Geotechnical Baseline Report for the Big Fill Landslide M.P. 12.8 Wyoming Highway 22 ND32401

Wilson - Idaho State Line



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Introduction

During the night of June 7th, 2024, a 200 foot section of WY-22, and associated fill, failed at what is known locally as the Big Fill (MP 12.8). Road distress on June 6th predated the failure and was evident in the form of roadway cracks and deformed guard rail. Roughly 70 feet of fill, a variable depth of native ground, and both lanes of WY-22 were involved in the landslide. The failure closed the road until a detour could be established to the south, inside the original road alignment curve.

Geologic Setting

Local geologic information, derived from mapping published by Pampeyan (1967), Schroeder (1972), and work done by Zeller (1982), shows a complicated network of thrust faulting and rock of varying lithologies and ages. The area immediately surrounding the Big Fill contains rocks of Cambrian, Ordovician, Mississippian, Triassic, and Cretaceous age thrust in multiple directions and tilted at various angles, in some places near vertical (Fig. 1).

Directly under the Big Fill, Schroeder maps a unit he names "Ku - Sedimentary Rocks, undifferentiated" and describes as "Mainly black shale and brownish-gray-weathering fine-grained 'salt and pepper' sandstone." Although undifferentiated in this specific area, Pampeyan (1967) and Schroeder (1969) map similar units to the west and south respectively as the Aspen Shale and Bear River Formation. It is difficult to determine if the Ku unit is the Aspen Shale or Bear River Formation, or a combination of the two, due to the complicated structure. In addition, Zeller (1982) maps the unit as the Cretaceous-aged Frontier Formation. Regardless of the actual formation present, all potential Cretaceous units in the area are known to include shale and clay.

Schroeder (1972) maps the Cache Creek Fault just north and the Jackson Fault just south of the Big Fill (Fig. 1). The Cache Creek Fault is thrust to the south, placing Triassic Chugwater (siltstone and shale) and Dinwoody formations (siltstone) on top of the Cretaceous undifferentiated rocks. The northward-thrusted Jackson Fault placed

Mississippian Mission Canyon Limestone, Ordovician Bighorn Dolomite, and Cambrian Gallatin Formation (dolomite, limestone, and shale) over the same Cretaceous unit (Fig. 1).



Figure 1. Map taken from Schroeder, 1972. Geologic units are as follows: Gallatin Formation ($\mathcal{C}g$), Bighorn Dolomite (Ob), Mission Canyon Limestone (Mm), Chugwater Formation ($\mathcal{R}c$), Darby Formation ($\mathcal{R}d$), and Sedimentary Undifferentiated (Ku). Big Fill noted by red arrow.

Original Design/Construction

Wyoming Highway 22 was built in the 1960s. The road section known as the "Big Fill" was constructed with an average grade of 10.5%. Based on available plans (Fig 2), the fill is generally 70-75 feet tall to the north, and 40-45 feet to the south. Plans also show the original ground surface under the fill is uneven. It is likely that fill thickness varies appreciably across the Big Fill. Additional construction plans for the fill have proven difficult to locate. However, based on fill exposed by the headscarp of the landslide the material was locally sourced. Likely from the nearby outcropping Mission Creek Limestone, Bighorn Dolomite, and other competent lithologies.



Figure 2. Original plans sheet dated 1964 showing design of the Big Fill

Event Description

Based on conversations with District 3 staff, primarily from the Jackson office, the roadway on the Big Fill has required occasional maintenance for several years. In 2022,

cracks developed in the response, roadway. In WYDOT geologists drilled and logged a borehole on the Big Fill in September of 2022 (SL22-1). The log record shows dense silty sand to a depth of ~50 feet and dense clayey sand with rock (possibly bedrock) to a total borehole depth of 79.5 feet Most recently, a portion of the roadway on the Big Fill was resurfaced in 2023, resulting in ~18 inches of pavement to correct the super elevation and improve ride and plowing conditions. Shortly after resurfacing was complete, WYDOT geologists, with the help of Salisbury Associates, installed an inclinometer tube (TBF-01) just north of the guardrail (SL23-1), near SL22-1 in August of 2023. The inclinometer log shows the



borehole passed through the existing fill (~85 feet) into an olive-gray to gray fine grained sandstone. Readings of the inclinometer on May 24th, 2024 showed ~0.10 inches of movement had occurred since instillation (Fig. 3).

A significant weather system moved into the area of Teton Pass a week prior to the failure. On June 3rd a U.S. Department of Agriculture SNOTEL site located on Phillips Bench (3 miles east of the Big Fill) recorded a daily rainfall total of 1.2 inches. The same SNOTEL site, and a Bridger-Teton Avalanche Center weather station (MP 12, WY-22) showed a significant increase in temperatures after the precipitation event which continued up to and past the failure event. Perhaps more significantly, the stations show minimum temperatures no longer dropping below freezing after May 30th, a week prior to failure (Fig. 4).



Figure 4. Graph showing maximum, minimum, and average temperatures (F°) at the Phillips Bench (PB) and Milepost 12 (MP 12) weather stations. Black horizontal line denotes 32° F. Blue vertical line denotes June 7th, 2024.

On June 6th a motorcyclist lost control after hitting a bump in the roadway at about MP 12.8. Wyoming Highway Patrol responded and contacted the Jackson regional office to investigate the roadway. Additional road deformation, including sharp bumps and arcuate cracks, was identified. The cracks were marked to monitor potential movement and

sealed by District Maintenance. WY-22 was closed temporarily while the road was being looked at until the determination to reopen the roadway was made. Construction cones were placed around road damage to direct traffic and the road was reopened the afternoon of June 6th. During their investigation staff from the Jackson regional office noted tension cracks in the fill which extended from the arcuate cracks in the roadway north to the base of the fill. Staff also noted a spring, roughly 45 feet below the road surface. The spring was emitting orange mud and water (Fig 5), flows were approximated at less than 1 cfs.

WYDOT geologists mobilized on June 6th, reaching the site on June 7th. Over the night of June 6th, a mudslide overtopped WY-22 at MP 15, closing the road. A forthcoming baseline geotechnical written by T. Sullivan will detail that event. Geologists and Jackson office personnel observed the cracks, filled on the 6th, were noticeably wider, up to 2 inches, and had offset vertically. Paint was used to remark the cracks and lathe was placed and marked for continued measurement purposes. Inclinometer TBF-01, located just north of the westbound guardrail on the fill, was measured and was sheared at 70 feet. Tension cracks in the fill, more prevalent than the day before, were walked by geologists down the fill slope to the north and into the native ground. Cracks were primarily oriented north-to-south, however a few cracks were found in a west-to-east orientation about 50 feet to the north of the fill/native ground transition. The spring continued to emit similar material, however staff noted the flow seemed to have slightly increased from the day before. Returning to the road surface, staff noted that the cracks in the roadway had widened and offset vertically further. Measurements of lathe showed additional vertical displacement of 3 inches over the course of an hour (Fig 5).

Due to the additional movement, it was decided by WYDOT staff to attempt to remove the driving force from the instability by stripping the pavement from the failure. An emergency contract was entered into with Evans Construction (Jackson, WY) to begin removing pavement and guard rail the afternoon of June 7th (Fig. 5). The roadway continued to move as work began on the roadway, reaching up to 6 inches per hour by 6:00 p.m. on June 7th. Work continued until 10:00pm when darkness and crew safety necessitated stoppage. Overnight on June 7th, a 200 foot length of WY-22 and associated fill failed in a northward direction. The failure plane continued from the shoulder of the eastbound lane, through the fill, and continued into a variable thickness of native ground (Fig. 5). WY-22 had remained closed since the mudflow earlier on June 7th. Instability in the failure continued for roughly 12 hours at which point ground conditions stabilized, excluding an occasional minor rock fall.



Figure 5. Images taken on June 6th and 7th, prior for failure, showing: 1) Cracking of the roadway, top photo looking west, bottom photo looking east, 2) Spring near base of fill slope with flowing water and orange mud, 3) Observed vertical movement over the course of an hour on June 7th, and 4) Removing pavement to decrease driving forces on the evening of June 7th.

Geotechnical Investigation/Results

Post Event Field Observations

After the failure, a site investigation noted groundwater was seen seeping out of the headscarp at roughly 20 feet below the top of remaining fill (Fig. 6). A second "spring" was encountered at the base of the headscarp (Fig 6). The lower spring is considered to be at, or near, the contact with native ground. The landslide itself is roughly 70,000 square feet in total with a maximum headscarp height of ~70 feet. The free face of the headscarp shows only fill material, native ground is not exposed.

Landslide debris extends downslope, to the north, where it dammed a small creek. Water in the creek had pooled behind the toe of the landslide to a three foot depth, however water was moving under the slide mass, exiting the mass to the west. The slide body is made up of mixed material with no discernable blocks of bedrock. Numerous springs were observed within the body of the slide as well.

Geologists investigated the steep and heavily treed area surrounding the landslide. The perimeter of the landslide was traversed, including hillsides south, east and west of the slide. For the most part the local geology is consistent with Schroeder's mapping. The small knoll to the west of the Big Fill is made up of Mission Canyon Limestone and the ridge to the east is composed of competent rocks of the Mission Canyon, Bighorn Dolomite and Gallatin Formation. Lower slopes, north of the ridges, are hummocky and contain moist areas (Fig 6).

At the time of site investigation, surface water was flowing towards the Big Fill from the ridge to the east. Water was collecting in a ditch roughly 200 feet up the slope from the roadway borrow (Fig 6) and in the borrow ditch itself. In both cases water was infiltrating into the ground and not continuing at the surface down-drainage. A spring was discharging water at the base of the fill downhill from the ditches but east of the landslide (Fig 6). No apparent surface water was flowing eastward from the knoll, suggesting the majority of the water in the fill embankment originated from the east.



Figure 6. Field images showing: 1) Seep in head scarp (red arrow), 2) Spring at base of head scarp, 3) Man made ditch in forest east of the Big Fill, 4) Spring downhill from ditch, east of the Big Fill, and 5) Hummocky and wet ground west of the Big Fill.

Geotechnical Investigation

In total, 14 boreholes have been drilled in the immediate area of the Big Fill. Prior to the failure, two boreholes, SL22-1 and SL23-1 were completed. Post failure, 12 borehole sites; nine test holes (24-01 – 24-08, 24-11), one water monitoring well (24-10), and two inclinometer tubes (24-09 and 24-12) were drilled (Fig. 7). Full borehole logs with select cross sections and laboratory results for all sites can be found in Appendix A and B respectively.

SL22-1 was drilled in October of 2022 to assess the fill and underlying native ground with rig 823 (CM-75). The borehole reached a depth of 79.5 feet. Although not definitive, native ground was likely encountered at about 50 feet. 10 Standard Penetration Tests (SPTs) were completed in the fill and native ground.

Borehole SL23-1 was drilled to a depth of 115 feet by Salisbury Associates in August of 2023 to install an inclinometer (TBF-01) in response to road distress. An olive-gray to

gray sandstone was encountered at 85 feet and is considered to be the contact between fill and native ground at this site. Two SPTs were completed and samples were submitting for classification to the Foundation Laboratory. A memorandum, written by T. Sullivan, regarding SL23-1 can be found in Appendix C.

Beginning on June 10th, 2024 and continuing for the next three weeks, WYDOT Geology completed 12 boreholes with rig 4001. A split spoon sampler was used to collect 69 samples during Standard Penetration Testing. Of the samples collected, 37 were submitted to the Foundation Lab for characterization. Seven grab samples were also collected, five from existing fill (BF S#1-S#5), one from outcrop east of the landslide (OS-01) and one at site 24-02 for an Alkali Test (Fig. 7). Lab results for samples can be located in Appendix B.



Figure 7. Map showing borehole, fill, and outcrop sample locations at the Big Fill.

Geotechnical Results and Discussion

Classification of samples taken in the fill at sites SL22-01 and SL23-01 show variable percentages of fines passing a #200 sieve. Values range from 9 to 45% depending on the depth of sample (Table 1.) AASHTO classification are primarily group A-1 for fill

samples, however borehole SL22-1 shows an A-4 at a depth of 68feet (7589 feet.) At the time of sampling, moisture content was less than 10%, except for the A-4 sample which shows a slightly higher moisture content of 13.5. Uncorrected N-Values primarily ranged from 13-30, while a higher blow count of 47 was recorded at 58 feet.

SPT tests taken in native ground largely show A-6 classification. The moisture content of the A-6 samples varies from 8 to 21% with an average of 13.5%. Many of the samples taken in the native ground are made up of weathered to un-weathered shale, although there are sandy intervals encountered on occasion. Uncorrected N values range from 10 to refusal depending on depth of sample (Table 1).

Samples from boreholes 24-01, 24-02, 24-08, and a sample collected from an outcrop east of the landslide rated as A-7 (Fat Clay). The fat clay samples obtained from sites 24-01 and 24-02 are comprised of white to gray homogenous clay with moisture contents higher than the A-6 samples (28-33%). Samples 12 and 13, taken from 24-02, were remolded and tested for direct sheer. Results showed a peak ϕ angle of 15 degrees and a residual of 10. Clays sampled at 24-08 and at outcrop are gray-maroon clays with moisture contents of 34 and 17% respectively (Table 1). The elevation of the fat clay layer in boreholes 24-01 and 24-02 is 7536 feet and 7541 feet respectively, thicknesses of the fat clay varies from 16 feet at borehole 24-02 to 6 feet and 22-01. Stratigraphic tops or thicknesses were not obtained at 24-08 or outcrop.

