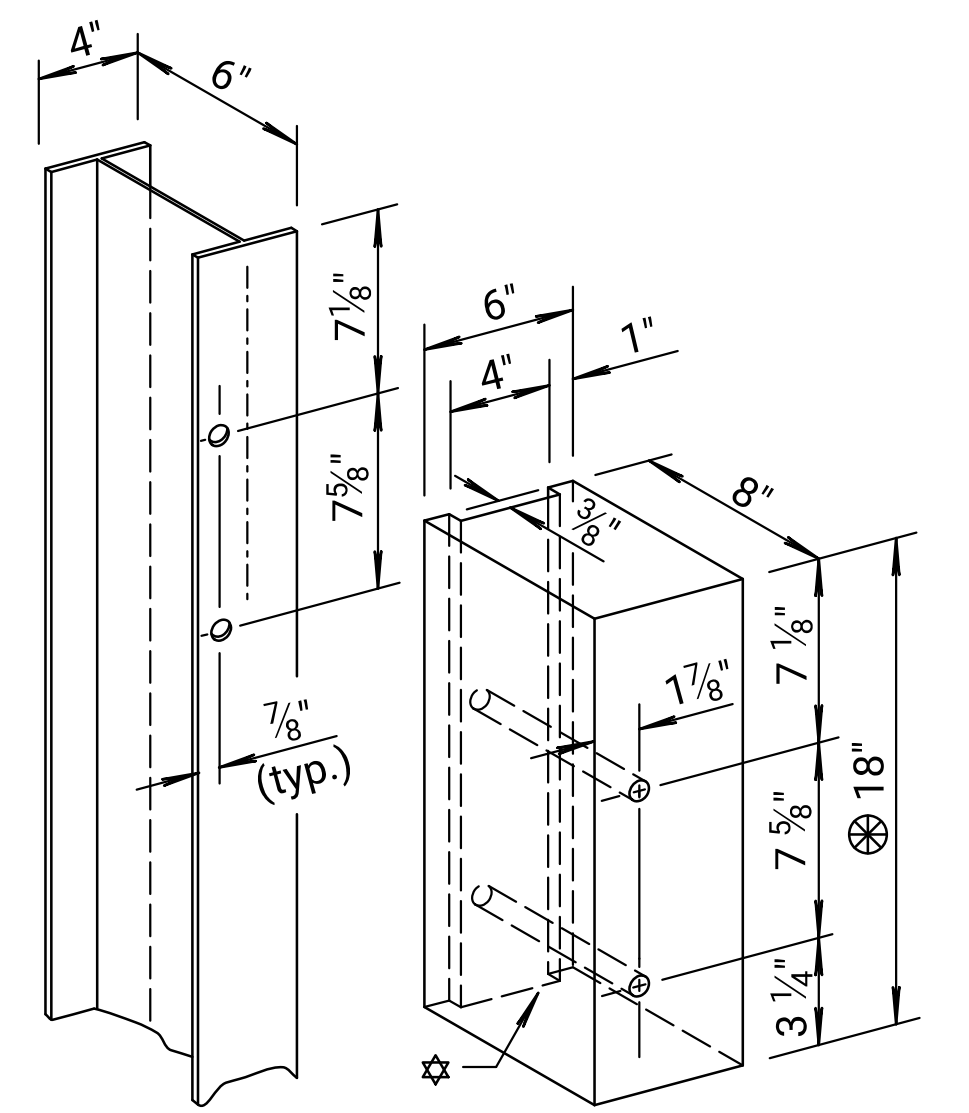
W-BEAM POST DETAILS

STEEL POSTS

GENERAL NOTES (Steel Posts)

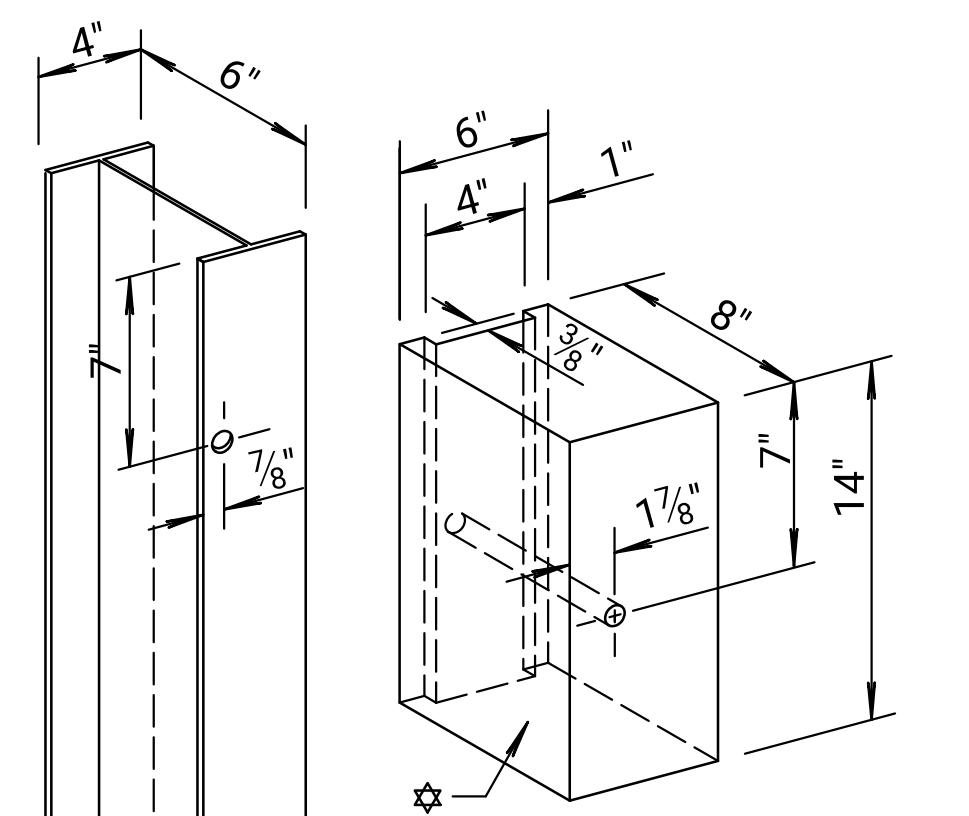
Use grade of steel for steel posts that meets the requirements of the standard specifications. Hot dip galvanize the posts after fabrication, see standard specifications. Use only one post/blockout type within guardrail run, this excludes the guardrail end terminals. For wood/polymer blockout requirements see standard specifications. Approved polymer blockouts may be substituted for wood blockouts. Only one type of blockout is permitted on each guardrail installation. This excludes the guardrail end terminals. Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations. Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post. Contractor must obtain Engineer approval prior to cutting post shorter than 6'-6" except as allowed on Standard Drawing RD617. All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made. Where guardrail posts are installed in pavement, form openings in the pavement for the guardrail posts.

⊗ See Standard Drawing RD613 for Thrie-Beam Transition Section Details.



THRIE BEAM HOLE PUNCHING DETAILS

Note: All holes 1 3/16 inch dia.

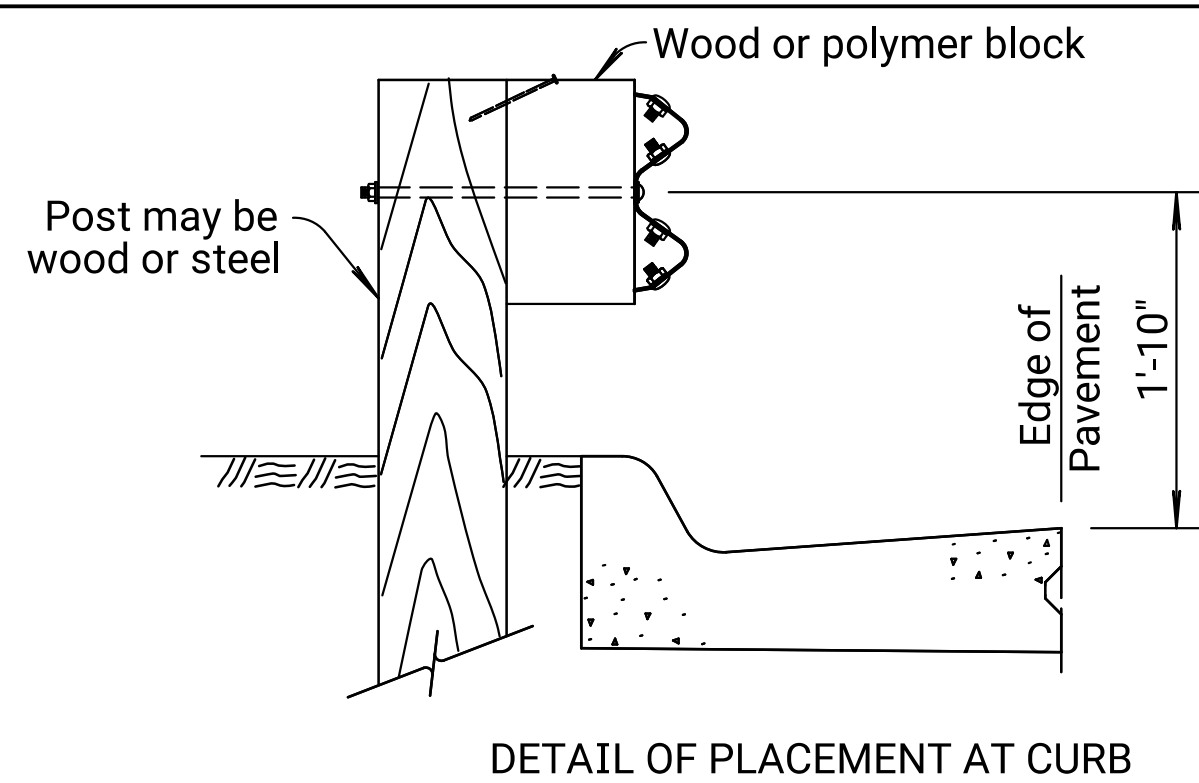


'W' BEAM HOLE PUNCHING DETAILS

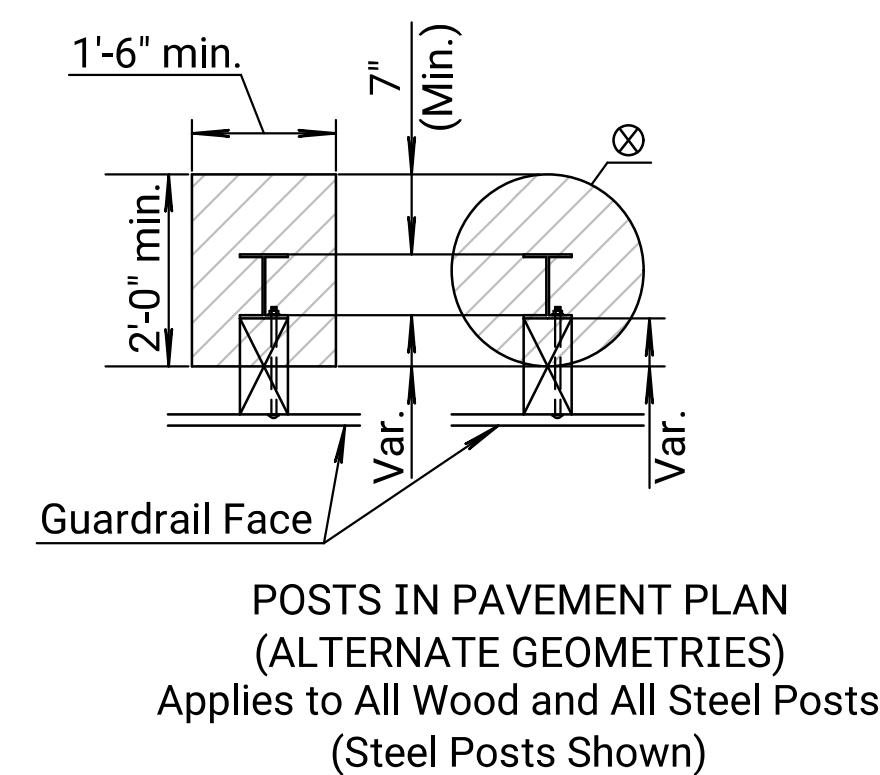
Note: All holes 1 3/16 inch dia.

⊗ Non-Metallic (Polymer) or Treated Wood Block

GENERAL NOTES (Wood Posts)
Give all wood posts and wood blocks a preservative treatment, see standard specifications. Thoroughly saturate all cuts, injuries and bolt holes on wood posts and blocks with preservative. Use only one type of preservative treatment on a project. Use S4S rectangular posts and wood blocks, see standard specifications. Use only one post/blockout type within guardrail run, this excludes the the guardrail end terminals. Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations. Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post. Contractor must obtain Engineer approval prior to cutting post shorter than 6'-6". Approved polymer blockouts may be substituted for wood blockouts. Only one type of blockout is permitted on each guardrail installation. This excludes the guardrail end terminals unless certified by the manufacturer. All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made. Where guardrail posts are installed in pavement, form openings in the pavement for the guardrail posts.



DETAIL OF PLACEMENT AT CURB

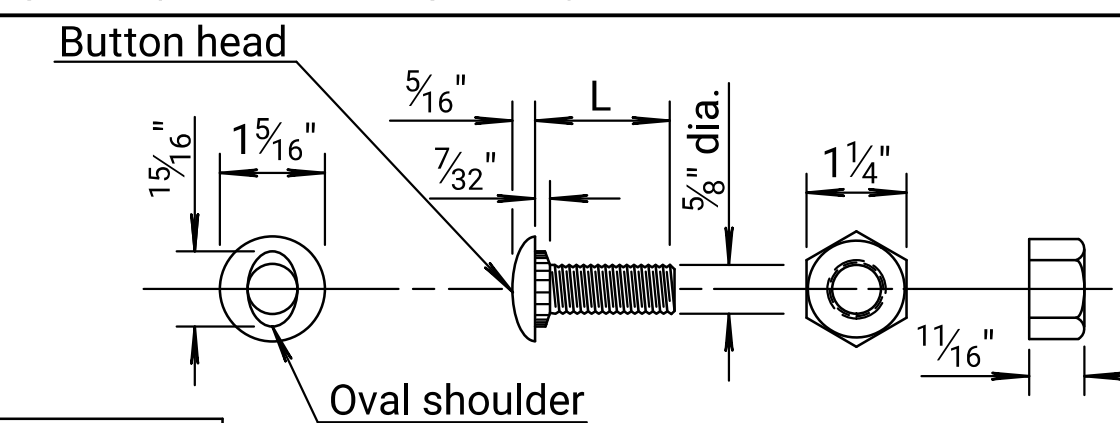


POSTS IN PAVEMENT PLAN (ALTERNATE GEOMETRIES) Applies to All Wood and All Steel Posts (Steel Posts Shown)

- ☑ Slurry Grout (Low Strength). See KDOT's Standard Specifications
- ⊗ Diameter may vary from 1'-6" (min.) to 2'-0".

Note: Low Strength Grout must have a 28-day compressive strength of 120 psi or less. All work and materials related to posts in pavement are subsidiary to other guardrail bid items. Rectangular geometry shown in Posts in Pavement detail. Circular geometry, as shown on this sheet, may be used at the Contractor's option.

BOLT SIZE SCHEDULE	
Bolt	L
A	8 1/2"
B	1 1/4"
C	18"



BOLT & NUT DETAILS

Galvanize all bolts, nuts, and washers in accordance with the KDOT's Standard Specifications.

NO.	DATE	REVISIONS	BY	APPD
13	09-05-18	Added Det., Post In Pavement	A.L.R.	T.T.R.
12	12-14-10	Revised notes, 28" w-be	S.W.K.	J.O.B.
11	06-30-04	Remove steel blockout and notes	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

RD611

DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

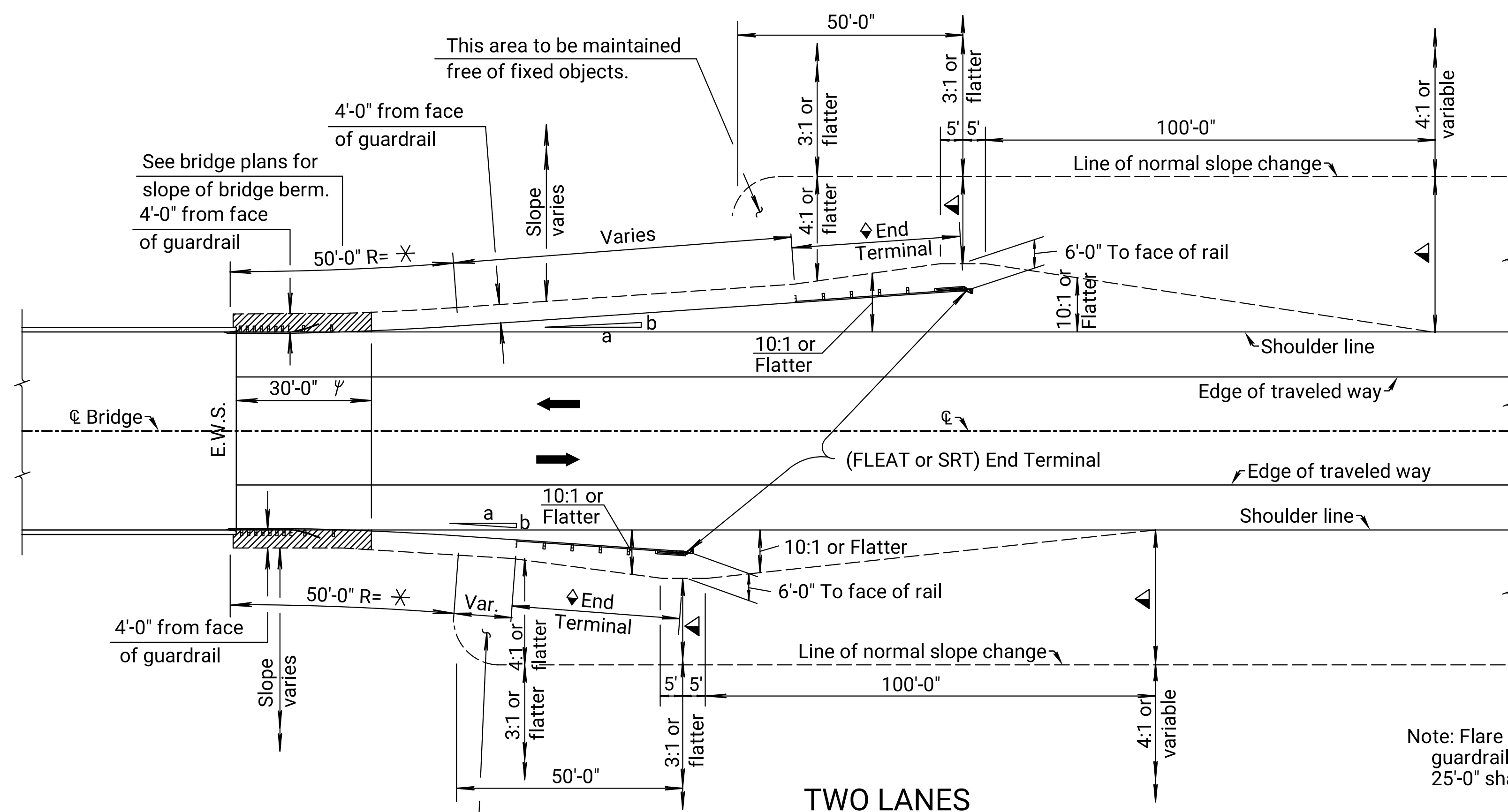
09-25-18 APPD. Scott W. King

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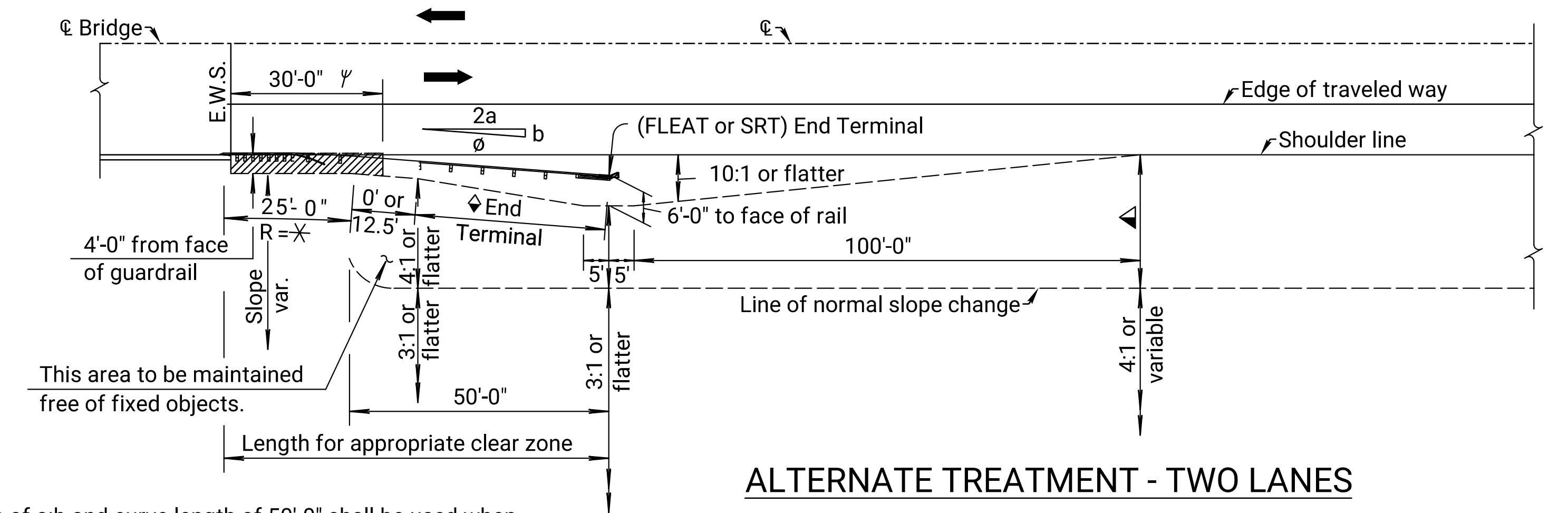
GENERAL NOTE
 For guardrail and rubrail sections, details, and general notes see KDOT's 'W-Beam with Rubrail Bridge Approach Transition Details' Standard Drawings. For post details see KDOT's 'Guardrail Post Details' Standard Drawings.
 The ratio of a:b may be specified as zero for long runs of guardrail in high fill areas.
 Widening, slopes & transition for Four Lane will be similar to that shown on two lane detail.

DESIGN PARAMETERS				
Design Speed (mph)	Flare Rate (a:b)	Radius (R)	Flare Rate (2a:b)	Radius (R)
70	15:1	375.55'	30:1	375.14'
60	14:1	350.59'	26:1	325.16'
55	12:1	300.69'	24:1	300.17'
50	11:1	275.76'	21:1	262.70'
45	10:1	250.83'	18:1	225.23'
40	8:1	201.04'	16:1	200.26'

Notes to Designer: Guardrail length of need shall be determined in accordance with the AASHTO Roadside Design Guide using $L_1 = 25'$ for flare rate of a:b and $L_1 = 12.5'$ for flare rate of 2a:b for a typical installation as shown on this sheet. This sheet shall be used when the flared guardrail design for typical layout shown (FLEAT or SRT) is selected. Material for asphalt widening shall be included in the plan quantities.

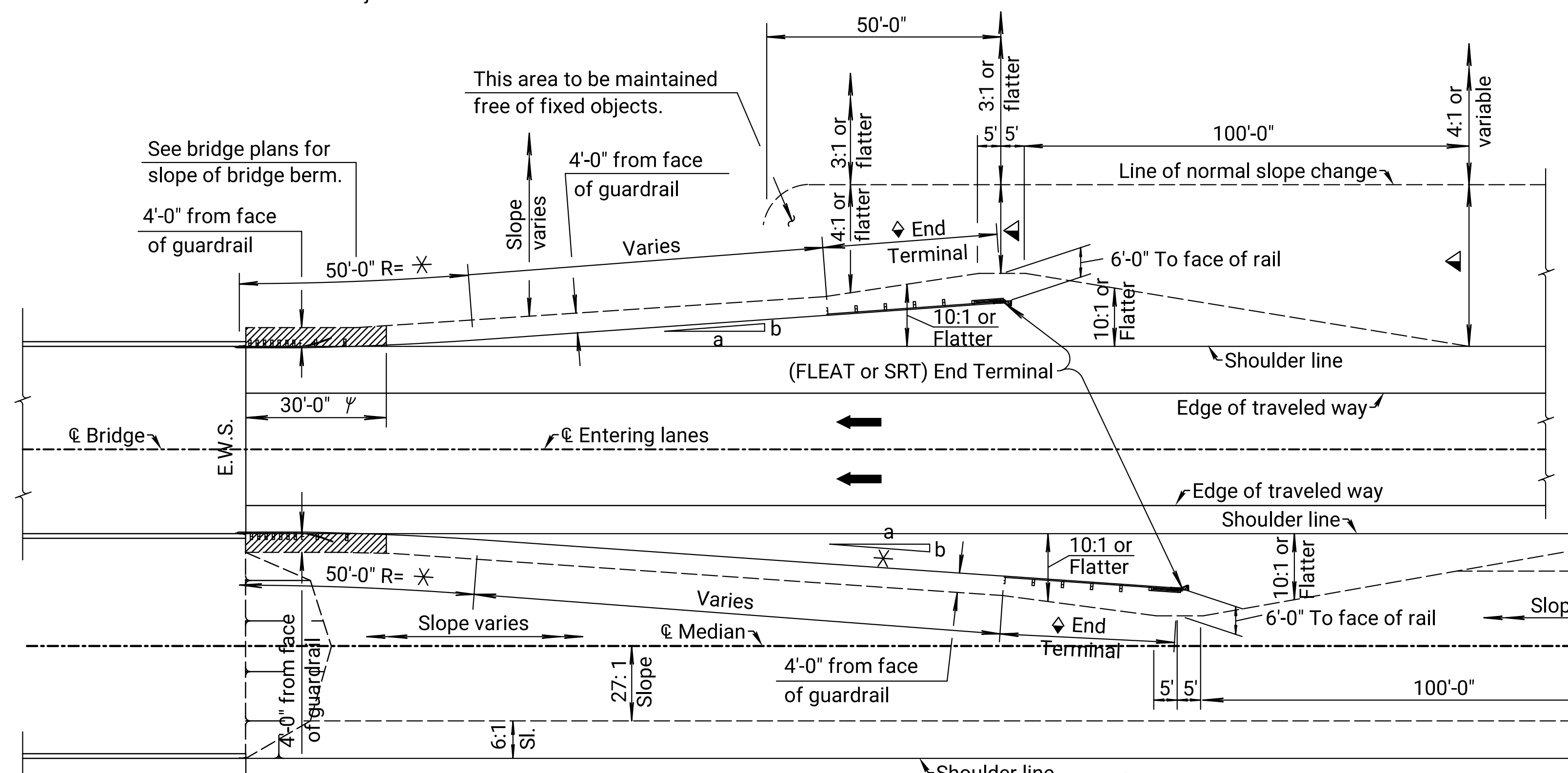


TWO LANES

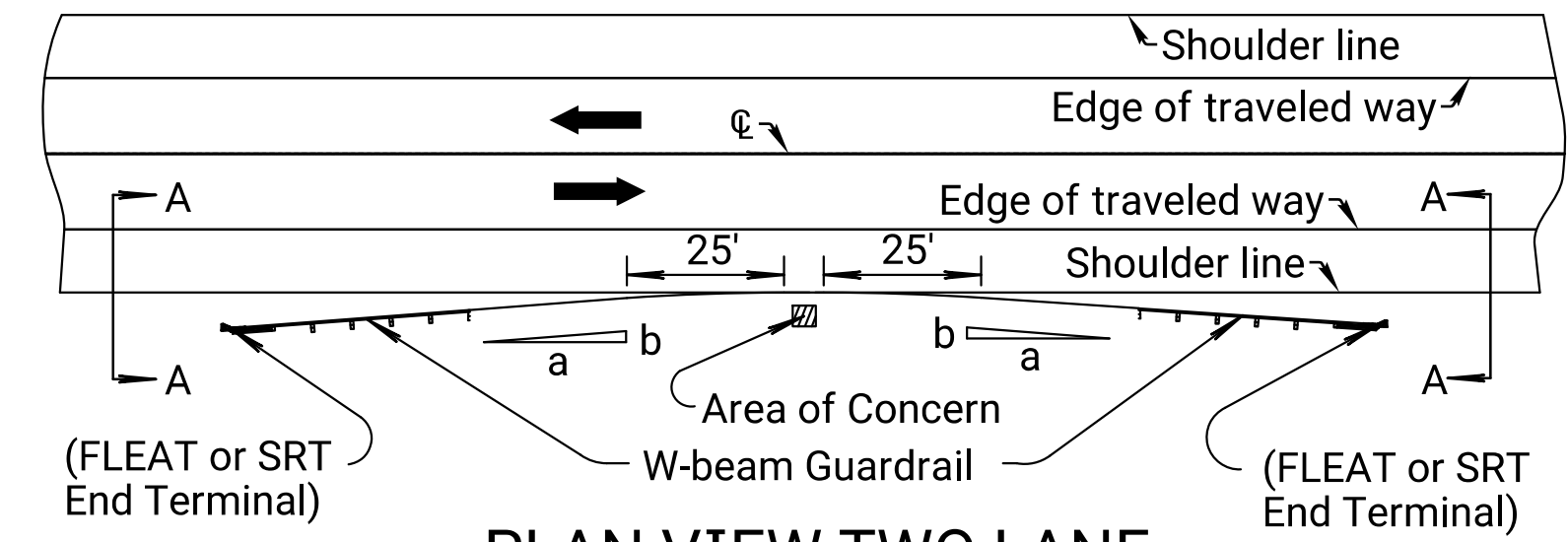


ALTERNATE TREATMENT - TWO LANES
 Flare Rate = 2a:b
 (GUARDRAIL LENGTHS OF 62.5' AND 75')

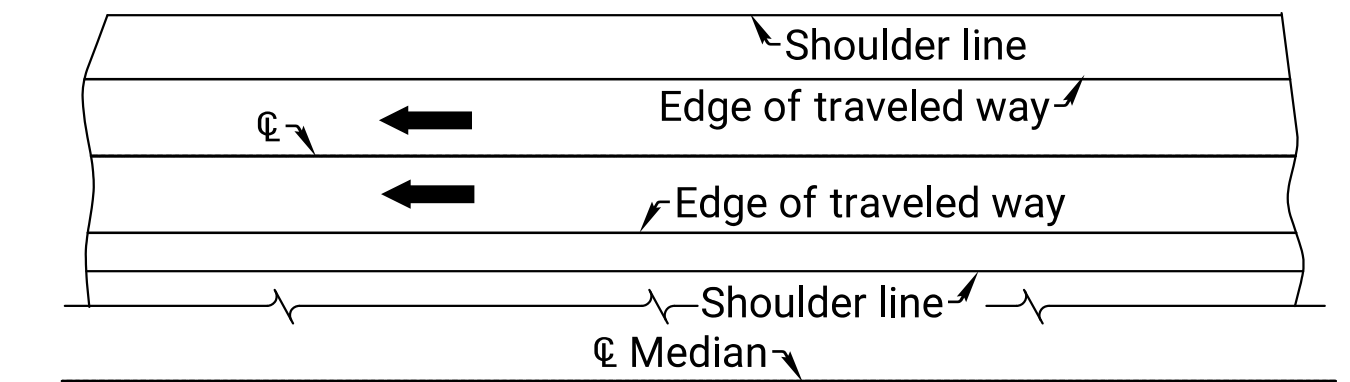
Note: Flare rate of a:b and curve length of 50'-0" shall be used when guardrail is beyond shy line, flare rate of 2a:b and curve length of 25'-0" shall be used when guardrail is located inside the shy line.



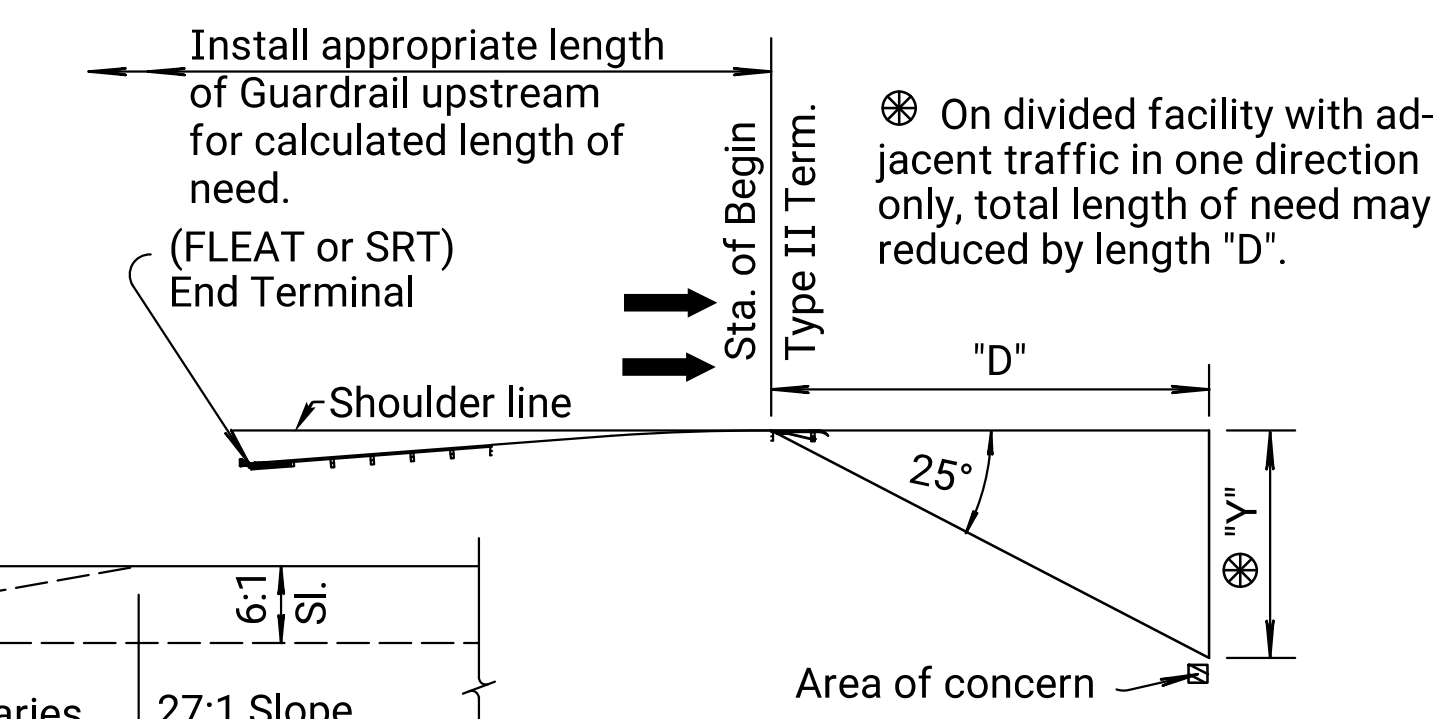
FOUR LANES - DIVIDED



PLAN VIEW TWO LANE

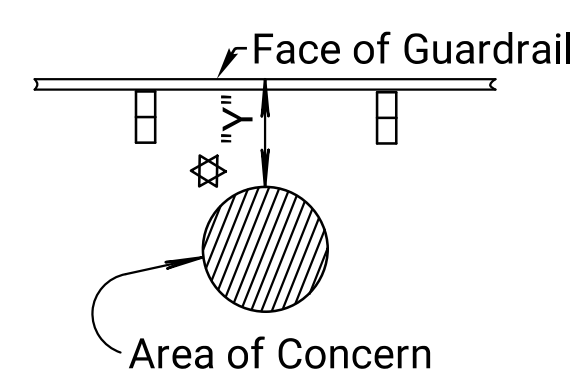


PLAN VIEW FOUR LANE



⊗ On divided facility with adjacent traffic in one direction only, total length of need may be reduced by length "D".

DETAILS OF GUARDRAIL PROTECTION AT ROADSIDE OBSTACLE



ENLARGEMENT - AREA OF CONCERN

- ✱ See table on this sheet for radius and flare rate.
- ▲ Normal project side slope. See typical sections.
- ◆ See KDOT's 'Guardrail Auxiliary Details' Standard Drawing.
- ♣ 4" Asphalt material placed on 4'-0" embankment widening unless flume inlet and slope drain is constructed.

NO.	DATE	REVISIONS	BY	APPD
08	06-05-18	Removed Flare-beyond-the-Flare	A.L.R.	T.T.R.
07	05-15-17	Removed X-LITE	A.L.R.	S.W.K.
06	07-02-09	Added roadside obstacle details	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

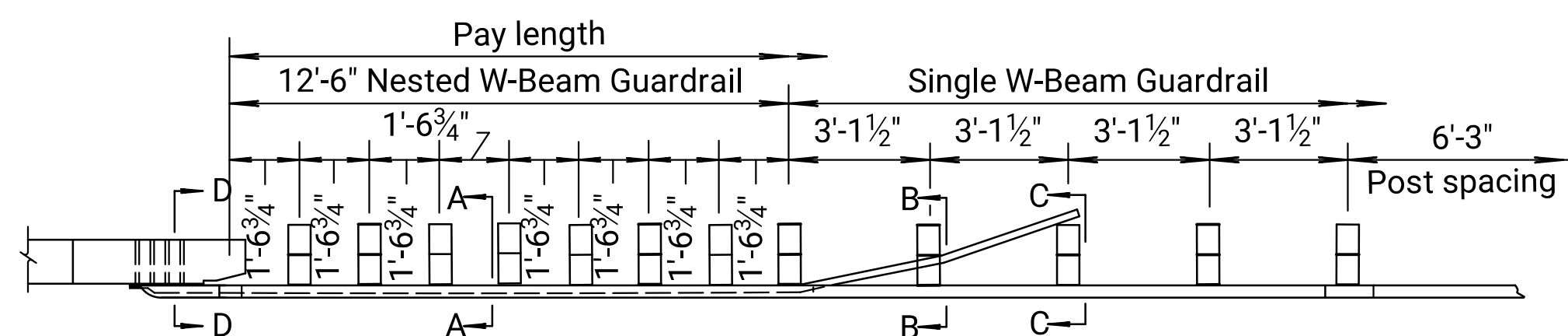
**W-BEAM WITH RUBRAIL
 BRIDGE APPROACH TRANSITION
 TYPICAL ALIGNMENTS (FLARED)**

RD615A

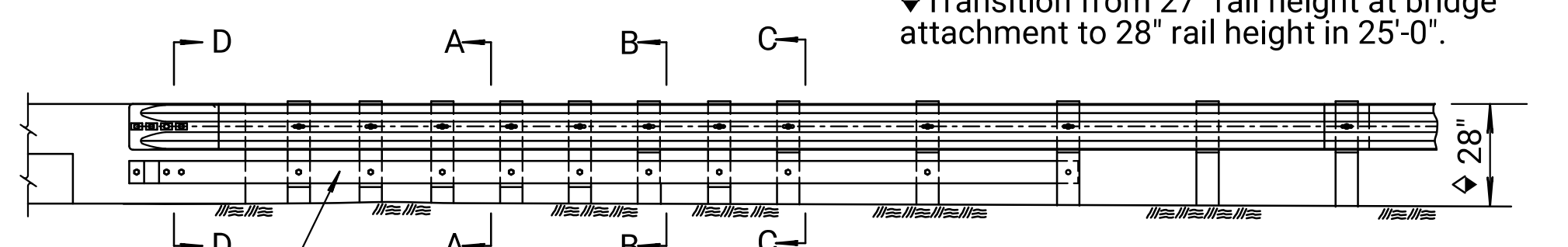
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK.

Scott W. King

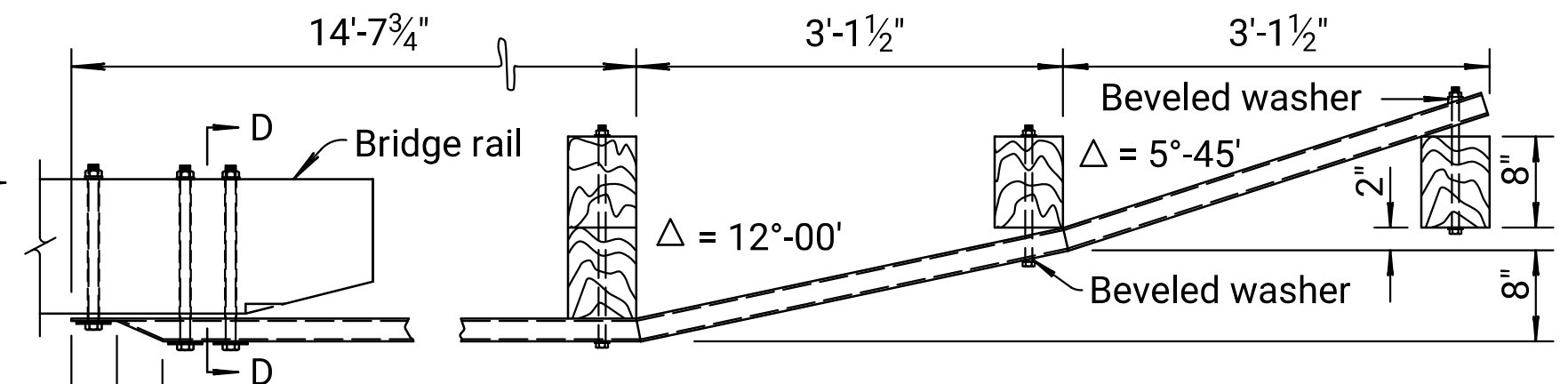
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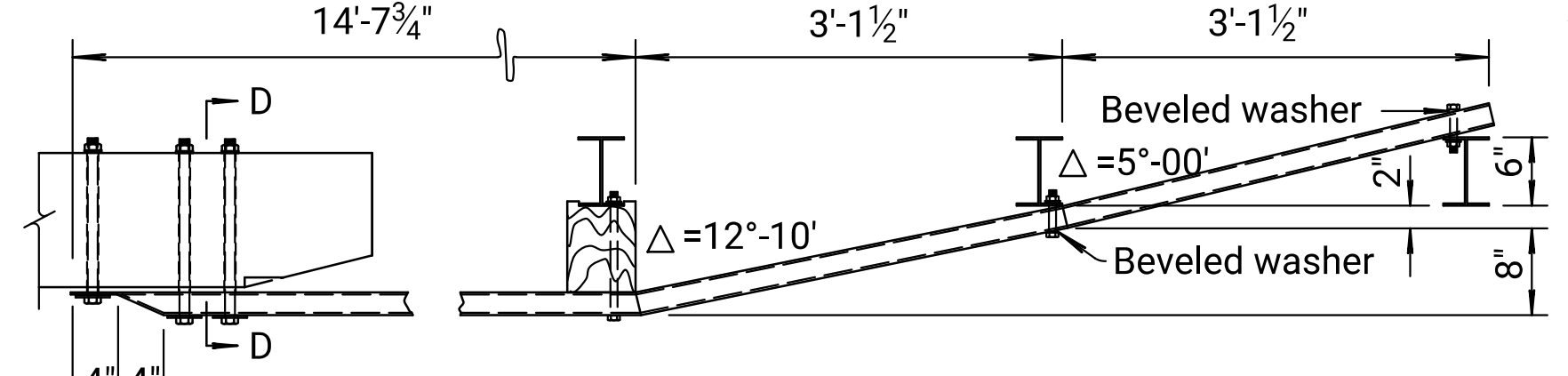
PLAN



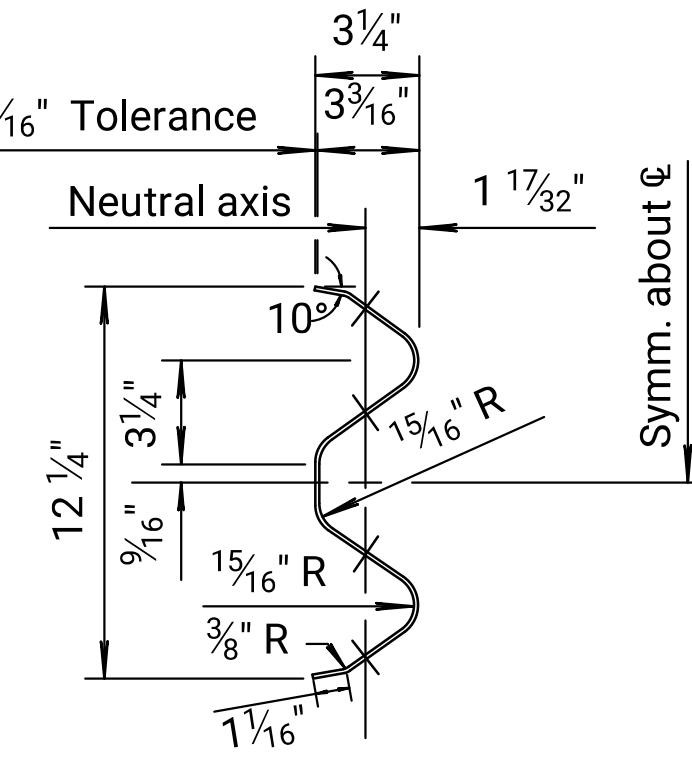
ELEVATION



PLAN OF RUBRAIL ON WOOD POSTS

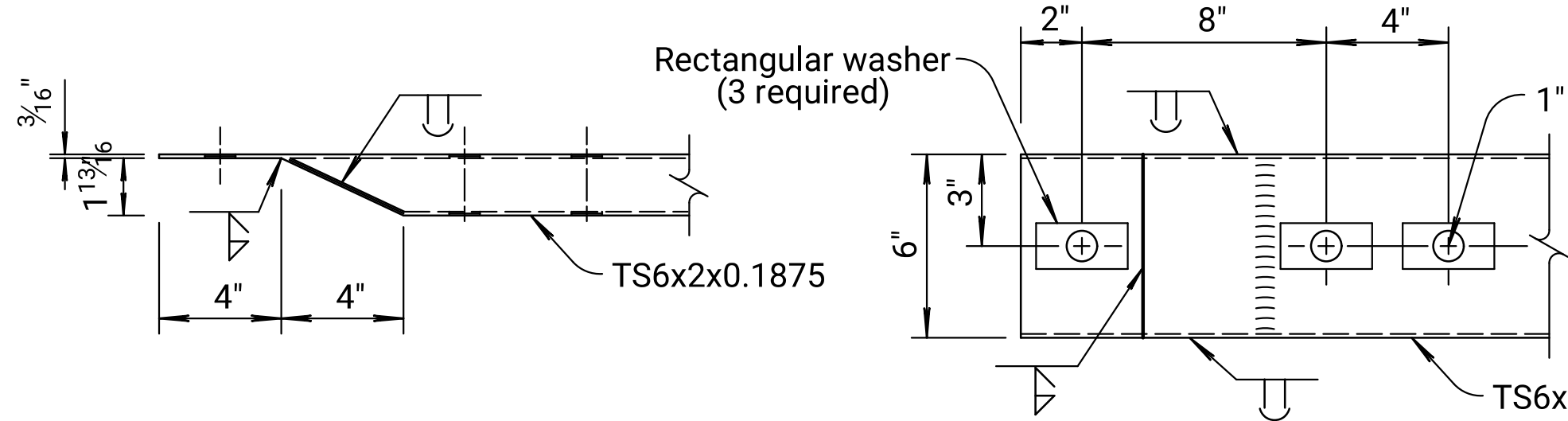


PLAN OF RUBRAIL ON STEEL POSTS

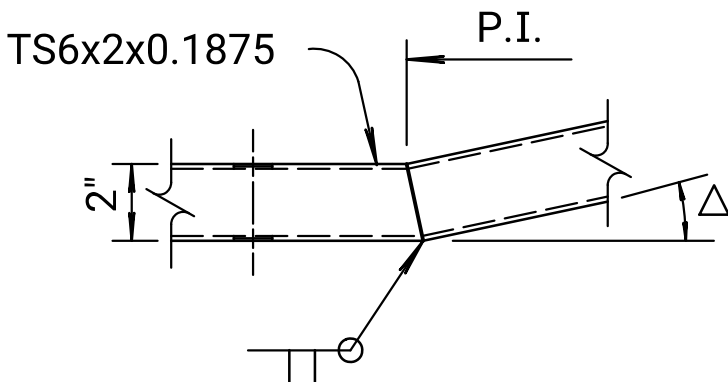


SECTION THRU TYPICAL W-BEAM RAIL ELEMENT

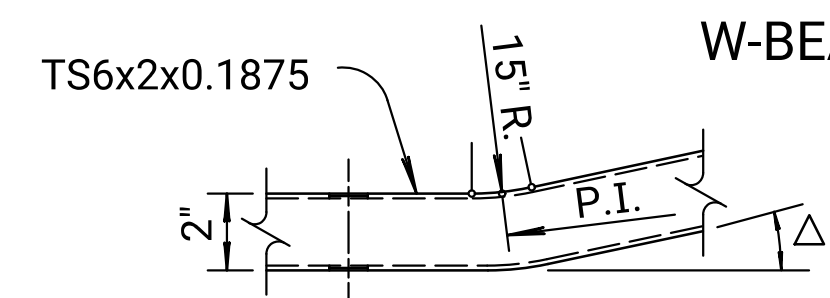
GENERAL NOTE
 Include all material and work for this installation in the pay item "Steel Plate Guardrail" paid by the lineal foot.
 Use 10 or 12 gauge steel guardrail elements unless otherwise called out, see standard specifications.
 Bridge Rail Transition consists of one 12'-6" W-beam section nested in back of one 25'-0" section. Furnished remaining rail elements in either 12'-6" or 25'-0" sections.
 Guardrail parts furnished under this specification shall be interchangeable with similar parts regardless of the source or manufacturer.
 Shop fabricate tubular steel rubrail from ASTM A36 structural steel, form angles in rubrail by shop bending or welding. Rubrail is subsidiary to the bid item "Steel Plate Guardrail".
 Galvanize rail elements, post fittings, bolts, nuts, washers and anchor bolts after fabrication in accordance with the standard specifications.
 Shop or field drill holes in posts and/or tubular steel rubrail for attachment. When holes are field drilled touch up any damage to the galvanized coating with zinc based paint.
 Shop bend rail when radius is less than 150'.
 Fabricate Special End Shoe from 10 gauge steel in accordance with standard specifications.
 The Special End Shoe has the same section as guardrail and is subsidiary to guardrail.
 Lap guardrail splices, including Special End Shoe, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.
 See Std. Drawing RD611 for additional details of posts not shown on this sheet.



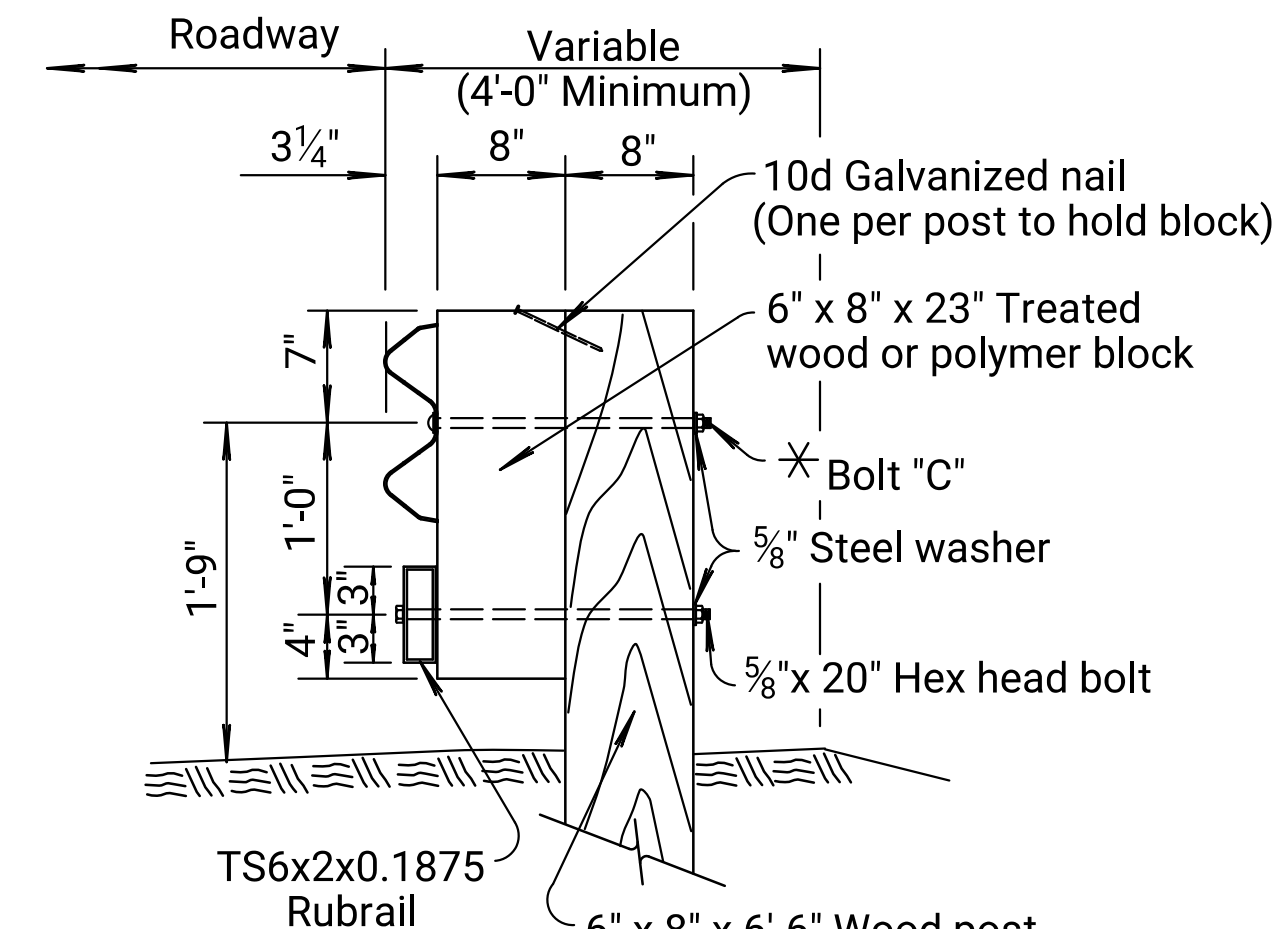
TYPICAL END RUB RAIL DETAILS



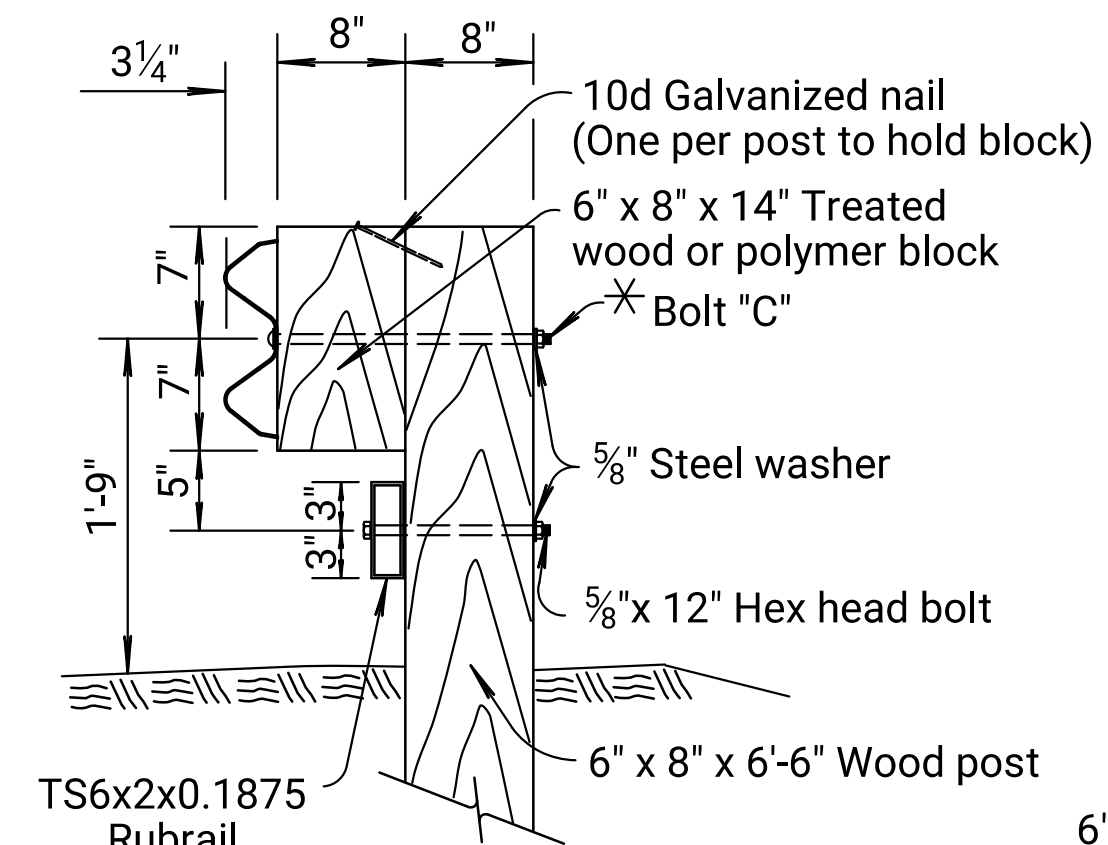
SHOP WELDED OPTION



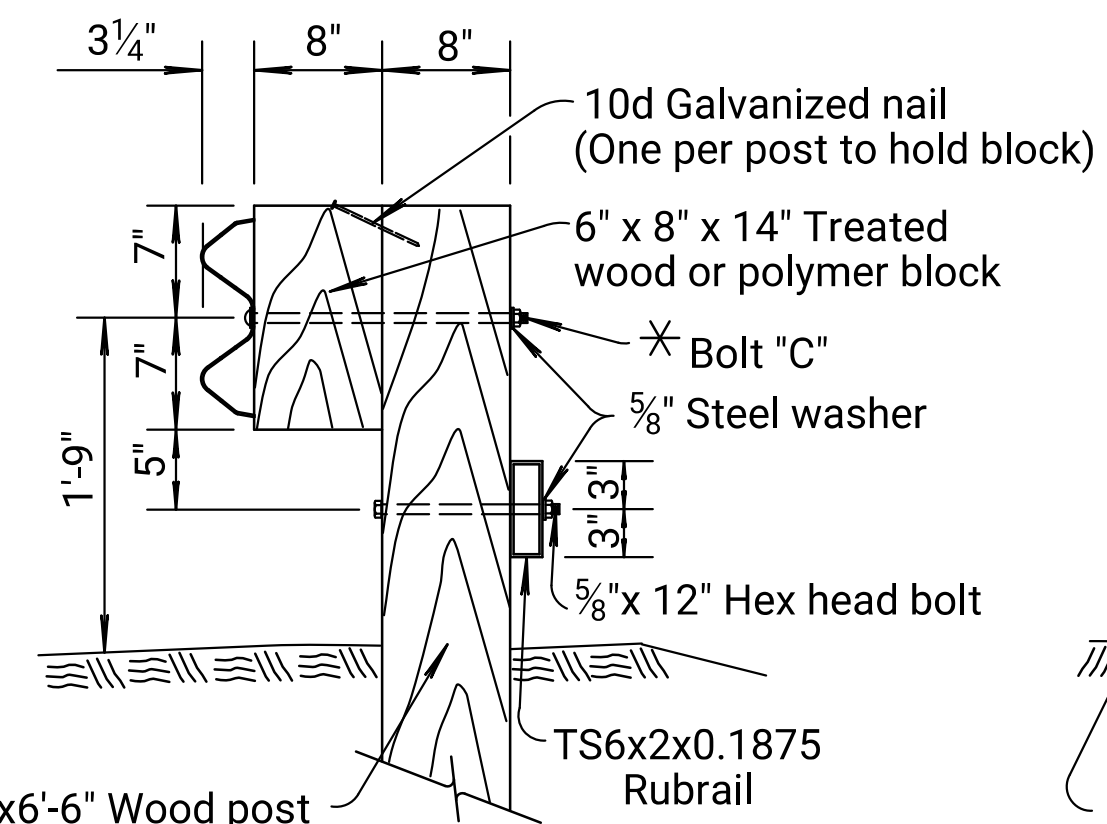
SHOP BENT OPTION



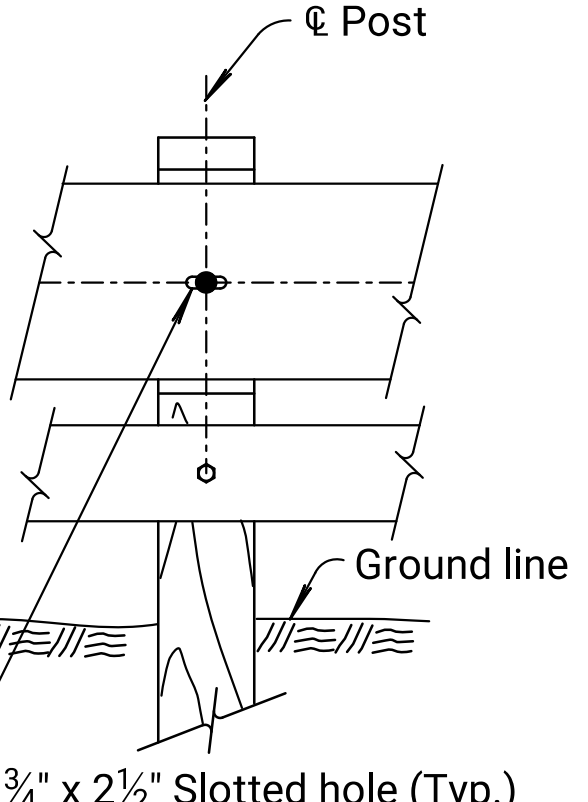
SECTION A-A



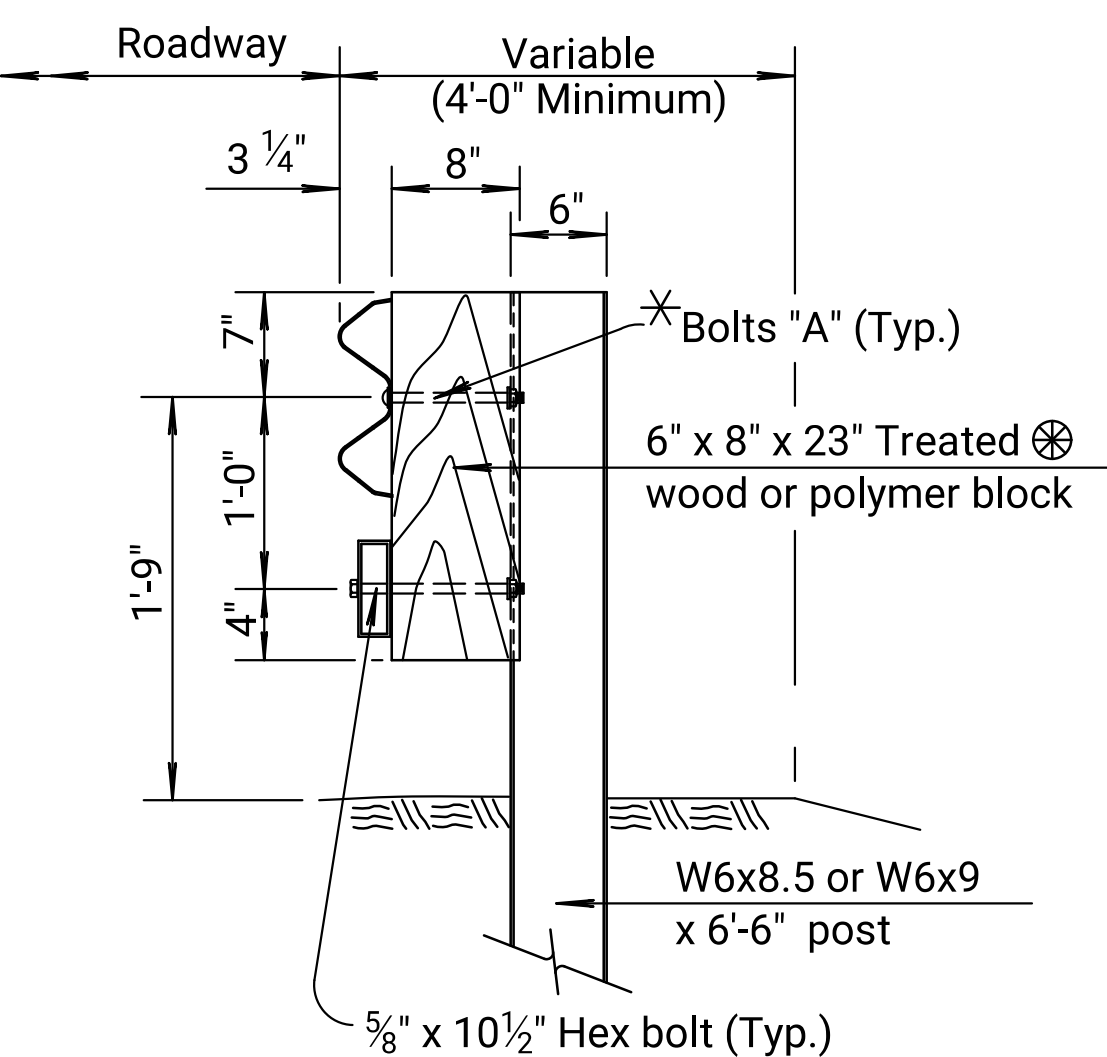
SECTION B-B



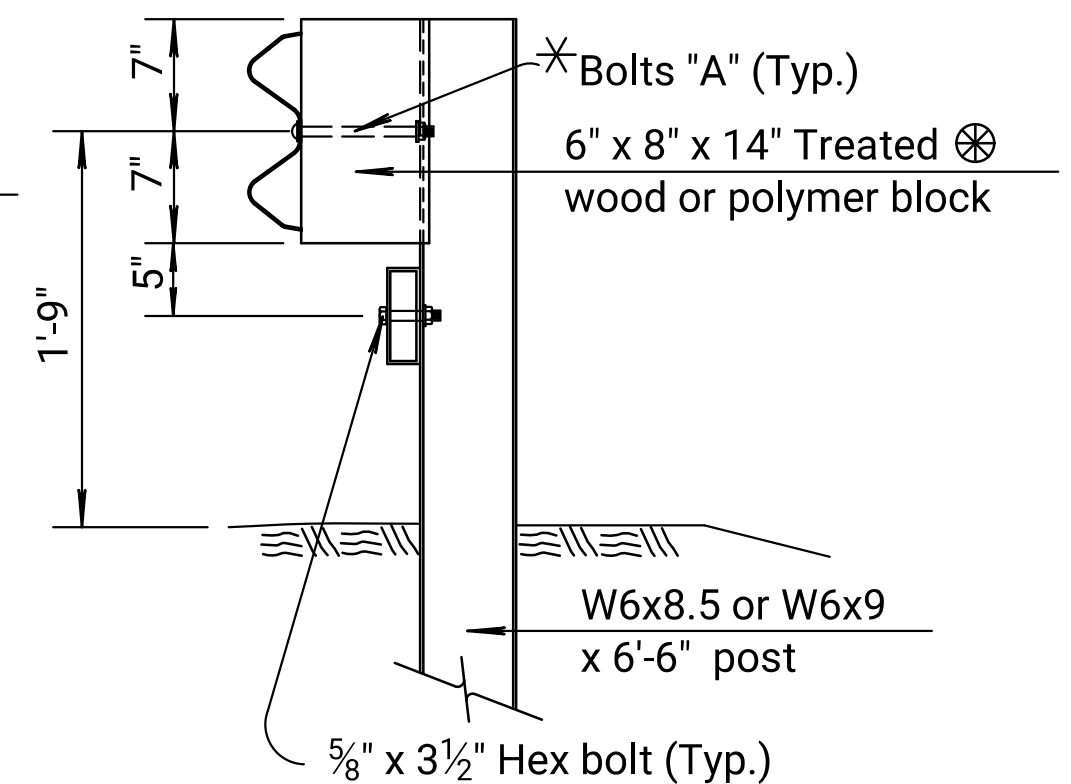
SECTION C-C



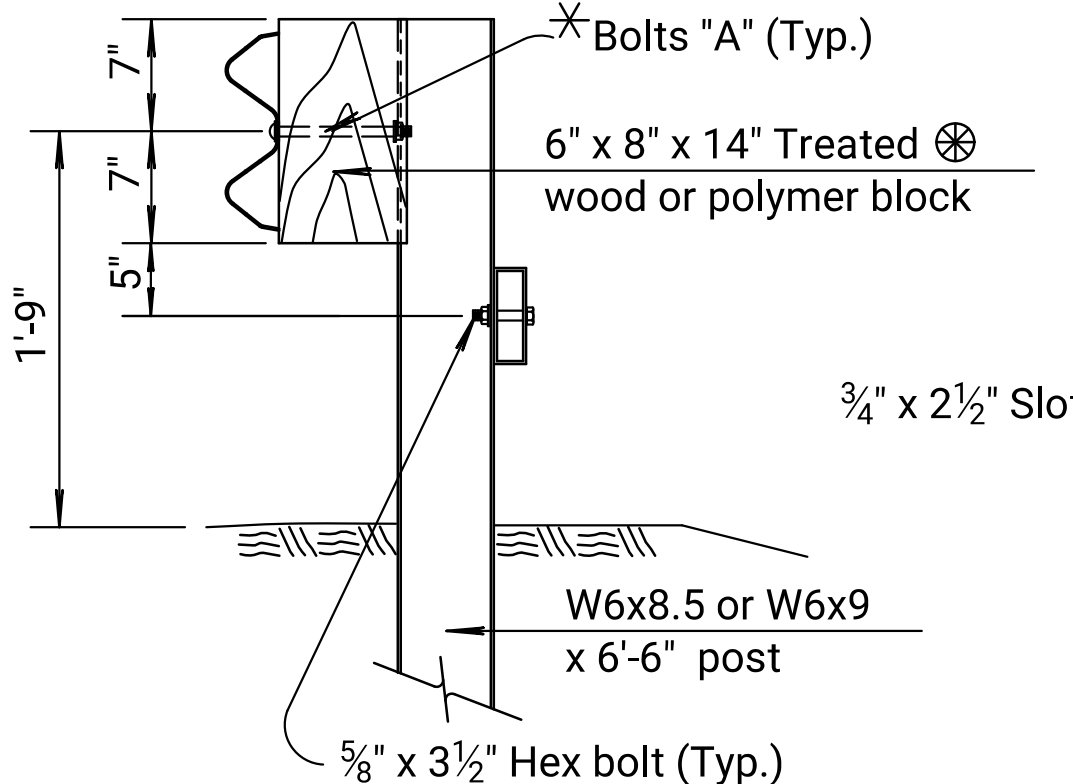
ELEVATION WITH RUBRAIL



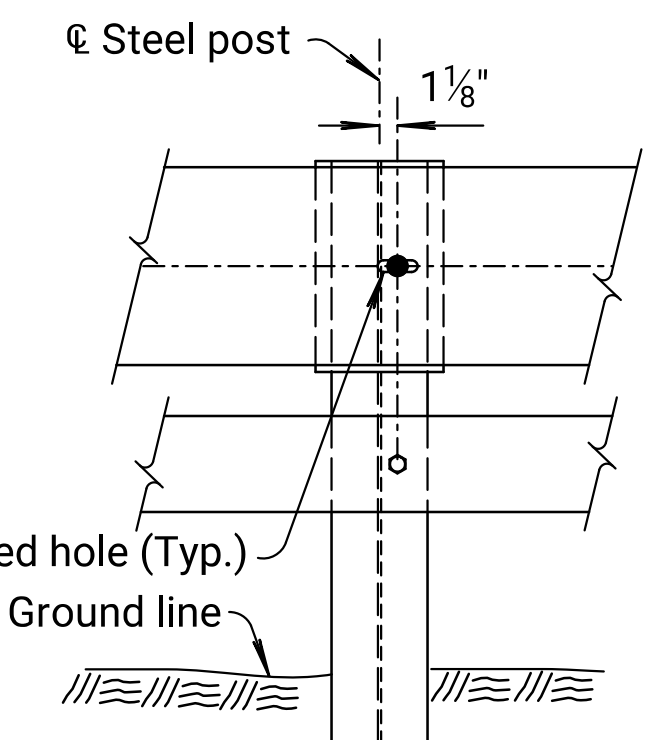
SECTION A-A



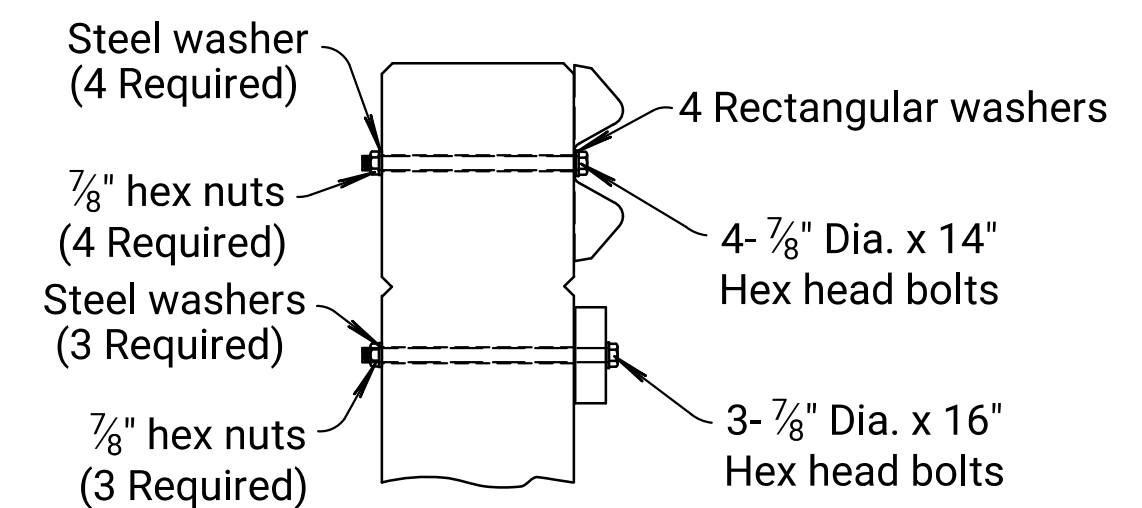
SECTION B-B



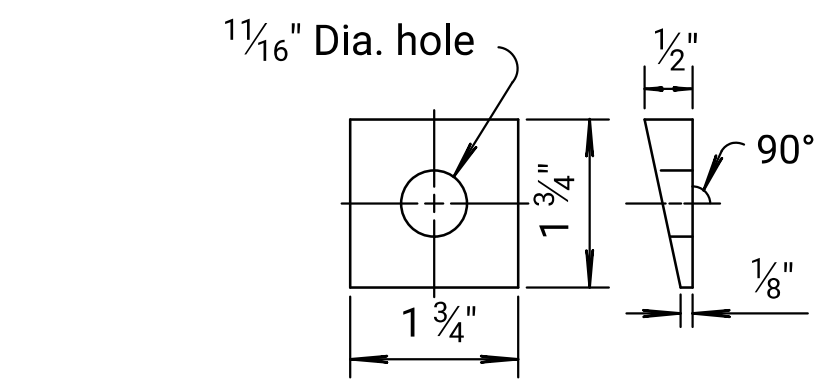
SECTION C-C



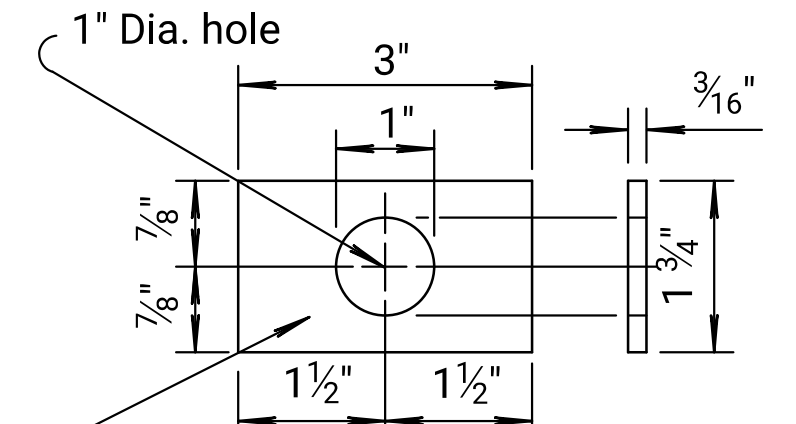
ELEVATION WITH RUBRAIL



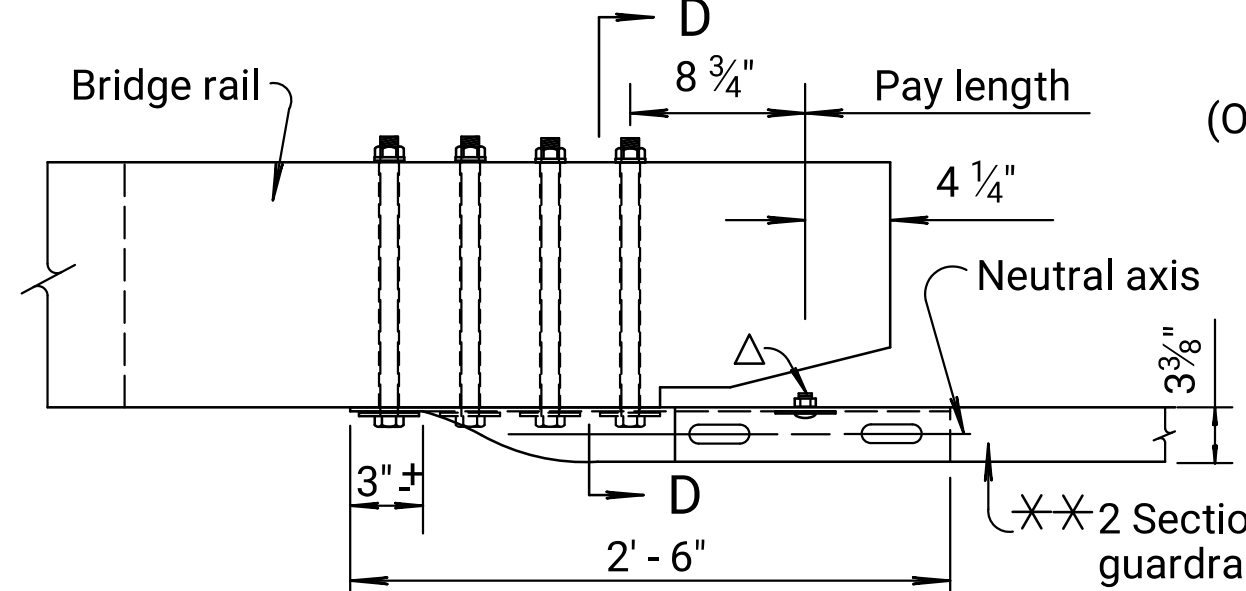
SECTION D-D



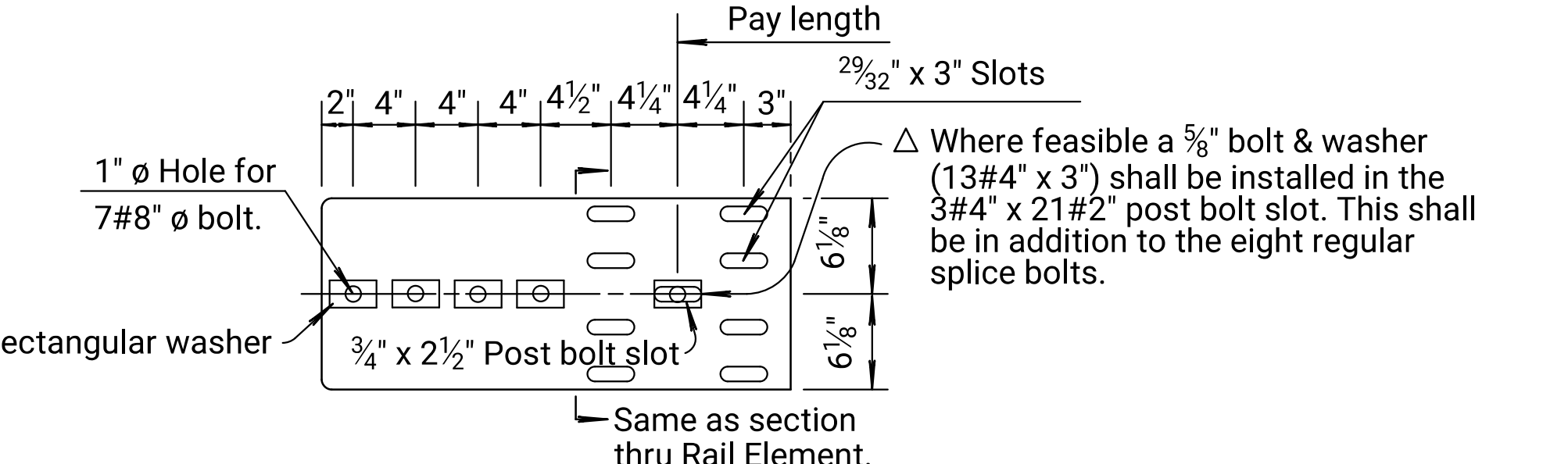
BEVELED WASHER



RECTANGULAR WASHER (Other approved washer may be used.)



PLAN SPECIAL END SHOE



ELEVATION SPECIAL END SHOE

One section of the two shall be considered as subsidiary to the bid item "Steel Plate Guardrail".

WOOD POSTS

* See Standard Drawing RD611 for details of Bolts A, & C.

STEEL POSTS

⊗ Blocks used with steel posts shall be grooved to fit over the flange of the post and may be Wood or Polymer.

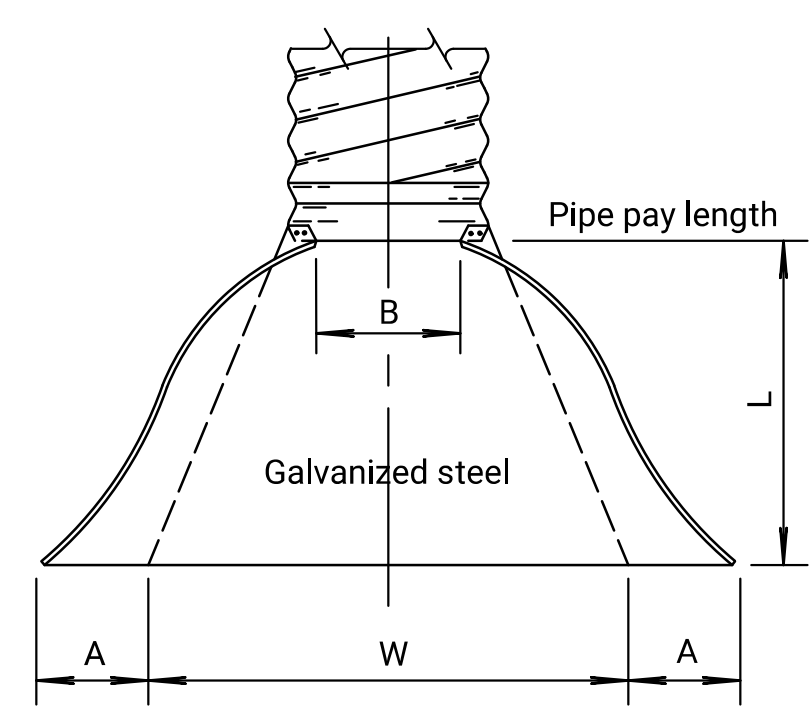
NO.	DATE	REVISIONS	BY	APPD
14	12-14-10	Revised notes 28" rail height	S.W.K.	J.O.B.
13	04-02-08	Removed Galvanized callout	S.W.K.	J.O.B.
12	02-06-07	Corrected spelling error	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION				
W-BEAM WITH RUBRAIL BRIDGE APPROACH TRANSITION DETAILS				
RD616				
DESIGNED	APPD.	QUANTITIES	TRACED	James O. Brewer
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

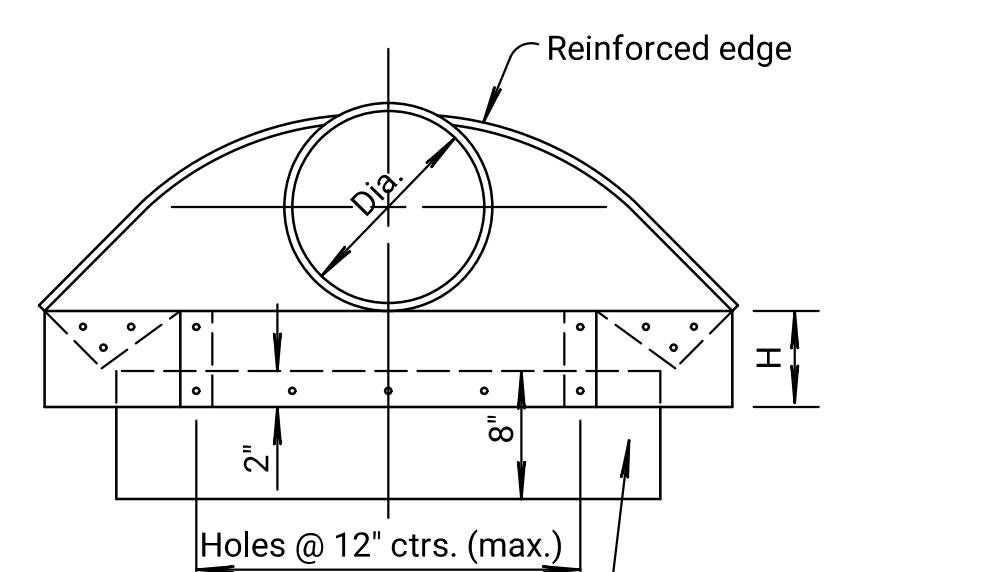
Plotted by: rsnw 3-JUL-2024 18:05
 File: rd616.dgn

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	9	44

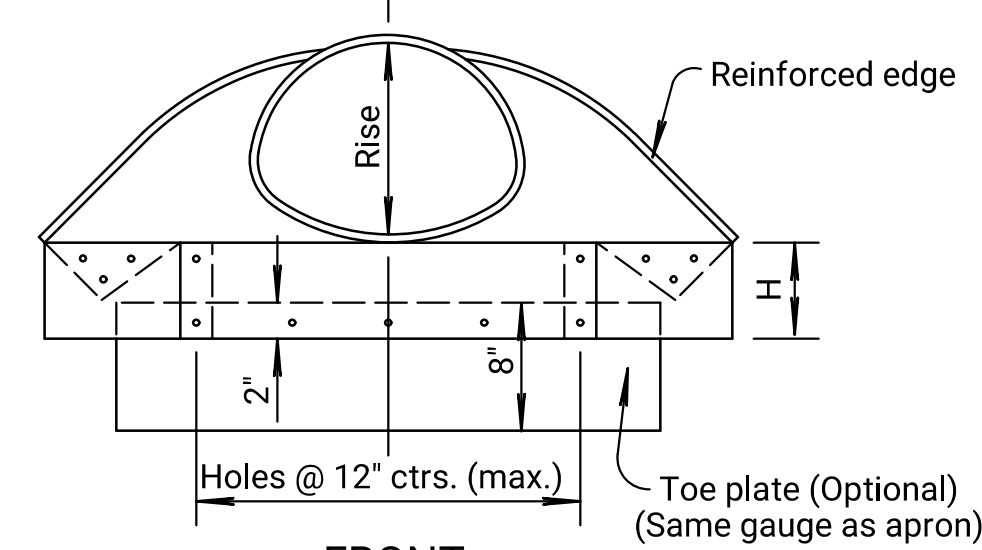
Note to Designer: KDOT Pipe Policy provides guidance in identifying the prohibited and/or restricted uses of CSP, ACSP, PEP, PVCP, CAP & RCP. Provide end sections of the same type and coating as the pipe. Exceptions to this are noted in the Standard Specifications. Refer to the KDOT Design Manual, Volume I (Part C), Road Section, "Elements of Drainage & Culvert Design" for structural pipe design information which includes: corrugations, sizes, gauges, maximum/minimum fill heights and classes of pipe.



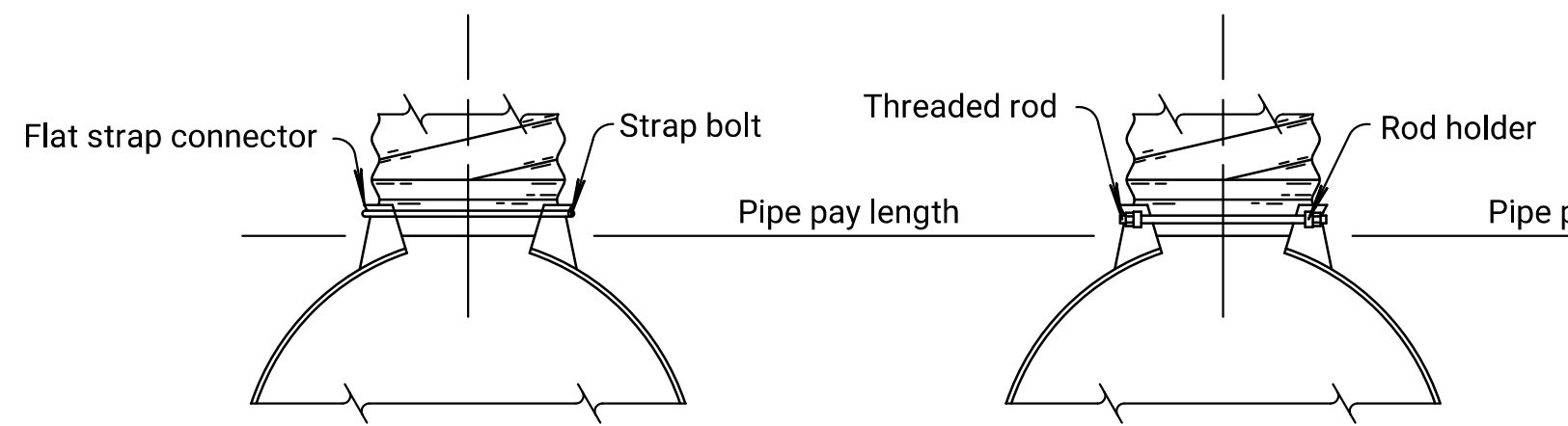
PLAN
(Illustrated with Type #3 Connection)



FRONT

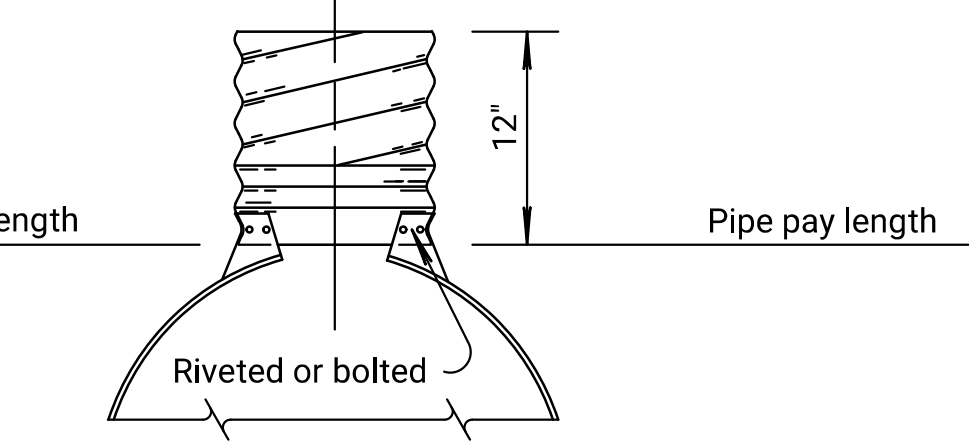


FRONT

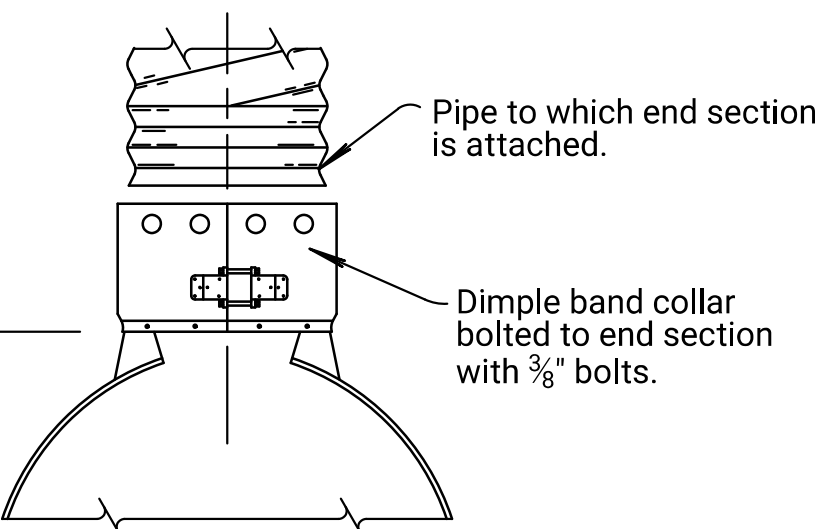


TYPE 1
Available in sizes 12" through 24" only.

TYPE 2
Available in sizes 30" and 36" Round and 17"x13" through 57"x38" Pipe-Arches.



TYPE 3
Available in sizes 42" through 96" Round and 60"x46" through 81"x59" Pipe-Arches.



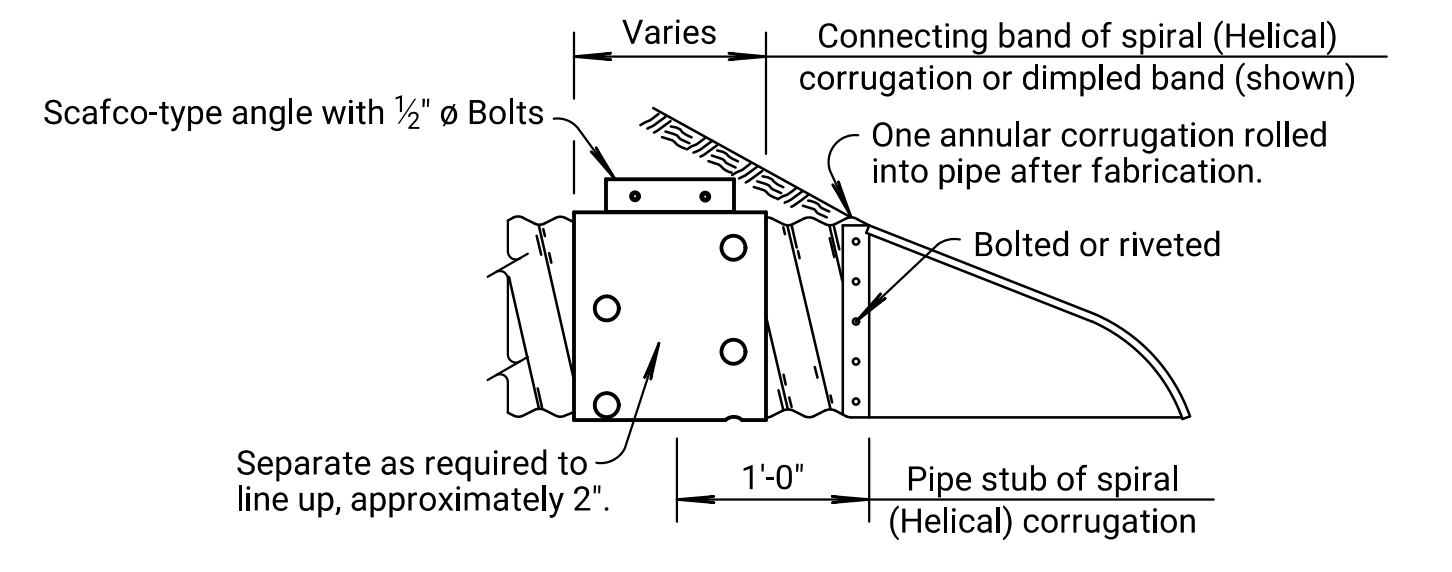
TYPE 5
Available for all Round and equivalent Pipe-Arch sizes, (Type 1 and Type 2 connections are recommended for the smaller sizes with annular ends).

GENERAL NOTE for END SECTIONS
End section material shall follow KDOT Pipe Policy for geographic location. Location shall govern use of CS (Galvanized), ACS (Aluminized) or CA (Aluminum) (Type I) End Section. Pipe material and End Section material shall be the same with no mixing of types per location.

Toe plate extension, when specified, is an accessory and shall be the same gauge and metal as end section. Toe plate shall be punched to match holes in apron lip and attached with furnished 3/8" diameter nuts & bolts.
 W + 10" for 12" to 30" diameter pipes inclusive.
 W + 20" for 36" to 84" diameter pipes inclusive.
 W + 10" for pipe-arches with a rise of 13" to 29" inclusive.
 W + 20" for pipe-arches with a rise of 33" to 59" inclusive.

Multiple panel end sections may contain dual gauges of like metal and shall have lap seams which are tightly joined with rivets or bolts. For 60" and larger diameter round pipe end sections and 77"x52" arch pipe end sections, the reinforced edges are supplemented with stiffener angles. The angles are attached with nuts and bolts. Angle reinforcement may be required under the center panel seams of 73"x55" and larger arch pipe end sections depending on manufacturer.

Other approved designs may be used in lieu of type shown.
 Connection of end sections by welding will not be permitted.



SPIRAL (HELICAL) CORRUGATION

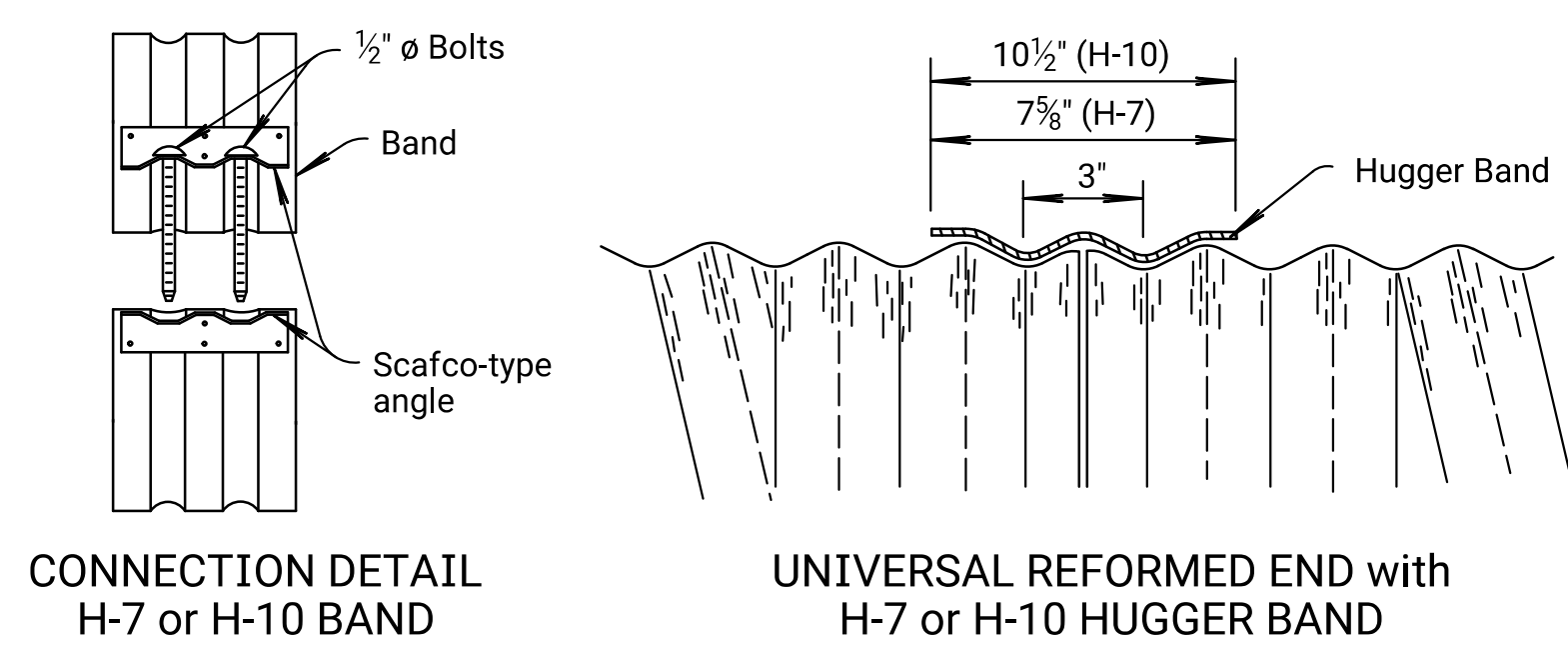
For all sizes of round and arch culvert pipes having Spiral (Helical) corrugations, the end sections and connecting bands shall be as shown above.

Thickness CSP/ACSP	Thickness CAP	Gauge
0.064"	0.060"	16 ga.
0.079"	0.075"	14 ga.
0.109"	0.105"	12 ga.
0.138"	0.135"	10 ga.
0.168"	0.164"	8 ga.

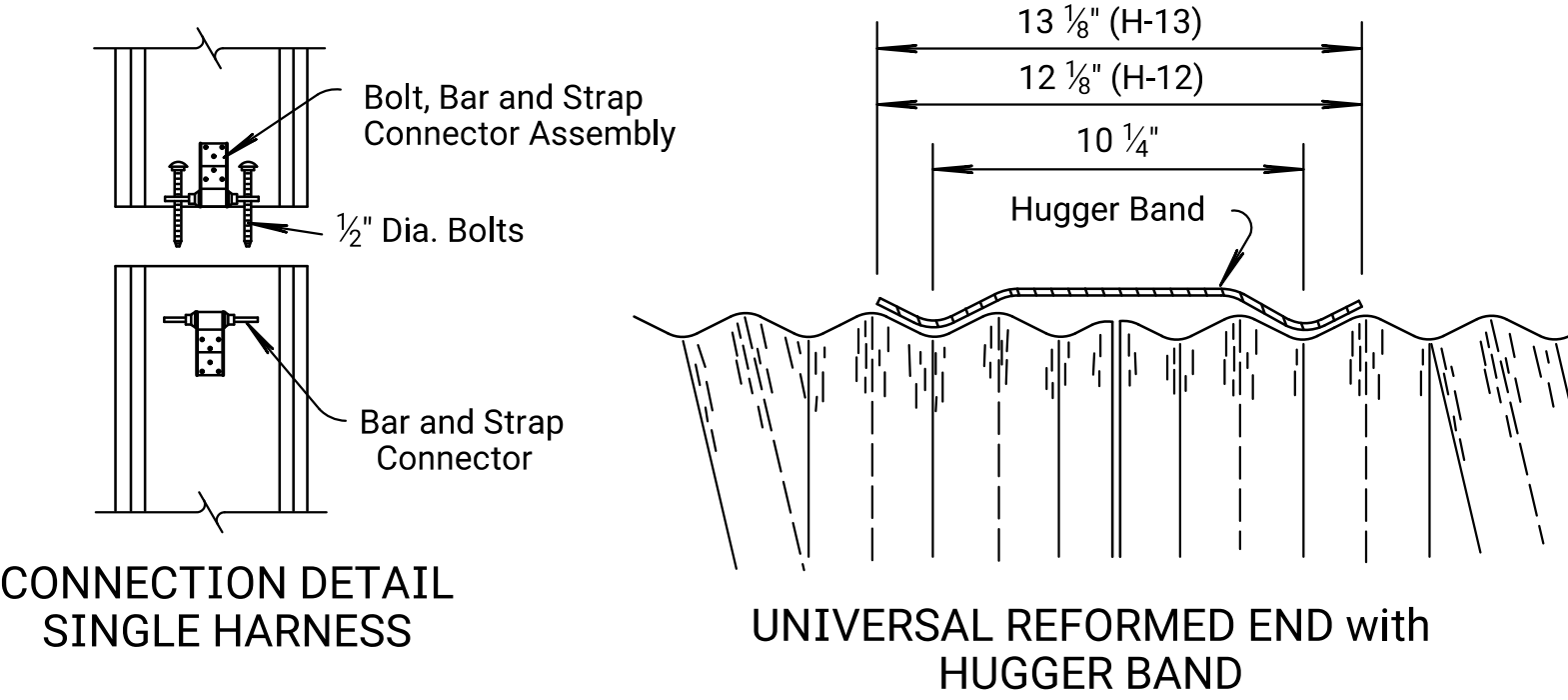
Pipe Dia. (In.)	CS, ACS or CA Gauge	Dimensions in Inches					Approx. Slope
		A (min.)	B (max.)	H (min.)	L (±2")	W (min.)	
12"	16	5	7	6	21	22	2 1/2: 1
15"	16	6	8	6	26	28	2 1/2: 1
18"	16	7	10	6	31	34	2 1/2: 1
21"	16	8	12	6	36	40	2 1/2: 1
24"	16	9	13	6	41	46	2 1/2: 1
30"	14	11	16	8	51	55	2 1/2: 1
36"	14	13	19	9	60	70	2 1/2: 1
42"	12	15	25	10	69	82	2 1/2: 1
48"	12	17	29	12	78	88	2 1/2: 1
54"	12	17	33	12	84	100	2 1/2: 1
60"	12/10	17	36	12	87	112	2: 1
66"	12/10	17	39	12	87	118	2: 1
72"	12/10	17	44	12	87	120	2: 1
78"	12/10	17	48	12	87	130	1 1/2: 1
84"	12/10	17	52	12	87	136	1 1/2: 1
90"	12/10	17	58	12	87	142	1 1/2: 1
96"	12/10	17	58	12	87	144	1 1/2: 1

Bid Designation Sq. Ft.	Nom. W.W. Area Sq. Ft.	Pipe Arch Span & Rise	Dimensions in Inches 2 1/2" x 1/2" Corrugations					Dimensions in Inches 3" x 1" or 5" x 1" Corr.					Approx. Slope		
			CS, ACS or CA Gauge	A (min.)	B (max.)	H (min.)	L (±2")	W (min.)	CS, ACS or CA Gauge	A (min.)	B (max.)	H (min.)		L (±2")	W (min.)
1.0	1.1	17" x 13"	16	5	9	6	20	28						2 1/2: 1	
1.5	1.6	21" x 15"	16	6	11	6	24	34						2 1/2: 1	
2.0	2.2	24" x 18"	16	7	12	6	28	40						2 1/2: 1	
2.5	2.9	28" x 20"	16	7	16	6	32	46						2 1/2: 1	
3.0 or 4.0	4.5	35" x 24"	14	9	16	6	39	58						2 1/2: 1	
5.0 or 6.0	6.5	42" x 29"	14	11	18	7	46	73						2 1/2: 1	
7.0 or 8.5	8.9	49" x 33"	12	12	21	9	53	82						2 1/2: 1	
10.0 or 11.0	11.7	53" x 41"							12	17	26	12	63	88	2: 1
10.0 or 11.0	11.6	57" x 38"	12	16	26	12	62	88							2: 1
12.5 or 14.0	15.6	60" x 46"							12	17	36	12	70	100	2: 1
12.5 or 14.0	14.7	64" x 43"	12	17	30	12	69	100							2: 1
16.5	19.3	66" x 51"							12/10	17	36	12	70	112	1 1/2: 1
16.5	18.1	71" x 47"	12/10	17	36	12	77	112							1 1/2: 1
21.0	23.2	73" x 55"							12/10	17	36	12	77	124	1 1/2: 1
21.0	21.9	77" x 52"	12/10	17	36	12	77	124							1 1/2: 1
25.0	27.4	81" x 59"							12/10	17	44	12	77	136	1 1/2: 1
25.0	26.0	83" x 57"	12/10	17	44	12	77	130							1 1/2: 1
32.0	32.1	87" x 63"							12/10	17	44	12	77	136	1 1/2: 1
36.0	37.0	95" x 67"							12/10	17	44	12	87	160	1 1/2: 1
42.0	42.4	103" x 71"							12/10	17	44	12	87	172	1 1/2: 1
47.0	48.0	112" x 75"							12/10	17	44	12	87	172	1 1/2: 1

(Information listed in these tables are nominal and may vary by manufacturer.)



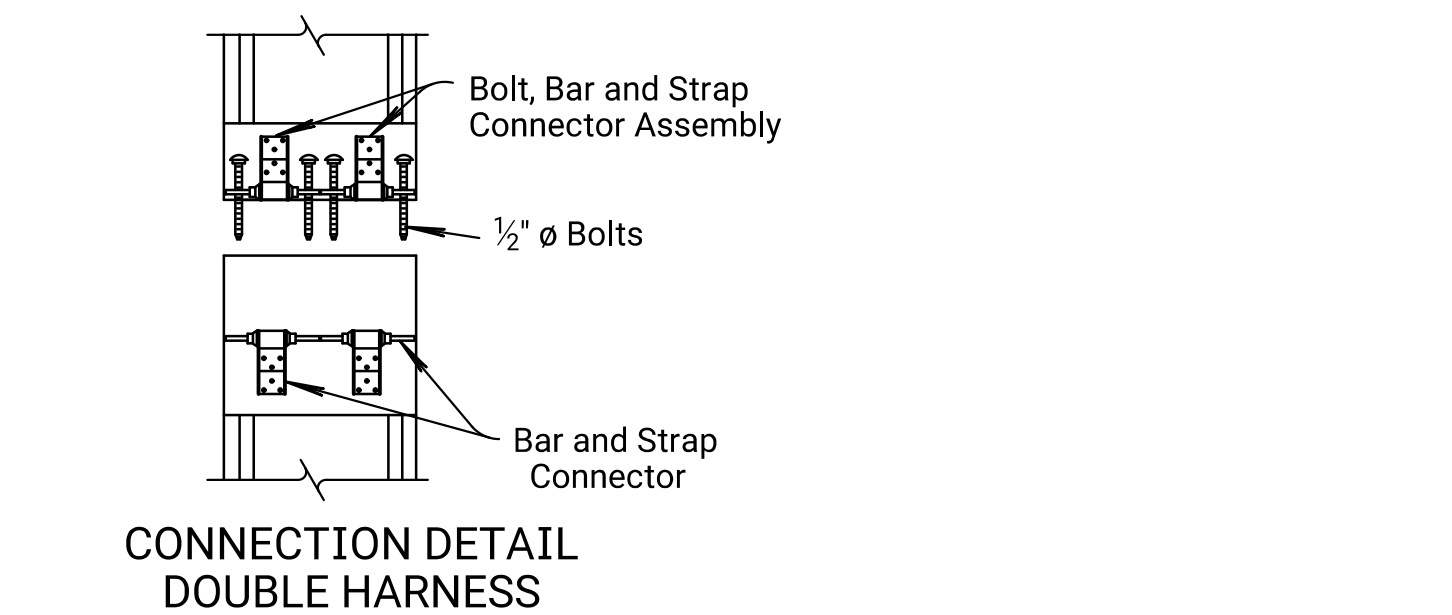
DETAILS FOR H-7 HUGGER BAND (12" thru 36") or H-10 HUGGER BAND (12" thru 120")



DETAILS FOR H-12 or H-13 HUGGER BAND

Pipe Dia. Inches	Minimum Gauge of Round Pipe				
	2 1/2" x 1/2" Corr.	3" x 1" Corr.	5" x 1" Corr.	2 1/2" x 1/2" Corr.	3" x 1" Corr.
	CSP or ACSP	CSP or ACSP	CSP or ACSP	CAP	CAP
12"	14			16	
15"	14			16	
18"	14			16	
21"	14			16	
24"	14			16	
30"	14			14	16
36"	14			14	16
42"	14			12	16
48"	12	14	16	14	16
54"	12	14	16	14	16
60"	10	14	16	14	16
66"	10	14	16	14	16
72"	10	14	16	14	16
78"	8	14	14	14	14
84"	8	14	14	14	14
90"		14	14	14	14
96"		12	12	12	12
102"		12	12	12	12
108"		12	12	12	12
114"		12	12	12	12
120"		10	10	10	10

Bid Designation Sq. Ft.	Pipe Dimension Span & Rise	Sq. Ft.	Equiv. Round Pipe Diameter	Minimum Gauge of Arch Pipe				
				2 1/2" x 1/2" Corr.	3" x 1" Corr.	5" x 1" Corr.	2 1/2" x 1/2" Corr.	3" x 1" Corr.
				CSP or ACSP	CSP or ACSP	CSP or ACSP	CAP	CAP
1.0	17" x 13"	1.1	15"	14			16	
1.5	21" x 15"	1.6	18"	14			16	
2.0	24" x 18"	2.2	21"	14			16	
2.5	28" x 20"	2.9	24"	14			14	
3.0 or 4.0	35" x 24"	4.5	30"	14			14	
5.0 or 6.0	42" x 29"	6.5	36"	14			12	
7.0 or 8.5	49" x 33"	8.9	42"	14			12	
10.0 or 11.0	53" x 41"	11.7	48"		14			
10.0 or 11.0	57" x 38"	11.6	48"	12			10	
12.5 or 14.0	60" x 46"	15.6	54"		14			14
12.5 or 14.0	64" x 43"	14.7	54"	12			10	
16.5	66" x 51"	19.3	60"		14			14
16.5	71" x 47"	18.1	60"	10			8	
21.0	73" x 55"	23.2	66"		14			14
21.0	77" x 52"	21.9	66"	8				
25.0	81" x 59"	27.4	72"		14	12		12
25.0	83" x 57"	26.0	72"	8				
32.0	87" x 63"	32.1	78"		12	12		12
36.0	95" x 67"	37.0	84"		12	12		12
42.0	103" x 71"	42.4	90"		12	12		10
47.0	112" x 75"	48.0	96"		12	12		8
54.0	117" x 79"	54.2	102"		10	10		
60.0	128" x 83"	60.5	108"		10	10		
67.0	137" x 87"	67.4	114"		10	10		
74.0	142" x 91"	74.5	120"		8	8		



CONNECTION DETAIL DOUBLE HARNESS

GENERAL NOTE for METAL PIPE
Culvert "Type" listed may be CSP, ACSP, CAP, RCP, PVCP & PEP within guidelines of KDOT Pipe Policy for geographic location. More than one pipe "Type" may be acceptable for a design location with allowable types listed for each site.
There shall be no payment for gain in pipe length due to fit of pipe at connecting band.
When Hugger Bands are used, the H-7 Hugger Band may be used on circular pipes 36" diameter and smaller or pipe arches 42" x 29" and smaller. The H-10 Hugger Band may be used on 12" thru 120" pipe. The H-12 or H-13 Hugger Band are for pipe sizes larger than 36" diameter or 42" x 29" arch pipe.

⊗ Pipe gauge listed in the tables on this sheet are minimum for E=750 p.s.i. soil. Pipe gauge will be determined for each site based on the Design Manual Volume I- Part C Fill Height Tables and shall be listed in the Pipe Culvert Summary. Gauges shown on this Standard Drawing are KDOT minimum and may not be industry minimum gauge.

In geographic areas that allow CSP (24" or smaller arched or round pipe) for entrance and side road installation with less than 3,000 AADT, 16 gauge ACSP may be substituted for 14 gauge CSP.
Aluminum or aluminized pipes or end sections shall be coated with an asphaltic paint when in contact with fresh concrete in accordance with the Standard Specifications.

04	09-10-09	Rev. Round and Arch tables, add. Alum.	S.W.K.	J.O.B.
03	01-20-09	Rev. Round and Arch tables, add. Alum.	S.W.K.	J.O.B.
02	04-18-08	Rev. layout, details, tables and notes	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

METAL END SECTION FOR ROUND & ARCH METAL CULVERTS (TYPE I) & PIPE GAUGE TABLES

RD660

FHWA APPROVAL	12-16-09	APPD	James O. Brewer
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	11	44

SUMMARY OF QUANTITIES											
Item Location	Excavation		Concrete		Reinforcing Steel	* Piles (Steel)	Cast Steel	Contractor	Slope	Geotextile	
	Class I Cu. Yds.	Class II Cu. Yds.	(Grade 4.0) (AE) (SW) Cu. Yds.	(Grade 4.0) (AE) Cu. Yds.	(Grade 60) Lbs.	(HP 12x53) Lin. Ft.	Pile Points Each	Furnished PDA Each	Protection (Riprap Stone) Cu. Yds.	Fabric Sq. Yds.	
Abutment No. 1	51		**		**	250	4	1	203	73	
Pier No. 1		60		24	965	360	6				
Pier No. 2		60		24	965	370	6	1			
Abutment No. 2	51		**		**	240	4		140	47	
Substr. Total				48	1930						
Superstr. Total			205.9		56,860						
Total	102	120	205.9	48	58,790	1220	20	2	343	120	

** Quantities are included in the Superstr. Total Quantity.

* NOTE: Only steel pile HP12X53 shall be used on this project.

† Summary of Piling

Abutment No. 1 3 @ 60' & 1 @ 70' for use with the PDA
 Pier No. 1 6 @ 60'
 Pier No. 2 5 @ 60' & 1 @ 70' for use with the PDA
 Abutment No. 2 4 @ 60'

GENERAL NOTES

CONSTRUCTION SPECIFICATIONS: Kansas Department of Transportation, Standard Specifications for State Road and Bridge Construction, 2015 Version, and Special Provisions.

EMBANKMENT: Complete the embankment at the abutments as shown on the Bridge Excavation sheet prior to driving the abutment piling or commencing with the abutment footing excavation.

BRIDGE EXCAVATION: Elevation 1933.00 shall designate the Excavation Boundary Plane of Class I and Class II Excavation; Class I above the plane, Class II below the plane. See the Bridge Excavation sheet for the limits of pay excavation.

BACKFILL COMPACTION: Compact backfill at the abutments.

PILING: Driven piles will develop their capacity from skin friction and end bearing in the overburden soils and weathered bedrock. Driving shall stop when in the opinion of the Engineer additional driving may damage the piling. Drive all piling to the Pile Driving Formula Load of:

Abutment No. 1	55 Tons
Pier No. 1	84 Tons
Pier No. 2	84 Tons
Abutment No. 2	55 Tons

As a minimum drive each pile to the load and penetration, but in no case shall the pile be driven to more than 110% of Pile Driving Formula Driving Load. At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Engineer may request that the Pile Driving Analyzer (PDA) equipment be used.

CONTRACTOR FURNISHED PDA: Use the Pile Driving Analyzer equipment at the locations shown on the Construction Layout. Use Pile Driving Analyzer equipment and methods compliant with KDOT Special Provisions. The piling shall remain in place as permanent piling. Drive the piling to the resistance value of (Strength I divided by Phi). At any location where problems are experienced, pile damage suspected, or the Pile Driving Formula Load occurs significantly above the design tip elevation, the Engineer may request that the Pile Driving Analyzer (PDA) equipment be used.

REMOVAL OF EXISTING STRUCTURE: The Contractor shall remove the existing 17'-25'-17' Steel Beam Continuous (SBMC) Bridge #000730643005408. All items of the existing structure shall become the property of the Contractor and shall be removed from the site.

DEMOLITION PLANS: This is a Category A Demolition. Submit Detailed Demolition Plans to the Owner's designated Engineer for review and distribution per KDOT Specifications. No Demolition work will begin without approved Demolition Plans. A Licensed Professional Engineer is not required.

CONCRETE: Superstructure concrete is bid as Concrete (Grade 4.0)(AE)(SW). Substructure concrete is bid as Concrete (Grade 4.0)(AE). Bevel all exposed edges of all concrete with a 3/4" triangular molding, except as otherwise noted on the plans. Construction joints are optional with the Contractor, but if used, place only at locations shown, or at locations approved by the Engineer.

REINFORCING STEEL: All reinforcing steel dimensions are to the centerline of bars unless otherwise noted. All reinforcing steel shall conform to the requirements of ASTM A615, Grade 60.

CAMBER: Provide camber as shown on the Camber Diagram unless the Contractor uses either long span steel beam falsework (concrete dead load deflection greater than 1/4") or timber falsework with greater than 12'-0" clear span. If either case exists, submit falsework plans that show the additional required camber.

FALSEWORK PLANS: A licensed Professional Engineer shall design the falsework details. Details shall bear the seal of a licensed Professional Engineer. Submit electronic plans conforming to Section 105 of the Standard Specification with details in compliance with KDOT Specifications to the Field Engineer for review.

FALSEWORK INSPECTION: This project has falsework plan requirements which are considered "Category 2" by KDOT specifications. If falsework deficiencies or variations from the approved and sealed plans are found, the falsework design Engineer of Record will provide written approval of the changes. If for the convenience of the Contractor the falsework becomes "Category 1" by the use of non-typical supports; then the inspection and review requirement of "Category 1" will be fully enforced, but at no cost to the State. "Category 2" falsework inspection is not paid for directly, but is subsidiary to other bid items.

FALSEWORK: Leave the falsework in place for the entire unit until 15 days after the last concrete pour for the unit or longer as directed by the Engineer.

CONTRACTOR CONSTRUCTION STAKING: Contractor Construction Staking for clear span bridges requires two independent surveys. See KDOT Specifications.

CONCRETE PLACING SEQUENCE: The sequence of placing concrete in the slab and curbs shall be as shown, or the Contractor may submit an alternate placing sequence for review. Submit the alternate placing sequence to the Engineer at the Preconstruction Conference. Include the proposed rate of concrete placement in C.Y./h, the plant capacity, placement direction, construction joint location, a description of the equipment used in placing the concrete, proposed admixtures, and the quantity of concrete in each placing segment. Any additional cost for the Contractor's alternate plan of placing concrete, including admixtures, shall be at the Contractor's expense and shall be considered subsidiary to the bid item, "Concrete (Grade 4.0)(AE)(SW)". Approval of the Contractor's alternate sequence is required prior to placement of concrete in the deck.

CONSTRUCTION LOADS: Limited traffic is permitted on the new one-course deck during the curing period, keep any exposed deck wet during the curing period. See KDOT Specifications Section 710 Table 710-1 and 710-2 for additional information.

TEMPERATURE: The design temperature for all dimensions is 60° F.

QUANTITIES: Items not listed separately in the Summary of Quantities are subsidiary to other items in the proposal.

DIMENSIONS: All dimensions shown on the design plans are horizontal dimensions unless otherwise noted. Make necessary allowances for roadway grade and cross slope.

CONSTRUCTION JOINTS: The construction joints shown are optional with the Contractor. If used, place the construction joints only at locations shown or at locations approved by the Engineer.

DECK FINISHING: Set the finishing machine normal to the centerline of the structure for striking off and screeding the concrete.

CORRAL RAIL: Build the corral rail after the falsework is struck.

SLOPE PROTECTION (Riprap Stone): Place Slope Protection (Riprap Stone) to the limits and thicknesses shown on the plans or as directed by the Engineer. Use Riprap Stone classified as Light 200 Pounds. Place a 10 foot wide mat of geotextile under the slope protection on the berm and berm slopes and centered on the drip lines of the slab.

INDEX TO BRIDGE DRAWINGS	
Sheet No.	Drawing
11	General Notes and Quantities
12	Contour Map
13	Construction Layout
14	Engineering Geology
15	Abutment Details
16	Pier Details
17-18	Superstructure Details
19	Corral Rail
20	Bill of Reinforcing Steel and Bending Diagrams
Standards	
21	Bridge Excavation
22	Standard Pile Details
23	Supports and Spacers for Reinforcing Steel

DESIGN DATA

DESIGN SPECIFICATIONS: AASHTO Specifications, 8th Edition. Load and Resistance Factor Design.

DESIGN LOADING: HL-93

Design Dead Load includes an allowance of 25 psf for a future wearing surface.

UNIT STRESSES:

Concrete (Grade 4.0)(AE)	f'c =	4 ksi
Concrete (Grade 4.0)(AE)(SW)	f'c =	4 ksi
Reinforcing Steel (Grade 60)	fy =	60 ksi
Steel Piles	fy =	50 ksi

LRFD DESIGN PILE LOAD:

Design Loading (Tons/Pile)	Strength 1	Service 1	Phi
Abutment	55	36	0.65
Piers	84	58	0.65

LRFR RATING FACTORS		
Rating Level	Inventory	Operating
Truck		
HL-93	1.46	1.90
NRL (40 Tons)		2.06
EV2 (28.75 Tons)		3.02
EV3 (43 Tons)		2.08
AASHTO LRFD 8th Edition		

6	10/19/15	Added Asbestos NOT8221 Option	JPJ	CER
5	2/4/15	Modified Per 2015 Specification	JPJ	CER
4	4/7/14	Current Release	JPJ	CER
3	2/12/14	Added Benchmark	JPJ	CER
2	08/2/12	ADDED NOT3135 & NOT3145	JPJ	TLF
1	04/29/10	ADDED RATING TABLES	JPJ	KFH
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 000730643005409 (OS 93) Sta. 102+85.00

GENERAL NOTES AND QUANTITIES

Proj. 73 C-5224-01 Pawnee Co.

SHEET NO.	OF	SCALE	APPD
DESIGNED	OF	DETAILED	QUANTITIES
DESIGN CK.	OF	DETAIL CK.	QUAN. CK.
			CADD
			CADD CK.

Std. Base File:
 Plotted By: rsnw
 File: BR200.dgn
 Plot Location:
 Plot Date: 3-JUL-2024 18:06

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	12	44

Utility	Owner	Contact	Phone
Telephone	AT&T	Chance Z. Collins	316-776-6010

CP #1
 N = 545,897.98 E = 15,566,658.85
 1. Set 5/8" Rebar Flush
 2. App. ϕ of N-S Rd. 24.4' W
 3. N-S Running Fc. Line 9.4' E
 4. Spk. in S. Face of Post 81.9' NW

P.O.T Sta. 97+00.00
 N 15,566,639.0539' E 545,702.0903'
 Not Set

CP #2
 N = 546,374.03 E = 15,566,656.20
 1. Set 5/8" Rebar Flush
 2. N-S Running Fc. Line 10.3' E
 3. App. ϕ of N-S Rd. 25.2' W
 4. E of Fence Running N-S 81.9' NW

P.O.T Sta. 109+00.00
 N 15,566,598.9813' E 546,963.7815'
 Not Set

CP #3
 N = 546,600.82 E = 15,566,606.69
 1. Set 5/8" Rebar Flush
 2. N-S Running Fc. Line 9.7' W
 3. App. ϕ of N-S Rd. 25.9' E
 4. W. of Fence Running N-S 54.6' E

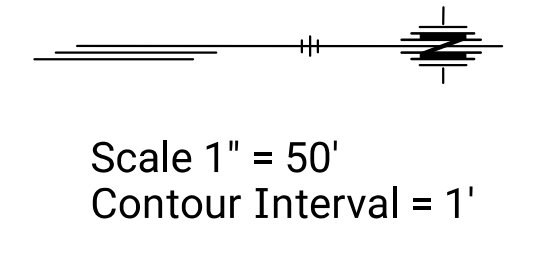
General Notes

The Contractor shall remove the existing 17'-25'-17" Steel Beam Continuous (SBMC) Bridge #000730643005408. All items of the existing structure shall become the property of the Contractor and shall be removed from the site.

The Contractor shall complete the Embankment as shown on the bridge excavation sheet prior to the driving of the abutment piling.

The Contractor shall excavate the channel at the bridge site to the limits shown prior to the construction of the bridge.

All trees, hedge rows, shelterbelts, and wood shrubs not shown to be removed and located between the construction limits and the right-of-way line or easement lines shall be spared unless directed by the Engineer to be removed.

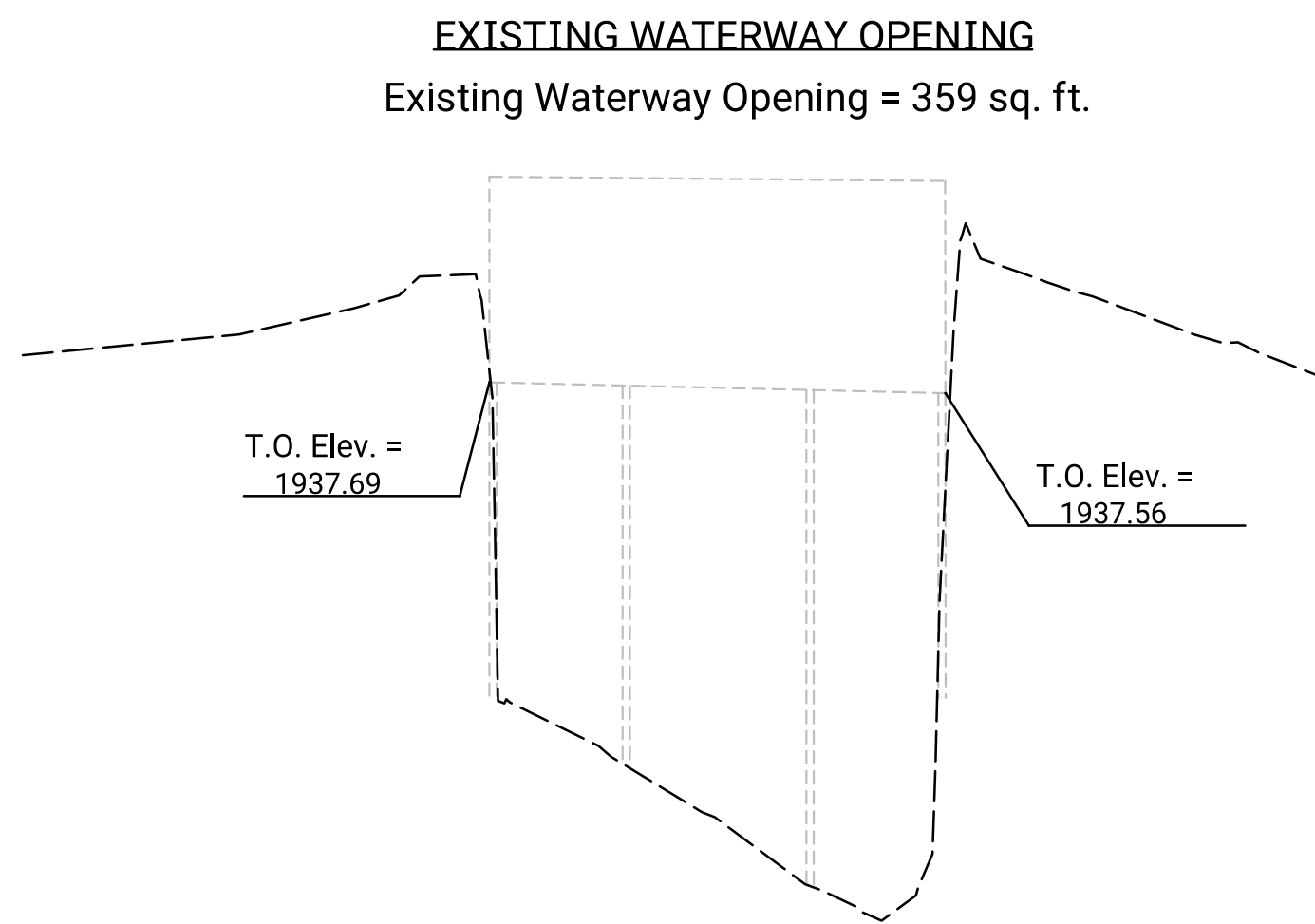
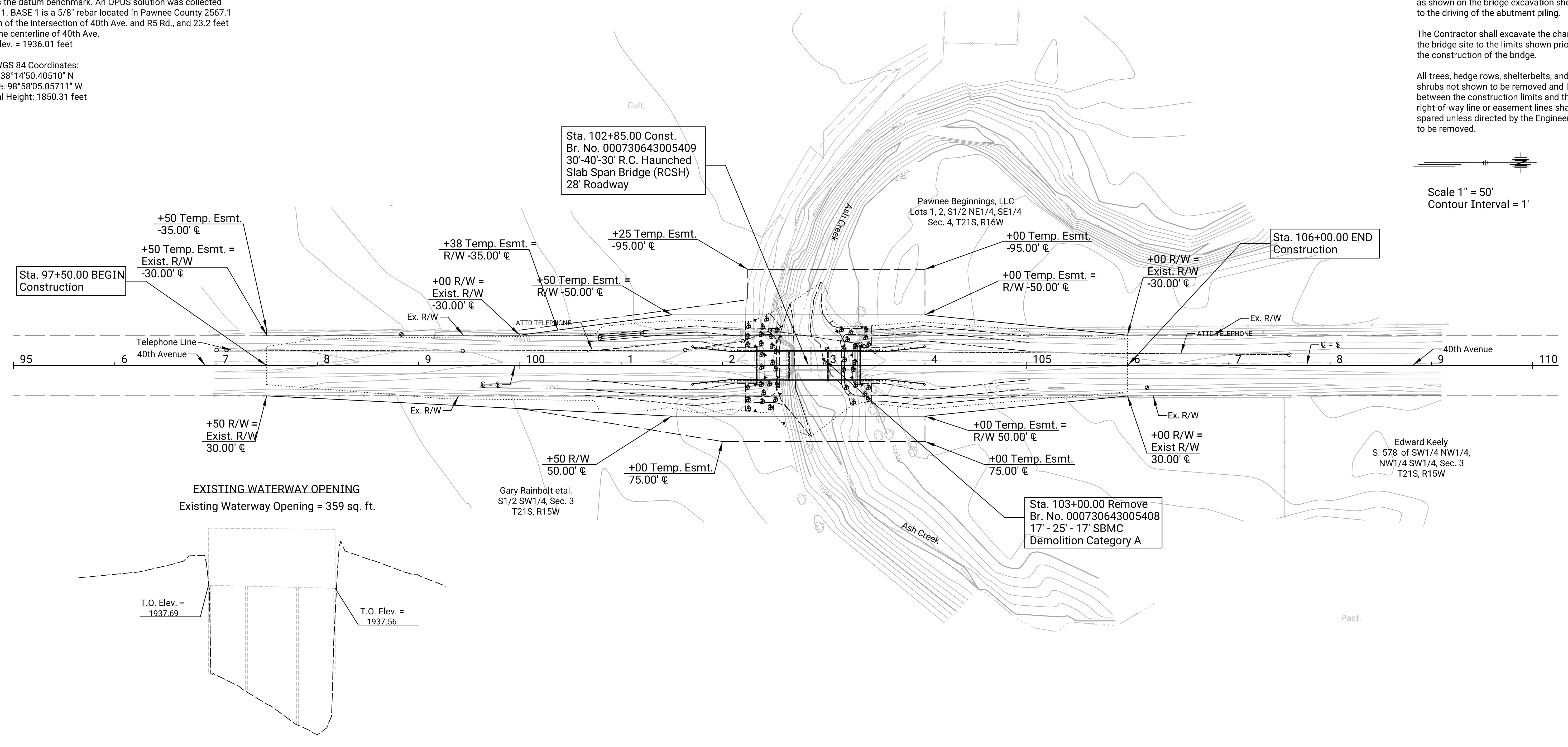


Horizontal Project Datum: KRCS Zone 15 Larned for project coordinates.

Vertical Datum: North American Datum NAVD 88 (Geoid 18)

DATUM BENCHMARK NAVD 88
 BASE 1 is the datum benchmark. An OPUS solution was collected for BASE 1. BASE 1 is a 5/8" rebar located in Pawnee County 2567.1 feet north of the intersection of 40th Ave. and R5 Rd., and 23.2 feet west of the centerline of 40th Ave.
 BASE 1 Elev. = 1936.01 feet

BASE 1 WGS 84 Coordinates:
 Latitude: 38°14'50.40510" N
 Longitude: 98°58'05.05711" W
 Ellipsoidal Height: 1850.31 feet



Plotted By: rsnow
 File: 2301810_bbr-01.dgn
 Plot Date: 3-JUL-2024 18:06

BM#1 Top of Driven "T" Post 60.8' W. of N-S Fc. Line & 385.1' S. of S. End of Ex. Bridge. Elev. = 1935.65

BM#2: MAG Nail Top Center of W. Handrail of Ex. Bridge Elev. = 1940.43

BM#3: Top of Driven "T" Post 1' W. of N-S Fc. & 288.0' N. of N. End of Ex. Bridge Elev. = 1935.69

Br. No. 000730643005409 (OS 93) Sta. 102+85.00
 CONTOUR
 40TH AVE OVER ASH CREEK
 Proj. 73 C-5224-01 Pawnee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED		DETAILED	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.
			CADD
			CADD CK.