Test Hole	Sample Number	Sample Depth	Lithology	Uncorrected N-Value	AASHTO	< #200	M%
SL22-1							
	1	8.0' - 9.5'	silty gravel with sand	48	A-1-b	18.9	3.8
	2	18.0' - 19.5'	clayey gravel with sand	13	A-2-6(0)	25.3	7.2
	3	28.0' - 29.5'	silty gravel with sand	15	A-1-b	19.4	4.6
	4	38.0' - 39.5'	well-graded gravel with clay	30	A-2-4(0)	8.9	1.8
	5	48.0' - 49.5'	silty clayey gravel with sand	22	A-1-b	19.4	4.8
	6	58.0' - 59.5'	silty gravel with sand	47	A-1-b	23.4	5.3
	7	68.0' - 69.5'	silty gravel with sand	21	A-4(0)	45.3	13.5
	8	78.0' - 79.5'	silty gravel with sand	29	A-1-b	19.5	8.1
SL23-1							
	1	10.5' - 11.5'	silty gravel with sand	21	A-1-a	13.7	1.6

Table 1.	Classification	results for	selected	SPT samples
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	2	20.5' - 21.5'	poorly graded gravel with silt and sand	12	A-1-a	11.2	5.5
SL 24-1							
	3	18.0' - 19.5'	gravelly lean clay	49	A-6(6)	63.8	10.6
	6	33.5' - 34.5'	fat clay	13	A-7-6(30)	85.6	28.8
	8	43.0' - 43.8'	lean clay with sand	100/0.25'	A-6(7)	84.8	13.9
SL 24-2							
	12	38.5' - 39.5'	Remold with #13	15	NA	NA	NA
	13	43.5' - 44.5'	Remold with #12	27	NA	NA	NA
	14	48.5' - 49.5'	fat clay	18	A-7-6(53)	98.8	33.3
	16	58.5' - 59.5'	lean clay with sand	80	A-6(7)	79.8	8.7
SL 24-3							
	20	32.5' - 33.5'	sandy lean clay	42	A-6(8)	57.8	16.0
	21	37.5' - 38.5'	lean clay	54	A-6(10)	98.1	14.4
	22	42.0' - 43.1'	sandy lean clay	100/1'	A-6(4)	66.9	9.9
SL 24-4							
	24	18.0' - 19.0'	sandy lean clay with gravel	NA	A-6(4)	52.8	16.0
	25	29.5' - 30.5'	silty clay with sand	25	A-4(3)	80.0	15.0
	27	49.5' - 50.5'	lean clay	52	A-6(17)	89.7	14.6
SL 24-5							
	31	29.5' - 30.5'	lean clay with sand	20	A-6(10)	78.4	19.8
	32	39.5' - 40.5'	lean clay with sand	24	A-6(15)	74.3	20.1
	33	49.5' - 50.5'	gravelly lean clay with sand	21	A-6(6)	51.1	13.2
	34	59.5' - 60.5'	sandy lean clay	31	A-6(7)	70.4	21.1
	35	69.0' - 70.2'	lean clay with sand	100/0.7'	A-6(7)	80.0	15.0
SL 24-6							
	36	13.0' - 14.5'	sandy lean clay with gravel	10	A-4(3)	62.7	11.5
SL 24-7							
	43	18.0' - 19.5'	lean clay	16	A-6(14)	92.0	15.8
	49	68.0' - 69.5'	lean clay	56	A-6(21)	89.7	9.5
SL 24-8							
	52	24.0' - 25.5'	fat clay	13	A-7-	95.0	34.2
					6(101)	51.0	11.0
	55A	54.0' - 55.0'	silt with sand	98	A-4(0)	71.2	11.9
0.0.01	55B	55.0' - 55.5'	lean clay	NA	A-4(4)	86.0	8.3
OS-01							1
OT 04 44	60	0.0'	sandy lean clay with gravel	NA	A-7-6(9)	56.8	15.2
SL 24-11						10.5	
	64 and 65	32.0' - 33.5'	clayey gravel with sand	18	A-6(3)	40.6	9.0
	66	42.0' - 43.5'	clayey gravel with sand	36	A-2-6(2)	30.5	8.1

Based on borehole logs and cross sections (Appendix A), it is expected that bedrock (shale) will be encountered at decreasing elevations to the north in the Big Fill area (Fig. 8). Bedrock can be found at a higher elevation to the south which is consistent with elevation trends shown in the original road plans. The fat clay encountered in 24-01 and

24-02 should be expected at ~7540 feet and higher near borehole 24-08 (7582 feet). Based on the complexity of local geology it is difficult to project any specific lithology or strata between boreholes with confidence. Numerous SPT samples suggest vertical framework within the shales as collected, suggesting steeply dipping geologic units. The logs and cross sections also hint at a complex substructure with varying lithologies which is consistent with the geologic interpretations of Schroeder and Zeller.



Figure 8. Map showing elevations of first encountered bedrock (shale).

Modeling and Results

WYDOT contracted with RJ Engineering and Consulting, Evergreen, CO, to model the slope failure using internally collected geotechnical information and calculate a Factor of Safety for the post-failure slope and expected detour. Found in Appendix D, the report also makes monitoring recommendations.

References

Pampeyan, E.H., Schroeder, M.L., Schell, E.M., and Cressman, E.R., 1967, Geologic map of the Driggs quadrangle, Bonneville and Teton Counties, Idaho, and Teton County, Wyoming, U.S. Geological Survey, Mineral Investigations Field Studies Map MF-300, 1:31,680.

Schroeder, M.L., 1972, Geologic map of the Rendezvous Peak quadrangle, Teton County, Wyoming, U.S. Geological Survey, Geologic Quadrangle Map GQ-980, 1:24,000.

Zeller, C.G., 1982, Structural Geology Along Teton Pass, Wyoming, Rocky Mountain Association of Geologists Geologic Studies of the Cordilleran Thrust Belt, pg. 831-842, Vol II, 1982

APPENDIX A – BOREHOLE LOGS AND ASSOCIATED CROSS SECTIONS

Borehole logs and cross sections contained in Appendix A were created using BoreDMTM software. Soil and rock descriptions are generalized for display and organization purposes. Laboratory results displayed on logs and cross sections are also found in Appendix B – Borehole and Sample Results, please refer to Appendix B for detailed information.

ND32401 - The Big Fill - Borehole and Cross Section Plan View

Cross section lines are approximate. Lines CS-01, 02, 03, and 04 run West to East. CS-05, 06, and 07 run South to North.



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\$	Cross	Section 02									
3	Cross	Section 03									
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\$	Cross	Section 05									
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(Continued Next Page)

WYOMING DEPARTMENT OF TRANSPORTATION	LOG OF BORING	BORING NO. SL22-1 PAGE OF 3 3
PROJECT B229066 (Statewide Slide monitoring)	STATION	NORTHING LATITUDE 43.503675
LOCATION Wilson-Idaho State Line	OFFSET	EASTING LONGITUDE -110.975851
DATE STARTED DATE COMPLETE	0 10/12/2022 SURFACE ELEVATI	ON TYPE <u>SL</u> TOTAL DEPTH <u>79.5 ft</u>
DRILLER G. Olson GEOLOGIST B. 3		LING NATD
DRILL RIG 823-CME 75 ALKALI SAMPLES	AFTER/A	
DRILLING METHOD DRIVE POINT/ DRILL RATE (min/ft) d (ft) SAMPLE TYPE & NUMBER & NUMBER & NUMBER RECOVERY (ft) CORE REC % (ft) RCOVERY (ft) CORE REC % (rt) RCOVERY (ft) CORE REC % (rt) RCOVERY (ft) CORE REC % (rt) NANE SHER (rt) NANE SHEAR (rt) NANE SHEAR (rt) CONNTS (rt) NANE SHEAR (rt) CONNTS	UNCONFINED (ksf) (ksf) (ksf) (ks) (%) PLASTICITY INDEX (%) (%) (%) GRAPHIC LOG	MATERIAL DESCRIPTION ified (Visual) Classification System for Soil Graphic Log based on Unified (Visual) Classification (Consistency, Color, Soil Type, Degree Saturation)
	B.1 NV NP 19.5	layey sand with rock, possibly Bedrock

Bottom of borehole at 79.5 feet.

	WYOMING DEPARTMENT						DG	OF BORING			BORING NO.	SL23-01							
	BAIMAN			KAN	500	RIAII	UN							•.			PAGE 1	OF 4	
PR	OJEC	ТВ	322906	6 (St	atewic	de Slide	e mor	nitori	ng)					ST	TION	NN	ORTHING	LATITUDE 43.503	3704
LO	CATIC	ON _	Wilsor	Idah	o Sta	te Line		_					_	OFI	SET	EA	ASTING	LONGITUDE -110	.97584
DA	TE ST	ART	ED _	08/01	/2023	DA	TE C	OMP	LETE	D_0	8/04/	2023		SU	RFAC	E ELEVATION	TYPE _SL	TOTAL DEPTH1	15 ft
DR	ILLER	₹ <u>C</u> a	aleb Ze	emme	er	GE	OLO	GIST	<u>T. S</u>	ulliva	an		_	Į,	AT TI	ME OF DRILLING	NATD		
DR	ILL R		_F70			ALł	KALI	SAN	IPLES	_				Ţ,	FTE	R N/A			
DRILLING	DRIVE POINT/ DRIVE POINT/ DRILL RATE (min/ft)	O DEPTH (ft)	SAMPLE TYPE & NUMBER	SAMPLE	CORE REC % (RQD %)	BLOW COUNTS (N VALUE)	VANE SHEAR (ksf)	NSCS	AASHTO CLASSIFICATION	UNCONFINED (ksf)	WATER CONTENT (%)	LIQUID LIMIT	PLASTICITY INDEX	PASSING #200 (%)	GRAPHIC LOG	M Unified (Vi Graphic L (Consiste	IATERIAL DESCRIPT isual) Classification S .og based on Unified (Visual ency, Color, Soil Type, Degr	FION ystem for Soil) Classification ee Saturation)	ELEVATION (ft)
IGEIDRAFTIBORINGS.GPJ 2 3/8" CORE BARREL		 5														Sand, gravelly, <i>Mostly plow san</i>	loose, light tan nd - <1/2"	6.5	
22_MP12.8																Boulder/cobbles	s, limestone, gray, har	d.5	
FILL_WY2																		10.0	
11 A WI DRIVE FUINT - VVURNING DATA TEMPLATE,GUT - 0/0/24 13:26 - J.:LANDSLIDESTETON FASSITHE		 <u>15</u> 20 <u>25</u> <u>30</u>	2	0.5	0%	04-5-7 (12)										Isolated clayey z	zones - lots of washou	ut	
העוואט רעיט זע געונ	20						Contin	ued	Next F	age)									

1	v 🔊	VYOM	ING	DE	PART	ME	NT				10				DINC	BORING NO.	SL23-01	
		F TR	ANS	PO	RTATI	ON						JG	Ur		JKING	PAGE 2	OF	
PRO	JECT B	229066	(Sta	tewic	le Slide	mor	nitori	ng)			-		ST	ATION	N		LATITUDE 43.503	704
LOC		Vilson-	Idaho	Stat	te Line								OF	FSET	E/	ASTING	LONGITUDE -110.	97584
DAT	E START	ED 08	3/01/2	2023	DAT	TE C	omp	LETE	D_0	8/04/2	2023		SU	RFACE	E ELEVATION	TYPE SL	TOTAL DEPTH 11	5 ft
DRI	LLER <u>Ca</u>	leb Zer	nmer		GEO	OLO	GIST	<u>T. S</u>	ulliva	in			Į,	AT TIN	E OF DRILLING	NATD		
DRI		F70				(ALI	SAN	IPLES			-		Ţ,	AFTER	R <u>N/A</u>			
DRILLING	DRIVE POINT/ DRILL RATE (min/ft) C DEPTH (ft)	SAMPLE TYPE & NUMBER	SAMPLE RECOVERY (ft)	CORE REC % (RQD %)	BLOW COUNTS (N VALUE)	VANE SHEAR (ksf)	nscs	AASHTO CLASSIFICATION	UNCONFINED (ksf)	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	PASSING #200 (%)	GRAPHIC LOG	N Unified (Vi Graphic L (Consist	MATERIAL DESCRIPT isual) Classification Sy .og based on Unified (Visual) ency, Color, Soil Type, Degre	ION rstem for Soil Classification ee Saturation)	ELEVATION (ft)
															Gravel, sandy v (continued)	vith random Cobbles a	nd Boulders, dense	

WYOMIN		NT	1.00		BORING NO.	SL23-01
OF TRAI	NSPORTATION		LOG	OF BORING	PAGE	OF
PRO IECT P220066 (Statawida Slida mar	siterine)				
LOCATION Wilson-Ida	aho State Line	hitoring)				LATTUDE 43.503704
DATE STARTED 08/0	01/2023 DATE C	OMPLETED	08/04/2023	SURFACE ELEVATION	TYPE SL	TOTAL DEPTH 115 ft
DRILLER Caleb Zemn	mer GEOLOG	GIST T. Su	llivan	∇ AT TIME OF DRILLING	NATD	101/2021 111 <u>- 110 12</u>
DRILL RIG _LF70	ALKALI	SAMPLES		AFTER N/A		
£			⊢ ×			
DRILLING METHOD DRIVE POINT/ DRILL RATE (mint d (ft) (ft) (ft) SAMPLE TYPE & NUMBER	RECOVERTEC COREREC % (ROD %) BLOW BLOW COUNTS (N VALUE) VANE SHEAR (ks)	USCS AASHTO CLASSIFICATION	UNCONFINED (Ksf) WATER CONTEN (%) LIQUID LIMIT (%) PLASTICITY INDE	Unified (%) Unified (%) Unified (%) Graphic (Consi	MATERIAL DESCRIPT /isual) Classification Sy Log based on Unified (Visual) stency, Color, Soil Type, Degre	TON vstem for Soil Classification
				(consi Gravel, sandy (continued)	ve-gray to gray, hard, fi 45 to 65 degree fracture and weathered below 1	84.8 ne-grained, fractured s. Sandstone is 04 feet.

WYOMING DEPARTMENT OF TRANSPORTATION	LOG OF BORING	BORING NO. SL23-01 PAGE OF
BRO IECT R220066 (Statewide Slide menitering)		
LOCATION _Wilson-Idaho State Line	STATION NO OFFSET EA	STING LONGITUDE -110 97584
DATE STARTED	D _08/04/2023 SURFACE ELEVATION	TYPE_SL_TOTAL DEPTH _115 ft
DRILLER Caleb Zemmer GEOLOGIST T. Su	ullivan $\[mathbb{V}]$ AT TIME OF DRILLING _	NATD
DRILL RIG LF70 ALKALI SAMPLES	AFTERN/A	
DRILLING METHOD DRIVE POINT/ DRILL RATE (min/ft) GG (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	WATER CONFINED (ksf) WATER CONTENT (ksf) WATER CONTENT (%) PLASTICITY INDEX (%) PASSING #200 (%) Cousister (Cousister) (Cousister)	ATERIAL DESCRIPTION sual) Classification System for Soil bg based on Unified (Visual) Classification ency, Color, Soil Type, Degree Saturation)
	Image: Second	ncy, Color, Soil Type, Degree Saturation) -gray to gray, hard, fine-grained, fractured 115.0 tom of borehole at 115.0 feet.