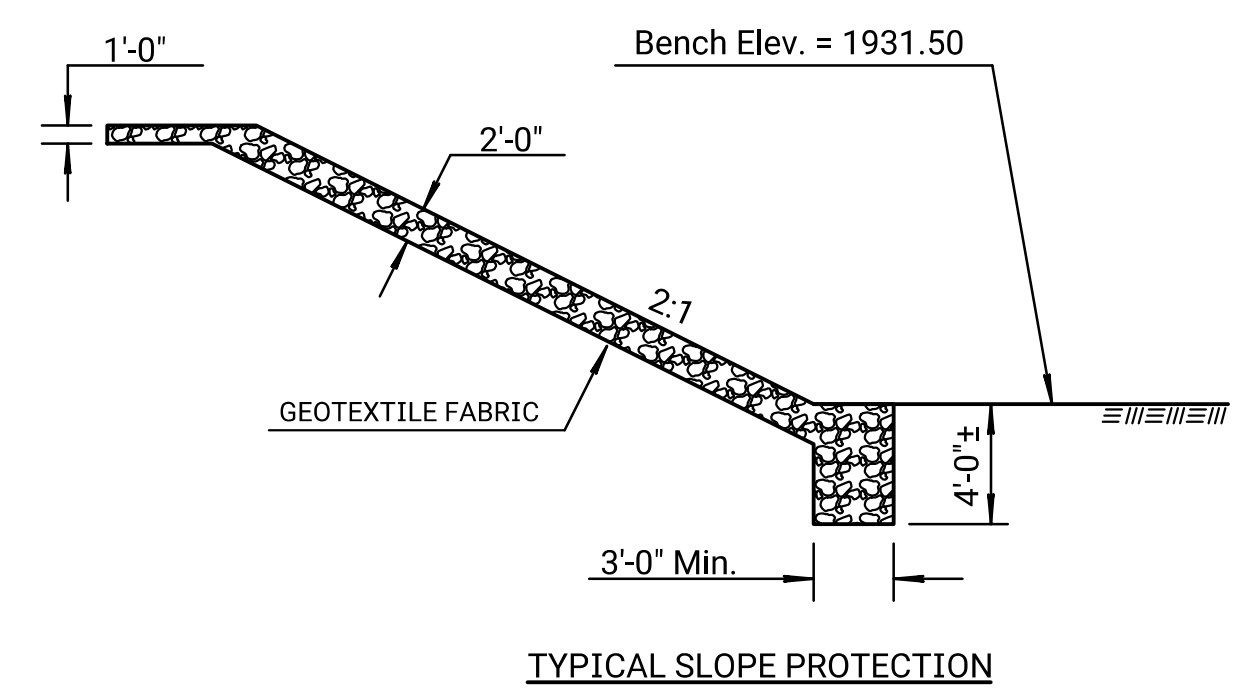
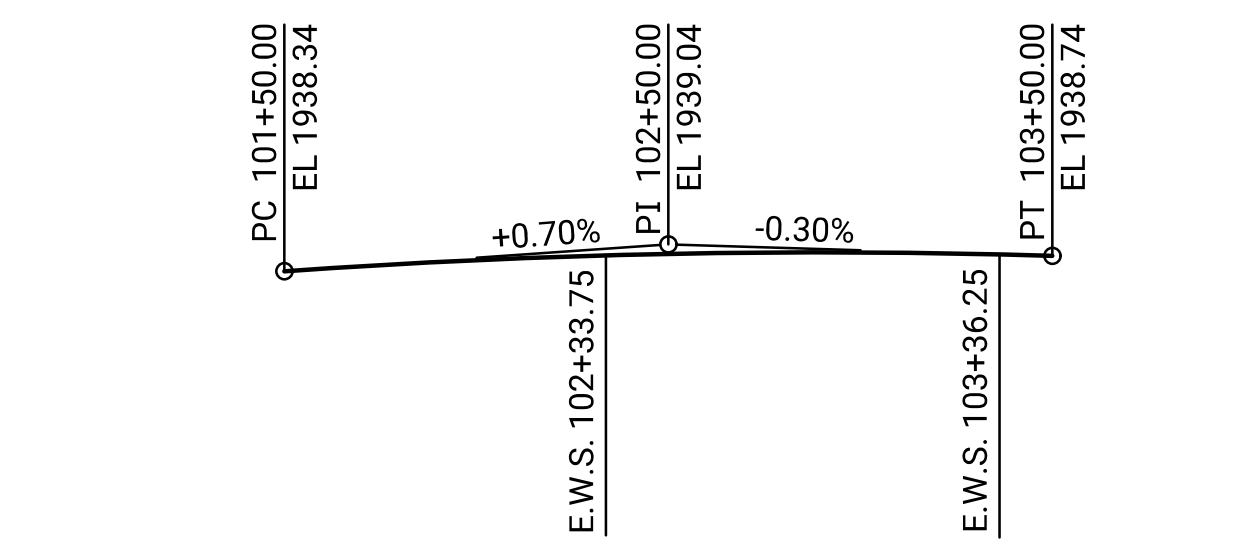
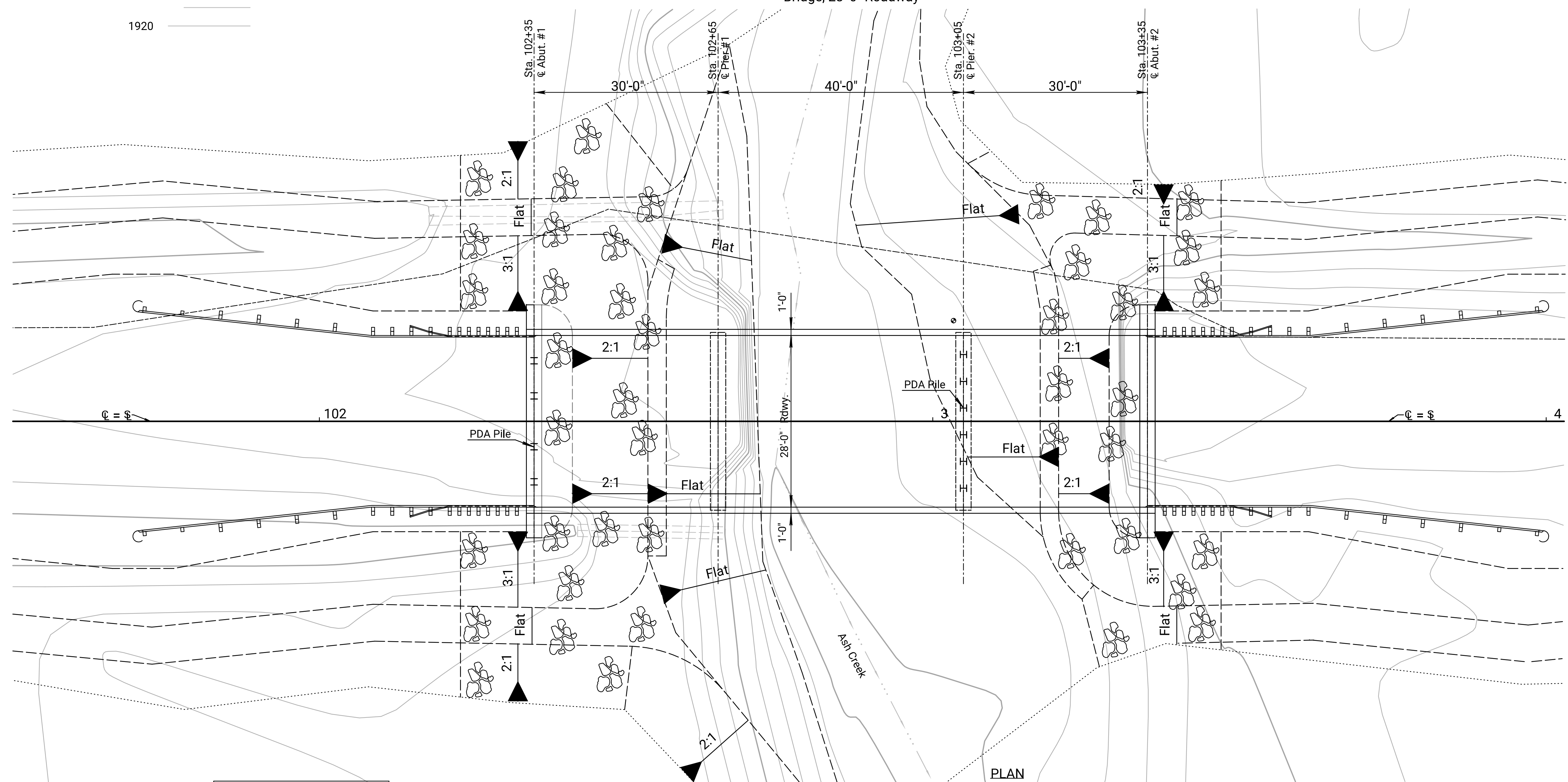
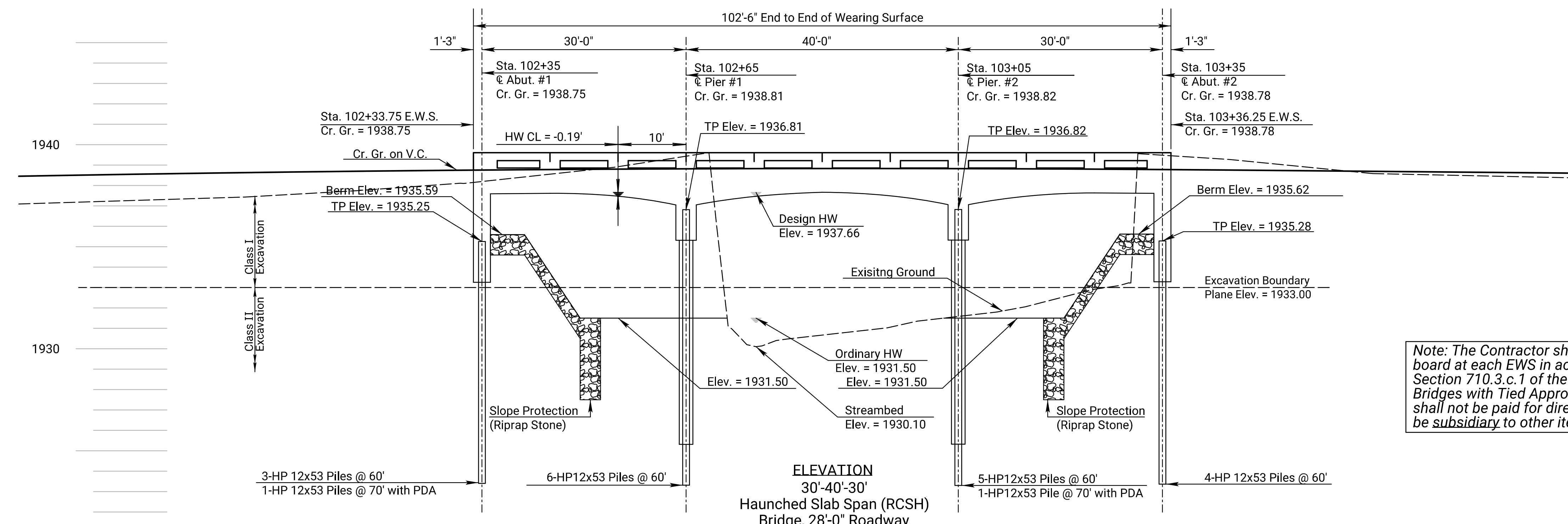
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	13	44

DRAINAGE DATA

Drainage Area	125.1 Sq. Mi.
Design Frequency	10 Yr.
Design Discharge @ (Q10)	1390 cfs
Design Velocity @ (Q10)	2.8 fps
Design High Water Elevation	1937.66 Ft.
Change in Design Backwater	-0.17 Ft.
Design Backwater Elevation	1937.85 Ft.
Overtopping Elevation (Sta. 99+95)	1937.80 Ft.
Overtopping Discharge	1390 cfs
Overtopping Frequency	10 Yr.
Discharge @ (Q100)	4580 cfs
Change in Backwater @ (Q100)	-0.16 Ft.
Backwater Elevation @ (Q100)	1939.31 Ft.
Historic Highwater Elevation	NA Ft.
Ordinary Highwater Elevation	1931.50 Ft.
Total Waterway Provided	502 Sq. Ft.
Design Waterway Provided	502 Sq. Ft.
Estimated Ordinary Highwater Discharge	26 cfs

Legend
TP = Top of Pile

Note: The Contractor shall place a header board at each EWS in accordance with Section 710.3.c.1 of the Specifications for Bridges with Tied Approaches. This work shall not be paid for directly, but shall be subsidiary to other items in the contract.



SLOPE PROTECTION (Riprap Stone): Place Slope Protection (Riprap Stone) to the limits and thickness shown on the plans or as directed by the Engineer. Use Riprap Stone Classified as Light 200 Pounds. Place a 10 foot wide mat of geotextile under the slope protection on the berm and berm slopes and centered on the drip lines of the slab.

Scale 1" = 10'

Elevations shown are to the NAVD 88 vertical datum.

BM#1 Top of Driven "I" Post 60.8' W. of N-S Fc. Line & 385.1' S. of S. End of Ex. Bridge. Elev. = 1935.65

BM#2: MAG Nail Top Center of W. Handrail of Ex. Bridge Elev. = 1940.43

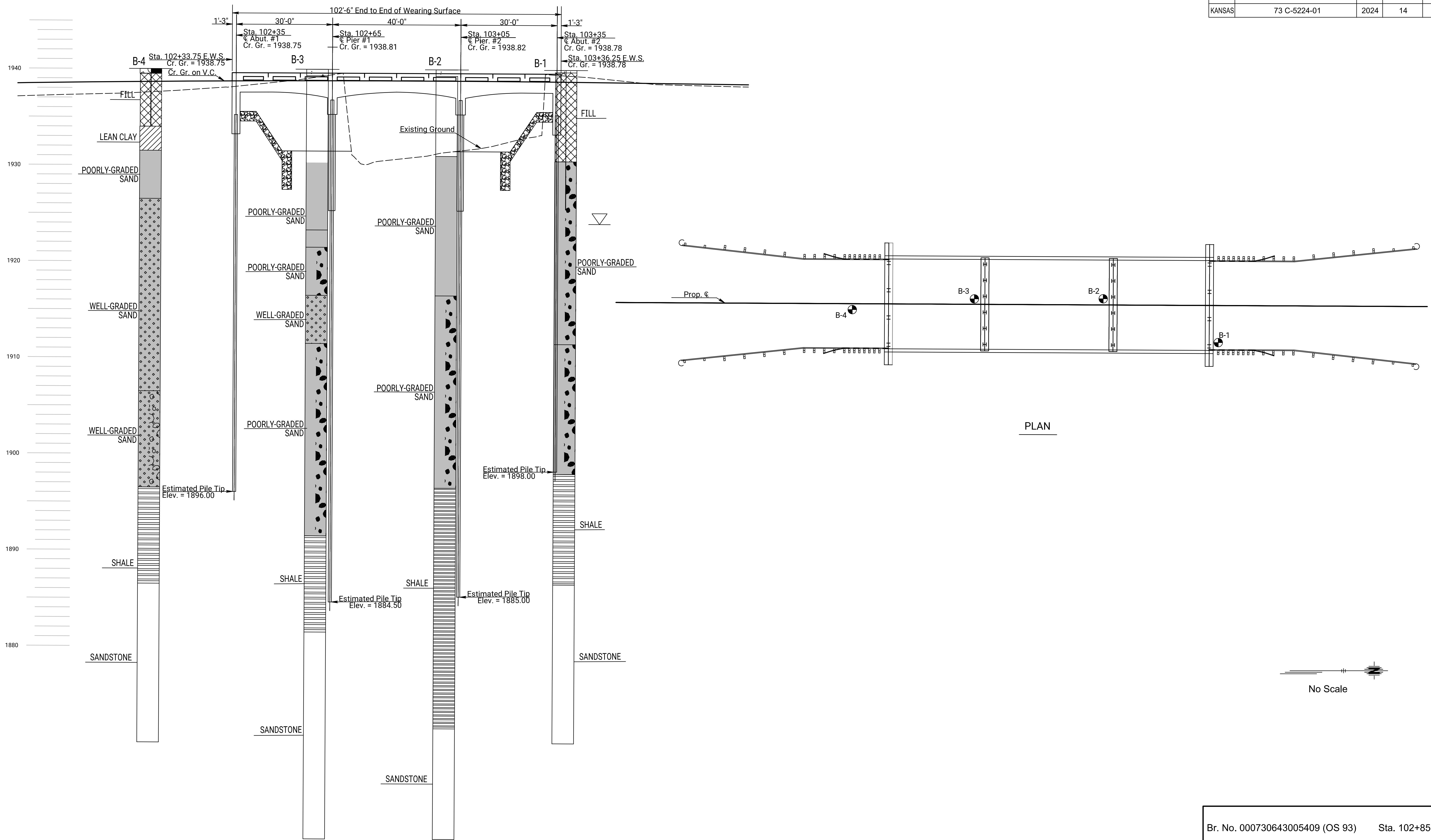
BM#3: Top of Driven "I" Post 1' W. of N-S Fc. & 288.0' N. of N. End of Ex. Bridge Elev. = 1935.69

Br. No. 000730643005409 (OS 93) Sta. 102+85.00
CONSTRUCTION LAYOUT
40TH AVE OVER ASH CREEK
Proj. 73 C-5224-01 Pawnee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED		SCALE	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.
			CADD CK.

Plotted By: rsnow
File: 2301810_bbr-02.dgn
Plot Date: 3-JUL-2024 18:06

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	14	44



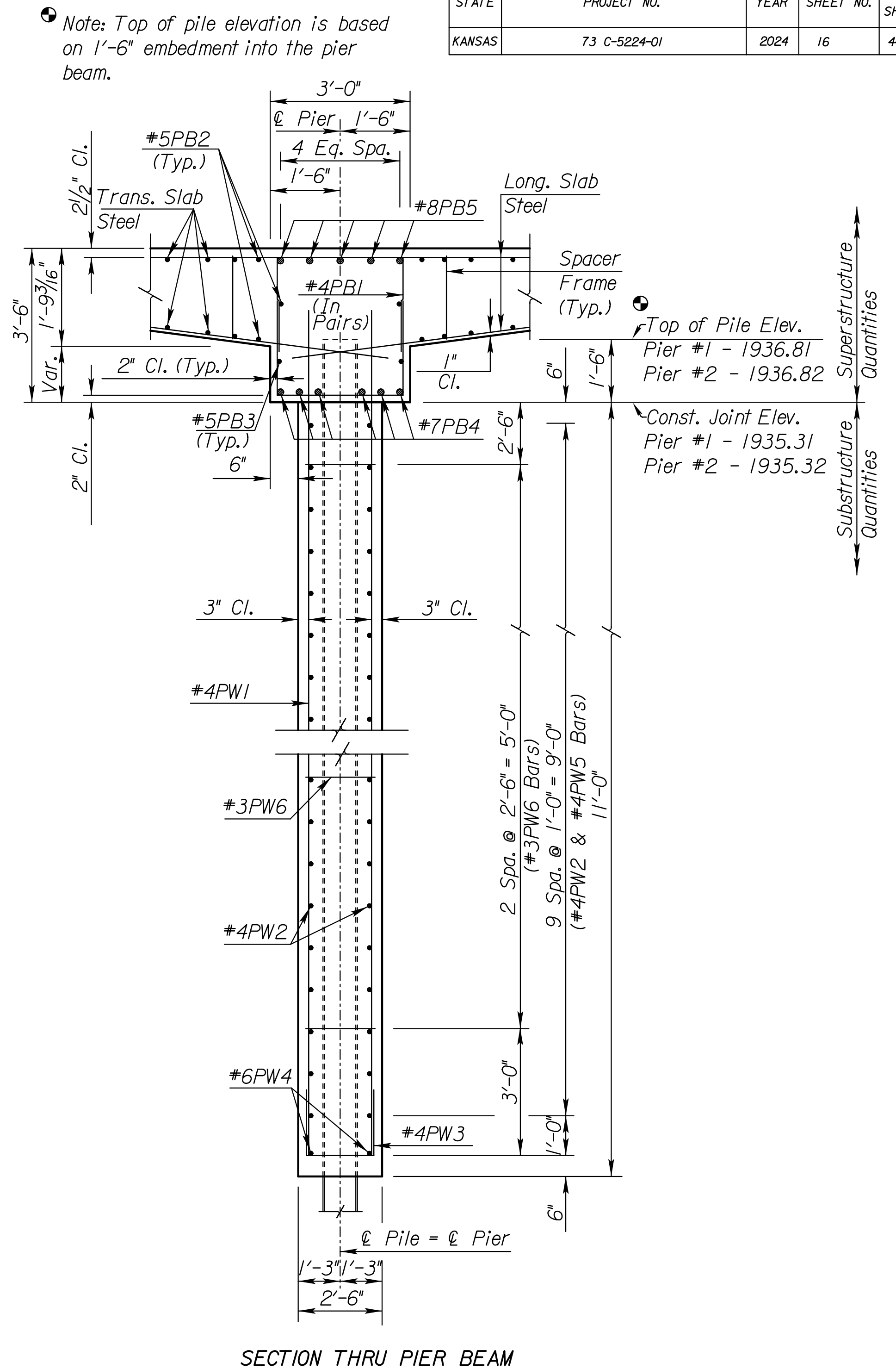
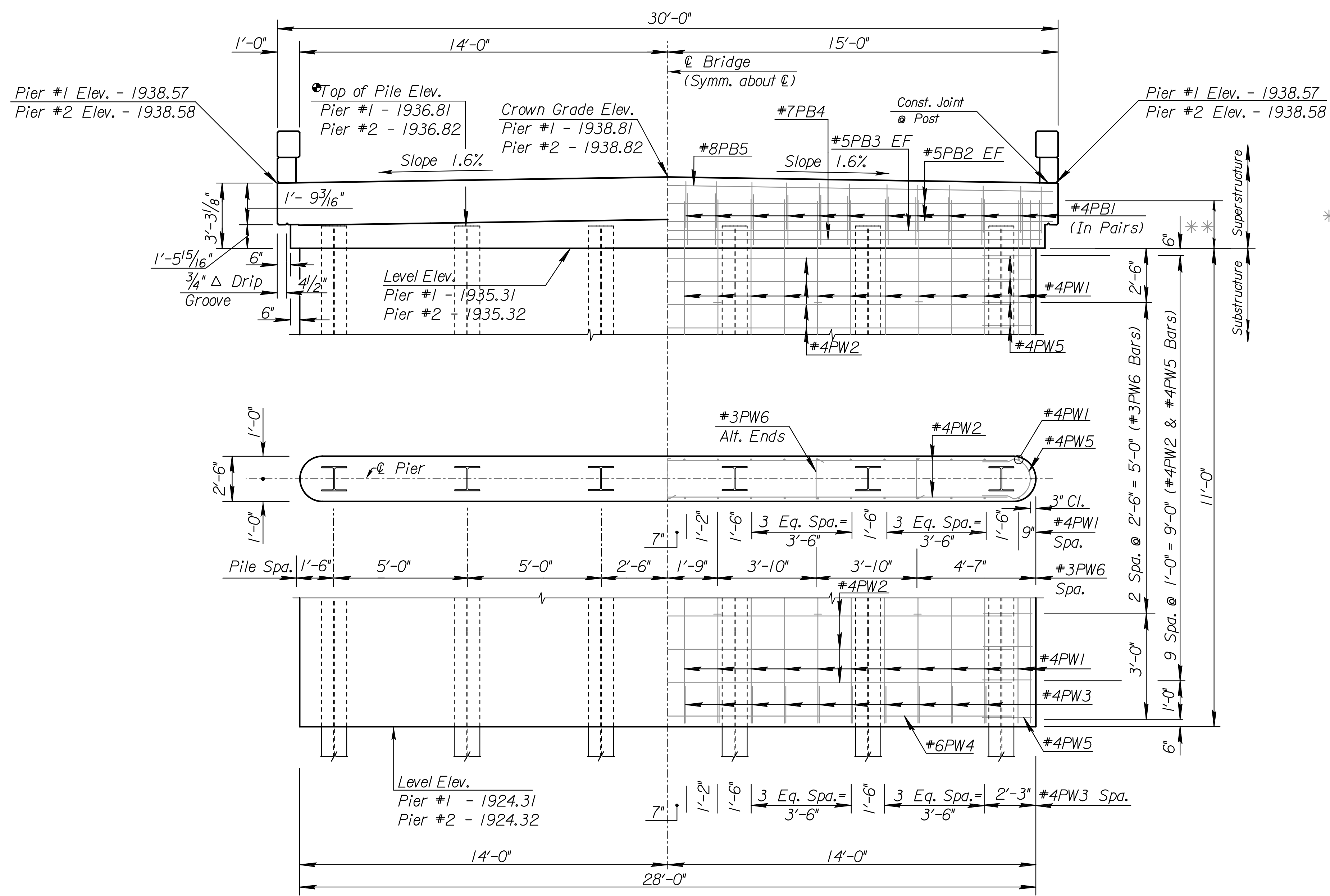
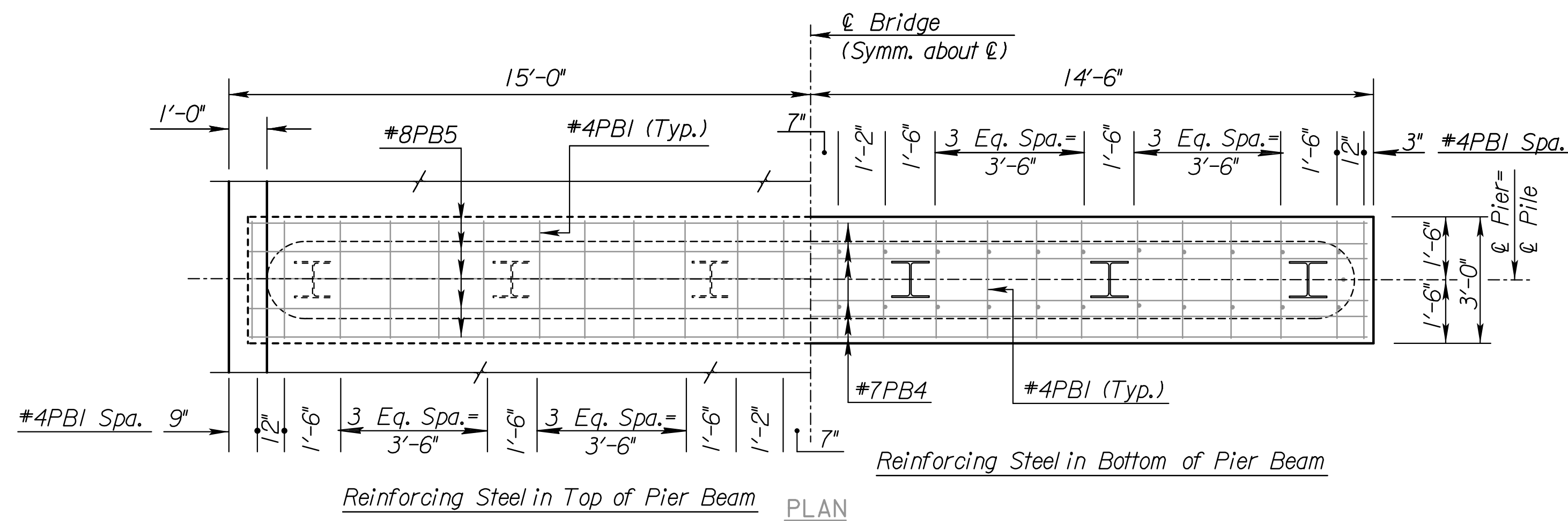
Plotted By: rsnow
File: 2301810_bbr-03.dgn
Plot Date: 3-JUL-2024 18:06

No Scale

Br. No. 000730643005409 (OS 93)		Sta. 102+85.00	
GEOLOGY SHEET			
40TH AVE OVER ASH CREEK			
Proj. 73 C-5224-01		Pawnee Co.	
SHEET NO.	OF	SCALE	APP'D
DESIGNED		DETAILED	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.
			CADD
			CADD CK.

ELEVATION

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	16	44



Note: Top of pile elevation is based on 1'-6" embedment into the pier beam.

** 2'-0" Min. #4PW1 Embedment

NO.	DATE	REVISIONS	BY	APP'D
3				
2				
1				

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 000730643005409 (OS 93) Sta. 102+85.00
PIER DETAILS
Proj. 73 C-5224-01 Pawnee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	DRT	DETAILED	DRT
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD DRT

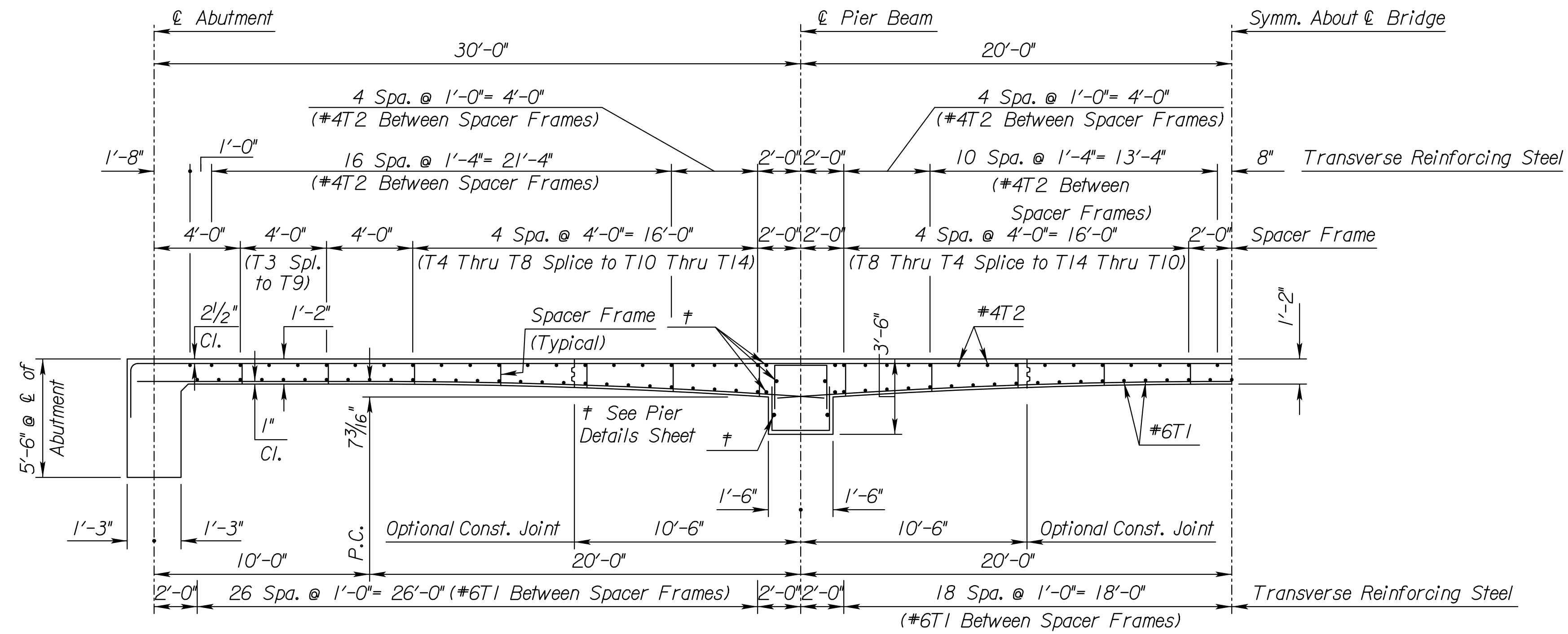
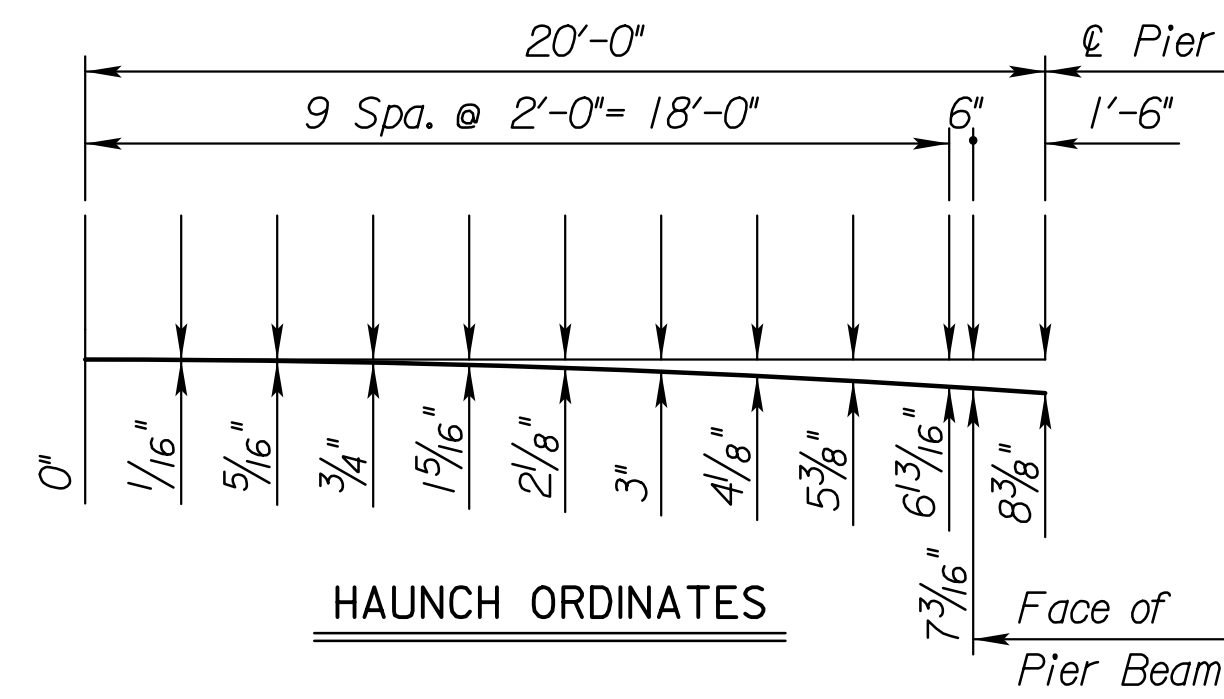
P:\23\2301818\Cadd\Plans\US_Bridges\Bridges_Details\Pier_Details.dgn
6/8/2024 4:44 AM
40R/VER

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	17	44

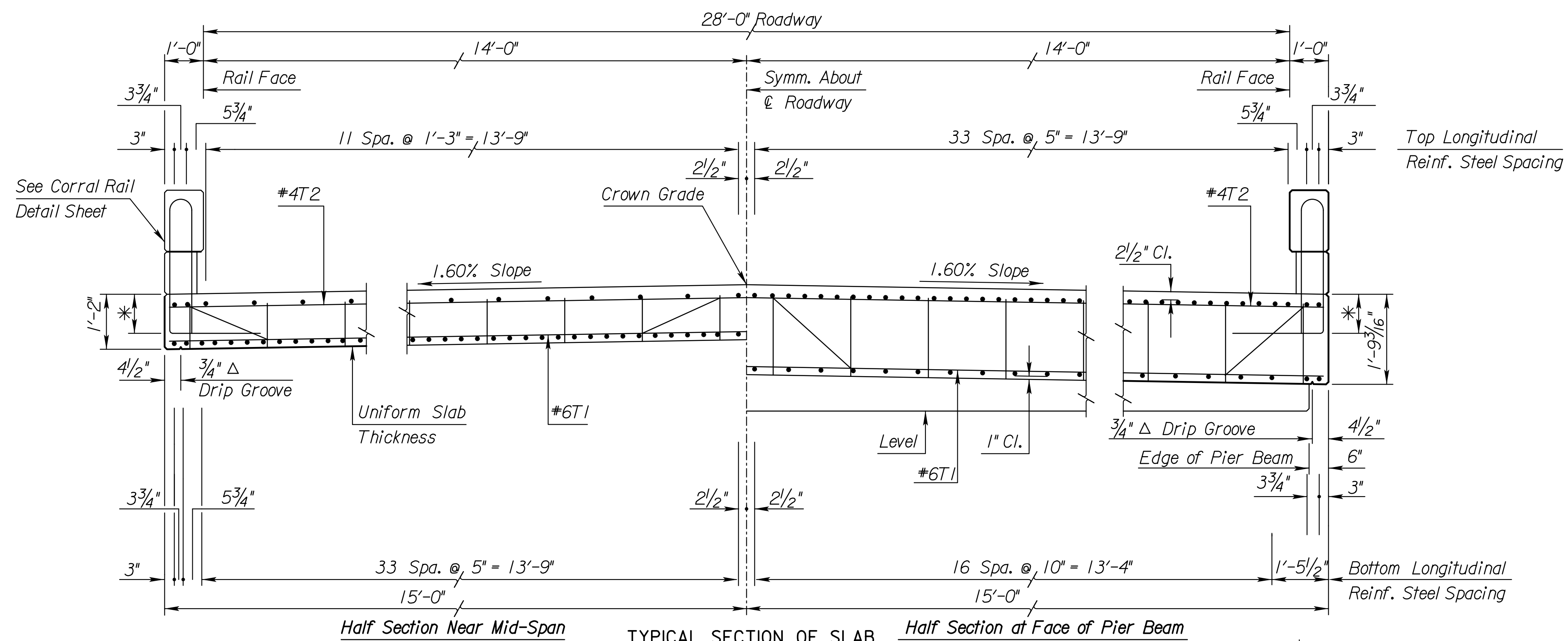
Plot 3
 Longest Span Length = 40'
 Total No. of Spans = 3
 Railing Type = Corral

Plot Location:
 Roadway Width = 28'
 Skew and Direction = 0
 Loading = HL-93

Plotted By: rsnw
 File: br511.dgn
 Plot Date: 3-JUL-2024 18:07



HALF LONGITUDINAL SECTION ALONG C BRIDGE



* See Corral Rail Detail Sheet.

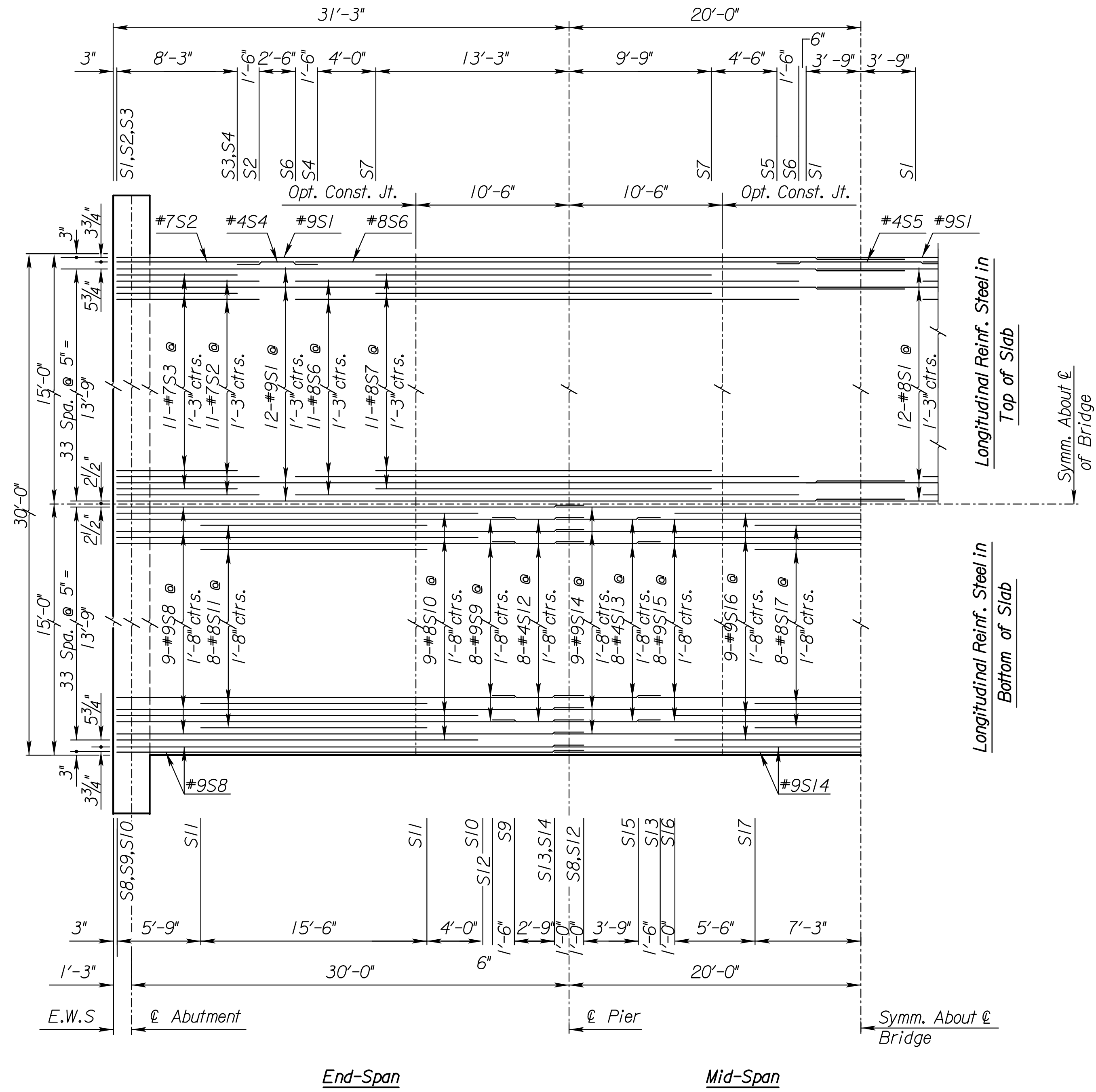
4					
3					
2					
1					
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
Br. No. 000730643005409 (OS 93) Sta. 102+85.00					
SUPERSTRUCTURE DETAILS					
Proj. 73 C-5224-01 Pawnee Co.					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	DRT	DETAILED	DRT	QUANTITIES	CADD
DESIGN CK.	MEH	DETAIL CK.		QUAN. CK.	CADD CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	18	44

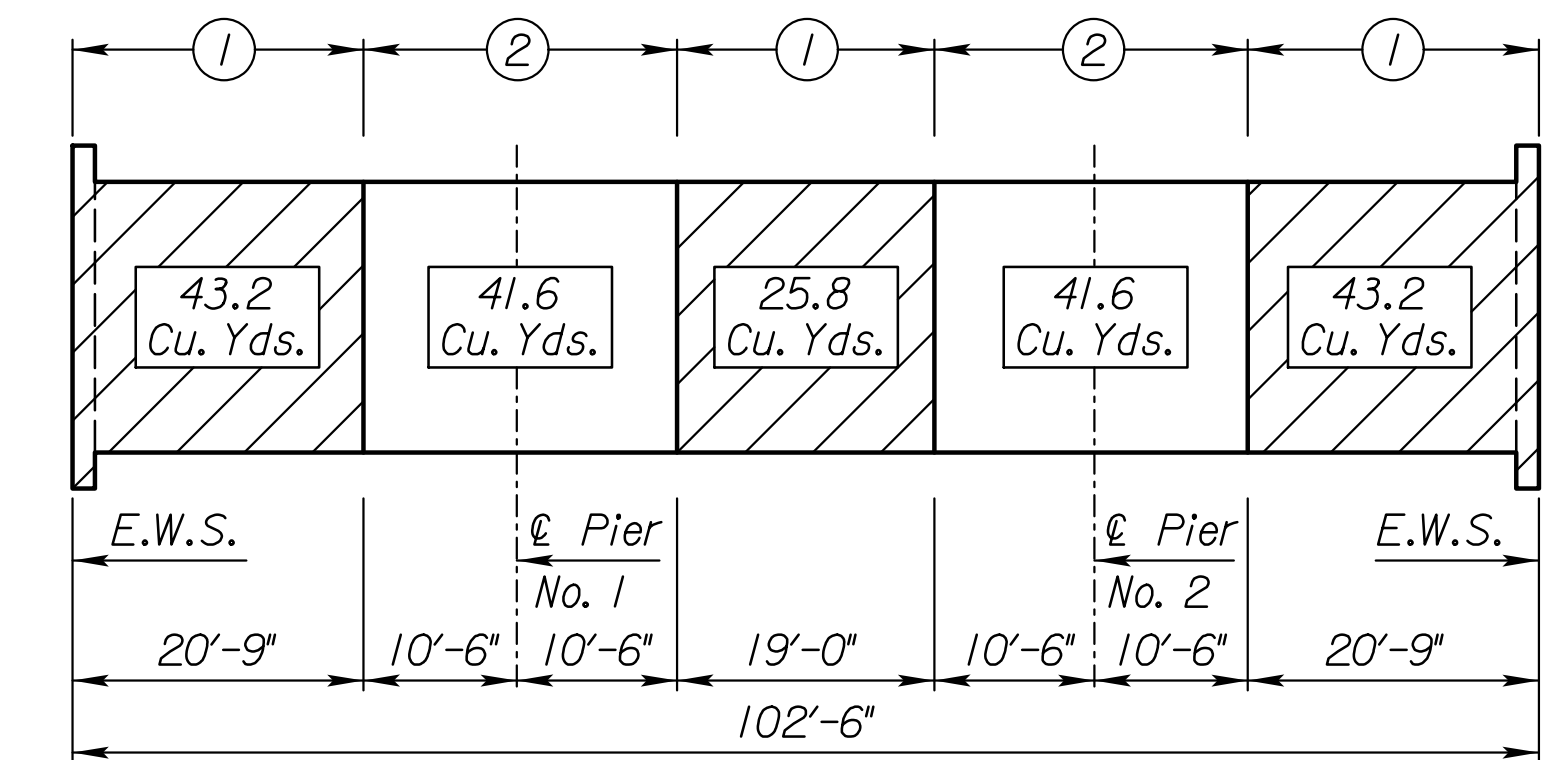
Plot 2	Longest Span Length = 40'
17F0br511.dgn	Roadway Width = 28'
	Skew and Direction = 0
	Loading = HL-93
	Rating Type = Corral

LFD & LRFR RATING FACTOR		3'	
Truck	2 1/2'	Ln.	Oper.
		1.84	3.07
LFD	HT	1.45	1.47
LRFR	HL-93	1.40	1.46
		1.82	1.90

Note to designer: Do not remove this information



Note:
See longitudinal section for transverse reinforcing steel.



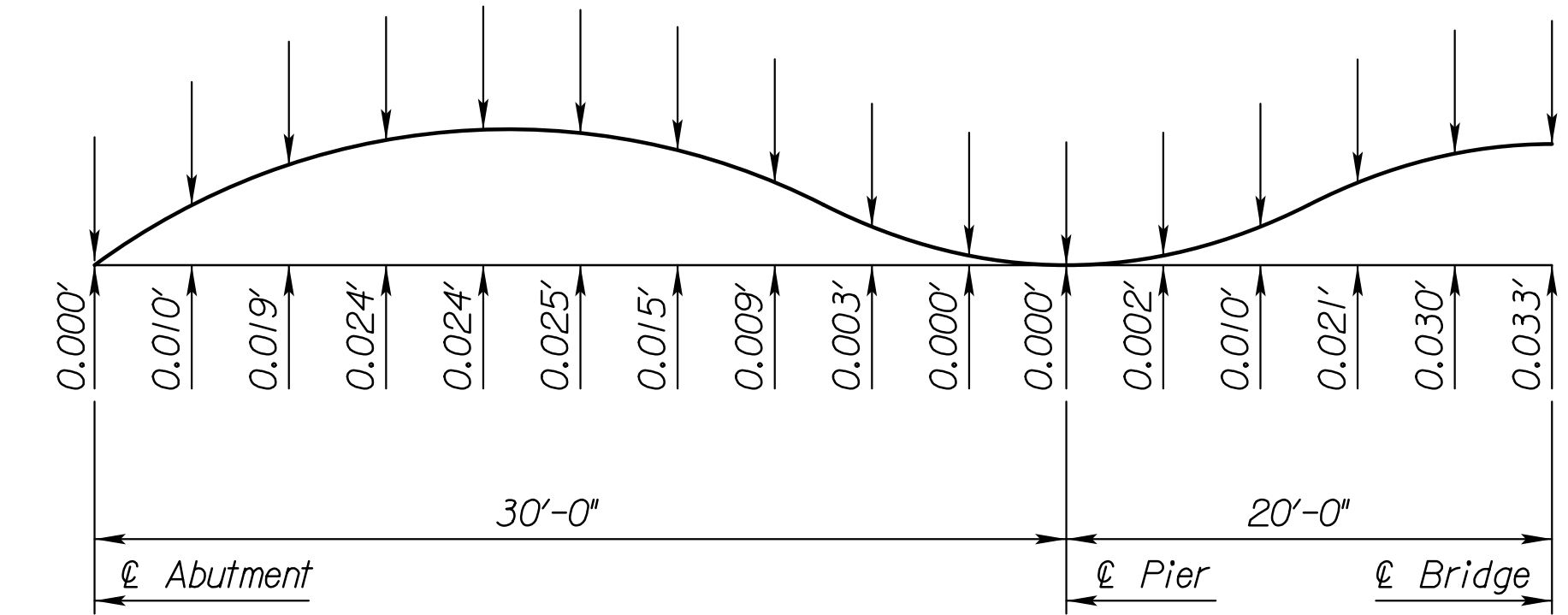
CONCRETE PLACING SEQUENCE DIAGRAM

CONCRETE PLACING SEQUENCE

When long span steel beams having a concrete dead load deflection greater than 1/4" are used or when timber falsework with greater than 12'-0" clear span is used, follow the placing sequence shown. Segmental, combined or continuous pours are allowed, but stop a discontinuous pour at a construction joint short of a pier.

When timber falsework with 12'-0" or less clear span is used, the Contractor, subject to the approval of the Engineer, may use a continuous pour or may discontinue the pour at any construction joint shown.

The Contractor may place the corral rail continuously from one end of the bridge to the other.



DEAD LOAD CAMBER DIAGRAM AT TENTH POINTS

Long Term Deflections = Initial Deflections x 3.5
(Initial Deflections Based on E = 3,644 x 10⁶ p.s.i.)
(camber values in feet)

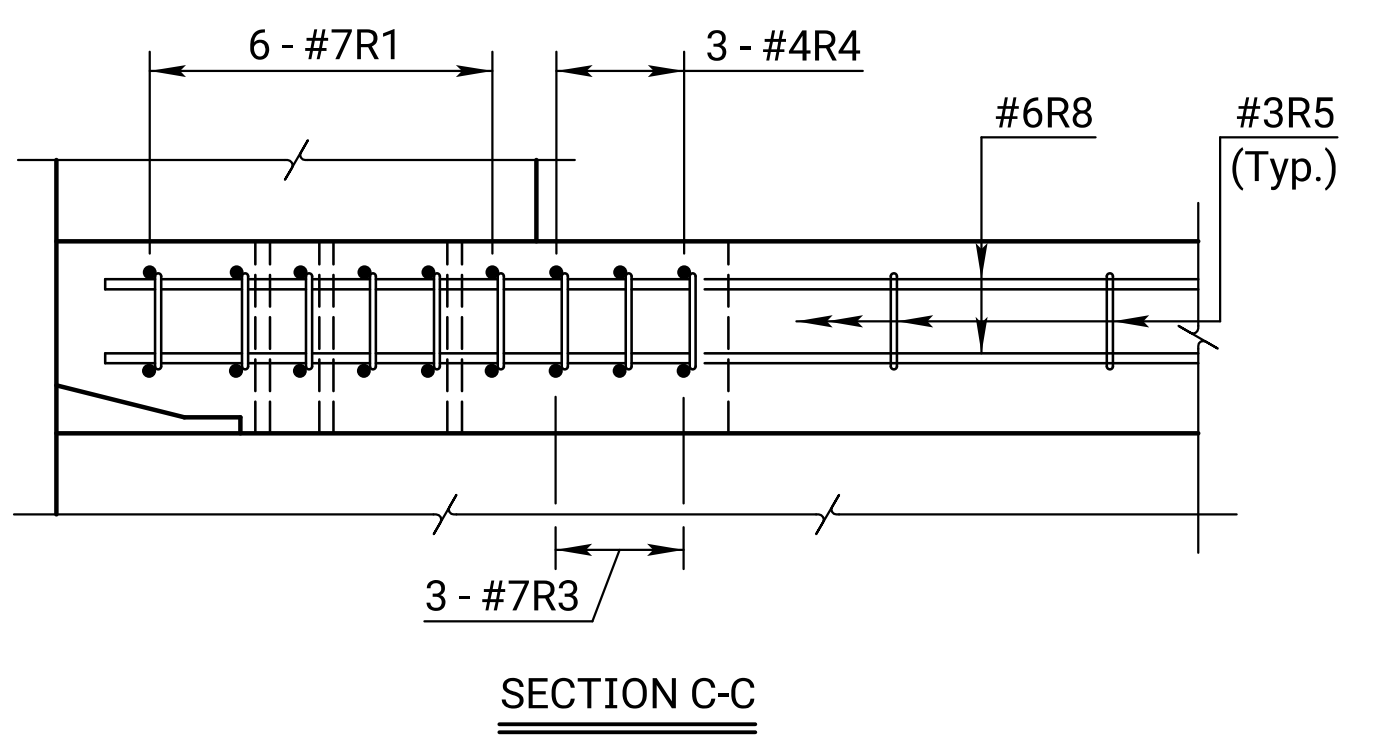
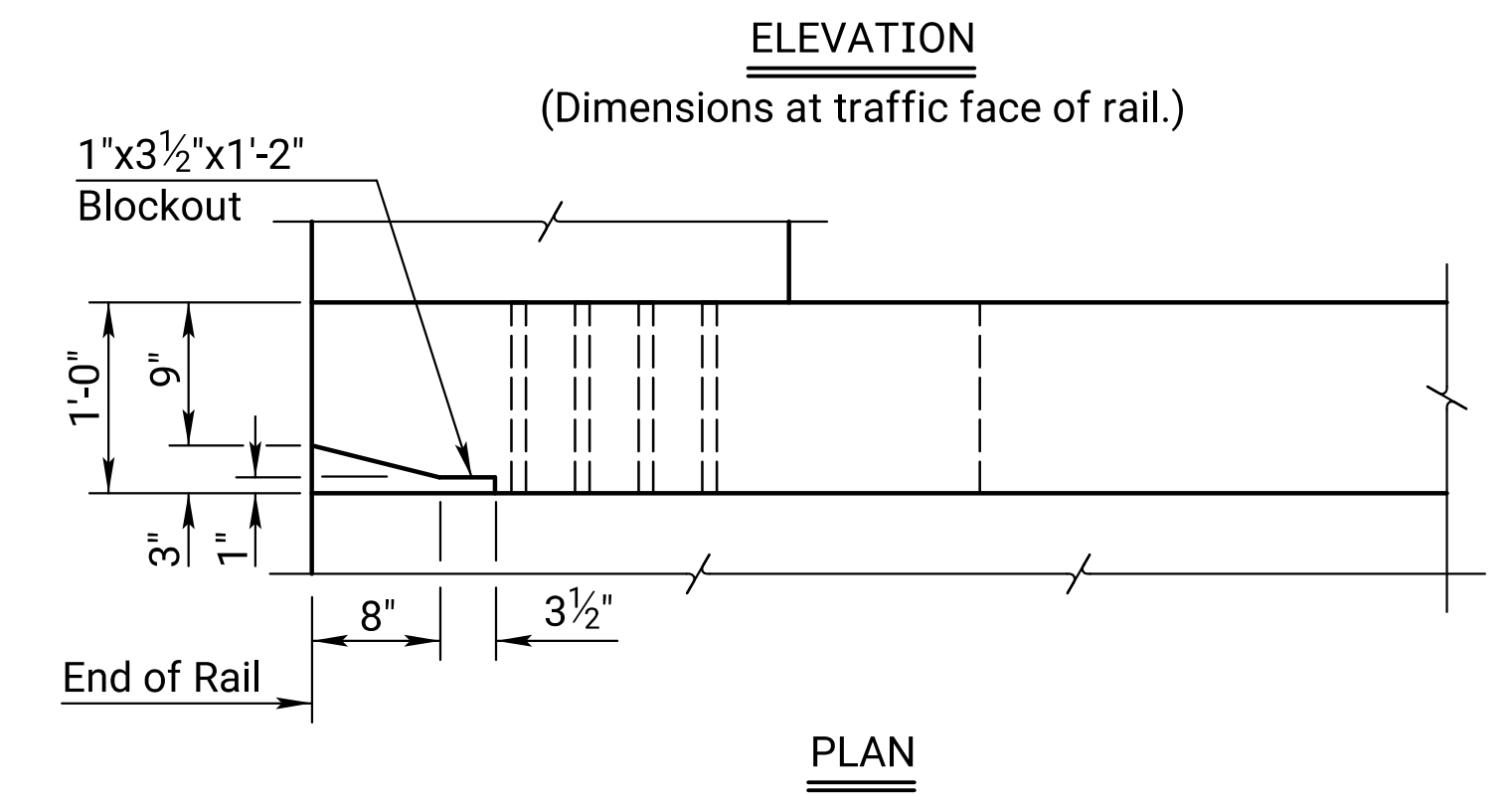
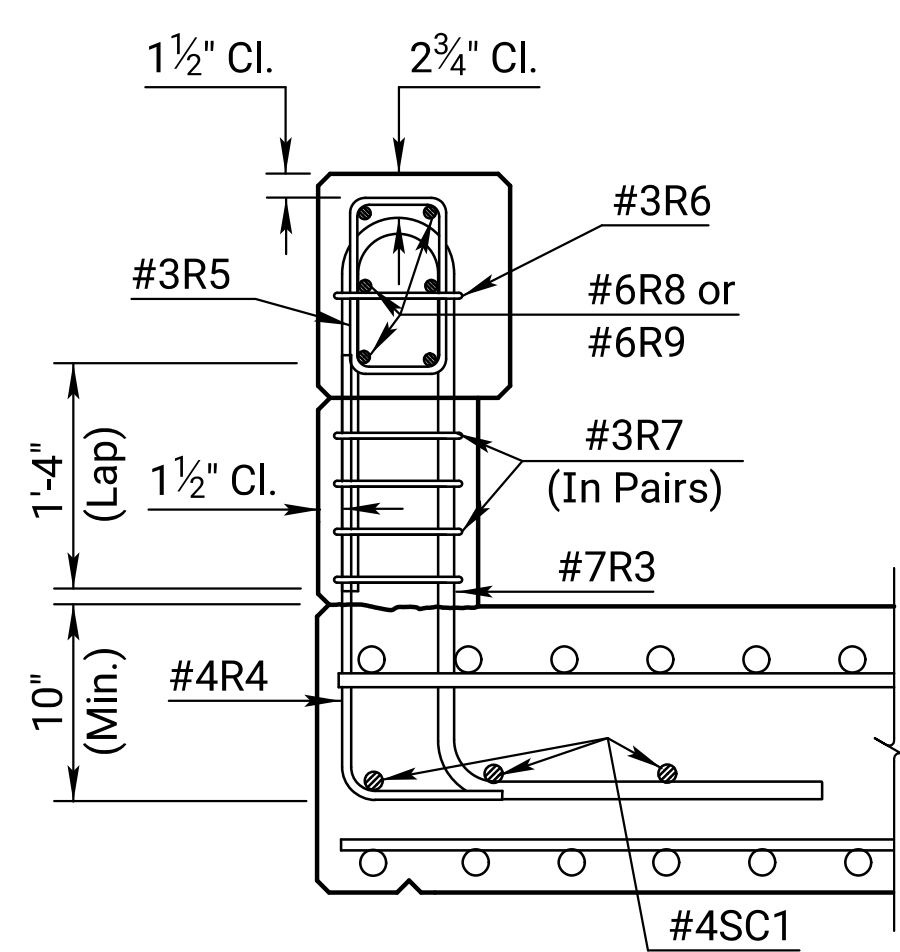
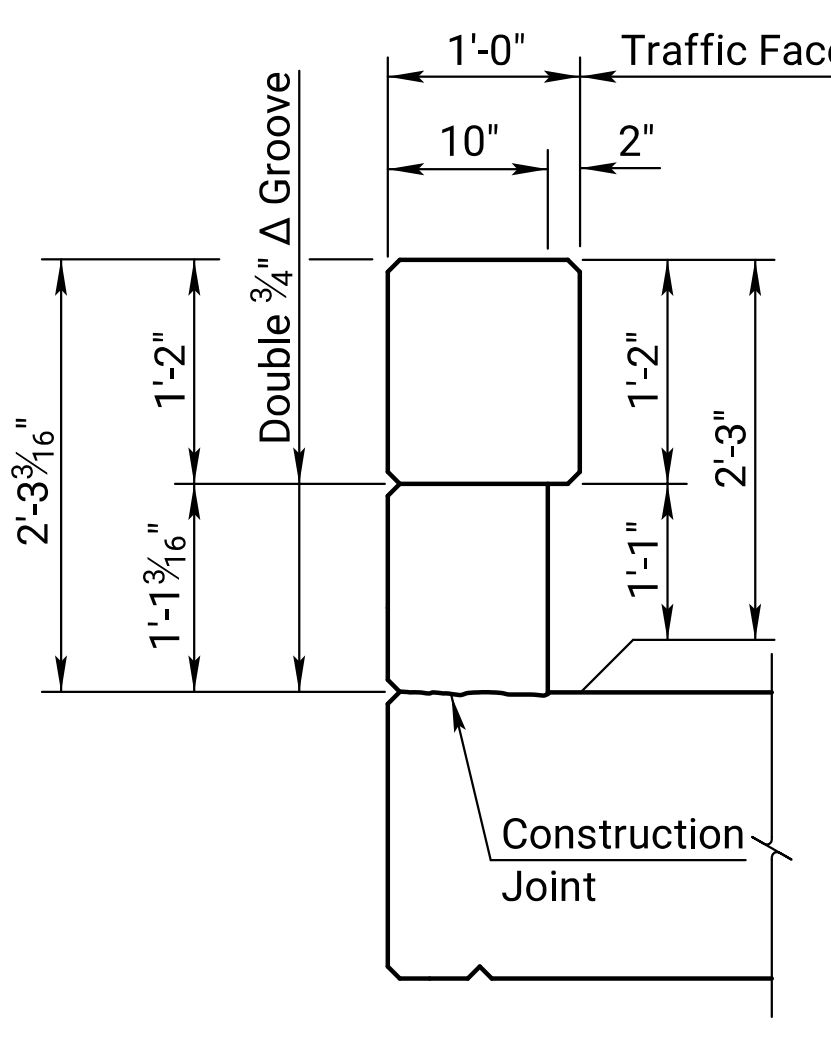
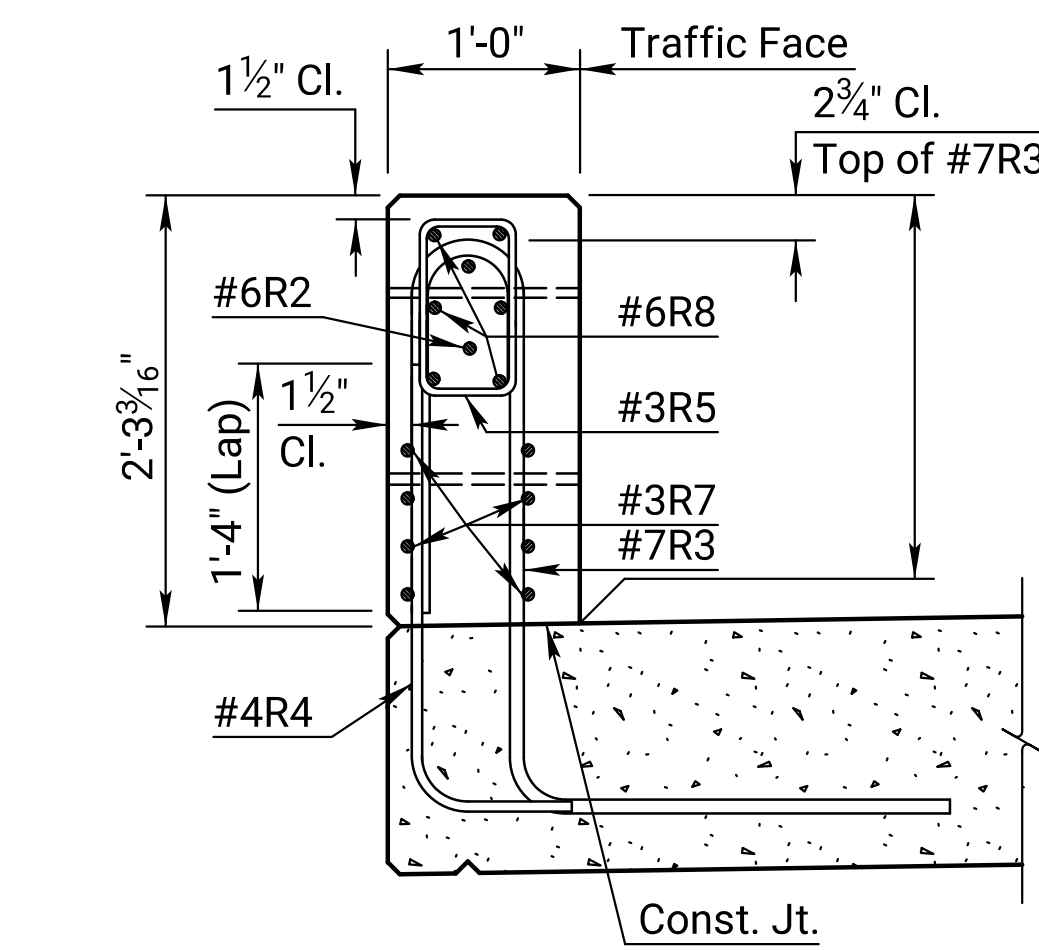
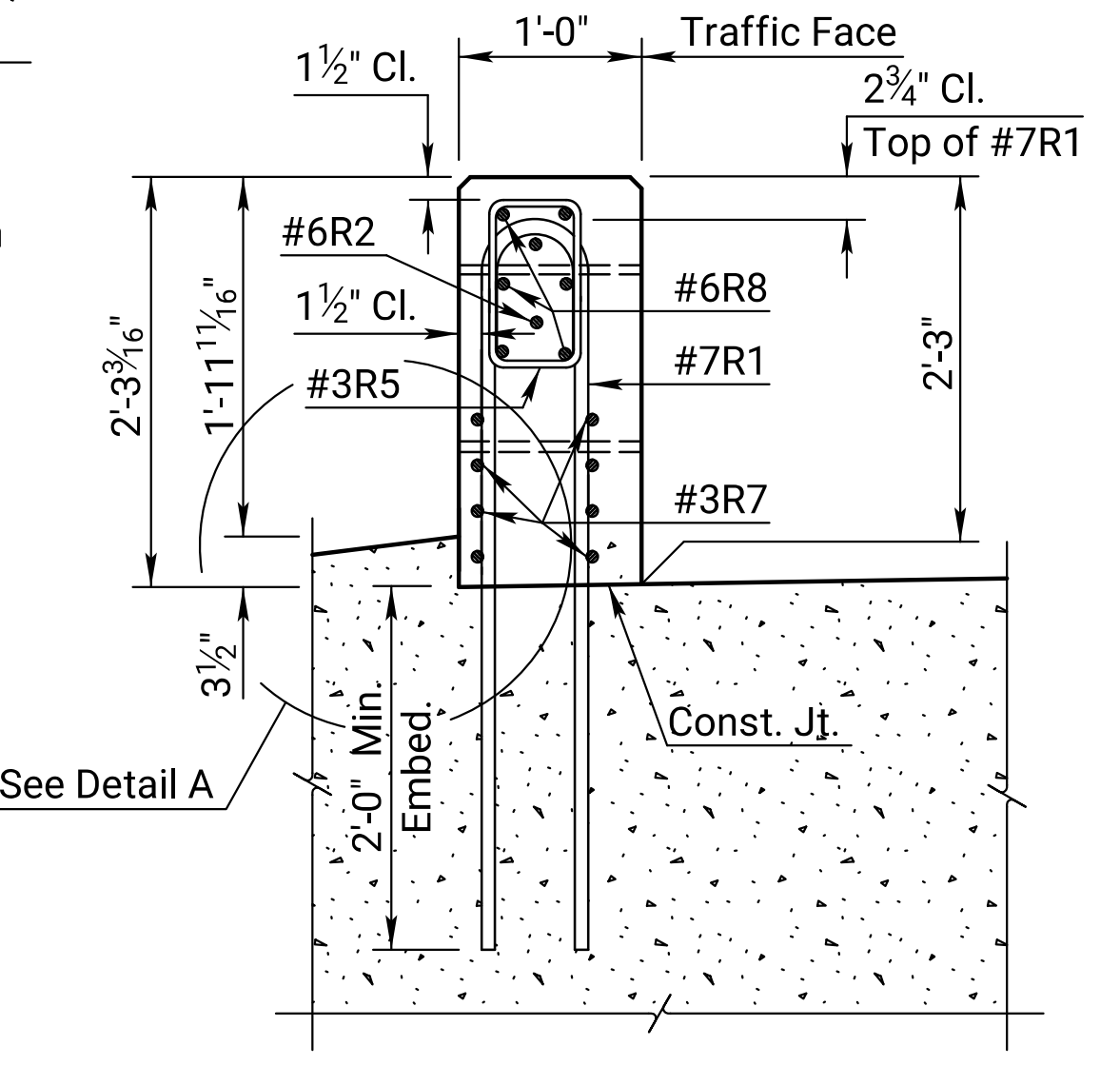
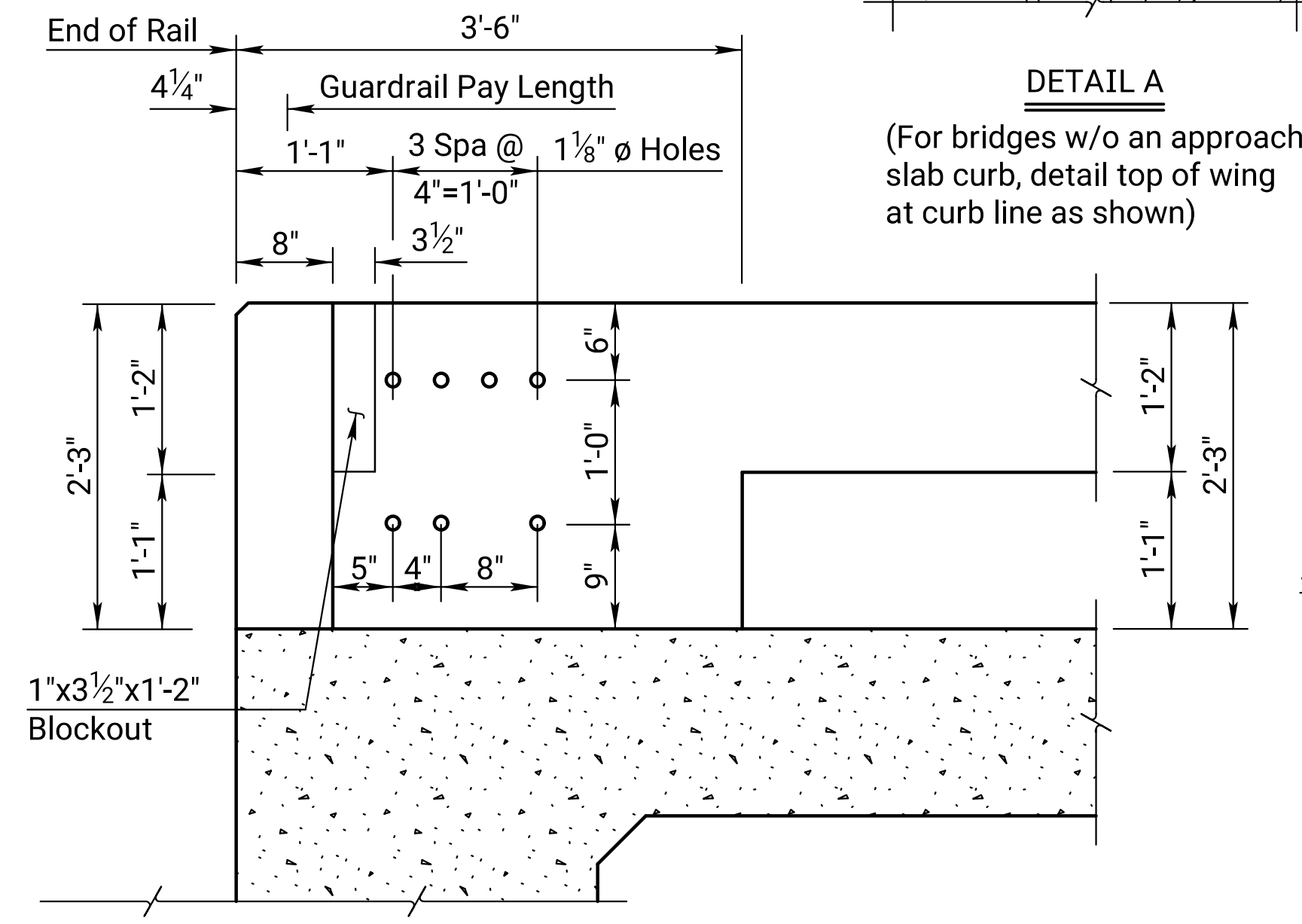
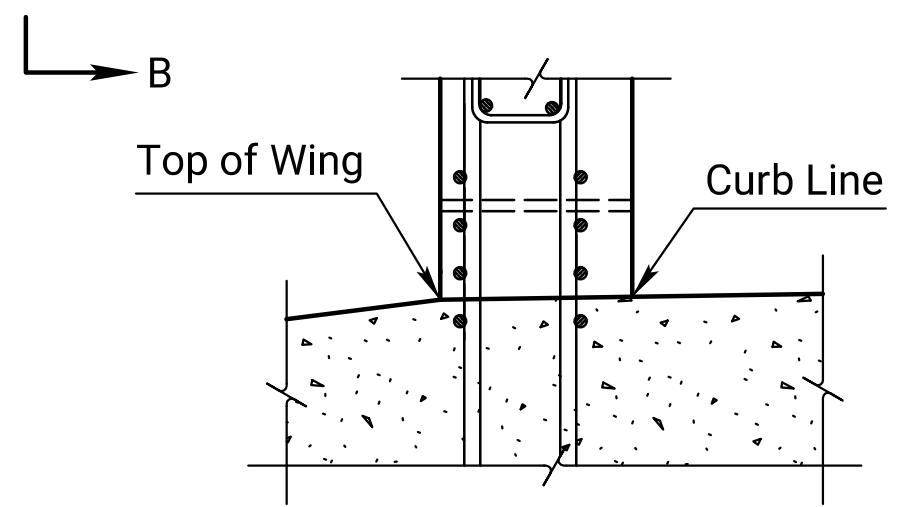
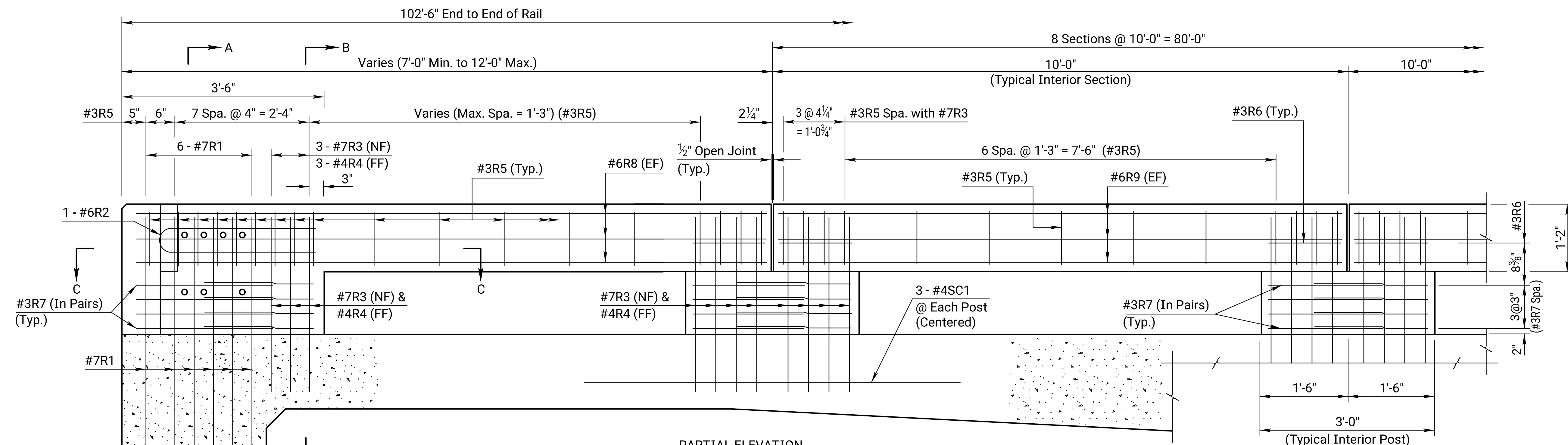
Plotted By: rsnow
File: br511.dgn
Plot Date: 3-JUL-2024 18:07

4	08/22/23	Summary of Quantities corrections	MLL	MAH
3	03/12/12	ADDED TO Elevation Table	JPJ	TLF
2	02/08/11	ADDED QUANTITIES	JPJ	TLF
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION
Br. No. 000730643005409 (OS 93) Sta. 102+85.00
SUPERSTRUCTURE DETAILS
Proj. 73 C-5224-01 Pawnee Co.