BORING LOG WTAB DATA W DRIVE POINT - WORKING DATA TEMPLATE.GDT - 6/6/24 13:26 - J:LANDSLIDESITETON PASSITHE BIG FILL WY22 MP12.8(GEIDRAFT)BORINGS.GPJ

	100			:	5300 Bishop	Boulevard		The E	Big I	Fill La	ndslide				
Let CEU	DLOGY	ĩ		(Office: (307)	777-4475		Lat/Lor	า: 43.	504025	/-110.9763	32	SO	L BORING: 2	24-01
Proje	ect N	umber:	ND32401			Drilling	Date:	06/11	/2024	1	Dr ———— Co	illing ompleted:	06/11/20)24	
Drille	er:		Craig Walk	ker				Todd	Sulliv	van	M	ethod:	Auger		
Surf	ace E	Elevation	:7571.69'			Lat / lor	na:	43.50)4025	, -110.9	 76332 Dr	ill Ria:	4001 CN	ME-1050	
				mploo		-	Ŭ	Lab							
				ampies		_									
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-F	Moisture Content (%	Graphic Log	Mate	erials Descripti	on	Well	Elevation (ft)
								4			Brown, mc	ist, Lean Clay			7570
	5 -	8 ft	1	\times	2-7-9 (16)	1.5	-								7565
	10 - 	13 ft					-								7560
	 15 – 	18 ft	2	\times	15-27-19 (46)	1.5	-				Siltstone, o very fractu infilling. Fa unit.	dark gray, Hard, red with clay ult gouge in lowe	13.5 er		7555
	20 -		3	\times	13-25-24 (49)	1.5	240187	31-17-14	10.6		Weathered silty, weath	l shale, tan to gra nered	18.8 ay,		
		23 ft	4	\times	20-48-52 (100/11.5")	0.9	-				Shale, blad	sk	23.5		7550
∑ A	fter D	rilling (Al	D)	ĬŲ.	Weather	red Shale				Dep	pth		Comme	nt	
After	Drillin	g (AD)			Shale					3.	.5	Water encount	acountered 3.5 hours after drilling @		
. L	ean C	Clay		\times	SPT - St	tandard P	enetratio	n Test			_	3.5 ON U6/11 Water encounte	red 6 0 hou	d 6 0 hours after drilling @ 0'	
≣ S	iltstor	ne				-				(0	on 06/11			ۍ ت



1	1901				5300 Bishop	o Boulevard		The E	Big	Fill La	ndslide	•			
GEO	DLOGY	7			Office: (307) 777-4475		Lat/Loi	n: 43.	503883	/-110.9759	928	SO	IL BORING: 2	4-02
Proj	ect N	umber:	ND3240	1		Drilling	Date:	06/11	/2024	1	D C	rilling ompleted:	06/11/20)24	
Drille	er:		Craig W	alker		_ Logger:		Todd	Sulliv	/an	M	lethod:	Auger		
Surf	ace E	Elevation	n: <mark>7585.65</mark>	'		_ Lat / lor	ıg:	43.50)3883	s, -110.9 [°]	75928 D	rill Rig:	4001 CM	ME-1050	
		-	T	Samples		1		Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI	Moisture Content (%)	Graphic Log	Mat	terials Descriptic	on	Well	Elevation (ft)
		8 ft	9		1-3-3 (6) 3-8-10 (18)	0.2					Cobbles a Gravel Mixed Ma	and Boulders			7585
°0°0°0 °0°0°0 °0°0°0	Cobl	bles and	Boulders		Mixed	d Material				De	pth		Comme	nt	
	Grav	/el			SPT -	Standard	Penetrat	tion Te		· ·	-		-		



	100				5300 Bishop	Boulevard		The	Big	Fill La	andslide	•			
R GE	HOGY	•			Office: (307)	///-44/5		Lat/Lo	n: 43	.50388	3/-110.9759	28	SO	IL BORING:	24-02
	-		1	Samples	;			Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PI-PI	Moisture Content (%)	Graphic Log	Mat	erials Description	1	Well	Elevation (ft)
	 55	53 ft	15		6-12-18 (30)	0					Shale, Gra Very Hard 55 feet	A, light gray to hite, stiff, slightly hoist, very uniform ay, Very Stiff to , Dry to Wet above	52.0		7535
	60 -	58 ft 63 ft	16		12-20-60 (80) x-100 (100/2")	0.7	240193	26-14-12	9				~ 63.2 ~~		7525
	65 -														7520
	70 -														7515
	СН			\geq	SPT - S	Standard I	Penetrati	on Tes		De	-		Comme	nt	
	Shale					-					-		-		

	200			ţ	5300 Bishop	Boulevard		The E	Big	Fill La	Indslide]
EE0	DLOGY	ĩ		(Office: (307)) 777-4475		Lat/Lo	n: 43.	.503836	/-110.9756	25	SO	L BORING: 2	4-03	
Proje	ect N	umber:	ND32401			Drilling	Date:	06/12	2/202	4	Dr ———— Co	illing mpleted:	06/12/20)24		
Drille	er:		Craig Wal	ker		Logaer:		Todd	Sulliv	van	Me	ethod:	Auger			
Surfa	ace E	Elevatior	n: 7576.89'			Lat / lor	ng:	43.50)3836	6, -110.9 [°]	75625 Dr	ill Rig:	4001 CN	ME-1050		
			S	amples		_		Lab				-				
.				<u>p.ee</u>		(t)		- G	(0)	-						
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (f	Lab Sample ID	tterberg Limits (LL-PL-	Moisture Content (%	Graphic Log	Mate	erials Descripti	on	Well	Elevation (ft)	
X 8 5								×		0 0 0 0 0 0 0 0 0 0 0 0	Cobbles a Pockets of clayey Sar	nd Boulders , slightly moist id.			7575	
7	_									0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0						
10										000000 0000000000000000000000000000000						
10 13	5 -									000000						
16	_									00000					7570	-
19	_									000000						
46	_									000000 0000000000000000000000000000000						
32	10 -									000000						
21	–	12 ft								000000					7565	
20 34	<u> </u>		18	\times	4-8-11 (19)	0.5										
20	_									00000						
17	15 -									000000						
35	_									000000					7560	
30 70										000000						
53	_									000000						
26	20 -									000000 000000 000000						
36	_	00.5								000000					7555	
45		22 ft	19		7-13-87 (100)	0.5				000000						
44 38				\sim						000000						
										000000						ļ
$\underline{\nabla}$	At T	ime of Dr	illing (ATD)	0000 0000 0000 0000	Cobb	les and Bo	oulders			Dej 1	pth 3	Water encounte	Comme red 1 hours	nt s after drilling @	13'	_ _∑
	Afte	r Drilling	(AD)	\geq	SPT					1	2	Water encounte on 06/12	red 24 houi	rs after drilling @) 12'	┨⊒

1	200				5300 Bishop	Boulevard		The I	Big	Fill La	andslide			
	DLOGY	r			Office: (307)	777-4475		Lat/Lo	n: 43.	503836	6/-110.9756	25 5	SOIL BORING: 2	4-03
				Sample	S			Lab						
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI	Moisture Content (%)	Graphic Log	Mat	erials Description	Well	Elevation (ft)
35 26 28 22 15 15 15 18	 30										Cobbles a Pockets of clayey Sai Weathered dry to wet. shear zon siltstone fr matrix	f slightly moist nd. <u>26.0</u> d shale, gray, hard, WeatheredThin es with fine agments in clay		7550
27 33 47 66 86	 35	32 ft	20		13-18-24 (42)	0.5	240194	32-13-19	16.0		Shale, loo	36.0 ks undisturbed)	7545
100/8"	40 -	37 ft 42 ft	21		13-22-32 (54)	1.5	240195	30-19-10	14.4					7540
	45 -	47 ft	22		26-50-50 (100/1') x-100 (100/.75")	0.01	240196	24-13-11	9.9		Sandstone very hard,	46.0 e, Shaley, gray, saturated 47.1)	7530
*0*0*0 *0*0*0	Cobbl	es and Bo	oulders	•	Sandst	one	•			De	pth	Com	iment	
	Weath	nered Sha	le		SPT					1	13	Water encountered 1 ho on 06/11	ours after drilling @	13'
	Shale	1				-				1	12	Water encountered 24 h on 06/12	nours after drilling @) 12'

A	202				5300 Bisho	p Boulevard		The	Big	Fill La	Indslide				
N GEO	LOGY	•			Office: (307) 777-4475		Lat/Lo	n: 43.	503253	s/-110.9760 [°]	15	SOII	L BORING: 2	24-04
Proje	ect N	umber:	ND3240	1		_ Drilling	Date:	06/12	2/2024	4	Dr	illing	06/12/20	24	
Drille			Craia W/	alkar		Loggor		Todd	Quillis	(a n	M	othod:	Augor		
Surf		levation	01alg 11			_ Logger		13 5	13253	2 _110 0	76015 nd	ill Rig:		IE-1050	
Sulla			1. 1015.02			_ Lat / 101	iy.	40.0	00200	, -110.3		ili rug.	4001 Civ	12-1030	
	-			Sample	S	1		Lab		-					
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-P	Moisture Content (%)	Graphic Log	Mate	erials Descriptio	on	Well	Elevation (ft)
	 5 -							4			FILL, sligh Well-grad Placed rec	tly moist, e d Fill with Clay , ently			7615
	 10 										Soft to me wet, Lean fine graine	dium stiff, moist t Clay (CL), with d Gravel	12.0 0		7605
	_ 	18 ft	24								Ū				7600
					_		240197	29-16-13	16						
	20 - 										Silty Clay	(CL-ML), Clay,	22.0		7595
	_										Gravel, tar moist, inta	n, very stiff, sl. ct and undisturbe	d		
				,						De	pth		Commer	nt	<u> </u>
∑ At	t Time	e of Drillin	ng (ATD)		CL-ML				 	1	5	Water encounte	ed .2 hours	after drilling @	0 15'
W	/ell-gra	aded Fill			Grab - (Grab Sam	ole		<u> </u>	1	J	on 06/12			
C	L					-					-		-		

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	1001				5300 Bishop	Boulevard		The E	Big I	Fill La	andslide			
E	DLOGY				Office: (307)	777-4475		Lat/Lor	า: 43.	503253	8/-110.9760 ⁻	15 SOI	L BORING: 2	24-04
	-			Samples	;			Lab						
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI	Moisture Content (%)	Graphic Log	Mat	erials Description	Well	Elevation (ft)
		50 ft	27		10-18-34 (52)	1	240199	36-16-20	14.6		Shale, har zones, Fin	d, gray with wet e siltstone		7565
		54 ft	28		x-100 (100/2''')	0.1	-				fragments Siltstone, s Shaley, pu	53.0 shaley, dry, ilverized, dry 54.2		7560
	 60													7555
	 65 - 													7550
	70 -													7545
										De	pth	Comme	nt	
	Shale			\mid	SPT - S	tandard P	enetratio	on Test		1	15	Water encountered .2 hour	s after drilling @	0 15'
= (Siltstor	ne, shaley	/, dry	<u>`</u>		-					-	-		

	100			:	5300 Bishoj	o Boulevard		The E	Big	Fill La	andslide				
GE	DLOGY	r			Office: (307) 777-4475		Lat/Loi	n: 43.	.503167	/-110.9750	08	SOIL	BORING:	24-05
Proje	ect N	umber:	ND3240	1		_ Drilling	Date:	06/13	3/202	4	Di 	rilling	06/13/202	24	
Drille	ər:		Craig Wa	alker		Logger:		Todd	Sulliv	van	M	ethod:	Auger		
Surf	ace E	Elevatior	n: 7681.10'			_ Lat / lor	ng:	43.50)3167	7, -110.9	75008 D	rill Rig:	4001 CM	E-1050	
				Samples				Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI)	Moisture Content (%)	Graphic Log	Mat	erials Descriptio	on	Well	Elevation (ft)
											Fill, Pad F with Cobb	ill: Gravel, sandy es, dry, angular	5.0		7680
	5 -	7 ft	29	\times	8-6-5 (11)	1.2	-				Fill, Emba Sand, grav Cobbles, r moist to m limestone	nkment Fill: /elly with Clay an nedium dense, sl oist, tan, clasts	d.		7675
															7670
	15 - 	19 ft	30		6-7-10 (17)	0.8	-								7665
											Lean Clay brown, me sl. moist, r	r (CL), Clay, olive dium to very stiff, ninor carbonaceo	23.0 Jus		7660
					/ -	1	1			De	pth		Commen	t	
	Fill				SPT					30	0.5	Water encount 30.5' on 06/13	ered .2 hour	s after drilling	g @
	CL					-				30	0.5	Water encounter	ed 3 hours a	fter drilling @) 30.5'
	TO BA				5300 Bishop	Boulevard		The E	Big I	Fill La	andslide				
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C GE	DLOGY				Office: (307)	777-4475		Lat/Lor	า: 43.	503167	/-110.9750	08	SOIL BORING: 2	24-05	
	-			Samples	5			Lab	1	_					
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-P	Moisture Content (%)	Graphic Log	Mat	erials Description	Well	Elevation (ft)	
	_	29 ft									Lean Clay brown, me sl. moist, r layers and	r (CL), Clay, olive edium to very stiff, ninor carbonaceous fine gravel		7655	
	30 - 		31	$\left \right\rangle$	10-10-10 (20)	1.1	-	30-14-16	19.8					7650	
	_ _ 35 -														
		39 ft	32		6-11-13	1.4	_							7645	
	40 -			\times	(24)			37-14-23	20.1					7640	
	45 -	49 ft	33		6-10-11	1								7635	
			33		(21)			32-12-20	13.2		nth				
$\underline{\nabla}$	At Ti	me of Dril	lling (ATD)	CL					30	0.5	Water encountered	.2 hours after drilling	@ <u></u>	
	After	Drilling (AD)		SPT		_			30	0.5	Water encountered 3 on 06/13	hours after drilling @	30.5'	

	100				5300 Bishop	Boulevard		The E	Big I	Fill La	andslide				1
	DLOGY	5			Office: (307)) 777-4475		Lat/Lor	า: 43.	503167	/-110.9750	08	SOIL BORING	: 24-05	
				Sample	s			Lab	1						ĺ
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI)	Moisture Content (%)	Graphic Log	Mate	erials Description	Well	Elevation (ft)	
		50 ft	33		6-10-11 (21)	1		32-12-20	13.2		Lean Clay	(CL), Clay, olive		7000]
		- - - - - - - - - -	34		9-18-13 (31)	0.6		27-13-14	21.1		sl. moist, r layers and	ninor carbonaceous I fine gravel		7630 7625 7620	
		69 ft	35		17-62-38 (100/9")	0.7		28-17-11	15		Shale, mo gray, very moist, mix very fine s within the zones	ttled gray and dark hard, dry to sl. ed with abundant iltstone fragments claystone moist	39.8	7615	
	CL				SPT					De 3	ο.5	Water encountered	omment .2 hours after drilli	ng @	
	Sha	le				-				3	0.5	Water encountered 3 on 06/13	hours after drilling	@ 30.5'	