SHEET NO.	OF	SCALE	APP'D
DESIGNED	DRT	DETAILED	DRT
DESIGN CK.	MEH	DETAIL CK.	
QUANTITIES	BRW	CADD	RCJ
QUAN. CK.		CADD CK.	

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	19	44



LEGEND
 NF = Near Face
 FF = Far Face
 EF = Each Face

Plotted by: rsnw 3-JUL-2024 18:07
 File: br183a.dgn

03	12-03-21	Changed Bridge Number Plate detail	M.L.L.	M.A.H.
02	06-30-05	Current Release		
01				
NO.	DATE	REVISIONS	BY	APPD

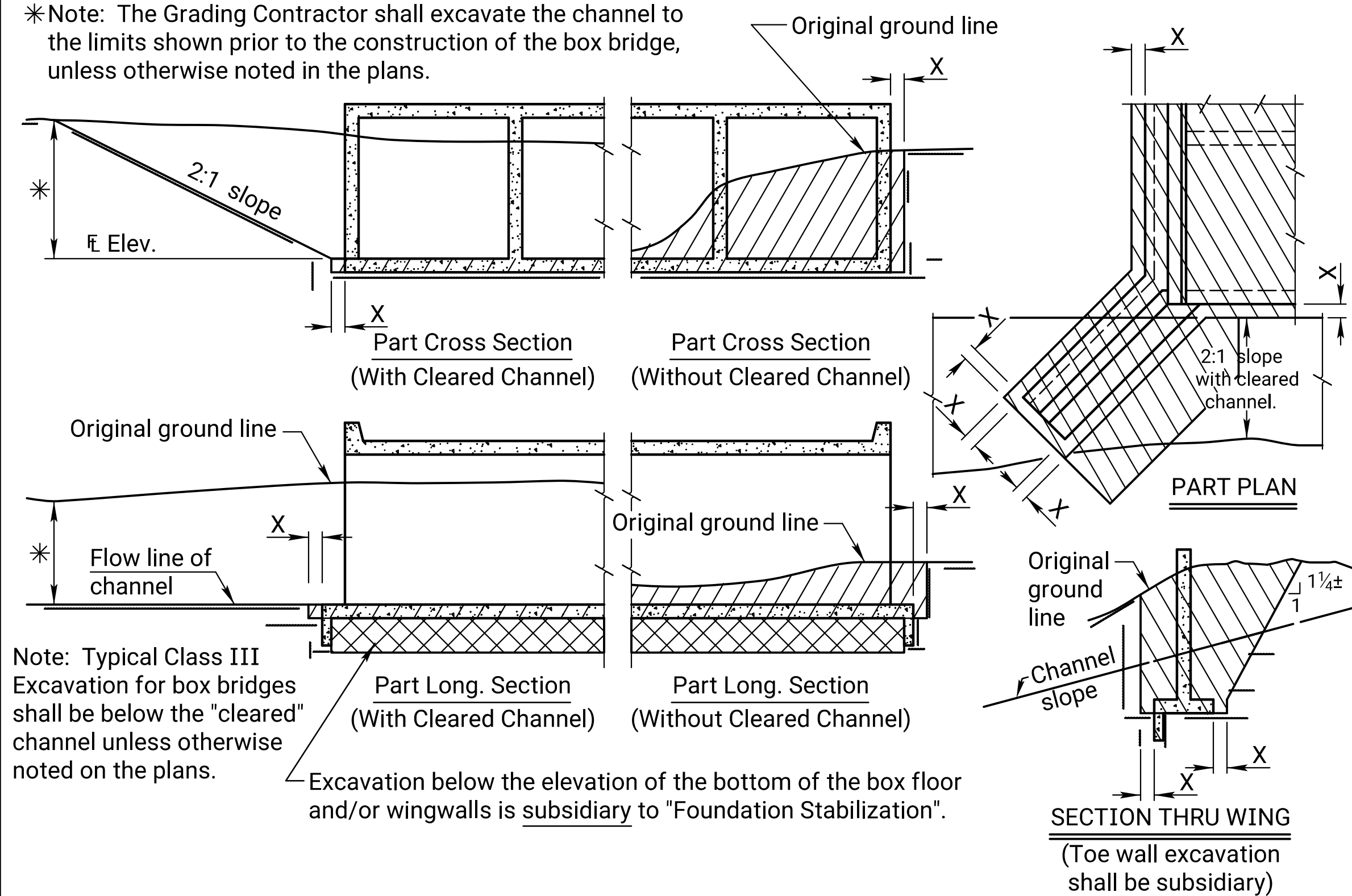
KANSAS DEPARTMENT OF TRANSPORTATION
 Br. No. 000730643005409 (OS 93) Sta. 102+85.00

27" KANSAS CORRAL RAIL
 (W-BEAM WITH RUBRAIL)
 R.C. HAUNCHED SLAB (Without Curb)
 Proj. 73 C-5224-01 Pawnee Co.

DESIGNED	DETAILED	QUANTITIES	CADD
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	21	44

* Note: The Grading Contractor shall excavate the channel to the limits shown prior to the construction of the box bridge, unless otherwise noted in the plans.

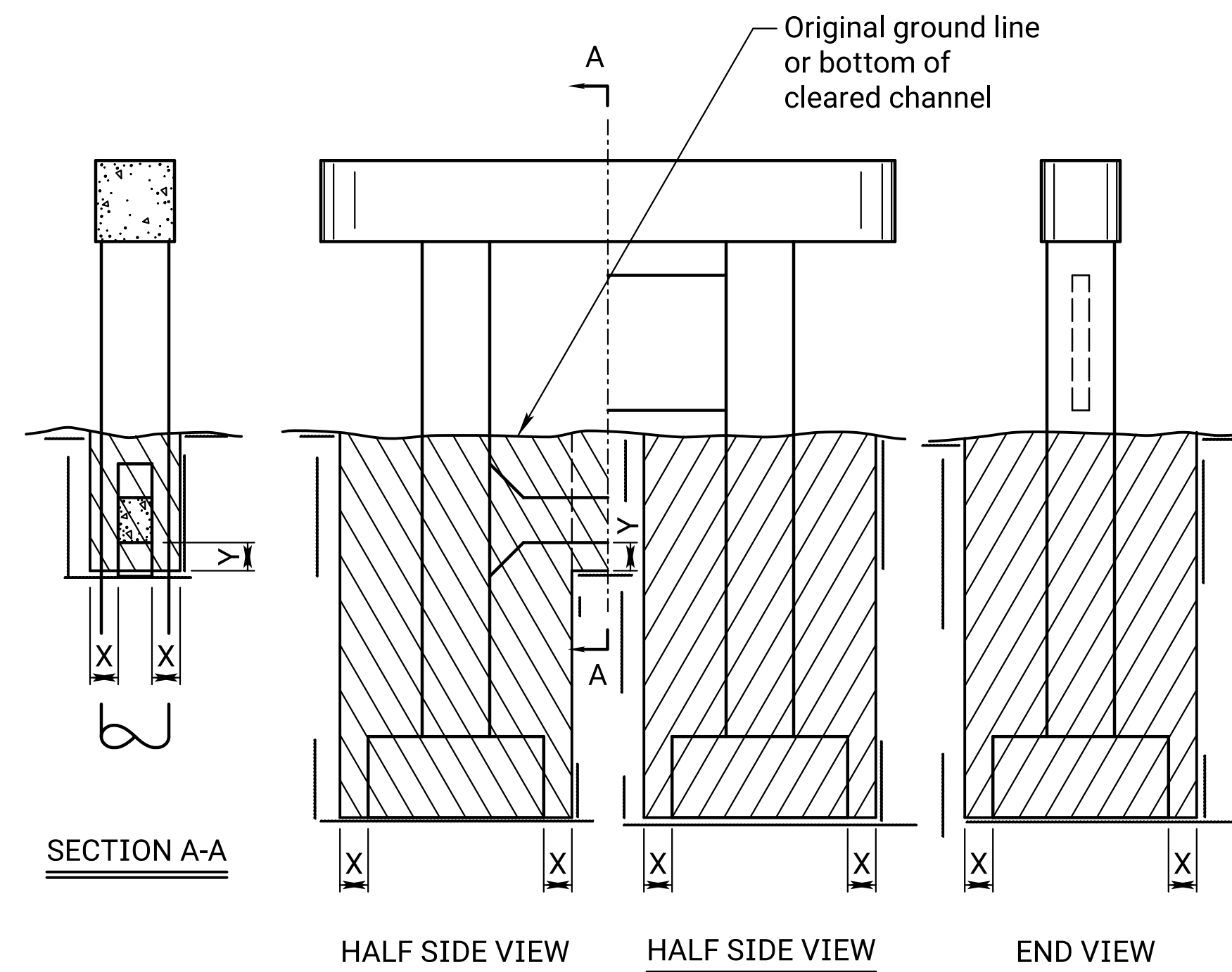


Note: Typical Class III Excavation for box bridges shall be below the "cleared" channel unless otherwise noted on the plans.

Excavation below the elevation of the bottom of the box floor and/or wingwalls is subsidiary to "Foundation Stabilization".

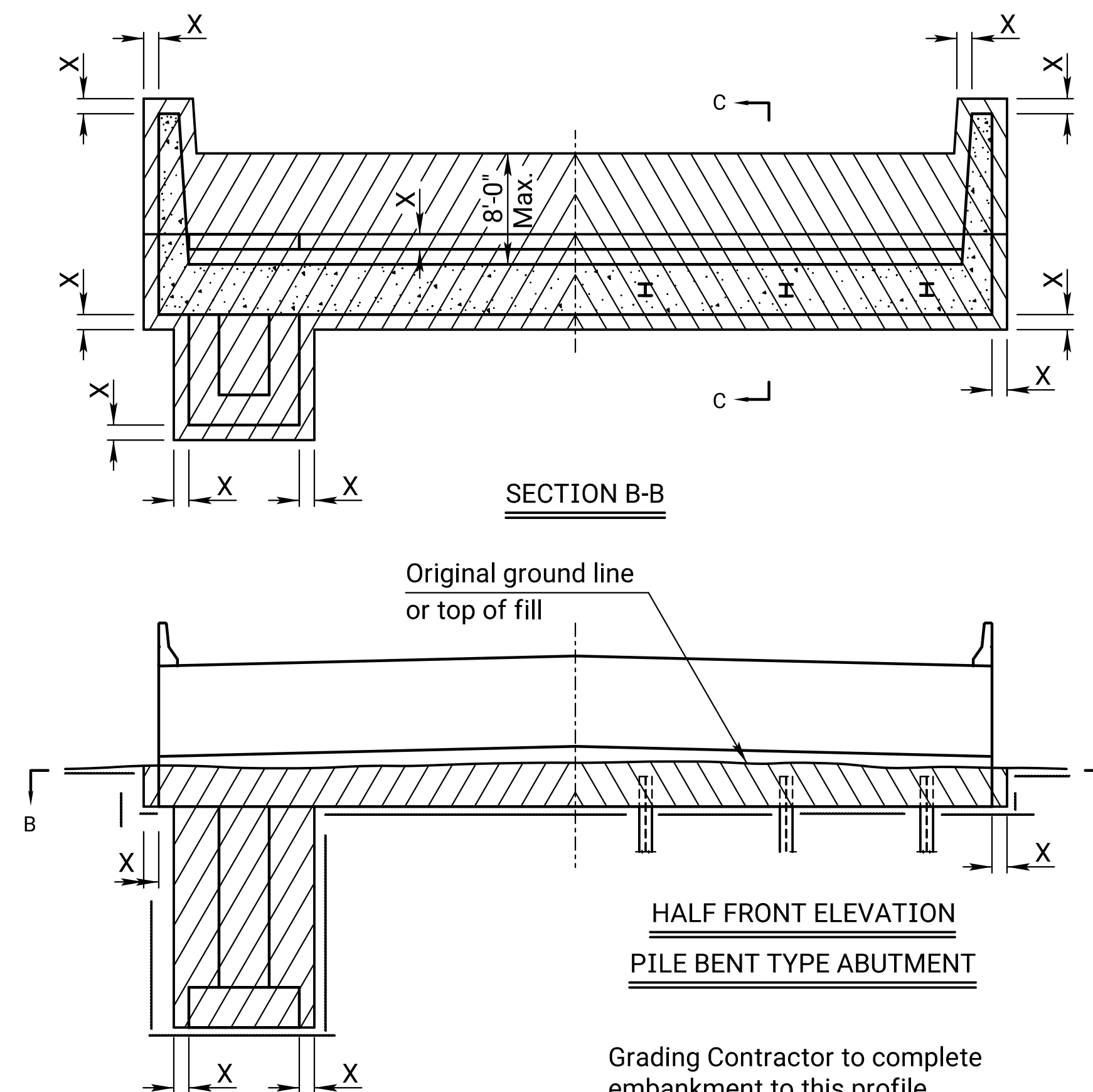
EXCAVATION DETAILS FOR REINFORCED CONCRETE BOX CULVERT

Note: Excavation for culverts less than bridge length and the additional excavation for "Embedded Structures" shall not be paid for as Class III Excavation, but shall be subsidiary to Grade 4.0 Concrete.



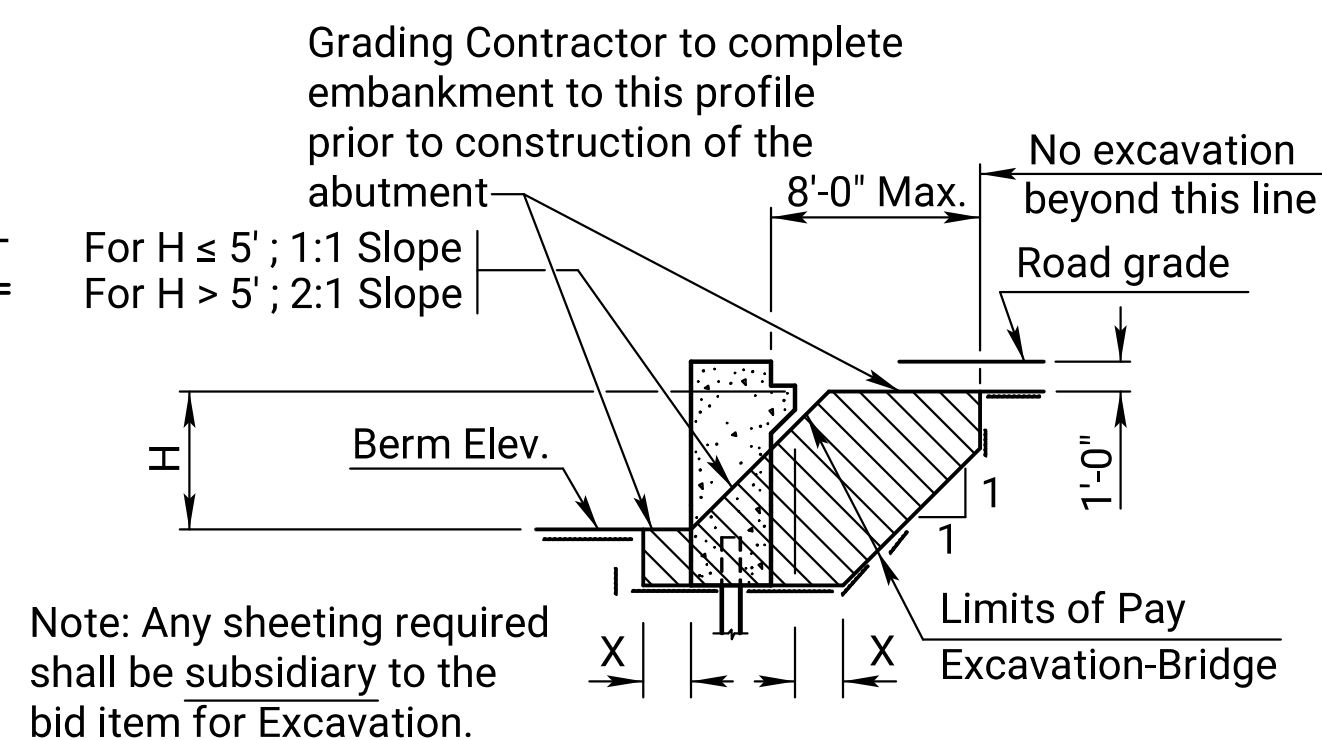
EXCAVATION DETAILS FOR TYPICAL PIERS

See detail when rock or shale (rock) is encountered. ☉



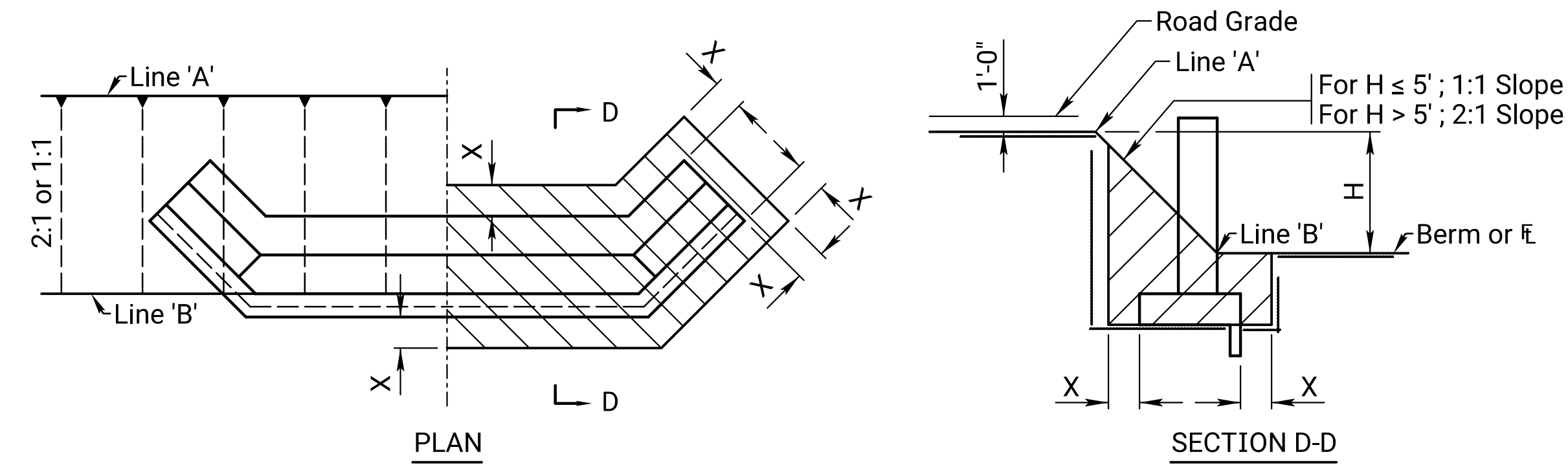
EXCAVATION DETAILS FOR TYPICAL ABUTMENTS

Note: Bridge Contractor shall finish the embankment and berms after the construction of the abutment and dispose of any excess material as approved by the Engineer.



EXCAVATION DETAILS FOR TYPICAL ABUTMENTS

See detail when rock or shale (rock) is encountered. ☉



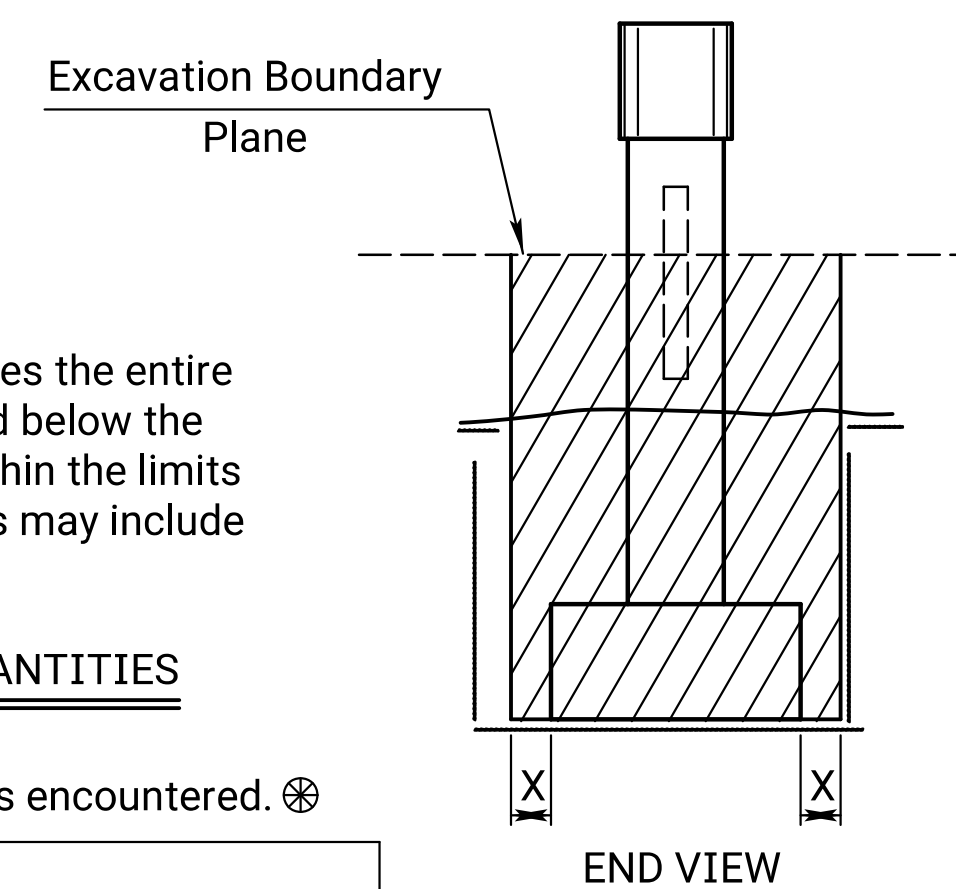
EXCAVATION DETAILS FOR ABUTMENTS WITH FLARED WINGWALLS

(Toe wall excavation shall be subsidiary)

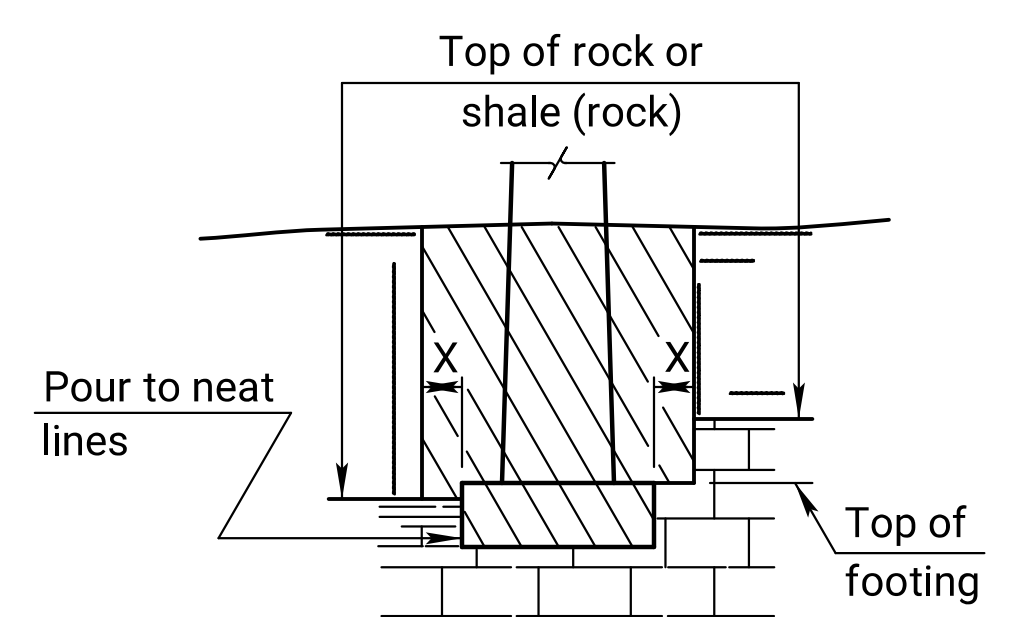
Note: Class II Excavation includes the entire volume of whatever nature found below the "Excavation Boundary Plane", within the limits specified for measurement. This may include water or air.

CLASS II EXCAVATION QUANTITIES

See detail when rock or shale (rock) is encountered. ☉



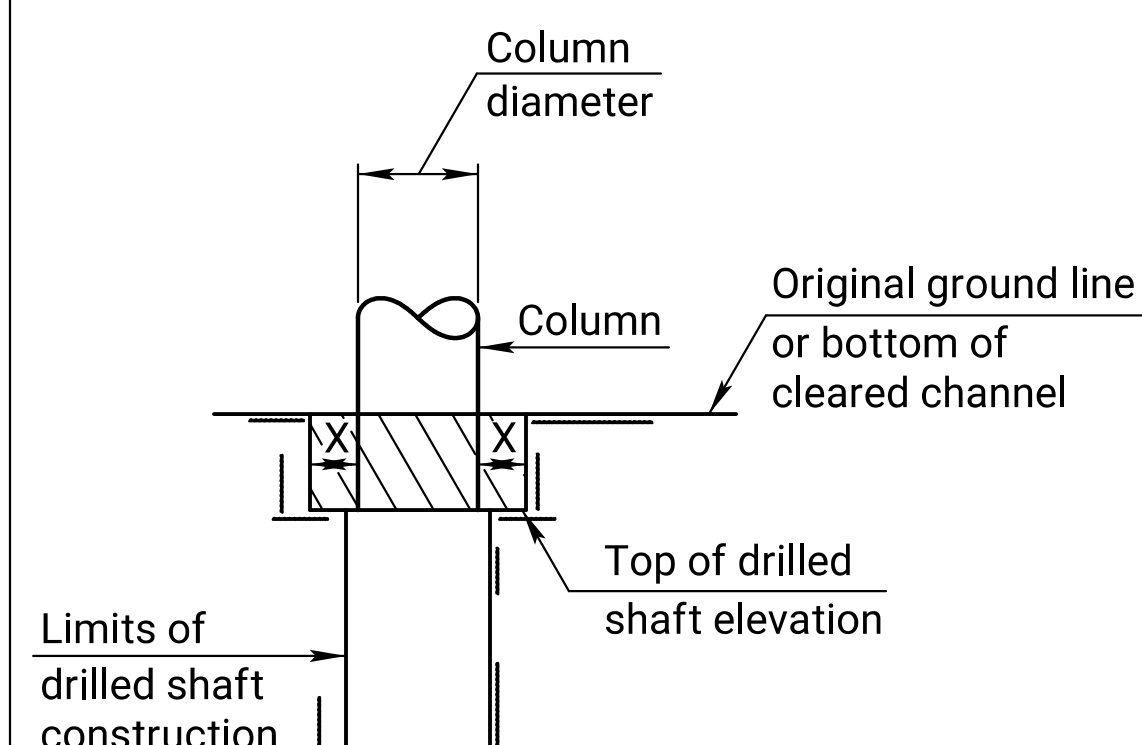
END VIEW



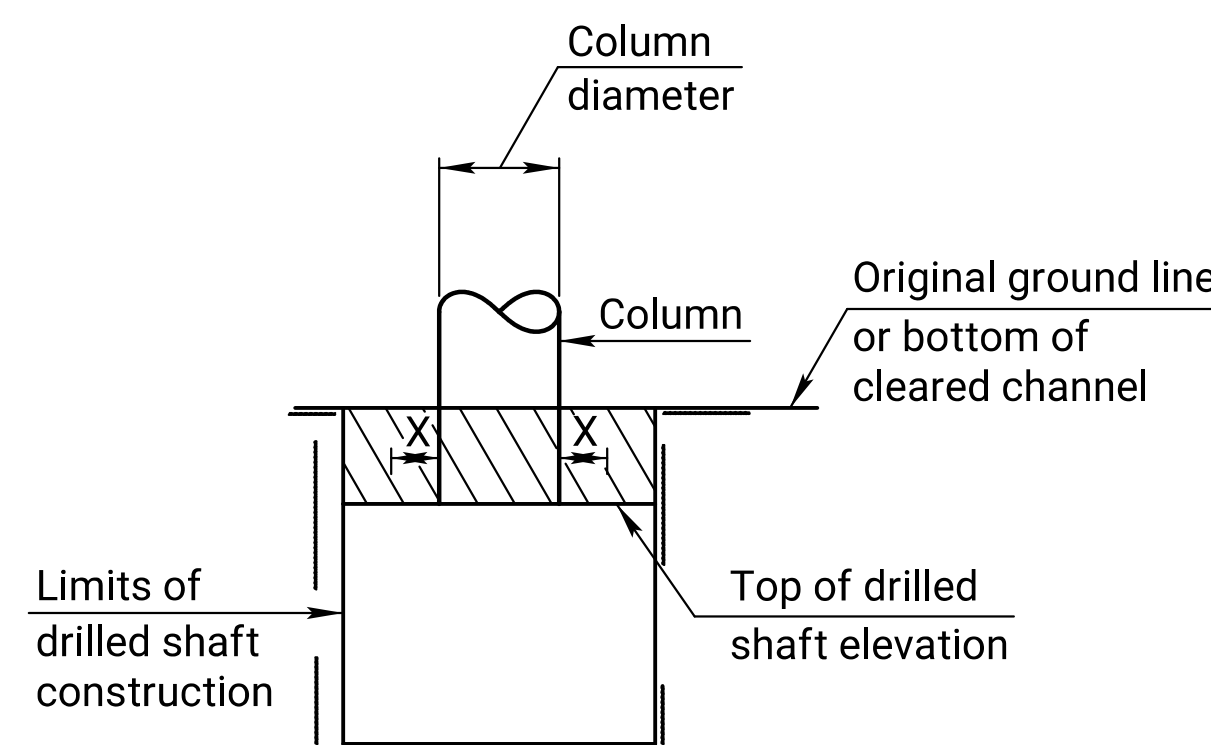
EXCAVATION DETAIL FOR FOOTINGS IN ROCK OR SHALE (ROCK)

(Piers and Abutments)

Note: Excavation below top of rock, hard shale or below top of footing, whichever is lower, shall be to neat lines of the concrete construction.



DETAIL A



DETAIL B

DRILLED SHAFT DETAILS

Note: Whenever the limits of the drilled shaft construction are greater than the Column Diameter + 2X, the limits of Class I, II or III Excavation shall be the limits of the drilled shaft construction. (See Detail B)

Note: All bridge excavation shall be computed on the basis of the cross-hatch areas and boundary lines indicated on this sheet and the Excavation Boundary Plane on the Construction Layout.

Sides of trenches in hard or compacted soil including embankments shall be shored, sheeted, braced or otherwise supported when the trench is more than 5 feet in depth and 8 feet or more in length. In lieu of the shoring, the sides of the trench above the 5 foot level may be sloped to preclude collapse. The slope for average soils shall be 1:1. If the angle of repose of the soil is less, flatter slopes shall be required.

Dimension "X" shall be 2'-0" unless indicated otherwise on the general plans.

Dimension "Y" shall be 1'-6" unless indicated otherwise on the general plans.

06	08-15-12	Embedment Excavation Subsidiary	J.P.J.	T.L.F.
05	05-15-12	Revised Wing Excavation	J.P.J.	T.L.F.
04	03-03-10	Revised Wing Excavation	J.P.J.	T.L.F.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

BRIDGE EXCAVATION (LRFD)

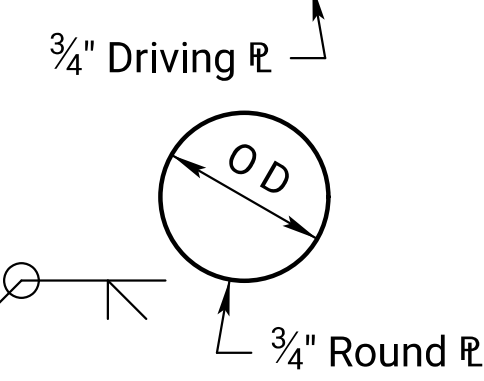
FHWA APPROVAL		04-17-10	APPD	Terry L. Fleck
DESIGNED	DETAILED	R.D.R.	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	L.R.R.	QUAN.CK.	TRACE CK.

OD	10 3/4"	T. = ¶¶
OD	12 3/4"	T. = ¶¶
OD	14"	T. = ¶¶

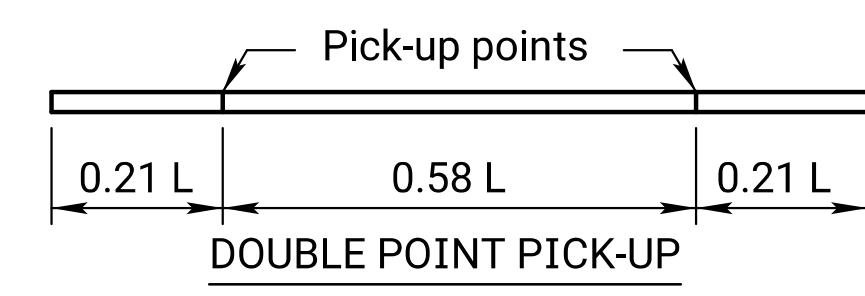
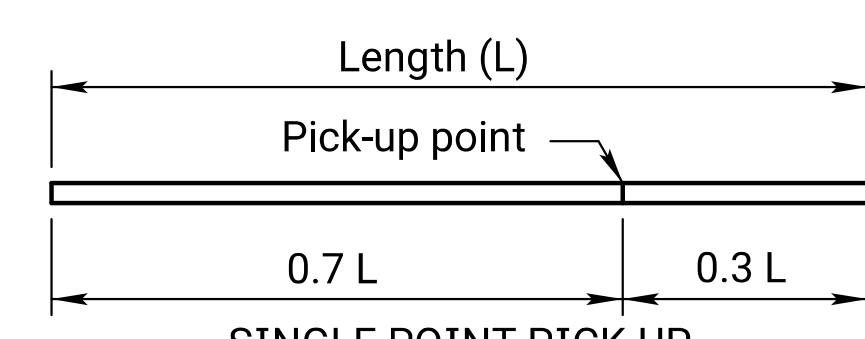
¶¶ See the Geology Report or "Summary of Quantities" for Pipe Pile wall thickness

Note: Pile shall be driven with a steel head having a projecting ring fitting inside the pipe. Clearance between ring and pipe should be 1/4".

Note: Pile pipe may be spiral welded, longitudinal welded, or seamless steel pipe.



PLAIN ROUND CAST-IN-PLACE CONCRETE PILES

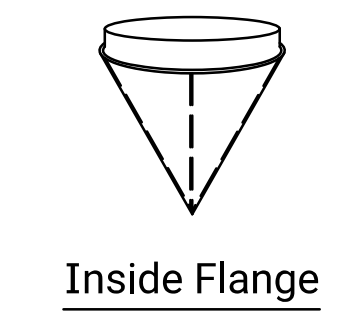
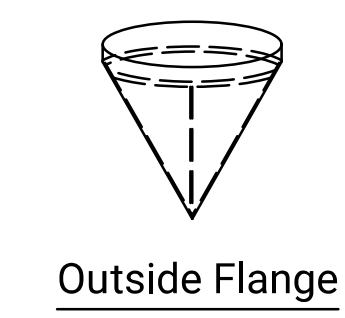


PICK-UP POINTS FOR PRESTRESSED PILING

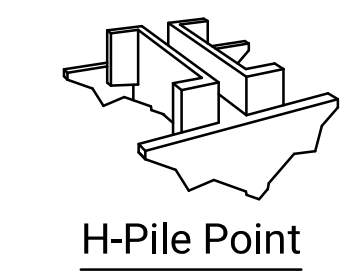
Max. length - 55' single point pick-up
Max. length - 80' double point pick-up

Note: Piles shall be marked at Pick-up points to indicate proper points for attaching handling lines.

12" OR 14" PRESTRESSED CONCRETE PILES

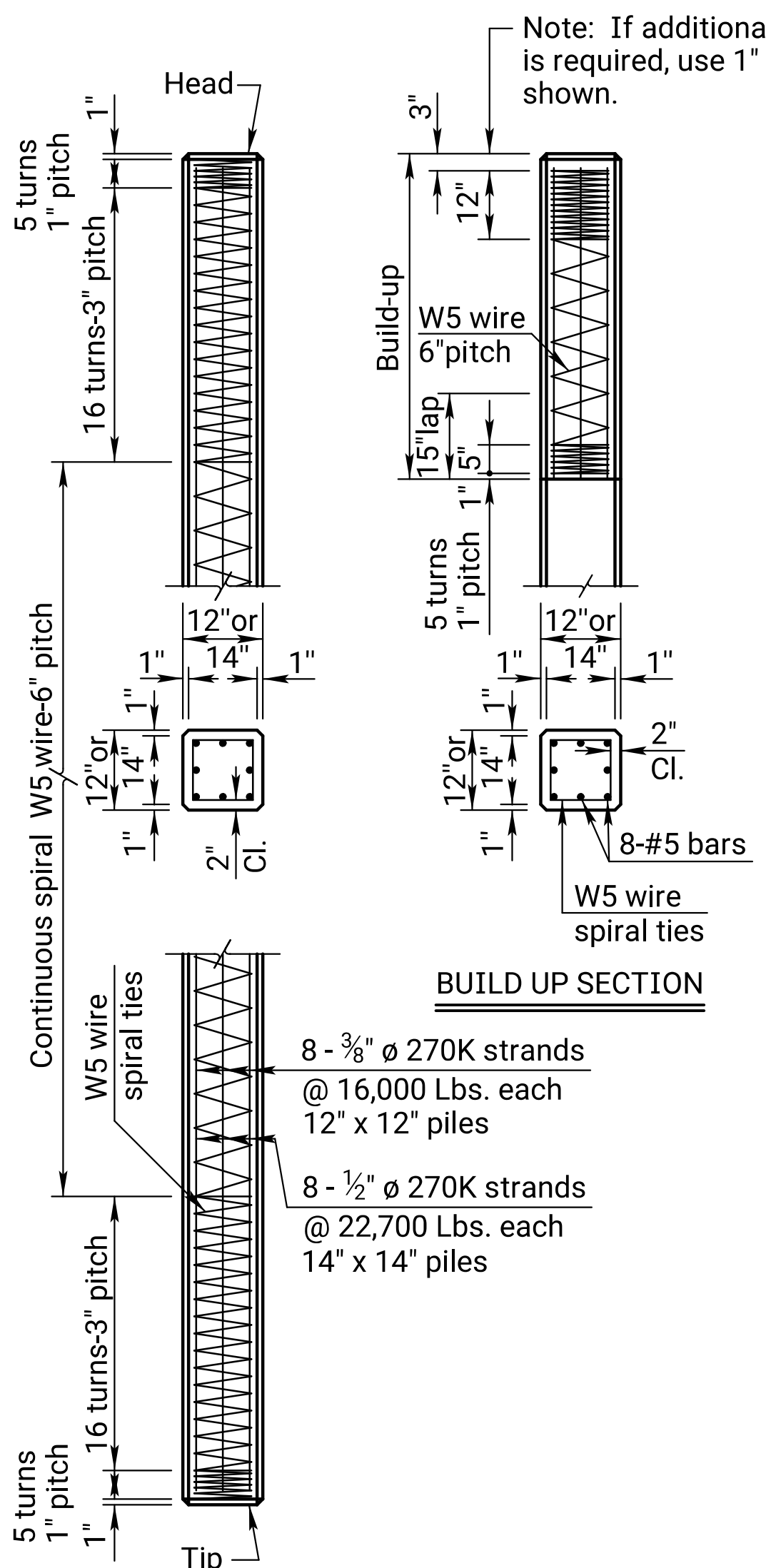


SHELL PILE POINT



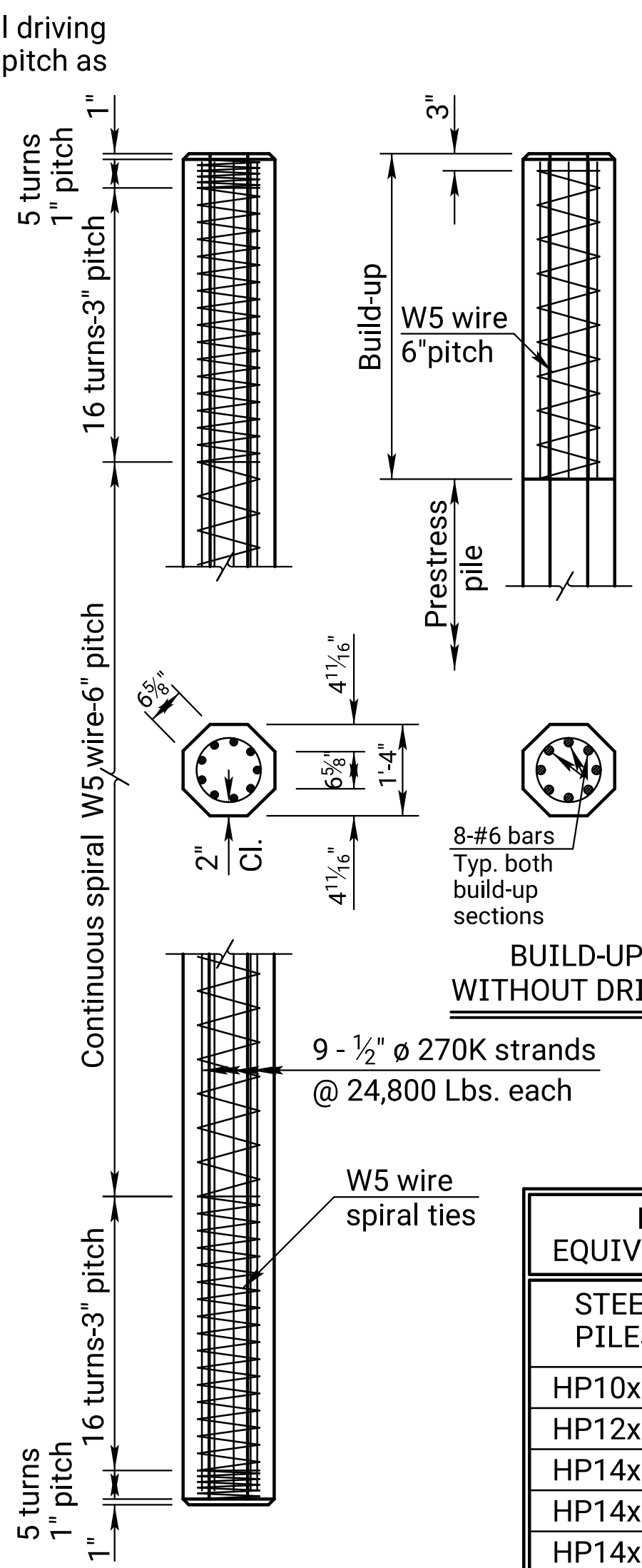
CAST STEEL PILE POINT

The pile point shall be a one-piece unit of cast steel. Weld pile points in accordance with manufacturer's recommendations to each steel pile before driving.



Note: If additional driving is required, use 1" pitch as shown.

BUILD UP SECTION



FOR INFORMATION ONLY EQUIVALENT POINT BEARING PILES		
STEEL PILES	CONCRETE PILES	
	Pipe	Pre-stress
HP10x42	10 3/4"	
HP12x53	12 3/4"	
HP14x73	14	12
HP14x102		14
HP14x117		16

Weld Symbology Definition

Use grinder to bevel edges of splice as shown in weld symbology and drawing. In addition to bevels, produce clean, bare, and shiny surfaces at and around the splice welding location.

Lay full penetration root weld from beveled side of splice.

Back gouge root weld from side opposite of root welding application making sure to remove all foreign materials, porous steel, and inclusions from root weld. Finish welding the non beveled side of the splice.

Finish welding beveled side of the splice while removing slag, foreign materials, porous steel, and inclusions in between welding passes, use of a grinder may be needed.

Verify that enough filler metal has been correctly placed in all weld locations to obtain a flush or convex surface with no concavity produced upon completion of the final welds.

SPICES: Splices for steel piles and shell piling shall be in accordance with details shown on this sheet and the Standard Specifications.

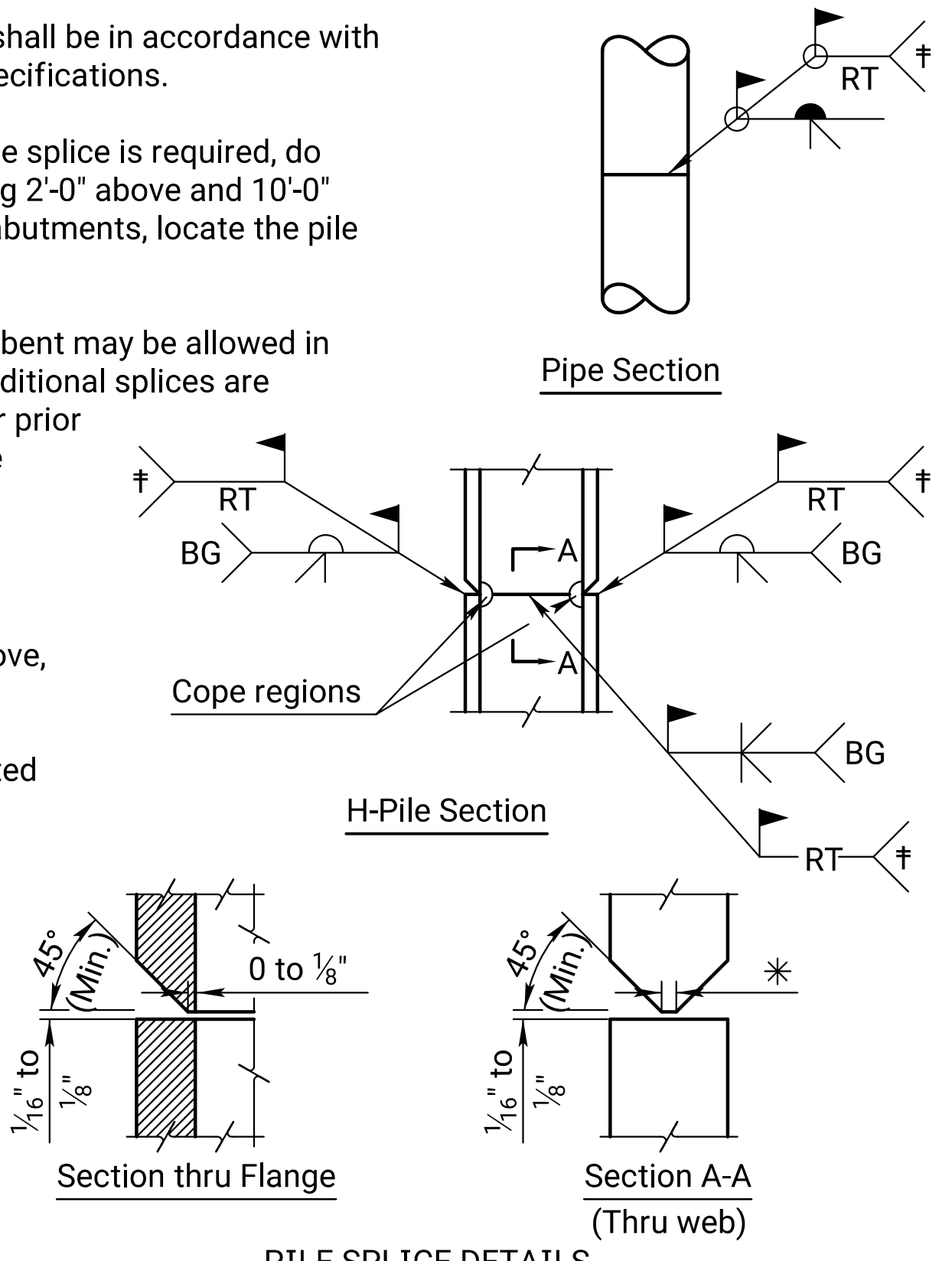
For integral pile bent abutments and piers, if a pile splice is required, do not locate the pile splice within a region extending 2'-0" above and 10'-0" below the bottom of the concrete web wall. For abutments, locate the pile splice at least 10'-0" below top of fill.

With the approval of the Engineer, one splice per bent may be allowed in the region described above without testing. If additional splices are anticipated, based on the geology, the Contractor prior to driving, will locate the splice so that the splice will not fall within the regions described above.

¶ For integral pile bent abutments and piers, if a splice is located within the regions described above, then the Contractor will test the welds by Radiograph (RT) test methods. Repair and retest any welds not passing the test(s). Each weld tested will have written confirmation of results. Report these results to the Engineer. This work is not paid for directly, but is subsidiary to "Piles".

* Minimum as required by welding process.

BG = Backgouge



GENERAL NOTES

PRESTRESSED PILES: Fabricate prestressed concrete pile splices in accordance with the Manufacturer's recommendations subject to the approval of the Engineer.

Method of attachment of pile to build-up may be by any of the methods given in the notes on "Alternate Methods." If mild reinforcing steel is used for attachment, the area shall be no less than that used in the build-up.

ALTERNATE METHODS: Method of attachment of a pile to build-up may be by any of the following methods:

1. Cut off at least 2'-0" of pile and expose a minimum of 2'-0" of strands.
2. Cast 8-#6, or 8-#5 bars (equally spaced) into pile head. All bars shall extend into pile head and project from pile head a minimum of 2'-0".
3. Drill 8 holes in pile head (equally spaced) for installation of 8 grouted dowel bars of same size and length as in 2.
4. Provide cored holes for bars as in 3.

No bars or strands are to extend from head of pile or build-up into footing or pile cap unless approved by the Engineer.

TEST PILES: Drive test piles where called for on the bridge plans. The test piles located within the limits of the substructure will become a part of the bridge pile system.

DRIVING FORMULA: Driving formula shall conform to the Standard Specifications.

MEASUREMENT AND PAYMENT: Measurement and payment for all piles shall comply with the Standard Specifications.

REINFORCEMENT: Use reinforcing steel conforming to ASTM A615, Grade 60. Hoops and spirals may be either plain or deformed bars.

PRESTRESSING STEEL: Use uncoated seven-wire low relaxation prestressing strand conforming to ASTM A416, Gr. 270.

STEEL PILE: Steel pile shall conform to the requirements of the Standard Specifications.

PILE POINTS: Pile points shall conform to the dimensions shown and to requirements of the Standard Specifications.

SPECIFICATIONS: Standard Specifications for State Road and Bridge Construction as currently used by the Kansas Department of Transportation.

CONCRETE: Concrete for cast-in-place shall be f'c = 3,500 PSI. Concrete for prestressed shall be f'c = 5,000 PSI.

WELDING: All field welding shall meet the requirements of the Standard Specifications.

Use only Shielded Metal Arch Welding SMAW (stick welding) for pile splices.

Use only low hydrogen E7018, 7016, or 7015 series welding rod (electrode) for all welding applications during pile splicing.

New electrodes are to be purchased for each KDOT project. The electrodes shall arrive on the project in factory hermetically sealed containers, opened and labeled with indelible ink in front of the engineer. The label shall include the current date and the project number. If the container seal is questionable or shows signs of damage the electrode is to be dried in an oven at least one hour at a temperature of 700°F to 800°F.

Upon removal from intact hermetically sealed factory packaging or the drying oven the electrode is to be placed in a storage oven with a minimum temperature of 250°F.

When electrodes are removed from the hermetically sealed container or storage oven and exposed to the atmosphere for less than 4 hours place into the storage oven for at least 4 hours before removing for use.

If electrode is exposed to the atmosphere for 4 hours or more (or 9 hours for moisture resistant electrodes designated with an R in their labeling) then electrode can be dried in a drying oven at a temperature of 450°F to 550°F.

If the electrode is exposed to the atmosphere for 4 hours or more a second time or the rod becomes wet discard rod.

CAST-IN-PLACE SHELLS: Steel shells for cast-in-place concrete piles shall conform to the requirements of the Standard Specifications.

All piles driven without a mandrel shall be of the minimum thicknesses shown. Piles driven with a mandrel shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion and/or buckling due to soil pressure after the mandrel is removed.

Remove, replace or correct to the satisfaction of the Engineer improperly driven, broken or otherwise defective pipe piles. Otherwise drive an additional pile at no extra cost.

The Contractor shall maintain a light suitable for visual inspection of the pile on the job at all times prior to and during the filling of the pipe.

PAIN: All paint shall comply with the Standard Specifications, or as specified on the plans.

MILL TEST REPORTS: Steel piles test reports and steel shell test reports shall comply with the Standard Specifications.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	22	44

NO.	DATE	REVISIONS	BY	APPD
04	08-16-18	Add splice web section, clarify note	M.L.L.	J.P.J.
03	09-15-15	Clarify Notes	J.P.J.	C.E.R.
02	06-18-12	Clarify f'c, rod type, use and weld	J.P.J.	T.L.F.

KANSAS DEPARTMENT OF TRANSPORTATION

STANDARD PILE DETAILS

BR110		10-04-12		APPD.	Terry L. Fleck
DESIGNED	J.P.J.	DETAILED	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		

GENERAL NOTES

Reference is made to the latest edition of the CRSI "Manual of Standard Practice" for recommended industry practices concerning reinforcing steel.

Use only the following types of bar supports:

- 1) Wire Bar Supports:
 - a) Epoxy coated reinforcing: Class 1 Protection
 - b) Non-epoxy coated reinforcing: Class 1, 2, or 3 Protection
- 2) Plastic Bar Supports
- 3) Supplementary bars

When securing epoxy coated reinforcement, use tie wires or metal clips that are epoxy or plastic coated.

Do not weld reinforcing steel to bar supports or to other reinforcing steel. Shop weld spacer frames for haunched slabs.

Tie bars at all intersections around the perimeter of each mat and at not less than 2'-0" centers or at every intersection, whichever is greater.

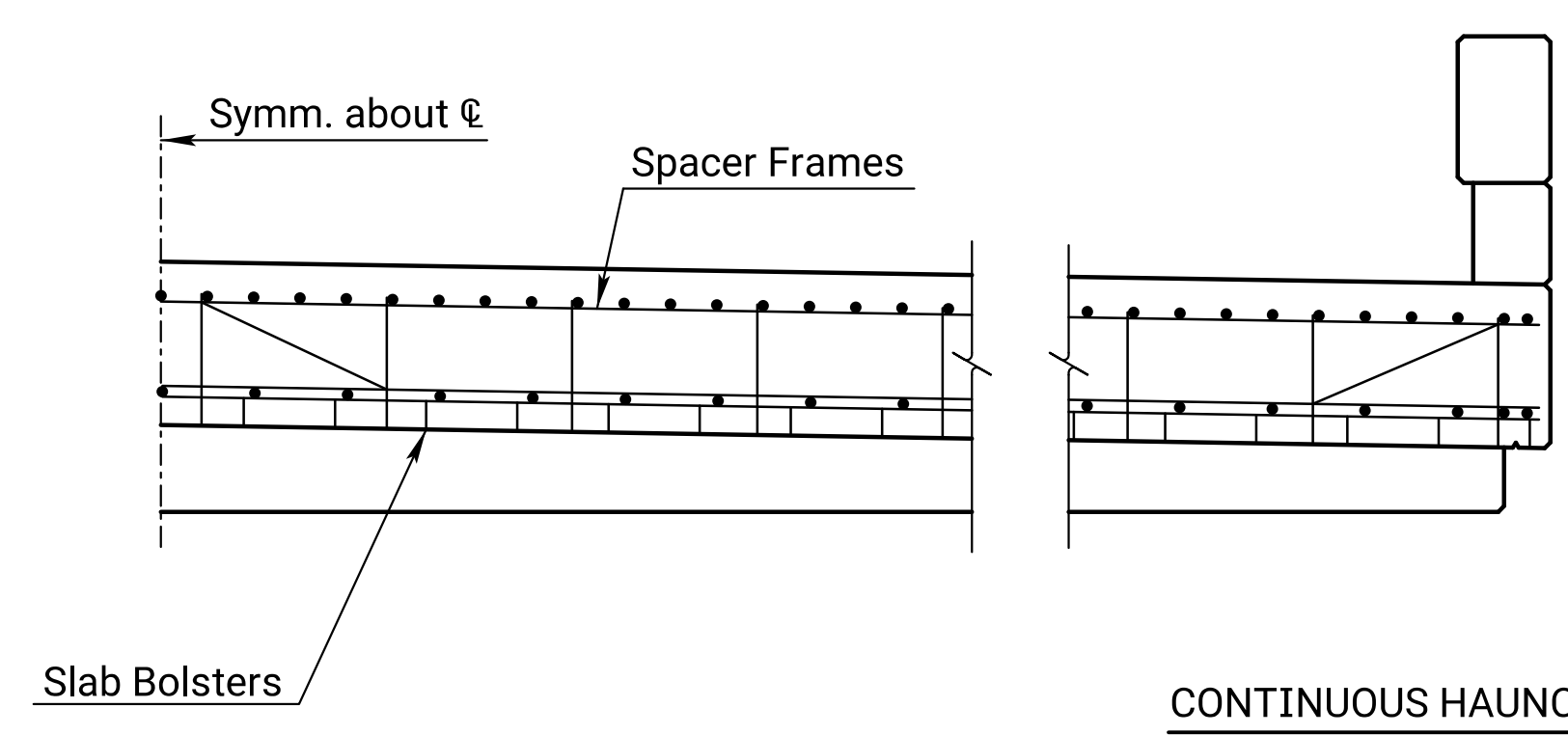
Where more than one length of bar support is required, lap the end legs so they are locked or tied together.

Use proper height supports to maintain the distance between the reinforcing and the formed surface or the top surface of deck slabs within 1/4" of that indicated on the plans.