1	100				5300 Bisho	o Boulevard		The	Big	Fill La	ndslide				
Log CEC	LOGY	•			Office: (307) 777-4475		Lat/Lo	on: 43	50296/-	-110.97709		SO	IL BORING: 2	4-06
Proje	ect N	umber:	ND3240	1		_ Drilling	Date:	06/1	7/202	4	Di ———— Ce	rilling ompleted:	06/17/2	024	
Drille	er:		Craig W	alker		_ Logger:		Lief	Swan	oom	M	ethod:	Auger		
Surfa	ace E	Elevatior	n: <u>~7599.5</u>	1'		_ Lat / lor	ng:	43.5	0296,	-110.97	709 Di	rill Rig:	4001 CI	ME-1050	
				Sample	s			Lab		_					
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI	Moisture Content (%)	Graphic Log	Mat	erials Descriptic	n	Well	Elevation (ft)
3 4 5 10 13 16 19 18 18 18 18 13 14 18		<u>8 ft</u>	35		5-4-7 (11)	1					Fill, Loggi Silt. sand, and boulde Soil, Light organic ric with roots Clayey Si light brown and silt wit sand and s rounded g	ng Road Pad: limetone cobbles ers. to dark brown, th clay, and silt (topsoil) It , Medium dense n moist clayey silt th minor clay, small angular to ravel Sand , Stiff to d to orange, moist	2.0 3.0 , ,		7595
35			36	\searrow	2-4-6 (10)	1.4	240205	23-14-8	11.5		limestone	origin.			
43 43 42	 15										Siltatono	Van bard van	16.0		7585
61	_										dense, wh	ite to gray, light			
56	_	18 ft	37		41-46-54	1.5					clayey, lim	y silt and small			
60	_			\mid	(100)						slight wear	thered shale - uni	t		7580
125	20 -										appeared	onewed up			
165	_	23 ft	38		22-29-50 (79/1')	1.5									7575
 	:			13		th Sand]	<u> </u>	<u> </u>			0		
S S	Boil				Siltston	e				De	pth -		Comme -	nt	
	Clayey	v Silt		>	SPT - S	Standard P	enetratic	on Test			-		-		

	A COM				5300 Bishop	Boulevard		The	Big	Fill La	andslide)			
	OLOGY	7			Office: (307)	777-4475		Lat/Lo	n: 43.	50296/	-110.97709)	SO	IL BORING:	24-06
				Sample	s			Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-P	Moisture Content (%)	Graphic Log	Mat	erials Descriptio	n	Well	Elevation (ft)
		33 ft	39		×-100 (100)	0.75					Siltstone, dense, wh brown her clayey, lim siltstone fr slight wea appeared Shale, Ver brown, fra to unweat reactive w stained.	Very hard, very hite to gray, light matite stained hy silt and small ragments and thered shale - unit "chewed up" ry hard, gray ictured weathered hered shale - very rith HCL, hematite Very dense, yellow , siltstone with thin s - very fine a, dry. Limy' clay hematite stained.	29.0 38.0		7555
	Siltst	one		$\left \right>$	SPT - S	Standard F	Penetrati	on Tes		De	pth		Comme	ent	
	Shale	•				-					-		-		

	200				5300 Bishop	Boulevard		The l	Big	Fill La	andslide			
	DLOGY				Office: (307)) ///-44/5		Lat/Lo	n: 43.	50296/	-110.97709) 5	SOIL BORING:	24-06
	-		1	Samples	6	1		Lab	1					
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI	Moisture Content (%)	Graphic Log	Mat	erials Description	Well	Elevation (ft)
		53 ft	41		82-100 (100)	0.75					Siltstone, to orange, clay layers sandstone layers are	Very dense, yellow siltstone with thin s - very fine e, dry. Limy' clay hematite stained.		
											- -	54.5	;	7545
	55 - 60 - 													7540
	_													7535
	65 - 													7530
	70 -													
												1		7525
				<u> </u>						De	pth	Com	ment	
	Siltst	tone			🤇 SPT -	Standard	Penetra	tion Te			-			

A	202				5300 Bisho	p Boulevard		The E	Big	Fill La	ndslide				
GEO	LOGY	r			Office: (307) 777-4475		Lat/Lor	n: 43	.50264/-	110.97524		SOI	L BORING: 2	24-07
Proje	ect N	umber:	ND3240)1		_ Drilling	Date:	06/18	3/202	4	Di ————————————————————————————————————	rilling ompleted:	06/18/20)24	
Drille	er:		Craig W	/alker		Logger:	:	Lief S	Swan	bom	М	ethod:	Auger		
Surfa	ace E	Elevatior	n:~7697.6	68'		_ Lat / lor	ng:	43.50)264,	-110.97	524 Di	rill Rig:	4001 CN	/IE-1050	
				Samples	;			Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI)	Moisture Content (%)	Graphic Log	Mat	erials Descriptic	on	Well	Elevation (ft)
	5 	8 ft	42		6-8-9 (17) 5-6-10 (16)	0.75					Fill, Fill - M dense/very gravel, col to 1.5 feet rounded. M white to br carbonate origin) Silty Clay brown, dry roots, coal	Medium y stiff, Silt, sand, obles, boulders up ; angular to Minor clay., ashy rown. Calcium rich (limestone (CL), Stiff, dark , silty clay with fragments.	12.0		7695 7690 7685 7685
	 				(16)		240206	33-16-16	15.8		Weathered	d shale, Weathere	24.5 ed		7675
	Fill			¥7	🖉 Weat	thered sha	le			De	pth		Comme	nt	
	C				67 / 007	Ctondard	Donates	tion To			-		-		
	UL			/	SPI-	Standard	renetral	uon le			-		-		

	1007				5300 Bishop	Boulevard		The	Big	Fill La	andslide)			
	OLOGY	•			Office: (307)	777-4475		Lat/Lo	n: 43	.50264/	-110.97524	Ļ	SO	IL BORING: 2	4-07
				Sample	s			Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI)	Moisture Content (%)	Graphic Log	Mat	erials Descriptio	n	Well	Elevation (ft)
		28 ft 38 ft 48 ft	44		4-11-10 (21) 16-24-23 (47) 21-31-26	1.25		After			Weathered shale Shale, Ver dense/har blue-gray silty to arg with carbo lenses, sil speckled s calcium ca poorly cer sandstone Dry to slig Sulfuric oo test hole v approxima	d shale, Weathered ry stiff to d, blue green to to white to black, jillaceous shale onaceous and coal ty very fine grain sandstone, with arbonate cement - mented. Shale and e are interbedded. ihtly moist. dor emitted from while drilling at ately 50 feet.	d		7670 7665 7665 7655
							-								
⊥ ¥¶71	<u> </u>		I				<u> </u>			De	pth		Comme	ent	<u> </u>
	Weath	nered sha	le		SPT - S	standard I	Penetrati	on Tes			-	1	-		
	Shale					-					-	Ì	-		

	1001				5300 Bishop	Boulevard		The	Big	Fill La	andslide	•			
	OLOGY				Office: (307)) 777-4475		Lat/Lo	on: 43	.50264/	-110.97524	ŀ	SO	L BORING: 2	24-07
				Samples	S			Lab							
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI)	Moisture Content (%)	Graphic Log	Mat	erials Descriptio	n	Well	Elevation (ft)
		54 ft 58 ft 68 ft	48 47 47 47 49		20-23-26 (49) 18-25-31 (56)	1.3	240207	38-14-24	9.5		Shale, Ver dense/har blue-gray i silty to arg with carbo lenses, silt speckled s calcium ca poorly cen sandstone Dry to slig Sulfuric oc test hole w approxima	ry stiff to d, blue green to to white to black, jillaceous shale onaceous and coal ty very fine grain sandstone, with arbonate cement - nented. Shale and e are interbedded. htly moist. dor emitted from vhile drilling at ately 50 feet.	69.5		7645 7640 7635 7635
	-														7625
	Shale				SPT - S	Standard F	Penetrati	on Tes		De	pth		Comme	nt	
╞═╴	Grob	Grab S-	mplo	/	<u>\</u>						-		-		
-	Grab -	Grap Sa	nipie			-			1		-	1	-		

Office: (307) 777-4475 Lat/Lon: 43.50367/-110.97697 SOIL BORING: : Project Number: ND32401 Drilling Date: 06/18/2024 Drilling Completed: 06/18/2024 Driller: Craig Walker Logger: Lief Swanborn Method: Auger Surface Elevation: -7606.41' Lat / long: 43.50367,-110.97697 Drill Rig: 4001 CME-1050 Surface Elevation: Office: (i) II II III III along gauge gauge III Gauge IIII IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	
Project Number: ND32401 Drilling Date: 06/18/2024 Drilling Completed: 06/18/2024 Driller: Craig Walker Logger: Lief Swanborn Method: Auger Surface Elevation: ~7606.41' Lat / long: 43.50367, -110.97697 Drill Rig: 4001 CME-1050 Image: Surface Elevation: Samples Lab Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Surface Elevation: Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Image: Samples Im	4-08
Driller: Craig Walker Logger: Lief Swanbom Method: Auger Surface Elevation: ~7606.41' Lat / long: 43.50367, -110.97697 Drill Rig: 4001 CME-1050 Image: Complexity of the strength of the streng strength of the strength of the strength of the stre	
Surface Elevation: -7606.41' Lat / long: 43.50367, -110.97697 Drill Rig: 4001 CME-1050 august of the second secon	
Samples Lab Divaring Cone Samples Arresterior Ballow Counts Ballow Counts Calibrit of Sample Arresterior Ballow Counts Ballow Counts Sample Graphic Arresterior Ballow Counts Ballow Counts Sample Graphic Ballow Counts Sample Graphic Ballow Counts Sample Graphic Ballow Counts Sample Graphic Ballow Counts Sample Graphic View Counts Ballow Counts Sample Counts	
Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession Image: Second procession	
4 ft 3.0 5 - 50 10 - 3.2-5 10 - 0.8	Elevation (ft)
	7605
14 ft 51 6-7-8 (15) 0.9 15 - - 16 - 16 - 17 - 18 - 19 - 11 - 11 - 15 - 16 - 17 - 18 - 19 - 10 - 10 - </td <td>7595</td>	7595
20 - 21.0 20 - 24 ft 22 4.5-8 1.4 240208 117-25-92 34.2	7585
Image: At Time of Drilling (ATD) Image: Attraction of Depth Comment	=
Y After Drilling (AD) Shale Y Water encountered 0.5 hours after drilling	@
Topsoil X SPT - Standard Penetration Test 11.5' on 06/18 at 4:05PM Water encountered 24.5 hours after drillin	
Silty Clay - 12.2 Water choose 24.5 Hours alter drifting 12.2' on 06/20 at 4:00PM - 12.2' on 06/20 at 4:00PM	<u>۳</u>

	1991				5300 Bishop	Boulevard		The E	Big I	Fill La	andslide			
	DLOGY	r			Office: (307)	777-4475		Lat/Lor	n: 43.	50367/-	-110.97697		SOIL BORING: 2	4-08
			1	Sample	es			Lab						
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-PI	Moisture Content (%)	Graphic Log	Mat	erials Description	Kell	Elevation (ft)
		25 ft	52		4-5-8 (13)	1.4	240208	117-25-92	34.2		Shale, Stif dark grav	f to very stiff, to grav - maroon.		
	30 -	<u>34 ft</u>	53		18-17-20 (37)	1.5					dark gray	o gray - maroon, llaceous. Plastic.		7580
	35 -													7570
	45 -	44 ft	54		8-12-19 (31)	1.5					Siltstone, clayey silts green sha to slightly i	46 Very hard, white, sone and gray to e, shilty shale. Dry moist.	5.0	7560
	<u> </u>					e t <i>i</i>	_			De	pth	Co	mment	
	Shal	le		/	X SPT -	Standard	Penetral	tion Te		11	1.5	Water encountered 0 11.5' on 06/18 at 4:05	0.5 hours after drilling 5PM	@
	Silts	tone				-				12	2.2	Water encountered 24 12.2' on 06/20 at 4:00	4.5 hours after drilling PM	@

1	100				5300 Bishop	Boulevard		The E	Big	Fill La	andslide				
	DLOGY	•			Office: (307)	///-44/5		Lat/Lor	า: 43.	50367/-	-110.97697		SOI	L BORING: 2	24-08
	-			Samples				Lab	1						
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-P	Moisture Content (%)	Graphic Log	Mat	erials Descriptior	1	Well	Elevation (ft)
	_	54 ft									Siltstone, clayey silts green shal to slightly i	Very hard, white, sone and gray to le, shilty shale. Dry moist.			7555
			55	\sum	38-37-61 (98)	1.4	240209	NP	11.9						
	55 -						240210	19-10-9	8.3						7550
		59 ft	56		51-95- 5/0.03' (100)								60.5		
	 65 -														7545
	 70 -														7540
	-														7535
										De	pth	Water encounter	Comme	nt	
								1		1	1.5	11.5' on 06/18 at	ea 0.5 ho 4:05PM	urs after drilling	@
	Siltst	tone			SPT -	Standard	Penetrat	tion Te		12	2.2	Water encountere 12.2' on 06/20 at	ed 24.5 hc 4:00PM	ours after drilling	g @

	1991			5300 Bishop	Boulevard	The I	Big Fill Lands	slide			
	OLOGY			Office: (307)	777-4475	Lat/Lo	n: 43.503398/-110).975619		SOIL BORI	NG: 24-09
Proj	ject Nu	ımber:	ND32401		Drilling Date:	06/2	5/2024	Drilli — Corr	ng ipleted:	06/25/2024	
Drill	er:		Craig Walker		Logger:	Mich	ael Killion	Meth	nod:	Auger	
Sur	face El	levation	1:7665.38'		Lat / long:	43.50	03398, -110.97561	9 Drill	Rig:	4001 CME-1050	
Dvnamic Cone	Penetrometer	Depth (ft)	Graphic Log		Mate	rials Des	scription			Well	Elevation (ft)
		5		Tan, slightly limestone si Tan, slightly subrounded	r moist, Loose to I ubrounded to angu moist to moist, M I gray limestone gr	Medium I ular rock	Dense Fill, 1/2" to 6" Stiff Fill, 1/2" to 2"	5	.0		7660
		15									7050
		20									7645
							Depth			Comment	
	Loose	to medi	um dense Fill		-		82		Wa	ter encountered @ 82'	
×****	-					1				-	

	5300 Bishop Boulevard	The E	Big Fill Landslide			
Carology	Office: (307) 777-4475	Lat/Lo	n: 43.503398/-110.97561	19	SOIL BORIN	G: 24-09
Dynamic Cone Penetrometer Depth (ft) Graphic Log	Mate	rials Des	cription		Well	Elevation (ft)
	Tan, slightly moist to moist, M subrounded gray limestone g	ledium to ravel	Stiff Fill, 1/2" to 2"			
						7635
						7625
						7620
					Comment	
Medium to stiff FIII	-		- 02		-	

	ALO			5300 Bishop Boulevard	The I	Big Fill Landslide			
(GEOLOGY			Office: (307) 777-4475	Lat/Lo	n: 43.503398/-110.9756	19	SOIL BORIN	G: 24-09
	Dynamic Cone Penetrometer	Depth (ft)	Graphic Log	Mater	rials Des	scription		Well	Elevation (ft)
				Tan, slightly moist to moist, M subrounded gray limestone gr	edium to ravel	Stiff Fill, 1/2" to 2"			7610
									7605
		65 -		Gray, saturated, Medium Sar	ndy Silt, ∣	Native Material	65.0		7600
		-	× × · × · × · × · ×	Shale, extremely weak , silty o	clayey Sh	ale	67.0		
		70 - 						7595	
						Donth	1	Commont	
	Medium	to stiff ⊑Ⅲ				82		Water encountered @ 82'	
	Modium	Sondy C	1+			02			
- 3333	wealum	Sandy Si	IL	-		-		-	

AND	5300 Bishop Boulevard	The E	Big Fill Landslide			
CEOLOGY	Onice. (307) 777-4475	Lat/Lor	n: 43.503398/-110.97561	9	SOIL BORIN	G: 24-09
Dynamic Cone Penetrometer Depth (ft) Graphic Log	Materia	als Des	cription		Well	Elevation (ft)
	Shale, extremely weak , silty cla	ayey Sha	ale			
					7585	
90 -						7580
95 -					-	7570
			Depth		Comment	
			82	I	Water encountered @ 82'	
At Time of Drilling (ATD)	Shale		-		-	



A REAL		5300 Bishop Boulevard	The E	Big Fill Landslide			
GEOLOGY		Omce: (307) 777-4475	Lat/Lor	n: 43.503398/-110.97561	19	SOIL BORIN	G: 24-09
Dynamic Cone Penetrometer Depth (ft)	Graphic Log	Mate	rials Des	cription		Well	Elevation (ft)
		Shale, extremely weak , 1000 Shale) psi down	pressure,silty clayey	27.7		
13	 0 						7535
13	5						7530
14	 0 - 						7525
14	 5 -						7520
	-						
				Depth		Comment	
Shale		-		δ2			

	POR I			5300 Bishop Boulevard	The	Big Fill Landsli	de			
	OLOGY			Office: (307) 777-4475	Lat/Lo	on: 43.503394/-110.97	75616		SOIL BORI	NG: 24-10
Proj	ect Nu	mber:	ND32401	Drilling Date:	06/2	26/2024	Drilling – Compl) leted:	06/26/2024	
Drille	er:		Craig Walker	Logger:	Nath	nan Willis	Metho	d:	Auger	
Surf	ace El	evation	:~7665.38'	Lat / long:	43.5	503394, -110.975616	Drill R	ig:	4001 CME-1050	
Dvnamic Cone	Penetrometer	Depth (ft)	Graphic Log	Mate	erials De	scription			Well	Elevation (ft)
		5 10 15		FILL, loose, light brown to bi subangular, Silty Sand Fill,	rown, dry f gravel, lim	to slightly moist, nestone gravel up to 4"	18.0			7650
	20 - 20 - - - - - - - - - - - - - - - - - - -			FILL, loose, brown, slightly r limestone gravel up to 1/2" FILL, loose, brown, moist, S less gravel than above mate	noist, Silt ilty Sand rial	y Sand Fill, gravel,	23.0			7645
			$\sim\sim\sim\sim\sim$			Depth			Comment	
××××]			49.6	Wa 49	ater encou .6'	ntered 22 hours after o	drilling @
	Silty s	and Fill		-		-			-	





A	001			ŧ	5300 Bishop	Boulevard		The E	Big l	Fill La	ndslide				
GEO	LOGY	1		(Office: (307)) 777-4475		Lat/Lor	n: 43.	504026	/-110.9758	44	SO	L BORING: 2	24-11
Proje	ect N	umber:	ND32401			Drilling	Date:	06/26	6/2024	4	Dr ———— Co	illing ompleted:	06/26/20)24	
Drille	er:		Craig Wa	lker		_ Logger:		Micha	ael Ki	llion	M	ethod:	Auger		
Surfa	ace E	Elevation	n: 7577.87'			_ Lat / lor	ıg:	43.50)4026	, - 110.9	75844 Dr	ill Rig:	4001 CM	/IE-1050	
			S	Samples				Lab		-					
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft	Lab Sample ID	Atterberg Limits (LL-PL-I	Moisture Content (%	Graphic Log	Mate	erials Descriptio	on	Well	Elevation (ft)
2 9											Slide debri moist to m	is, tan, slightly oist, Slide			
12	_										Debris, 1/4 gray limes	4" to 4" angular tone gravel,Loos anse_sandy silt to	e to		
10	_										silty sand	shise, sandy shi u	,		7575
19	_														
13	5 -									8°0°0°					
10		7 ft													
10	_		61	\times	5-2-3 (5)	0.6									7570
6	_														
9	10 -									0.000 0.000					
12	_														
8	_														7565
8	_														
8	15														
10	15 -														
9	_	17 ft			7 40 7	0.7									
9	_		02	\times	(19)	0.7									7560
9	_									6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6					
13	20 -														
10	-														
13															7555
16															
										0000 0000		1			
										De	pth -	No free wa	Comme ter encoun	nt tered on 06/26	
SPT - Standard Penetration							tion Te		52	2.8	Water encounte 52.8' on 02/07	ered 20 ho	urs after drilling	@	

1	100				5300 Bishop	Boulevard		The E	Big	Fill La	andslide			
	DLOGY	r			Office: (307)	///-44/5		Lat/Lor	n: 43	.504026	6/-110.9758	44 SC	DIL BORING: 2	4-11
				Samples	3			Lab		_				
Dynamic Cone Penetrometer	Depth (ft)	Depth of Sample (ft)	Sample Number	Sample Graphic	Blow Counts (N/Refusal)	Recovery Length (ft)	Lab Sample ID	Atterberg Limits (LL-PL-P	Moisture Content (%)	Graphic Log	Mate	erials Description	Well	Elevation (ft)
13 16 13	_	27 ft	63		11-10-6 (16)	1	-				Slide debr moist to m Debris , 1/ gray limes medium de silty sand	is, tan, slightly oist, Slide 4" to 4" angular tone gravel,Loose to ense, sandy silt to		7550
13 15							-				Sity Sana	30.0		7550
27 27 24	_	32 ft	64		6-6-11	0.3					Brown, mc 1/16 to 1/4 gravel,stiff silty clay	bist, Mixed Material , " limestone to very stiff,		
15 27				\mid	(17)		240238							7545
43 56 65		37 ft	65		8-8-10	0.15								
59 56	_			\mid	(18)		240239	33-15-18	9					7540
41 32 35	40 -	42 ft										42.0	_	
3	_		66	\times	9-14-22 (36)	0.8	240240	32-12-20	8.1		Gray, sligh Material, t shale, den fine graine	uty moist, Mixed black very weak se, clayey, angular ed sand		7535
57 52	45 –	۸7 f +									Weathered brown/blad weak, mod	45.0 d shale, ck, extremely derately weathered		
60 79 84		<u>+1 IL</u>	67	\times	22-41-59 (100/11.4"')	1.5	-							7530
0.0 200	Slide	Debris			SPT -	Standard	Penetrat	ion Te:		De	pth -	No free water encou	ent ntered on 06/26	
	Weat	/eathered Shale -								52	2.8	Water encountered 20 ho 52.8' on 02/07	ours after drilling	@



Ţ

100 A			5300 Bishop Boulevard	The I	Big Fill Landsli	de		
GEOLOGY	5		Office: (307) 777-4475	Lat/Lo	n: 43.503535/-110.97	75978	SOIL BORIN	IG: 24-12
Project N	Number:	ND32401	Drilling Date:	06/27	7/2024	Drilling - Complete	ed: 06/27/2024	
Driller:		Craig Walker	Logger:	Nath	an Willis	Method:	Auger	
Surface	Elevation	:~7644.92'	Lat / long:	43.50	03535, -110.975978	_ Drill Rig:	4001 CME-1050	
Dynamic Cone Penetrometer	Depth (ft)	Graphic Log	Mate	rials Des	scription		Well	Elevation (ft)
	5		FILL, loose, brown, dry to slig Gravel	Jhtly moist	t, Silty Sand Fill with			7640
			FILL, brown, slightly moist to Gravel and Clay, limestone g	moist, Sa gravel up t	ndy Silt Fill with to 1/4"	13.0		
	15					7630		
	20		FILL, loose, brown, slightly m and Sand Fill with Silt and C	ioist to mo	oist, subangular, Gravel stone gravel up to 1/2"			7625
					Depth		Comment	7620
					53		Water encountered @ 53'	Ī
Silty	sand Fill		-		-		-	

	100A			5300 Bishop Boulevard	The	Big Fill Landslide			
ł	EOLOGY		1	Office: (307) 777-4475	Lat/Lo	n: 43.503535/-110.9759	78	SOIL BORIN	G: 24-12
	Dynamic Cone Penetrometer	Depth (ft)	Graphic Log	Mate	erials Des	cription		Well	Elevation (ft)
				FILL, loose, brown, slightly r and Sand Fill with Silt and Weathered shale, moderatel cuttings (from up-hole?)	noist to mo Clay, limes	bist, subangular, Gravel stone gravel up to 1/2"	40.0		7615 7610 7605
		 45 - 							7600
			317120	1		Depth		Comment	7595
	S Grav	el and san	d Fill	Weathered Shale		53	ļ	Water encountered @ 53'	<u> </u>
	×					-		-	

	100			5300 Bishop Boulevard	The I	Big Fill Landslide			
G	EOLOGY			Office: (307) 777-4475	Lat/Lo	n: 43.503535/-110.9759	78	SOIL BORIN	G: 24-12
	Dynamic Cone Penetrometer	Depth (ft)	Graphic Log	Mate	rials Des	cription		Well	Elevation (ft)
				Weathered shale, moderately cuttings (from up-hole?)	weathere	ed, minor gravel found in			7590
			SHI A			Depth		Comment	7570
	At Tir	ne of Drilli	ng (ATD)	Weathered Shale		53		Water encountered @ 53'	
	At Time of Drilling (ATD)			1	-		-		

	A*001			5300 Bishop Boulevard	The I	Big Fill Landslide			
E	EOLOGY			Omce: (307) ///-4475	Lat/Lo	n: 43.503535/-110.9759	78	SOIL BORIN	G: 24-12
Dunamia Pana	Dynamic cone Penetrometer	Depth (ft)	Graphic Log	Mate	rials Des	cription		Well	Elevation (ft)
				Weathered shale, moderately cuttings (from up-hole?)	weathere	ed, minor gravel found in	78.0		
		 80 - 							7565
		 							7560
		-							
		 90 – 							7555
		_							7750
		95 – 							/550
		_				Depth		Comment	7545
¥9774.	\M/aath	arad Sha	_			53		Water encountered @ 53'	
	vveath	ereu onal	c	-		-		-	















APPENDIX B – BOREHOLE AND SAMPLE RESULTS

										Per	cent Pass	through			_			
					Uncorrected				11144 (S.L.)	-	Western Review		and the second second					
Test Hole	Lab Number	Sample Number	Sample Depth	Lithology	N-Value	USCS	AASHTO	1"	3/4"	3/8"	#4	#10	#40	#200	LL	PL	P	M%
SL22-1	000000		0.01.0.51	10 months and a fill months and	10	014		05.0	70.0	00.5	50.0	00.5	07.0	40.0	5.0.7	ND	NID	
	220609	1	8.01 - 9.51	sitty gravei with sand	48	GM	A-1-D	60.0	19.3	00.0	53.9	39.0	27.0	18.9	INV 07.0	NP 17.0	NP 10.0	3.8
	220610	2	18.0" - 19.5"	clayey gravel with sand	13	GC	A-2-6(0)	100.0	93.4	75.0	09.1	40.1	32.6	25.3	27.8	17.0	10.8	(.2
	220611	3	28.0 - 29.5	sitty gravei with sand	15	GM	A-1-D	82.3	82.3	71.0	34.9	40.3	20.8	19.4	INV Of E	NP	NP	4.6
	220612	4	38.0 - 39.5	well-graded gravel with clay and sand	30	GVV-GC	A-2-4(0)	69.6	00.1	03.4	40.2	33.0	16.1	8.9	21.5	14.1	1.4	1.8
	220013	5	40.0 - 49.5	sitty clayey gravel with sand	22	GC-GM	A-1-D	100.0	07.0	09.2	54.7	41.2	27.0	19.4	10.5	15.5	5.2	4.0
	220614	5	58.0° - 59.5°	silty gravel with sand	47	GM	A-1-D	100.0	05.0	74.1	38.4	43.8	30.7	23.4	20.0	NP 040	NP 5.0	0.3
	220010	/	00.U - 09.0 70.0 - 70.6	silty gravel with sand	21	GIVI	A-4(0)	95.0	95.0	02.1	70.2	51.7	03.0	40.3	30.0	24.9	0.9	13.5
SI 23-1	220010	0	76.0 - 79.5	sity graver with sand	29	GIVI	A-1-0	0/.3	00.5	00.9	00.0	51.0	43.0	19.5	INV	INP	NP	0.1
	230466	1	10.5' - 11.5'	silty gravel with sand	21	GM	A-1-a	89.4	80.5	58.8	47.3	33.3	20.9	13.7	NV	NP	NP	1.6
	230467	2	20.5' - 21.5'	poorly graded gravel with silt and sand	12	GP-GM	A-1-a	66.9	50.6	31.2	26.8	22.0	15.1	11.2	NV	NP	NP	5.5
SL 24-1																		
-0	240187	3	18.0' - 19.5'	gravelly lean clay	49	CL	A-6(6)	100.0	100.0	92.0	76.6	70.4	66.7	63.8	31.2	17.2	14.0	10.6
	240188	6	33.5' - 34.5'	fat clay	13	CH	A-7-6(30)	100.0	100.0	100.0	100.0	98.5	96.2	85.6	50.5	15.8	34.7	28.8
	240189	8	43.0' - 43.8'	lean clay with sand	100/0.25'	CL	A-6(7)	100.0	100.0	98.8	96.1	95.6	94.4	84.8	27.1	16.2	10.9	13.9
SL 24-2		1.0																
	240190	12	38.5' - 39.5'	Remold with #13	15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
	240191	13	43.5' - 44.5'	Remold with #12	27	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
	240192	14	48.5' - 49.5'	fat clay	18	CH	A-7-6(53)	100.0	100.0	100.0	100.0	100.0	100.0	98.8	67.2	19.5	47.7	33.3
11211121212	240193	16	58.5' - 59.5'	lean clay with sand	80	CL	A-6(7)	100.0	100.0	100.0	97.2	93.8	87.1	79.8	26.0	14.1	11.9	8.7
SL 24-3			00.51 00.51		10		1 0/01		100.0							10.0	10.0	- 10.0
	240194	20	32.5' - 33.5'	sandy lean clay	42	CL	A-6(8)	100.0	100.0	93.3	85.7	11.1	69.9	57.8	31.7	12.8	18.9	16.0
	240195	21	37.0 - 38.0	lean clay	04	CL	A-6(10)	100.0	100.0	99.4	100.0	99.2	99.1	98.1	29.5	19.1	10.4	14.4
SI 244	240196	22	42.0 - 43.1	sandy lean clay	100/1	UL.	A-6(4)	100.0	100.0	100.0	100.0	99.1	91.3	66.9	23.0	12.8	11.0	9.9
<u>3L 24-4</u>	240107	24	18.0' - 19.0'	sandy lean clay with gravel	NIA	CI	A-6(4)	100.0	100.0	Q1 /	82.5	73.3	64.4	52.8	28.7	15.6	13.1	16.0
	240197	24	20.5' 30.5'	sality lean day with graver	25		A-0(4)	100.0	100.0	100.0	02.0	01.6	86.7	92.0 80.0	20.7	17.0	7.0	15.0
	240100	25	29.5 - 50.5 49.5' - 50.5'	lean day	52	CL	A-4(3) A-6(17)	100.0	100.0	00.0	08.7	95.6	02.1	89.7	35.6	16.0	19.6	14.6
SI 24-5	240100	27	43.5 - 50.5	lean day	52	UL	A-0(17)	100.0	100.0	33.0	30.2	30.0	32.1	03.7	00.0	10.0	10.0	14.0
02240	240200	31	29.5' - 30.5'	lean clay with sand	20	CI	A-6(10)	100.0	100.0	949	94 1	91.8	87.6	78.4	29.9	13.9	16.0	19.8
	240201	32	39.5' - 40.5'	lean clay with sand	24	CL	A-6(15)	100.0	100.0	97.2	93.2	87.6	81.9	74.3	36.9	13.9	23.0	20.1
	240202	33	49.5' - 50.5'	gravelly lean clay with sand	21	CL	A-6(6)	100.0	86.2	74.4	69.2	65.1	61.1	51.1	32.1	11.7	20.4	13.2
	240203	34	59.5' - 60.5'	sandy lean clay	31	CL	A-6(7)	100.0	100.0	98.5	94.9	88.5	77.6	70.4	26.7	13.2	13.5	21.1
	240204	35	69.0' - 70.2'	lean clay with sand	100/0.7	CL	A-6(7)	100.0	100.0	99.6	97.7	94.4	84.6	80.0	27.5	16.7	10.8	15.0
SL 24-6							• /											
65	240205	36	13.0' - 14.5'	sandy lean clay with gravel	10	CL	A-4(3)	100.0	92.6	88.3	83.9	77.8	68.0	62.7	22.6	14.4	8.2	11.5
SL 24-7		(17)			10000													2000 C 1111
	240206	43	18.0' - 19.5'	lean clay	16	CL	A-6(14)	100.0	100.0	99.5	98.3	97.1	94.5	92.0	32.5	16.2	16.3	15.8
	240207	49	68.0' - 69.5'	lean clay	56	CL	A-6(21)	95.4	95.4	95.4	95.1	94.8	93.5	89.7	38.2	14.3	23.9	9.5
SL 24-8																		
	240208	52	24.0' - 25.5'	fat clay	13	CH	A-7-6(101) 100.0	100.0	100.0	98.9	98.0	96.5	95.0	117.2	25.0	92.2	34.2
	240209	55A	54.0' - 55.0'	silt with sand	98	ML	A-4(0)	100.0	100.0	100.0	100.0	99.6	98.1	71.2	NV	NP	NP	11.9
	240210	55B	55.0' - 55.5'	lean clay	NA	CL	A-4(4)	100.0	100.0	100.0	98.7	96.4	92.3	86.0	18.7	9.5	9.2	8.3
OS-01																		
01 04 14	240237	60	0.0'	sandy lean clay with gravel	NA	CL	A-7-6(9)	100.0	100.0	95.6	81.1	70.4	63.1	56.8	42.6	23.3	19.3	15.2
SL 24-11	040000 and 000	64 and 65	20.01 22.51	alovey gravel with cond	10	~~~	A (C(2))	70.6	70 E	CE A	62.0	EC C	40.0	40 C	20.6	15.4	17.0	0.0
	240238 and 239	04 and 00	32.0 - 33.5	clayey gravel with sand	10	GC	A-0(3)	12.5	72.5	69.1	02.U	50.0	49.0	40.0 20.5	32.0	10.4	20.0	9.0
	240240	00	42.0 - 43.5	ciayey graver with sand	30	SU	A-2-0(2)	30.0	19.1	00.1	38.2	30.8	43.0	30.5	32.2	12.2	20.0	0.1