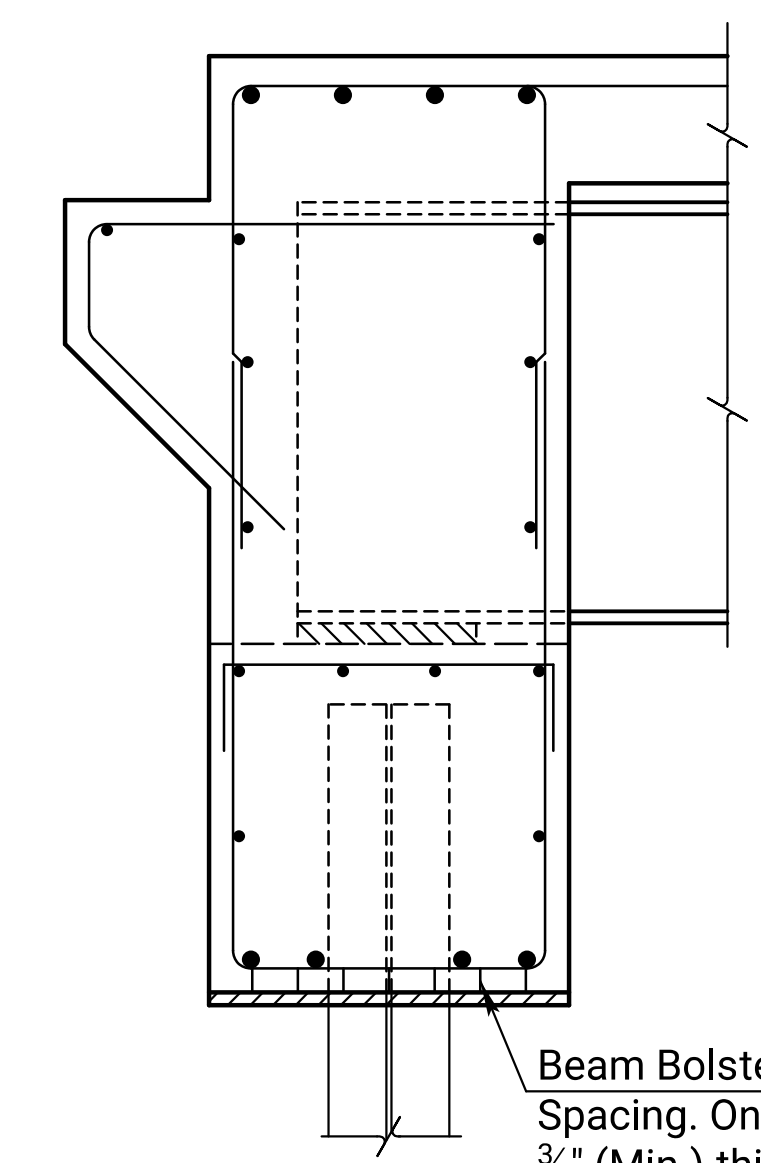
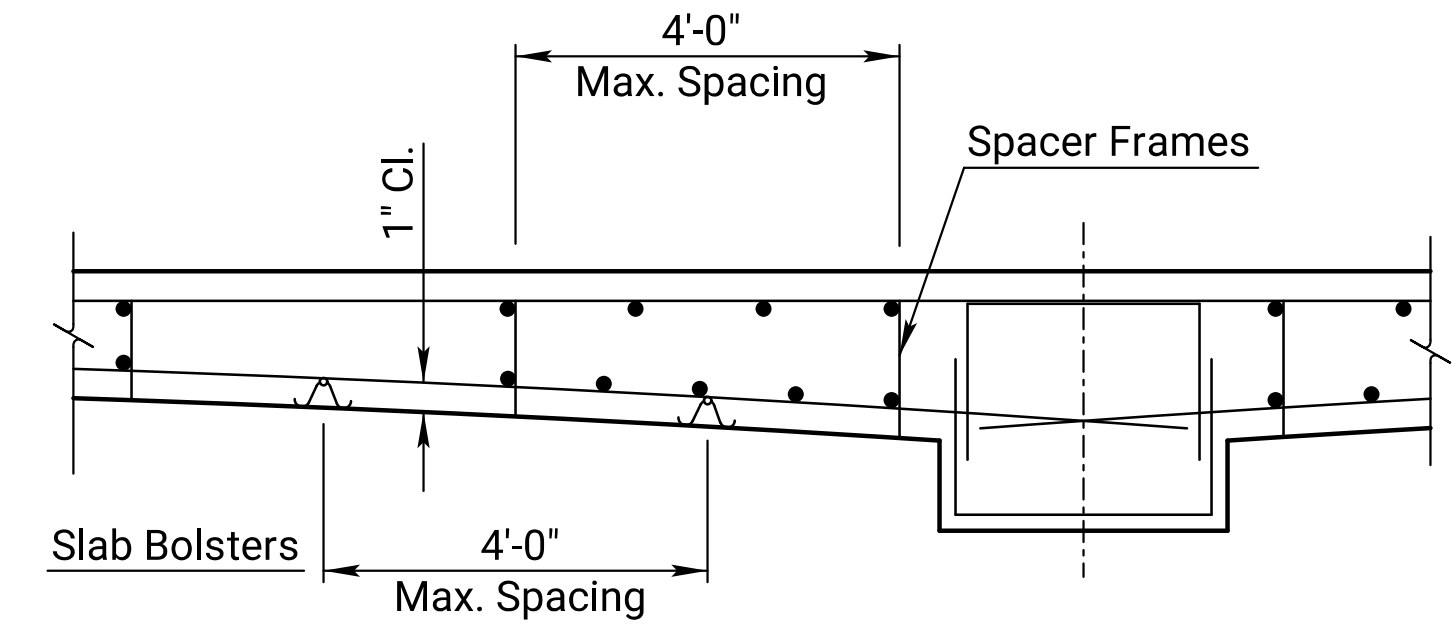
Spacings shown are maximums. Use sufficient supports, as determined by the Engineer, to retain the reinforcing steel in position.

Construct any platforms, required for the support of workers and/or equipment during concrete placement, directly on the forms and not on the reinforcing steel.

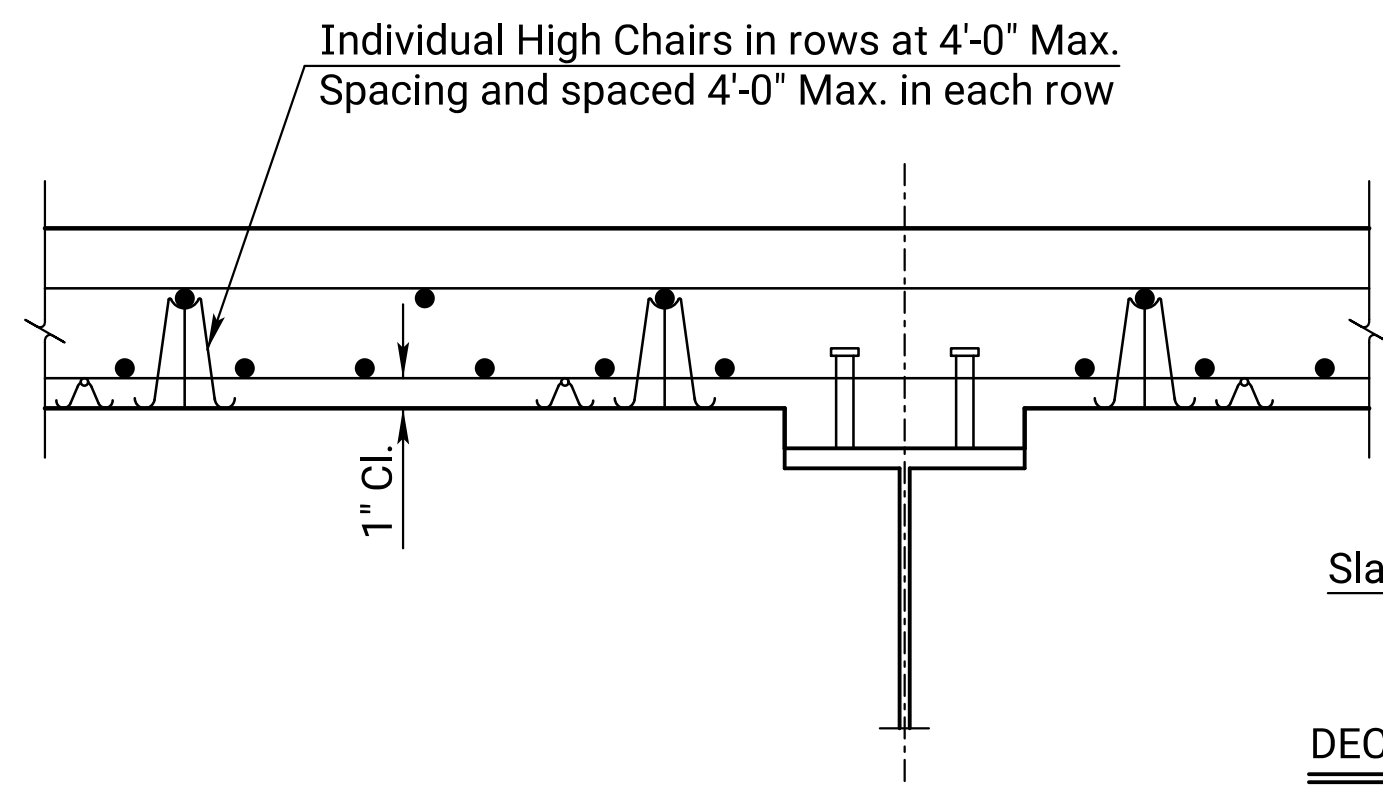
Designs and arrangements of Supports or Spacers other than as shown on this sheet, may be used with the permission of the Engineer.



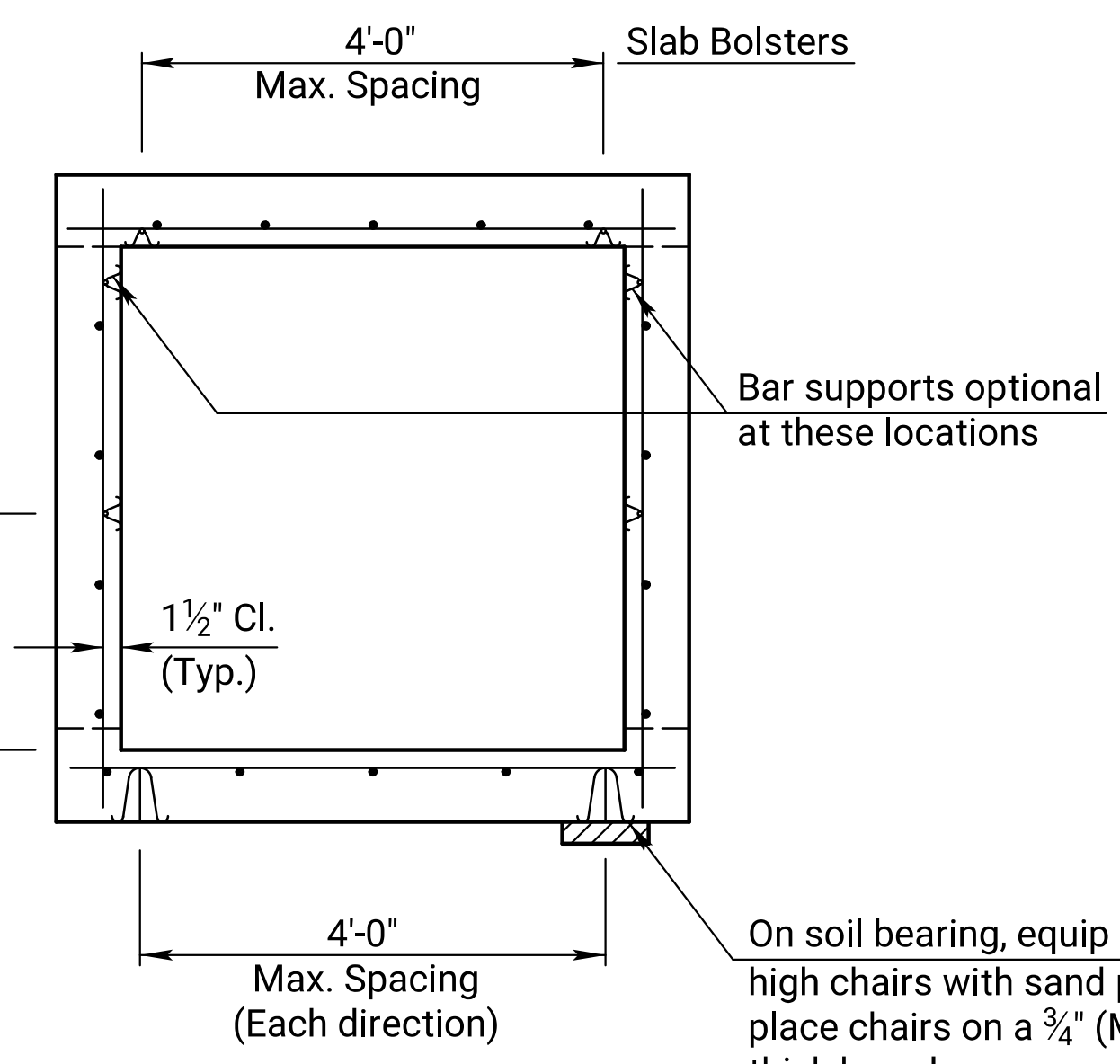
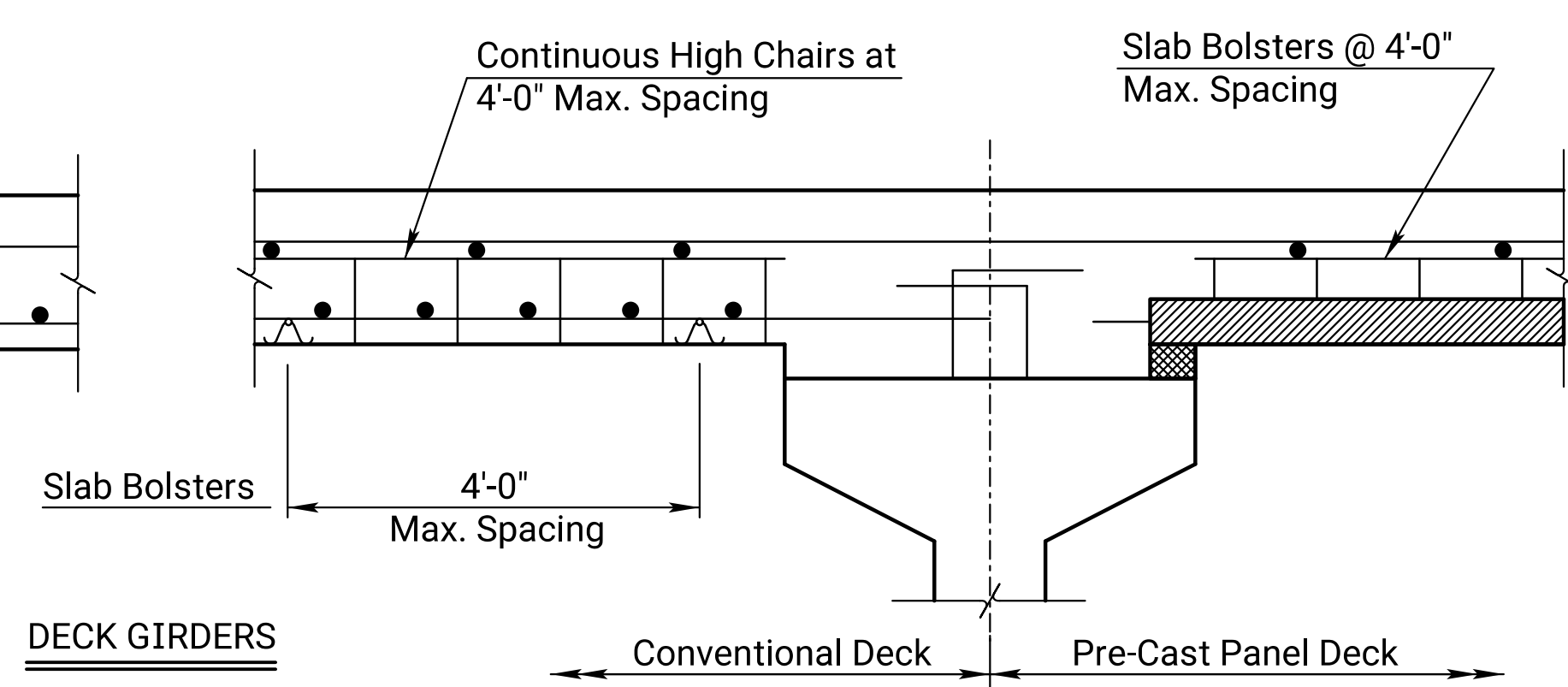
CONTINUOUS HAUNCHED SLAB



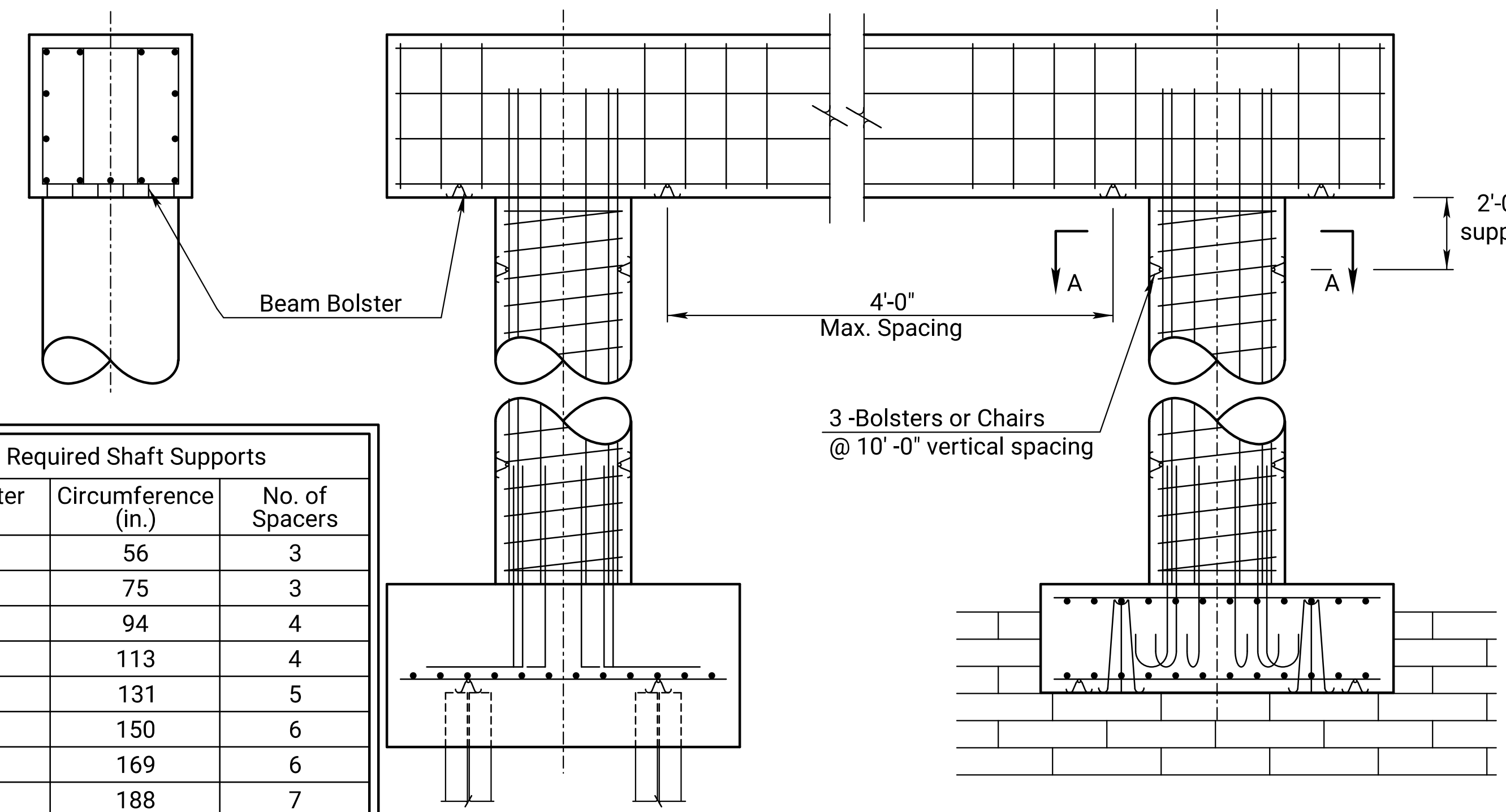
ABUTMENT



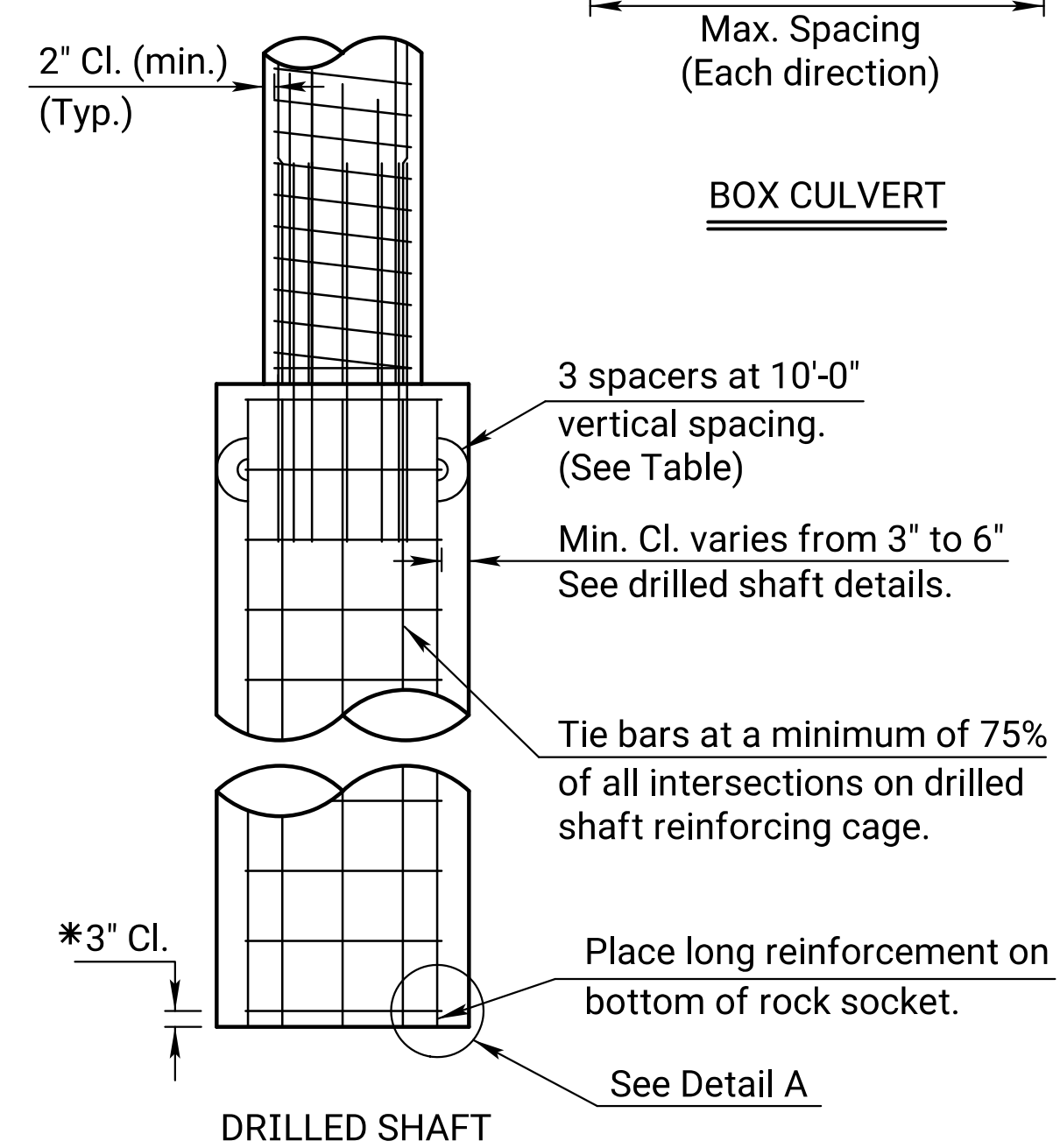
DECK GIRDERS



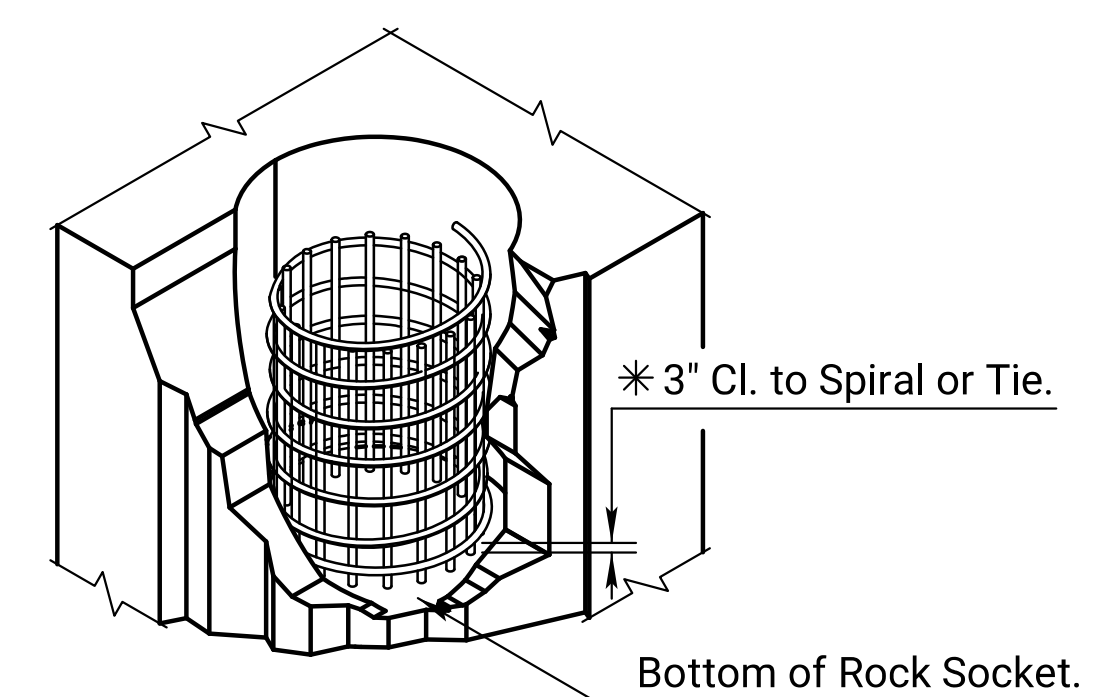
BOX CULVERT



PIER

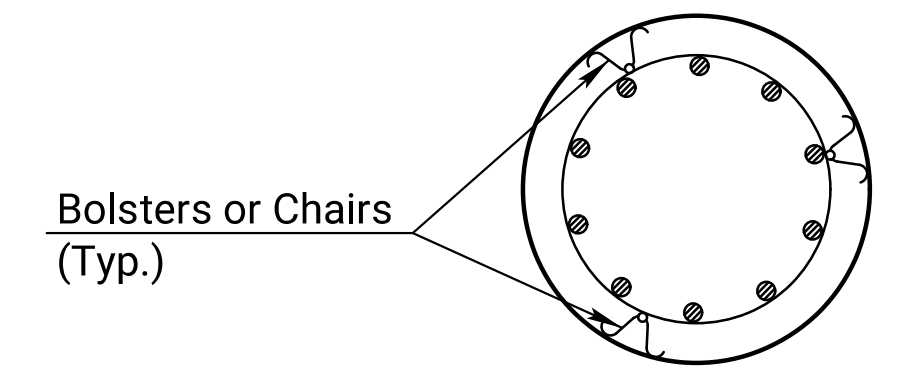


DRILLED SHAFT



DETAIL A

* Note: Longitudinal reinforcing steel is placed on the bottom of the rock socket. Maintain 3" clearance from the bottom of rock socket to the first spiral or tie bar.



SECTION A-A

Required Shaft Supports		
Diameter (in.)	Circumference (in.)	No. of Spacers
18	56	3
24	75	3
30	94	4
36	113	4
42	131	5
48	150	6
54	169	6
60	188	7
66	207	7
72	226	8
78	244	9
84	263	9
90	282	10
96	301	11
102	320	11
108	339	12

KANSAS DEPARTMENT OF TRANSPORTATION					
SUPPORTS AND SPACERS FOR REINFORCING STEEL					
BR120					
NO.	DATE	REVISIONS	BY	APPD	
05	11-10-10	Column Bar Supports Required	J.P.J.	T.L.F.	
04	12-01-05	Drilled Shaft Spiral Steel Placement	J.P.J.	K.F.H.	
03	08-21-00	Added Pre-Cast Panel Detail	R.A.M.	K.F.H.	
DESIGNED	R.A.M.	DETAILED	R.A.A.	QUANTITIES	TRACED
DESIGN CK.	L.R.R.	DETAIL CK.	R.A.M.	QUAN. CK.	TRACE CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	24	44

DRAINAGE STRUCTURES												
Station	Side	Size	Type	Conc. Gr. 4.0 Cu. Yds.	Reinf. Steel Lbs.	Cross Road Pipe (Feet)			Entrance Pipe (Feet)		End Section (Each)	
									(CSP, ACSP, CAP) 24"		(CS, ACS, CA) 24"	
101+00.00	Lt.	24"	EP						38		2	
TOTALS									38		2	

RECAPITULATION OF BRIDGE QUANTITIES		
BRIDGE NUMBER	STATION	SEE SHEET NO.
000730643005409	102+85.00	11

RECAPITULATION OF ROAD QUANTITIES		
ITEM	QUANTITY	UNIT
Contractor Construction Staking	Lump Sum	L.S.
Field Office & Laboratory (Type C)	1	Each
Foundation Stabilization (Set Price)	1	Cu. Yd.
Mobilization	Lump Sum	L.S.
Mobilization (DBE)	Lump Sum	L.S.
Removal of Existing Structures	Lump Sum	L.S.
Concrete for Seal Course (Set Price)	1	Cu. Yd.
Clearing & Grubbing	Lump Sum	L.S.
Common Excavation (Rural Small)	1461	Cu. Yds.
Compaction of Earthwork (Type B) (MR-90)	840	Cu. Yds.
Water (Grading) (Set Price)	1	M. Gal.
Curing Environment	1	L.S.
End Section (24")	2	Each
Entrance Pipe (24")	38	Lin. Ft.
Guardrail, Steel Plate	100	Lin. Ft.
Guardrail End Terminal (SRT) Alt. #1	4	Each
Guardrail End Terminal (FLEAT) Alt. #2	4	Each
Temporary Surfacing Material (Aggregate) (Set Price)	1	Cu. Yd.
Signing Object Marker (Type 3)	4	Each

** REMOVAL OF EXISTING STRUCTURES		
STATION	LOCATION	DESCRIPTION
102+41.59	Lt.	36" x 48' RCP
102+54.14	Rt.	24" x 24' RCP
103+00.00	€	17'-25'-17' Steel Beam Continuous (SBMC) Bridge
** FOR INFORMATION ONLY		

EARTHWORK																		
STATION to STATION	EXCAVATION				COMPACTION			NOT SUBGRADED THROUGH CUTS		* EMBANKMENT (CU.YDS.)		▲ PLACE. SELECT SOIL CU.YDS.						
	COMMON		ROCK		CONTR. FURN. CU.YDS.	TYPE AA MR- CU.YDS.	TYPE B MR- CU.YDS.	COMM. CU.YDS.	TYPE AA CU.YDS.	INITIAL CONSOL.	SETTLE- MENT							
	CU.YDS.	VMF	CU.YDS.	VMF														
97+50.00 TO 106+00.00	1461	0.73					840											
TOTALS											1461				840			

For Summary of Signing Object Markers See Sheet No. 2
 For Summary of Guardrail See Sheet No. 4
 For Temp. Erosion & Pollution Control Quantities See Sheet No. 25
 For Seeding Quantities See Sheet No. 34
 For Traffic Control Plan & Quantities See Sheet No. 40

NO.	DATE	REVISIONS	BY	APPD
02	01-14-08	Rem. Drainage Structure summary	S.W.K.	J.O.B.
01	01-09-91	Detailed on CADD	R.J.S.	J.O.B.

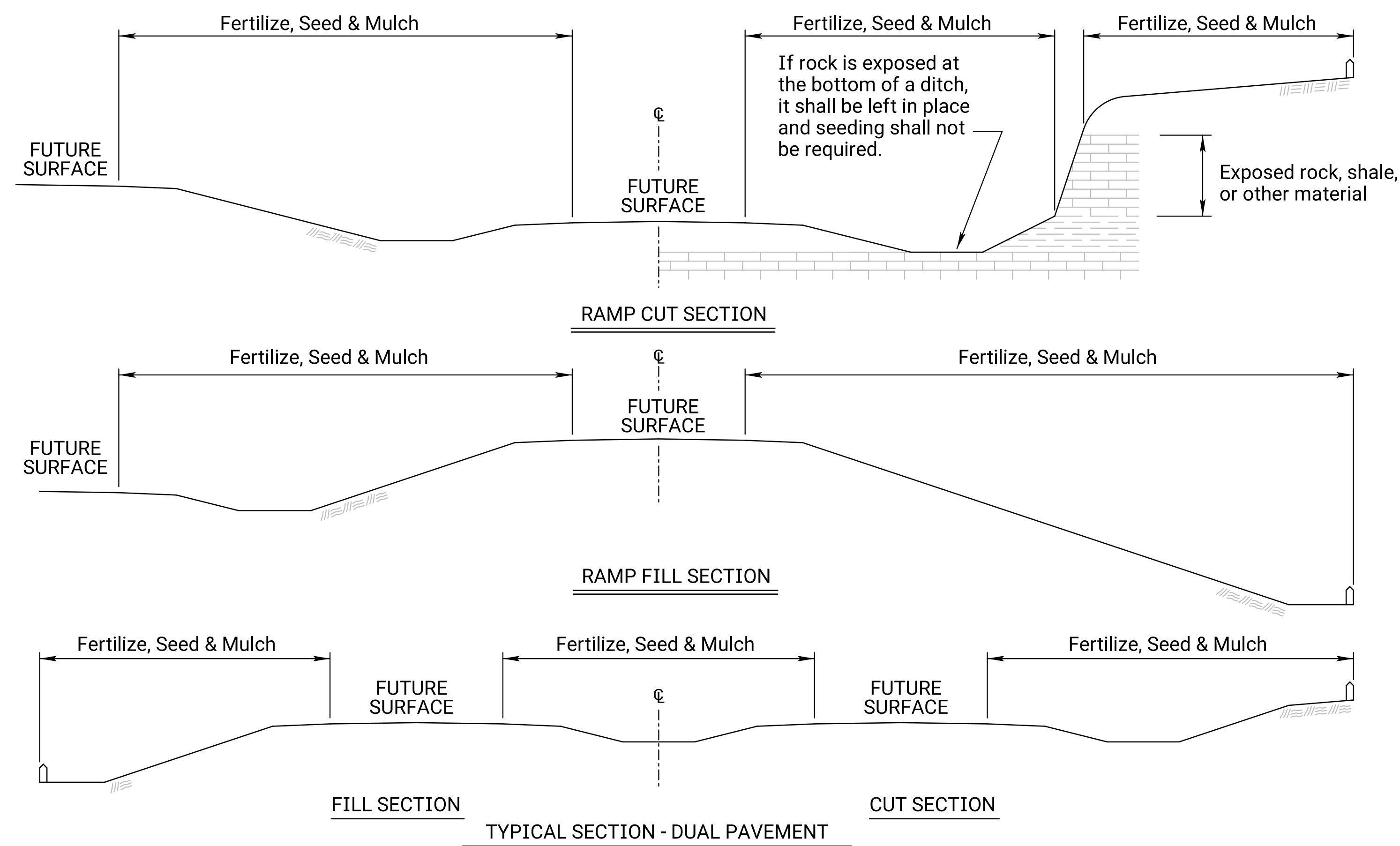
KANSAS DEPARTMENT OF TRANSPORTATION

SUMMARY OF QUANTITIES

RD050

DESIGNED	DETAILED	QUANTITIES	TRACED	B.N.B.
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	S.W.K.

FHWA APPROVAL 05-28-08 APPD. James O. Brewer



FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P₂O₅, K₂O listed in Summary of Quantities will be acceptable.

- * - N = Nitrogen Rate of Application
- ** - P₂O₅ = Phosphorous Rate of Application
- *** - K₂O = Potassium Rate of Application

The Contractor will be required to finish areas of excavation, borrow and embankment in accordance with the specifications. Areas that require installation or construction of temporary water pollution control items will be finished in reasonable close conformity to the alignment, grade and cross section shown on the plans or as established by the Engineer.

CLT = Construction Limit Tract. This area is defined by the entire disturbed area of the project that requires seeding and erosion control measures to be placed. Any impervious areas (i.e. pavement, gravel, riprap, etc.) shall not be included in this measurement.

Slope = Defined by the area of the project that requires Class 1 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

Channel = Defined by the area of the project that requires Class 2 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

GENERAL NOTES

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded, and mulched. Soil preparation shall conform to the Standard Specifications.

Temporary seeding shall be done during any time of the year that the soil can be cultivated. After the temporary seeding has been completed on the entire project, permanent seeding shall be done during the normal seeding season.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching materials is generally as follows:

$$1\frac{3}{4} - 2\frac{1}{4} \text{ Tons per Acre} = 1\frac{1}{2}'' \text{ loose depth spread uniformly over acre.}$$

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

SUMMARY OF SEEDING / EROSION CONTROL QUANTITIES

P.L.S. RATE/ ACRE		ACRES		BID ITEM	QUANTITY	UNIT
CLT	SL/CH	CLT	SL/CH			
200		0.58		Temporary Fertilizer (13 - 13 - 13)	116	LB
				Temporary Seed (Canada Wildrye)		LB
				Temporary Seed (Grain Oats)		LB
				Temporary Seed (Sterile Wheatgrass)		LB
	109.9		0.58	Soil Erosion Mix	63.7	LB
				Erosion Control (Class 1, Type C)	2828	SQ YD
				Erosion Control (Class 2, Type Y)		SQ YD
				Sediment Removal (Set Price)	1	CU YD
				Synthetic Sediment Barrier		LF
				Temporary Berm (Set Price)	1	LF
				Temporary Ditch Check (Rock)	57	CU YD
				Temporary Inlet Sediment Barrier		EACH
				Temporary Sediment Basin		CU YD
				Temporary Slope Drain		LF
				Temporary Stream Crossing		EACH
				Biodegradable Log (9")		LF
				Biodegradable Log (12")		LF
				Biodegradable Log (20")		LF
				Filter Sock (****)		LF
				Geotextile (Erosion Control)	500	SQ YD
				Silt Fence		LF
				SWPPP Design †	1	LS
				SWPPP Inspection †	23	EACH
				Water Pollution Control Manager †	23	EACH
900 lbs / acre				Mulch Tacking Slurry		LB
2 tons / acre				Mulching		TON
				Water (Erosion Control) (Set Price)	1	MGAL

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. See Permanent Seeding Summary of Seeding Quantities sheet LA850 for further details.

Geotextile (Erosion Control) shall be removed prior to placement of permanent slope protection.

Regreen and Quick Guard are the approved sterile wheatgrass products.

† If the total disturbed area of the project, not just the seeding area, is 1 acre or more, then these bid items must be included.

**** List size of material.

The amount of mulch and mulch tacking slurry in the bid quantities is estimated. (Acres of Seeding X 1.5 X 2 Tons/Acre). The estimated quantity includes mulching associated with both temporary and permanent seeding operations. The total mulch and mulch tacking slurry required shall be determined in the field. The bid item for mulching and mulch tacking slurry shall be paid for according to the Standard Specifications.

Quantities for all erosion control items are estimated to give full flexibility for compliance with the NPDES permit. Final quantities will be determined in the field.

SOIL EROSION MIX

PLS RATE	NAME	QTY (lb)
0.5	SEED (BLUE GRAMA GRASS) (LOVINGTON)	0.29
4.5	SEED (BUFFALO GRASS) (TREATED)	2.61
45	SEED (RYEGRASS) (PERENNIAL)	26.10
2.6	SEED (PRAIRIE JUNE GRASS)	1.51
6.3	SEED (SIDE OATS GRAMA) (EL RENO)	3.65
45	SEED (FESCUE) (TALL) (ENDOPHYTE-FREE)	26.10
6	SEED (WESTERN WHEATGRASS) (BARTON)	3.48
109.9	Total (lb)	63.74

The Soil Erosion Mix is to be placed under the Class 1 and/or Class 2 erosion control material.

The Soil Erosion Mix consists of the Shoulder Area of the Permanent Seed Mix used on the project.

Note: Fertilizer for Soil Erosion Mix is included and shown on the Summary of Seeding/Erosion Control chart above.

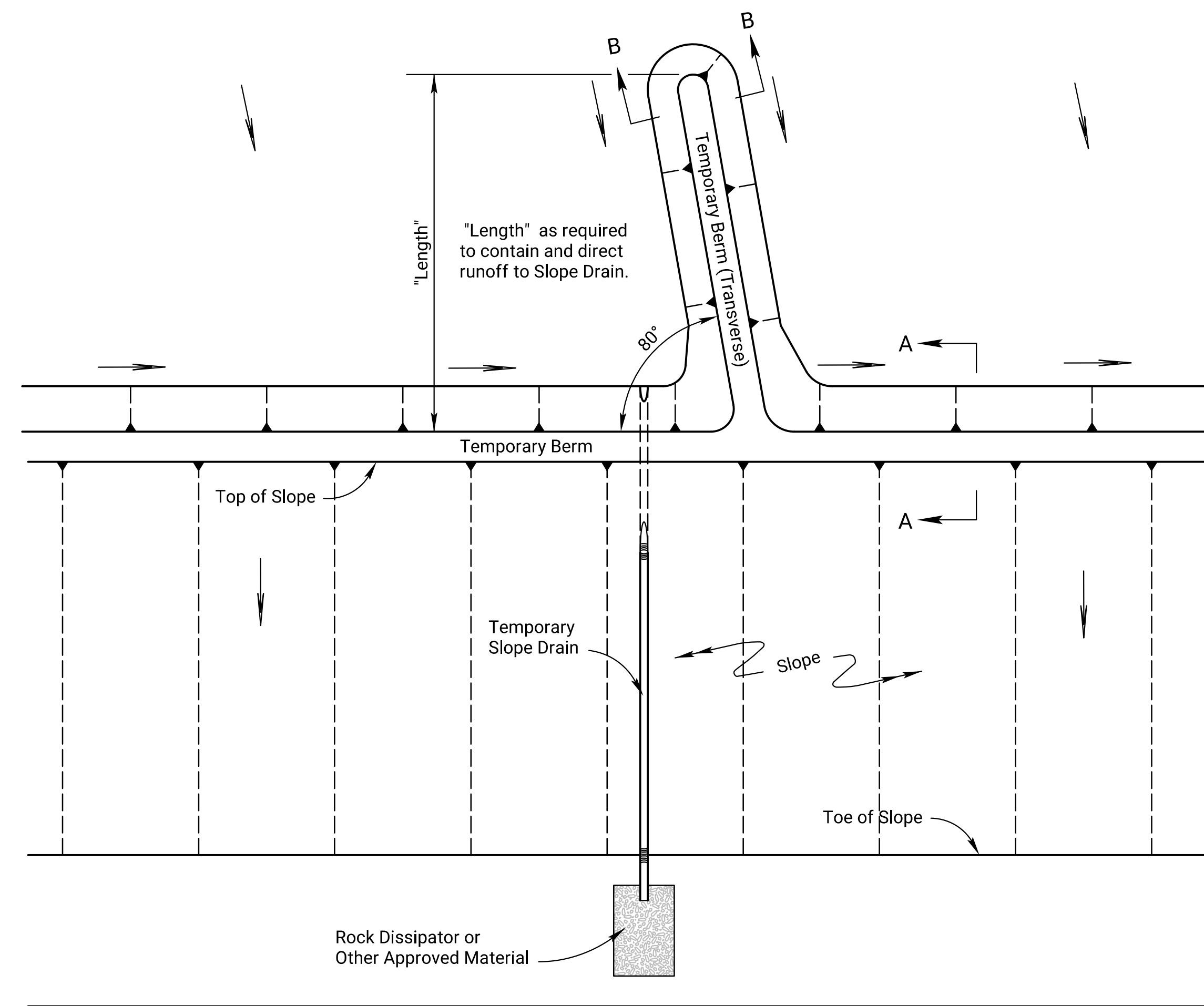
03	08-03-20	Added Note	M.R.D.	M.L.
02	12-01-17	Revised Standard	M.R.D.	S.H.S.
01	06-01-17	Revised Standard	M.R.D.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION

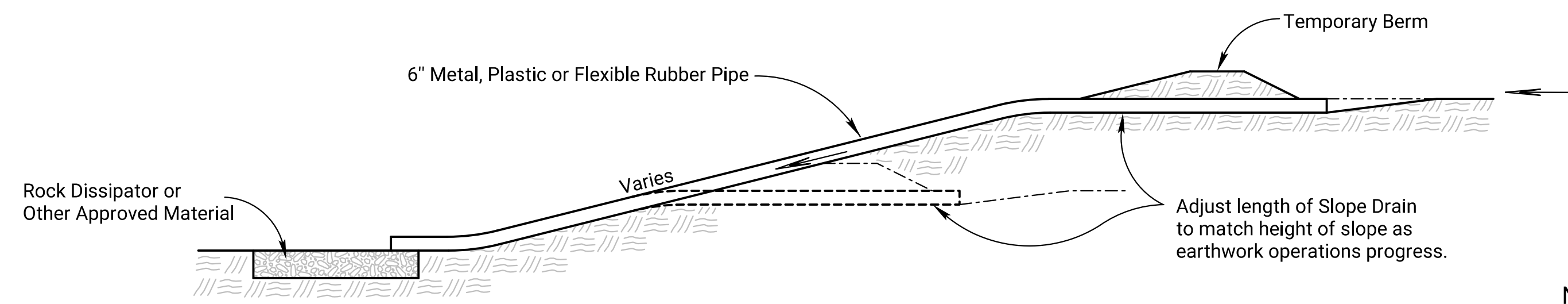
TEMPORARY EROSION AND POLLUTION CONTROL

LA852A		01-26-18		APPD.	Scott H. Shields
DESIGNED	M.R.D.	DETAILED	M.R.D.	QUANTITIES	TRACED
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.	TRACE CK.

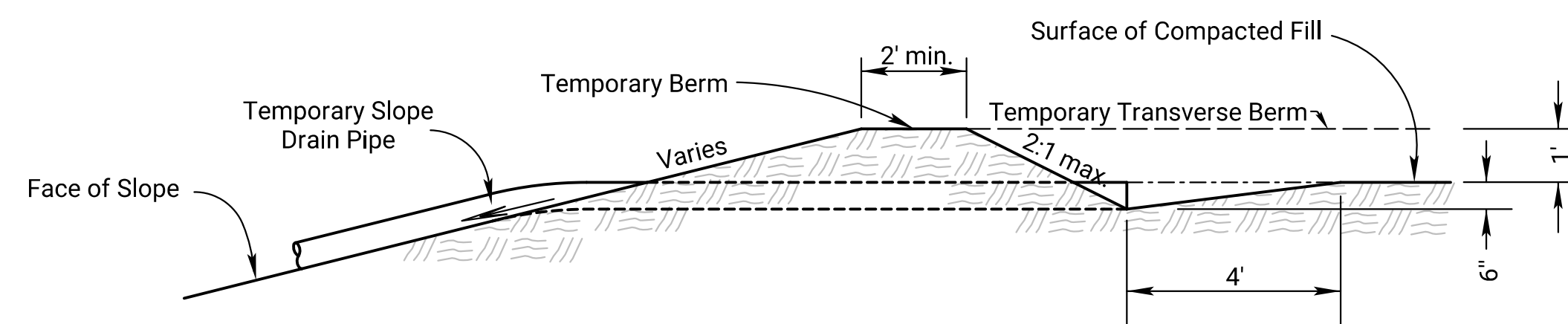
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	27	44



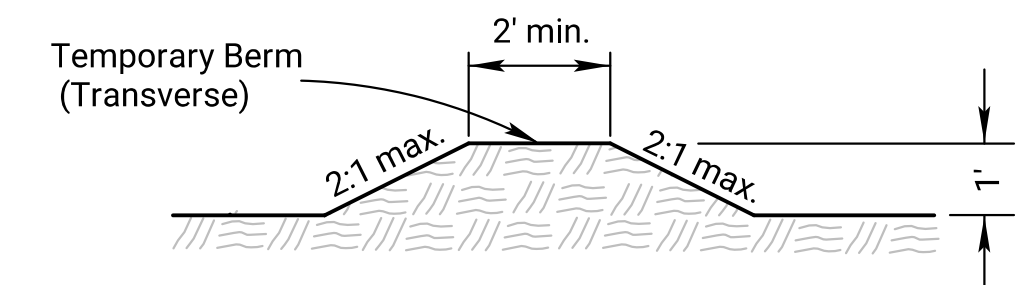
TYPICAL PLAN VIEW OF TEMPORARY BERM AND TEMPORARY SLOPE DRAIN
NO SCALE



TYPICAL PROFILE OF TEMPORARY SLOPE DRAIN
NO SCALE



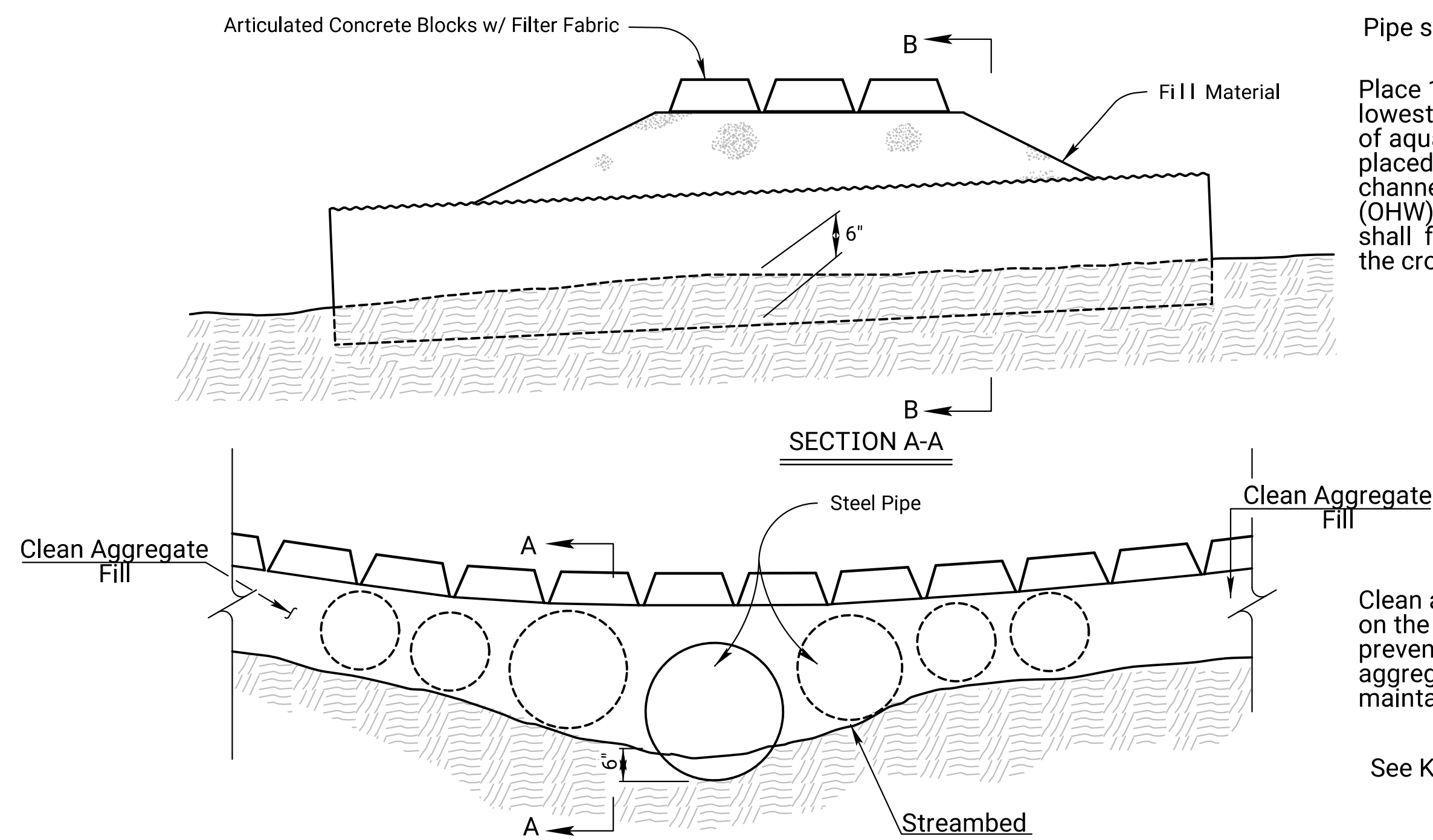
SECTION A-A
NO SCALE



SECTION B-B
NO SCALE

TYPICAL PROFILE OF TEMPORARY BERM
NO SCALE

- NOTES:
- 1) Temporary Slope Drain and Temporary Berm may be used on either project foreslopes or project backslopes.
 - 2) Discharge of Slope Drains shall be into stabilized ditch or area, or into Sediment Basin.
 - 3) Pipe shall be secured in place as approved by Engineer.
 - 4) Temporary Berms under 2,000 feet shall be bid by Set Price.

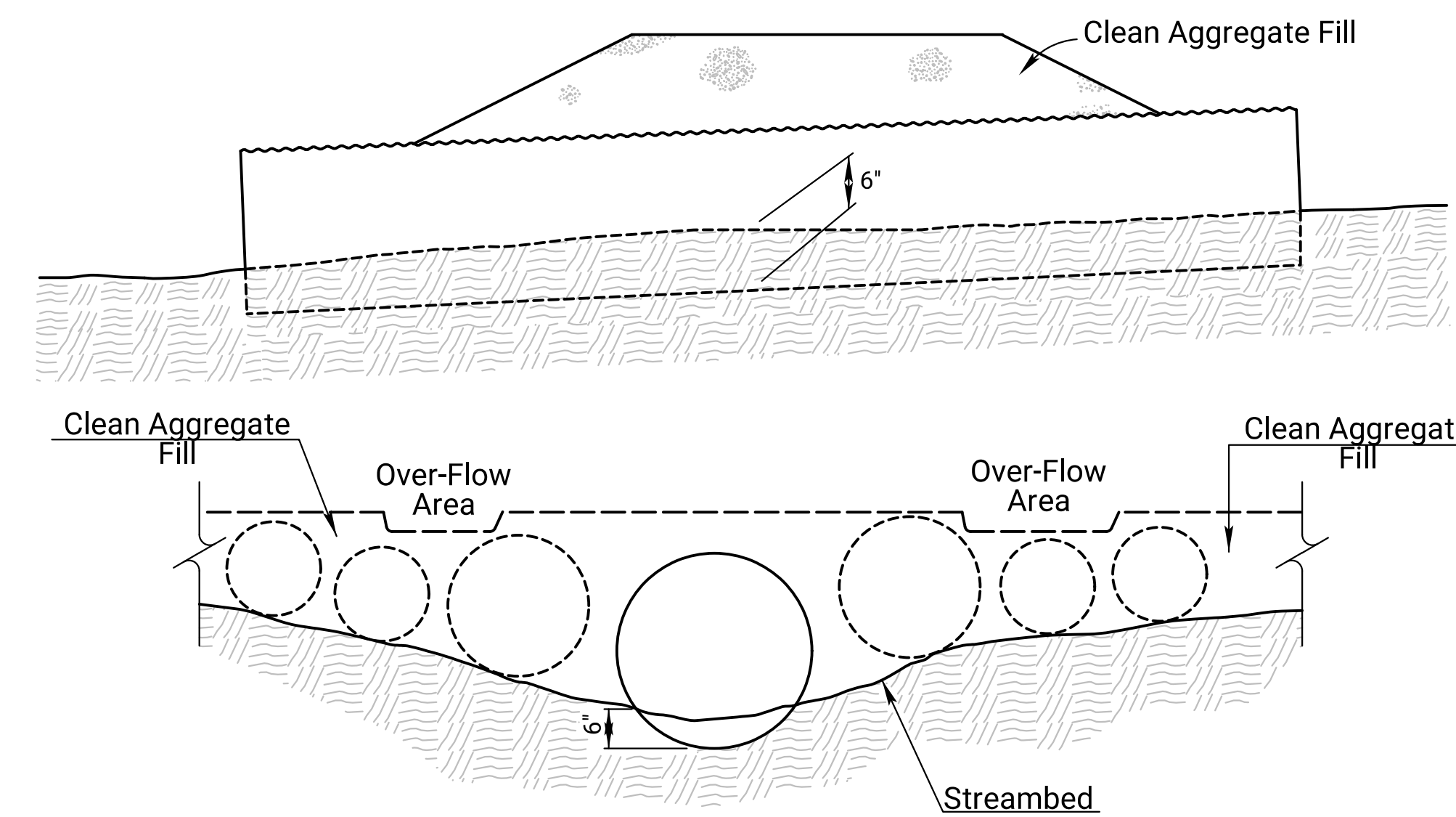


TEMPORARY STREAM CROSSING (ARTICULATED CONCRETE BLOCKS)
NO SCALE

Pipe size may vary.
Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

Clean aggregate fill will extend a minimum of 50' on the entrance and exit side of the crossing to prevent tracking. The aggregate shall be clean aggregate and a minimum of 6" thick and will be maintained through the use of the crossing.

See KDOT Specifications for more information.



TEMPORARY STREAM CROSSING (AGGREGATE)
NO SCALE

Pipe size may vary.
Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

Clean aggregate fill will extend a minimum of 50' on the entrance and exit side of the crossing to prevent tracking. The aggregate shall be clean aggregate and a minimum of 6" thick and will be maintained through the use of the crossing.

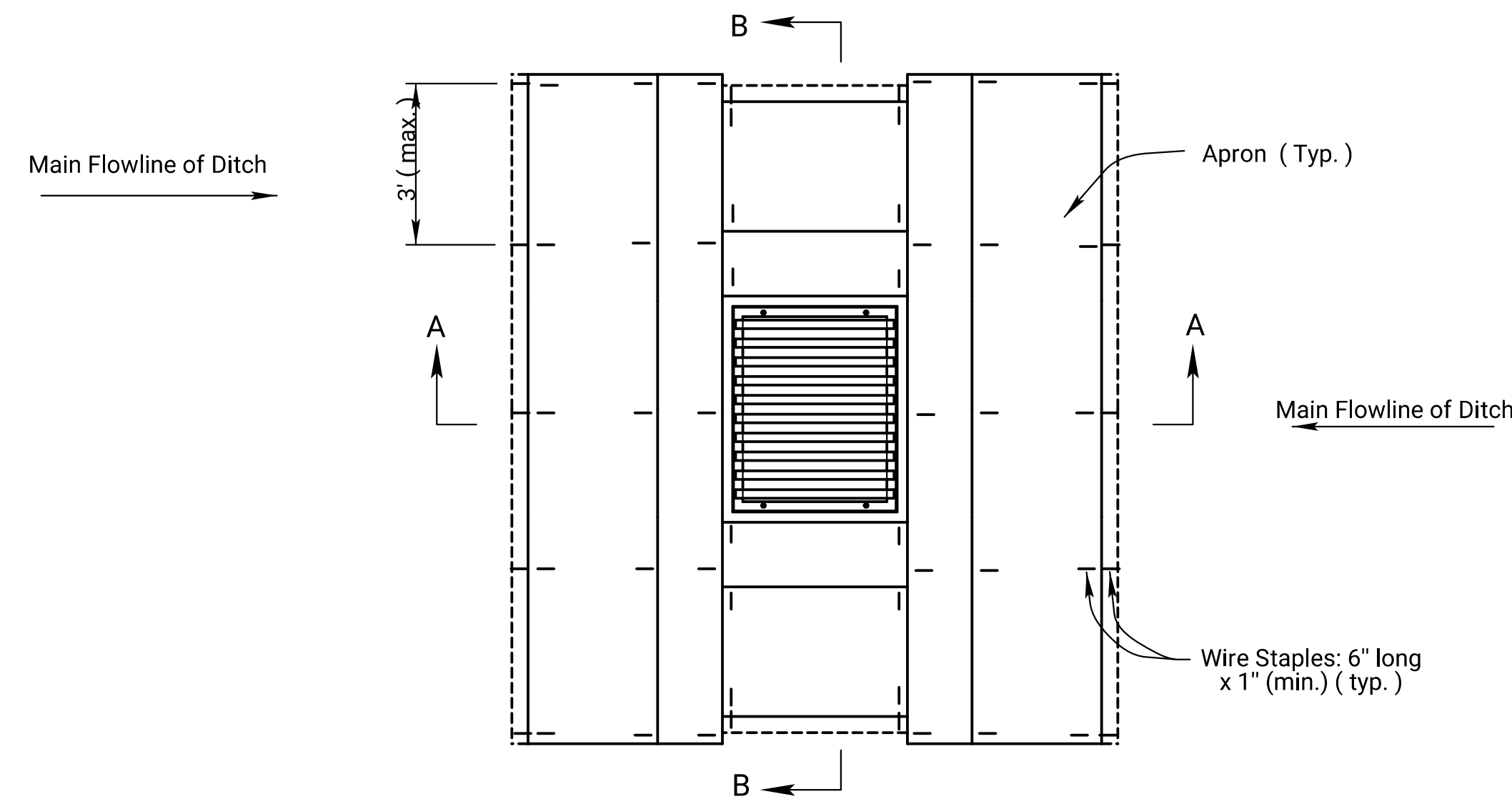
See KDOT Specifications for more information.

NO.	DATE	REVISIONS	BY	APPD
03	01-21-22	Temp Stream Crossing - Clean Aggregate Fill Note Added	M.R.D.	M.L.
02	08-24-21	Temp Stream Crossing - Clean Aggregate Fill Note Added	M.R.D.	M.L.
01	06-11-13	Revised Standard	M.R.M.	S.H.S.