Table A.1 WYDOT Geology Foundation Lab results for ND32401 select borehole samples

Form T-123

Rev. 9-11-01

WYOMING DEPARTMENT OF TRANSPORTATION Materials Program

REPORT OF TESTS ON ALKALI

Laboratory No's: As Listed Below Submitted by: Hammond Identification Marks: Source or Brand: Big Fill Silde Location: WY 22 Date : 16-Jul-24 At: FD (scan): W. Bybee

Date Sampled: 10-Jul-24 Date Rec'd: 12-Jul-24 Date Tested: 16-Jul-24 Project: ND32401

	FIELD NO.	LOCATION (Station and MilePost)	VERTICAL LIMITS	SOIL CHARACTERISTICS				CORROSION ANALYSIS		
LAB.								Material Type		
NO.					Minimum	Soluble	Sulfates,	METALLIC	N	ION-METALLIC
				pH	Resistivity	Salts	804	CR Clas	sification	Concrete
					(ohm-cm)	(%)	(%)	(See Rema	irks below)	Requirements
24-059		MP 12.8	0-2'	8.20	1300			CR1	CR1	Type II or 1L(MS) Cement

Remarks:

	APPLICATION										
		NEW Installatio	EXTEND Existing								
CR		S	tructural Plate (re								
Number	Metailic &		Bituminous	Decrease	Use	Perform	ALL Material Types				
1	Non-Metallic	No Modifications	Coating	Gage	Granular	Additional	(Including Structural Plate, etc.)				
			Both Sides	by 2	Backfill	Analysis					
1	Use acceptable	X									
2	material per		Х				If pipe or structure condition is				
3	WYDOT Standard		Х	X			SATISFACTORY, use existing				
4	Specifications		X	X			type, size, and backfill.				
5	and Concrete		Х	X	X		Otherwise, modify existing system.				
6	requirements					х					
7	as shown above					Х					

APPROVED BY: Ethan C

Ethan Crockett, P.E. Materials Staff Engineer TESTED BY: SN, SB, JPW

Table A.2 WYDOT Materials Lab result for ND32401 alkali sample


Tested By: DC

Table A.3 WYDOT Geology Foundation Lab results for ND32401 Direct Shear Test. Samples 12 and 13, from 24-02, were remolded for the test.

(Rev. 11-21)

AGGREGATE ANALYSIS

DEPARTMENT						TEST NUMBI	R: BF_Sample	1	
PROJECT NO(S) .: ND32	2401WP1			PROJECT NAM	PROJECT NAME: Teton Pass Slide - mm12.8				
ENGINEER: Bob -	lammond RE		TOWN: Wilson						
SAMPLE I.D.:	SAMPLED I	gy							
PIT OR QUARRY: Backf	ill Samples	from Sli	de Site			COUNT	Y: Teton		
QUANTITY: N/A						FOR USE .	s: Test Samp	le	
DATE RECEIVED: 8/7/2	2024					DATE TESTI	D: 8/7/2024		
	WE	IGHT	(lbs or k	:g)				Weight	% Retained =
	COARSI	E AGG.	E	FINE AG	G.			Retained (lbs or kg)	A or B D x 100
Sample	10078.8	= (E)	50	1.2	- (F)		Contraction of a state	24.00 70	25.0
After Wash			-	401.9		RETAINED N	$(A = \{4.75 \text{ mm}\} = (A)$	2602.70	Z5.8 = (H
Pass No. 200 [/5jum]			-	40.5		PASS	io, 4 [4.75 mm]= (B)	10077.70	/4.2 = (1
Tatal Pass No. 200 [75 jun], Fan			-	58 1			$\mathbf{JIAL}, \mathbf{A} + \mathbf{B} = (\mathbf{D})$	10077.70	
Fotar Pass No. 200 [75µm]				50.1			COMBR		
	WT DET	90 RET =	WT	90 RET =	90 RET		COMBIN	ED AGGREO	SALE
SIEVE	WI REI E		RET	<u>P x 100</u> F	<u>R x I</u> 100		% PA: 100 -	SSING S(Z)	SPEC
	=K	=L	=P	=R	=S	=Z	to 0.1 %	to 1%	% PASSING
1 1/2" [37.5 mm]							100.0	100	
1" [25mm]	115.40	1.1				1.1	98.9	99	
3/4" [19 mm]	98.70	1.0				1.0	97.9	98	
1/2" [12.5 mm]	374.90	3.7				3.7	94.2	94	
3/8" [9.5 mm]	215.40	2.1				2.1	92.1	92	
No. 4 [4.75 mm]	1798.30	17.8				17.8	74.3	74	
No. 8 [2.36 mm]			132.0	26.0	19.3	19.3	55.0	55	
No. 16 [1.18 mm]			115.0	22.7	16.8	16.8	38.2	38	
No. 30 [600 µm]			70.0	13.8	10.2	10.2	28.0	28	
No. 40 425 [µm]									
No. 50 [300 µm]			48.9	9.6	7.1	7.1	20.9	21	
No. 100 [150 µm]			45.1	8.9	6.6	6.6	14.3	14	
No. 200 [75 μm]			37.6	7.4	5.5	5.5	8.8	8.8	
Pass No. 200 [75 μm], Pan	7475.00	74.2	58.1	11.5	8.5				
TOTAL PASSING	#####	99.9	506.7	99.9					
SHAKER LOSS %	SHAKER LOSS % 0.0%		0.	1%			WET WT	(lb or kg)	22.3
FRACTURED FACES % One or more		SHAKE	R LOSS F	ORMULA		DRY WT	(b or kg)	22.2	
FLAT & ELONGATED %		([E or F]- T(DTAL PASSING	5) / [E or F] * 100		WET - DRY = MOISTURE			
FINENESS MODUL	US: see M.T.M., Sect		ct. 816.0:		%	% MOIST.=(MOIST./ DRY WT)x100			
BLOWS = Tin No.	Wet+Tare= AA	Dry + Tare= BB	Tare = CC	AA - BB = DD	Dry Wt = BB - CC = EE	(DD/EE)x 100	Corr. Factor	LL*Corr. Factor	PLASTIC INDEX (PI) =LL - PL
LIQUID LIMIT (LL)									
PLASTIC LIMIT (PL)									

REMARKS Deleterious Material in Sample. Bits of Aspalt were in this Sample.

TESTED BY Mark Schon

CERTIFICATION NO. 9864

Table A.4.a WYDOT Materials Lab results for ND32401 Fill Gradation Testing

(Rev. 11-21)