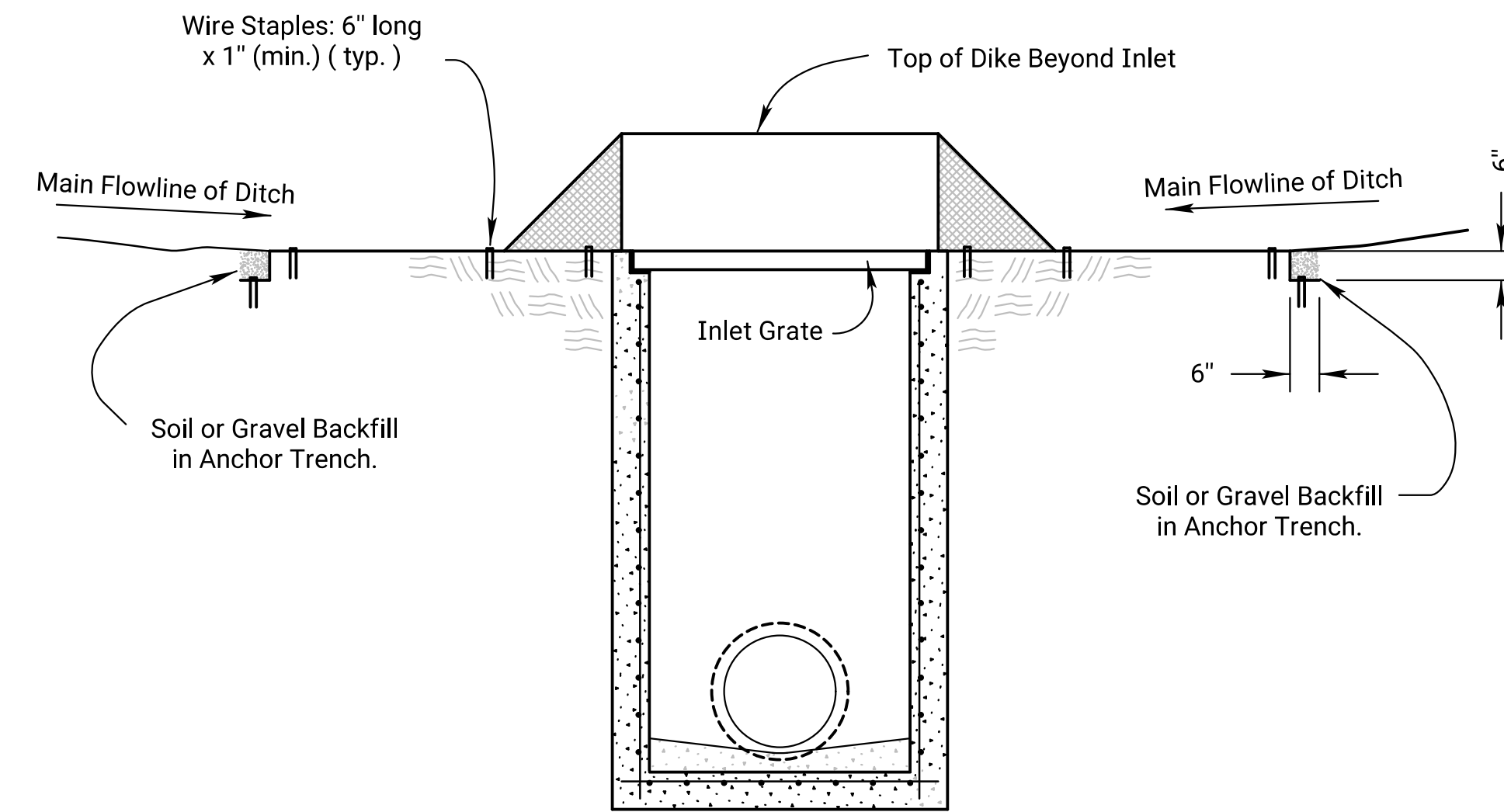
KANSAS DEPARTMENT OF TRANSPORTATION
TEMPORARY EROSION AND POLLUTION CONTROL
TEMPORARY SLOPE DRAIN, TEMPORARY STREAM CROSSING (AGGREGATE)
LA852B

DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

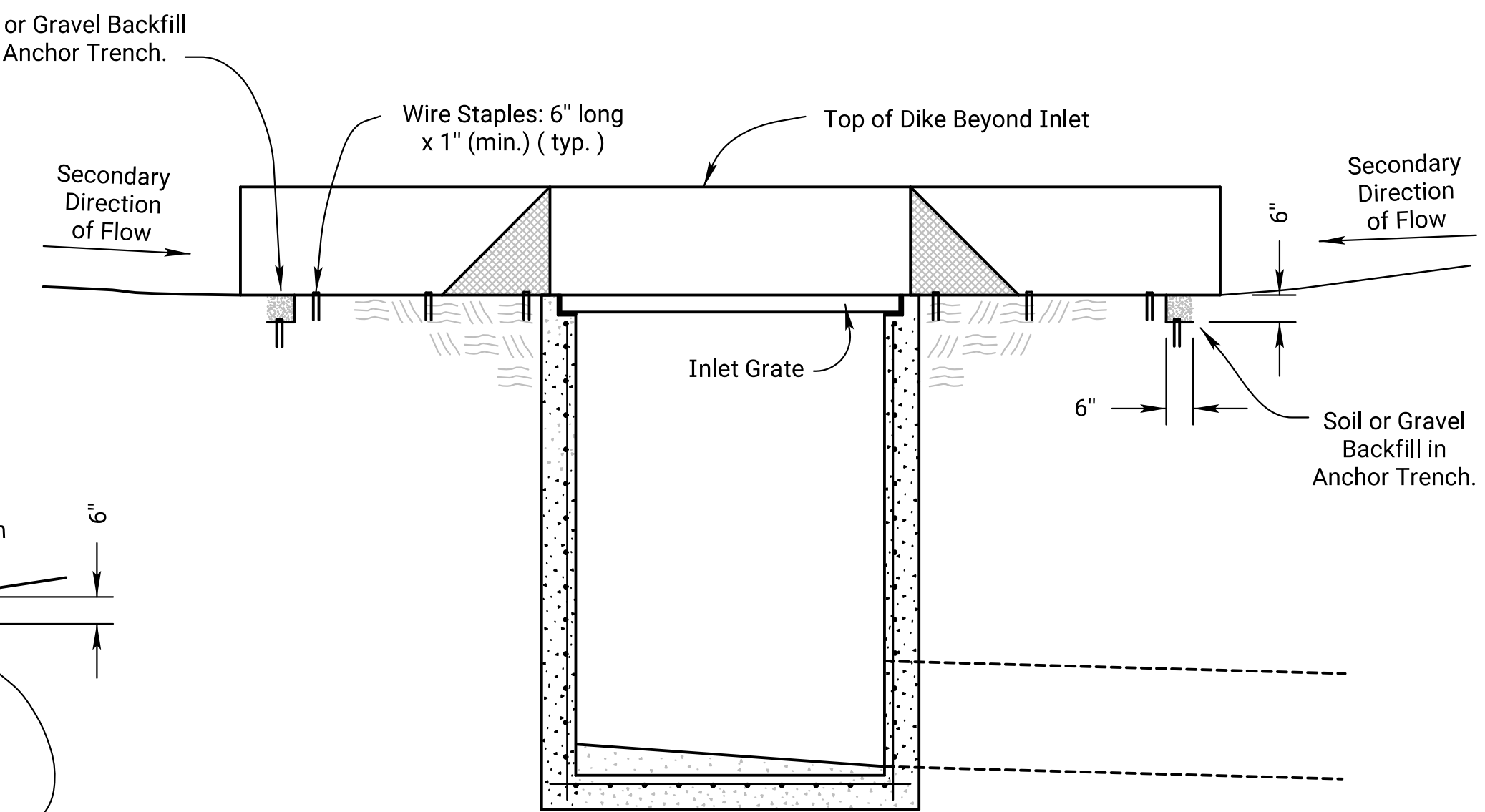
APPROVAL: 01-21-22, APPD. Mervin Lare



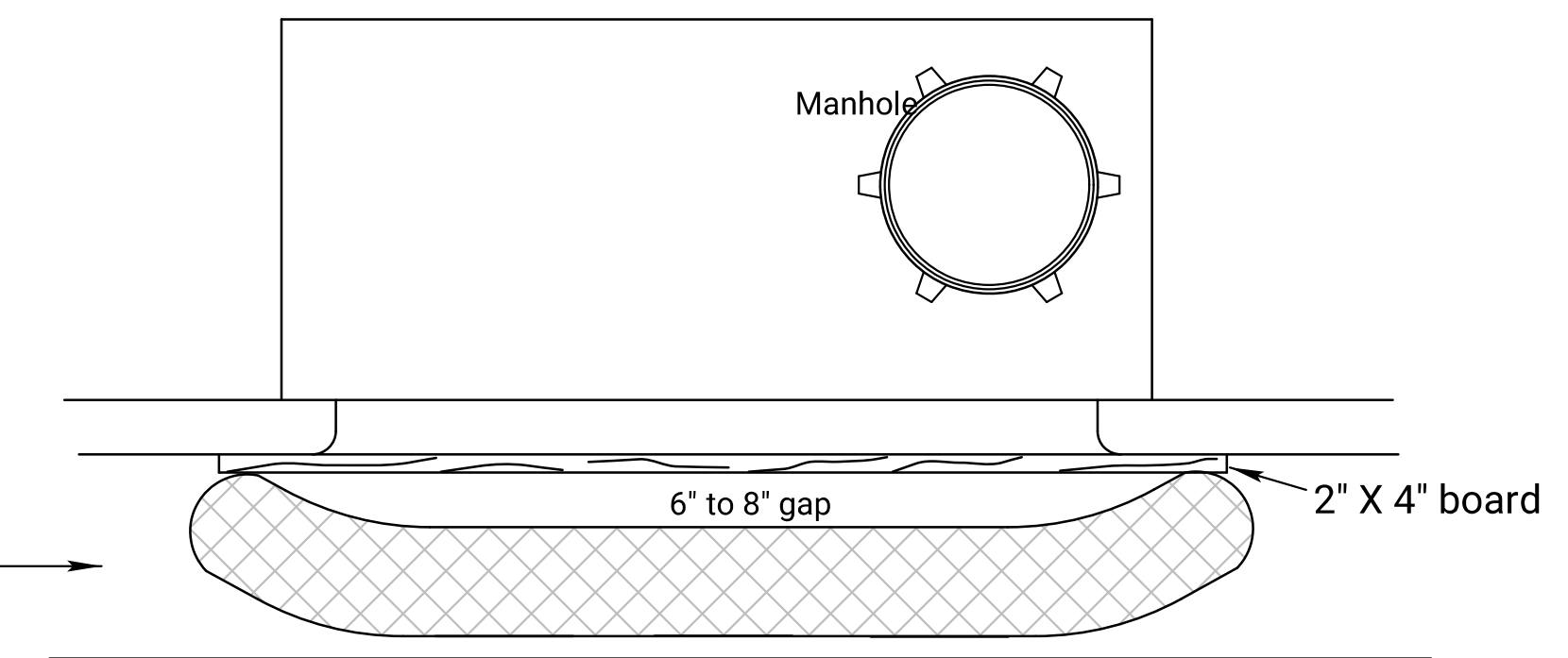
PLAN
TEMPORARY INLET SEDIMENT BARRIER
(TRIANGULAR SILT DIKE METHOD)
NO SCALE



SECTION A - A

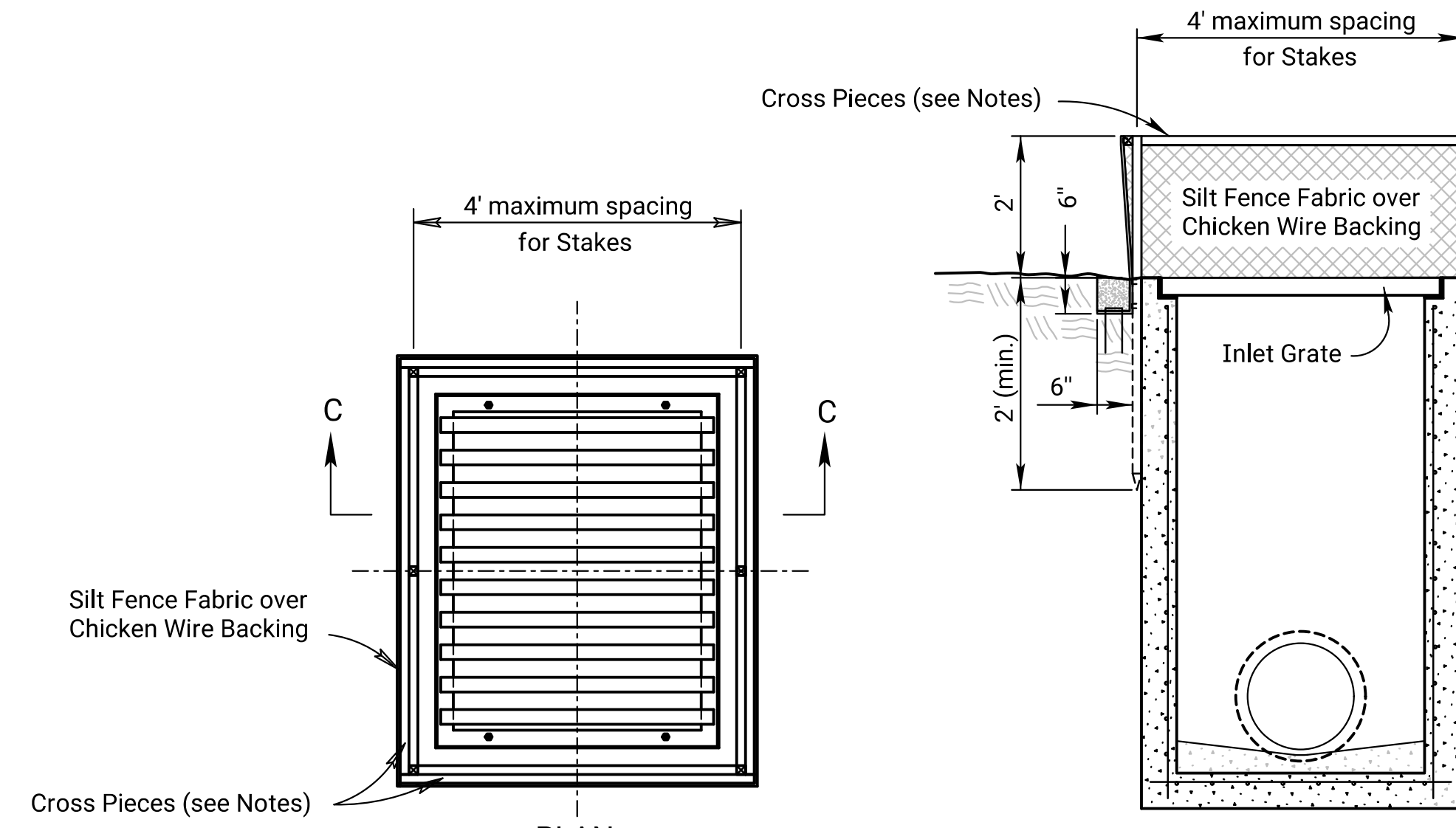


SECTION B - B



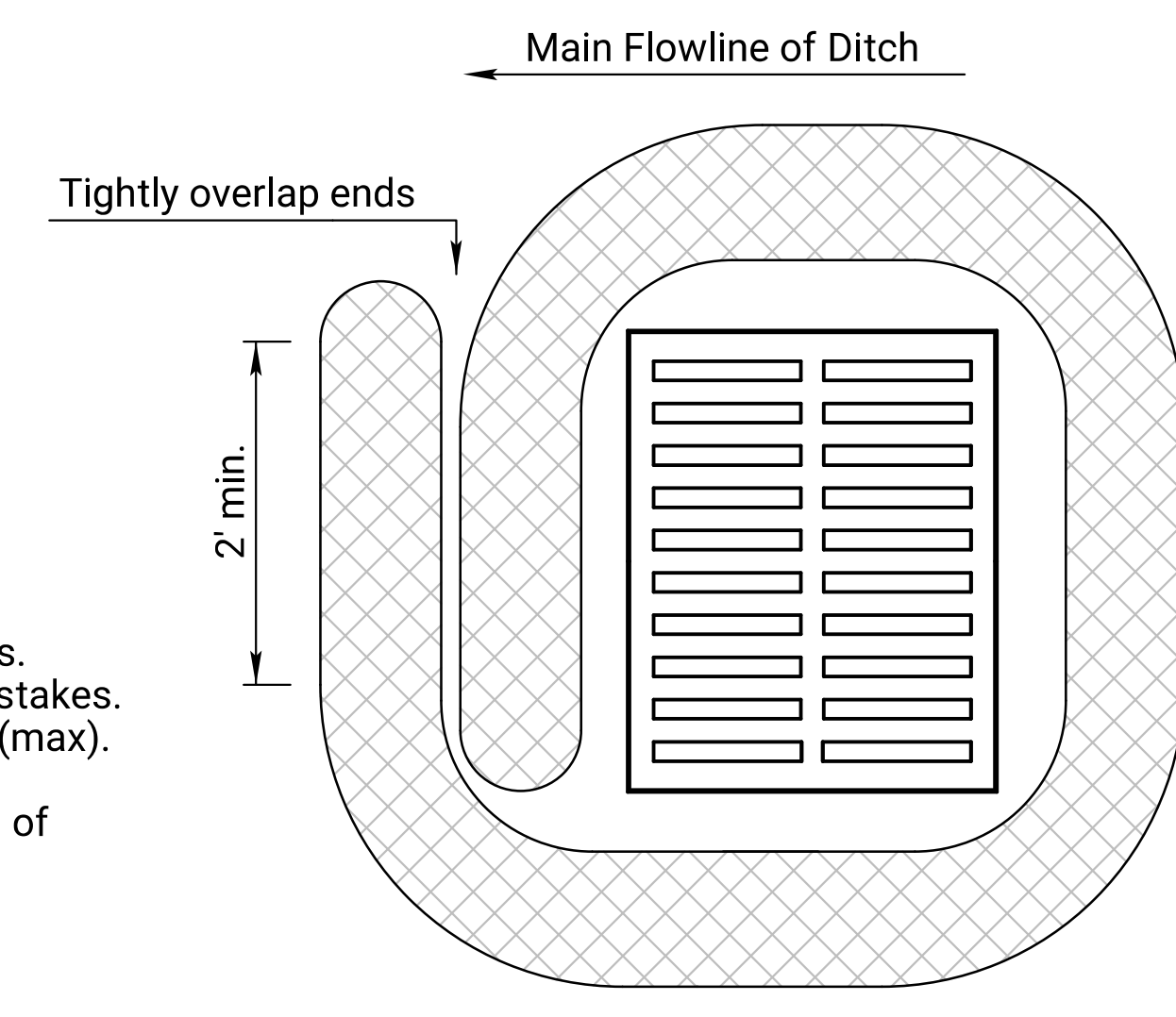
CURB INLET PROTECTION

1. If multiple gravel bags are required, place them in such a way that no gaps are evident.
2. Height of bags (8" minimum diameter) must not be above top of curb.
3. Alternative products may be used other than gravel bags such as the "Gutter Buddy". Products must be approved by the Engineer.
4. Curb inlet protection will be measured and paid for as Filter Sock.



PLAN
TEMPORARY INLET SEDIMENT BARRIER
(SILT FENCE METHOD)
NO SCALE

- SILT FENCE:**
1. Stakes shall be 4' (min.) long and of one of the following materials:
 - a. Hardwood - 1 3/4" x 1 3/4";
 - b. Southern Pine (No. 2) - 2 5/8" x 2 5/8";
 - c. Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
 - d. Synthetic - same strength as wood stakes.
 2. Cross pieces shall be of same material as stakes.
 3. Attach fence fabric securely on 6" centers (max).
 4. Use of high flow material is acceptable.
 5. Refer to plan sheets to estimate the length of silt fence required.



Drop inlet use
1'-6" TO 1'-8" diameter log
BIODEGRADABLE LOG/FILTER SOCK
DROP INLET PROTECTION

Note: 25% of log shall be keyed into ground during installation.
 Stake every 4'

Material Requirements	
Use 100% shredded mulch or other non-compost biodegradable material as fill for logs.	
No compost or fines.	
No hay or straw.	
Do not use material which prohibits water infiltration.	
Log Mesh:	
Use mesh with 1/4" openings or larger. Mesh must allow water infiltration but also hold fill material in place.	

NO.	DATE	REVISIONS	BY	APPD
03	09-26-19	Changed Direction of Main Flowline of Ditch Arrow	M.R.D.	S.H.S.
02	03-10-15	Revised Standard	R.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION
TEMPORARY EROSION AND POLLUTION CONTROL, TEMPORARY INLET SEDIMENT BARRIER (SILT FENCE)
TEMP. INLET SEDIMENT BARRIER (T.S.D.)
LA852C

DESIGNED	R.A.	DETAILED	R.A.	QUANTITIES	TRACED
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.	TRACE CK.

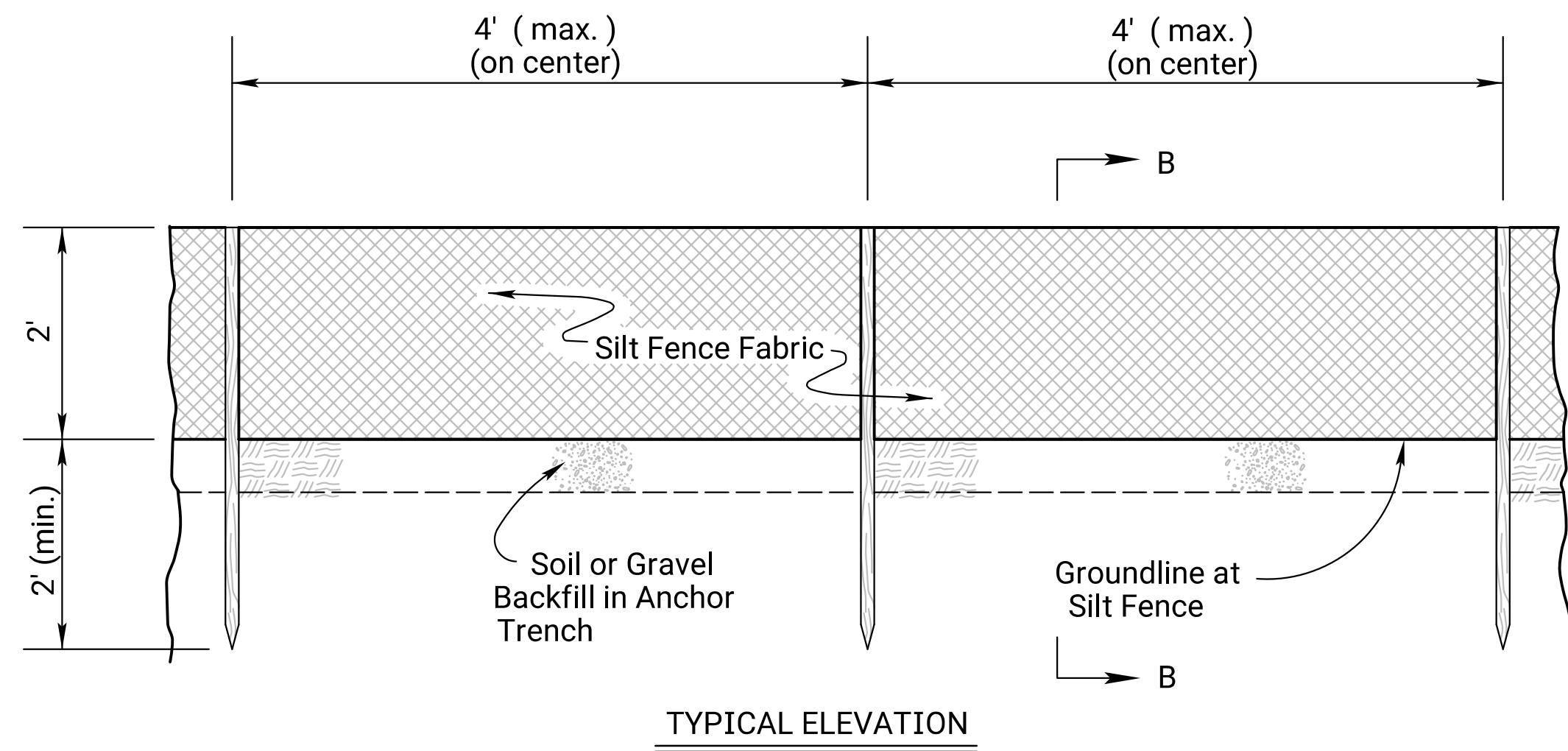
INSTALLATION NOTES

SILT FENCE:

- Stakes shall be 4' (min.) long and of one of the following materials:
 - Hardwood - 1 3/16" x 1 3/16";
 - Southern Pine (No. 2) - 2 5/8" x 2 5/8";
 - Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
 - Synthetic - same strength as wood stakes.
- Attach fence fabric with 3 zip ties within the top 8" of the fence. Alternate attachment methods may be approved by the Engineer on a performance basis.
- Use of high flow material is acceptable.
- Refer to plan sheets to estimate the length of silt fence required.

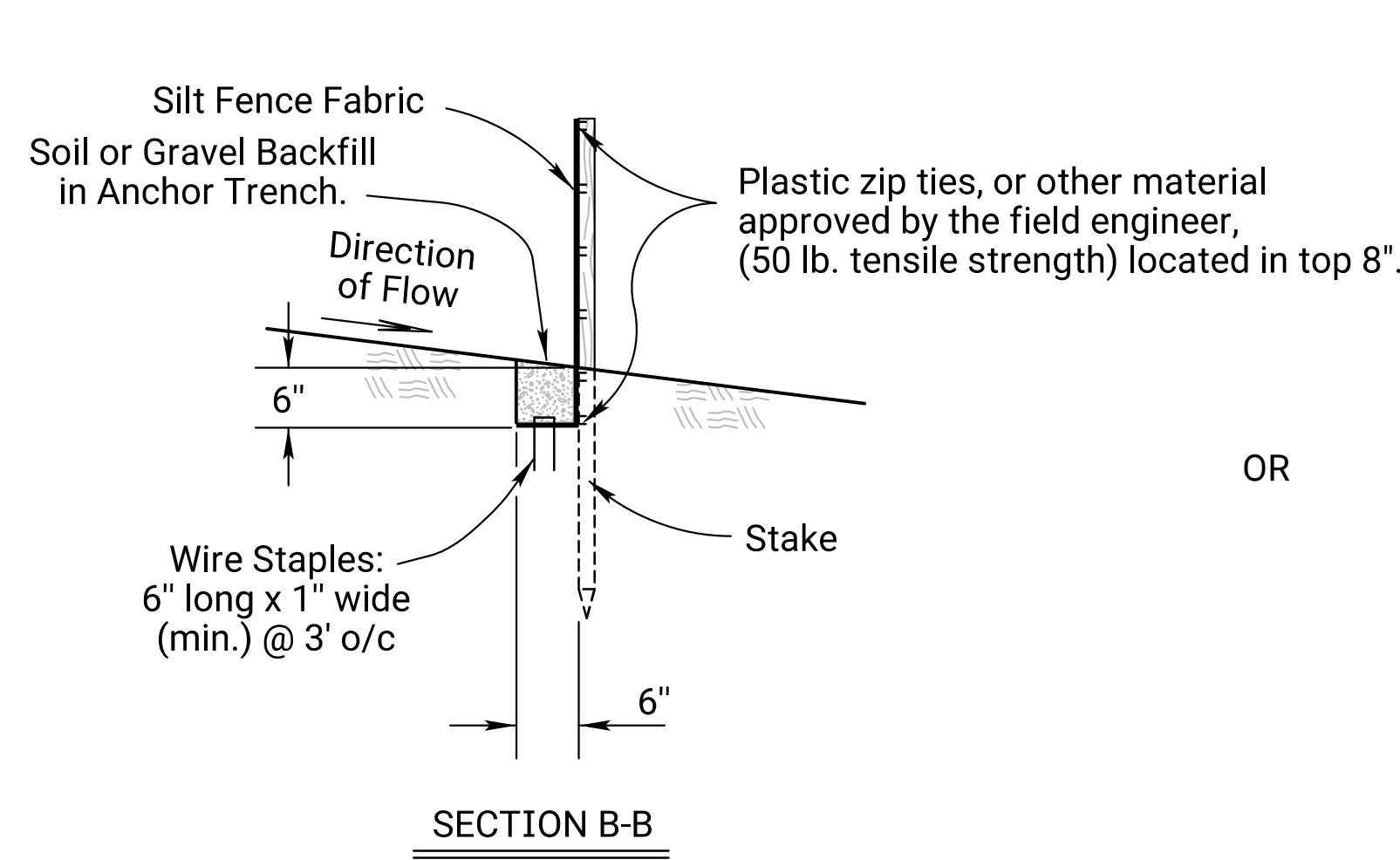
BIODEGRADABLE LOG OR FILTER SOCK

- Place biodegradable logs or filter sock tightly together minimum overlap of 18".
- Wood stakes shall be 2" x 2" (nom.).
- Refer to plan sheets to estimate length of biodegradable log and filter sock required.
- Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.
- Length of stakes should be 2 times the height of the log at a minimum with minimum ground embedment equal to the height of the log / sock.

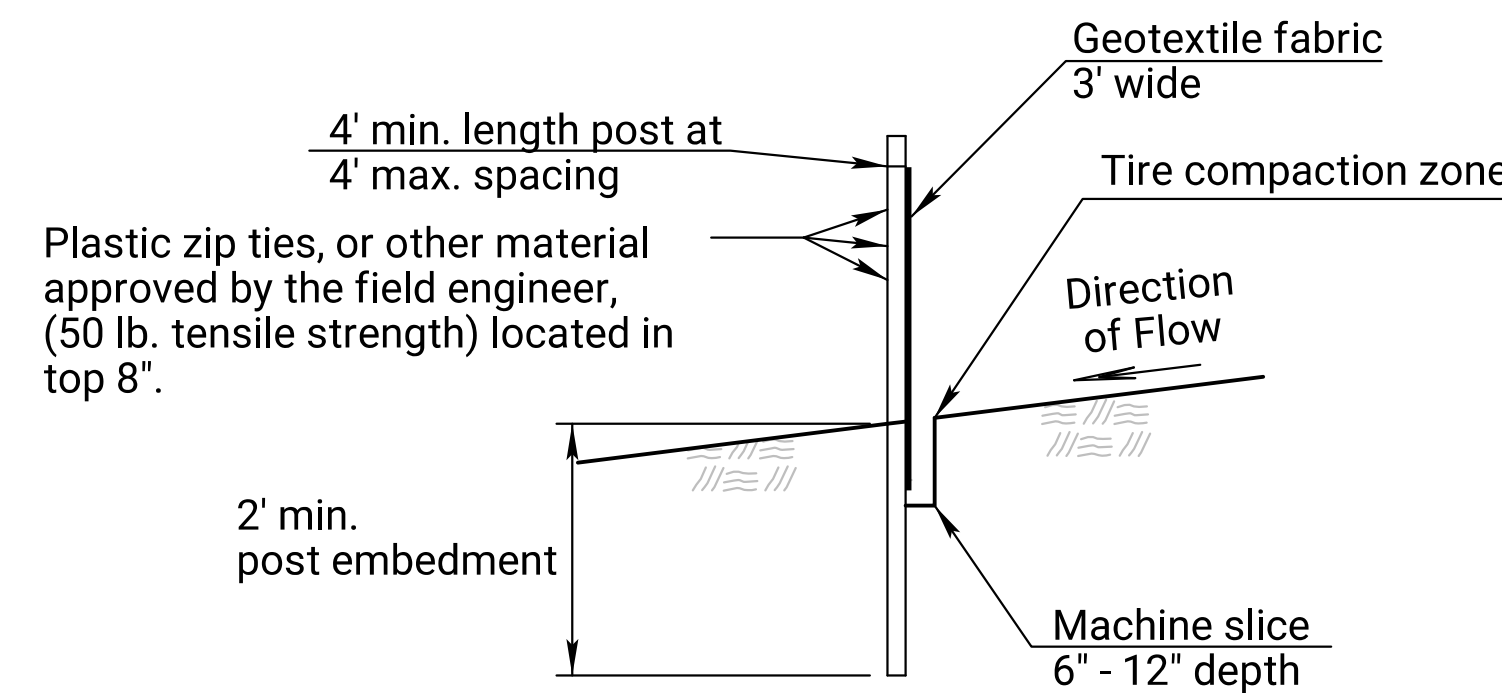


TYPICAL ELEVATION

SILT FENCE BARRIER
NO SCALE



SECTION B-B



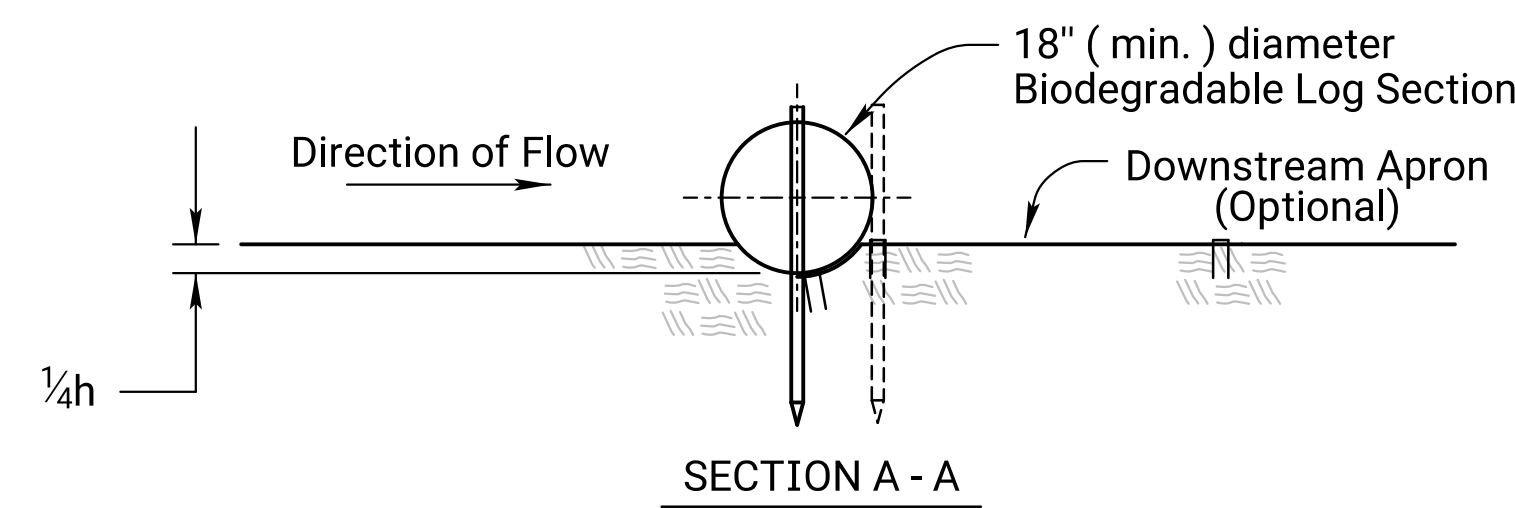
SECTION B-B

Biodegradable Log or Filter Sock Slope Interruptions

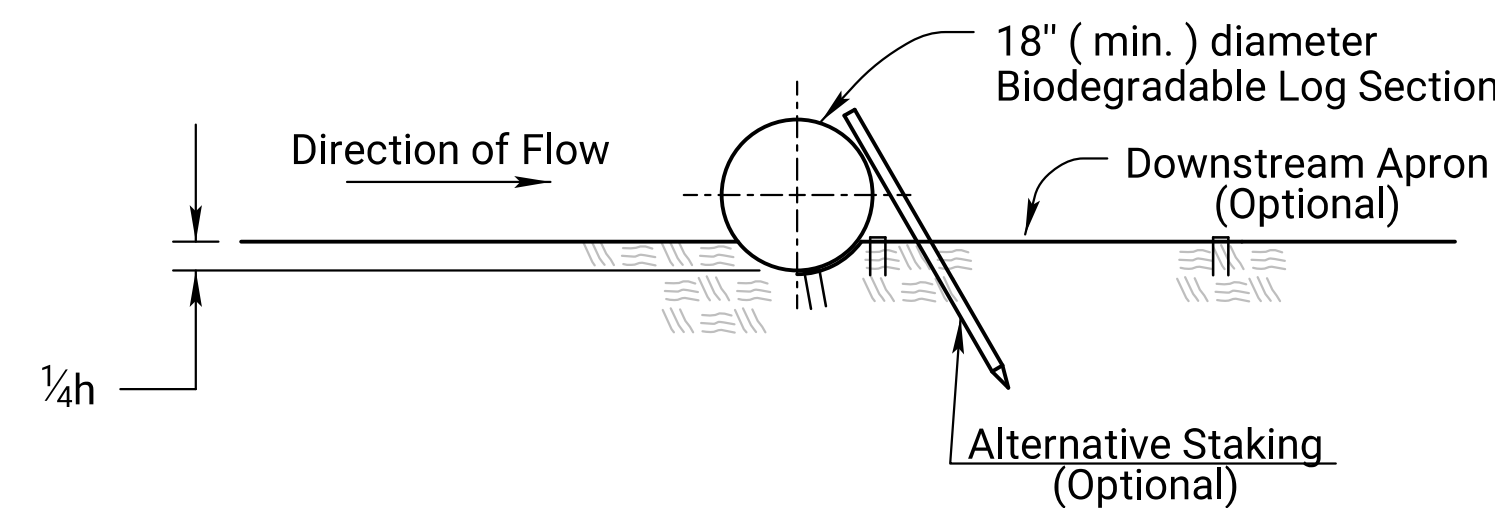
		PRODUCT		
		9" Sediment Log or 8" Filter Sock (ft)	12" Sediment Log or 12" Filter Sock (ft)	20" Sediment Log or 18" Filter Sock (ft)
Slope Gradient	≤4H:1V	40	60	80
	3H:1V	30	45	60

	BIODEGRADABLE LOG MATERIAL	
	LOW FLOW	HIGH FLOW
9"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
12"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
18"-20"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber

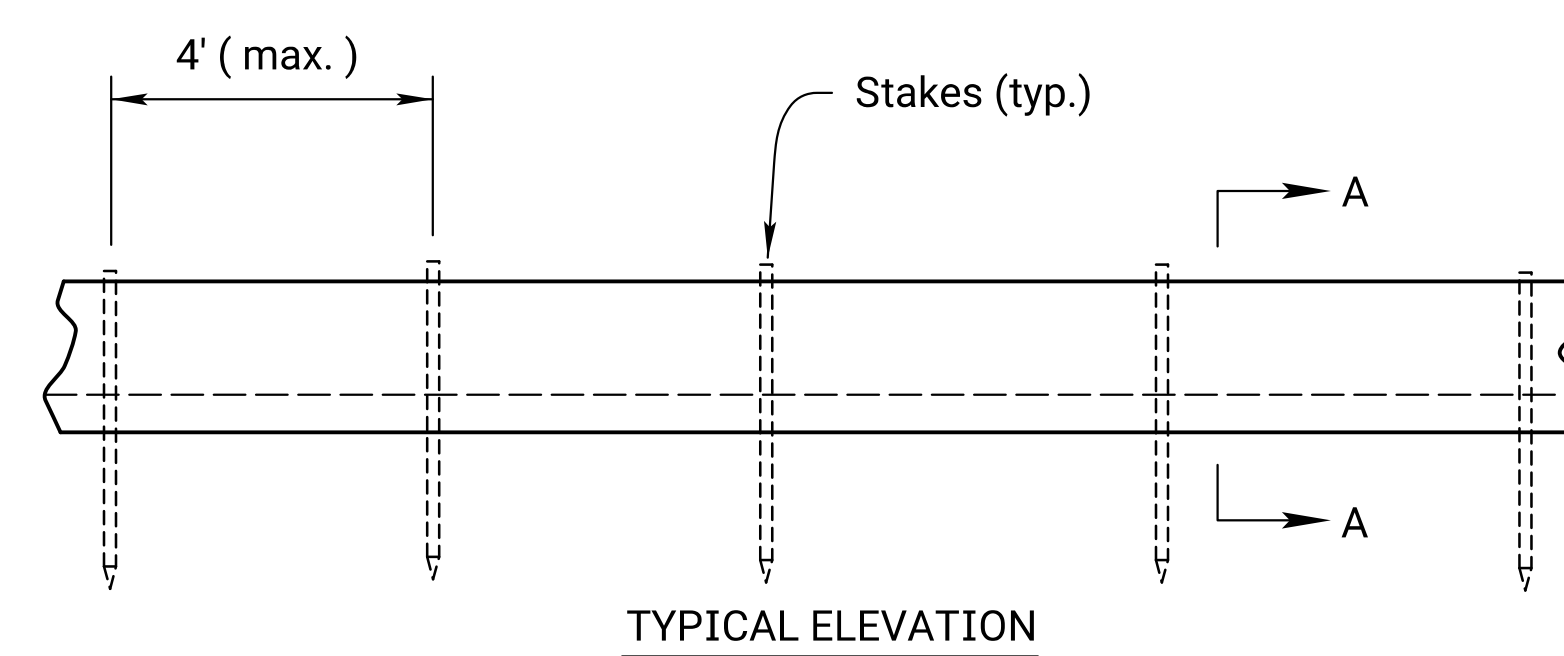
Deviations should be approved by the Field Engineer.



SECTION A - A



ALT. DETAIL
OPTIONAL



TYPICAL ELEVATION

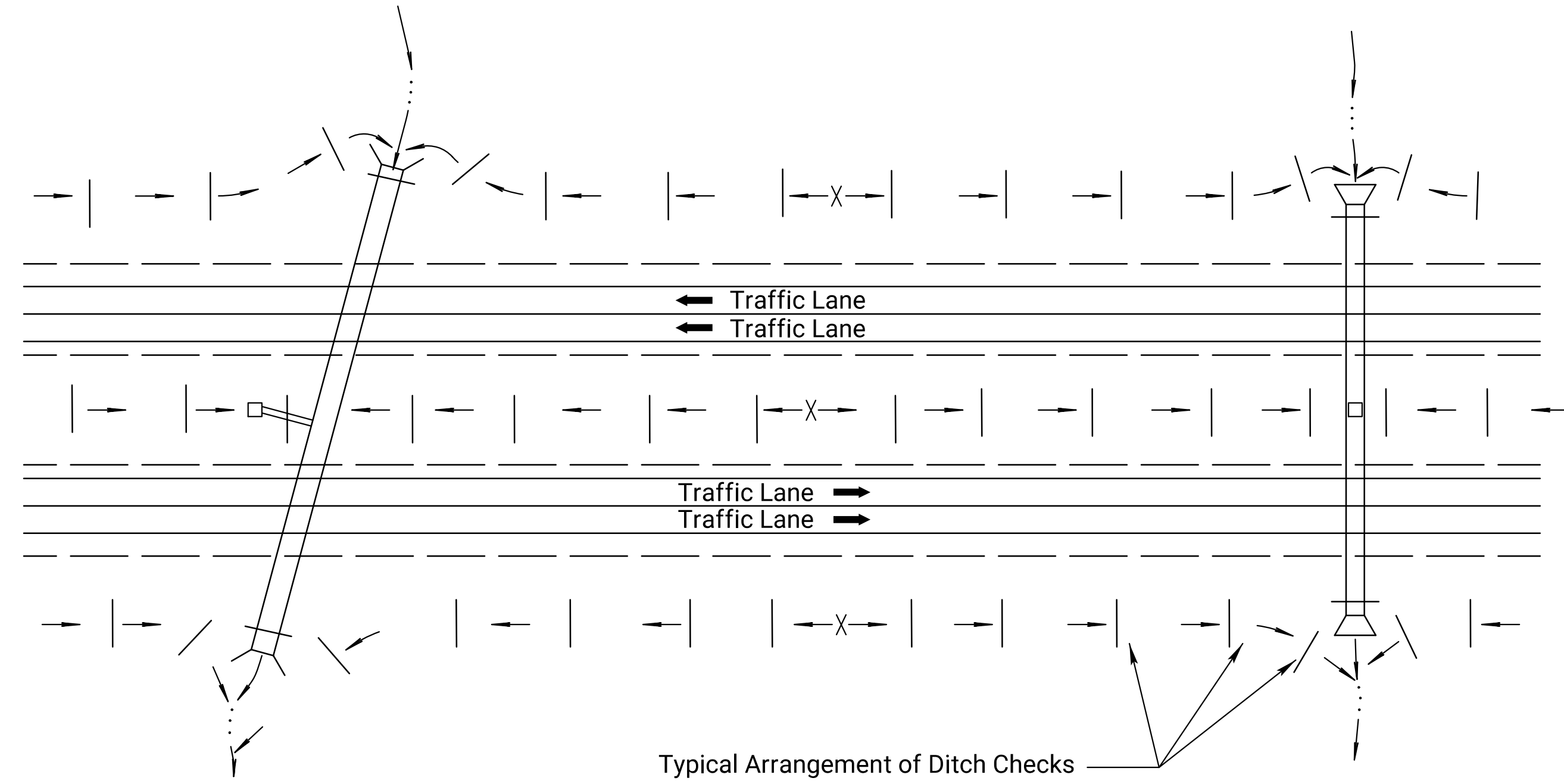
BIODEGRADABLE LOG SLOPE INTERRUPTIONS
OR Filter Sock

GENERAL NOTES

- Slope interruptions shall be placed along contour lines, with a short section turned upgrade at each end of the barrier.
- The maximum length of the slope interruptions shall not exceed 250 feet, and the barrier ends need to be staggered.
- Interruptions damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired immediately by Contractor at no additional cost to KDOT.
- Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

NO.	DATE	REVISIONS	BY	APPD
03	06-28-16	Revised Standard	R.A.	S.H.S.
02	03-01-15	Revised Standard	R.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL SLOPE INTERRUPTIONS BIODEGRADABLE LOG / SILT FENCE				
LA852D				
DESIGNED	S.H.S.	DETAILED	R.A.	QUANTITIES
DESIGN CK.	S.H.S.	DETAIL CK.		QUAN. CK.
APPD.			Scott H. Shields	TRACED
APPD.				TRACE CK.



TYPICAL DITCH CHECK LAYOUT PLAN
NO SCALE

20" BIOLOG CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
1.0	125
2.0	60
3.0	40
4.0	30
5.0	25

NOTE: Use this spacing for all except Rock Ditch Checks.

18" FILTER SOCK CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
1.0	110
2.0	55
3.0	35
4.0	25
5.0	20

NOTE: Use this spacing for all except Rock Ditch Checks.

GENERAL NOTES

- 1) The choice of ditch check methods is at the option of the Contractor.
- 2) Use only rock checks in situations where the ditch slope is 6 percent or greater.
- 2) Ditch checks damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired by Contractor at no extra cost to KDOT.

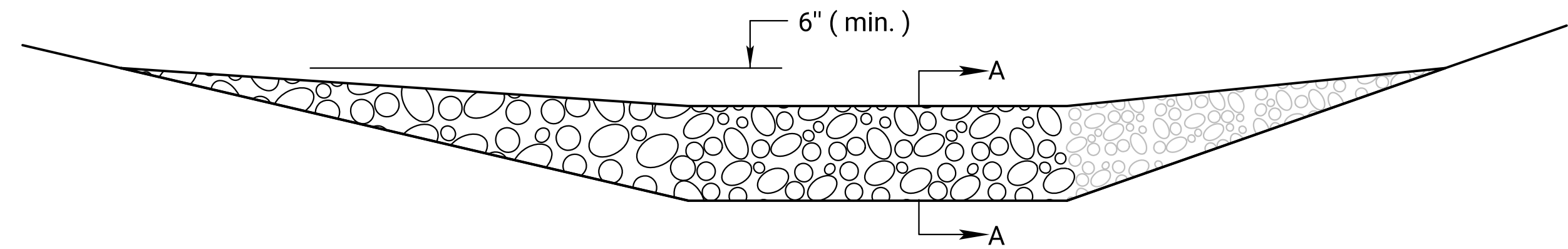
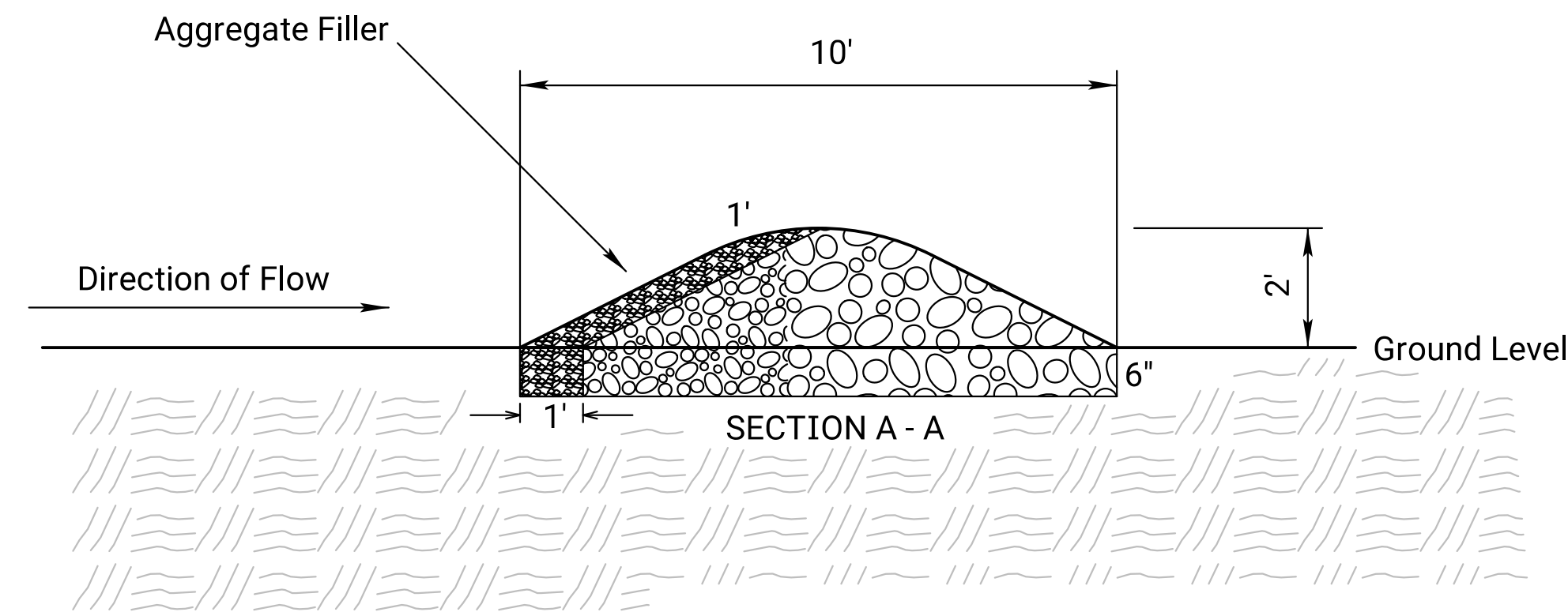
NO.	DATE	REVISIONS	BY	APPD
03	08-10-16	Revised Standard	R.A.A.	S.H.S.
02	06-28-16	Revised Standard	R.A.A.	S.H.S.
01	06-01-13	Revised Standard	M.R.M.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY EROSION AND POLLUTION CONTROL DITCH CHECKS

LA852E

DESIGNED	S.H.S.	DETAILED	R.A.A.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.	TRACE CK.	S.H.S.



TYPICAL ELEVATION

ROCK DITCH CHECK

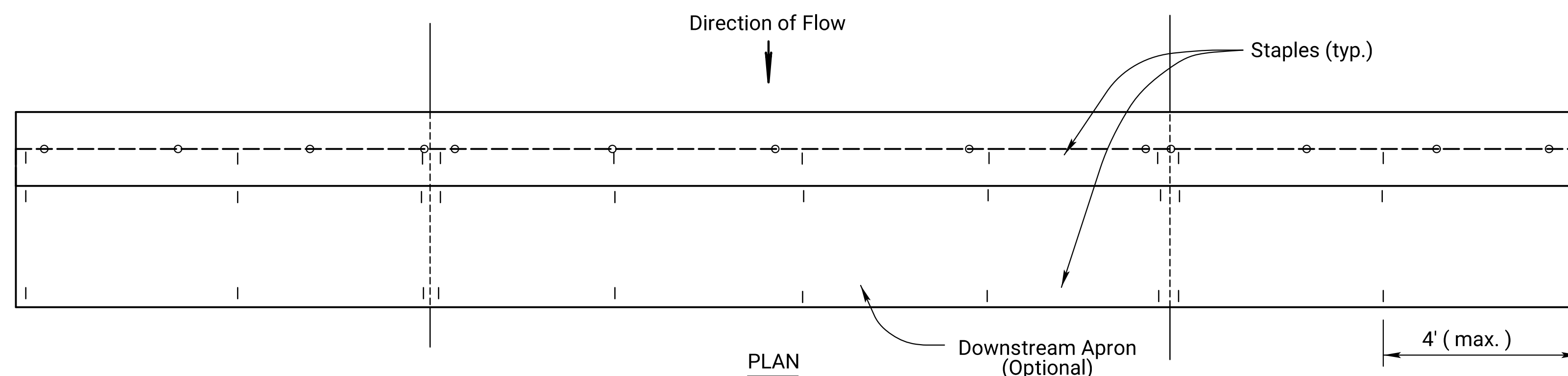
NO SCALE

TEMPORARY ROCK DITCH CHECK SPACING	
DITCH @ SLOPE (%)	SPACING INTERVAL (FEET)
5.0	60
6.0	50
7.0	43
8.0	36
9.0	33
10.0	29

NOTE: Use this spacing for Rock Ditch Checks only.

ROCK DITCH CHECK NOTES

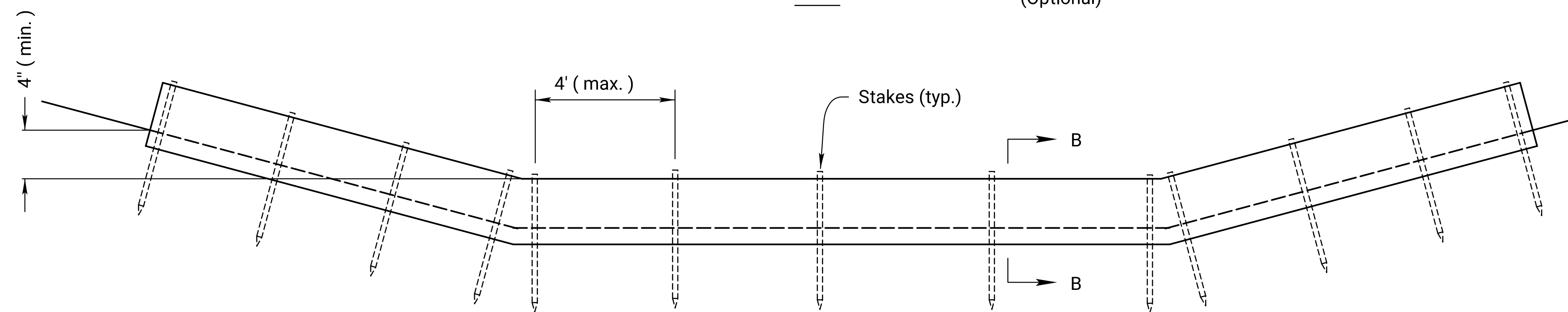
1. Rock shall be clean aggregate, D50-6" and aggregate filler.
2. Place rock in such manner that water will flow over, not around ditch check.
3. Do not use rock ditch checks in clear zone.
4. Excavation: The ditch area shall be reshaped to fill any eroded areas. Prior to placement of the rock, the ditch shall be excavated to the dimensions of the Rock Ditch Check and to a minimum depth of 6" (150mm). After placement of the rock, backfill and compact any over-excavated soil to ditch grade. This work shall be subsidiary to the bid item Temporary Ditch Check (Rock).
5. Aggregate excavated on site may be used as an alternate to the 6" rock, if approved by the Engineer.
6. The Engineer may approve the use of larger aggregates for the downstream portion of the check when conditions warrant their use.
7. When the use of larger rock is approved, D50-6" rock will be placed between the larger aggregate and the aggregate filler.
8. Aggregate filler will be placed on the upstream face of the ditch check. Aggregate filler will comply with Filter Course Type I, Division 1114.



PLAN

Downstream Apron (Optional)

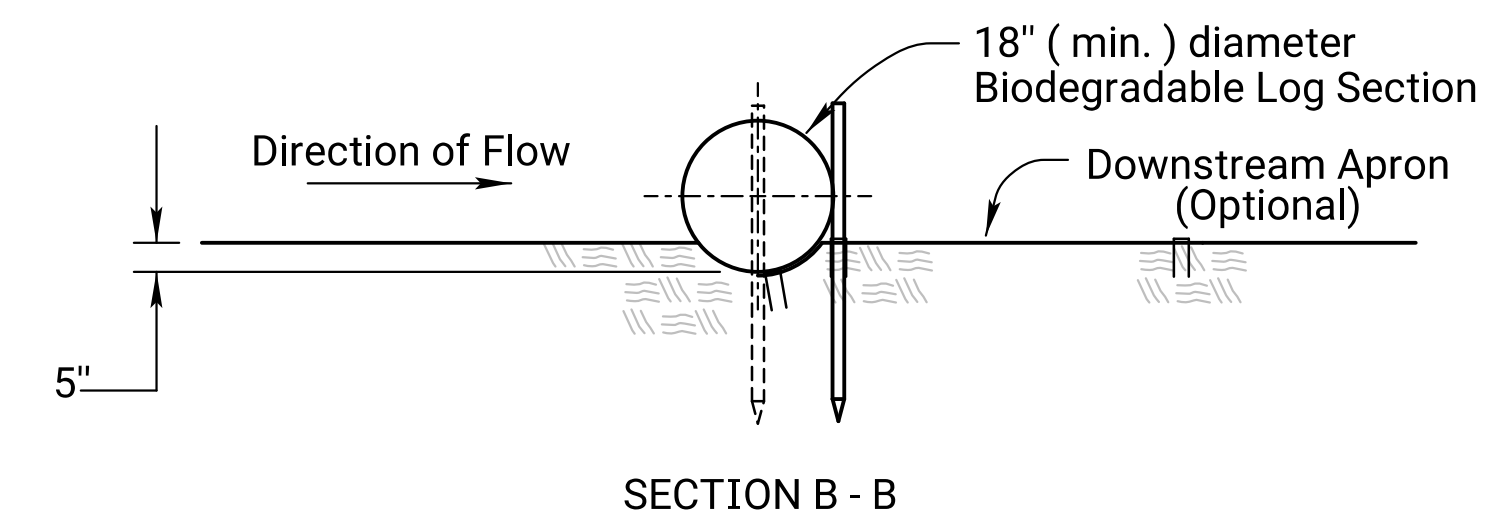
4' (max.)



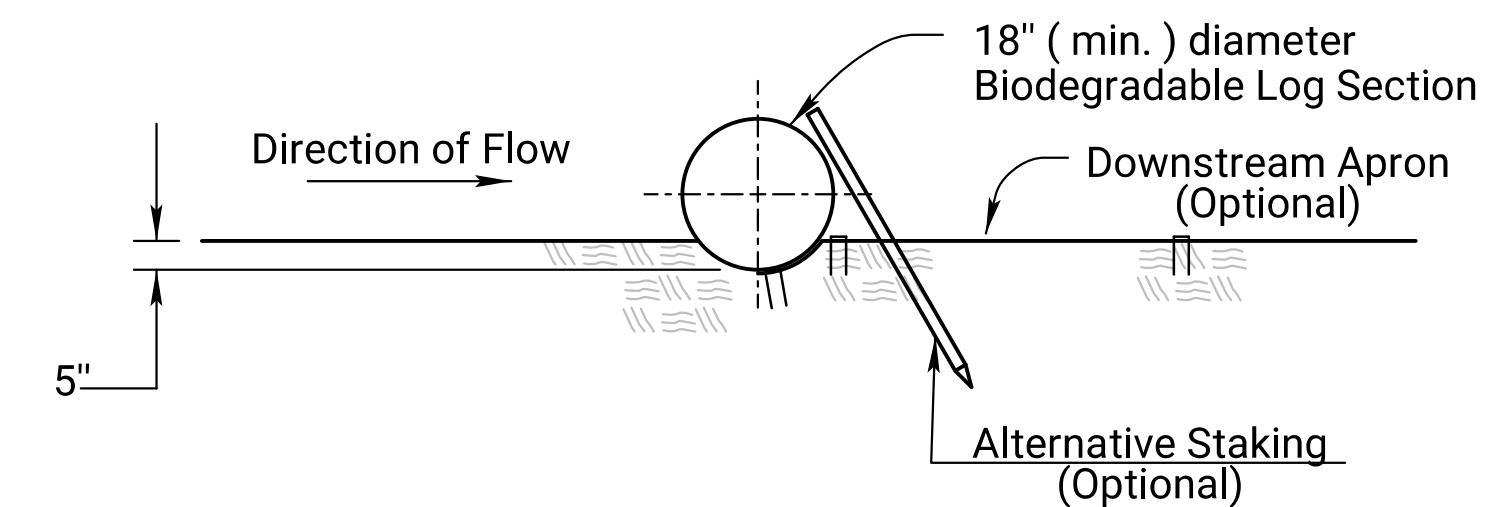
TYPICAL ELEVATION

BIODEGRADABLE LOG DITCH CHECK

OR Filter Sock Ditch Check
NO SCALE



SECTION B - B



ALT. DETAIL OPTIONAL

BIODEGRADABLE LOG DITCH CHECK NOTES

1. Use as many biodegradable log sections as necessary to ensure water does not flow around end of ditch check.
2. Overlap sections a minimum of 18".
3. Stakes shall be wood or steel according to Section 2114 of the Standard Specifications. Length of stakes shall be a minimum of 2 x the diameter of the log.
4. Use Erosion Control (Class 1) (Type C) as the downstream apron when required.
5. A downstream apron is required when directed by the Engineer. Apron material will be paid at the contract unit price.
6. Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.

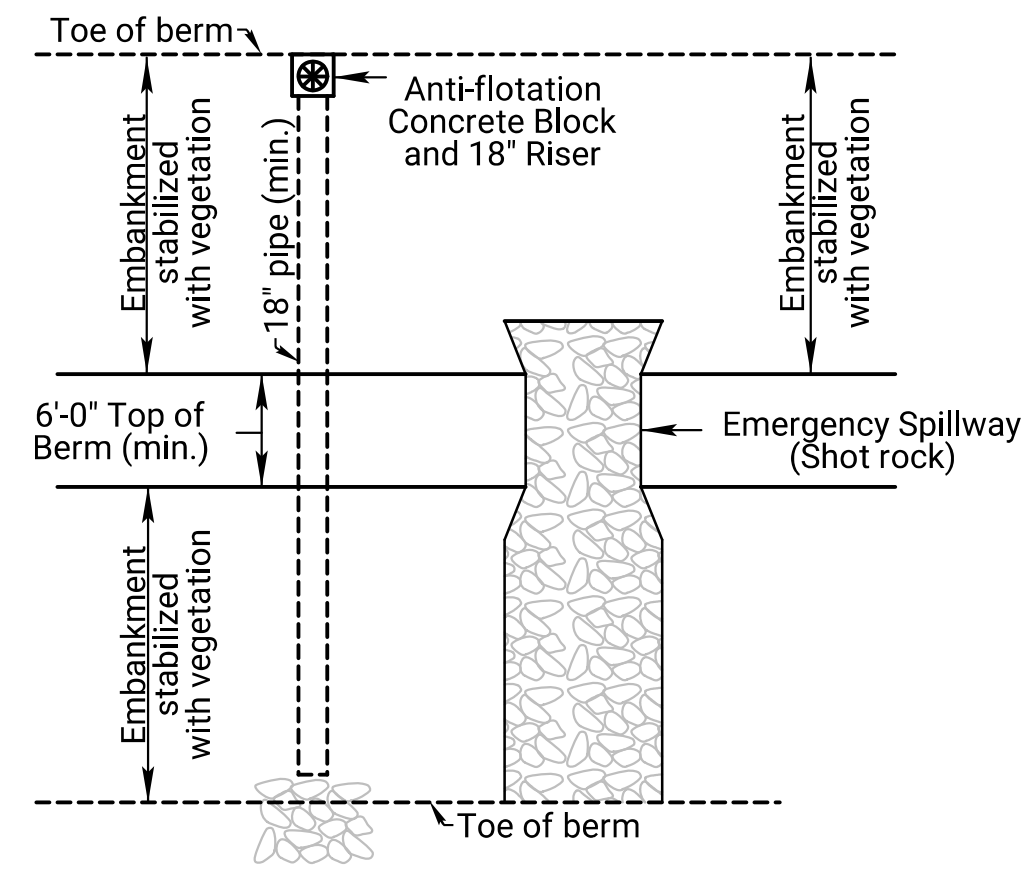
NO.	DATE	REVISIONS	BY	APPD
03	11-19-20	Revised Standard	M.R.D.	M.L.
02	08-10-16	Revised Standard	R.A.A.	S.H.S.
01	10-21-15	Revised Standard	R.A.A.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION
Mervin Lare

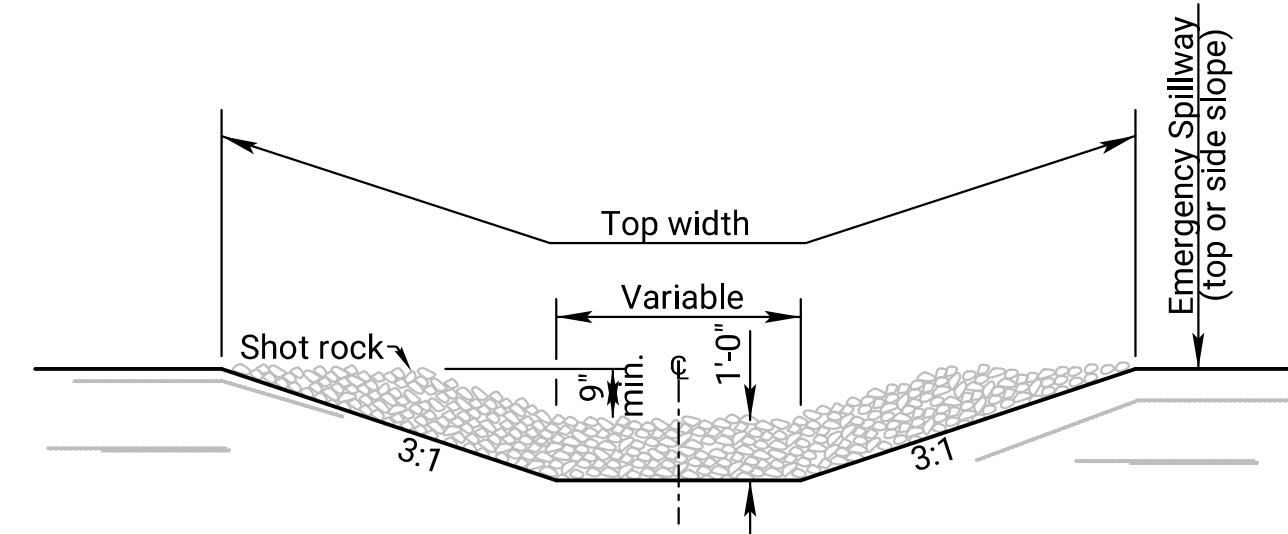
DESIGNED	M.L.	DETAILED	D.K.	QUANTITIES	TRACED	R.A.A.
DESIGN CK.	M.L.	DETAIL CK.	M.L.	QUAN. CK.	TRACE CK.	R.A.A.

TEMPORARY EROSION AND POLLUTION CONTROL
ROCK DITCH CHECKS
BIODEGRADABLE LOG DITCH CHECKS
LA852G

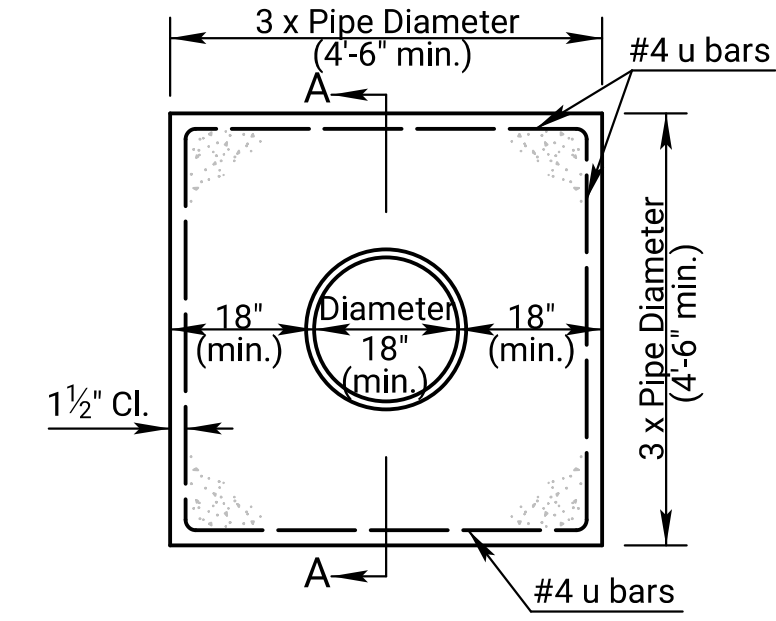
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	32	44



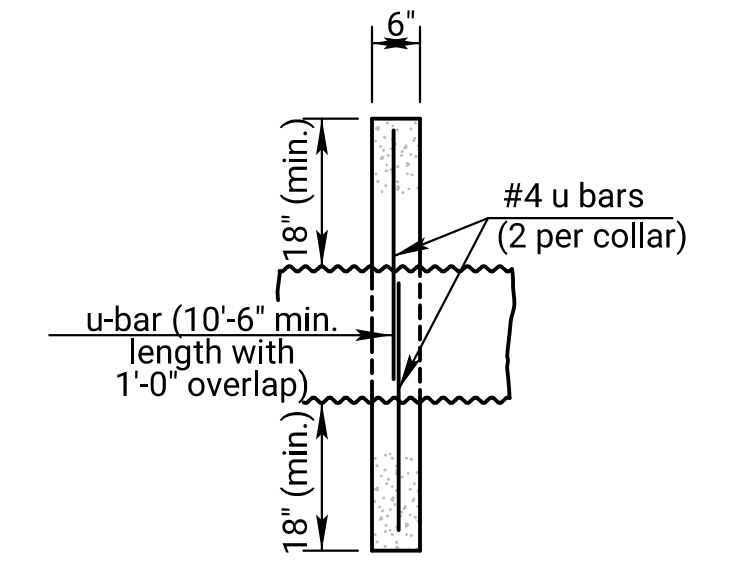
SEDIMENT STORAGE BASIN (PLAN)



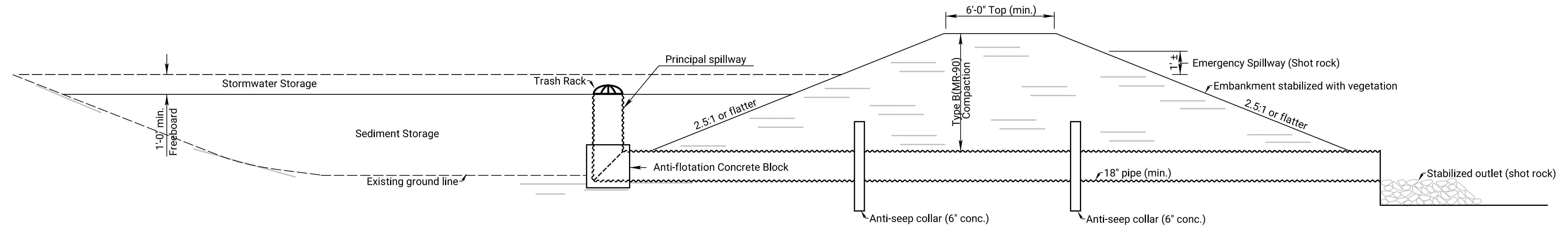
CROSS SECTION (EMERGENCY SPILLWAY)



CONCRETE ANTI-SEEP COLLAR



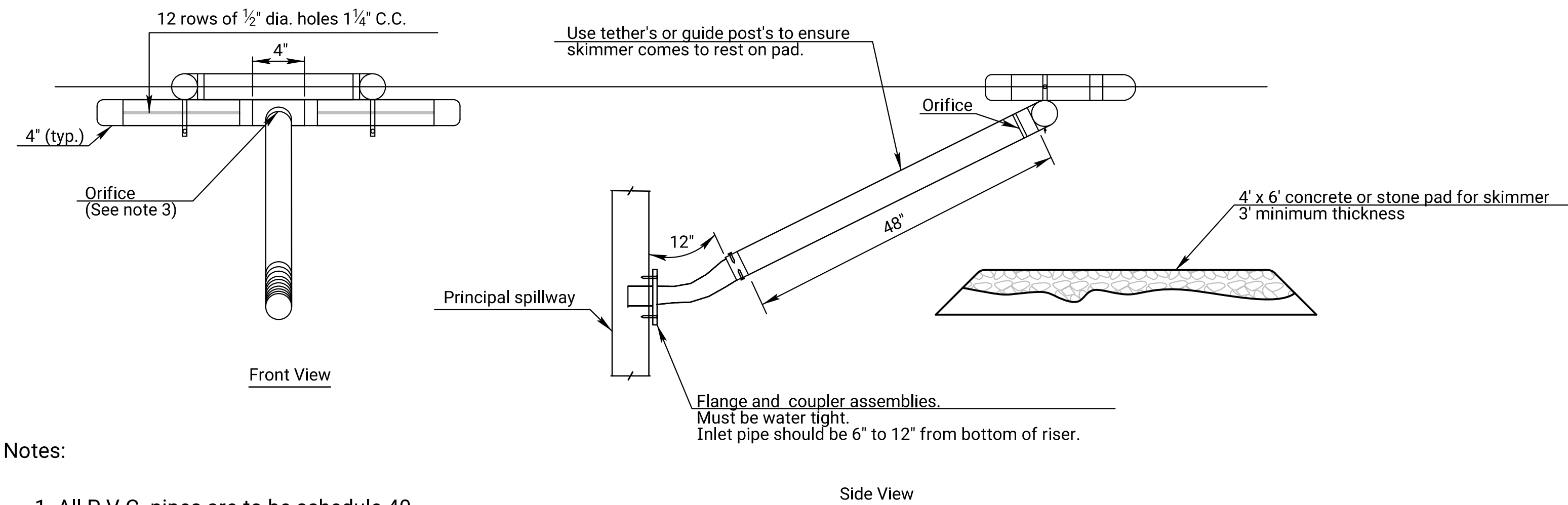
SECTION A-A



SEDIMENT STORAGE BASIN (ELEVATION)

NOTES:

- 1) Temporary Sediment Basins shall be constructed at locations as directed by the Engineer or as approved in the SWPPP Schedule. All work and materials necessary, including but not limited to, the fill material, compaction, drainage pipes, aggregates and all other incidentals necessary to construct the basin, shall be paid as "Temporary Sediment Basin".
- 2) Lengths and top dimensions shall be determined in the field by the Engineer.
- 3) Skimmer dewatering device required and must be used regardless the size of the drainage area.



SKIMMER DEWATERING DEVICE

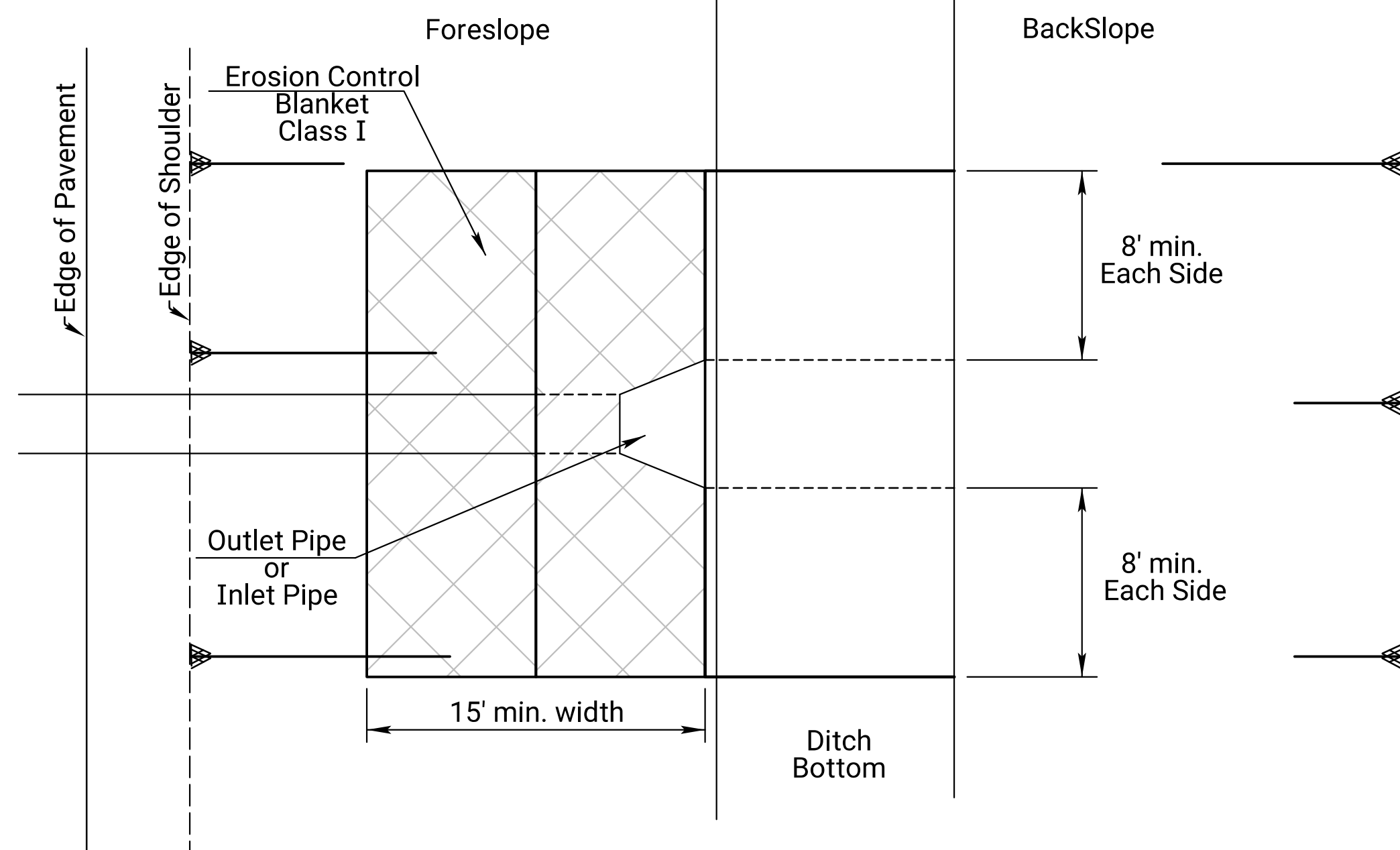
Notes:

1. All P.V.C. pipes are to be schedule 40.
2. HDPE flexible drain pipes is to be attached to the pond outlet structure with water-tight connections.
3. The orifice shall be sized of to provide drawdown time to 2 to 5 days and approved by the engineer.
4. Other skimmer designs maybe used that dewateres from the surface at a controlled rate. The design must be approved by the engineer.

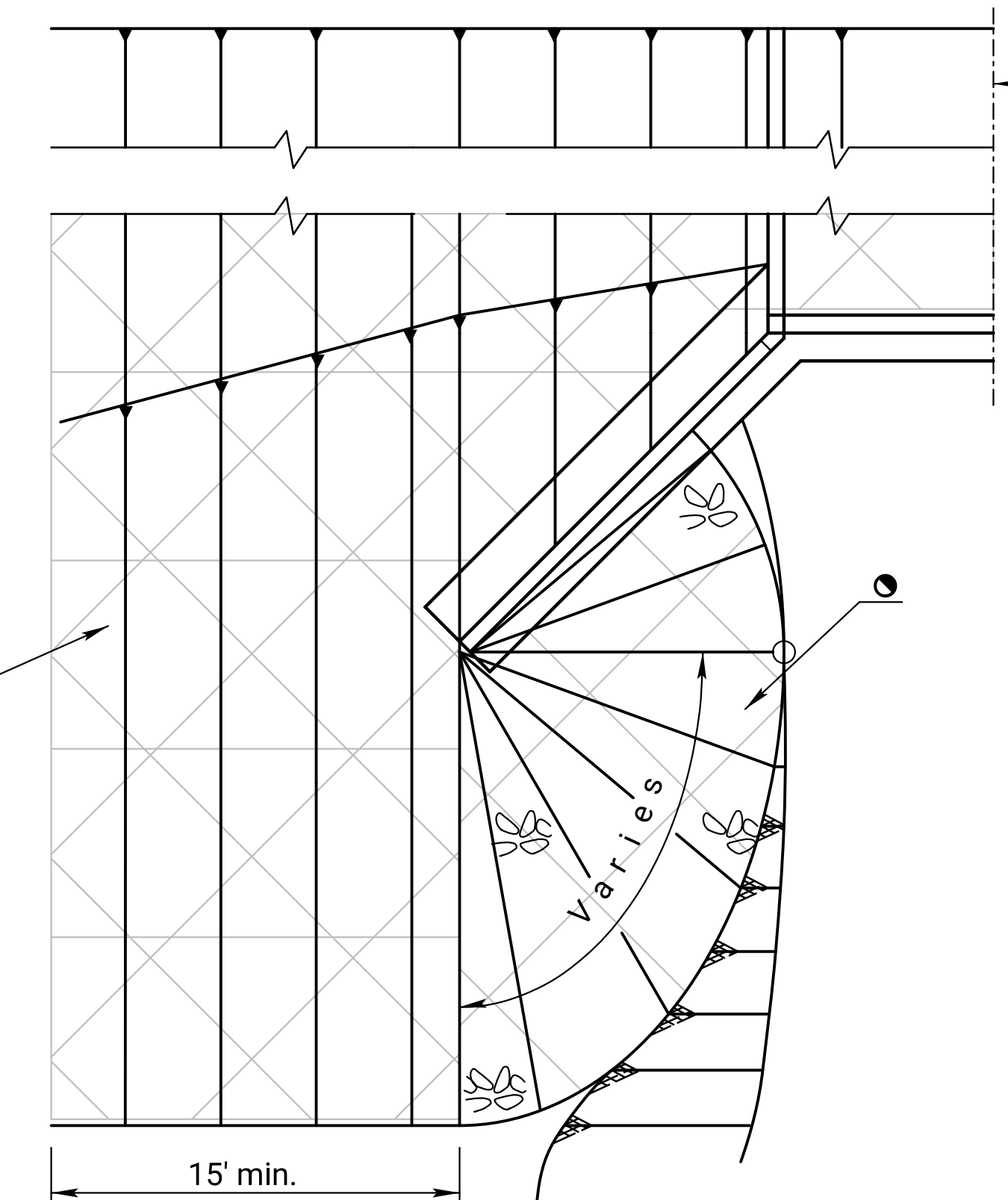
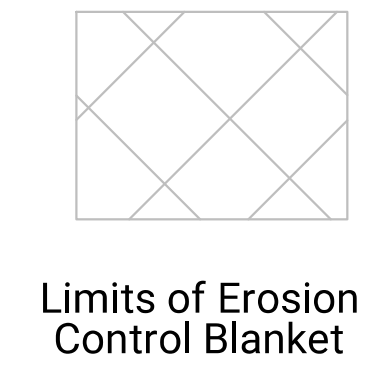
SEDIMENT STORAGE BASIN LOCATIONS		
STATION TO STATION	SIDE	REQUIRED STORAGE CAPACITY

02	09-03-13	Added Skimmer Dewatering Device	M.R.M.	S.H.S.
01	07-17-13	Revised Standard	M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL SEDIMENT STORAGE BASIN				
LA852H				
DESIGNED	B.B.	DETAILED	B.B.	QUANTITIES
TRACE CK.	S.H.S.	DETAIL CK.	S.H.S.	QUAN. CK.

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	33	44



PARTIAL PLAN PIPE



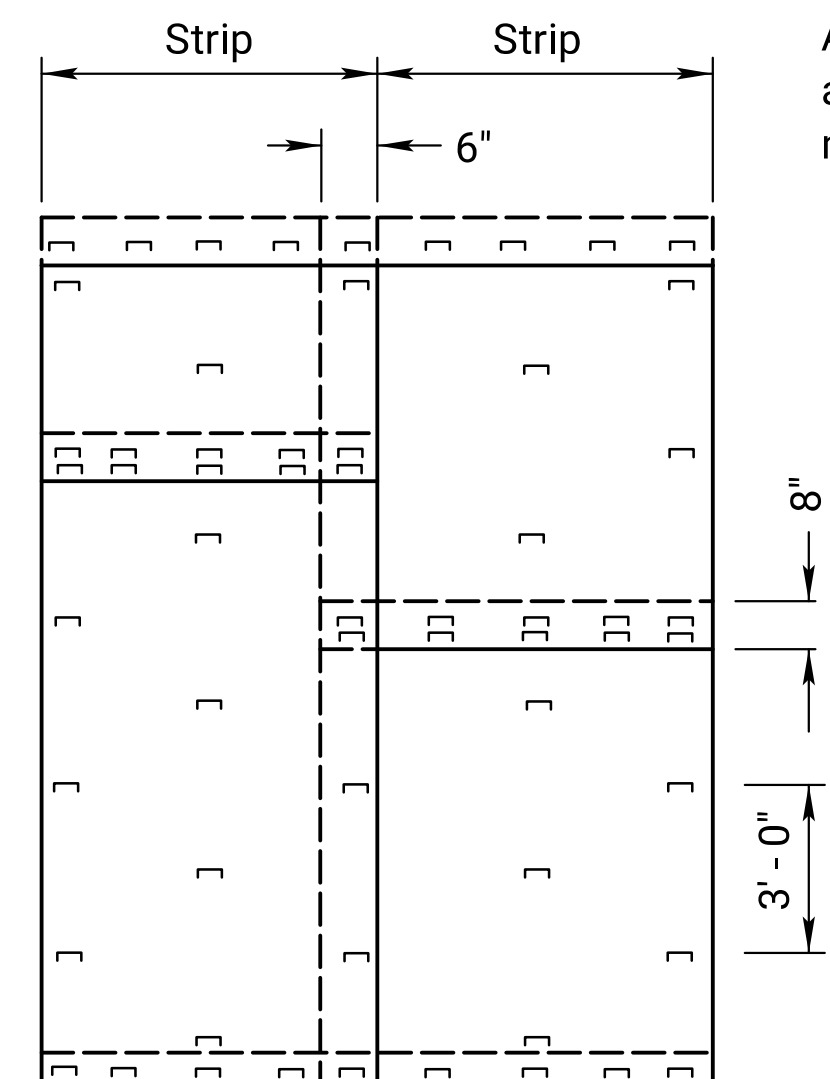
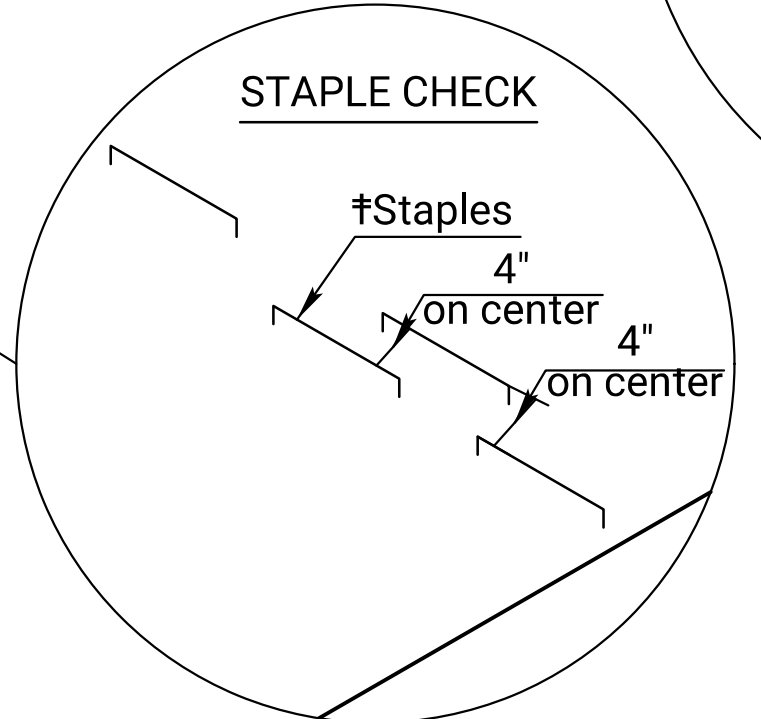
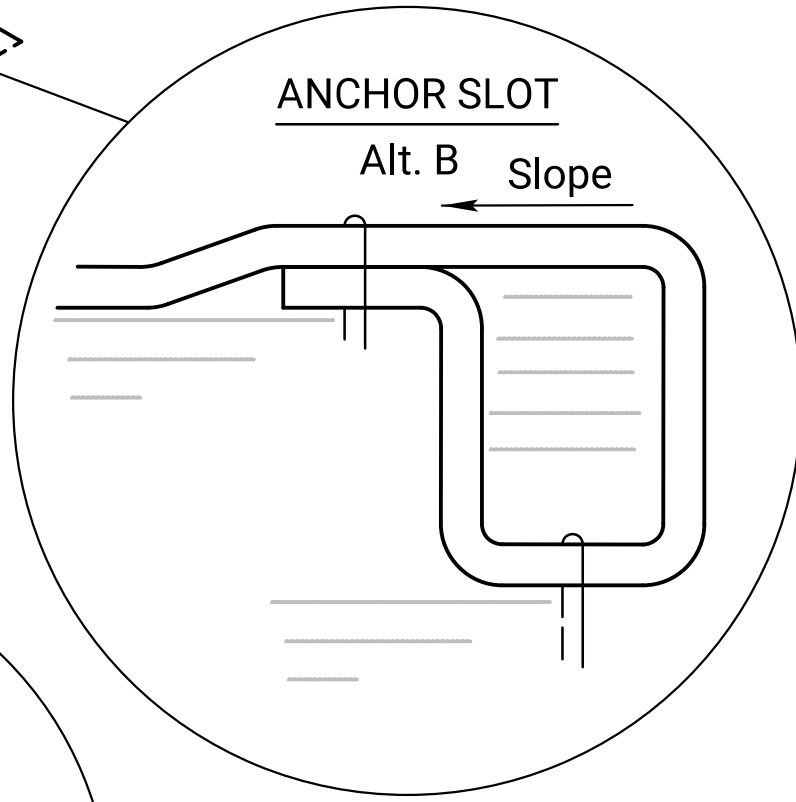
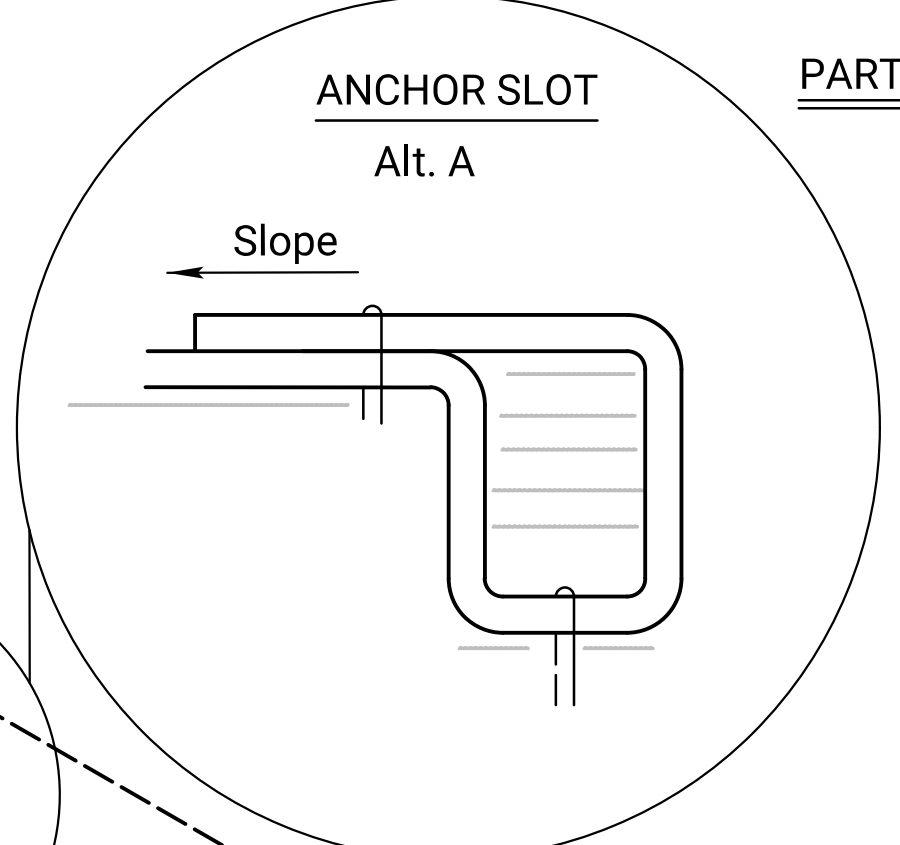
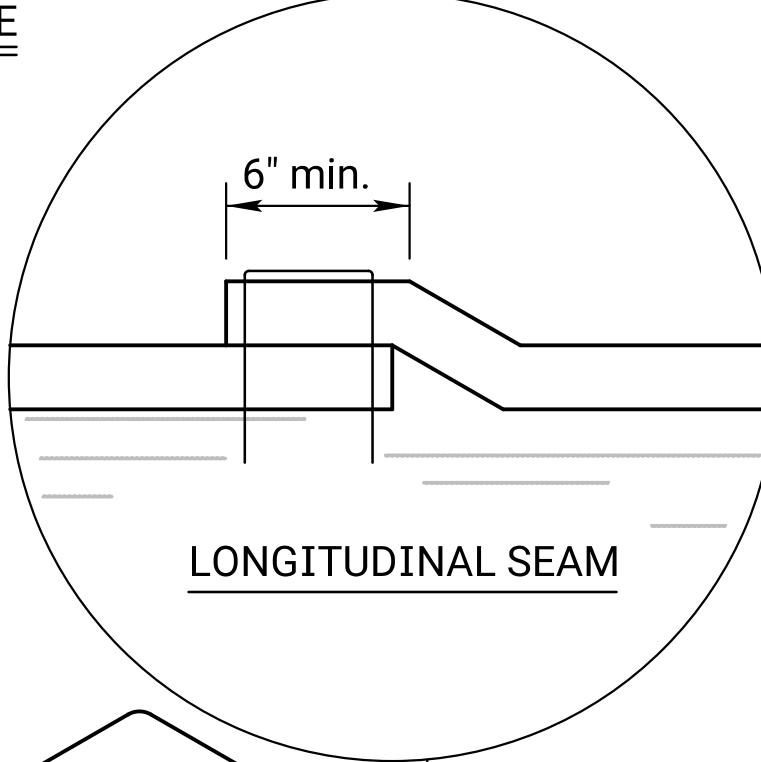
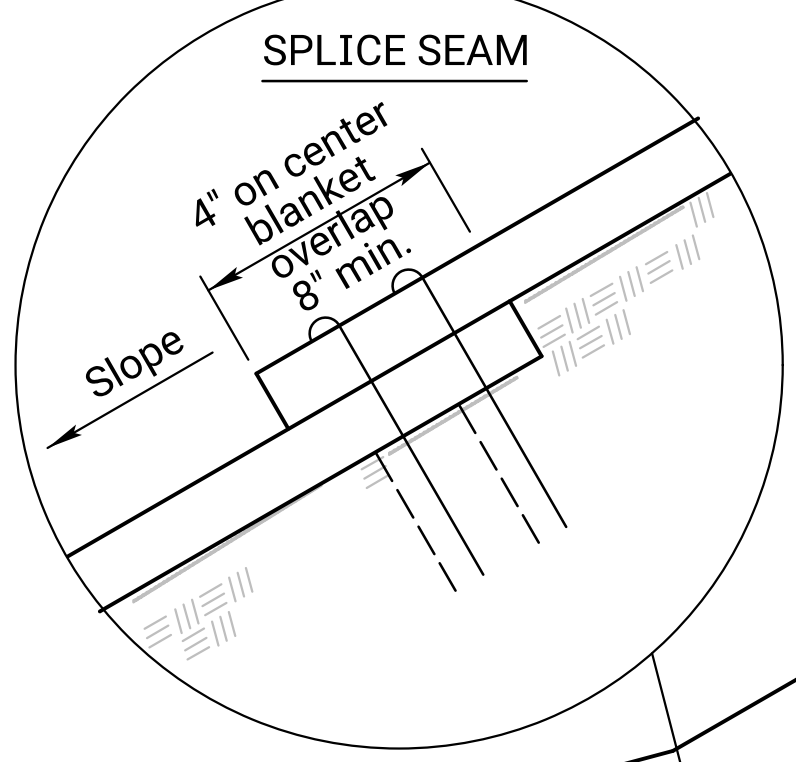
PARTIAL PLAN BOX CULVERT

INSTALLATION DETAILS FOR EROSION CONTROL CLASS 1

Erosion Control Blankets shall be laid loosely in the direction of the slope, beginning at the bottom of the slope. In order for blanket to be in contact with the soil, lay blanket loosely, avoiding stretching.

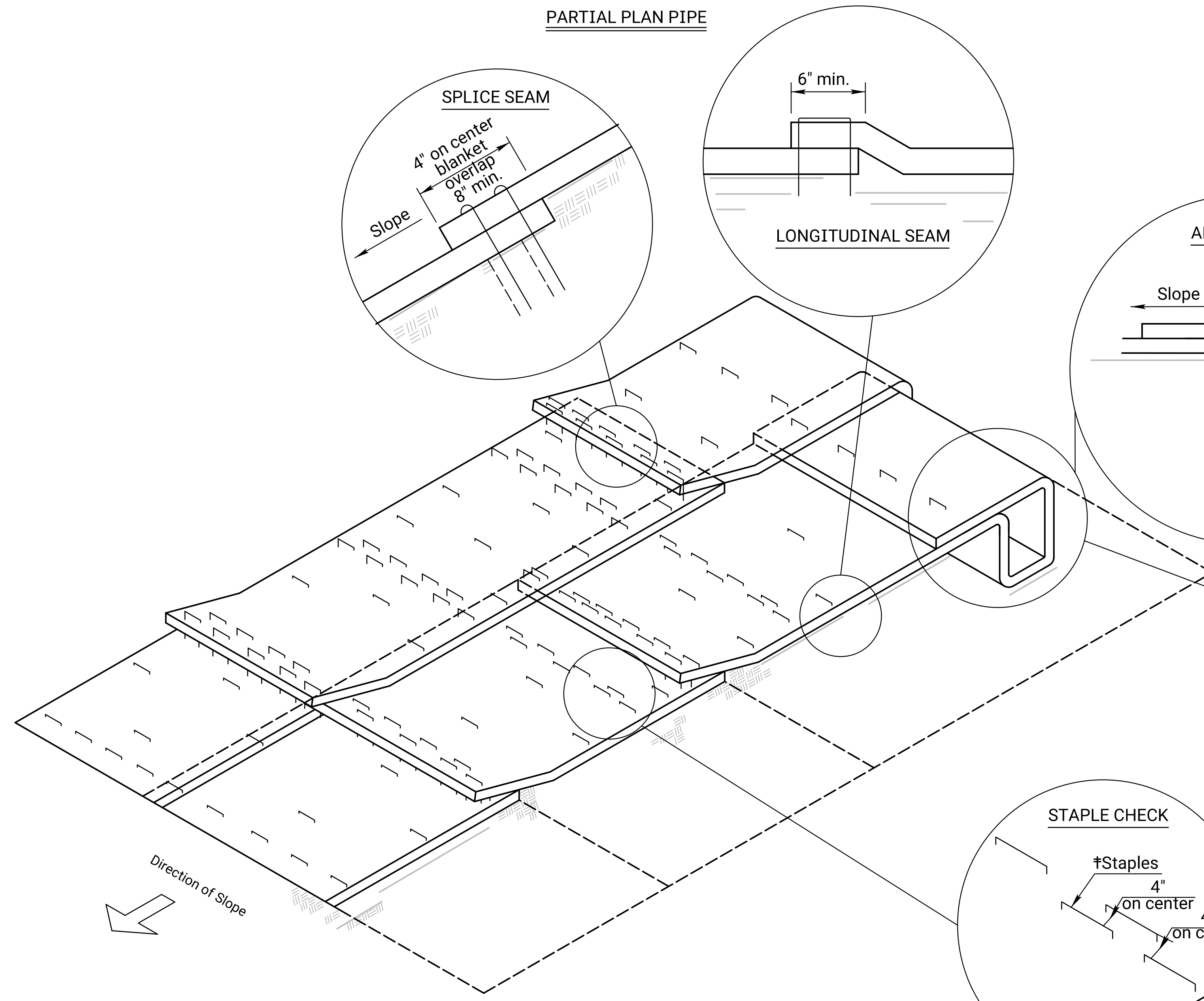
- ANCHOR SLOTS:** The top of the blanket should be "slotted in" at the top of the slope and anchored in place with anchors 6 inches apart. The slots should be 6 inches wide x 6 inches deep with the blanket anchored in the bottom of the slot, then backfilled, tamped and seeded.
- LONGITUDINAL SEAMS:** The edges of the blanket should overlap each other a minimum of 6 inches, with anchors catching the edges of both blankets.
- SPLICE SEAM:** When splices are necessary, overlap end a minimum of 8 inches in direction of water flow. Stagger splice seams.
- TERMINAL FOLD:** The bottom edge of the blanket shall be turned under a minimum of 4 inches, then anchored in place with anchors 9 inches apart.
- TYPICAL ANCHORS:** Anchor design shall be as recommended by the manufacturer.
- STAPLE CHECK:** #Establish Staples in 2 rows 4" on center apart. Staple Checks - shall be 30' apart.

● Erosion Control Class I may be omitted if the area is immediately covered by permanent slope protection (where directed by the plans).



PLAN VIEW - ANCHORING DIAGRAM

NOTE: Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards. Single post ring and shank staple is acceptable.



Plotted by: rsnow 3-JUL-2024 18:09
File: la855.dgn

KANSAS DEPARTMENT OF TRANSPORTATION					
INSTALLATION DETAIL EROSION CONTROL CLASS 1 SLOPE PROTECTION					
LA855					
FHWA APPROVAL		03-10-15		APPD	
DESIGNED	R.A.A.	DETAILED	R.A.A.	QUANTITIES	TRACED
					Scott H. Shields
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	R.A.A.	
04	03-01-15	Revised Standard		R.A.A.	S.H.S.
03	02-23-15	Revised Standard		R.A.A.	S.H.S.
02	09-15-14	Revised Standard		M.R.M.	S.H.S.
NO.	DATE	REVISIONS	BY	APPD	

GRASS & WILDFLOWER SEEDING SEASONS

COOL SEASON GRASSES	WARM SEASON GRASSES & WILDFLOWERS
February 15 thru April 20 August 15 thru September 30	November 15 thru June 1
SPECIES	SPECIES
Bluegrasses	Bermuda Grass
Brome Grasses	Big Bluestem
Canada Wildrye	Blue Grama
Fescues	Buffalo Grass
Prairie Junegrass	Indiangrass
Ryegrasses	Little Bluestem
Sterile Wheatgrass	Sand Bluestem
Tall Dropseed	Sand Dropseed
Western Wheatgrass	Sand Lovegrass
	Side Oats Grama
	Switchgrass
	Wildflower Mixes

When the area to be seeded is 1 acre or more, if Cool Season grasses are mixed with Warm Season grasses, seed the area during the Warm Season.

When the area to be seeded is less than 1 acre, seed the area any time of the year.

GENERAL NOTES

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded and mulched. Soil preparation shall conform to the Standard Specifications except as noted below.

All borrow areas shown on the plans are to be fertilized, seeded, and mulched. However, operation in borrow areas where crops are growing may be omitted when requested by the owner.

If temporary cover has provided stable slopes with no erosion, seed the permanent grasses into the existing cover. If there has been erosion that requires repair prior to seeding, then it may be necessary to regrade the area, resulting in bare ground.

FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P₂O₅, K₂O listed in Summary of Seeding Quantities will be acceptable.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching material is generally as follows:

1 3/4 - 2 1/4 Tons per Acre = 1 1/2" loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

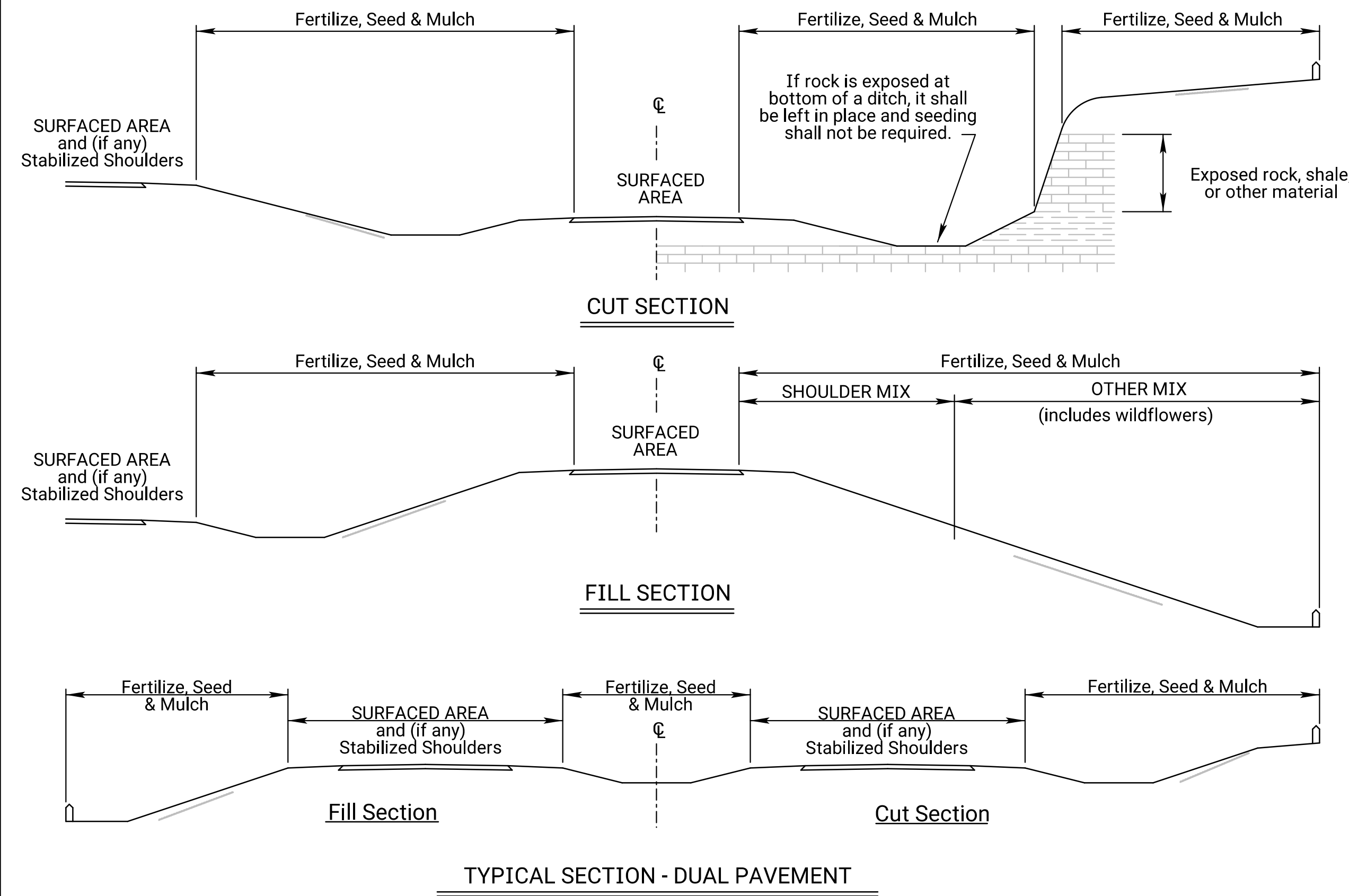
When seeding is less than 1 acre, temporary and permanent seeding shall be combined and seeded at the same time.

There is no seasonal restriction when seeding projects less than one acre.

SODDING SEASONS

COOL SEASON GRASSES	WARM SEASON GRASSES
March 1 thru April 15 September 1 thru November 15	May 15 thru September 1
SPECIES	SPECIES
Bluegrass Sod	Buffalo Grass Sod
Fescue Sod	

If the soil is workable, the Engineer may allow placement of sod between November 15 and March 1. If sod is placed during this time, maintain the sod until 20 days after the beginning of the spring sodding season.



NATIVE WILDFLOWER MIX 1

PLS RATE	NAME	QTY (lb)
0.3	Butterfly Milkweed	XX.XX
0.3	Common Milkweed	XX.XX
0.3	Black Eyed Susan	XX.XX
0.5	Blanket Flower	XX.XX
0.5	False Sunflower	XX.XX
0.5	Lance-Leaf Coreopsis	XX.XX
0.2	Maximilian Sunflower	XX.XX
0.1	New England Aster	XX.XX
0.2	Pinnate Prairie Coneflower	XX.XX
0.2	Plains Coreopsis	XX.XX
0.3	Purple Coneflower	XX.XX
0.3	Upright Prairie Coneflower	XX.XX
0.3	Dames Rocket	XX.XX
0.3	Lemon Mint	XX.XX
0.2	Pitcher Sage	XX.XX
0.2	Wild Bergamot	XX.XX
1.0	Illinois Bundleflower	XX.XX
0.2	Common Evening Primrose	XX.XX
0.1	Hoary Verbena	XX.XX
0.8	Purple Prairie Clover	XX.XX
0.3	Roundhead Lespedeza	XX.XX
3.0	Showy Partridge Pea	XX.XX
0.2	White Prairie Clover	XX.XX
10.3	Total (lb)	XX.XX

NATIVE WILDFLOWER MIX 2

PLS RATE	NAME	QTY (lb)
0.3	Butterfly Milkweed	
0.3	Black Eyed Susan	
0.5	Black Sampson Coneflower	
1.0	Blanket Flower	
0.2	Maximilian Sunflower	
0.2	Plains Coreopsis	
0.2	Upright Prairie Coneflower	
0.2	Western Yarrow	
0.3	Lemon Mint	
0.4	Pitcher Sage	
1.5	Illinois Bundleflower	
0.2	Common Evening Primrose	
1.0	Blue Wild Indigo	
0.4	Leadplant	
0.4	Purple Prairie Clover	
0.3	White Prairie Clover	
7.4	Total (lb)	

Package and deliver the wildflower seed separately from the grass seed mix. Package and deliver the Tall Drop Seed separately from the grass seed and the wildflower mix. Place the grass seed (except Tall Drop Seed) in the large seed box and drill (cover) seed 1/8" - 1/4". Place the wildflower seed in a separate seed box and drill (cover) seed 1/6" maximum. Place the Tall Drop Seed in a separate (third) seed box and place the seed (using the seed drill) on the soil surface.

OPTION: Broadcast Tall Drop Seed on the soil surface.

SUMMARY OF SEEDING QUANTITIES

P.L.S. RATE/ACRE		ACRES		BID ITEM	QUANTITY	UNIT
SHLDR	OTHER	SHLDR	OTHER			
				See LA 852A for Soil Erosion Mix to be used as Permanent Seeding		
				Mulching *		

SHLDR = Seeded with the Shoulder Mix. Typically 15 feet for 2-lane roads and 30 feet for 4-lane roads. Includes outside roadsides, turfed portions of shoulders, and turfed portion of the median.

OTHER = Seeded with the "Other" Mix. Designated as all other turf areas, except the Shoulder. Usually includes a Native Wildflower Mix.

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. All disturbed areas shall be seeded, fertilized and mulched at the listed rate per acre. The acres are estimated.

Refer to the Standard Specifications, Division 900, Section 904 'Seeding', and Section 907 'Sodding', for the seeding and sodding seasons.

* See LA852A for mulching quantity. The quantity of mulch is estimated (Acres of Seeding X 1.5 X 2 Tons/Acre). The total mulch required shall be determined in the field. The bid item for mulching shall be paid for according to the Standard Specifications.

NO.	DATE	REVISIONS	BY	APPD
02	11-25-20	Updated Seeding / Sodding Periods Charts	M.R.D.	M.L.
01	08-03-20	Revised Standard	M.R.D.	S.H.S.

KANSAS DEPARTMENT OF TRANSPORTATION

PERMANENT SEEDING SUMMARY OF SEEDING QUANTITIES

LA850

DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

05-06-19 APPD. Mervin Lare

STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	35	44

1) Design Speed: Those items delegated to temporary traffic control should be designed and installed using the posted/legal speed of the roadway prior to work starting.

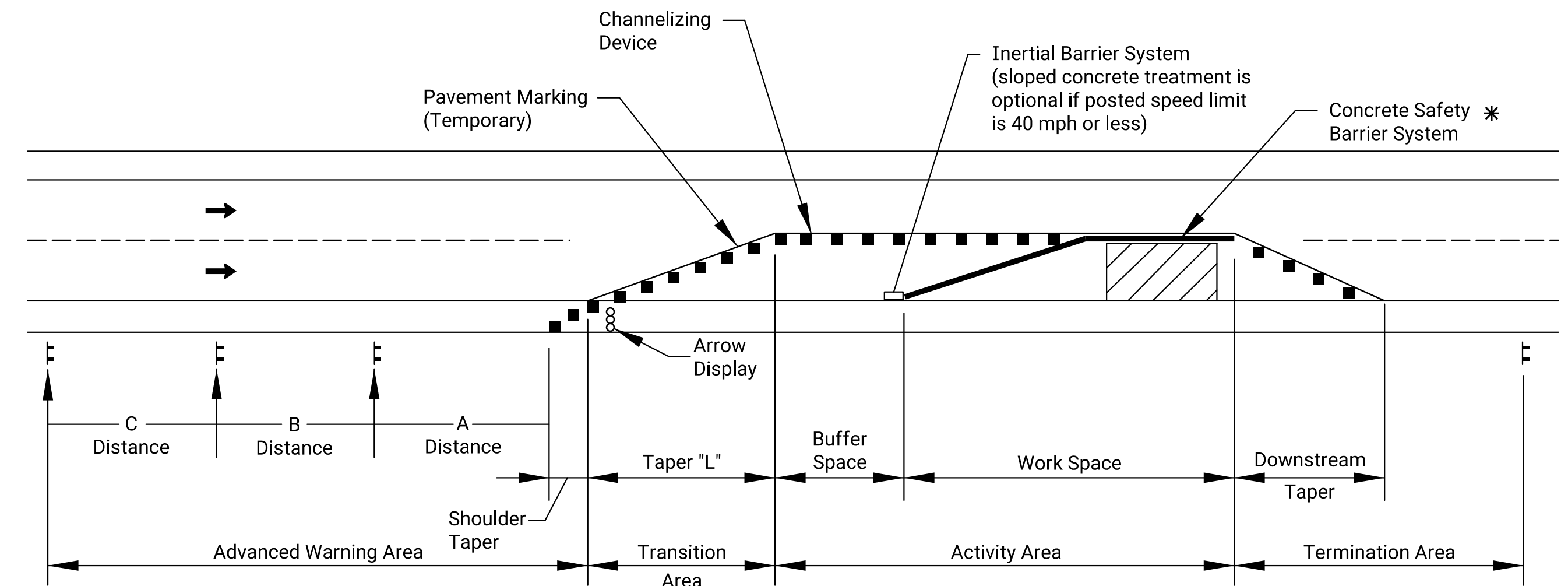
2) Minimum Lane Width: Lane widths shall be a minimum of 11' (measured between centerlines of pavement markings) or as shown on the plans, or as directed by the engineer. A lane width less than 11' may require restricted roadway width signing.

3) Consideration should be made to separate pedestrian and, if needed, bicycle movements from both work site activity and vehicular traffic. Unless a reasonable safe route that does not involve crossing the roadway can be provided, pedestrians should be appropriately directed with advance signing that encourages them to cross to the opposite side of the roadway. In urban and suburban areas with high vehicular traffic volumes, these signs should be placed at intersections (rather than midblock locations) so that pedestrians are not confronted with midblock work sites that will induce them to attempt skirting the work site or making a midblock crossing.

4) When existing pedestrian facilities are disrupted, closed, or relocated, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility.

5) When the driving surface open to traffic is milled or is a temporary surface made of loose material, or when directed by the engineer a W8-15 (Grooved Pavement) or W8-7 (Loose Gravel) sign shall be used on mainline approaches. This sign should be placed a "C" distance after the W20-1 (Road Work Ahead) sign. A W8-15p motorcycle plaque shall be used to supplement the W8-15 or W8-7 signs. All signs shall be displayed as long as the condition is present.

6) Alternative temporary rumble strip options may be available. Please contact the Temporary Traffic Control Unit for more information at 785-296-1179 or 785-296-1183.



TYPICAL WORK ZONE COMPONENTS

* When concrete barrier system is used, portable channelizing devices are not needed along the tangent barrier section.

Minimum advance warning sign spacing (in feet):

SPEED (MPH) *	A	B	C
URBAN (40 MPH OR LOWER)	100	100	100
URBAN (45 MPH OR HIGHER)	350	350	350
RURAL (55 MPH OR LOWER)	500	500	500
RURAL (60 MPH OR HIGHER)	750	750	750
EXPRESSWAY/FREEWAY	1000	1500	2640

* Posted speed prior to work starting
The minimum spacing between signs shall be no less than 100', unless directed by the engineer.
The spacing between any signs may be increased beyond the minimum values in the table above as approved by the engineer in order to maximize visibility.

Taper Formulas:

$L = WS$ for speeds of 45 MPH or more

$L = WS^2/60$ for speeds of 40 MPH or less

Where: L = Minimum length of taper in feet
 S = Numerical value of posted speed prior to work starting in MPH
 W = Width in offset feet

Shifting Taper = 1/2 L
Shoulder Taper = 1/3 L

Channelizer Placement:

- The spacing between devices in transition area (taper) should not exceed a distance in feet equal to 1/2 the posted speed limit in mph prior to work starting.
- The spacing between devices in the advanced warning area and the activity area should not exceed a distance in feet equal to two times the posted speed limit in mph prior to work starting.
- Channelizing devices shall be placed for optimum visibility, normally at right angles to the traffic flow.
- Place directional indicator barricades in series to direct traffic onto the new path. The arrow sign should not be visible to opposing traffic.
- Alternating diagonal orange and white striping must slope downward in the direction traffic is expected to pass.

Buffer Space

SPEED (MPH) *	20	25	30	35	40	45	50	55	60	65	70	75
LENGTH (ft)	115	155	200	250	305	360	425	495	570	645	730	820

* Posted speed prior to work starting

Neither work activity nor storage of equipment, vehicles, or material should occur in the buffer space. When a protection vehicle is placed in advance of the work space, only the space upstream of the vehicle constitutes the buffer space.

If temporary concrete safety barrier system is used to separate approaching traffic from the work space, the barrier system shall be considered part of the activity area. A full lane width should be available throughout the length of the buffer space. See typical work zone components above.

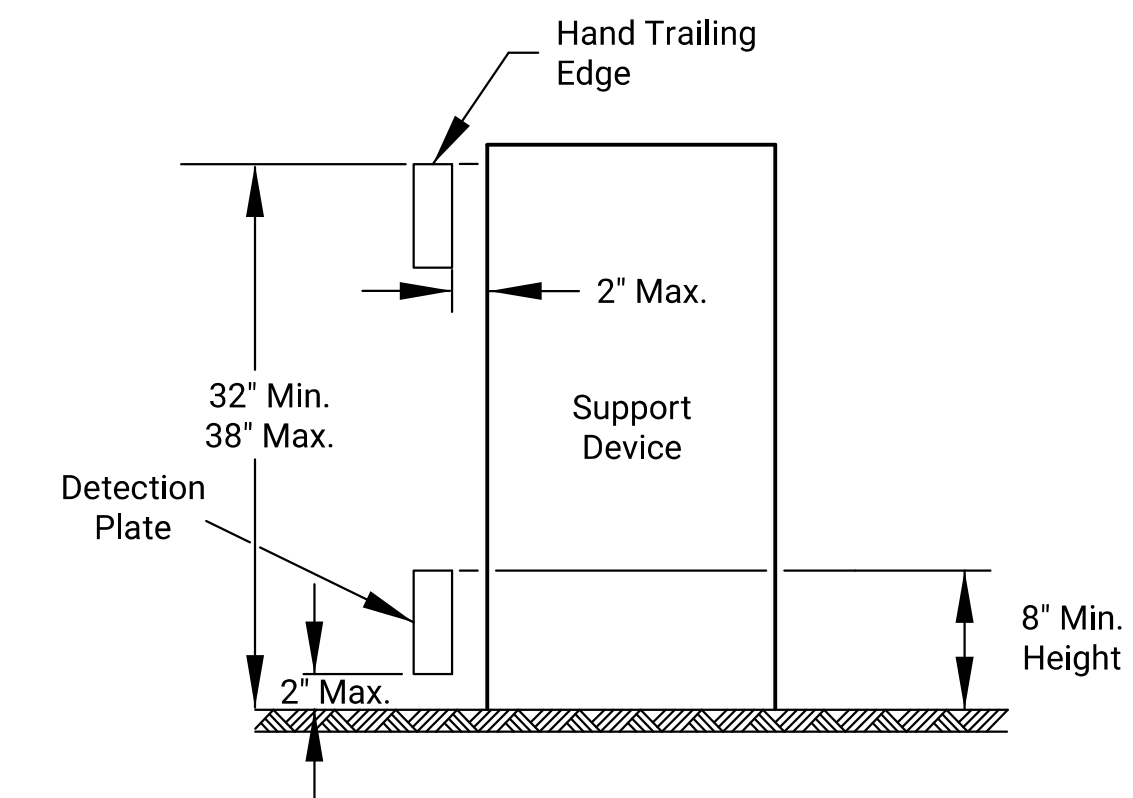
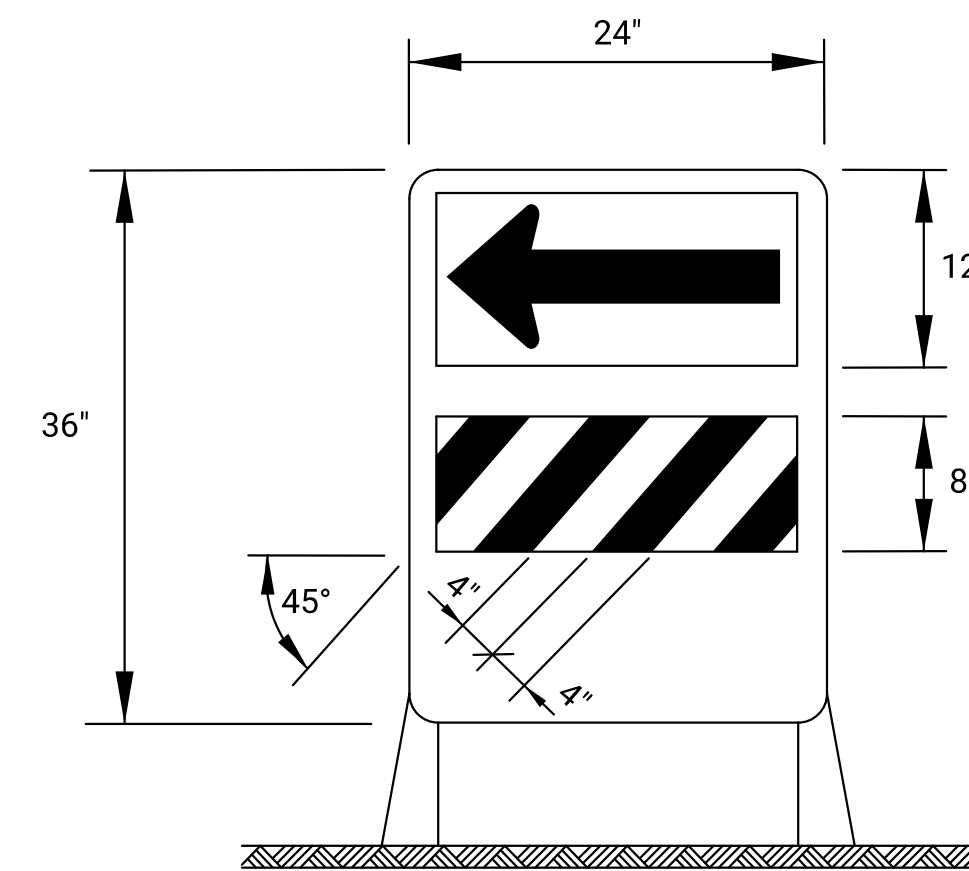
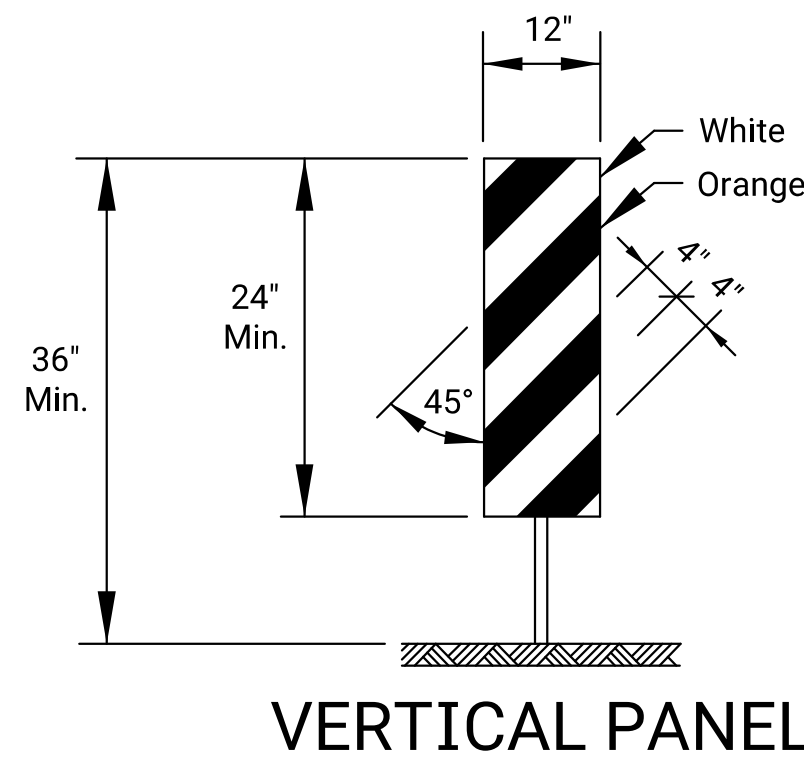
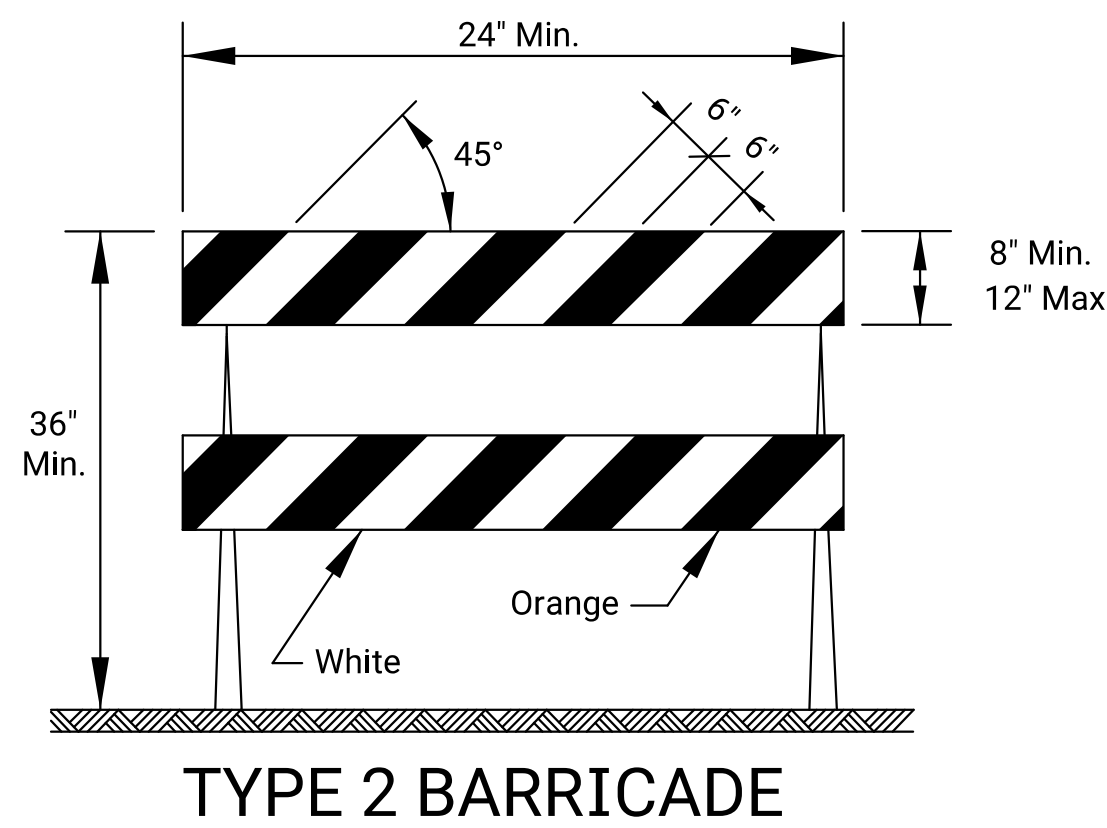
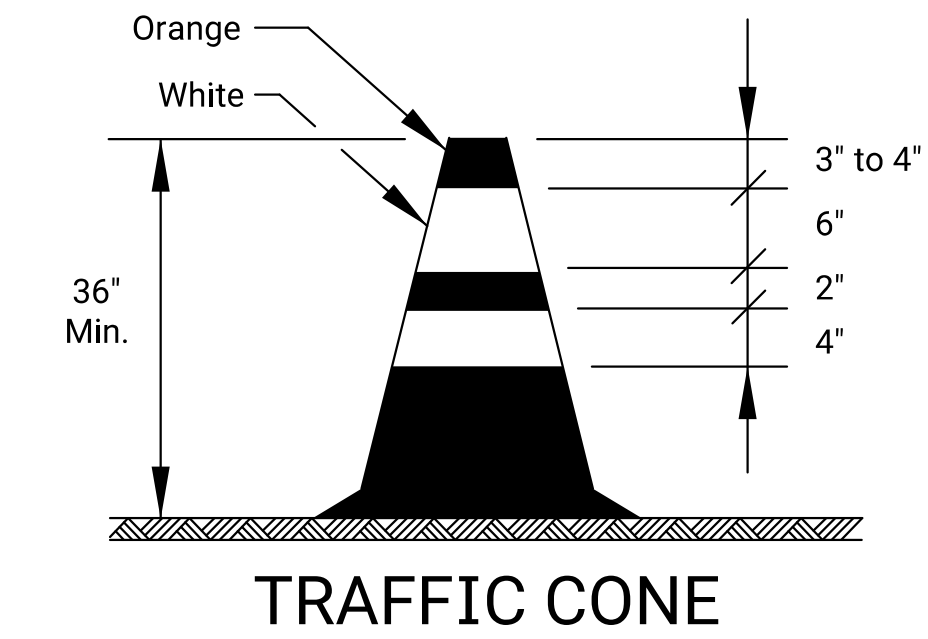
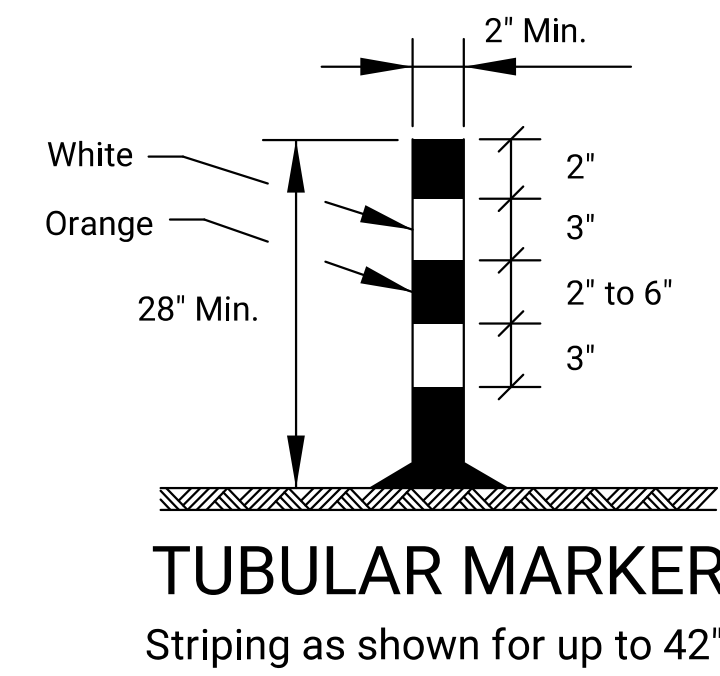
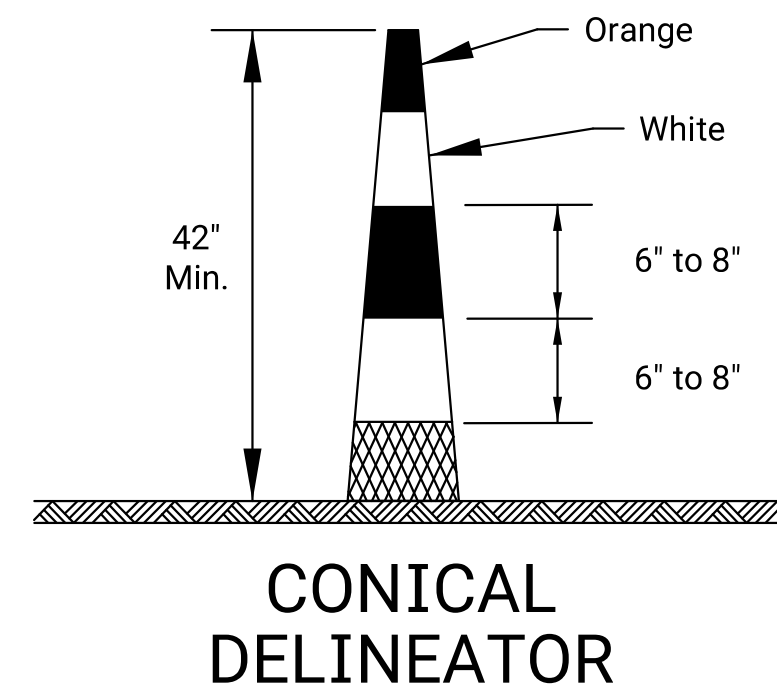
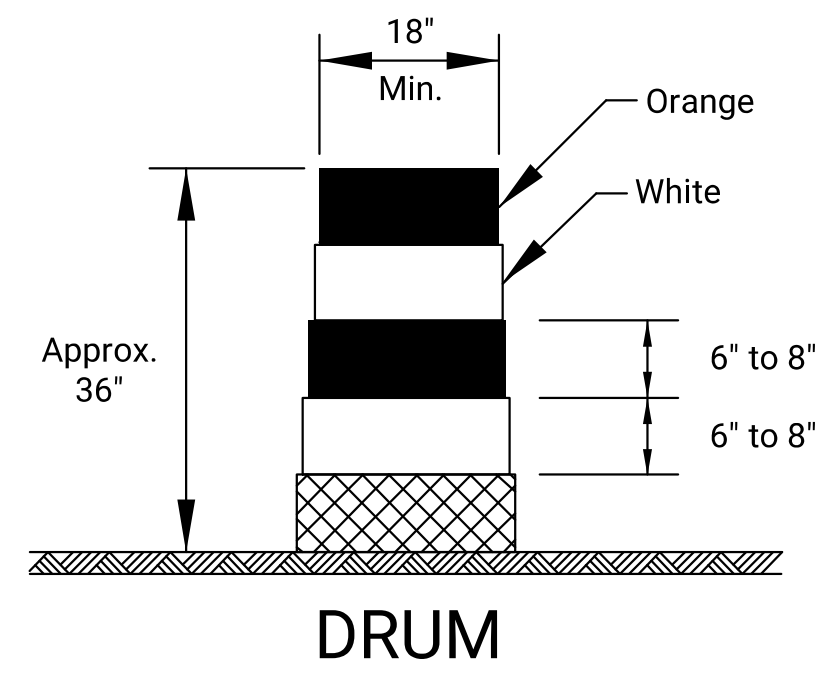
NO.	DATE	REVISIONS	BY	APPD
02	03-13-18	W8-15p usage changed to Shall	R.W.B.	E.K.G.
01	08-18-15	Channelizer spacing info	R.W.B.	K.E.

KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL GENERAL NOTES

TE700

FHWA APPROVAL		03-13-18		APPD.	Eric Koehler
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN. CK.	TRACE CK.



For rails less than 36" long, 4" wide stripes may be used. All stripes shall slope downward to the traffic side for channelization.

The stripes shall slope downward to the traffic side for channelization.