AGGREGATE ANALYSIS

PROJECT NORS: ND32401VP1 SAMPLE LD: TOTOR QUARKY: BochTII Somples from Slide Site OUNTY: Term OUNTY: TotA DATE RECEIVED: B/7/2024 SAMPLE DR: Term Slide Site OUNTY: Term SAMPLE DR: Term Slide Site OUNTY: Term OUNTY: Term SAMPLE DR: Term Slide Site OUNTY: Term SAMPLE DR: Term Slide Site OUNTY: Term SAMPLE DR: Term Slide Site SAMPLE DR: Term Slide Site <th cols<="" th=""><th>DEPARTMENT</th><th></th><th></th><th></th><th></th><th></th><th>TEST NUMBER</th><th>: BF Sample</th><th>2</th><th></th></th>	<th>DEPARTMENT</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>TEST NUMBER</th> <th>: BF Sample</th> <th>2</th> <th></th>	DEPARTMENT						TEST NUMBER	: BF Sample	2	
SAMPLE ID: PT OR QUARY: Boddfill Samples from Slide Site QUAVITY: IV/Λ DTR RECEIVED: 877/2024 TOWN: Wilson POR USE AS: Test Sample DOR USE AS: Test Sample POR USE AS: Test Sa	PROJECT NO(S) .: N	D32401WP1			PROJECT NAME: Teton Pass Slide - mm12.8						
SAMPLE D.D: SAMPLE D. BY: Todd W/WDOT Geology OUNTY: W/A DATE RECEIVED: W///2024 SAMPLE D. BY: Todd W/WDOT Geology COUNTY: Teton SAMPLE D. BY: Todd W/WDOT Geology SAMPLE D. BY: Todd W/WDOT Geology SAMPLE D. BY: Todd W/WDOT Geology OUNTY: W/A ON DECEMPTION SUGE STEE SAMPLE D. BY: Todd W/WDOT Geology SAMPLE D. BY: Todd W/WDOT Geology SAMPLE D. BY: Todd W/WDOT Geology OUNTY: Teton ON DECEMPTION SUGE STEE SAMPLE D. S: Teton TOTAL A: B. CO ON DECEMPTION SUGE STEED: BY//2024 SAMPLE D. S: Teton TOTAL A: B. CO OTAL A: B. CO OTAL A: B. CO SAMPLE D. S: Teton SAMPLE D.S: Teton TOTAL A: B. CO OTAL A: B. CO SAMPLE D.S: Teton SAMPLE D.S: Teton SAMPLE D.S: Teton <td>ENGINEER: BO</td> <td>ob Hammond RE</td> <td></td> <td colspan="5">TOWN: Wilson</td>	ENGINEER: BO	ob Hammond RE		TOWN: Wilson							
PIT OR QUARKY: BockHII Samples From Slide Site OUNTY: Teton COLUMY: SU/A DATE RECEIVED: WE I G HT (bx or kg) COLUMY: Teton Sample WE I G HT (bx or kg) POR USE AS: Test Sample Sample Sample Sample ADD Sample	SAMPLE I.D.:			SAMPLED BY: Todd w/ WYDOT Geology							
OLANTITI: N/A FOR USE AS: Test Sepile DATE RECEIVED: B/7/2024 DATE RECEIVED: B/7/2024 DATE RECEIVED: B/7/2024 COARSE AGG: FINE AGG. COARSE AGG: FINE AGG. COARSE AGG: FINE AGG. Second State AGG Second State AGG Second State AGG Second State AGG COARSE AGG. Second State AGG Second State AGG Pass No. 9(179m) Second State AGG COMENTE AGG Second State AGG Pass No. 9(179m) Second State AGG COMENTE AGG Second State AGG COMENTE AGG Second State AGG Second State AGG Second State AGG Second State AGG Second State AGG Second State AGG Second State AGG Second State AGG Second State AGG	PIT OR QUARRY: BO	ackfill Samples	from Sli	de Site			COUNTY	: Teton			
DATE RECEIVED: 8/7/2024 DATE RESIED: 8/7/2024 WE LG HT (bs or kg) we define a field of the second	QUANTITY: N	/A					FOR USE AS	8: Test Samp	le		
	DATE RECEIVED: 8/	/7/2024					DATE TESTEI	8/7/2024			
COARSE AGA: FINE AGA: A ar B 3 STATE AGAR: A 3 A ar B 3 STATE AGAR: A 3 A ar B 3 STATE AGAR: A 3 STATE A 3 <t< td=""><td></td><td>WE</td><td>IGHT</td><td>(lbs or k</td><td>g)</td><td></td><td></td><td></td><td>Weight</td><td>% Retained =</td></t<>		WE	IGHT	(lbs or k	g)				Weight	% Retained =	
Simple ByOLS -0.0 RETAINED No. 4 [4:75 mm] 39.7 -0.0 Pass No. 200 [75 µm], Pm -0.0 59.4 29.5 5395.00 60.3 -0.0 Pass No. 200 [75 µm], Pm -21.1 $707 LA. + B - 0.0$ 8949.40 60.3 -0.0 Pass No. 200 [75 µm], Pm -5 80.5 0.5	Gaund	COARSI	e AGG.	1	FINE AG	G.			(lbs or kg)	D x 100	
Alter Value VECTABLE / See (15 para) RETINEZ/See (15 para) 200 (75 para)	Sample	8903.9	= (E)	40	4075	- (F)	DETAILTE No.	4 14 76 mml - (4)	2554 40	20.7	
Name of the sum of th	Pass No. 200 [75um]				50 4		PASS No.	4 [4.75 mm] - (A)	5305.00	603 = a	
Total Pass No. 240 [75 µm] VICUUE VI	Pass No. 200 [75 um], Par				21.1		TASSIN	$\mathbf{AI} = \mathbf{A} + \mathbf{B} = (\mathbf{D})$	8949 40	00.5 - (x)	
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	Total Pass No. 200 [75um	1	-	-	80.5	- 1		AL, A · D · (D)	0212.10		
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$			04 BET -		04 DET -	06 BET		COMBIN	ED ACCRE	TATE	
SHEVE If ALL E KX 100 F RET $\frac{F}{F}$ $\frac{100}{100}$ $\frac{F}{F}$ $\frac{100}{100}$ $\frac{F}{F}$ $\frac{F}{F}$ $\frac{F}{F}$ $\frac{F}{F0}$ $\frac{F}{F0}$ $\frac{F}{F0}$ $\frac{F}{F0}$ $\frac{F}{F}$ $\frac{F}{F0}$ $\frac{F}{F}$ $\frac{F}{F0}$ $\frac{F}{F}$ $\frac{F}{F0}$ <	GUDUD	WT RET	%0 RE1 =	WT	90 KEI -	VO REI		T	ED AGGREQ	AIL	
K L -P R S 1.12" [37.5 mm] I I I I I III 1" [25mm] 235.70 2.6 I I III IIII IIIIII IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	SIZE	WIKEI	<u>K X 100</u> E	RET	<u>P x 100</u> F	<u>R x I</u> 100		% PA: 100 - 1	SSING S(Z)	SPEC	
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		=K	=L	= P	=R	=\$	=Z	to 0.1%	to 1%	76 FA35ENG	
1" [28m] 235.70 2.6	1 1/2" [37.5 mm]							100.0	100		
http://ite.stmm/microstrip.stmm/microttrip.stmm/microstrip.stmm/microstrip.stmm/microstrip.s	1" [25mm]	235.70	2.6	[2.6	97.4	97		
12^{12} (12.5 mm] 835.30 9.3 9.3 9.3 9.3 9.3 9.3 81.6 82 35^{12} (9.5 m) 568.90 6.4 7.0 7.0 7.0 7.0 7.0 14.9 6.0.3 6.0 - $No.414.75 mm$ 1332.0 14.9 11.2 2.4.1 14.5 14.5 45.8 46.0 -	3/4" [19 mm]	582.40	6.5				6.5	90.9	91		
\$38" 9.5 mm]568.906.49.9	1/2" [12.5 mm]	835.30	9.3				9.3	81.6	82		
No. 4 [4.75 m] 133.2.0 14.9 14.9 14.9 66.3 66.0 14.9 No. 5 [2.36 m] I 112.6 2.4.1 14.5 14.5 45.8 46.0 100.0 No. 5 [2.36 m] I I 85.9 18.4 11.1 34.7 35.0 100.0 No. 50 [60 µ] I I 59.9 12.7 7.7 27.0 27.0 27.0 100.0 No. 50 [60 µ] I I 50.9 10.9 66.0 66.6 20.4 20.0 100.0 No. 50 [60 µ] I I 50.9 10.9 50.5 14.9 15.0 10.0 1	3/8" [9.5 mm]	568.90	6.4				6.4	75.2	75		
No. 8 (2.3 6 m) Indextrement of the second of the sec	No. 4 [4.75 mm]	1332.10	14.9				14.9	60.3	60		
No. 16 [1.18 mm] Image: Sector S	No. 8 [2.36 mm]			112.6	24.1	14.5	14.5	45.8	46		
No. 30 (600 µm) Image: Signe state st	No. 16 [1.18 mm]			85.9	18.4	11.1	11.1	34.7	35		
No. 60 425 jum] Indextigation Indextigation <th <="" condextigation<="" td=""><td>No. 30 [600 µm]</td><td></td><td></td><td>59.4</td><td>12.7</td><td>7.7</td><td>7.7</td><td>27.0</td><td>27</td><td></td></th>	<td>No. 30 [600 µm]</td> <td></td> <td></td> <td>59.4</td> <td>12.7</td> <td>7.7</td> <td>7.7</td> <td>27.0</td> <td>27</td> <td></td>	No. 30 [600 µm]			59.4	12.7	7.7	7.7	27.0	27	
No. 50 (300 µm) Image: state of the	No. 40 425 [µm]										
No. 100 [150 µm] Image: state s	No. 50 [300 µm]			50.9	10.9	6.6	6.6	20.4	20		
No. 200 [75 µm] Image: style st	No. 100 [150 µm]			42.6	9.1	5.5	5.5	14.9	15		
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	No. 200 [75 µm]			35.0	7.5	4.5	4.5	10.4	10		
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Pass No. 200 [75 μm], Pan	5395.00	60.3	80.5	17.2	10.4					
SHAKER LOSS % 0.1% WET WT (b or kg) FRACTURED FACES % One or more SHAKER LOSS FORMULA (E or F)-TOTAL PASSING) / (E or F)*100 DRY WT (b or kg) DRY WT (b or kg) FLAT & ELONGATED % 1^{55} Ratio (E or F)-TOTAL PASSING) / (E or F)*100 $WET WT (b or kg)$ $WET WT (b or kg)$ FLAT & ELONGATED % 1^{55} Ratio (E or F)-TOTAL PASSING) / (E or F)*100 $WET WT (b or kg)$ $WET WT (b or kg)$ BLOWS = 1^{55} Rotio $UE or F)-TOTAL PASSING) / (E or F)*100$ $WET WT (b or kg)$ $WET WT (b or kg)$ BLOWS = 1^{55} Rotio $UE or F)-TOTAL PASSING) / (E or F)*100$ $WET WT (b or kg)$ $WET WT (b or kg)$ BLOWS = The $Mostare = C C C BBB$ Dry Wt = A A BE = B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = E B - C C = B -	TOTAL PASSING	8949.40	100.0	466.9	99.9			_			
FRACTURED FACES % One or more SHAKE LOSS FORMULA (E or F) TOTAL PASSING) / [E or F] * 100 DRY WT (b or kg) DRY WT (b or kg) FLAT & ELONGATED % 15 Ratio 15 Ratio $(E or F) - TOTAL PASSING) / [E or F] * 100 WET - DRY = MOISTURE WET - DRY = MOISTURE WET - DRY = MOISTURE BLOWS = TinNo. Mothtare BBB Dry + Tare BBB Tare = CC Mothtare BBC - CC = BC Dry Wt = BC - CC = BC (DD / E) \times 100 Corr. Factor LL^{\circ}Corr. Factor PLASTIC INDEX (P1) - L - PL LIQUID LIMIT (LL) Image: Corr Mathematic Mathemat$	SHAKER LOSS %	0.1	.%					WET WT	(lb or kg)		
IS Ratio IS Ratio WET - DRY = MOISTURE FINENESS Wet+Tarre Dry + Tarre Tarre Moisture Dry Wt= BLOWS = Tin Wet+Tarre Dry + Tarre Tarre Moisture Dry Wt= BLOUTD LIMIT (LL) C Icon Icon Icon Dry Dry Dry Dry BB Dry Wt= PLASTIC INDEX Moisture Dry Wt= Dry Wt= Dry Wt= Dry Wt= Dry Wt= Dry Wt= PLASTIC INDEX Dry C Icon Con LL*Cerr. Factor PLASTIC INDEX PLASTIC LIMIT (PL) C Icon Icon Icon Icon Icon	FRACTURED FACES	FRACTURED FACES % One or more		SHAKE	R LOSS F	ORMULA		DRY WT	(lb or kg)		
Image: Construct of the	FLAT & ELONGATED % 1:5 Ratio		([E or F] - TC	TAL PASSING)/[E or F] * 100		WET - D	RY = MOISTURE			
BLOWS = Tin WetTares Dry + Tares Moistures Dry Wt = Moistures Dry Wt = MOISTURE PLASTIC INDEX LIQUID LIMIT (LL) Image: Second se	FINENESS MO	DULUS: see M.1	US: see M.T.M., Sect. 816.0:				% M	DIST.=(MOIST./ DRY	7 WT)x100		
LIQUID LIMIT (LL) PLASTIC LIMIT (PL)	BLOWS =	Tin Wet+Tare= No. AA	Dry + Tare= BB	Tare = CC	Moisture = AA - BB = DD	Dry Wt = BB - CC = EE	% N (DD/EE) x 100	Corr. Factor	LL*Corr. Factor	PLASTIC INDEX (PI) =LL - PL	
PLASTIC LIMIT (PL)	LIQUID LIMIT (LL)										
	PLASTIC LIMIT (PL)										

REMARKS_Deleterious Material in Sample.

TESTED BY Mark Schon

CERTIFICATION NO. 9864

Table A.4.b WYDOT Materials Lab results for ND32401 Fill Gradation Testing

(Rev. 11-21)

AGGREGATE ANALYSIS

DEPARTMENT						TEST NUMBER	: BF_Sample	3		
PROJECT NO(S).: ND32	401WP1			PROJECT NAME: Teton Pass Slide - mm12.8						
ENGINEER: BOD H	ammond RE			TOWN: Wilson						
SAMPLE LD.:			SAMPLED BY: Todd w/ WYDOT Geology							
PIT OR QUARRY: Backf	ill Samples	from Sli	de Site			COUNTY	: Teton			
QUANTITY: N/A						FOR USE AS	: Test Samp	le		
DATE RECEIVED: 8/7/2	024					DATE TESTED	: 8/7/2024			
	WE	IGHT	(lbs or k	ug)				Weight	% Retained =	
	COARSE	AGG.		FINE AG	G.			Retained	A or B	
Sample	10650.4	= (E)	38	30.1	= (F)			(lbs or kg)	$\int D \int x 100$	
After Wash			- -	356		RETAINED No.	4 [4.75 mm] = (A)	2783.40	26.2 = (H)	
Pass No. 200 [75µm]		(24.1		PASS No.	4 [4.75 mm]= (B)	7860.00	73.8 = (1)	
Pass No. 200 [75 µm], Pan				14.2		тот	AL, $A + B = (D)$	10643.40		
Total Pass No. 200 [75µm]				38.3						
		% RET =	NUT	% RET =	% RET		COMBIN	ED AGGREC	SATE	
SIEVE	WT RET	<u>K X 100</u> E	RET	<u>P x 100</u> F	<u>R x I</u> 100		% PAS 100 - 5	SSING S(Z)	SPEC	
	=K	=L	=P	=R	=S	=Z	to 0.1 %	to 1%	% PASSING	
1 1/2" [37.5 mm]	1						100.0	100		
1" [25mm]	17.50	0.2				0.2	99.8	100		
3/4" [19 mm]	344.30	3.2				3.2	96.6	97		
1/2" [12.5 mm]	512.30	4.8				4.8	91.8	92		
3/8" [9.5 mm]	324.60	3.0				3.0	88.8	89		
No. 4 [4.75 mm]	1584.70	14.9				14.9	73.9	74		
No. 8 [2.36 mm]			86.4	22.7	16.8	16.8	57.1	57		
No. 16 [1.18 mm]			85.8	22.6	16.7	16.7	40.4	40		
No. 30 [600 µm]			63.0	16.6	12.3	12.3	28.1	28		
No. 40 425 [µm]										
No. 50 [300 µm]			39.8	10.5	7.8	7.8	20.3	20		
No. 100 [150 μm]			33.9	8.9	6.6	6.6	13.7	14		
No. 200 [75 µm]			32.9	8.7	6.4	6.4	7.3	7.3		
Pass No. 200 [75 μm], Pan	7860.00	73.8	38.3	10.1	7.5					
TOTAL PASSING	#####	99.9	380.1	100.1			-			
SHAKER LOSS %	SHAKER LOSS % 0,1%			DIOCOR	oman		WET WT	(lb or kg)		
FRACTURED FACES %	FRACTURED FACES % One or more			R LOSS F	ORMULA		DRYWT	(Ib or kg)		
FINENESS MODUL	US: con M T	M Sect	816 D	AL PASSING	97 [E of 1] , 100		WEI - D	WTw100		
DI ONTE I		JS: see M.I.M., Sect		. 816.0:			OISTUDE			
BLOWS = Tin No.	Wet+Tare= AA	Dry + Tare= BB	Tare = CC	AA - BB = DD	Dry Wt = BB - CC = EE	(DD/EE) x 100	Corr. Factor	LL*Corr. Factor	PLASTIC INDEX (PI) =LL - PL	
LIQUID LIMIT (LL)			-		· · · · · ·					
PLASTIC LIMIT (PL)										

REMARKS_Deleterious Material in Sample.