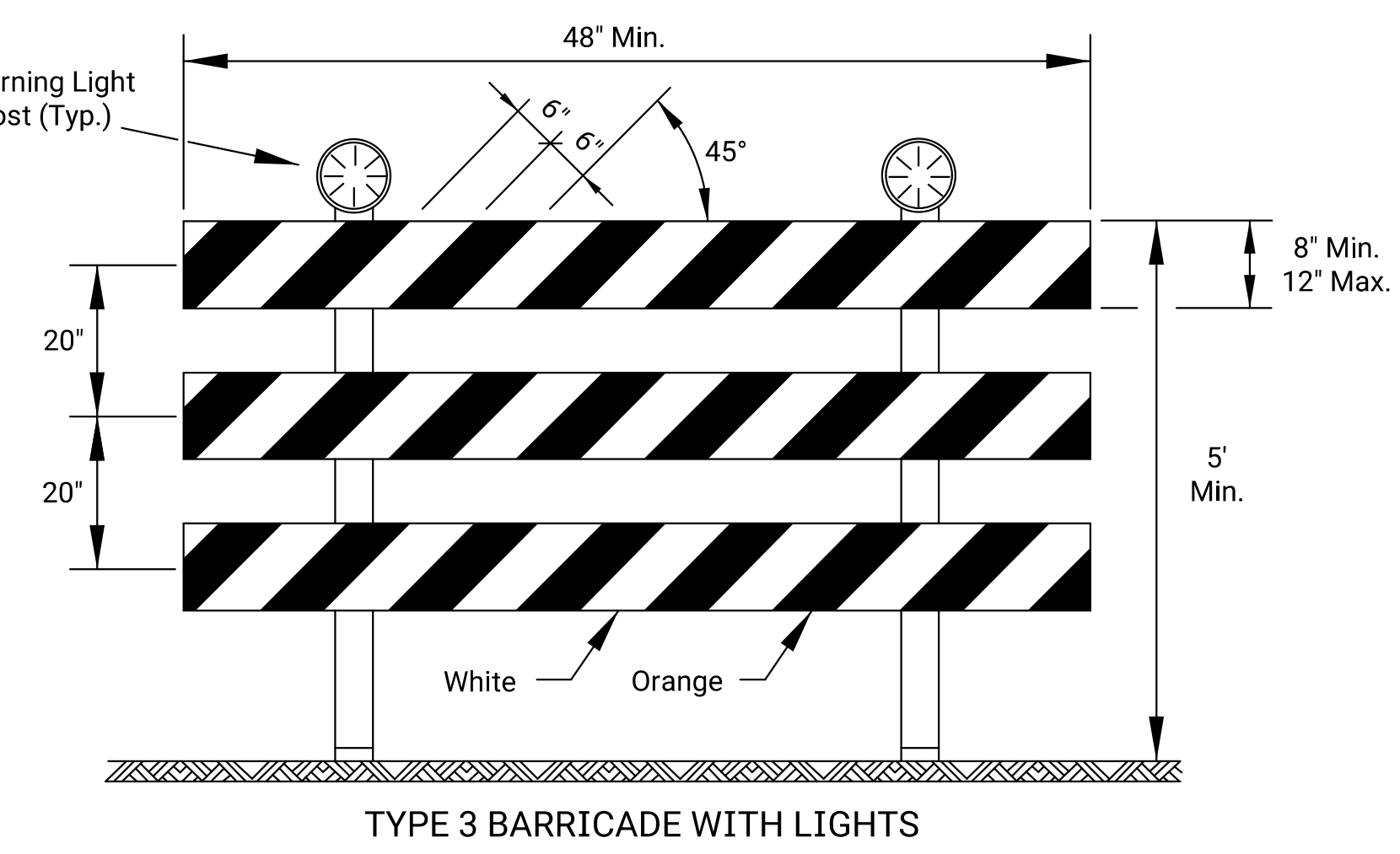
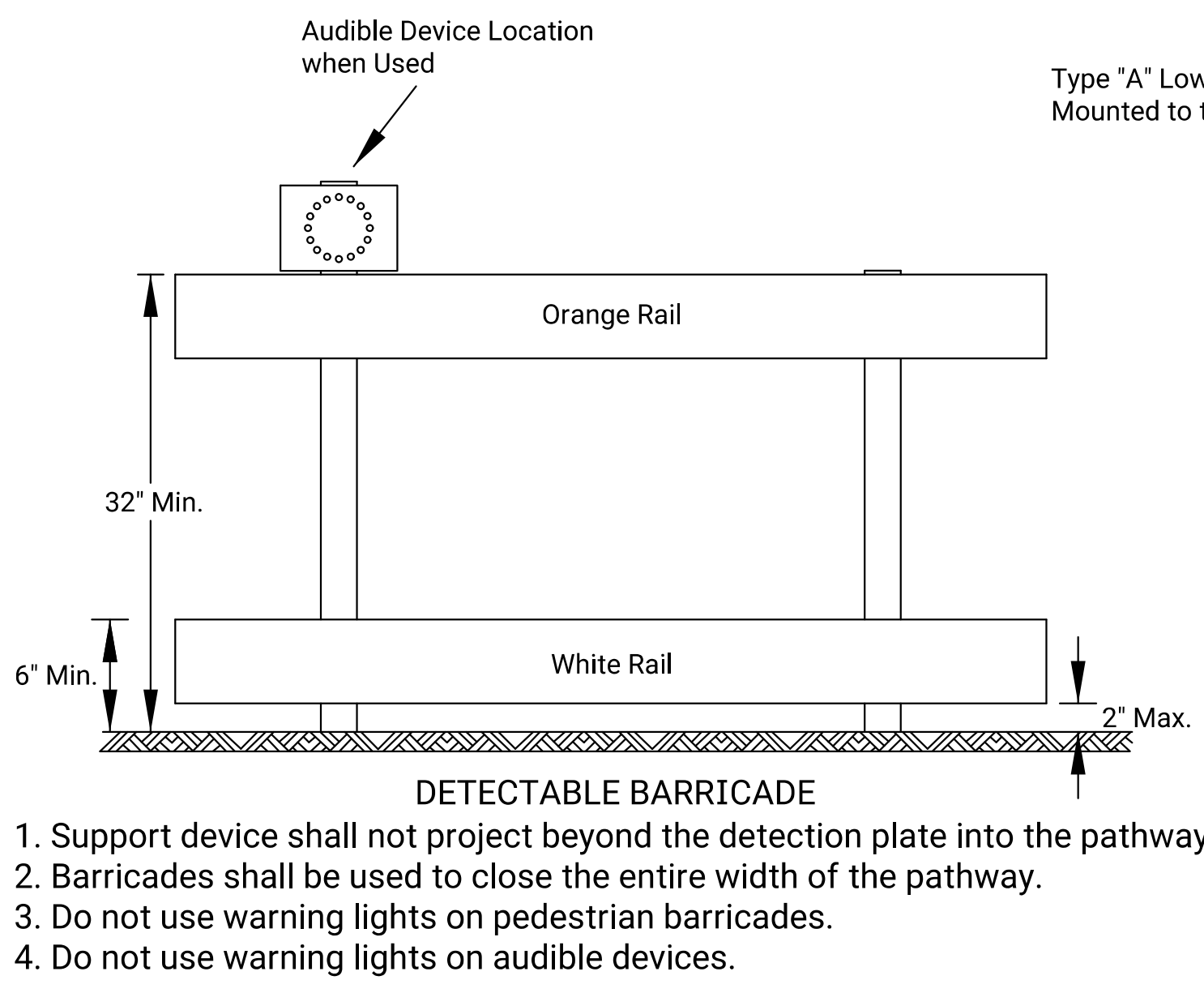
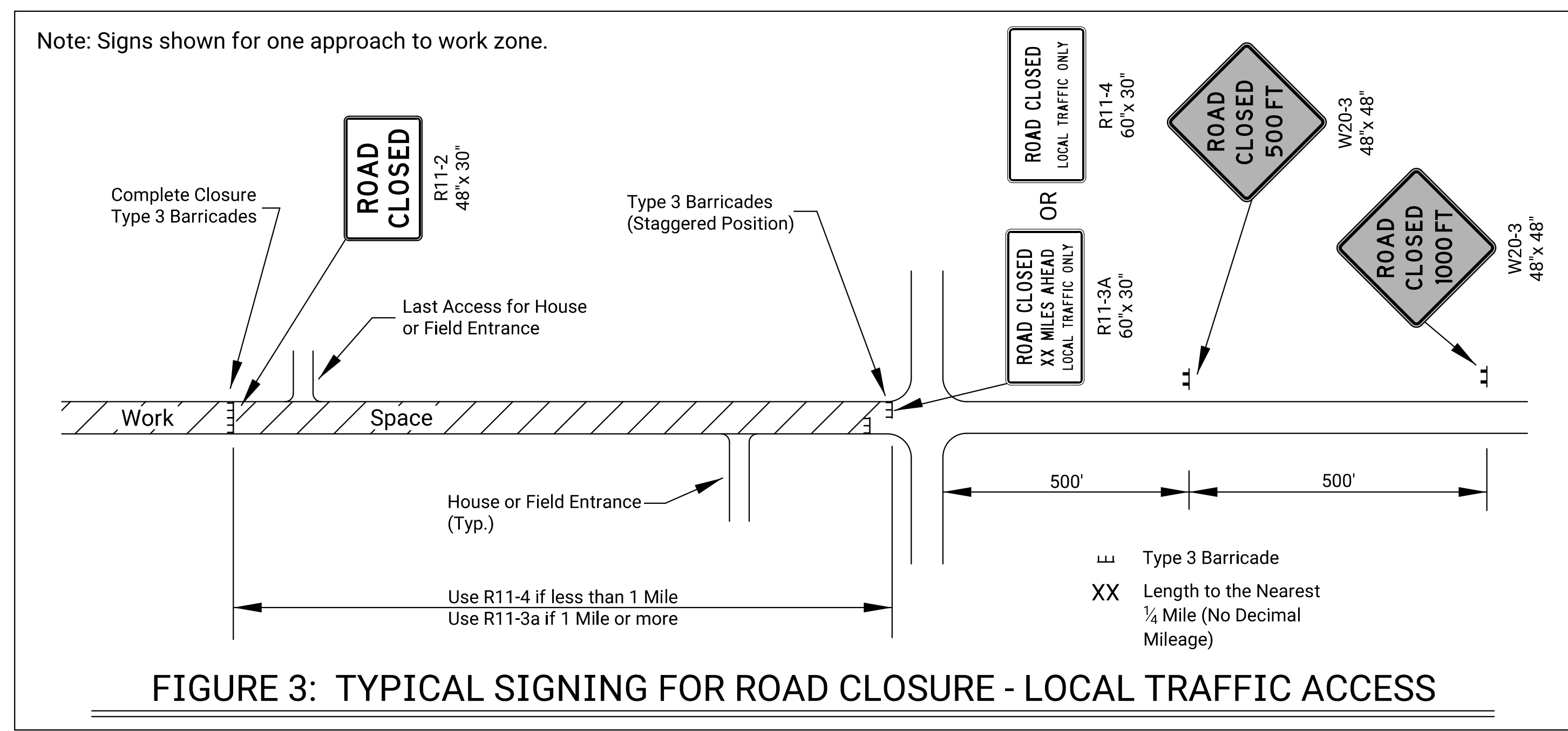
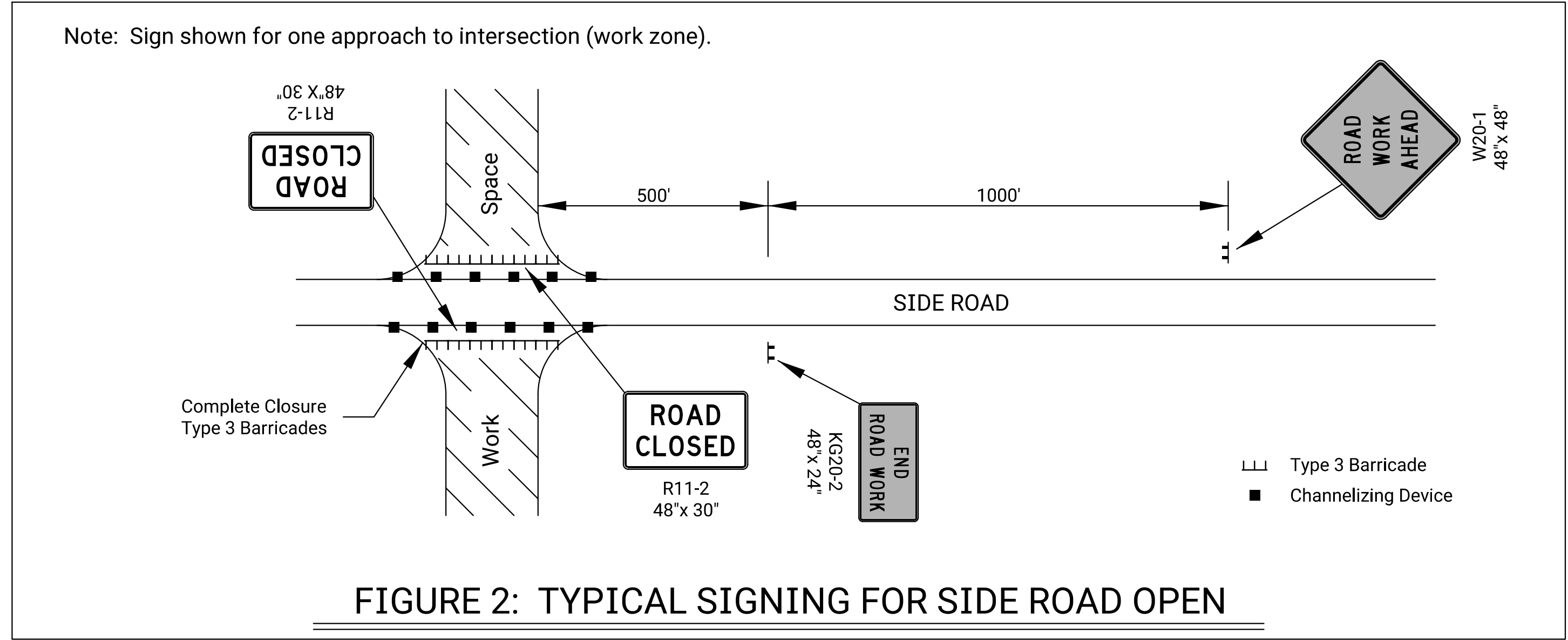
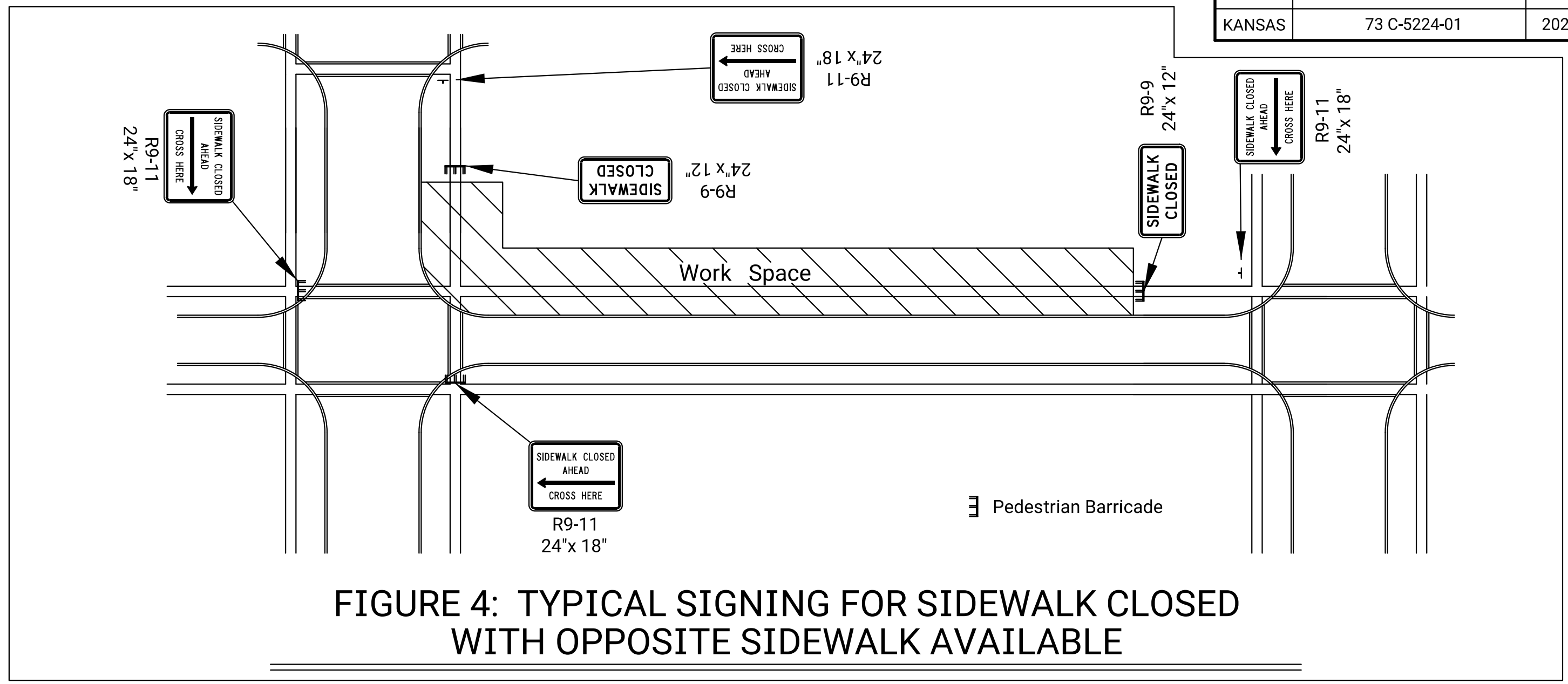
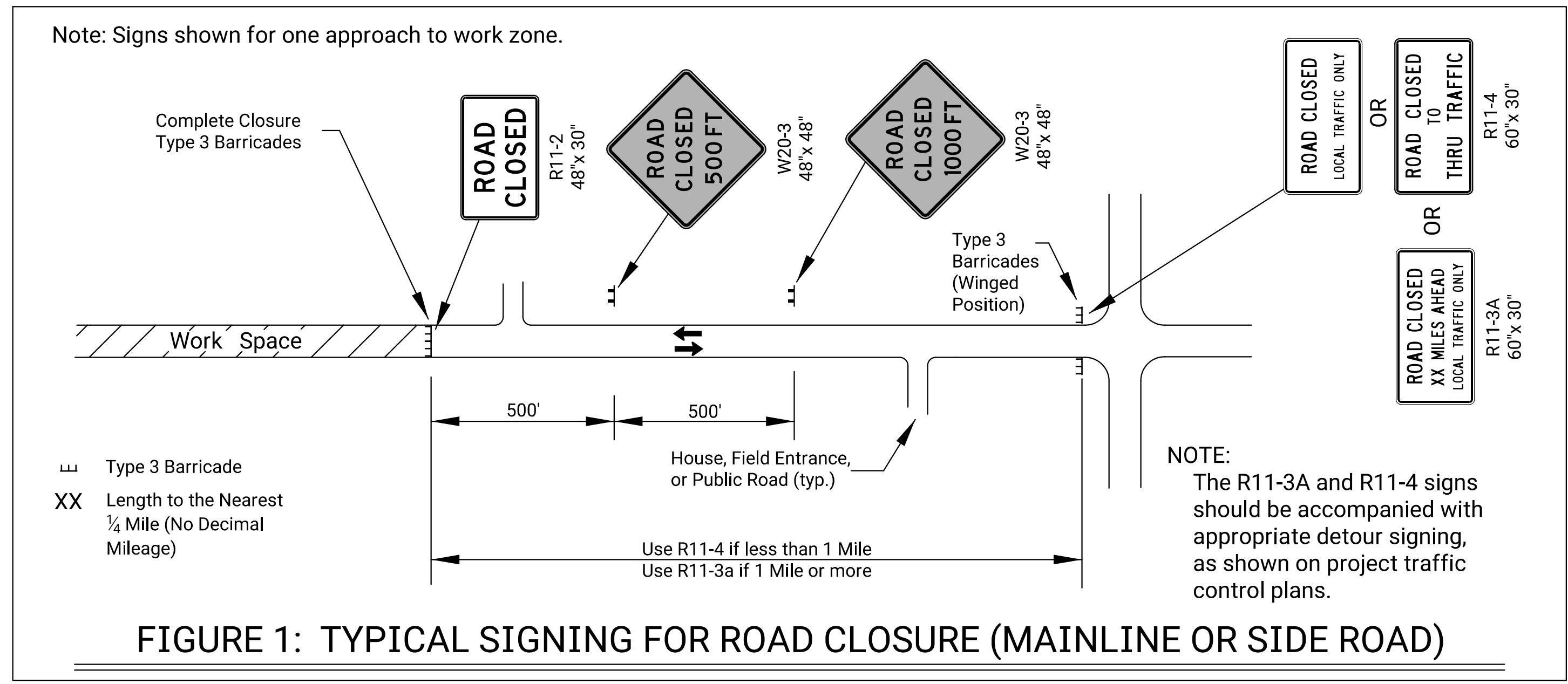
The stripes shall slope downward in the direction traffic is to pass. The direction indicator barricade shall be used in series to direct the motorist into the intended lane of travel.

1. Support device shall not project beyond the detection plate into the pathway.
2. Hand trailing edges and detection plates are optional for continuous walls.
3. Interconnect pedestrian channelizers to prevent displacement and to provide continuous guidance through or around work.
4. Alternate pathways shall be firm, stable, and slip resistant.
5. Treat height differentials > 1/2" in the surfaces of alternate paths with a firm, stable, and slip resistant temporary ramp having a slope of 12:1 or flatter and having a width equal to the alternate path.
6. Use alternating orange/white on interconnected devices.

Item	Location	Location									
		Cross-overs	Shoofly Divisions	Tangents	Tapers	Ramps	Head to Head	Object Identifier	Lead-in Devices	Gores	
Portable	Drums	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes	
	Conical Delineators	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes	
	Vertical Panels	(2)	(2)	(2)	(2)	(2)	(1,2)	Yes	(2)	(2)	
	Direction Indicator Barricade	No	No	No	Yes	No	No	No	No	No	
	Type 2 Barricade	(2)	(2)	(2)	(2)	No	No	Yes	No	No	
	Traffic Cones	No	No	(4)	(4)	(4)	No	(4)	(4)	(4)	
Fixed	Tubular Markers	(3)	(3)	(3)	No	(3)	Yes	No	Yes	Yes	
	Vertical Panels	(3)	(3)	(3)	(3)	(3)	(3)	Yes	(2,3)	(2)	

- (1) Not allowed on centerline delineation along freeways or expressways.
- (2) The stripes shall slope downward to the traffic side for channelization.
- (3) May be used upon the approval of the engineer.
- (4) Daytime operations only.

NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL CHANNELIZING DEVICES				
TE702				
FHWA APPROVAL		06-01-15		APPD.
DESIGNED	L.E.R.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACED	TRACE CK.



ROAD CLOSED GENERAL NOTES

As shown in Figure 1, at the point where thru traffic must detour and local traffic can proceed to the location where the roadway is completely closed, the R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) or R11-4 (ROAD CLOSED LOCAL TRAFFIC ONLY or ROAD CLOSED TO THRU TRAFFIC) sign shall be used with Type 3 barricades (winged position), placed on the shoulders of roadway.

As shown in Figure 3, when local traffic must be allowed access into the work zone, Type 3 barricades shall be longitudinally staggered to maintain the appearance of a closed roadway. A second line of end-to-end Type 3 barricades shall be placed just beyond the last access point in the work zone, to completely close the roadway.

The R11-4 (ROAD CLOSED TO THRU TRAFFIC or ROAD CLOSED LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is less than 1 mile.

The R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is 1 mile or greater.

The words "BRIDGE OUT" (or BRIDGE CLOSED) may be substituted for the words "ROAD CLOSED" on the R11-3a or R11-4 sign where applicable.

Plotted by: rsnw 3-JUL-2024 18:10
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NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL CLOSURES				
TE704				
FHWA APPROVAL		06-01-15		APPD.
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACED	TRACE CK.

SIGN LAYOUT INFORMATION



Std. Size
Expwy/Freeway
6" C
48"x 24"



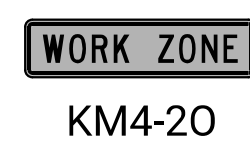
Std. Size
Expwy/Freeway
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48"x 48"



Std. Size
Expwy/Freeway
6" C
48"x 24"

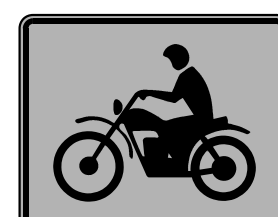


Std. Size
Expwy/Freeway
8" D
48"x 48"



Std. Size
3" C
24"x 6"

Expwy/Freeway
6" C
48"x 12"



Std. Size
Expwy/Freeway
30"x 24"



Mileage to be Determined
by the Engineer.



Std. Size
Expwy/Freeway
8" D
48"x 48"



Std. Size
Expwy/Freeway
48"x 48"

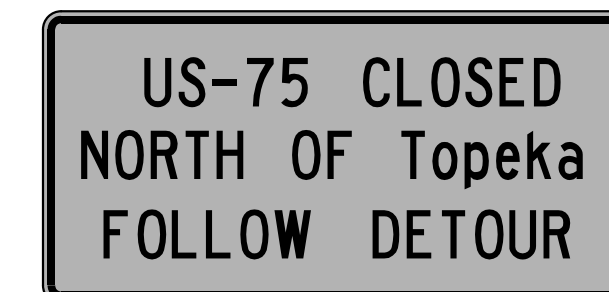


Std. Size
Expwy/Freeway
30"x 24"



Std. Size
6" C

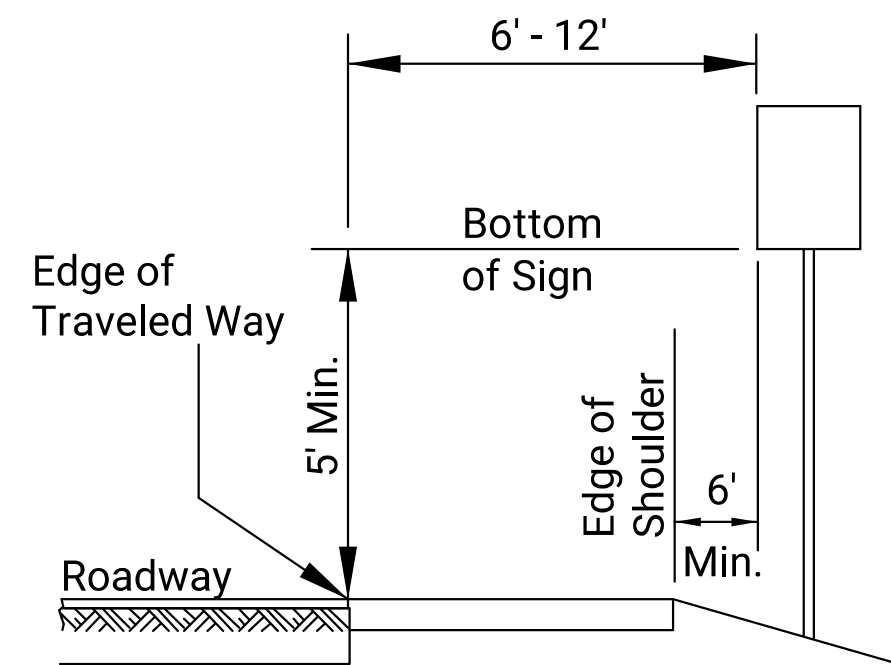
Expwy/Freeway
10" D



Std. Size
Uppercase: 6" C
Lowercase: 4.5" C

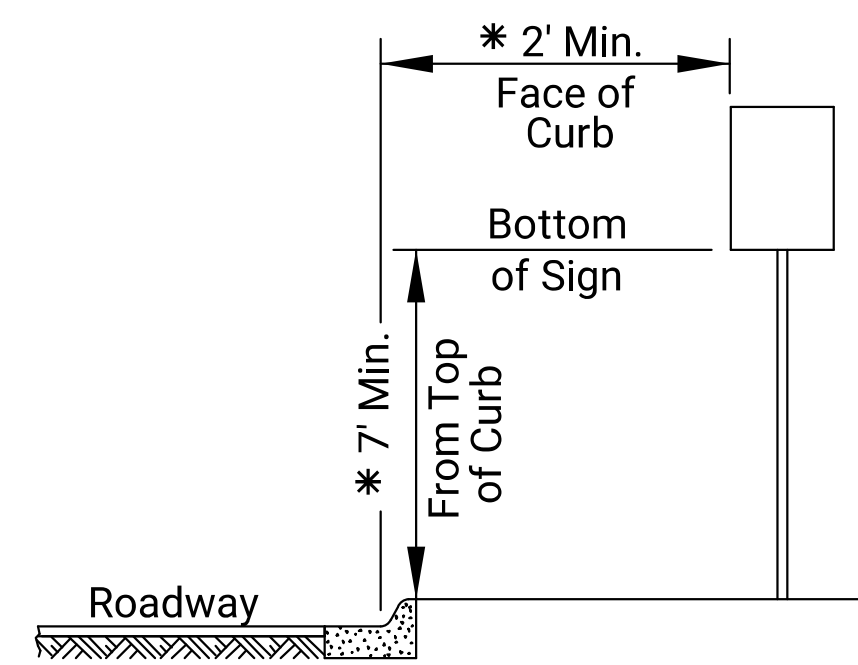
Expwy/Freeway
Uppercase: 10" D
Lowercase: 8" D

All city names and street names on special signs and destination signs must have upper and lower case letters.



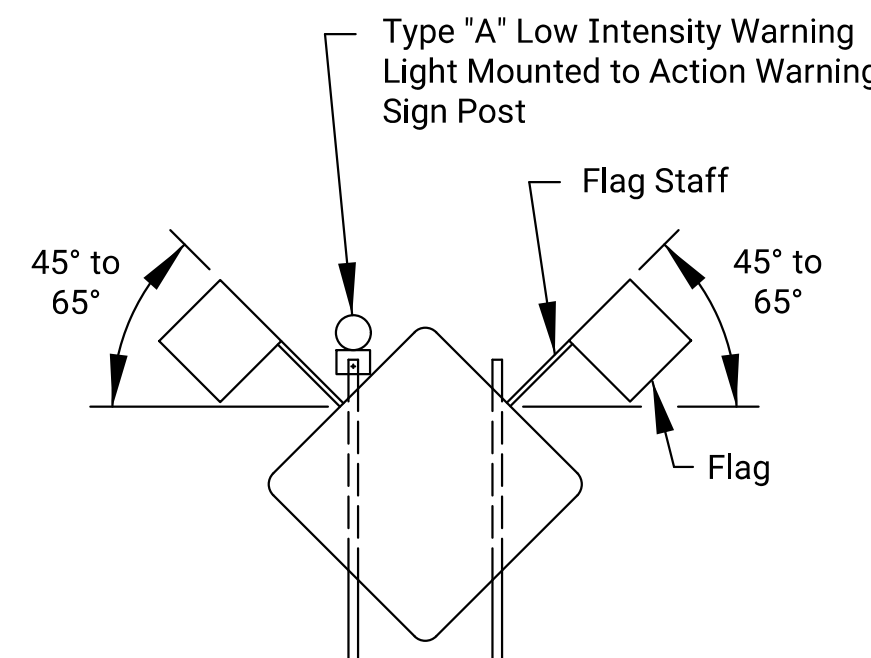
RURAL

- 1) Ground-mounted signs shall be mounted at a minimum height of 5' measured from the bottom of sign to the near edge of the pavement.
- 2) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- 3) The height of the secondary sign mounted below another sign may be 4' measured from the bottom of the sign to the near edge of the pavement. Signs shall not overlap each other.



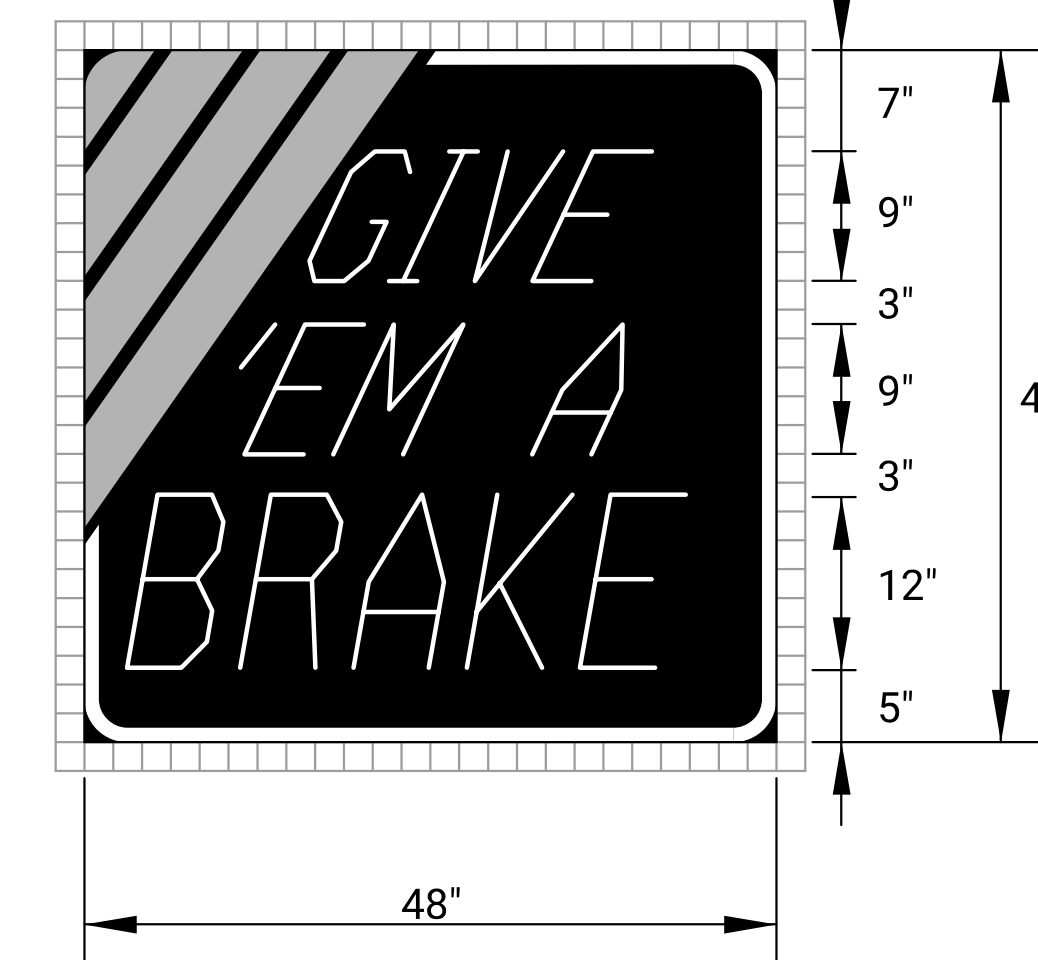
URBAN

- 1) Signs shall be mounted at a minimum height of 7' measured from the bottom of sign to the near edge of the pavement.
- 2) Neither portable nor permanent sign supports should be located on sidewalks or areas designated for pedestrian or bicycle traffic.
- 3) Signs mounted lower than 7' should not project more than 4" into pedestrian facilities.
- 4) The height from of the secondary sign mounted below another sign may be 6' measured from the bottom of sign to the near edge of the pavement. Signs shall not overlap each other.
- 5) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.
- * 6) Pedestrian detour signing shall be a minimum of 2' measured from the top of the pedestrian pathway to the bottom of the sign and shall not protrude into the walkway nor shall it project beyond the back of curb.

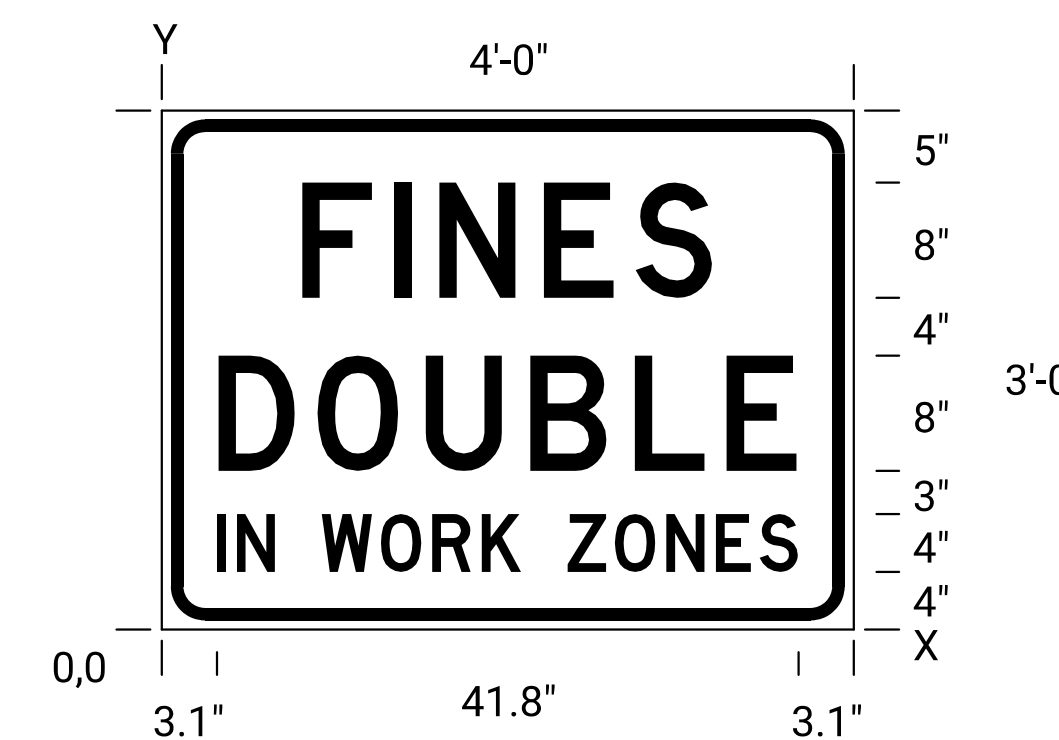


When the sign width is equal to or greater than 9', three or more wood posts may be used with a minimum of 4' between the centerline of each post. All signs less than 9' in width shall use a maximum of two wood posts.

- In the case of hitting rock when driving posts
1. Shift the sign location. Do not violate minimum sign spacing.
 2. With the engineer's approval, use acceptable alternative sign stands.



KI-104a



KI-105a

Sign Number	GIVE EM A BRAKE
Width x Height	4'-0" x 4'-0"
Border Width	1.0"
Corner Radius	4.0"
Stripe Width	3.0"
Mounting	Ground
Background	Type: Non-Reflective Color: Black
Legend/Border	Type: Reflective Color: White
Legend Font	Dutch 801 Roman SWC 25 Degree Slant
Stripes	Type: Reflective Color: Orange

Sign Number	FINES DOUBLE
Width x Height	4'-0" x 3'-0"
Border Width	0.9"
Corner Radius	3.0"
Mounting	Ground
Background	Type: Reflective Color: White
Legend/Border	Type: Non-Reflective Color: Black

Dimensions in inches Spacings are to start of next letter

Y FONT	LETTER SPACINGS													HT LEN	
23.0 D	F	I	N	E	S									8.0	
	9.7	6.4	3.2	7.3	6.4	5.4	9.7							28.6	
11.0 D	D	O	U	B	L	E								8.0	
	3.9	6.9	7.5	7.3	6.4	4.9	3.9							40.3	
4.0 D	I	N	W	O	R	K	Z	O	N	E	S			4.0	
	3.1	1.6	2.7	3.2	4.3	3.8	3.6	2.8	3.4	3.8	3.6	3.2	2.7	3.1	41.8

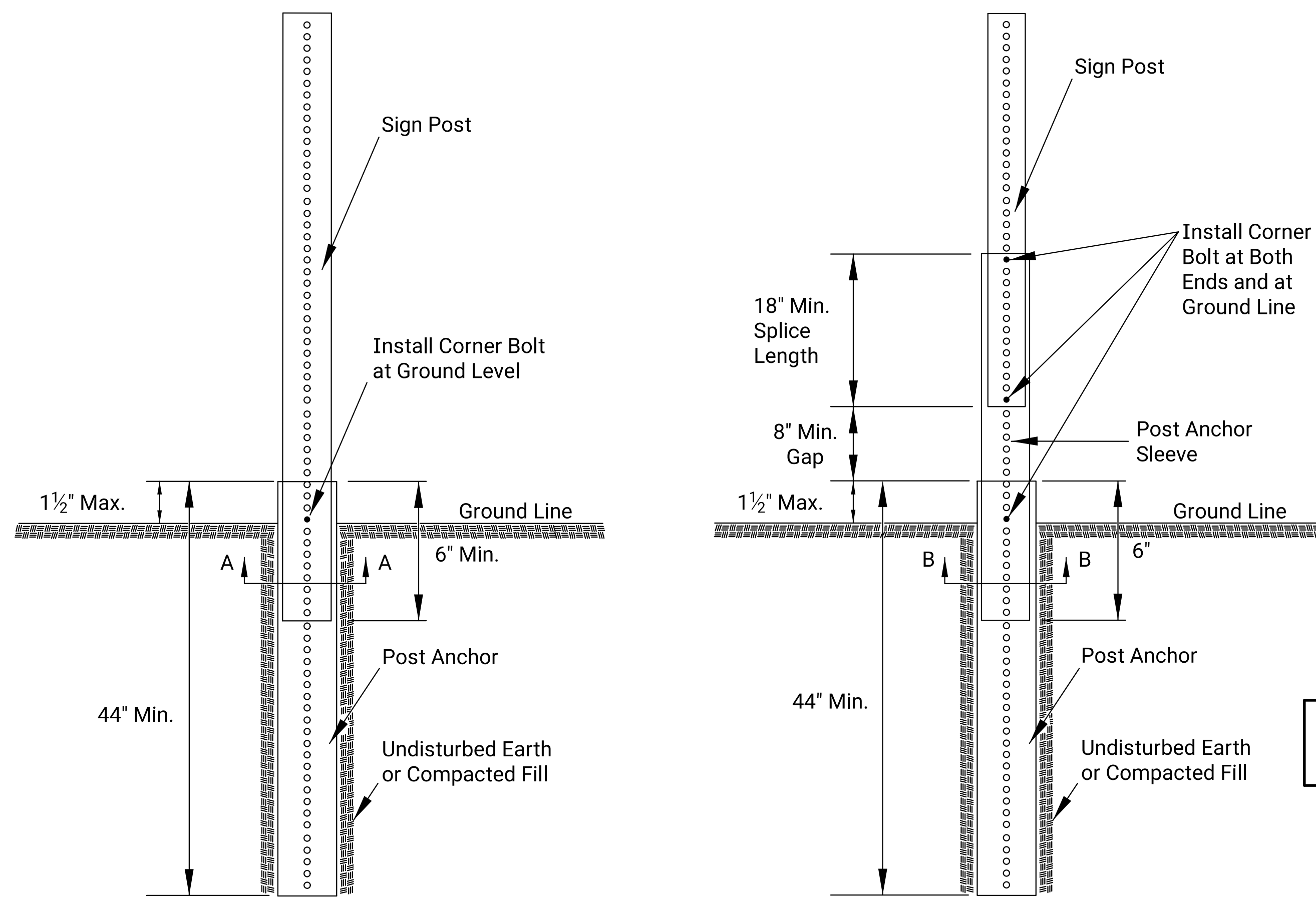
Notes:

- Typically, there are two sets of informational signs installed per project: one for each direction of traffic.
- Install signs a minimum of 500' in advance of the road work ahead sign. The engineer may designate a more appropriate location if conditions dictate.
- The informational signs are not to interfere with the traffic control signs for the project.

KANSAS DEPARTMENT OF TRANSPORTATION			
TRAFFIC CONTROL SIGN INFORMATION			
TE710			
FHWA APPROVAL	06-01-15	APPD.	Kristina Erickson
DESIGNED	R.W.B.	DETAILED	R.W.B.
QUANTITIES	TRACED	QUAN.CK.	TRACE CK.

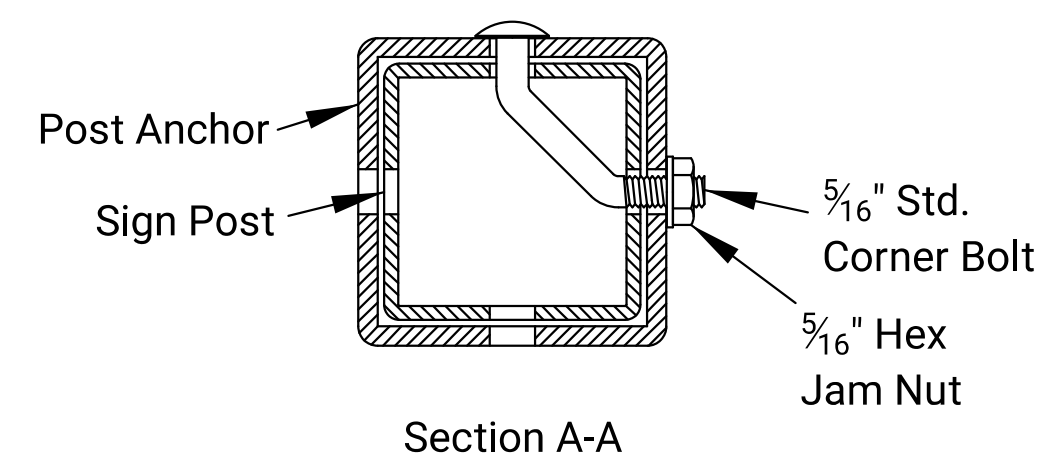
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	39	44

PERFORATED SQUARE STEEL TUBE (P.S.S.T.) POST SETUP

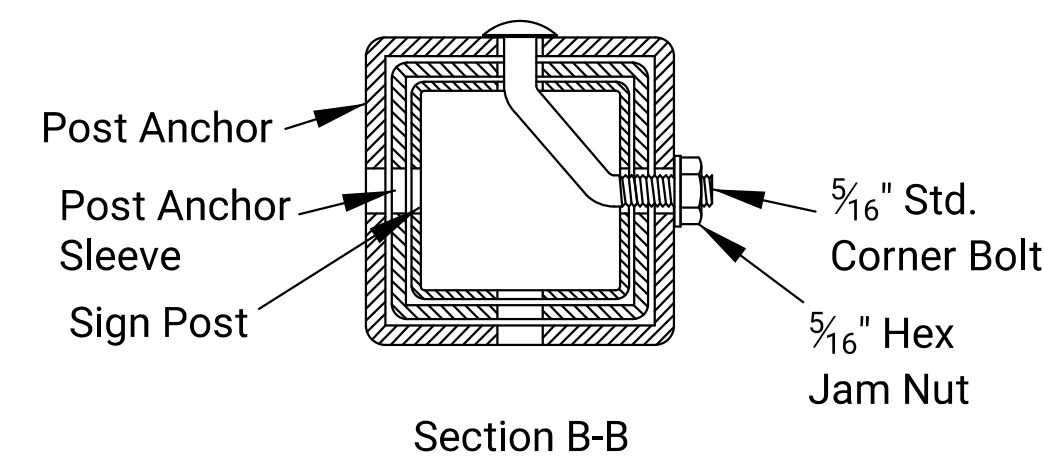


P.S.S.T. Detail

Telescoping P.S.S.T. Detail



Section A-A

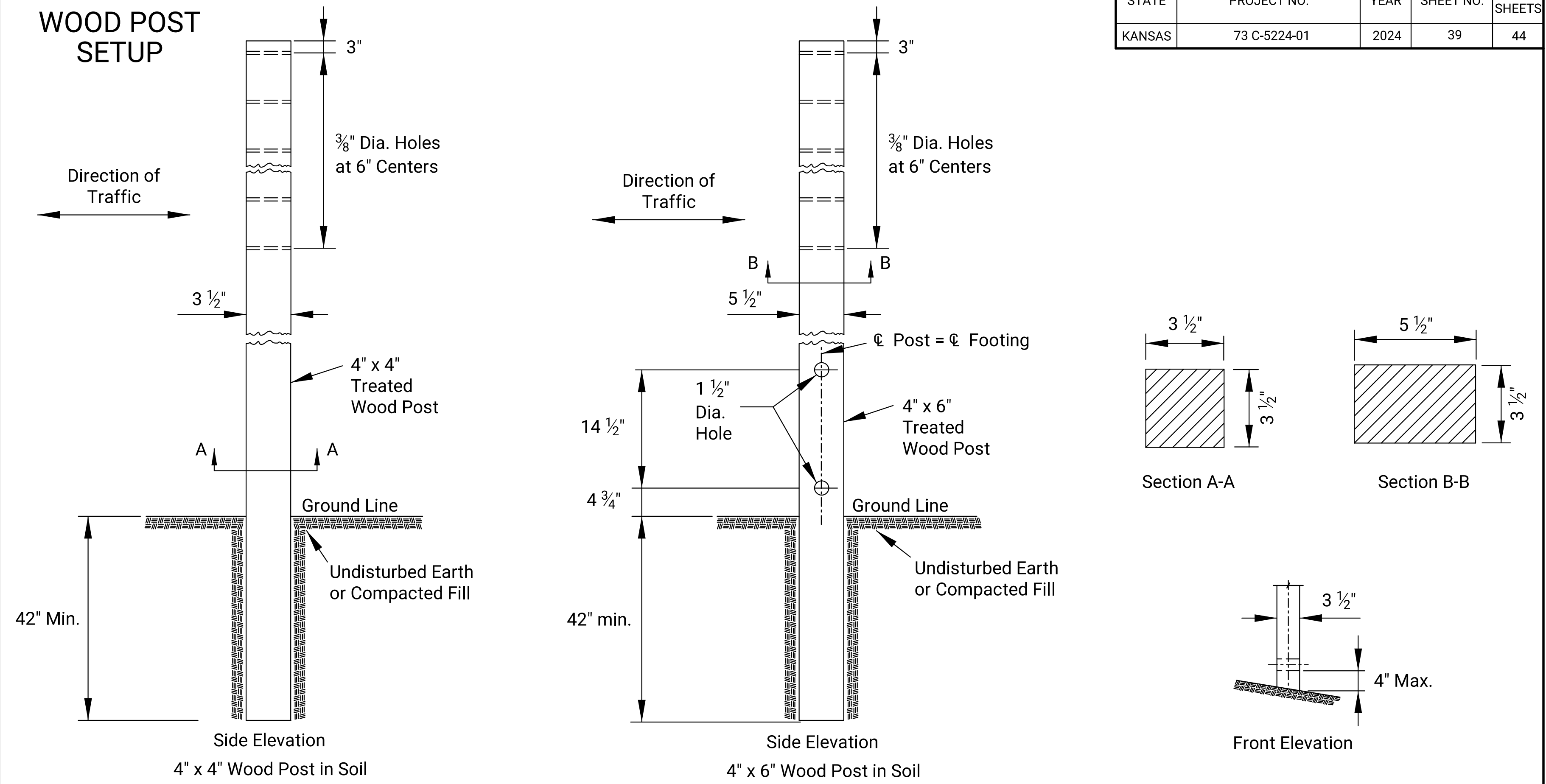


Section B-B

Details for 2", 2 1/4", or 2 1/2" sign posts

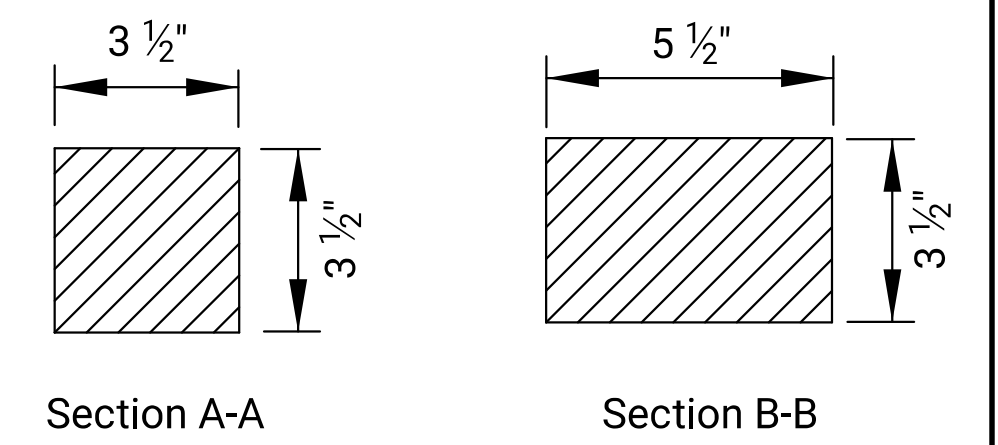
Place bolts in the same corner along each sign post.

WOOD POST SETUP



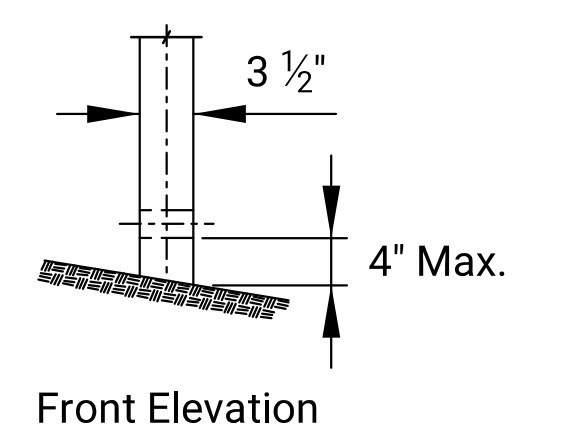
Side Elevation
4" x 4" Wood Post in Soil

Side Elevation
4" x 6" Wood Post in Soil



Section A-A

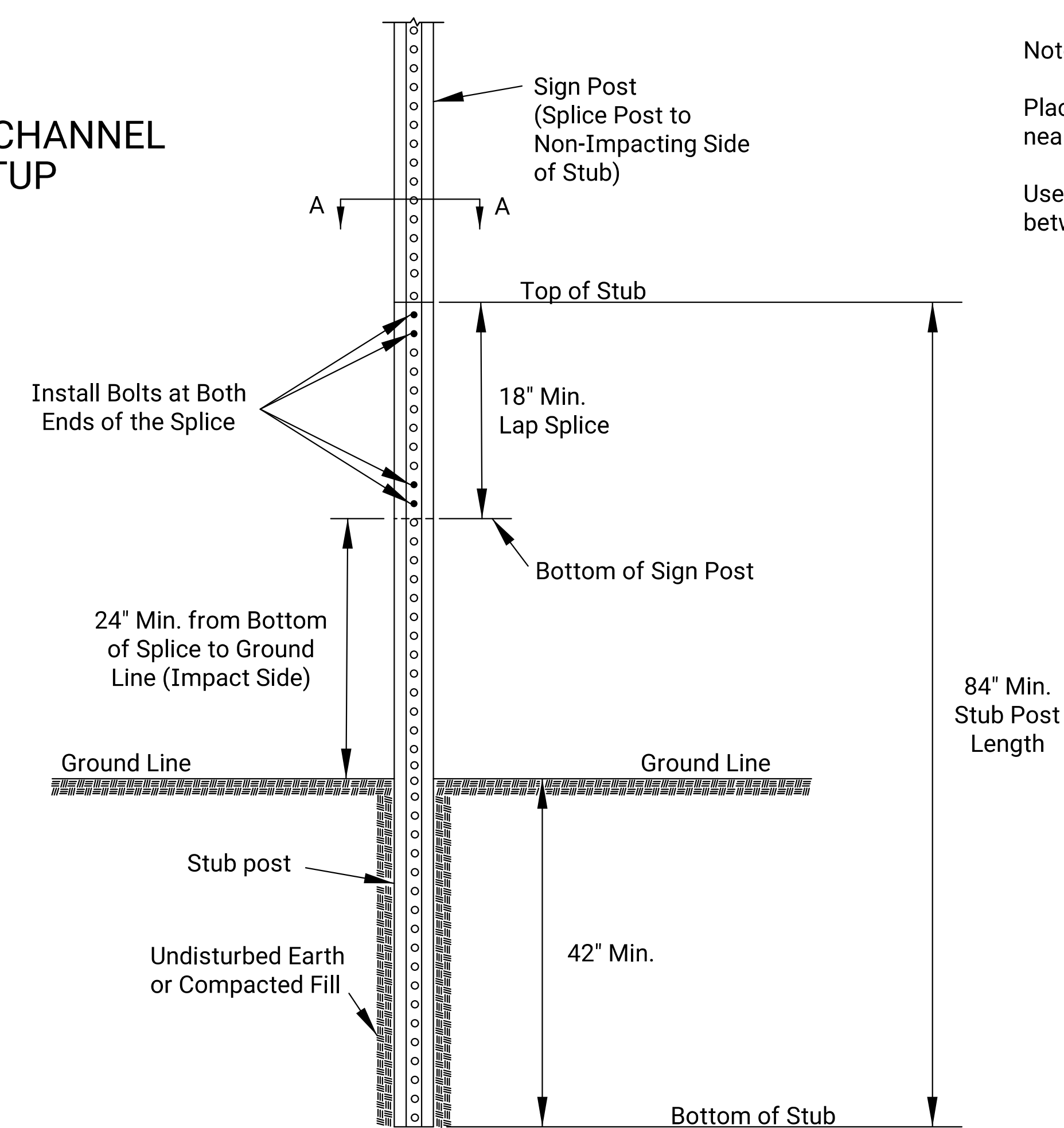
Section B-B



Front Elevation

See TE710 for Additional Details and Requirements

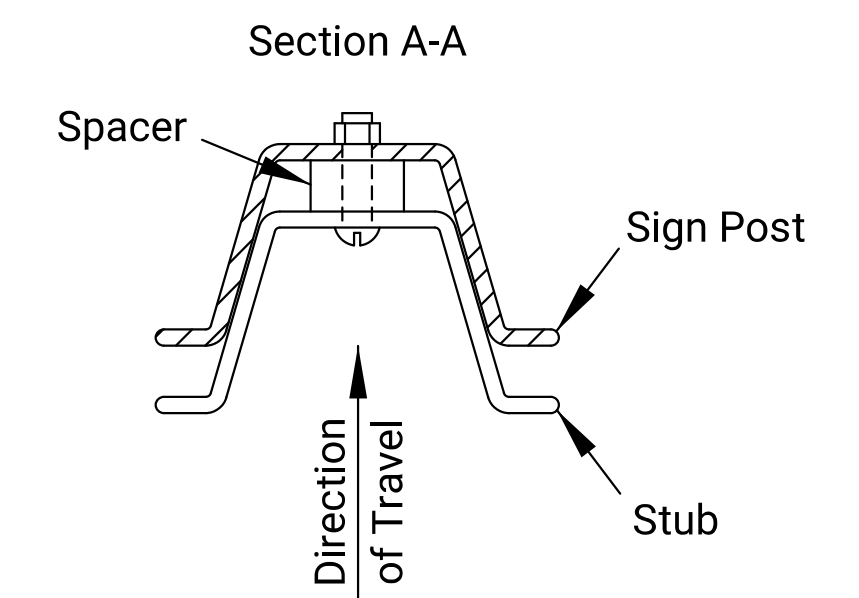
3 LB/F U-CHANNEL SETUP



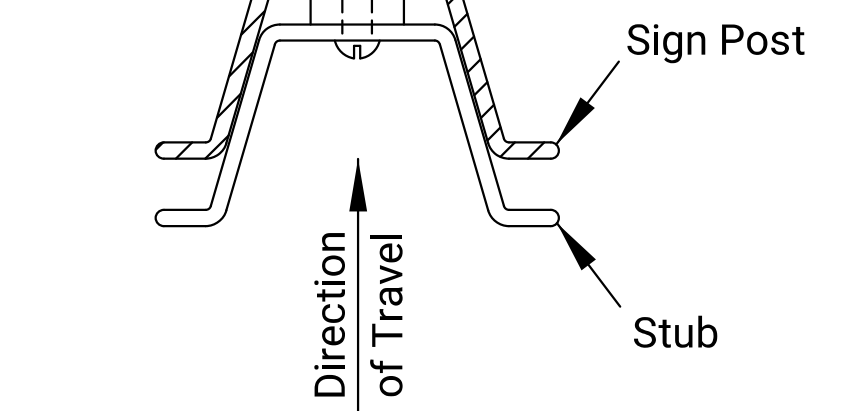
Notes:

Place two bolts at both ends of the splice through the holes nearest the ends of the splice.

Use manufacturer recommended spacers over the bolts between the spliced pieces of U-Channel.



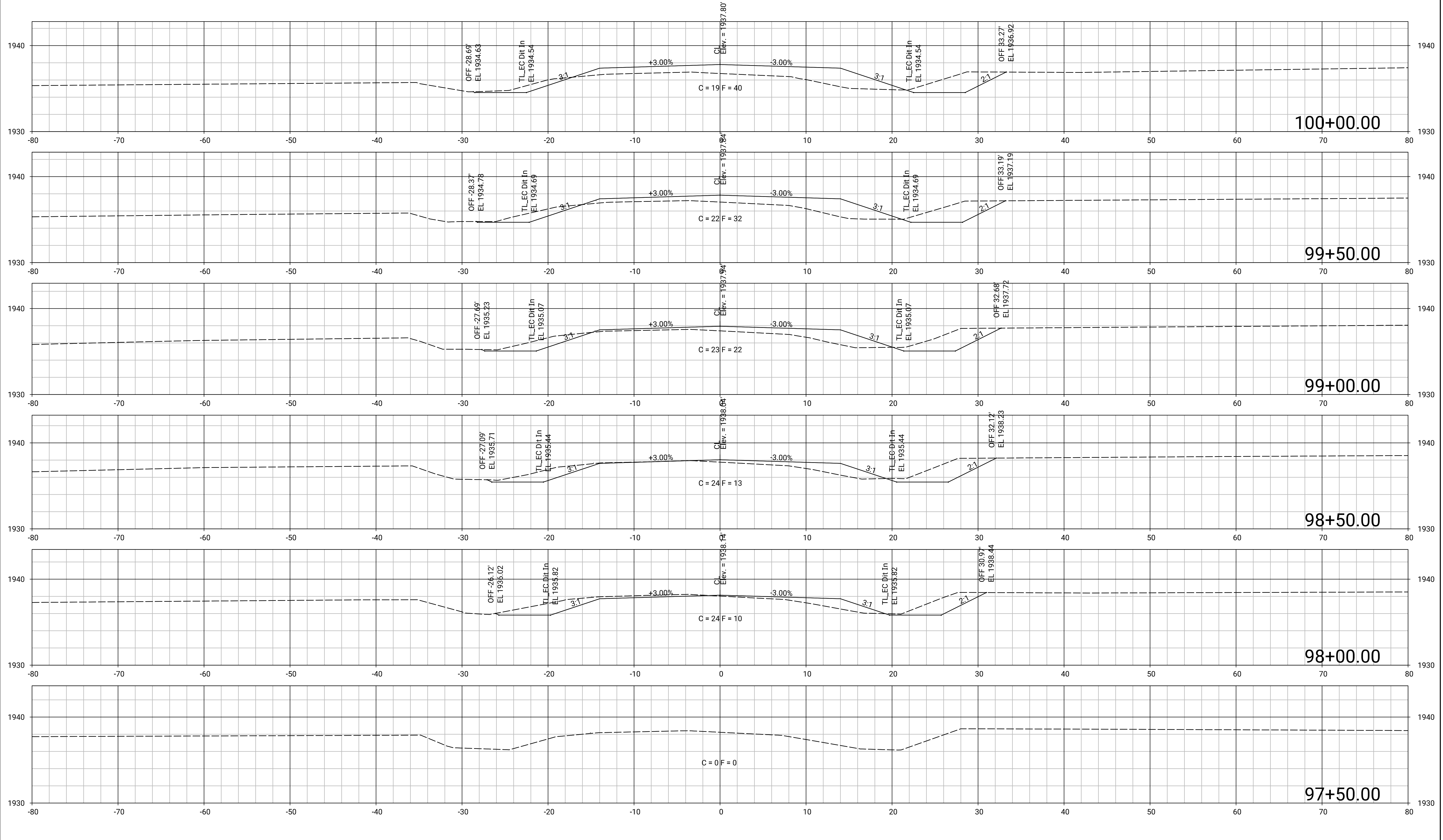
Section A-A



NO.	DATE	REVISIONS	BY	APPD
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL SIGN POSTS				
TE712				
FHWA APPROVAL	06-01-15	APPD.	Kristina Erickson	
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACED	TRACE CK.

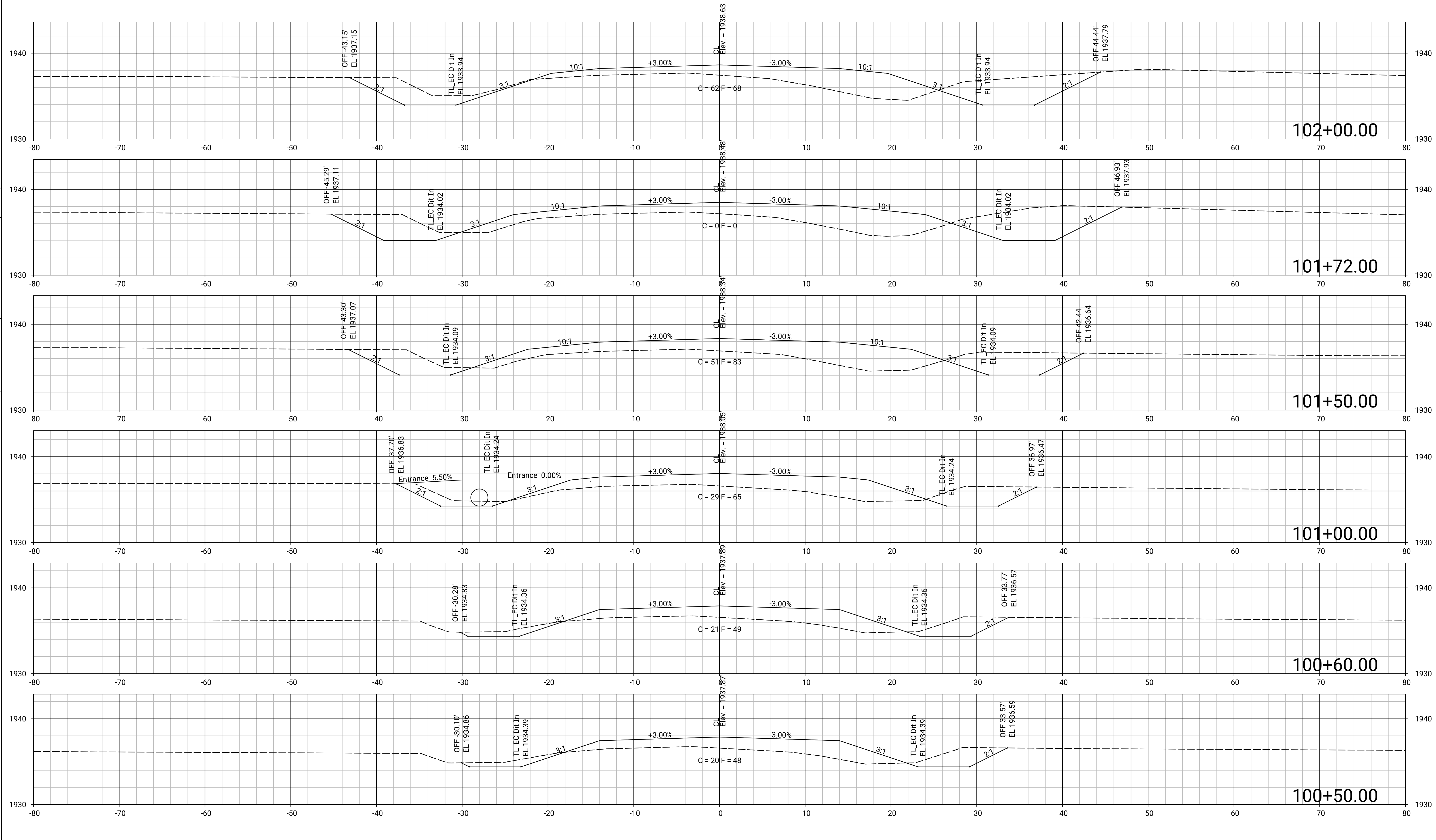
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	41	44

DATE	
BY	
REFERENCES NOTED	
REFERENCES CHECKED	



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	42	44

DATE	BY	REFERENCES NOTED	REFERENCES CHECKED

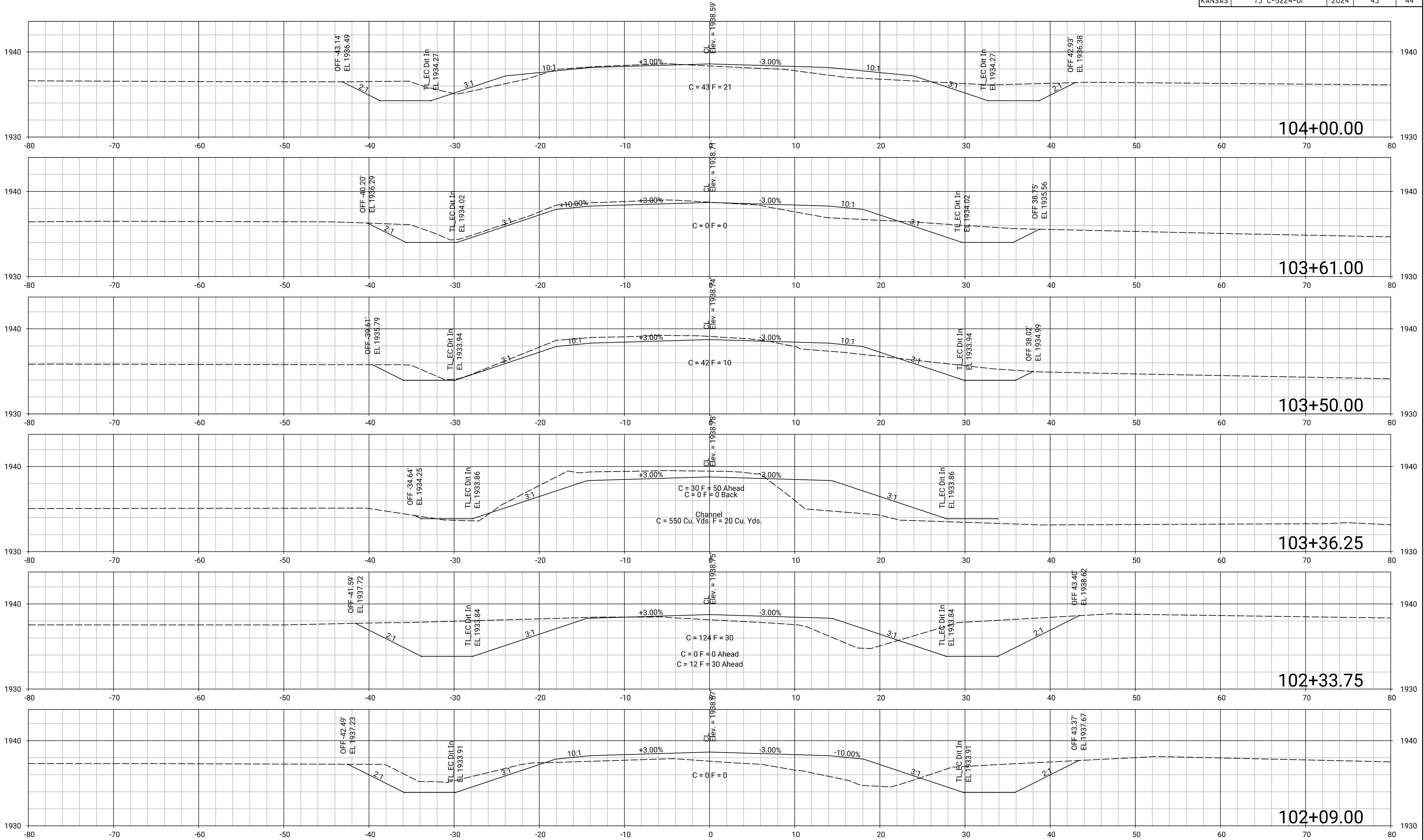


Plotted by : rsnow 3-JUL-2024 18:11
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STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	43	44

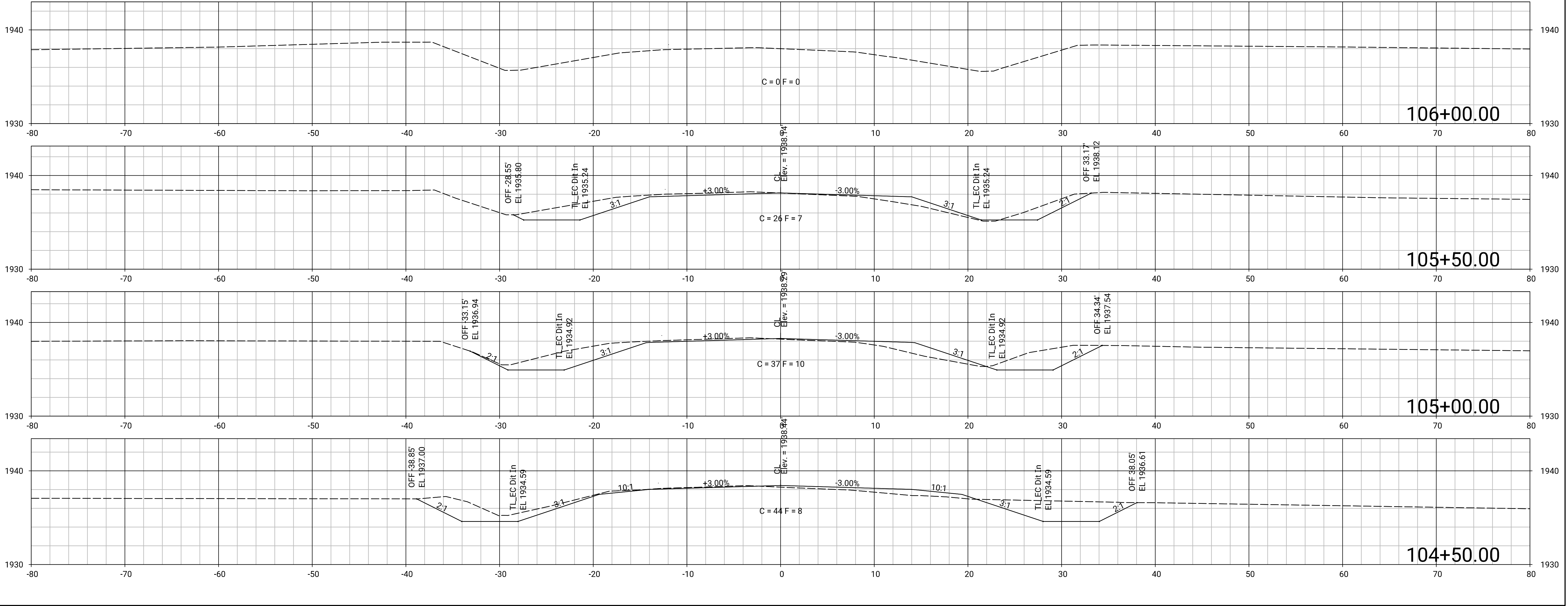
DATE	BY

Plotted by : rsnow 3-JUL-2024 18:11
 File : ka - rxs Container File.dgn



STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS	73 C-5224-01	2024	44	44

REFERENCES NOTED	DATE
REFERENCES CHECKED	



Plotted by : rsnow 3-JUL-2024 18:11
 File : ka - rxs Container File.dgn