TESTED BY Mark Schon

CERTIFICATION NO. 9864

Table A.4.c WYDOT Materials Lab results for ND32401 Fill Gradation Testing

(Rev. 11-21)

AGGREGATE ANALYSIS

DEPARTMENT						TEST NUMBE	R: BF_Sample	4		
PROJECT NO(S) .: ND	32401WP1			PROJECT NAME: Teton Pass Slide - mm12.8						
ENGINEER: Bot	Hammond RE		TOWN: Wilson							
SAMPLE LD.:			SAMPLED B	gy						
PIT OR QUARRY: Bac	kfill Samples	from Sli	de Site			COUNT	Y: Teton			
QUANTITY: N/	A				5	FOR USE A	s: Test Samp	le		
DATE RECEIVED: 8/7	7/2024				2. 	DATE TESTE	D: 8/7/2024			
	WE	IGHT	(lbs or k	g)				Weight	% Retained =	
	COARSI	E AGG.		FINE AG	G.			Retained (the or ba)	A or B x 100	
Sample	11847.8	= (E)	41	8.9	- (F)		and the strength	(103 01 Kg)		
After Wash	_			301.2		RETAINED No	. 4 [4.75 mm] = (A)	4574.30	38.6 = (H	
Pass No. 200 [75µm]				117.7		PASS N	o, 4 [4.75 mm]= (B)	7268.00	<u>61.4</u> = (I	
Pass No. 200 [75 µm], Pan			1	45		то	$\mathbf{TAL} \ , \ \mathbf{A} + \mathbf{B} = (\mathbf{D})$	11842.30		
Total Pass No. 200 [75µm]				162.7						
		96 RET =	WT	% RET =	% RET		COMBIN	ED AGGREO	GATE	
SIEVE SIZE	WT RET	<u>KX 100</u> E	RET	<u>P x 100</u> F	<u>R x I</u> 100		% PA: 100 -	SSING S(Z)	SPEC	
	=K	=L	=P	=R	=S	=Z	to 0.1 %	to 1%	% PASSING	
1 1/2" [37.5 mm]	133.90	1.1				1.1	98.9	99		
1" [25mm]	223.60	1.9				1.9	97.0	97		
3/4" [19 mm]	471.90	4.0			_	4.0	93.0	93		
1/2" [12.5 mm]	1172.10	9.9				9.9	83.1	83		
3/8" [9.5 mm]	673.50	5.7				5.7	77.4	77		
No. 4 [4.75 mm]	1899.30	16.0		2 2		16.0	61.4	61		
No. 8 [2.36 mm]			36.3	8.7	5.3	5.3	56.1	56		
No. 16 [1.18 mm]			50.2	12.0	7.4	7.4	48.7	49		
No. 30 [600 µm]			50.9	12.2	7.5	7.5	41.2	41		
No. 40 425 [µm]										
No. 50 [300 µm]			46.5	11.1	6.8	6.8	34.4	34		
No. 100 [150 µm]			36.0	8.6	5.3	5.3	29.1	29		
No. 200 [75 µm]			35.5	8.5	5.2	5.2	23.9	24		
Pass No. 200 [75 μm], Pan	7268.00	61.3	162.7	38.8	23.8					
TOTAL PASSING	#####	99.9	418.1	99.9						
SHAKER LOSS %	SHAKER LOSS % 0.0%		0.	2%			WET WT	(lb or kg)		
FRACTURED FACES %	1:5 Ratio		SHAKE	R LOSS F	ORMULA		DRY WT	(lb or kg)		
FLAT & ELONGATED %	6 1.5 Kano		([E or F] - T(OTAL PASSING	5) / [E or F] * 100	-	WET - D	RY = MOISTURE		
□ FINENESS MOD	ULUS: see M.1	.M., Sect	. 816.0:			% M	OIST.=(MOIST./ DRY	/ WT)x100		
BLOWS = T	in Wet+Tare= 0. AA	Dry + Tare= BB	Tare = CC	AA - BB = DD	Dry Wt = BB - CC = EE	(DD/EE) x 100	Corr. Factor	LL*Corr. Factor	PLASTIC INDEX (PI) =LL - PL	
LIQUID LIMIT (LL)			-				Contract With			
PLASTIC LIMIT (PL)										

REMARKS_Deleterious Material in Sample.

TESTED BY Mark Schon

CERTIFICATION NO. 9864

Table A.4.d WYDOT Materials Lab results for ND32401 Fill Gradation Testing

(Rev. 11-21)

AGGREGATE ANALYSIS

DEPARTMENT						TEST N	JMBER	: BF_Sample	5		
PROJECT NO(S) .: ND3	PROJECT	PROJECT NAME: Teton Pass Slide - mm12.8									
ENGINEER: Bob Hammond RE							TOWN: Wilson				
SAMPLE LD.:							SAMPLED BY: Todd w/ WYDOT Geology				
PIT OR QUARRY: Back	fill Samples	from Sli	de Site			C	OUNTY	: Teton			
QUANTITY: N/A					8	FOR	USE AS	: Test Samp	e		
DATE RECEIVED: 8/7/	2024					DATE T	ESTED	: 8/7/2024			
	WE	IGHT	(lbs or k	g)					Weight	% Retained =	
Eamala	COARSI	é AGG.	53	FINE AG	G.				(lbs or kg)	D x 100	
After Wash	97 34.1	- (E)	5 52	395.6	- (r)	DETAIN	JED No. 4	[4.75 mm] = (A)	3000 30	A11 = (H	
Pass No. 200 [75µm]	-	1	-	128		KLIAL	ASS No.	4 [4.75 mm] = (R)	5736.40	58.9 = 0	
Pass No. 200 [75 µm], Pan	1			43.3			тот	$\mathbf{AL} , \mathbf{A} + \mathbf{B} = (\mathbf{D})$	9735.70		
Total Pass No. 200 [75µm]				171.3			Constant of the				
		% RET =	1000000	% RET =	% RET			COMBINI	ED AGGREC	ATE	
SIEVE SIZE	WT RET	<u>K X 100</u> E	WT RET	<u>P x 100</u> F	<u>R x I</u> 100	ſ		% PAS 100 - S	SSING S(Z)	SPEC	
	= <u>K</u>	=L	=P	=R	=S		=Z	to 0.1 %	to 1%	% PASSING	
1 1/2" [37.5 mm]	110.20	1.1					1.1	98.9	99		
1" [25mm]	281.40	2.9					2.9	96.0	96		
3/4" [19 mm]	459.80	4.7					4.7	91.3	91		
1/2" [12.5 mm]	1023.80	10.5					10.5	80.8	81		
3/8" [9.5 mm]	563.80	5.8					5.8	75.0	75		
No. 4 [4.75 mm]	1560.30	16.0					16.0	59.0	59		
No. 8 [2.36 mm]			89.2	17.0	10.0		10.0	49.0	49		
No. 16 [1.18 mm]			82.4	15.7	9.3		9.3	39.7	40		
No. 30 [600 µm]			54.8	10.5	6.2		6.2	33.5	34		
No. 40 425 [µm]											
No. 50 [300 µm]			38.1	7.3	4.3		4.3	29.2	29		
No. 100 [150 μm]			41.6	7.9	4.7		4.7	24.5	25		
No. 200 [75 µm]			46.2	8.8	5.2		5.2	19.3	19		
Pass No. 200 [75 µm], Pan	5736.40	58.9	171.3	32.7	19.3						
TOTAL PASSING	9735.70	99.9	523.6	99.9				-			
SHAKER LOSS %	0.0)%						WET WT	(lb or kg)		
FRACTURED FACES %	FRACTURED FACES % One or more			R LOSS F	ORMULA			DRY WT	(borkg)		
FLAT & ELONGATED %			([E or F] - TC	OTAL PASSING)/[E or F] * 100		53-52.24	WET - D	RY = MOISTURE		
└ FINENESS MODU	LUS: see M.1	US: see M.T.M., Sect. 816.0:					% MO	IST.=(MOIST./ DRY	WT)x100		
BLOWS = Tin No.	Wet+Tare= AA	Dry + Tare= BB	Tare = CC	Moisture = AA - BB = DD	Dry Wt = BB - CC = EE	(DD/EE)	% M	OISTURE Corr. Factor	LL*Corr. Factor	PLASTIC INDEX (PI)=LL - PL	
LIQUID LIMIT (LL)											
PLASTIC LIMIT (PL)											

REMARKS Deleterious Material in Sample.

TESTED BY Mark Schon

CERTIFICATION NO. 9864

Table A.4.e WYDOT Materials Lab results for ND32401 Fill Gradation Testing

APPENDIX C – MEMORANDUM B249066





5300 Bishop Boulevard, Cheyenne, Wyoming 82009-3340

October 20, 2023



Darin J. Westby, P.E. Director

$\underline{\mathsf{M}} \underline{\mathsf{E}} \underline{\mathsf{M}} \underline{\mathsf{O}} \underline{\mathsf{R}} \underline{\mathsf{A}} \underline{\mathsf{N}} \underline{\mathsf{D}} \underline{\mathsf{U}} \underline{\mathsf{M}}$

TO:	John Eddins, P.E., District Engineer, Rock Springs
FROM:	Todd Sullivan, P.G., Engineering Geologist, Cheyenne
SUBJECT:	Inclinometer Installation, 'The Big Fill', WYO 22, MP 12.8
PROJECT:	B249066, Wilson-Idaho State Line, Teton County



An inclinometer was installed at the above-referenced location August 2-3, 2023, utilizing a contract portable drill operated by Salisbury & Associates from Spokane, Washington. The drill was set a few feet over the guard rail on sloped terrain (Figure 1) with a truck-mounted crane. The inclinometer location is shown on the Site Location Map (Figure 2). The inclinometer will be used to monitor slide movement or embankment instability in 'The Big Fill' and/or the underlying bedrock, as pavement distress and possible 'slumping' has been occurring at this location since 2004. Bedrock was encountered at a depth of eighty-five feet, and 2.75-inch SI casing was installed to a depth of 115 feet. Groundwater was not encountered. Water was used as the circulation medium during drilling, which rendered poor core recovery due to the flushing or 'washing out' of the fines, leaving gravel and cobble-sized fragments for recovery. Water was supplied by a WYDOT water truck, which was filled up at the Public Works Department in Victor, Idaho.



Figure 1: Drill location on embankment slope past the guard rail



Figure 2: Site Location Map

John Eddins B249066 Page 3

DISCUSSION

<u>TBF-01 (IN 23-01)</u>: Materials consisted of 6.5 feet of loose, gravelly sand over 3.5 feet of dense, boulders and cobbles, overlying 75 feet of medium dense, sandy gravel with cobbles and random boulders. Hard, fine-grained Upper Cretaceous sandstone bedrock with high-angle fractures was encountered at 85 feet. There were mineralized deposits on many of the fracture faces and there was a clayey zone just above the bedrock contact from 84.5 to 84.8 feet. The embankment is situated between two east-west trending faults; the Jackson Thrust Fault to the south and the Cache Creek Thrust Fault to the north.

Inclinometer casing was installed to a depth of 115 feet with 1.8 feet of stick-up and an A₀ direction of 30 degrees. Approximately 80 gallons of grout were used to fill the annular space, using six 47.5 pound bags of Portland cement and one 47.5 pound bag of quick gel. The inclinometer was initialized on August 24, 2023 and no movement was detected as of October 9, 2023 during the first sequential reading, shown on the inclinometer plot in Figure 3. The inclinometer will be monitored on a six month schedule to identify slide planes or displacement which shows up as deviations from the straight line plots. New pavement was placed in the area prior to the October reading, as the pronounced dip and pavement distress had been repaired and there was no additional distress observed. Heavy snow normally gets cast over the guardrail onto the slope each winter season. Maintenance personnel should be aware of the presence of the inclinometer and try to avoid damaging it during any maintenance activities.

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Figure 3: The Big Fill Inclinometer Plot

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Reviewed By: _ David 1 Jeen Principal Geologist

Approved By: m Chief Engineering Geologist

Attachments: None

cc: Peter Stinchcomb, P.E., District Construction Engineer, Rock Springs Tory Thomas, P.E., District Maintenance Engineer, Rock Springs Bob Hammond, P.E., Resident Engineer, Jackson Michael Jerup, Maintenance Supervisor, Jackson Geology (3)

APPENDIX D – RJ ENGINEERING & CONSULTING REPORT



June 28, 2024,

Project No. 24-014B-C1

Mr. James Dahill, PG WYDOT-Geology Program 5300 Bishop Blvd. Cheyenne, WY. 82009-3340

Subject: State Highway 22 Teton Pass Big Fill Detour - Global Stability Evaluation.

Dear Mr. Dahill,

RJ Engineering & Consulting, Inc. (RJ) was requested to provide an evaluation of the global stability of the detour constructed to open the corridor to the traveling public between Victor, Idaho and Jackson, Wyoming. The detour was constructed as an emergency measure after the existing embankment failed.

DISCUSSION / OBSERVATIONS

A bypass was constructed inside of the Big Fill Failure. WYDOT requested RJ provide an evaluation and analysis of the global stability of the detour roadway template.

RJ personnel visited the site on June 14, 24, and 25, 2024. Various observations regarding the slide were conducted for an evaluation process. Based on our

observations and requests from WYDOT for the slope evaluation, we requested the following:

- Use of the services of Chinook Landscape Architecture which uses drone mapping for detailed tools for use in site analysis.
- Additional inclinometers to be placed between the detour and the failed slope. The inclinometers should utilize a shape array system that can provide a subsurface movement warning system.
- A piezometer be placed near the inclinometer to monitor ground water levels within the embankment.
- Use of a radar slope scanning system to monitor slope movements of the embankment.
- An additional boring at the base of the existing head-scarp.

SITE ASSUPTIONS

Due to the emergency nature of the work, RJ is working with WYDOT in a teamwork fashion and has developed and reviewed various cross sections and subsurface conditions throughout the detour construction process.

PO Box 1080, Silt, CO 81652, (970) 230-9208 3082 Evergreen Parkway, Suite E, Evergreen, CO 80401

RJ has made the following general assumptions regarding the site and stability analysis:

- Soil parameters were mutually agreed upon with the design team for the stability analysis.
- Elevated surface and ground water was a significant contributor to the failure of the embankment.
- The embankment was likely nearly saturated in the lower half just prior to failure.
- The embankment materials exposed consist of blocks (cobble and boulder sized material) in a fine-grained matrix.
- The embankment soils have significant cohesion. The current head scarp slope faces are standing at 60 degrees or steeper in places.
- Groundwater levels within the current embankment have dropped but are still present.

STABILITY ANALYSIS

RJ has evaluated two cross sections:

- Alignment along the general slide path to the northwest.
- Approximately perpendicular to the roadway alignment to the northeast.

Figure 1 depicts the location of the two (2) cross sections evaluated.



Figure 1. Plan View of Cross Sections Evaluated.

NW Cross Section



Pre-Failure Stability Analysis Model

Post Failure Stability Analysis Model



NE Cross Section





Present Stability Analysis Model – Extended Limits



Based on our understanding of the project, data provided to us, and the current estimated groundwater levels, we offer the following:

- RJ has provided what we consider are the two critical analyzed sections.
- The detour template is at a Factor of Safety (FS) of greater than 1.20 for the current slope and water conditions.
- Head-scarp slopes faces may be at a FS of less than 1.20, but the FS between the slope face and the current detour roadway is greater than 1.20.
- Monitoring systems such as shape arrays, piezometers, and radar slope scanning are suggested to be installed and will be utilized to refine stability models. The monitoring systems will provide a layered early warning system in the event the slope moves, so action can be taken to reassess the slope and groundwater conditions.

LIMITATIONS

This study has been conducted in accordance with generally accepted geological and geotechnical engineering practices in this area for use by the client. The suggestions submitted in this report are based upon the data obtained from the ongoing evaluation of the site.

Geologic conditions and ground water levels will also change over time, so the results of the analyses and recommendations will also change over time and may need to be reassessed.

The report was prepared in substantial accordance with the generally accepted standards of practice for geotechnical engineering as exist in the site area at the time of our investigation. No warranties, express or implied, are intended or made.

Sincerely,



Ben Arndt, PE Principal

Reviewed by:

Rich > flo

Richard D. Johnson, PE Principal